

C:\Users\lesical\Documents\Paragon Star_Bldg2\Log_Rt21-Central_Ireedault\p@finklevilliams.com.rvt
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FIRE PROTECTION
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FINKLE + WILLIAMS
ARCHITECTURE

PROJECT ADDRESS
3201 NW PARAGON
PKWY
LEE'S SUMMIT, MO

PROJECT NUMBER
19050.01a

RELEASE DATE
06.02.22

ISSUED FOR
ADDENDUM 1

PARAGON STAR BLDG 2 / LOT 9

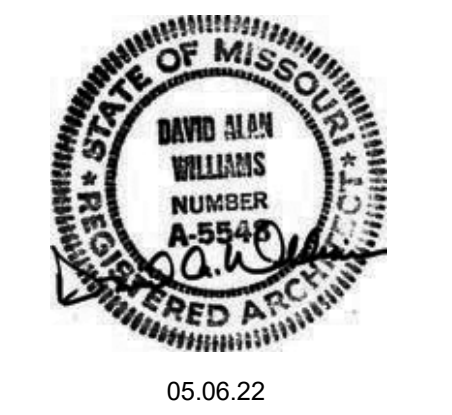


3201 NW PARAGON PKWY
LEE'S SUMMIT, MO

Project No.:	19050.01a
Date:	06.02.22
Issued For:	ADDENDUM 1

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REGISTRATION



PROJECT TEAM

ARCHITECT FINKLE+WILLIAMS
ARCHITECTURE

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Model 2	0.0000
Model 3	0.0000
Model 4	0.0000
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Model 100	0.0000

LANDSCAPE LAND 3

FOUNDATIONS BSE STRUCTURAL
ENGINEERS

STRUCTURAL BSE STRUCTURAL
ENGINEERS

UMBING HENDERSON
ENGINEERS

MECHANICAL HENDERSON
ENGINEERS

ELECTRICAL HENDERSON
ENGINEERS

RE PROTECTION HENDERSON
ENGINEERS

CONTRACTOR FOGEL-ANDERSON

INKLE + WILLIAMS
ARCHITECTURE

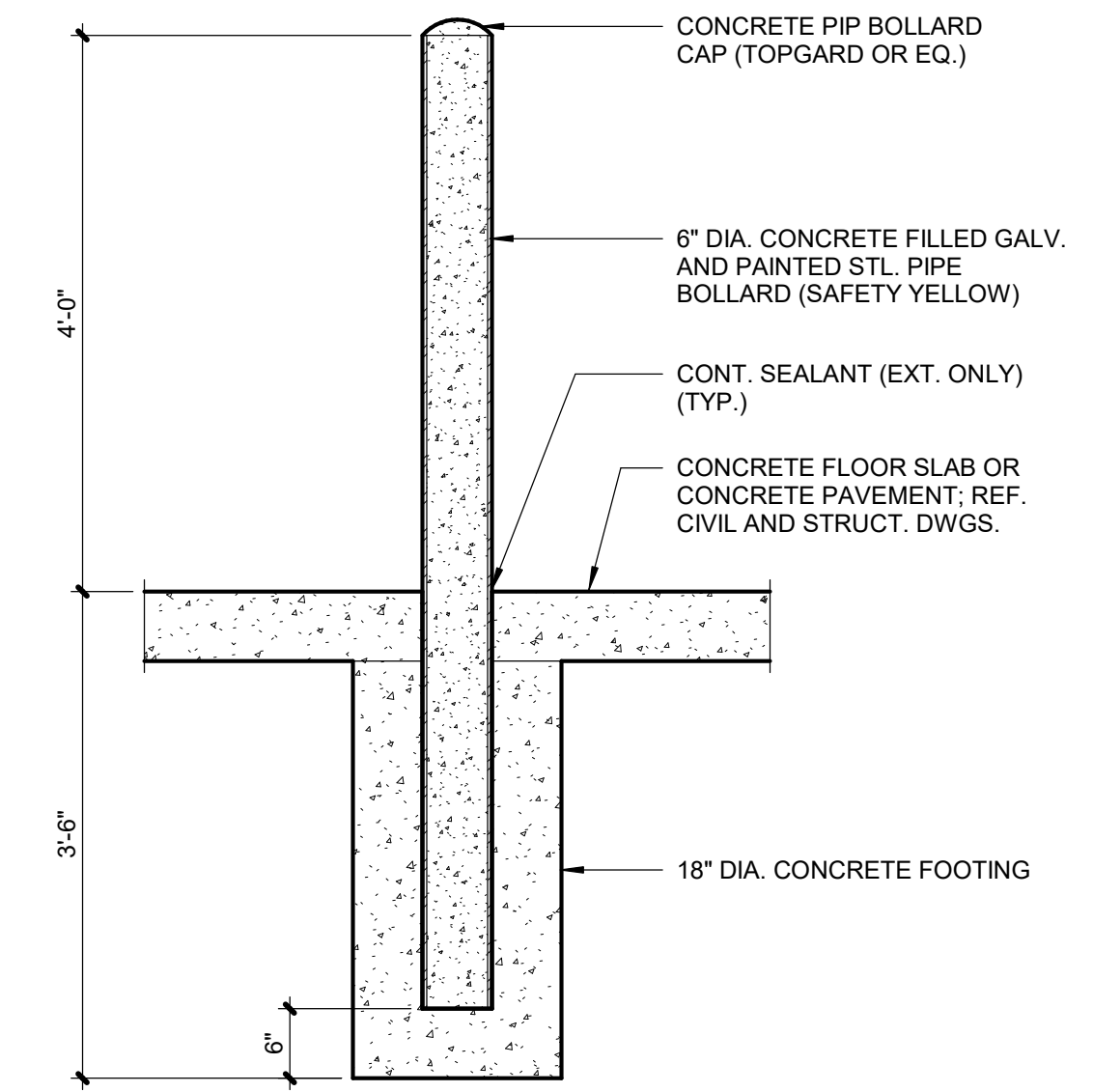
3787 RENNER BLVD., SUITE 100
LENEXA, KANSAS 66219
913.498.1550
www.finklewilliams.com

SHEET TITLE

ARCHITECTURAL SITE PLAN

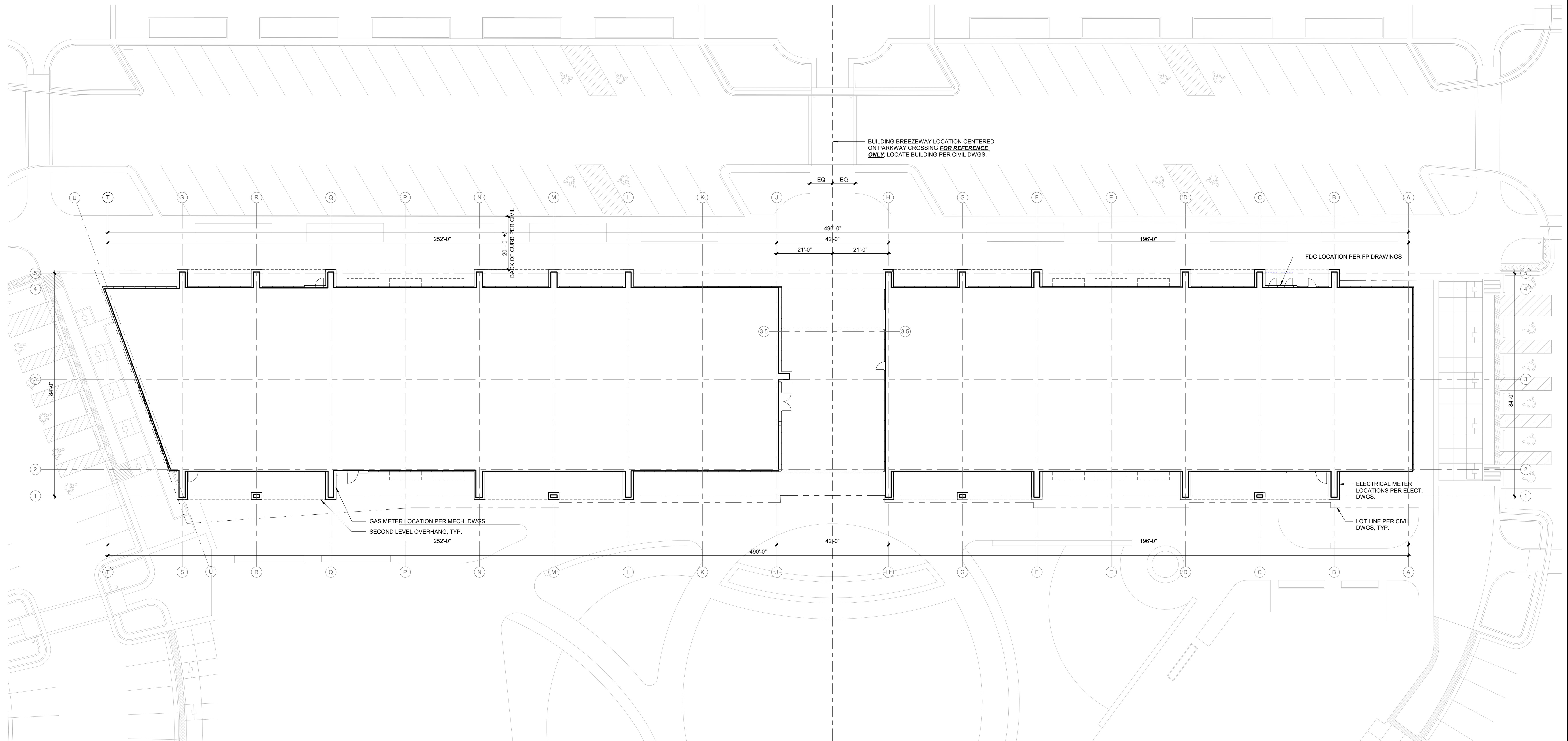
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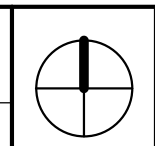
2	BOLLARD DETAIL
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A0.03	SCALE : 3/4" = 1'-0"
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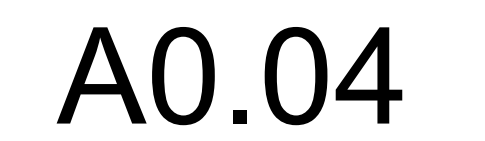


1 | ARCHITECTURAL SITE PLAN

A0.03	SCALE : 1/16" = 1'-0"
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C:\Users\jessica\Documents\Paragon Star_Bldg2Lot9_R21-Central_jreedshultz@finklewilliams.com.rvt



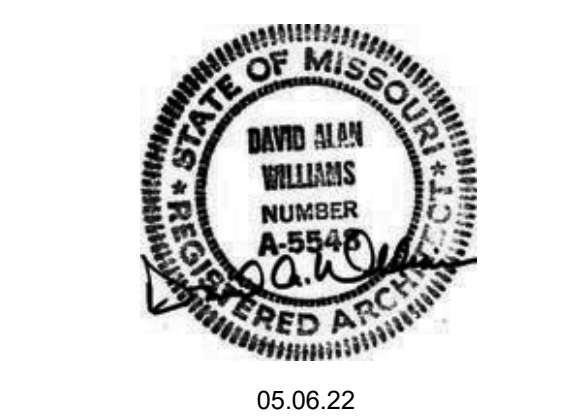
ARCHITECTURAL SLAB EDGE PLANS ARE FOR REFERENCE AND COORDINATION ONLY; PLEASE REF STRUCTURAL AND FOUNDATION DRAWINGS AND DISCUSS DISCREPANCIES WITH ARCHITECT.





REVISIONS		
No.	Date	Description

REGISTRATION



PROJECT TEAM

ARCHITECT	FINKLE+WILLIAMS ARCHITECTURE
CIVIL	GBA
LANDSCAPE	LAND 3
FOUNDATIONS	BSE STRUCTURAL ENGINEERS
STRUCTURAL	BSE STRUCTURAL ENGINEERS
PLUMBING	HENDERSON ENGINEERS
MECHANICAL	HENDERSON ENGINEERS
ELECTRICAL	HENDERSON ENGINEERS
FIRE PROTECTION	HENDERSON ENGINEERS
CONTRACTOR	FOGEL-ANDERSON



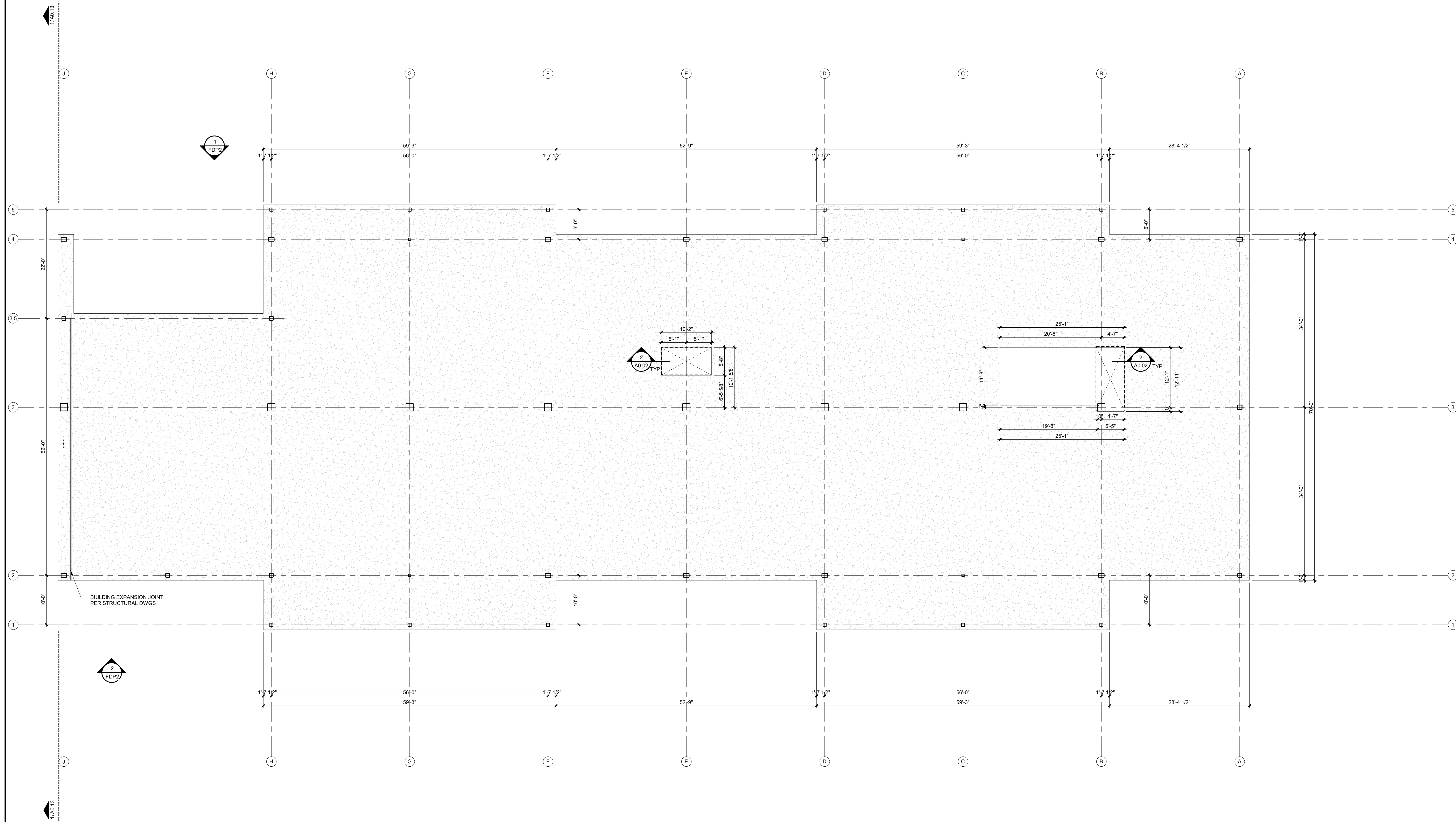
8787 RENNER BLVD., SUITE 100
LENEXA, KANSAS 66219
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www.finklewilliams.com

SHEET TITLE

SLAB EDGE
PLAN - 2ND
FLOOR EAST

SHEET NUMBER

A0.14



1. PROVIDE TERMITE CONTROL UNDER NEW FLOOR SLABS.
2. ALL STRUCTURAL STEEL TO BE FACTORY PRIMED GRAY.
3. ALL CMU CORNERS ARE TO BE CONSTRUCTED OF BULLNOSE BLOCK.
4. ALL SWITCHES, RECEPTACLES, PHONE/DATA, AND CONTROLS ARE TO BE GRAY COLOR WITH STAINLESS STEEL COVER PLATES.



Project No.:	19050.01a
Date:	06.02.22
Issued For:	ADDENDUM 1

[illegible]

PROJECT TEAM

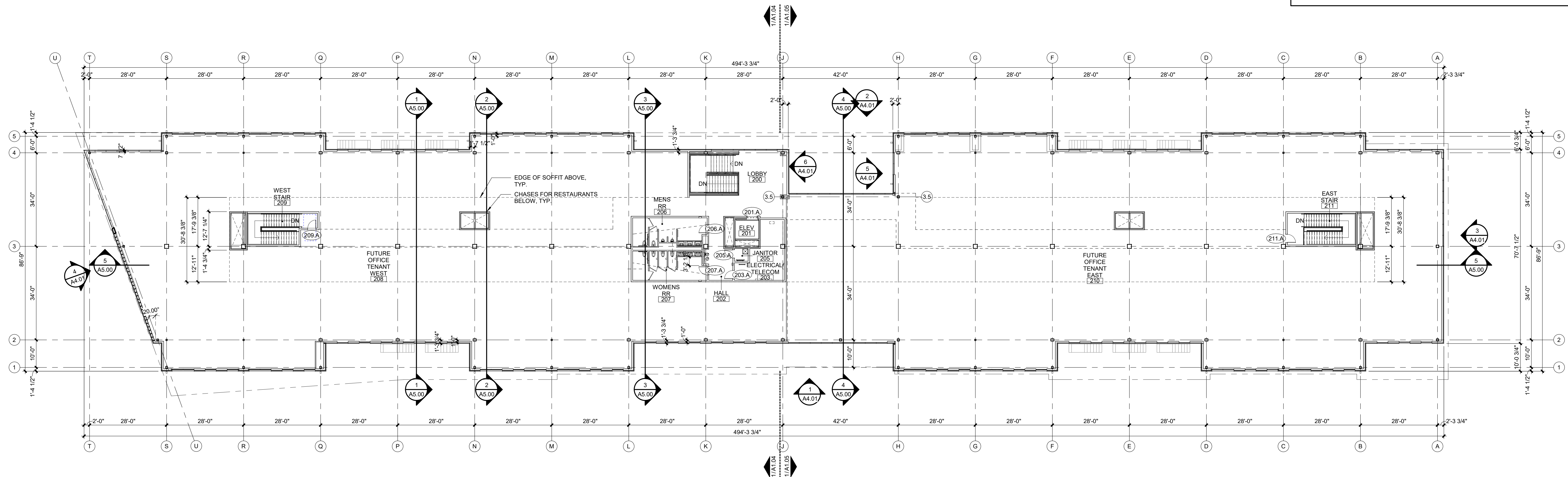
CONTRACTOR FUGEL-ANDERSON



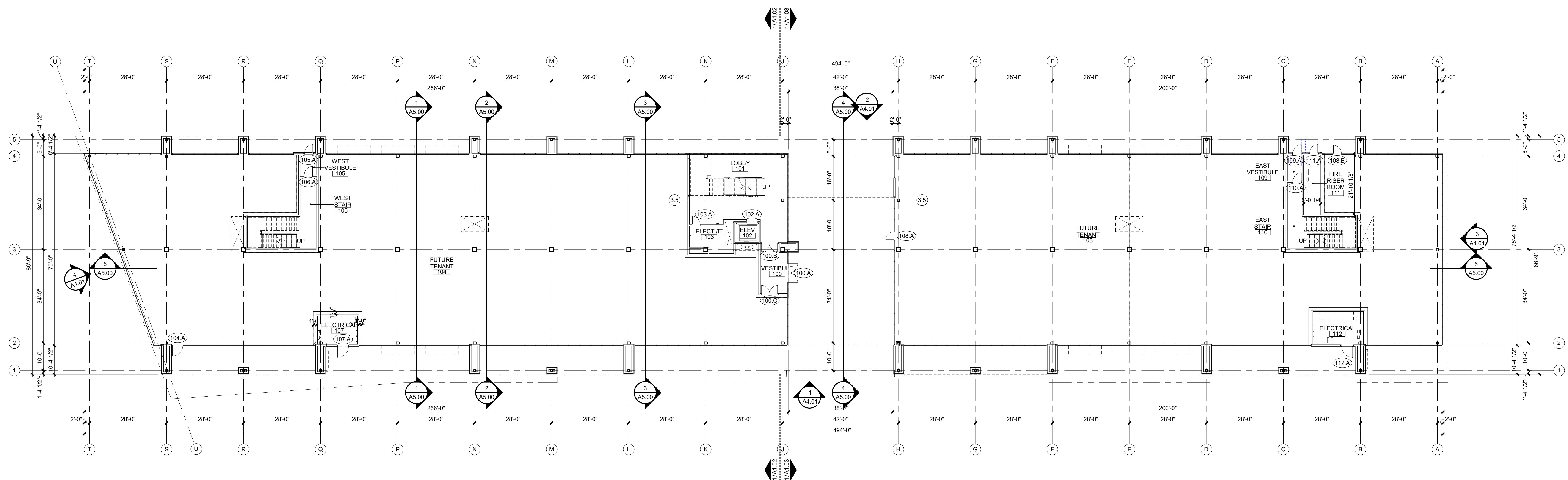
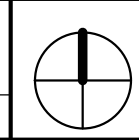
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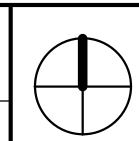
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A1.01	SCALE : 1/16" = 1'-0"
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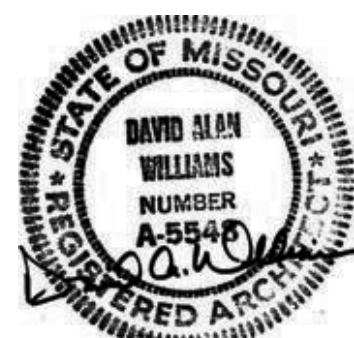






REVISIONS		
No.	Date	Description

REGISTRATION



05.06.22

PROJECT TEAM

ARCHITECT	FINKLE+WILLIAMS ARCHITECTURE
CIVIL	GBA
LANDSCAPE	LAND 3
FOUNDATIONS	BSE STRUCTURAL ENGINEERS
STRUCTURAL	BSE STRUCTURAL ENGINEERS
PLUMBING	HENDERSON ENGINEERS
MECHANICAL	HENDERSON ENGINEERS
ELECTRICAL	HENDERSON ENGINEERS
FIRE PROTECTION	HENDERSON ENGINEERS
CONTRACTOR	FOGEL-ANDERSON



FINKLE + WILLIAMS
ARCHITECTURE

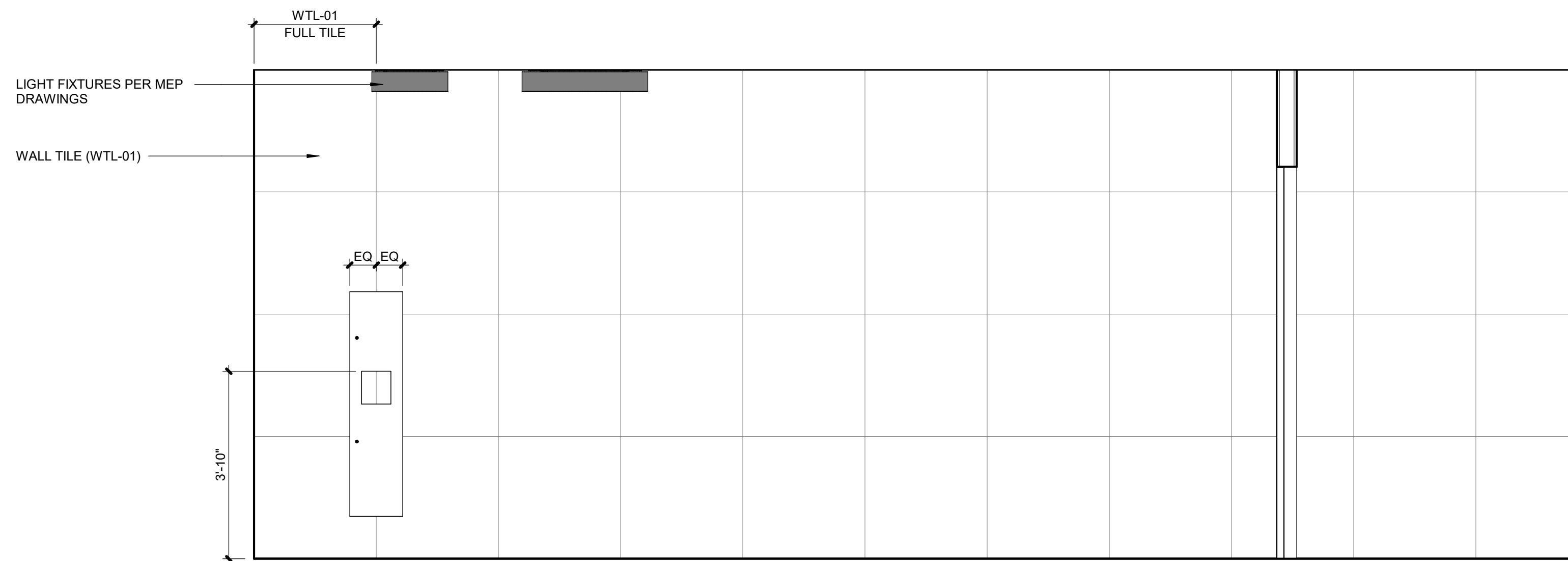
8787 RENNER BLVD., SUITE 100
LENEXA, KANSAS 66219
913.498.1550
www.finklewilliams.com

SHEET TITLE

INTERIOR
ELEVATIONS

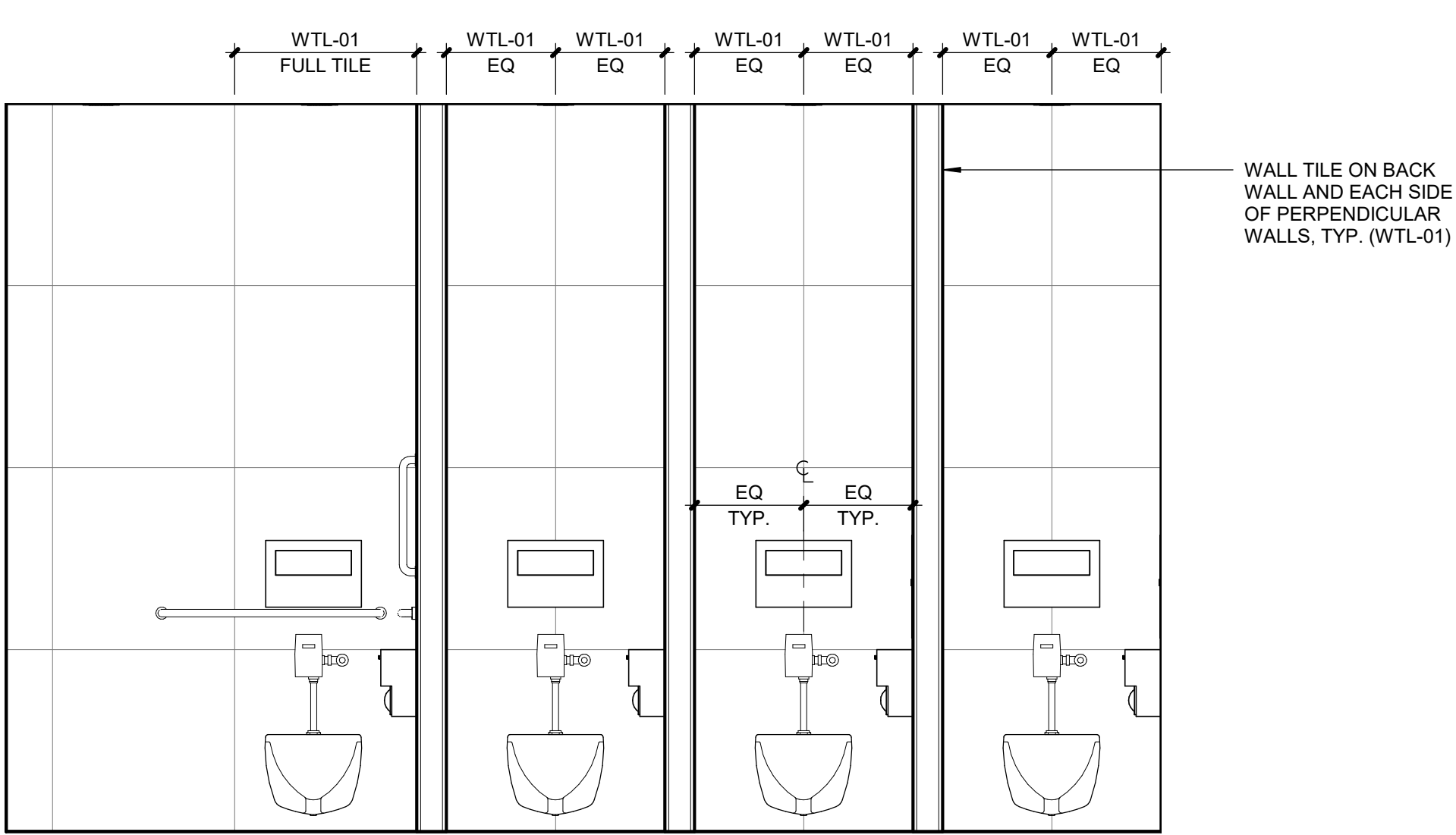
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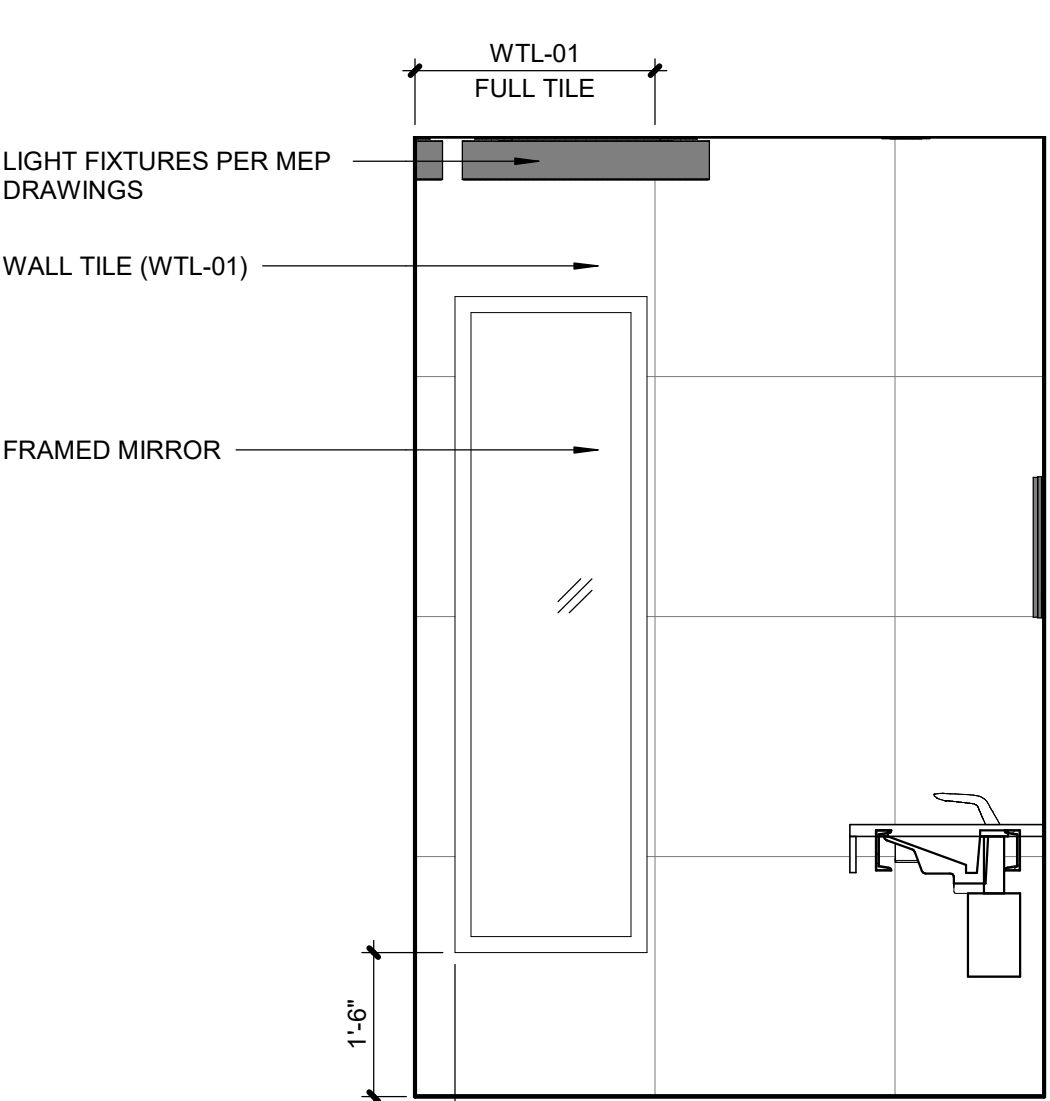
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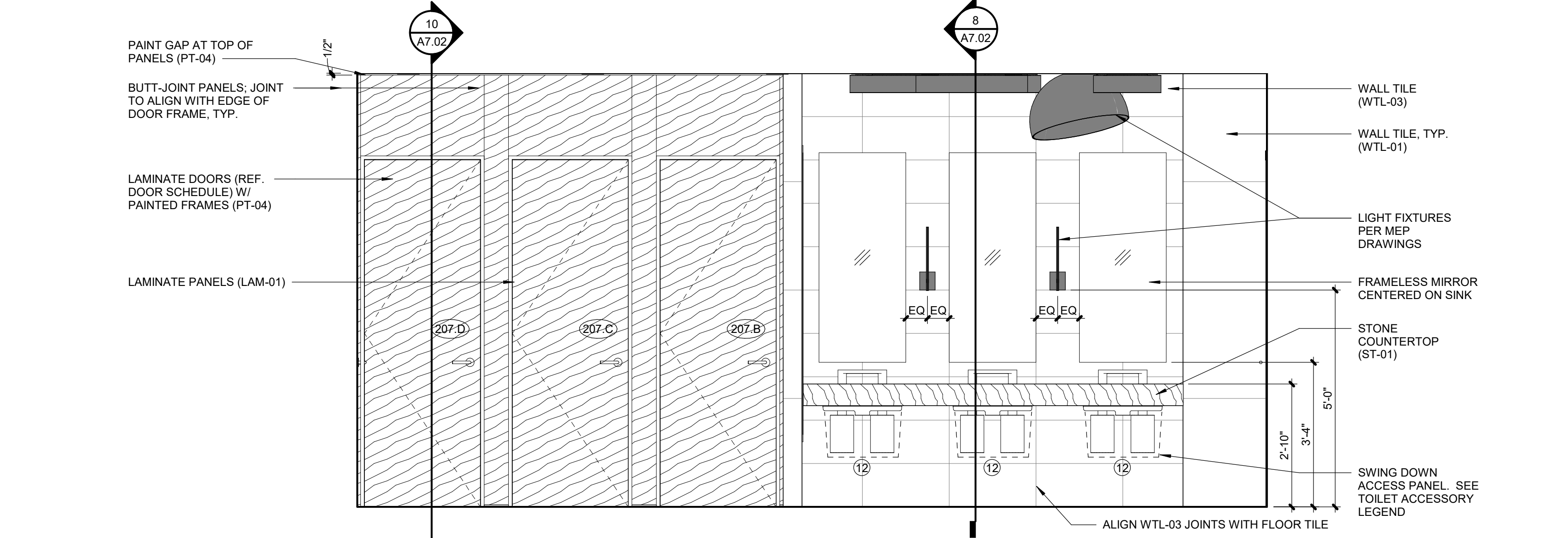
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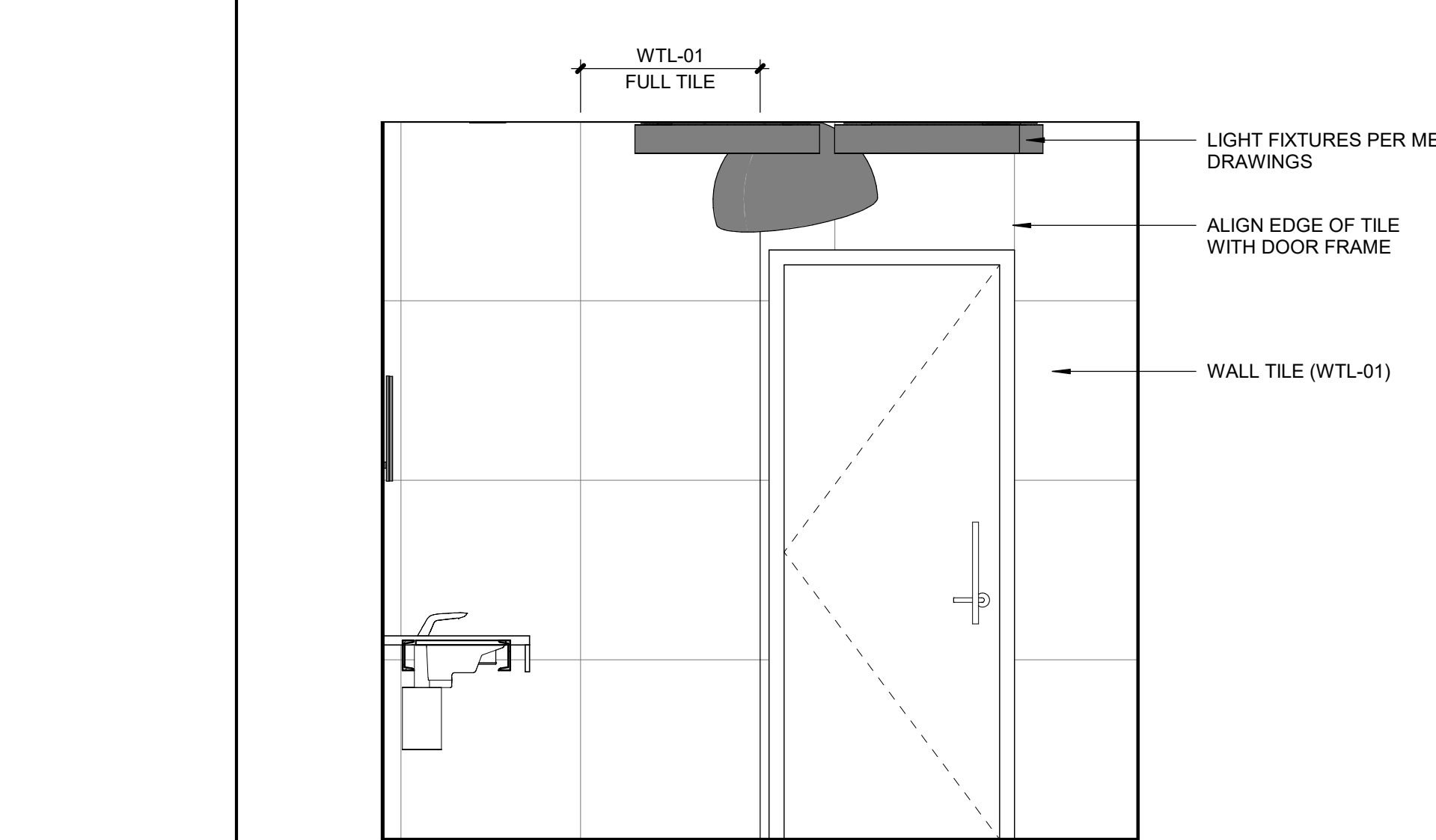
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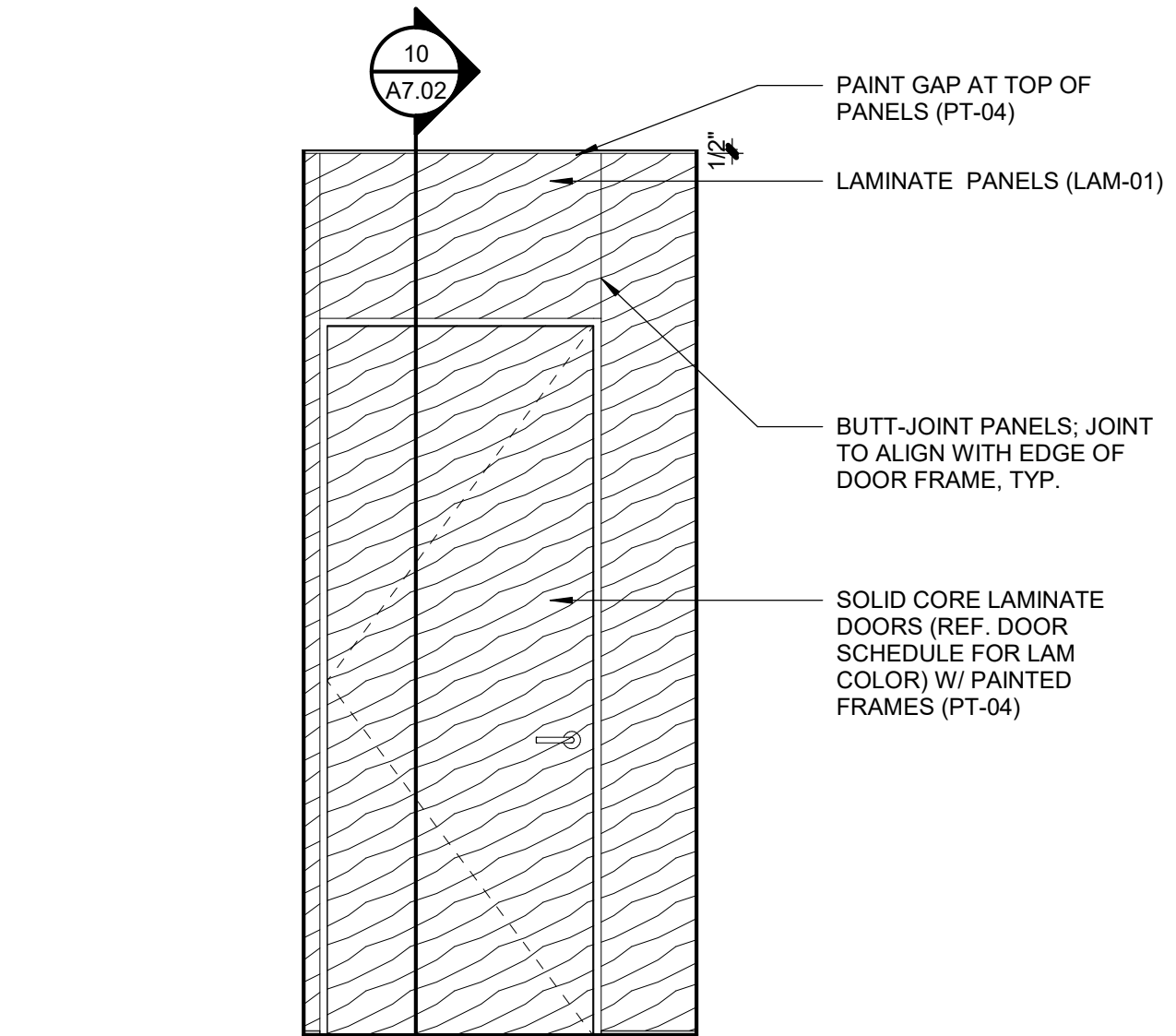
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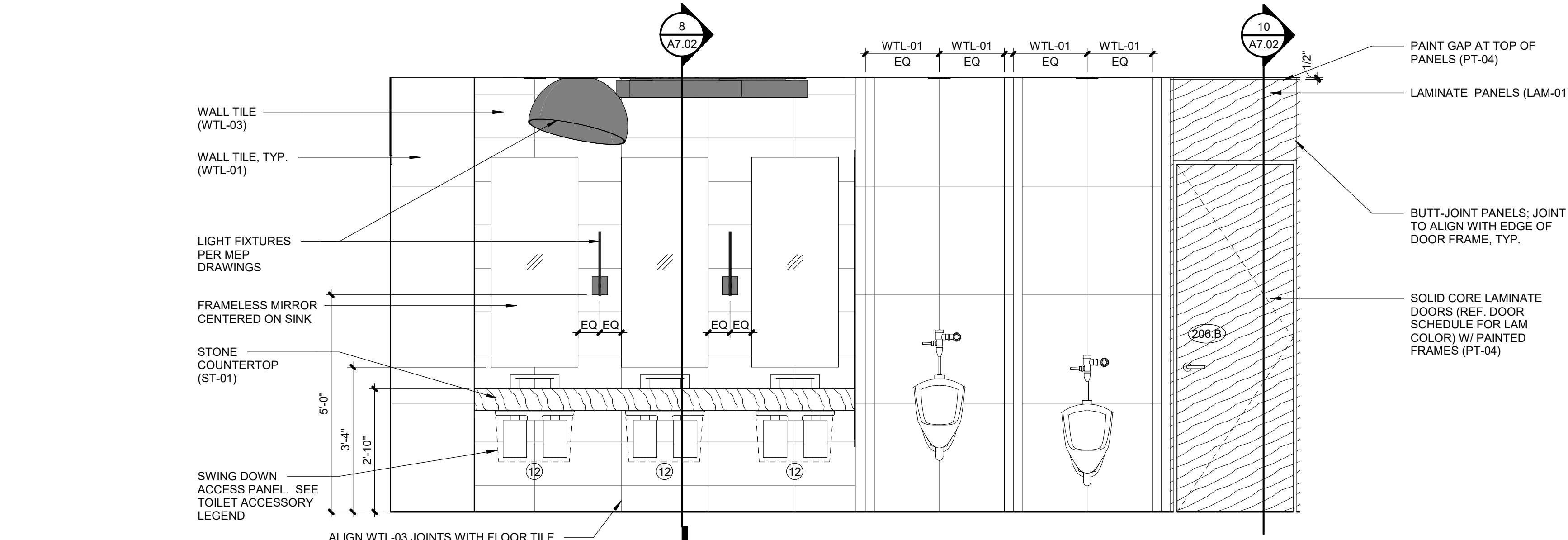
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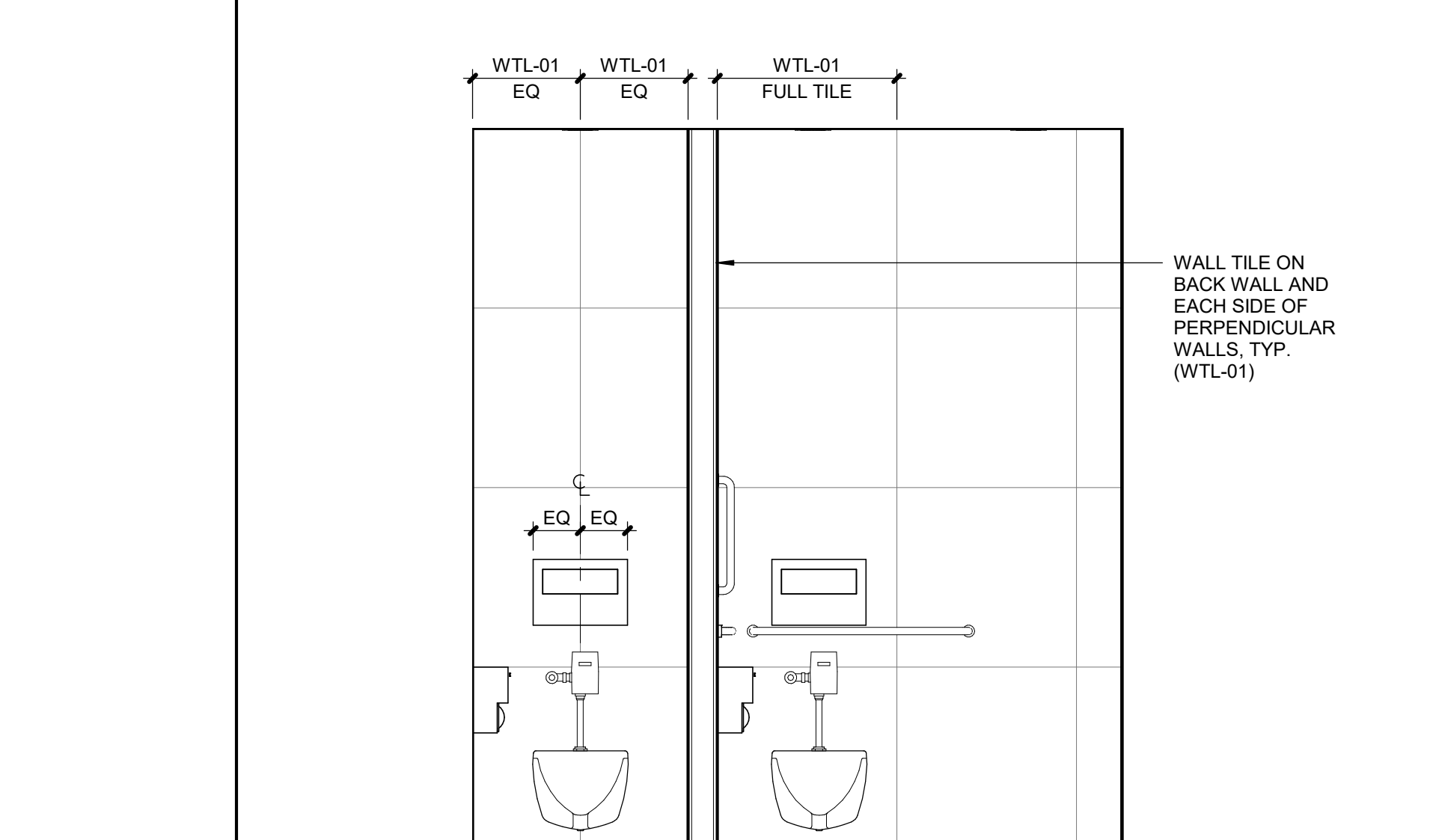
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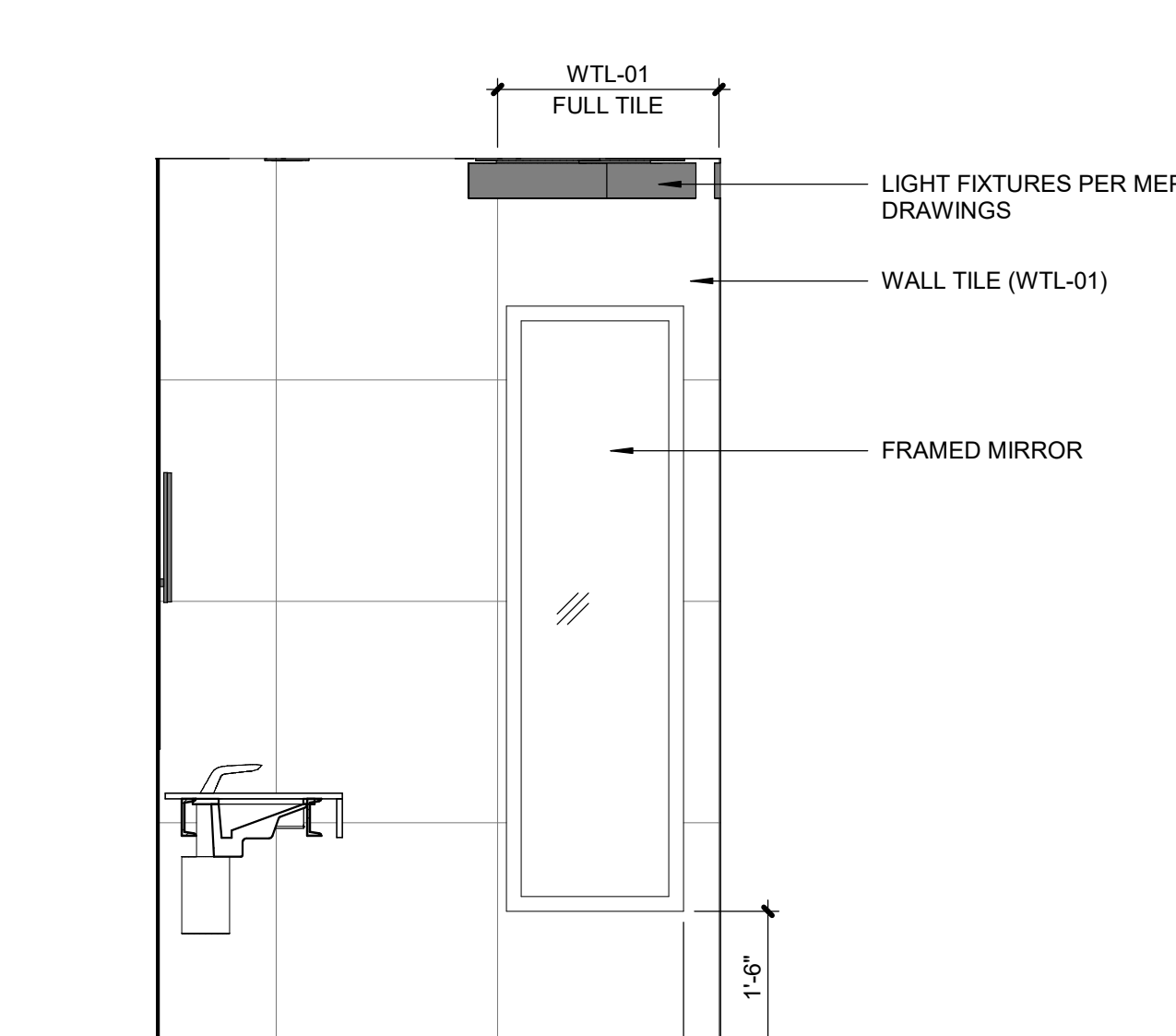
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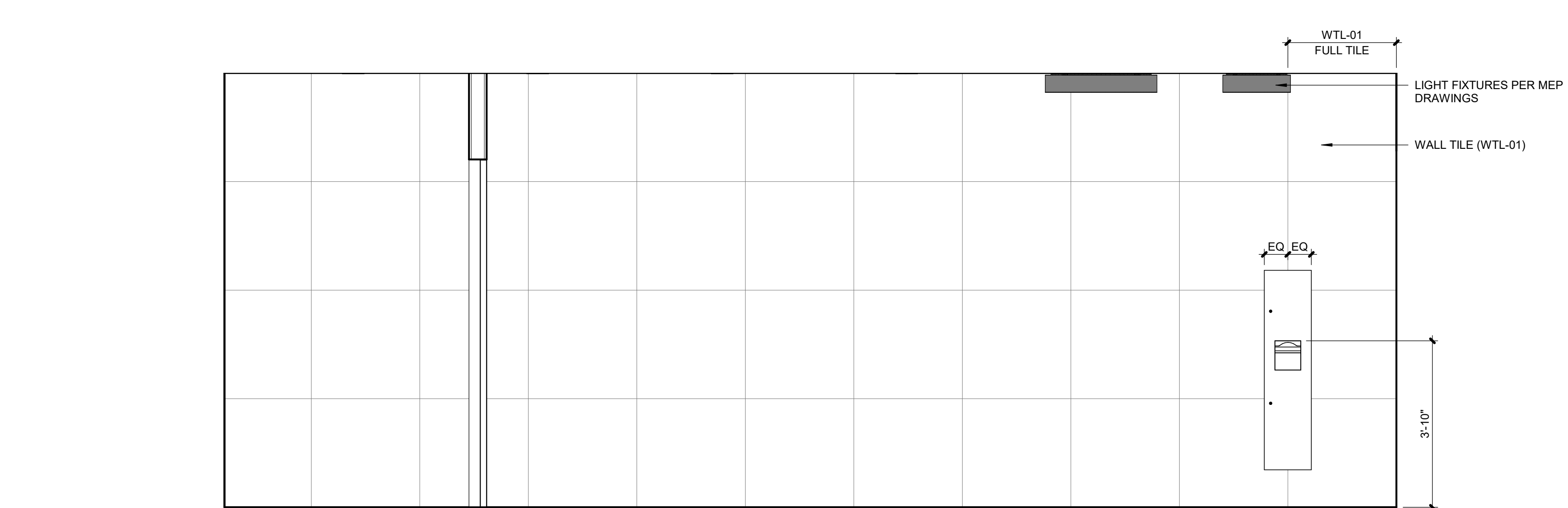
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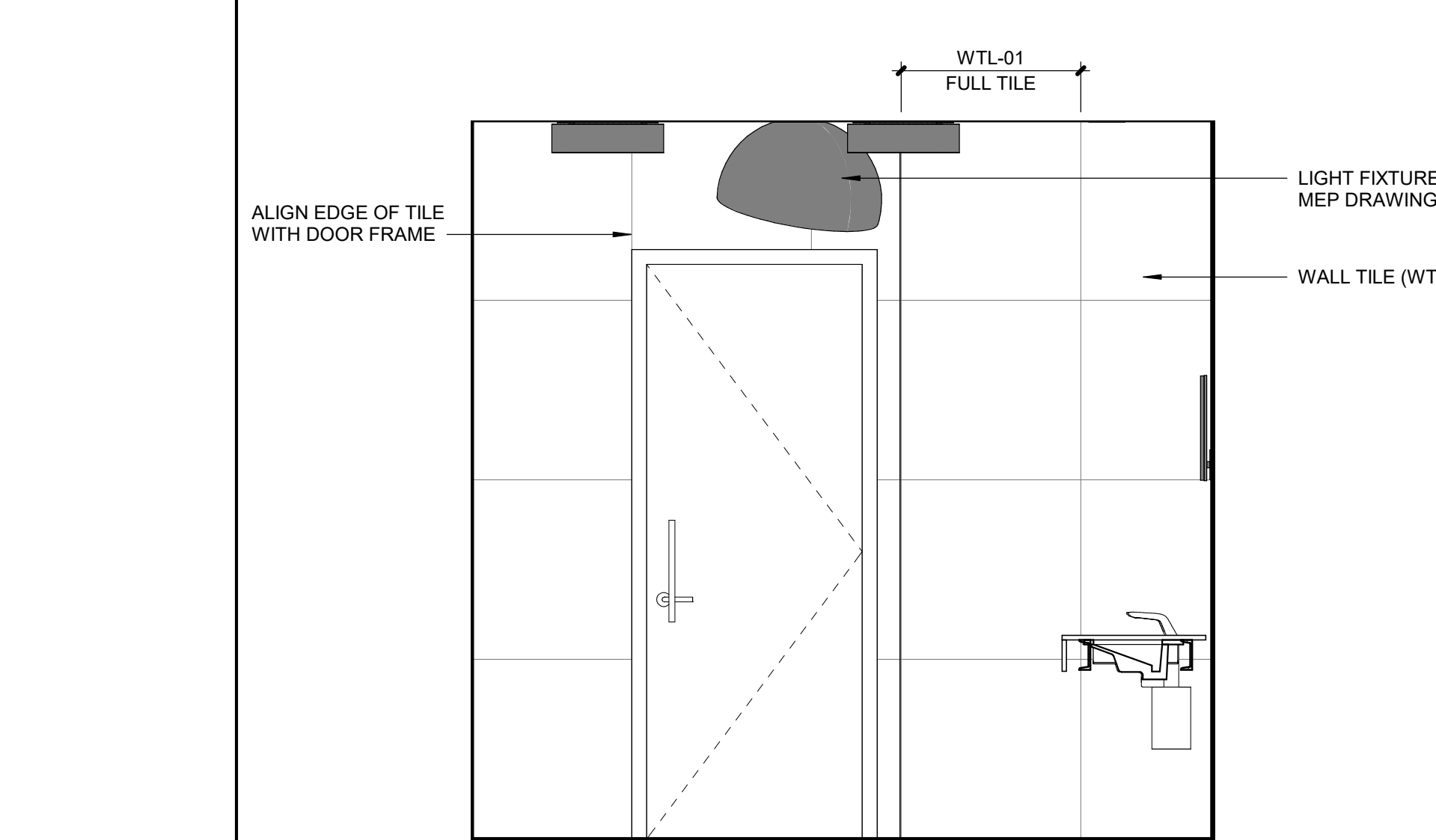
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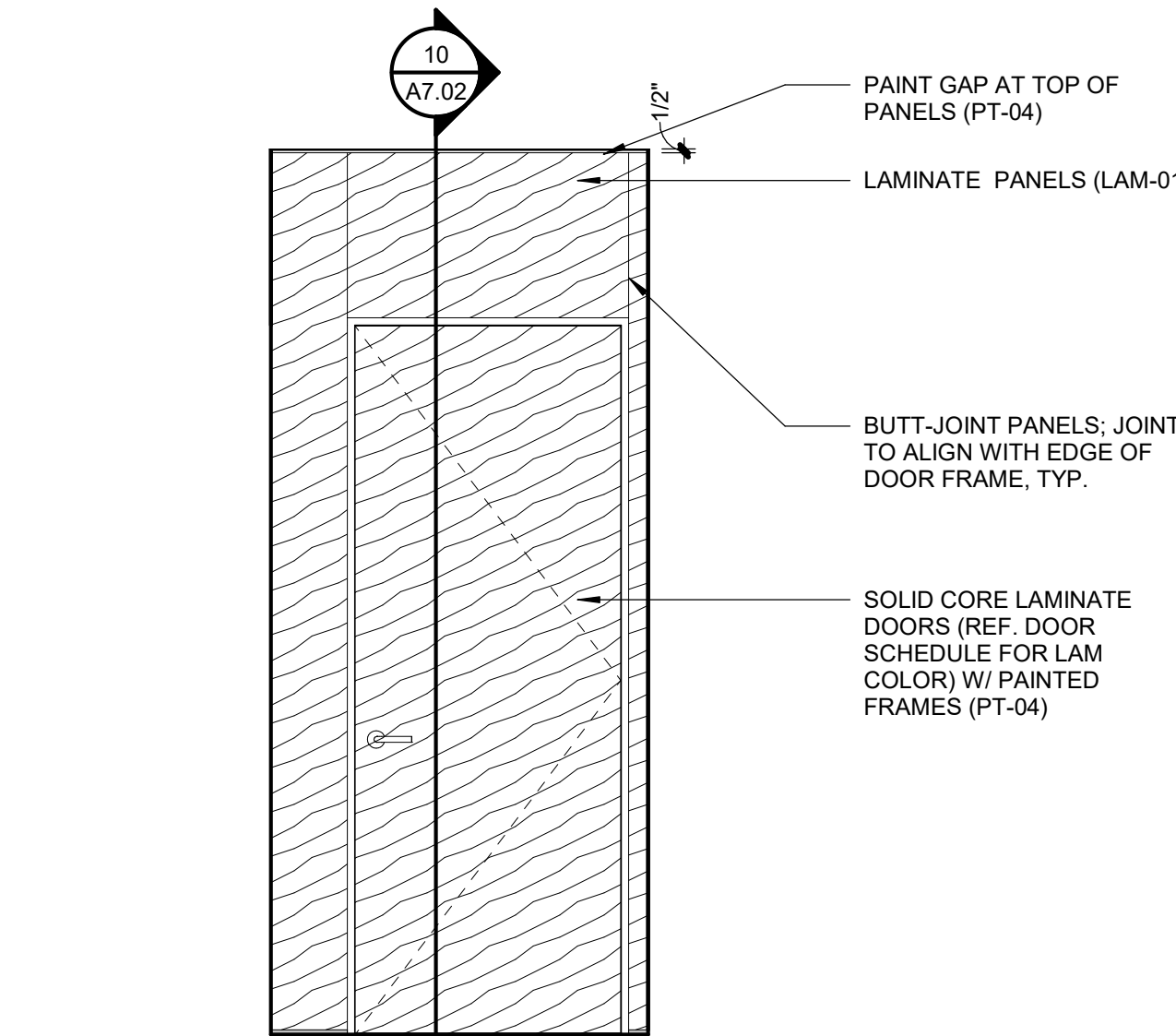
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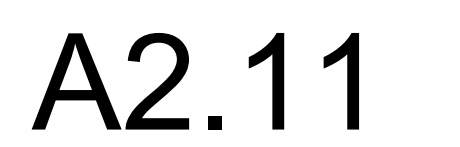
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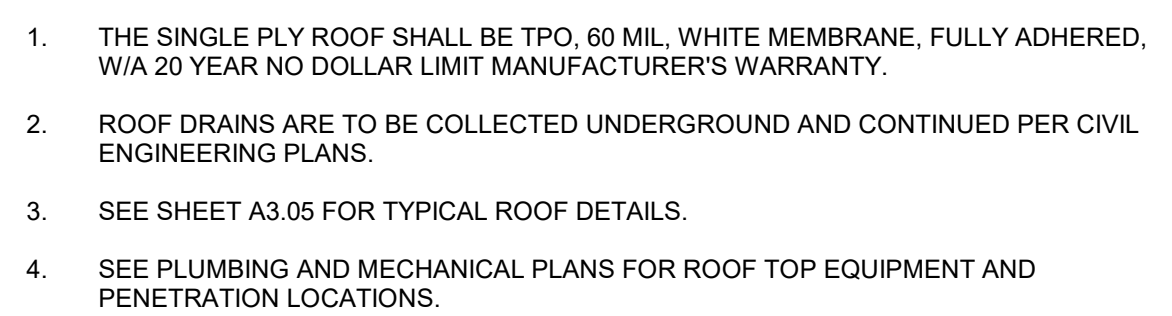


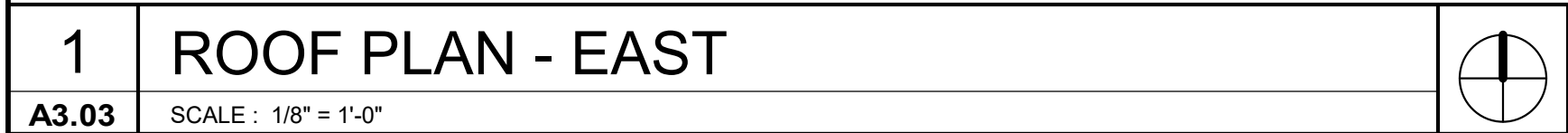
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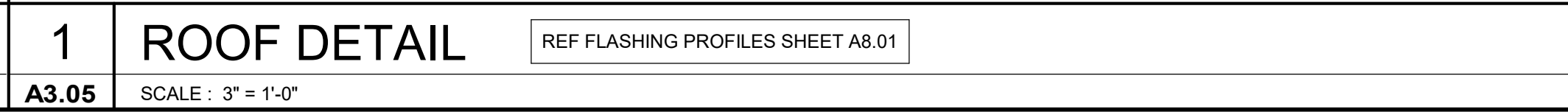
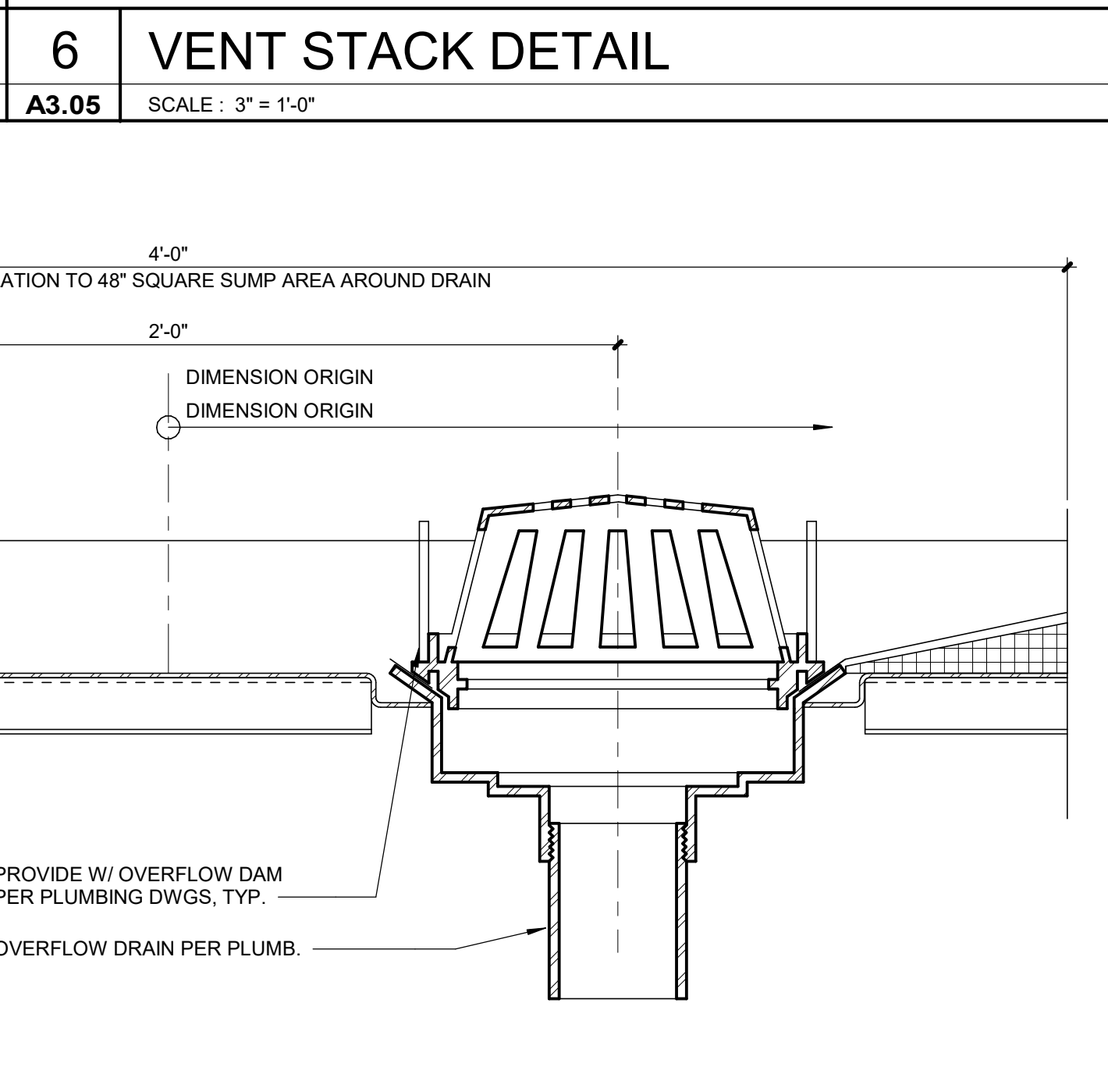
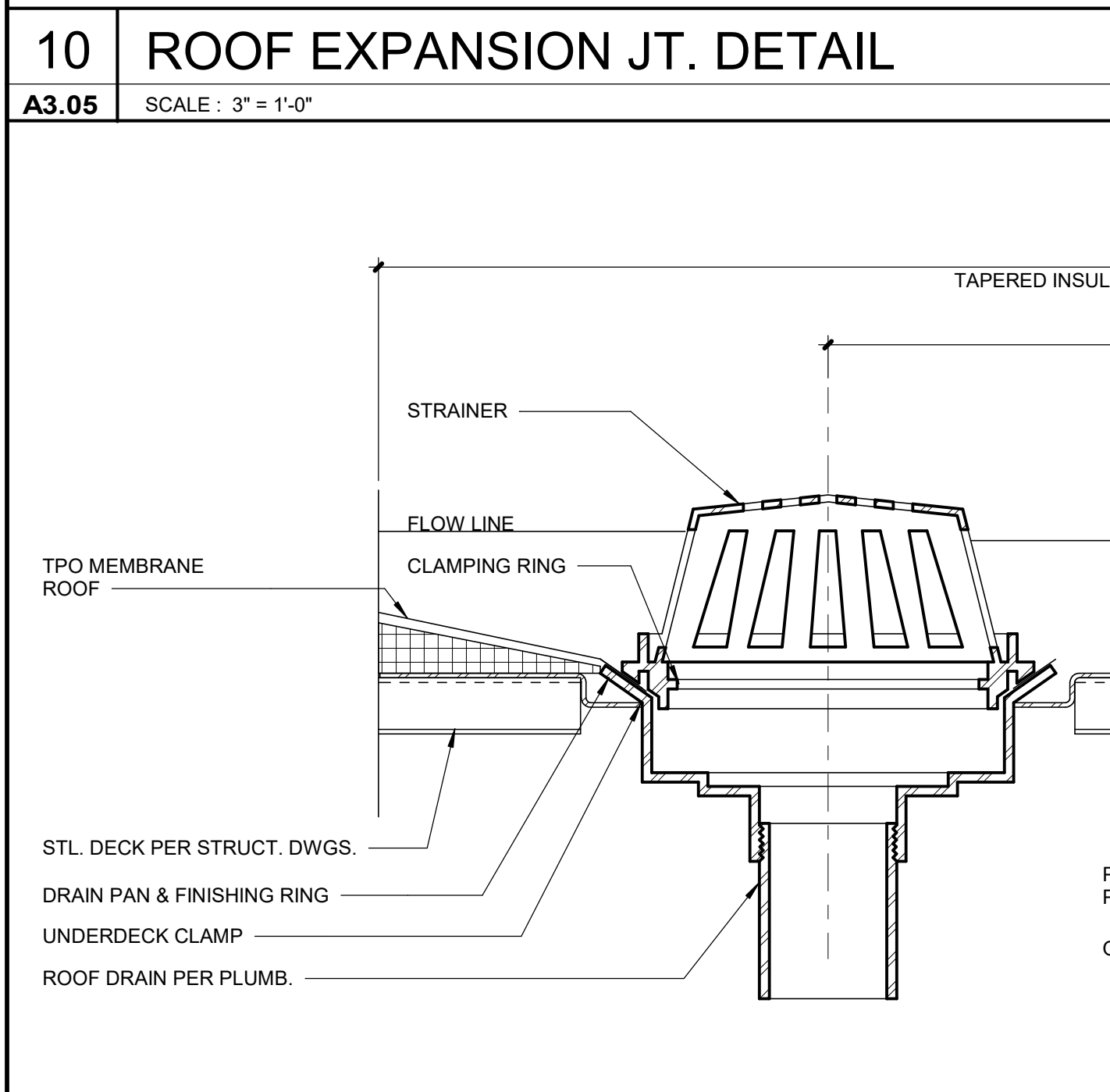
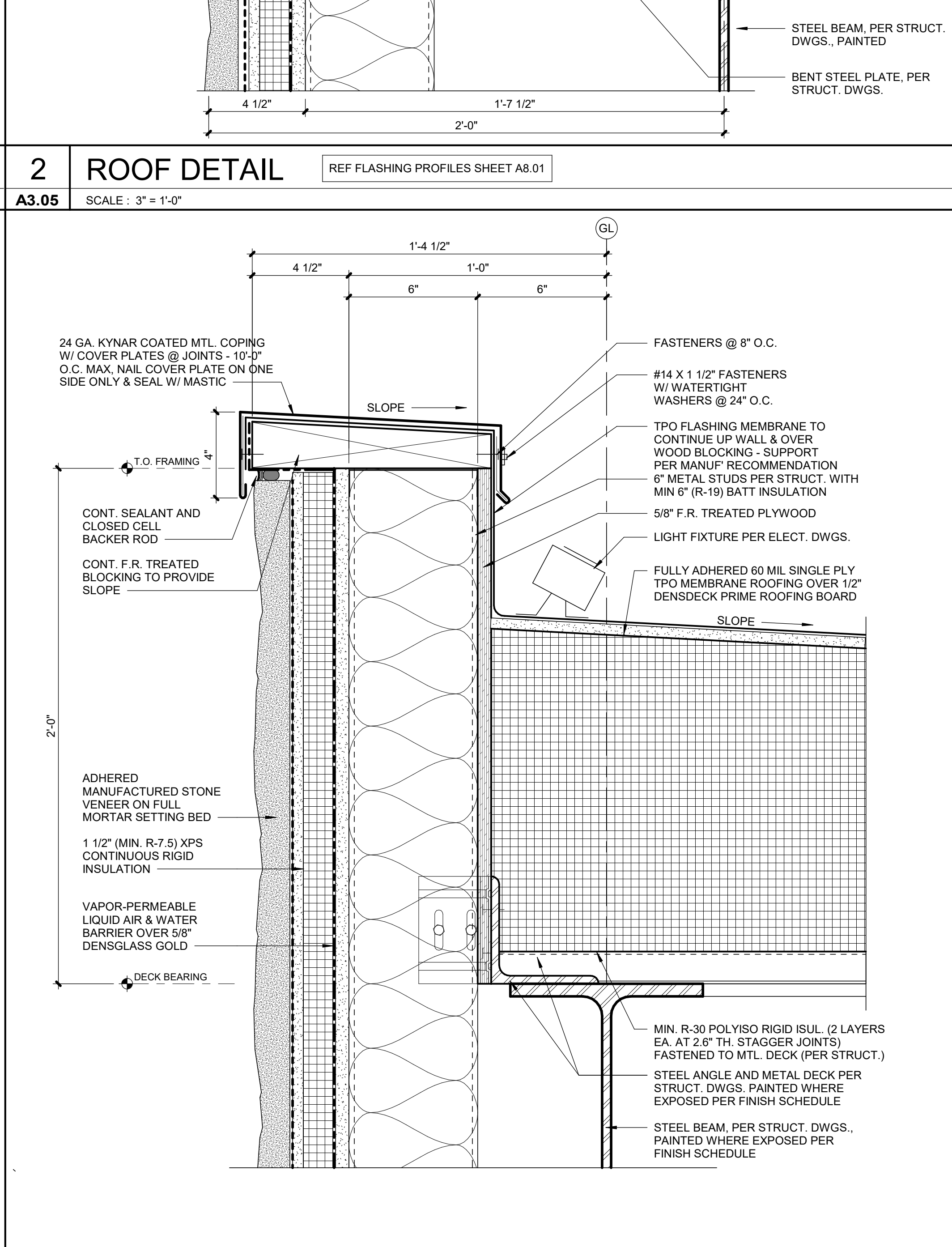
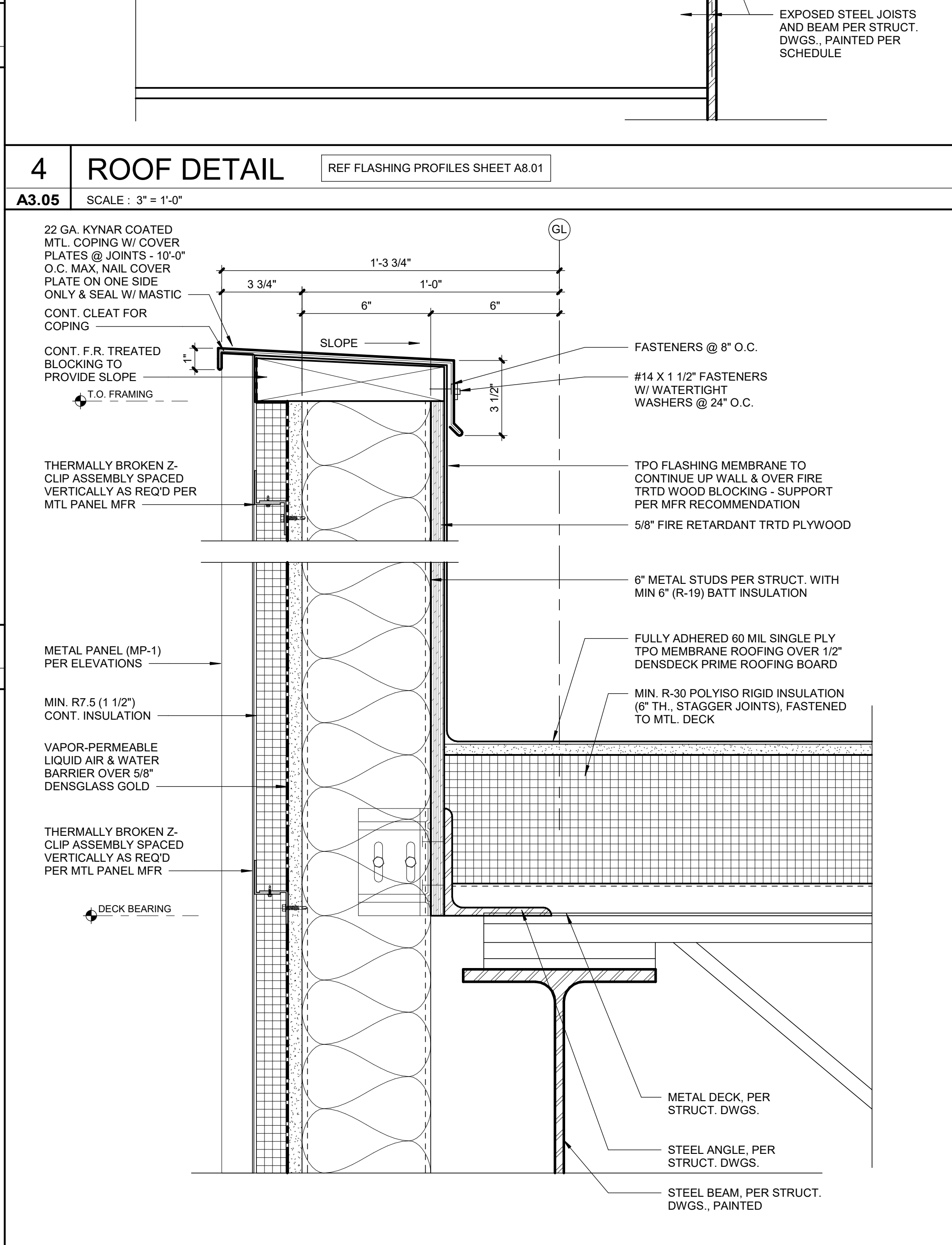
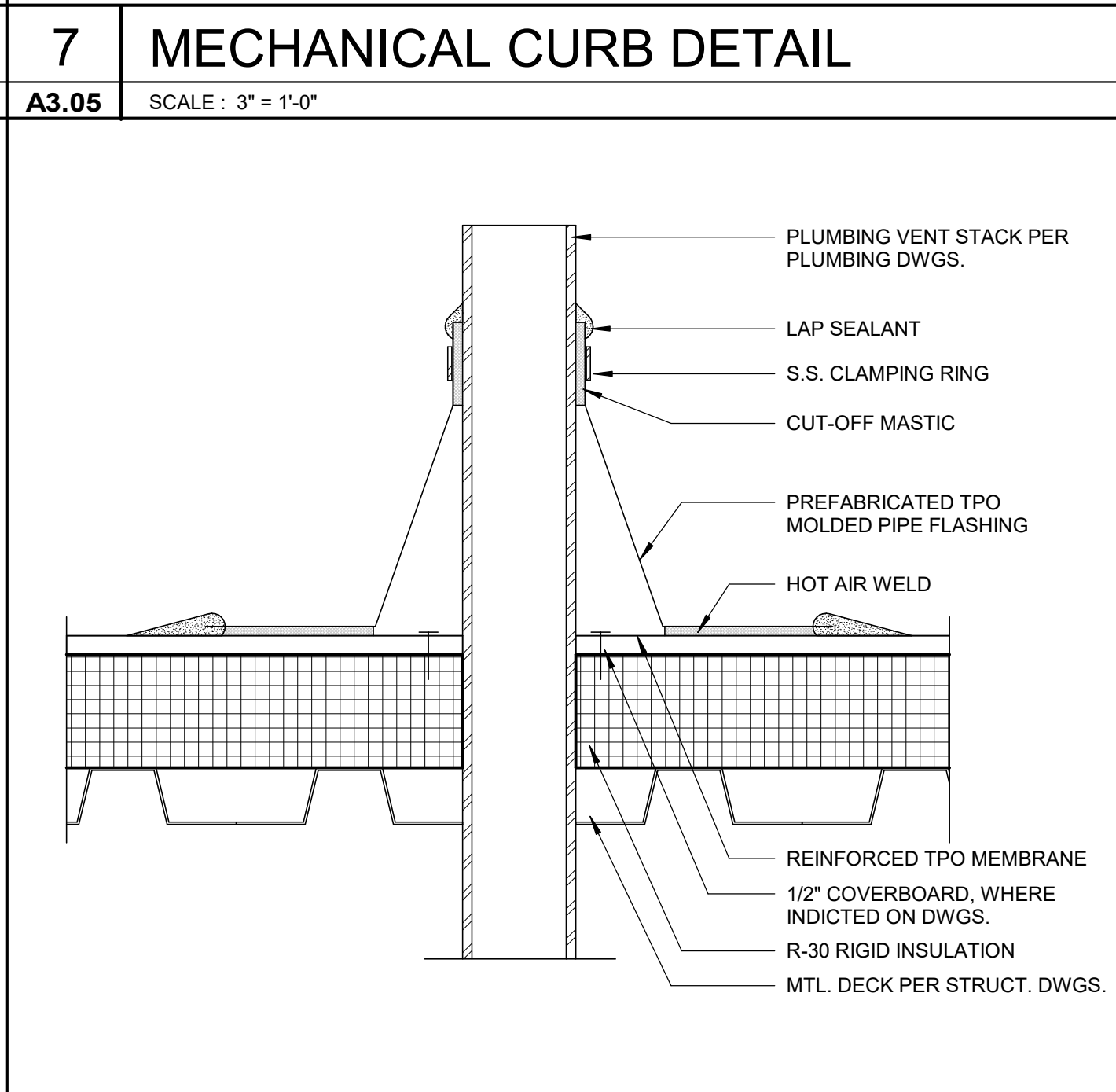
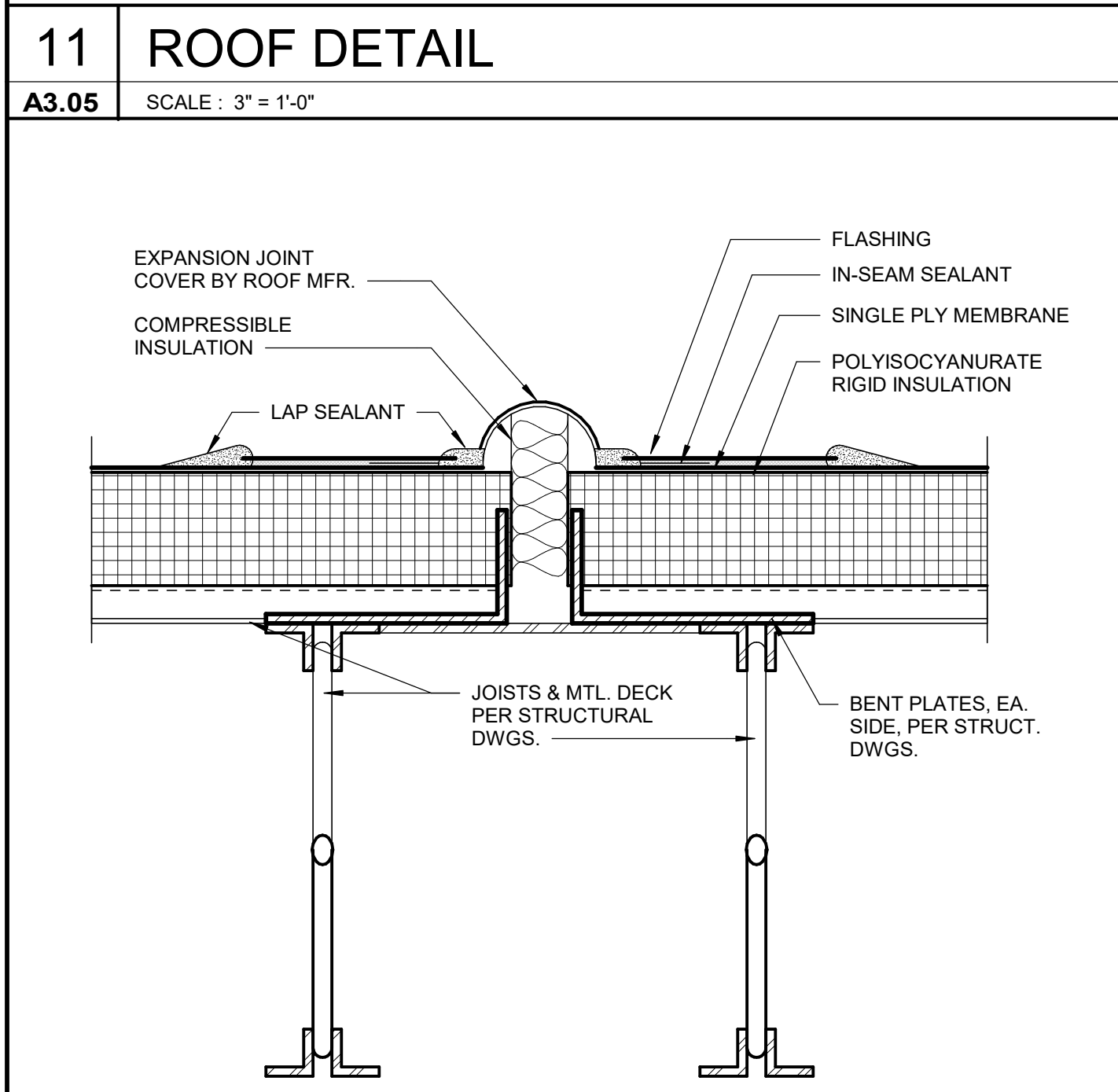
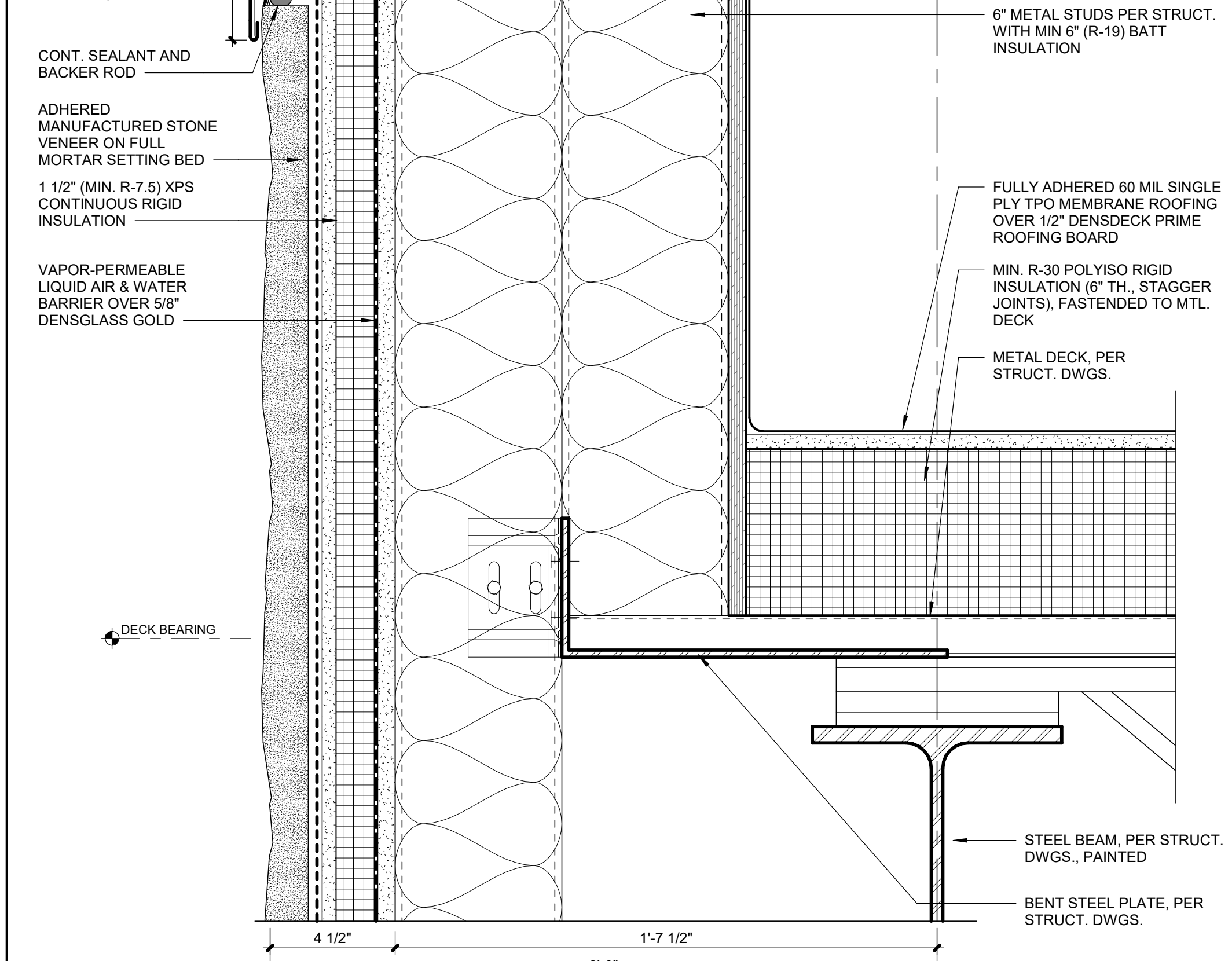
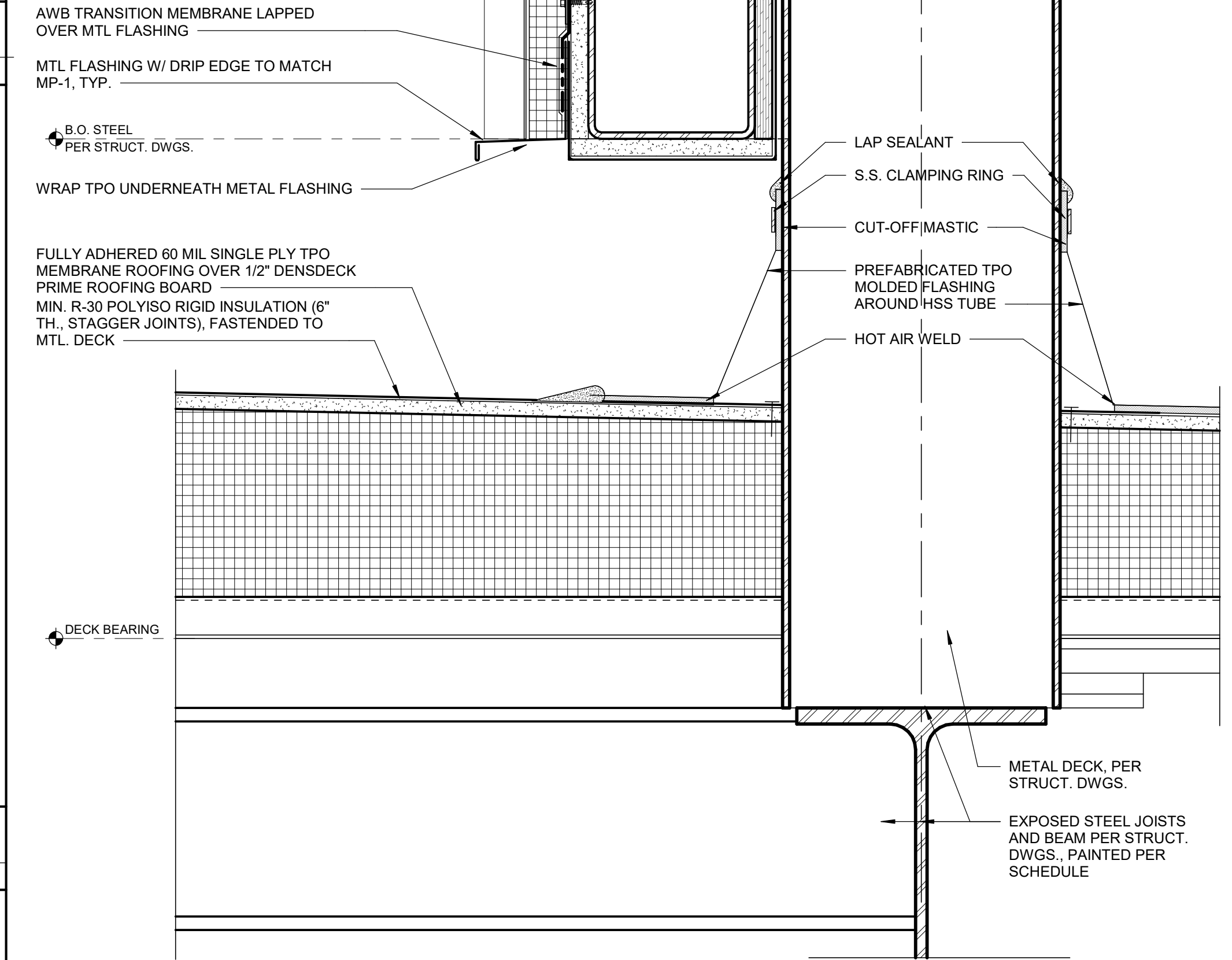
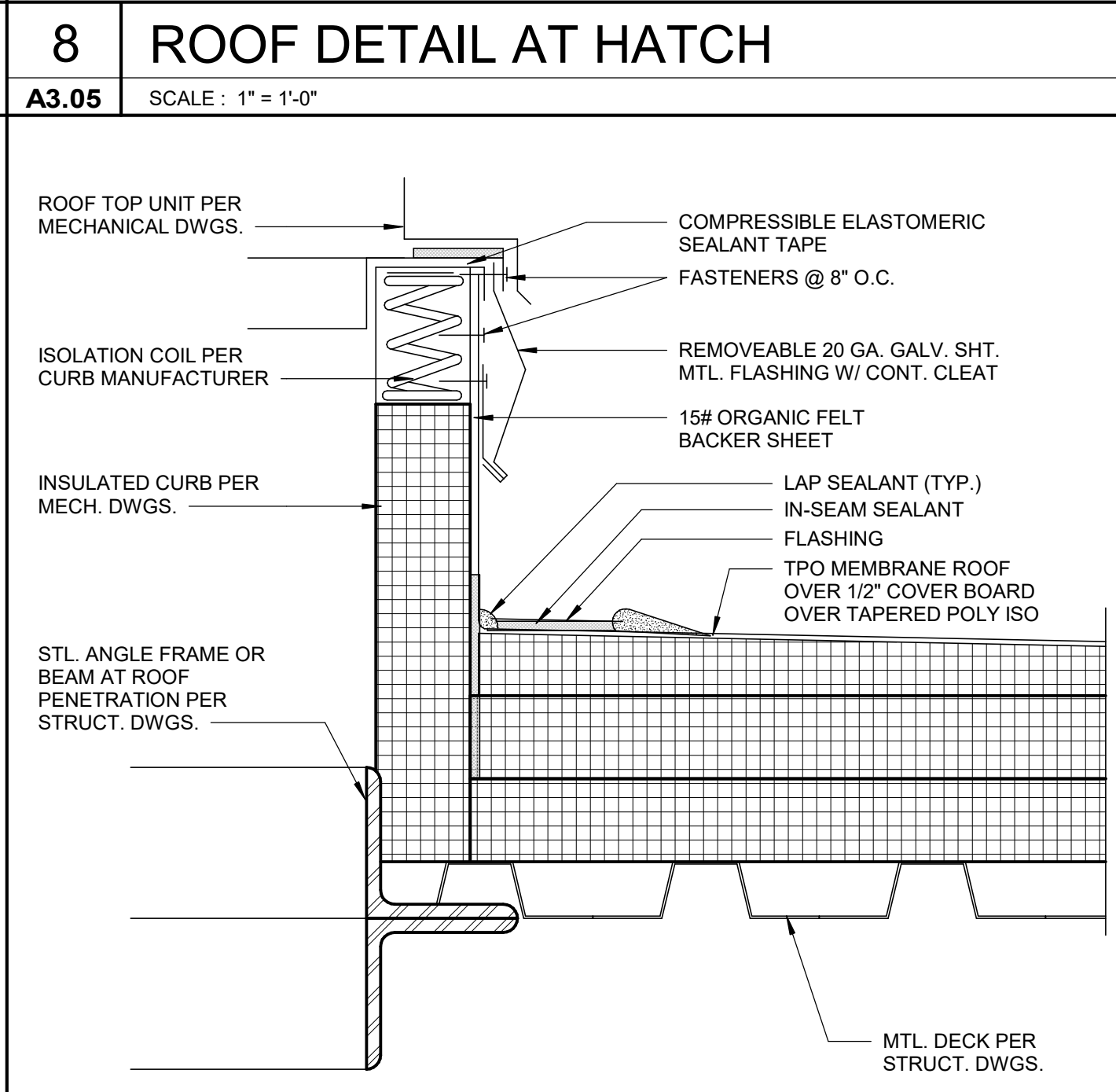
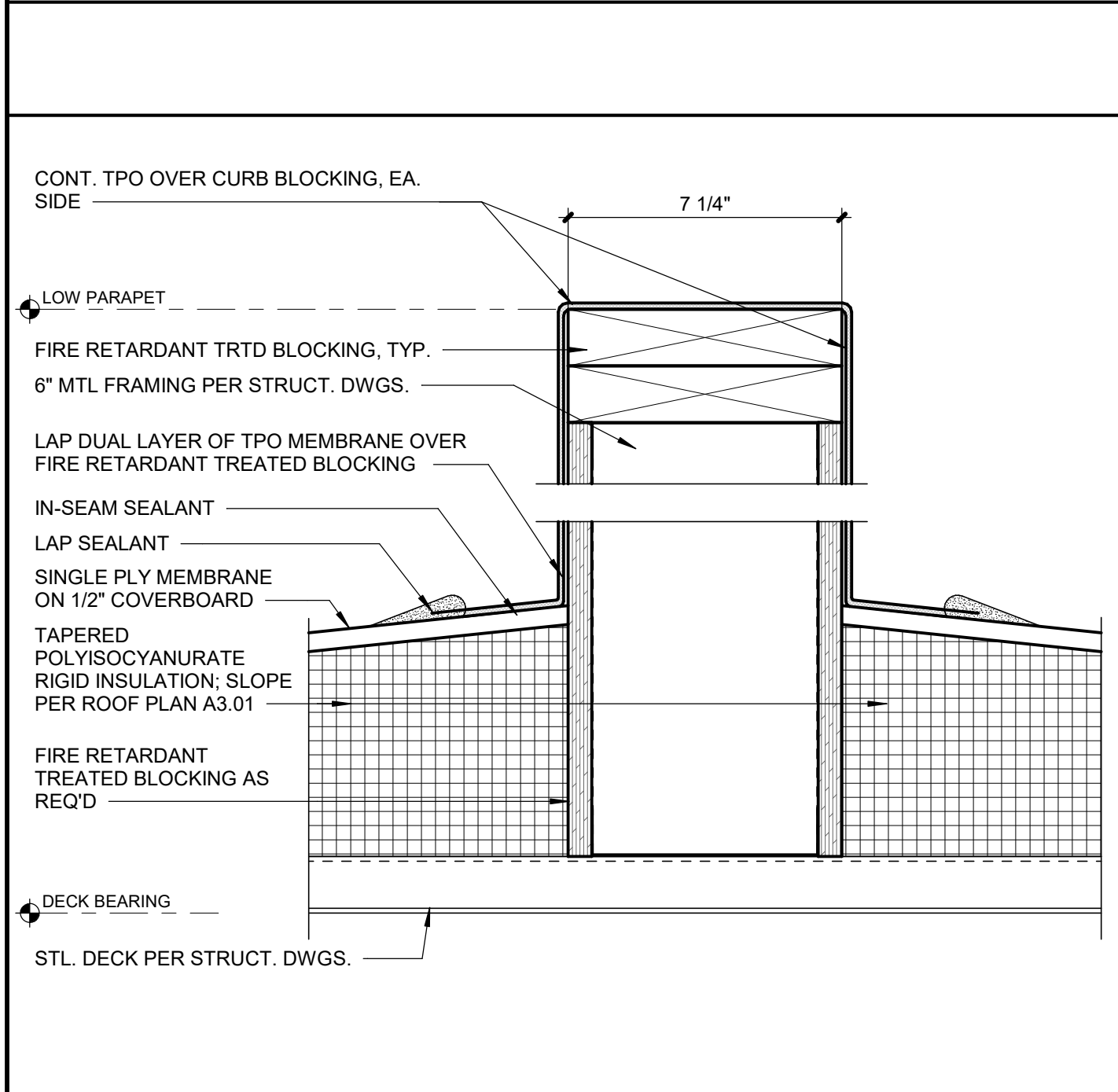
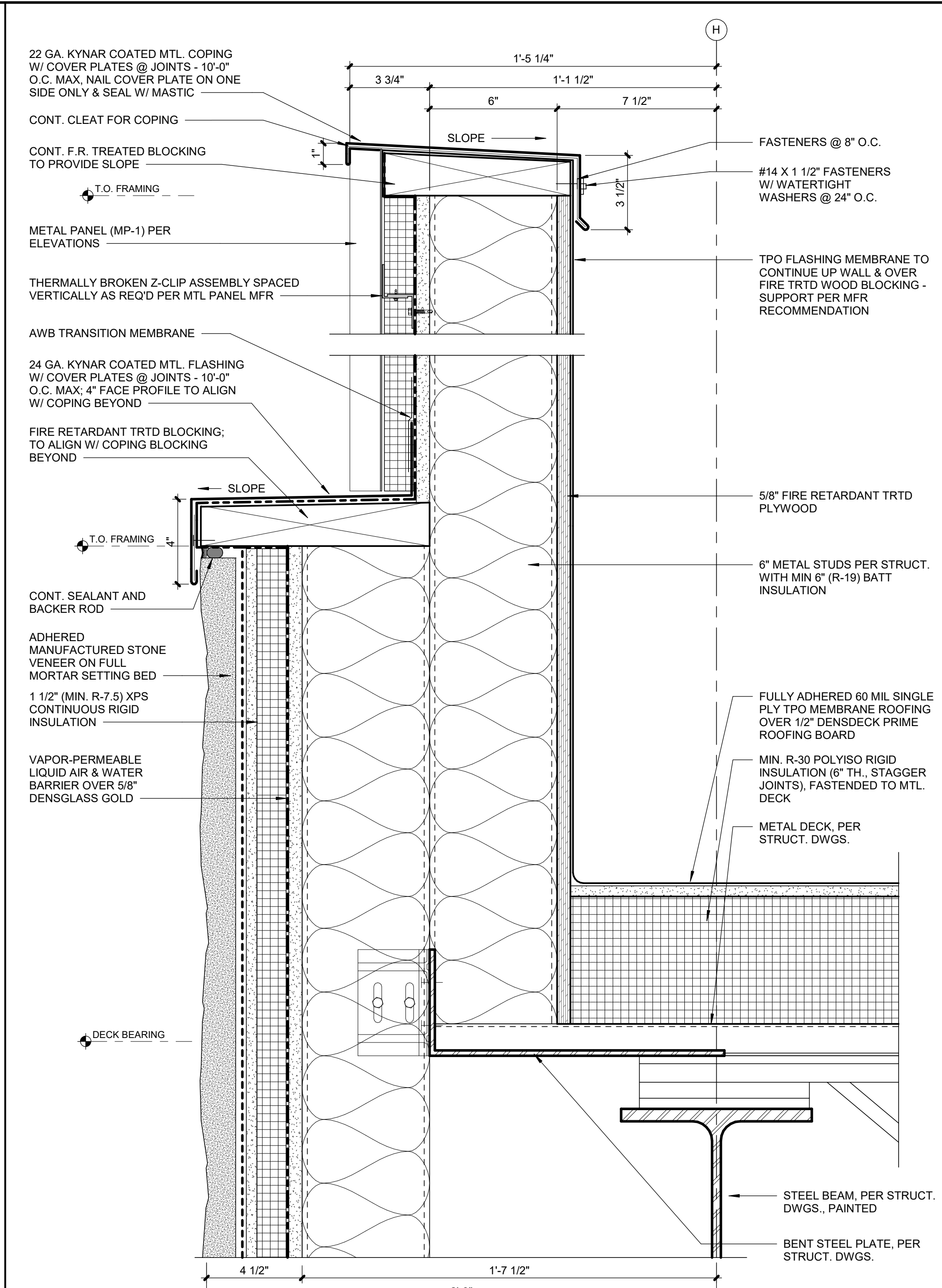
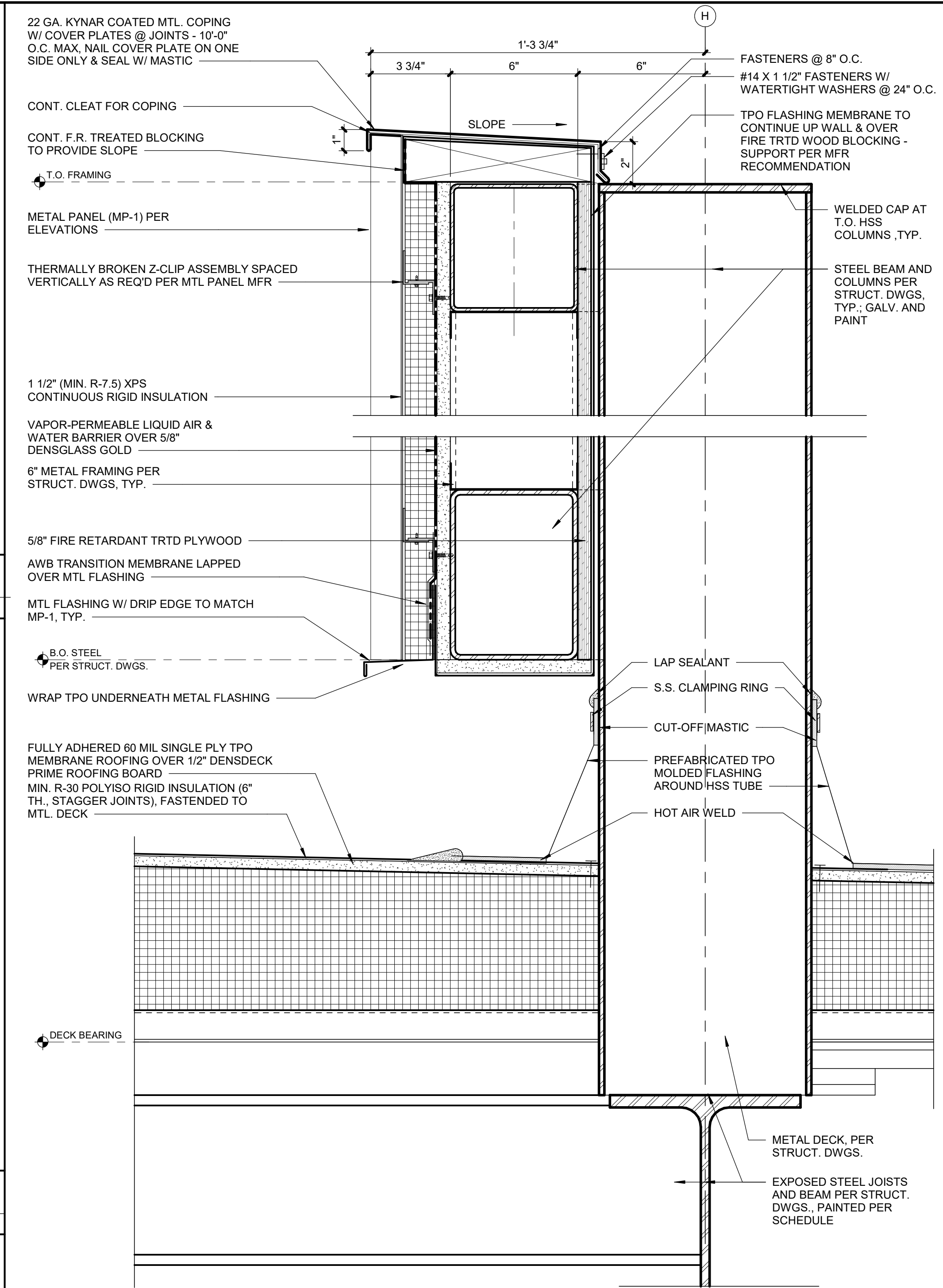
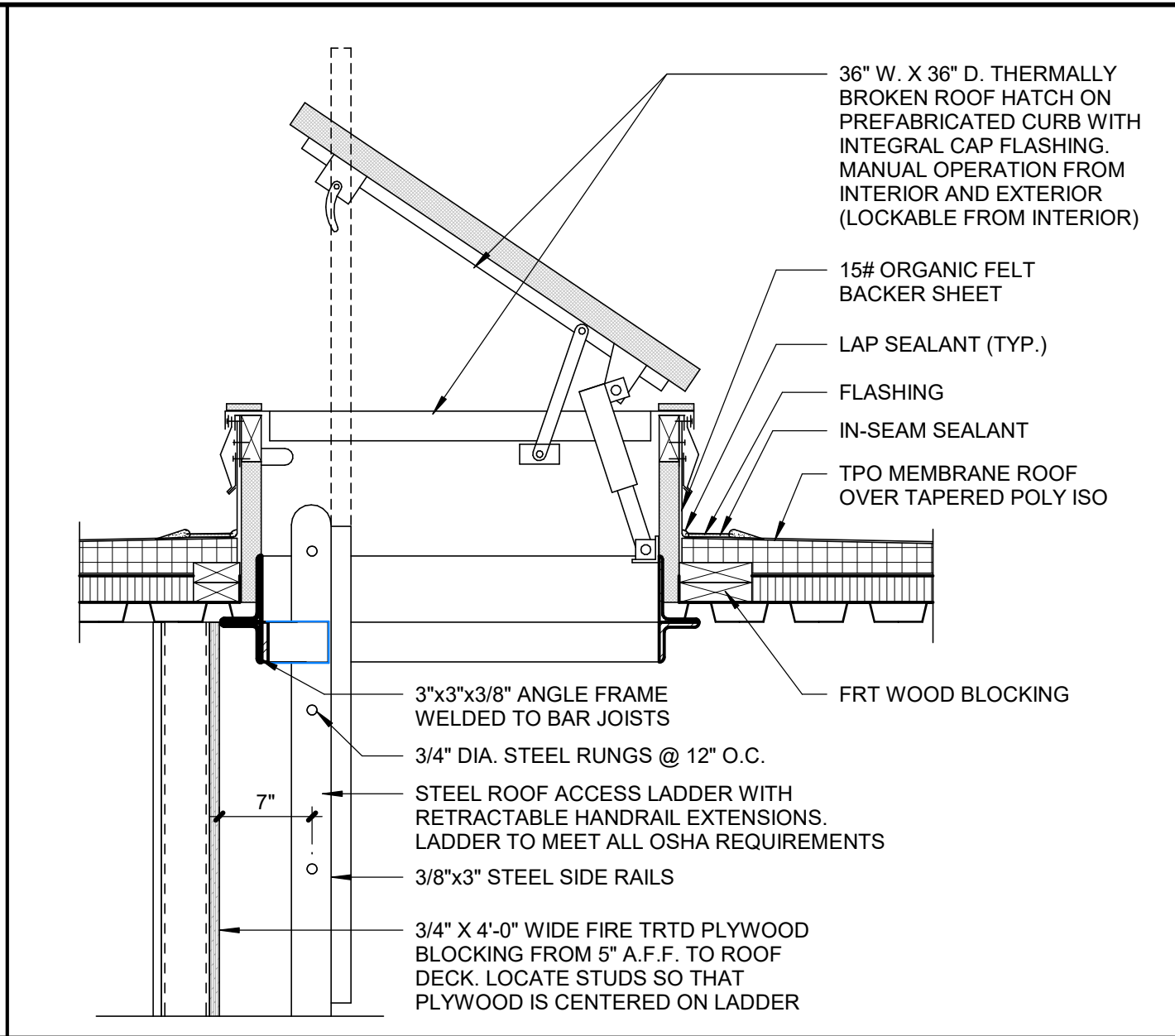
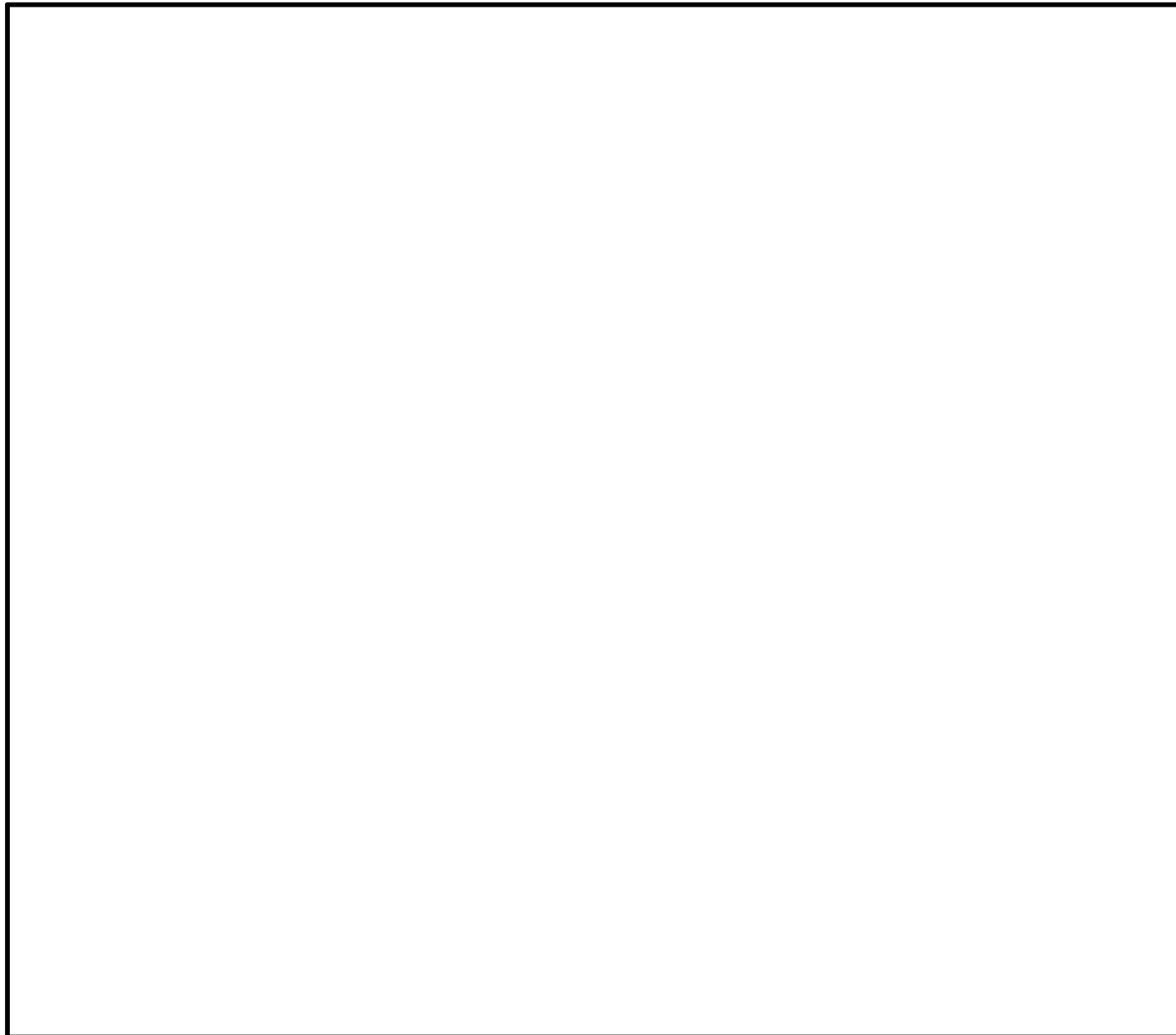


A2.11	SCALE : 1/2" = 1'-0"
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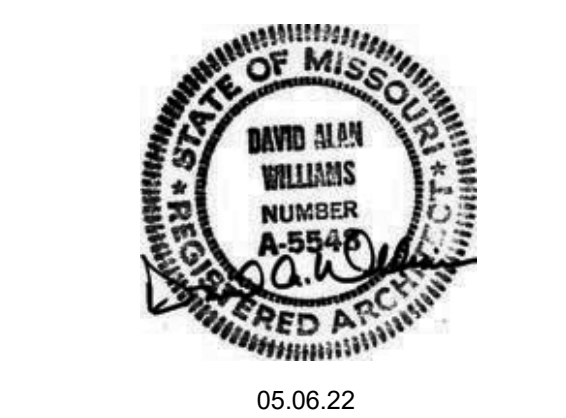




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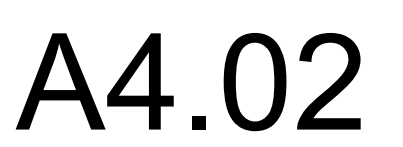


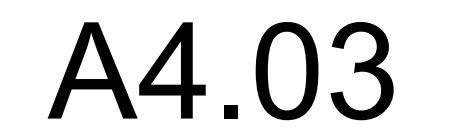
REVISIONS		
No.	Date	Description



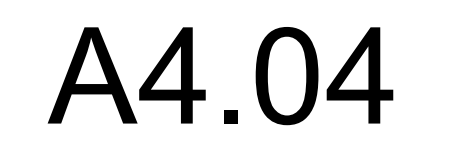
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CIVIL	GBA
LANDSCAPE	LAND 3
FOUNDATIONS	BSE STRUCTURAL ENGINEERS
STRUCTURAL	BSE STRUCTURAL ENGINEERS
PLUMBING	HENDERSON ENGINEERS
MECHANICAL	HENDERSON ENGINEERS
ELECTRICAL	HENDERSON ENGINEERS
FIRE PROTECTION	HENDERSON ENGINEERS
CONTRACTOR	FOGEL-ANDERSON

MP-1	WESTERN STATES 'WESTERN SNAP LOCK' STANDING SEAM METAL PANEL, 18" PANEL WIDTH, COLOR: MATTE MIDNIGHT BLACK
MP-2	KYNAR COATED 22 GA BREAK METAL COPING TO MATCH MP-1; BASIS OF DESIGN: WESTERN STATES 'MATTE MIDNIGHT BLACK' OR APPROVED EQ.
MP-3	WESTERN STATES 'T-GROOVE FLUSH' METAL PANEL, 12" MATTE MIDNIGHT BLACK
MP-4	KYNAR COATED 22 GA BREAK METAL COPING TO MATCH MP-3; BASIS OF DESIGN: WESTERN STATES 'MATTE MIDNIGHT BLACK' OR APPROVED EQ.
	KYNAR COATED 24 GA BREAK METAL COPING TO MATCH ST-1; BASIS OF DESIGN: WESTERN STATES COOL TECH 'POLAR WHITE' OR APPROVED EQ.
GL-1	2" X 4 1/2" WINDOW WALL, BLACK ANODIZED ALUMINUM FINISH W/ 1" INSULATED LOW-E GLAZING UNIT
GL-2	1 1/4" X 7 1/2" CURTAINWALL SYSTEM, BLACK ANODIZED ALUM FINISH W/ 1" INSULATED LOW-E GLAZING UNIT*
GL-3	IGU AT SECOND LEVEL STRUCTURE TO BE SPANDREL GLASS: PPG SOLARBAN 70XL SOLAR CONTROL LOW-E GLAZING UNIT W/ 'SUBUDDED GRAY' OPACIFIER ON 4TH SURFACE, OR APPROVED EQUIV.
WD-1	DELTA MILLWORKS ACCOYA 6" T&G WOOD SOFFIT, UNFINISHED; CONTRACTOR TO STAIN IN-FIELD TO MATCH ARCHITECT'S SAMPLE
ST-1	BORAL CULTURED STONE; HEWYN STONE, COLOR: ARCTIC; PATENT#: HSPF35809**
ST-2	ELDORADO CULTURED STONE MARQUEE24, COLOR: DOVETAIL**
ST-3	ELDORADO SPLIT EDGE 'WHITE CLOUD' SILL PIECE AT ALL WINDOW WALL OPENINGS IN ST-1 OR ST-2 WALLS **PROVIDE CONTROL JOINT AT SECOND LEVEL ON ALL ST-1 AND ST-2 LOCATIONS PER ELEVATIONS AND MFR RECOMMENDATIONS; STONE TO BE NO GROUT, DRY-STACK AESTHETIC.
MB-1	GROUND FACE 10Wx8Hx16L MASONRY BLOCK PER STRUCTURAL DWGS. BURNISHED FINISH, COLOR: MIDWEST SLATE
AWN-1	PRE-MANUF. SURFACE MOUNTED STANDING SEAM METAL TO MATCH MP-1 ON PAINTED ALUMINUM BRACKETS
FP-1	LOCATION FOR FDC AND KNOX BOX PER FIRE PROTECTION DRAWINGS; CENTER FIXTURES IN WALL UNO
PD-1	ROOF OVERFLOW DRAIN DOWNSPOUT COVER PER PLUMBING DRAWINGS; CENTER ON WALL SPACE UNO
WL-1	EXTERIOR DECORATIVE WALL LIGHT; AT 9'-0" AFF; CENTER ON WALL SPACE PER ELEVATIONS UNO
WL-2	EXTERIOR WALL-MOUNTED UP-LIGHT; ALIGN W/ SOFFIT AT 12'-0" AFF; CENTER ON WALL SPACE PER ELEVATIONS UNO
WL-3	PROVIDE -1 BOX FOR FUTURE WALL LIGHT PER TENANT AT 9'-0" AFF; CENTER ON WALL SPACE PER ELEVATIONS UNO

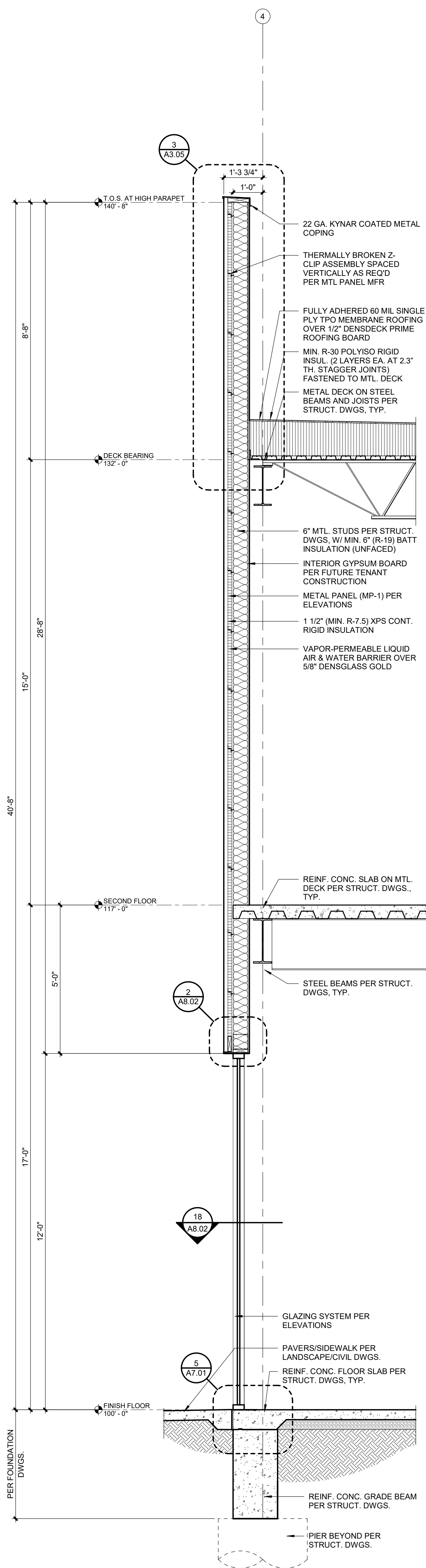




A4.03	SCALE : 1/8" = 1'-0"
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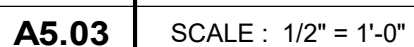
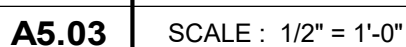
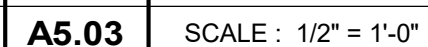
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A5.00	SCALE : 1/8" = 1'-0"
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1	WALL SECTION
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A5.02 SCALE : 1/2" = 1'-0"



Project No.:	19050.01a
Date:	06.02.22
Issued For:	ADDENDUM 1

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05.06.22

05.06.22

ARCHITECT FINKLE+WILLIAMS
ARCHITECTURE

JIL

LANDSCAPE LAND 3

FOUNDATIONS BSE STRUCTURAL
ENGINEERSSTRUCTURAL BSE STRUCTURAL
ENGINEERS

LUMBIN
HENDERSON
ENGINEERS

MECHANICAL HENDERSON
ENGINEERSELECTRICAL HENDERSON
ENGINEERS

RE PROTECTION HENDERSON
ENGINEERS

CONTRACTOR FOGEL-ANDERSON



SHEET TITLE

WALL SECTIONS

SHEET NUMBER

A5.04



Project No.: 19050.01a

Date: 06.02.22

Issued For: ADDENDUM 1

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REGISTRATION



15.06.22

PROJECT TEAM

ARCHITECT	FINKLE+WILLIAMS ARCHITECTURE
CIVIL	GBA
LANDSCAPE	LAND 3
FOUNDATIONS	BSE STRUCTURAL ENGINEERS
STRUCTURAL	BSE STRUCTURAL ENGINEERS
PLUMBING	HENDERSON ENGINEERS
MECHANICAL	HENDERSON ENGINEERS
ELECTRICAL	HENDERSON ENGINEERS
FIRE PROTECTION	HENDERSON ENGINEERS
CONTRACTOR	FOGEL-ANDERSON

FINKLE + WILLIAMS
ARCHITECTUR

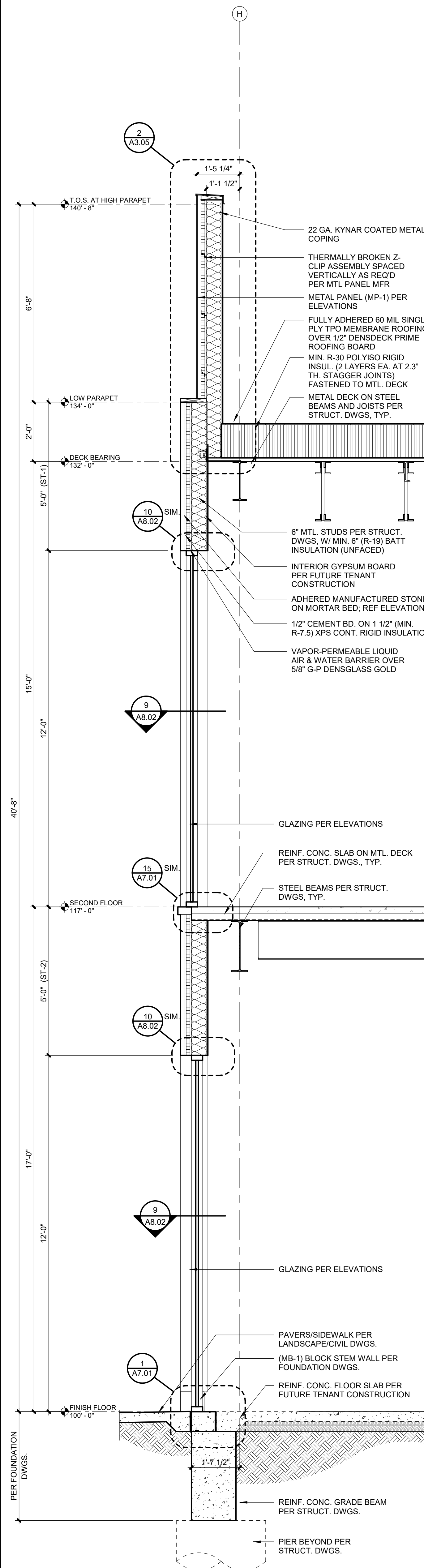
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SHEET TITLE

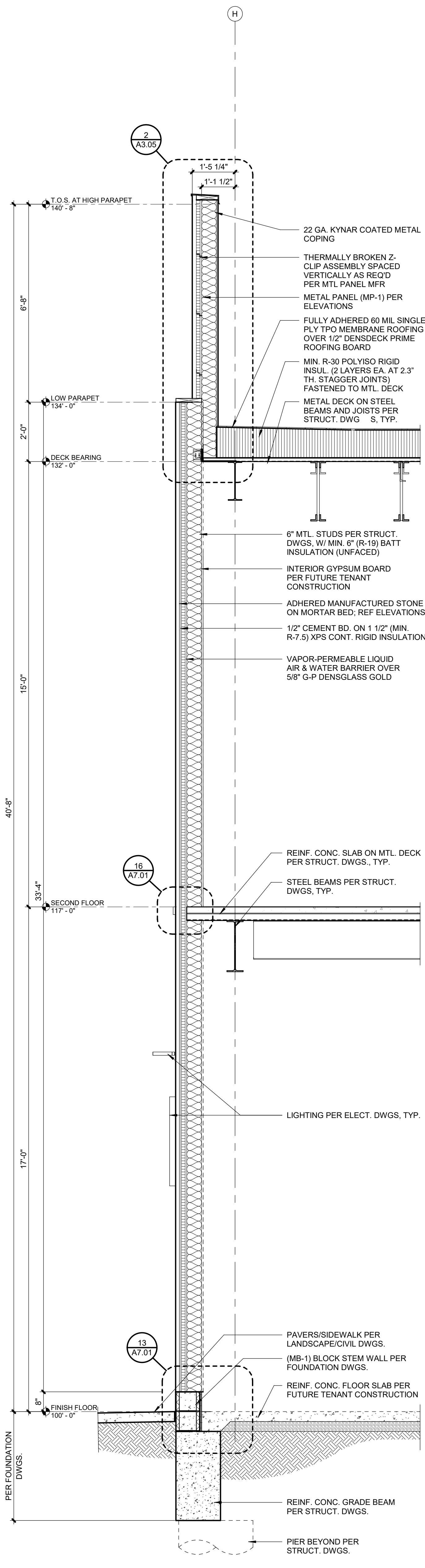
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SHEET NUMBER

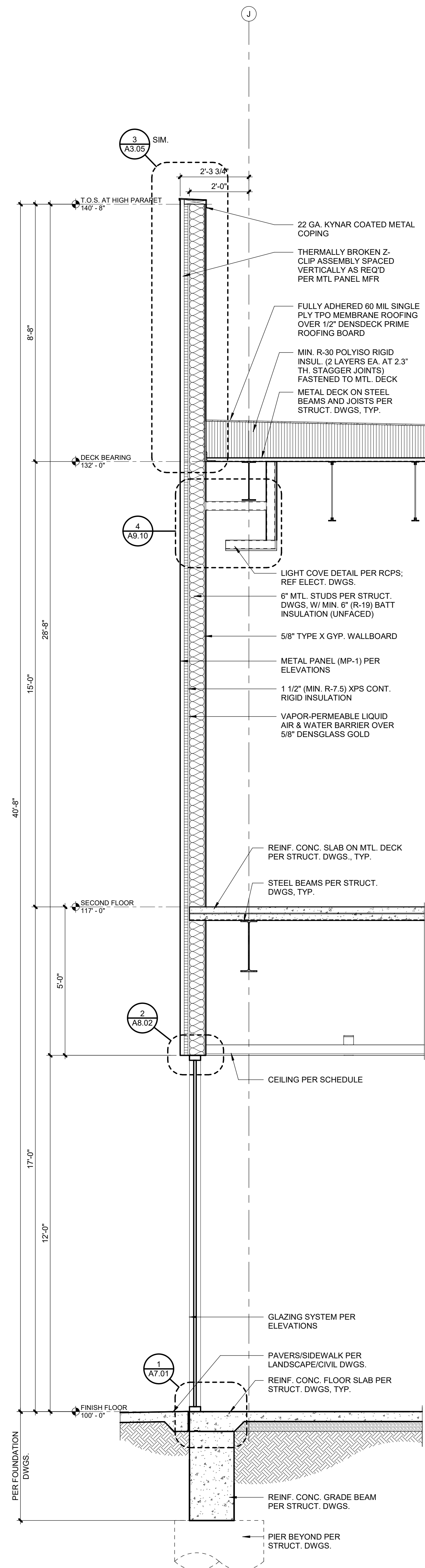
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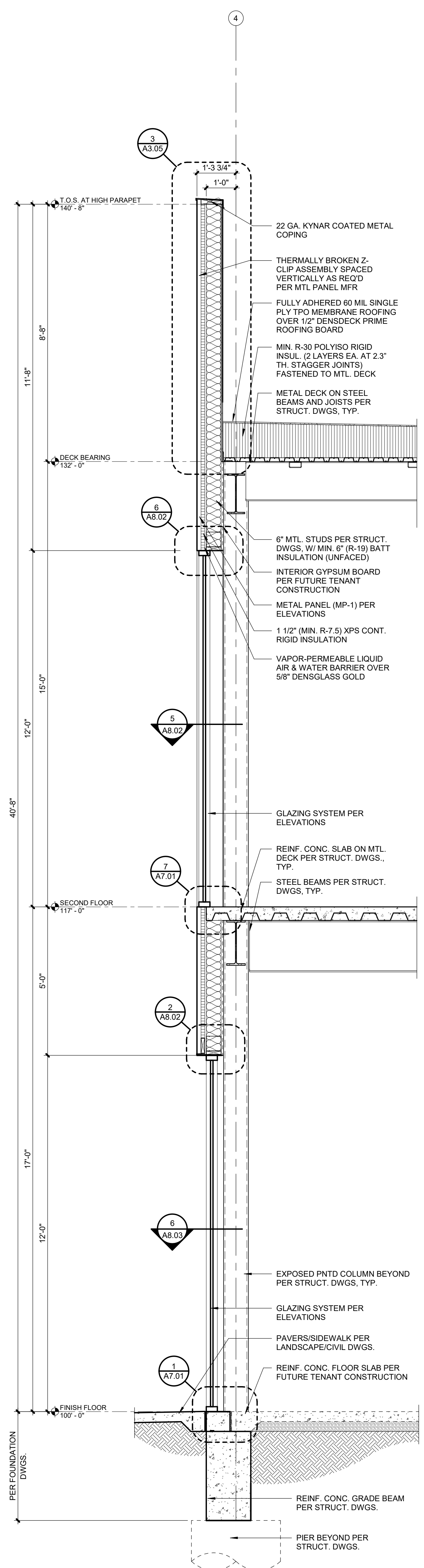
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A5.05	SCALE : 1/2" = 1'-0"



3	WALL SECTION
A5.05	SCALE : 1/2" = 1'-0"



2	WALL SECTION
A5.05	SCALE : 1/2" = 1'-0"



1	WALL SECTION
A5.05	SCALE : 1/2" = 1'-0"

PARAGON STAR
BLDG 2 / LOT 9

3201 NW PARAGON PKWY
LEE'S SUMMIT, MO

Project No.: 19050.01a
Date: 06.02.22
Issued For: ADDENDUM 1

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PROJECT TEAM

ARCHITECT	FINKLE+WILLIAMS ARCHITECTURE
CIVIL	GBA
LANDSCAPE	LAND 3
FOUNDATIONS	BSE STRUCTURAL ENGINEERS
STRUCTURAL	BSE STRUCTURAL ENGINEERS
PLUMBING	HENDERSON ENGINEERS
MECHANICAL	HENDERSON ENGINEERS
ELECTRICAL	HENDERSON ENGINEERS
FIRE PROTECTION	HENDERSON ENGINEERS
CONTRACTOR	FOGEL-ANDERSON



FINKLE + WILLIAMS
ARCHITECTURE

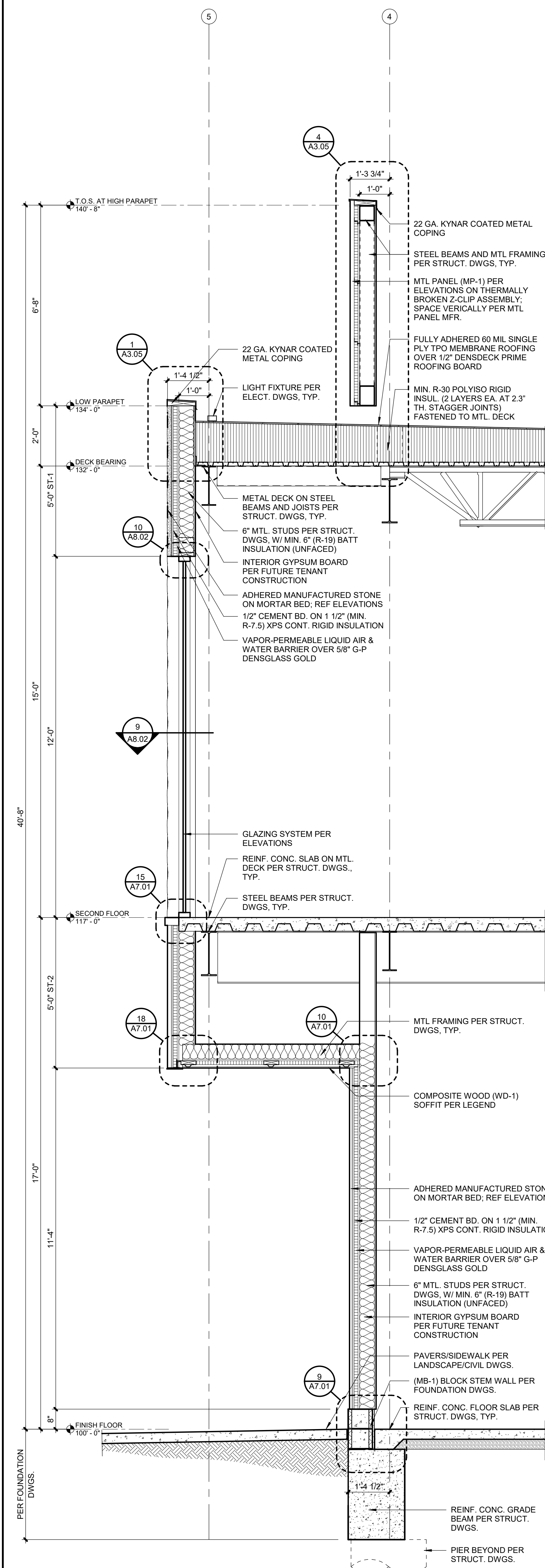
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SHEET TITLE

WALL SECTIONS

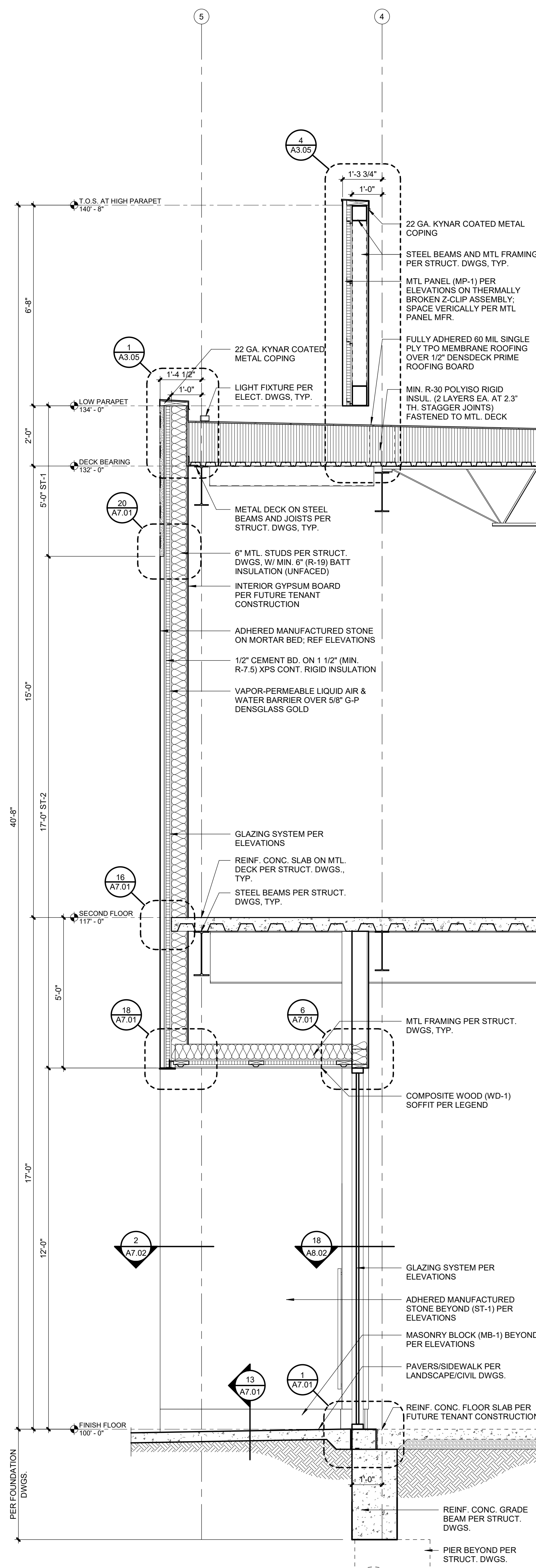
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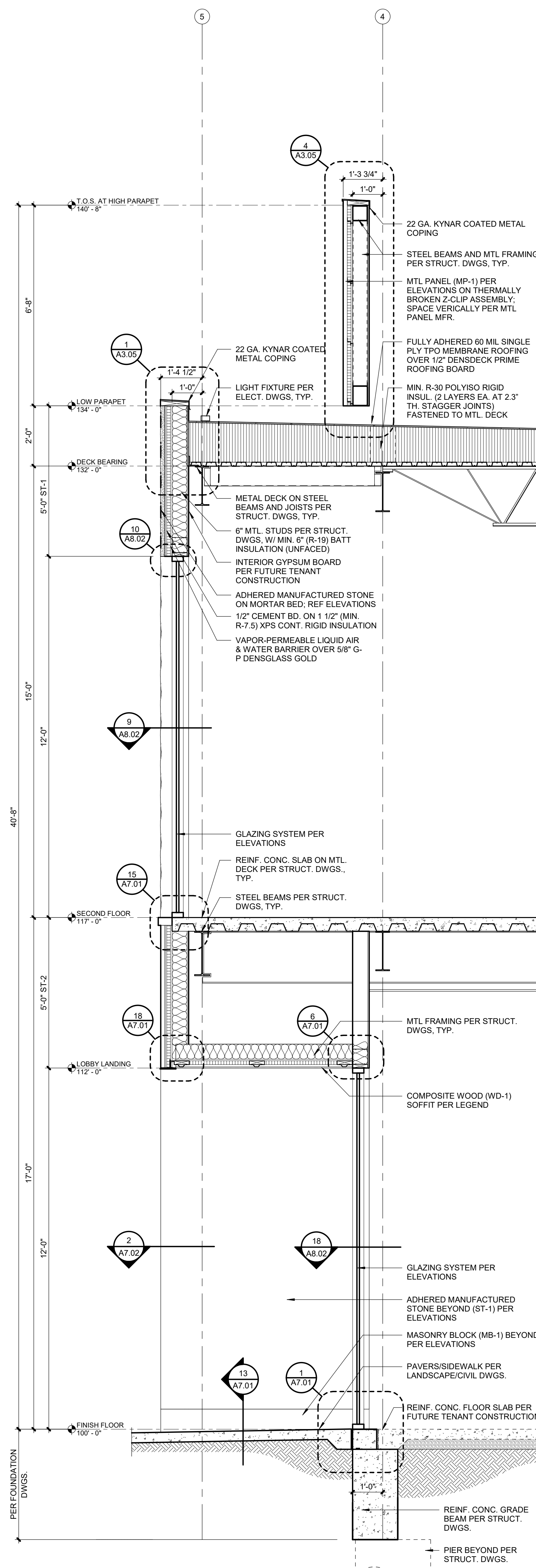
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A5.06 SCALE: 1/2" = 1'-0"



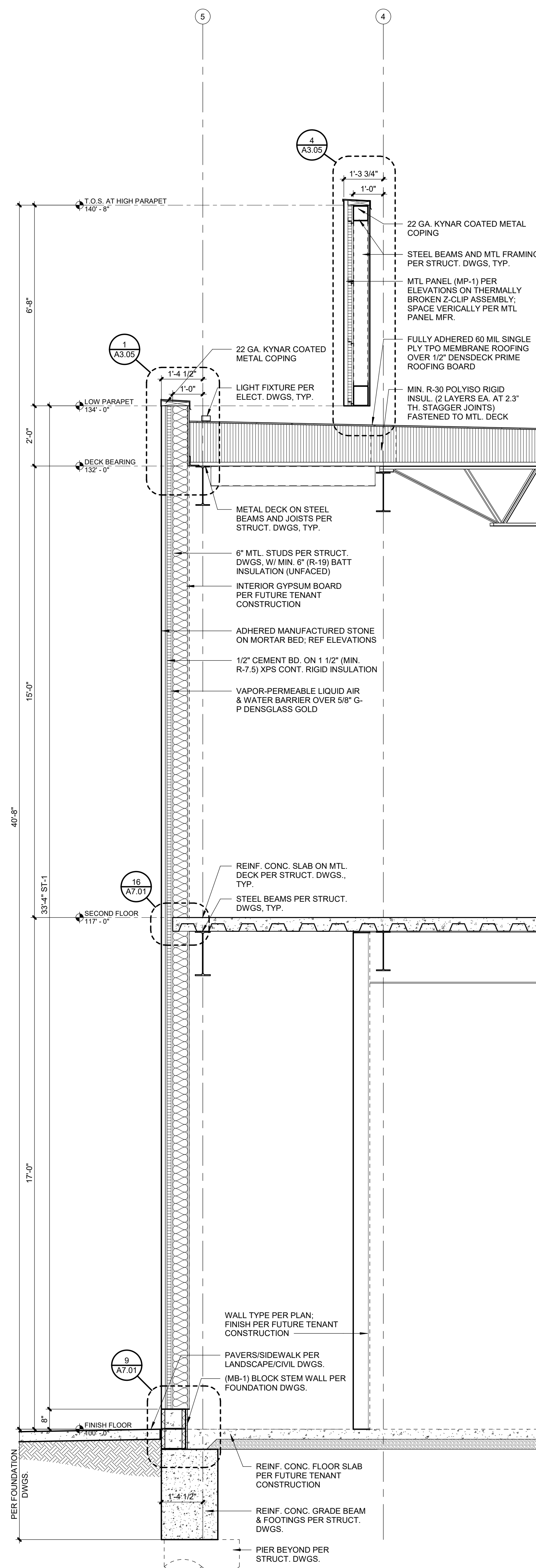
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A5.06 SCALE: 1/2" = 1'-0"



2 WALL SECTION

A5.06 SCALE: 1/2" = 1'-0"



1 WALL SECTION

A5.06 SCALE: 1/2" = 1'-0"

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05.06.22

PROJECT TEAM

ARCHITECT	FINKLE+WILLIAMS ARCHITECTURE
CIVIL	GBA
LANDSCAPE	LAND 3
FOUNDATIONS	BSE STRUCTURAL ENGINEERS
STRUCTURAL	BSE STRUCTURAL ENGINEERS
PLUMBING	HENDERSON ENGINEERS
MECHANICAL	HENDERSON ENGINEERS
ELECTRICAL	HENDERSON ENGINEERS
FIRE PROTECTION	HENDERSON ENGINEERS
CONTRACTOR	FOGEL-ANDERSON



FINKLE + WILLIAMS
ARCHITECTURE

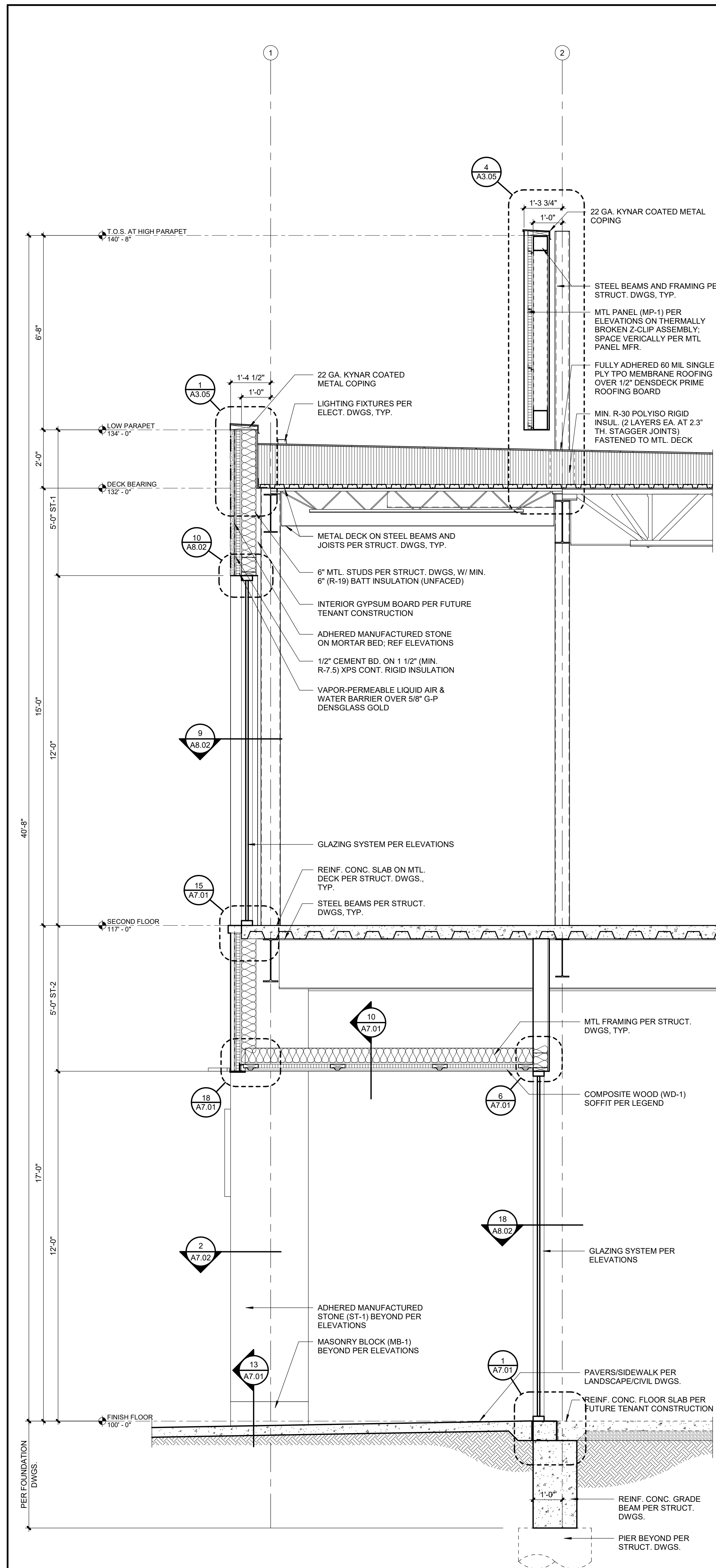
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SHEET TITLE

WALL SECTIONS

SHEET NUMBER

A5.07



REVISIONS		
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05.06.22

PROJECT TEAM

ARCHITECT	FINKLE+WILLIAMS ARCHITECTURE
CIVIL	GBA
LANDSCAPE	LAND 3
FOUNDATIONS	BSE STRUCTURAL ENGINEERS
STRUCTURAL	BSE STRUCTRAL ENGINEERS
PLUMBING	HENDERSON ENGINEERS
MECHANICAL	HENDERSON ENGINEERS
ELECTRICAL	HENDERSON ENGINEERS
FIRE PROTECTION	HENDERSON ENGINEERS
CONTRACTOR	FOGEL-ANDERSON



FINKLE + WILLIAMS
ARCHITECTURE

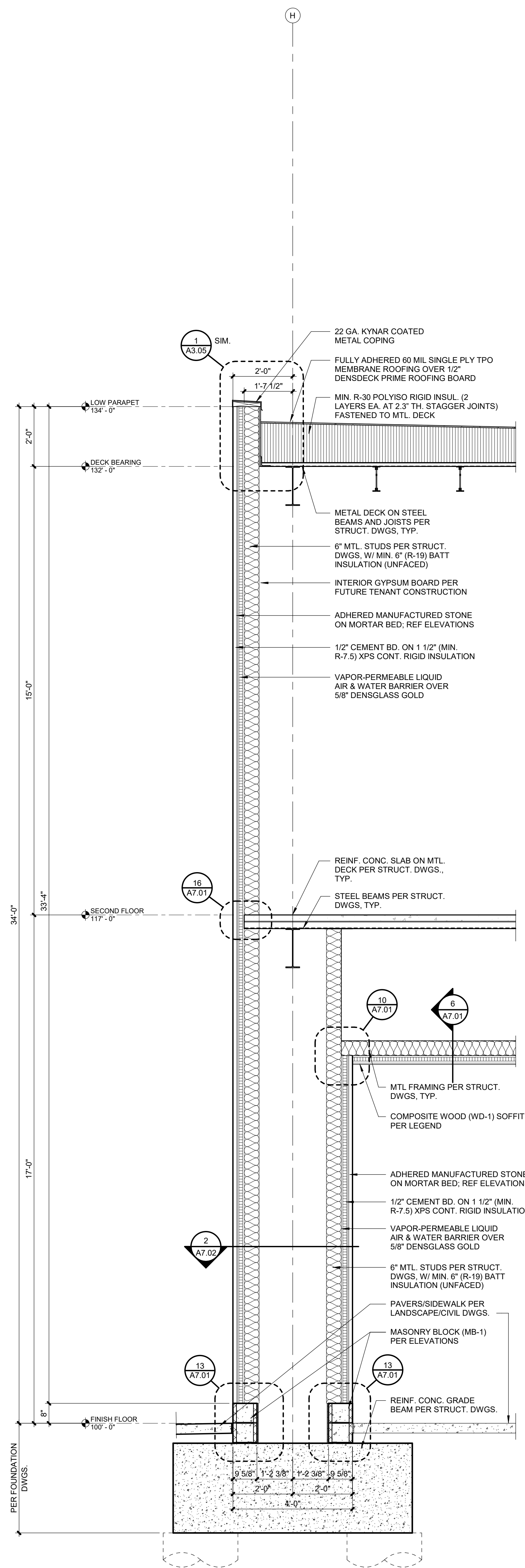
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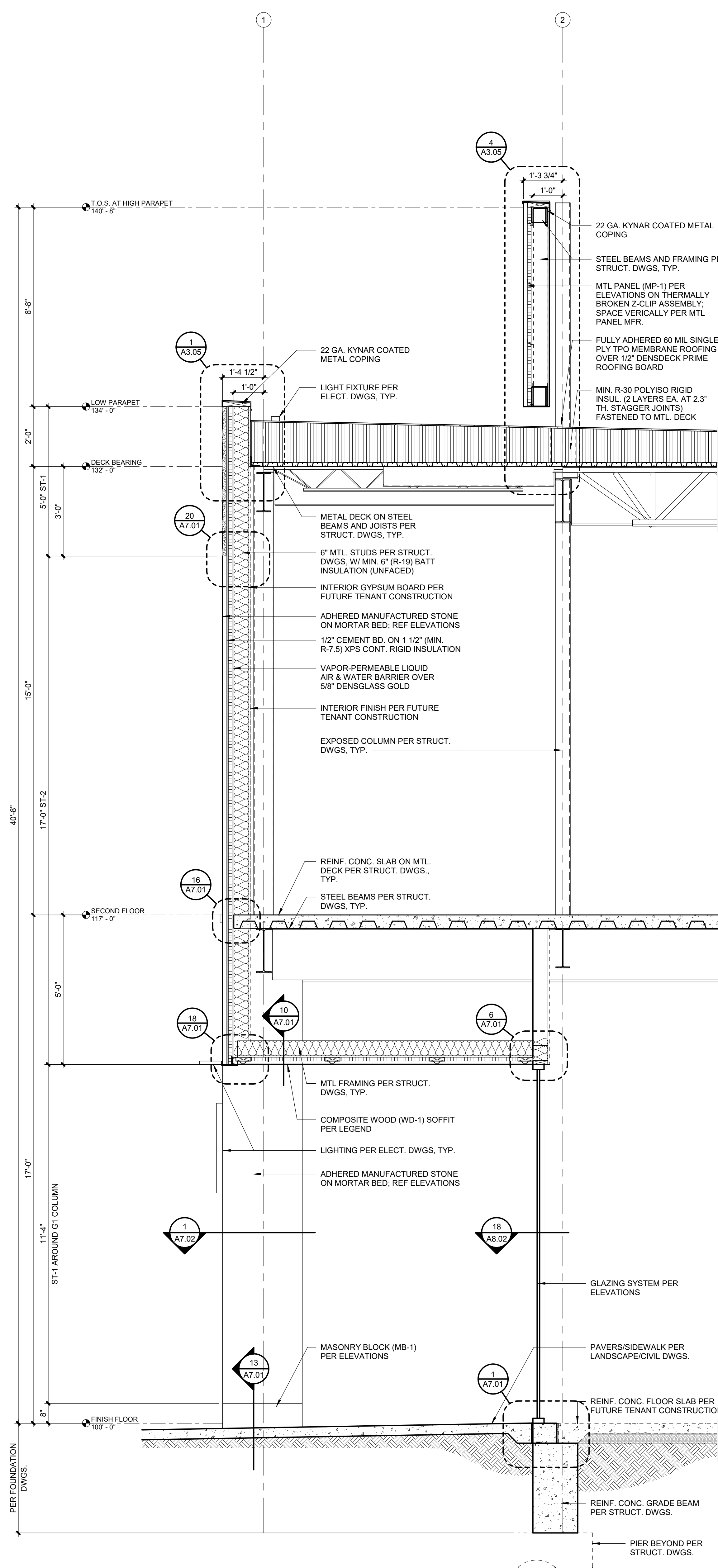
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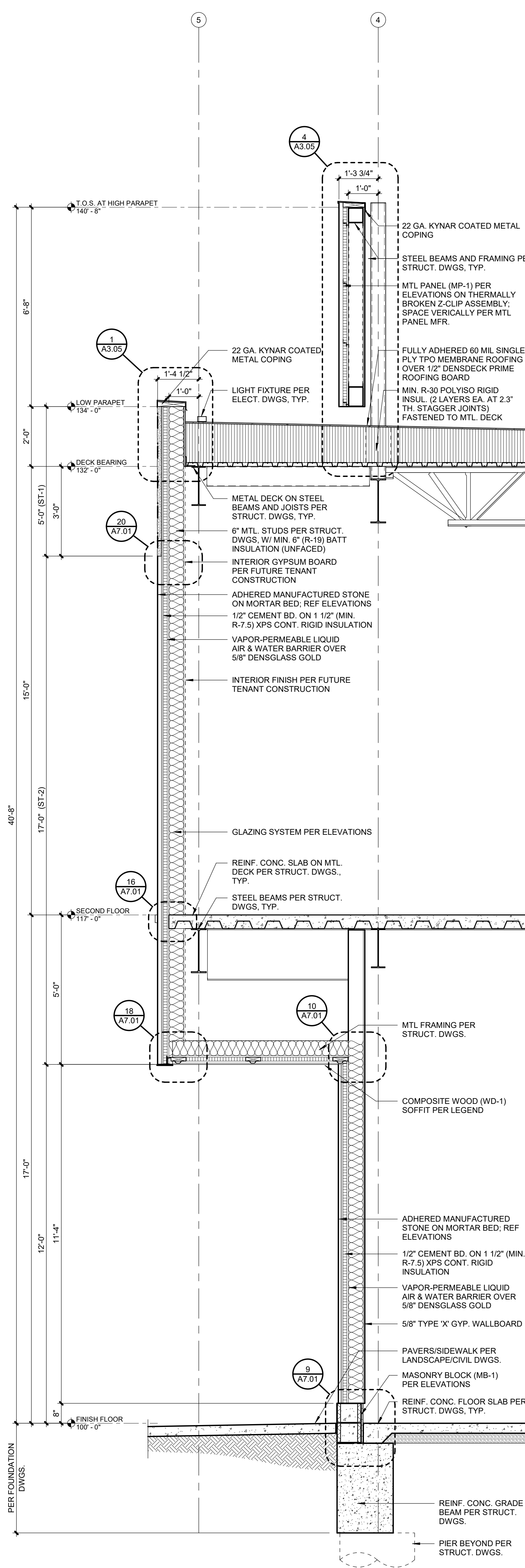
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A5.08 SCALE: 1/2" = 1'-0"



2 WALL SECTION

A5.08 SCALE: 1/2" = 1'-0"



1 WALL SECTION

A5.08 SCALE: 1/2" = 1'-0"

REVISIONS		
No.	Date	Description

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PROJECT TEAM

ARCHITECT	FINKLE+WILLIAMS ARCHITECTURE
CIVIL	GBA
LANDSCAPE	LAND 3
FOUNDATIONS	BSE STRUCTURAL ENGINEERS
STRUCTURAL	BSE STRUCTURAL ENGINEERS
PLUMBING	HENDERSON ENGINEERS
MECHANICAL	HENDERSON ENGINEERS
ELECTRICAL	HENDERSON ENGINEERS
FIRE PROTECTION	HENDERSON ENGINEERS
CONTRACTOR	FOGEL-ANDERSON



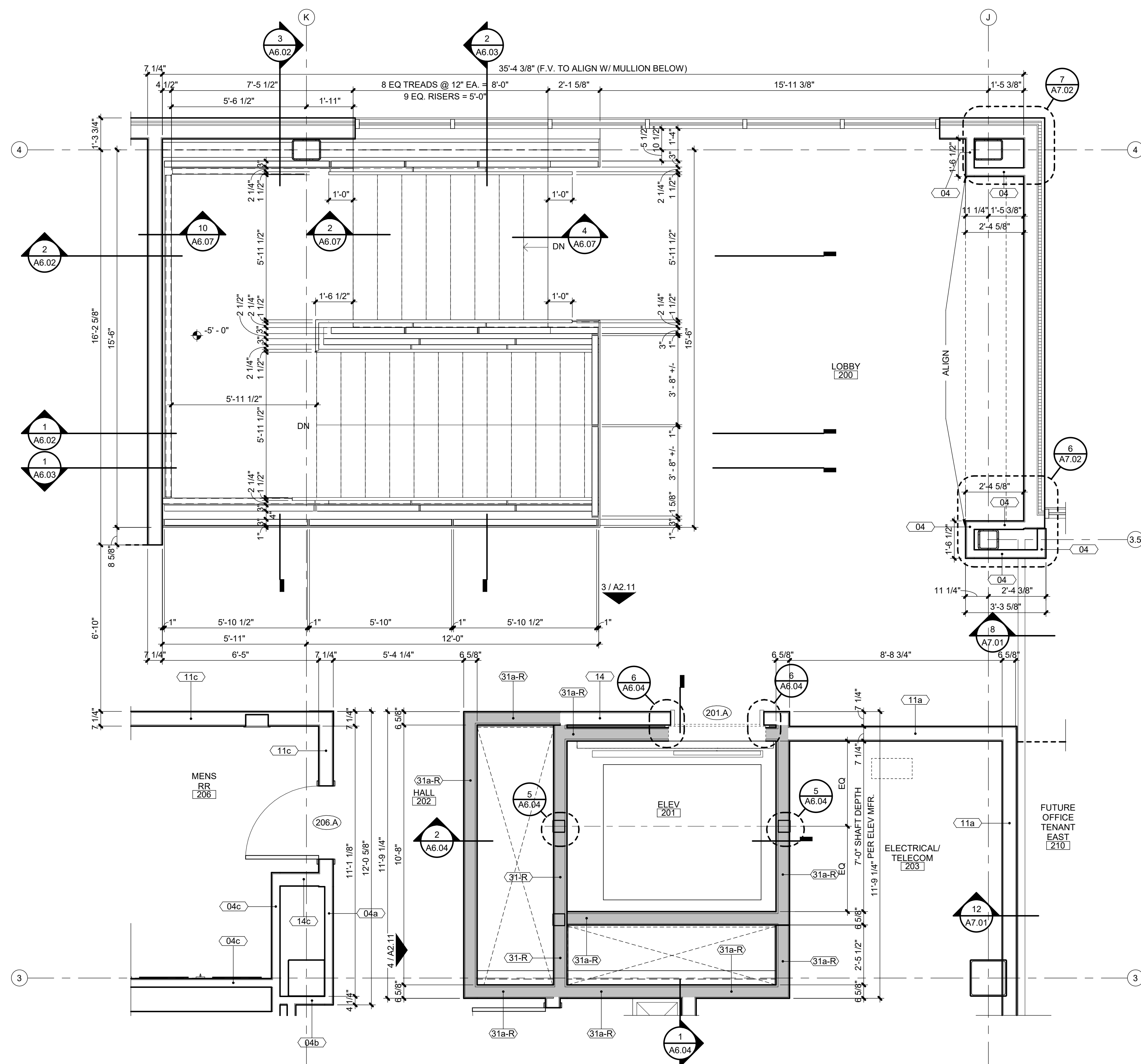
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SHEET TITLE

VERTICAL
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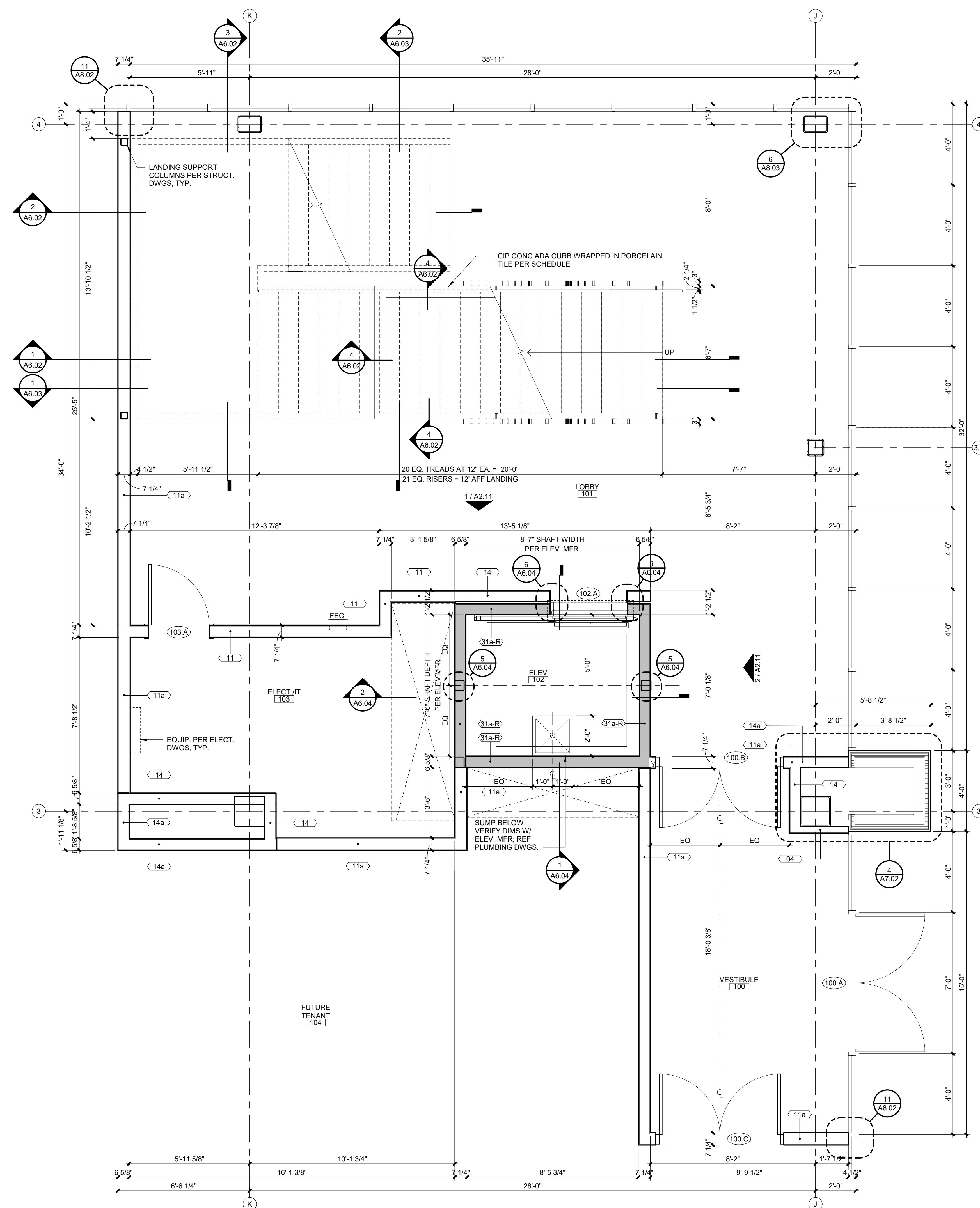
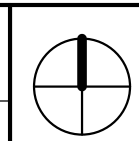
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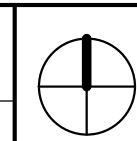
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A6.01 SCALE: 3/8\" = 1'-0"

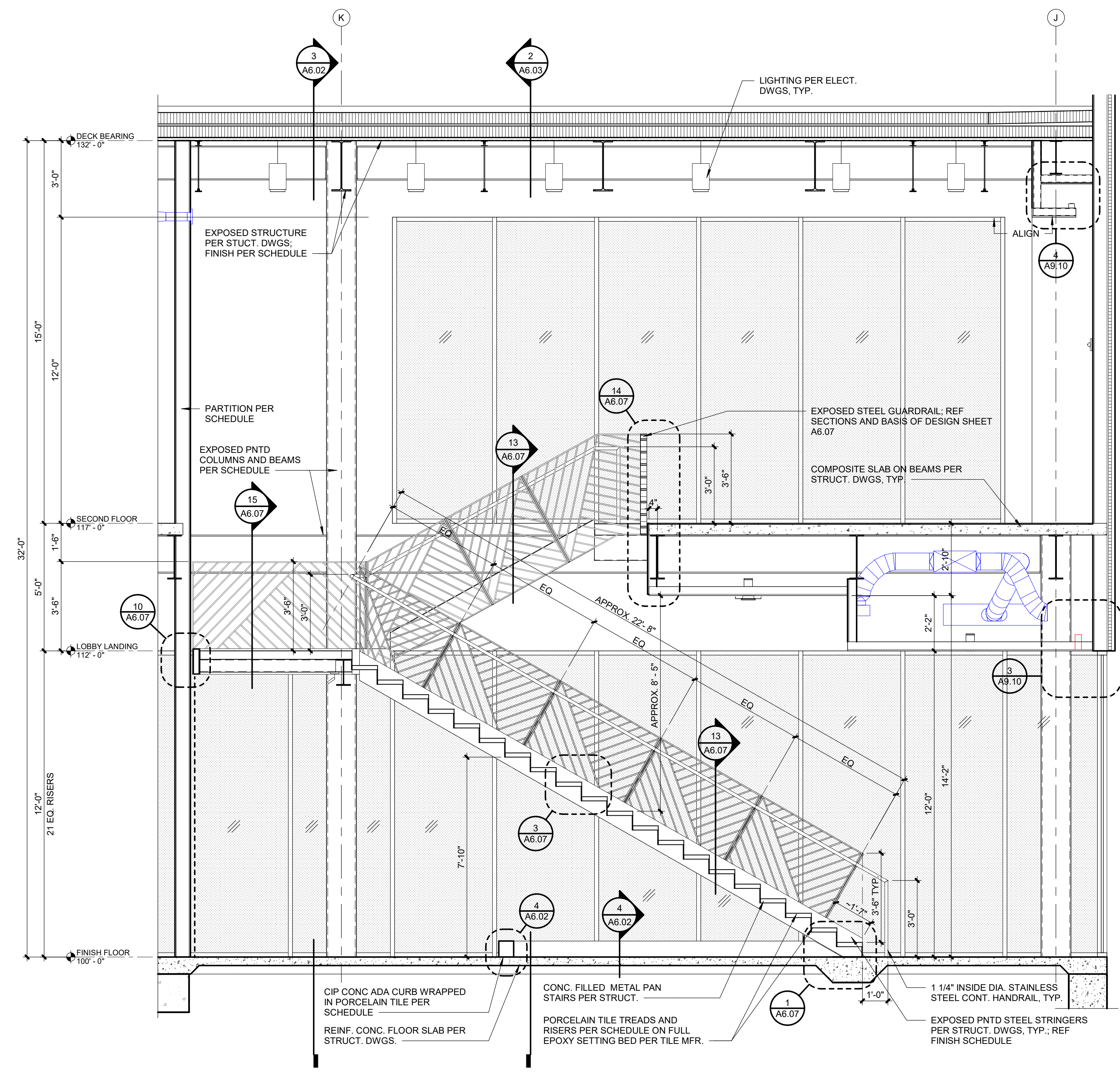


1 ENLARGED STAIR PLAN - FIRST LEVEL

A6.01 SCALE: 3/8\" = 1'-0"



A6.02	SCALE : 3" = 1'-0"
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A6.02	SCALE : 3/8" = 1'-0"
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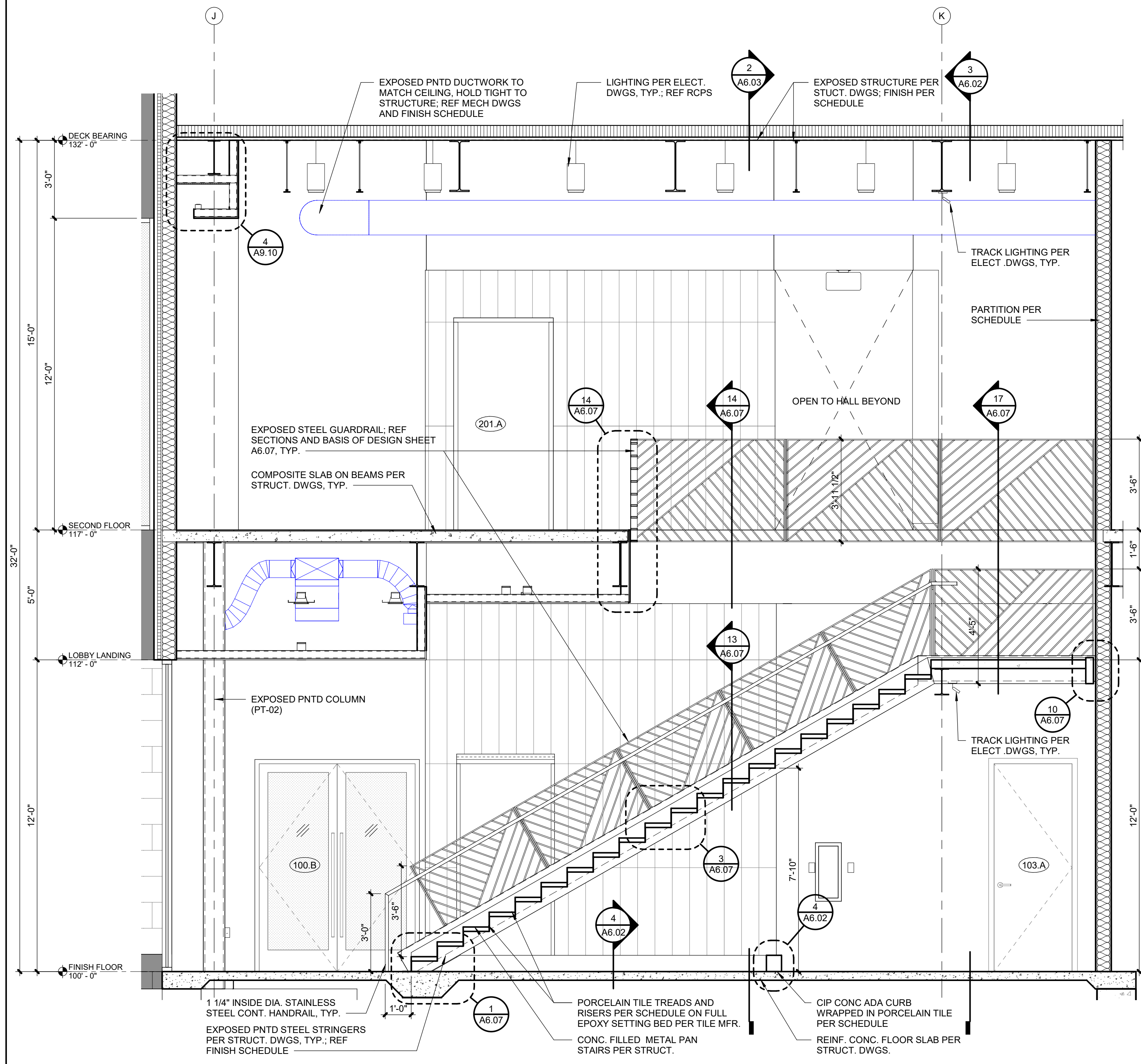
A6.02	SCALE : 3/8" = 1'-0"
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A6.02	SCALE : 3/8" = 1'-0"
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**THESE IMAGES ARE INTENDED FOR REFERENCE ONLY; MAY NOT ACCURATELY REFLECT CURRENT DESIGN. DISCUSS ANY DISCREPANCIES WITH ARCHITECT AND OWNER PRIOR TO FABRICATION*

A6.03



REVISIONS		
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REGISTRATION



05.06.22

PROJECT TEAM

ARCHITECT	FINKLE+WILLIAMS ARCHITECTURE
CIVIL	GBA
LANDSCAPE	LAND 3
FOUNDATIONS	BSE STRUCTURAL ENGINEERS
STRUCTURAL	BSE STRUCTURAL ENGINEERS
PLUMBING	HENDERSON ENGINEERS
MECHANICAL	HENDERSON ENGINEERS
ELECTRICAL	HENDERSON ENGINEERS
FIRE PROTECTION	HENDERSON ENGINEERS
CONTRACTOR	FOGEL-ANDERSON



FINKLE + WILLIAMS
ARCHITECTURE

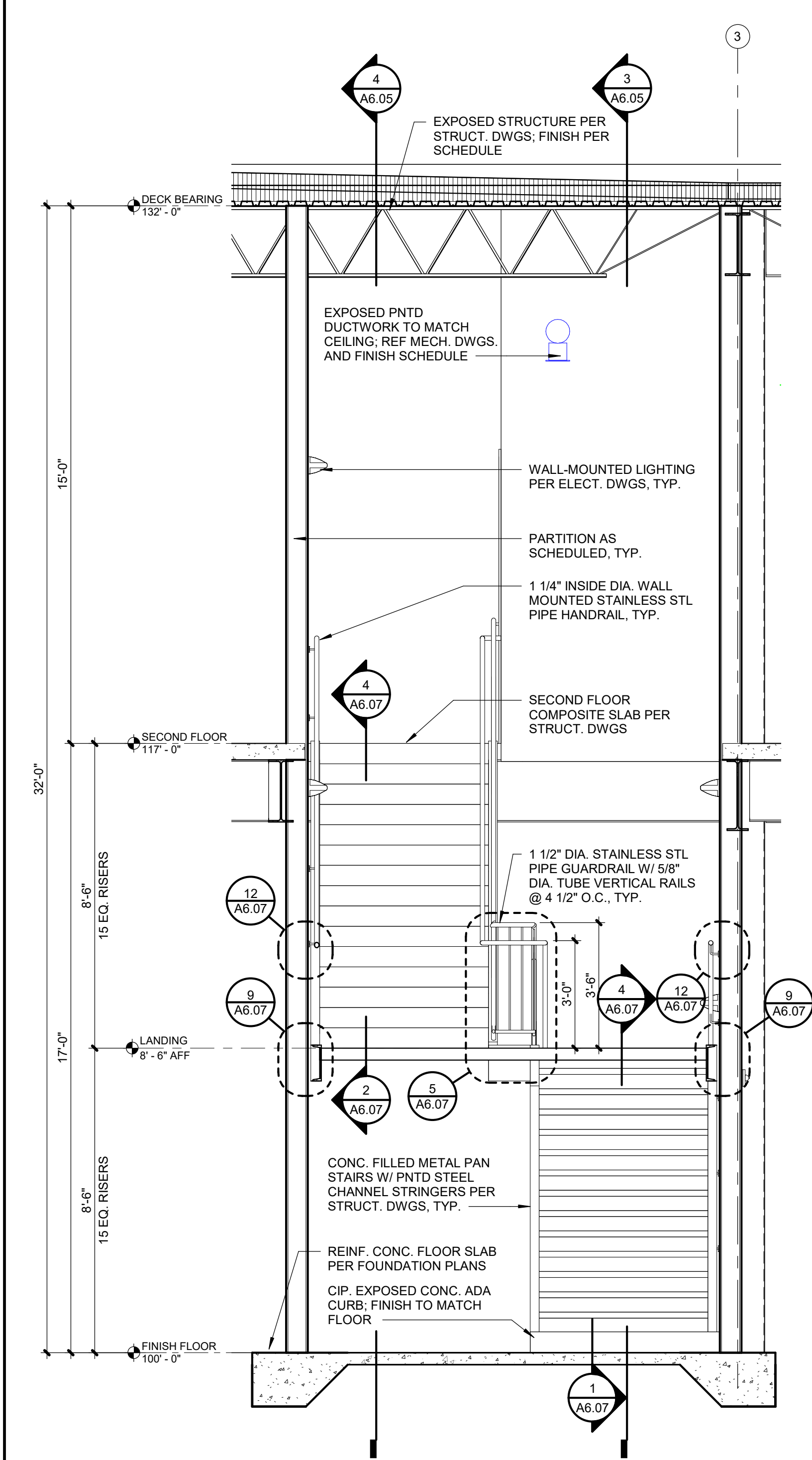
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SHEET TITLE

VERTICAL
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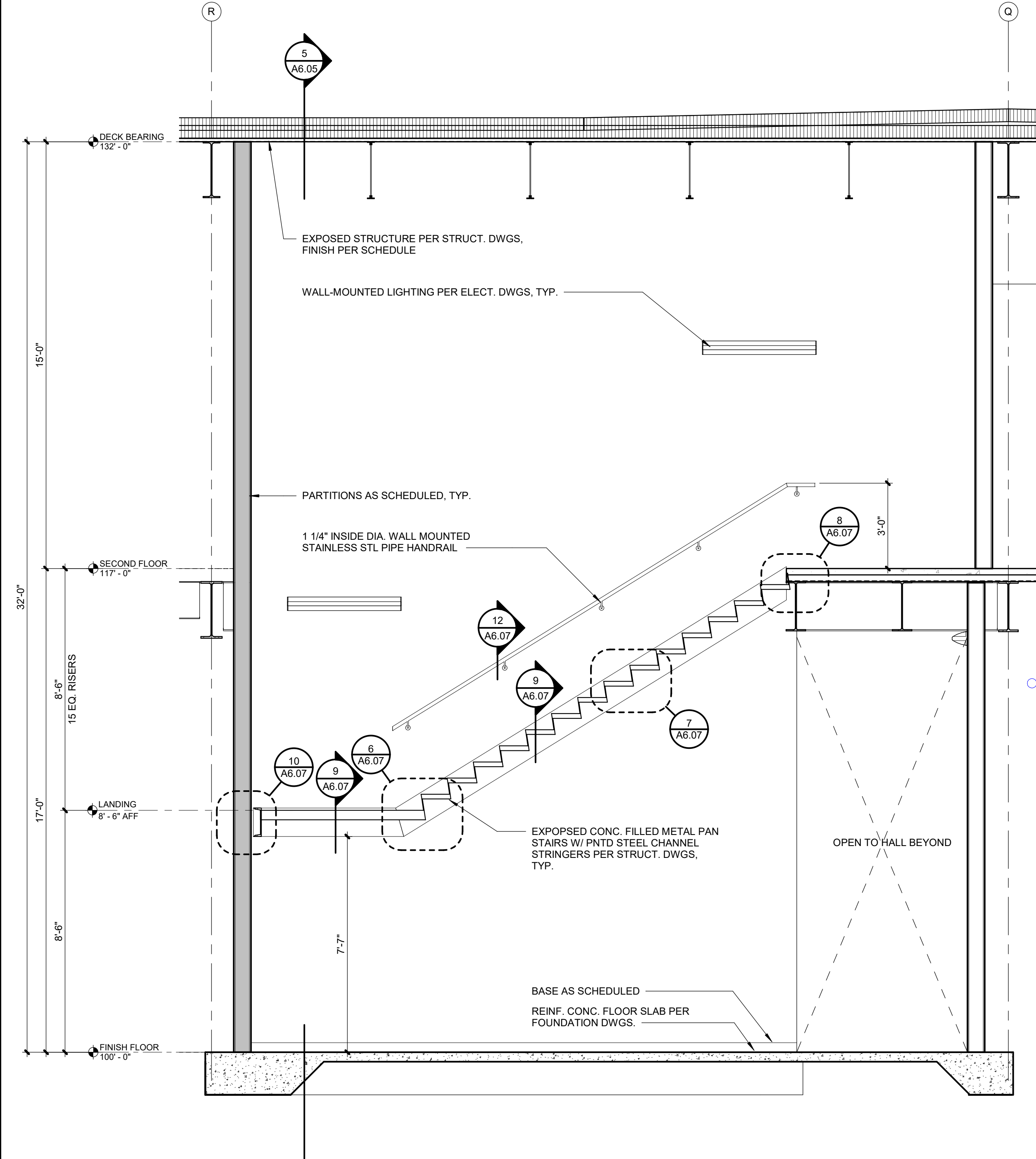
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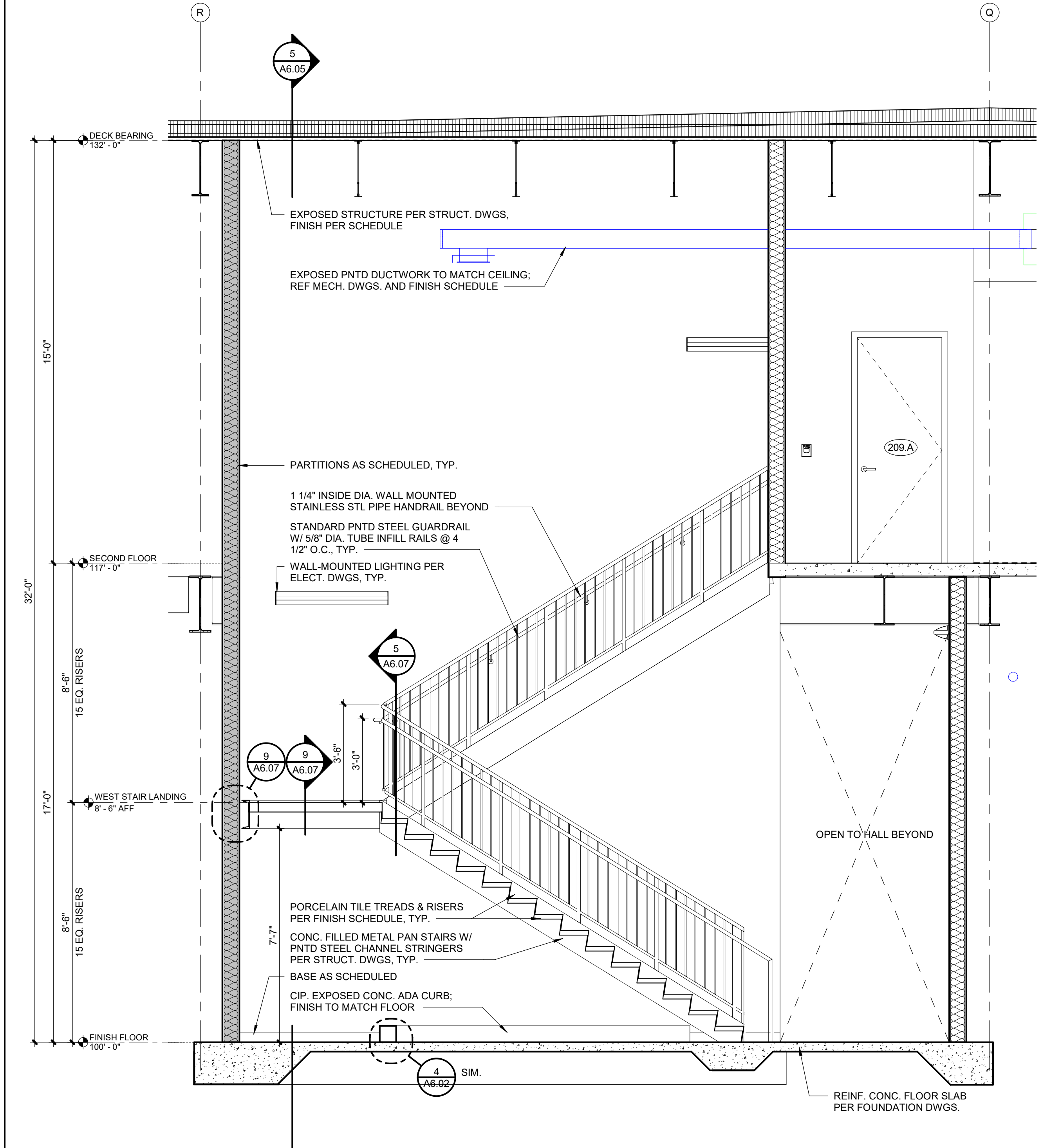
5 STAIR SECTION

A6.05 SCALE: 3/8" = 1'-0"



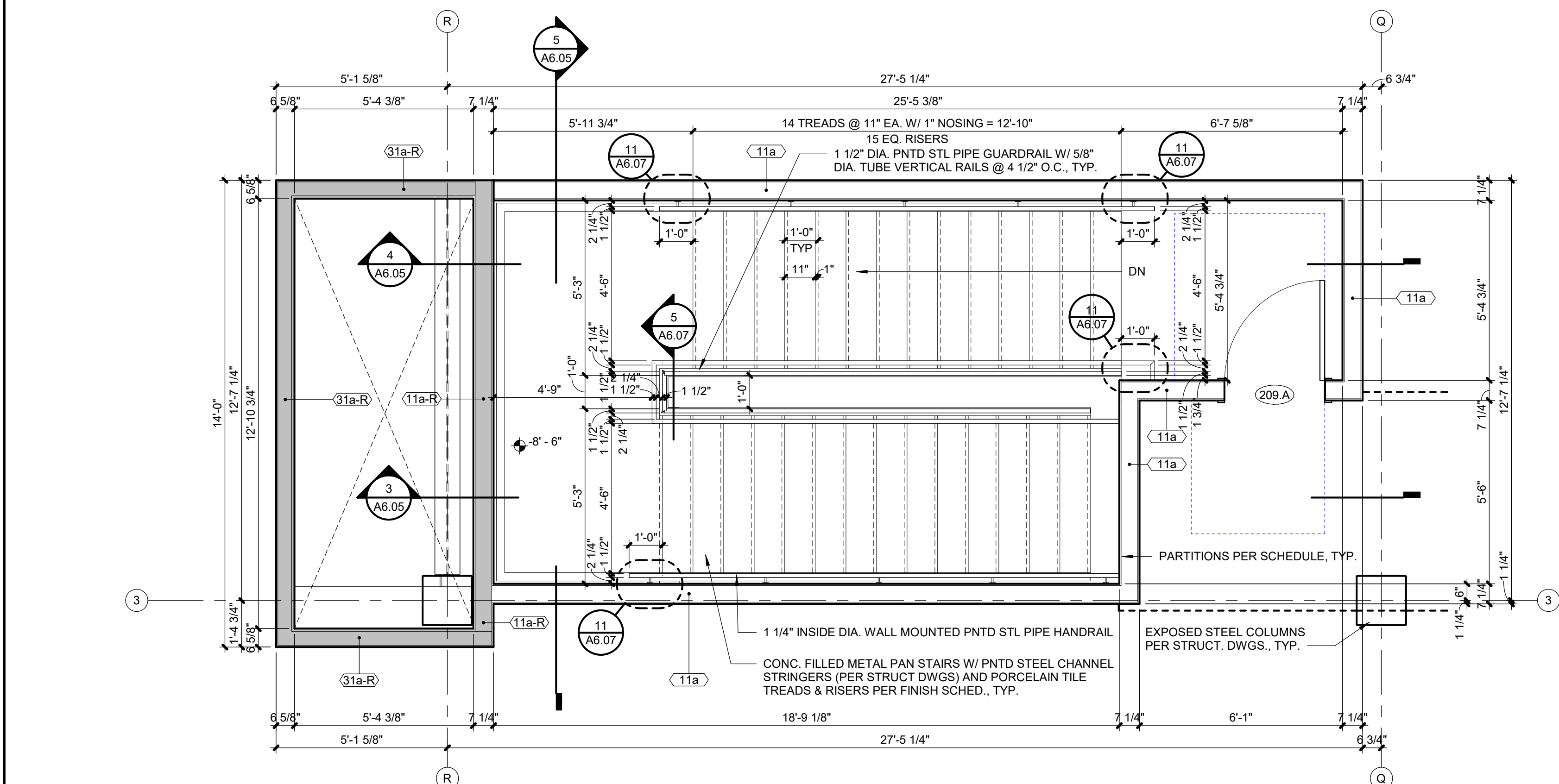
4 STAIR SECTION

A6.05 SCALE: 3/8" = 1'-0"



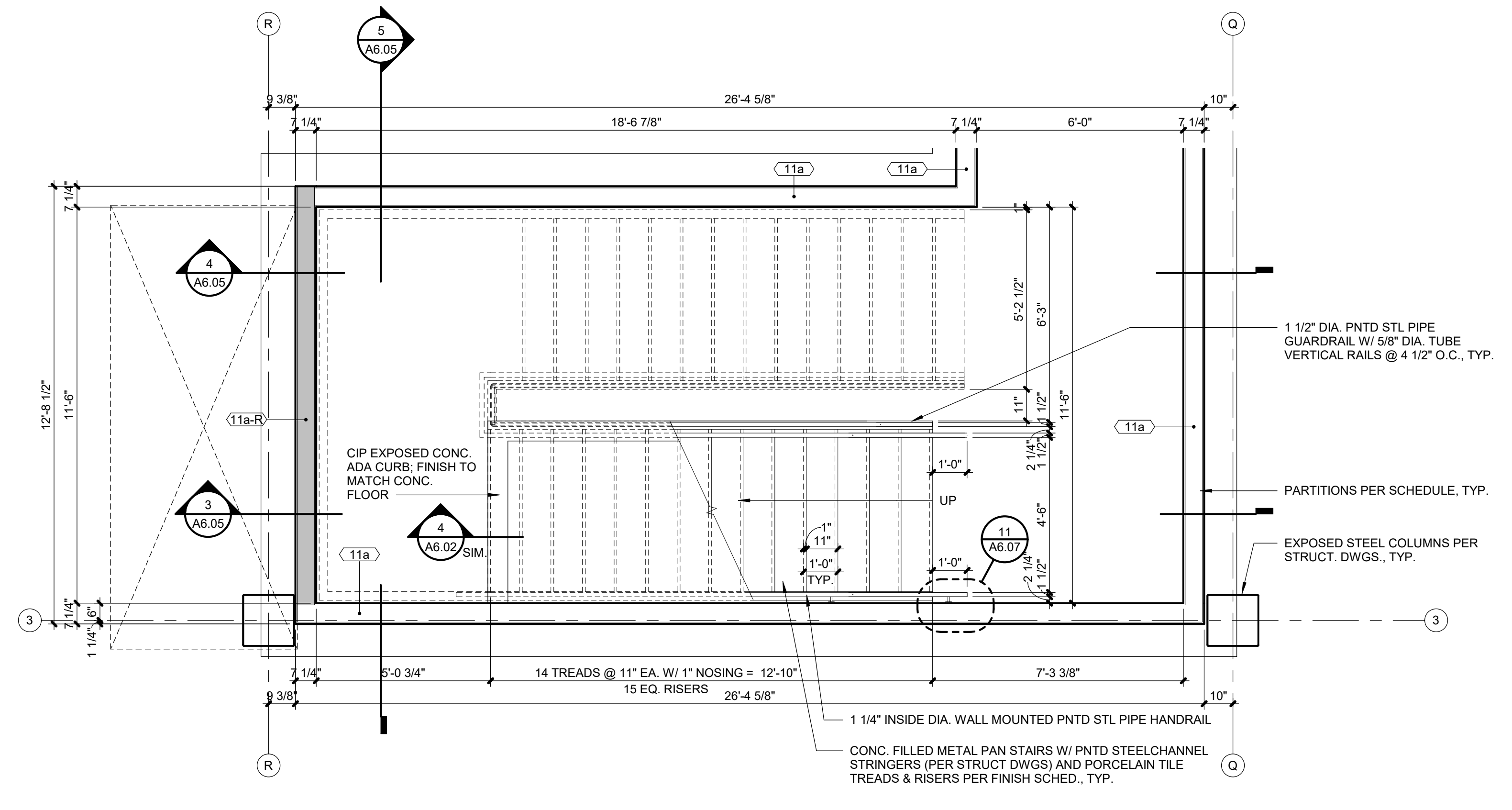
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A6.05 SCALE: 3/8" = 1'-0"



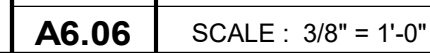
2 WEST STAIR 2ND FLOOR PLAN

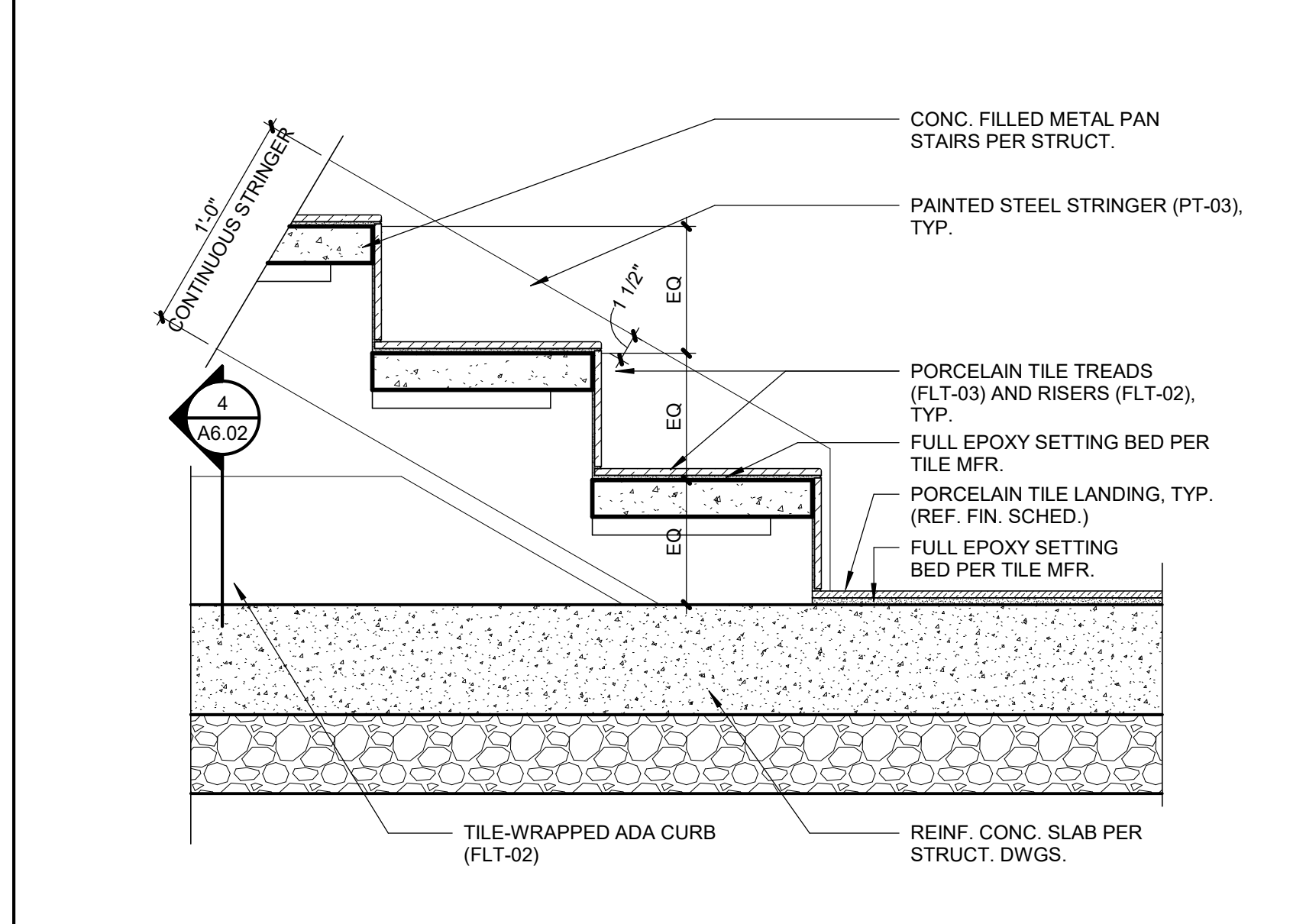
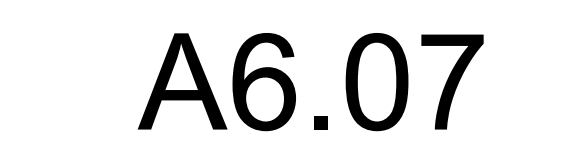
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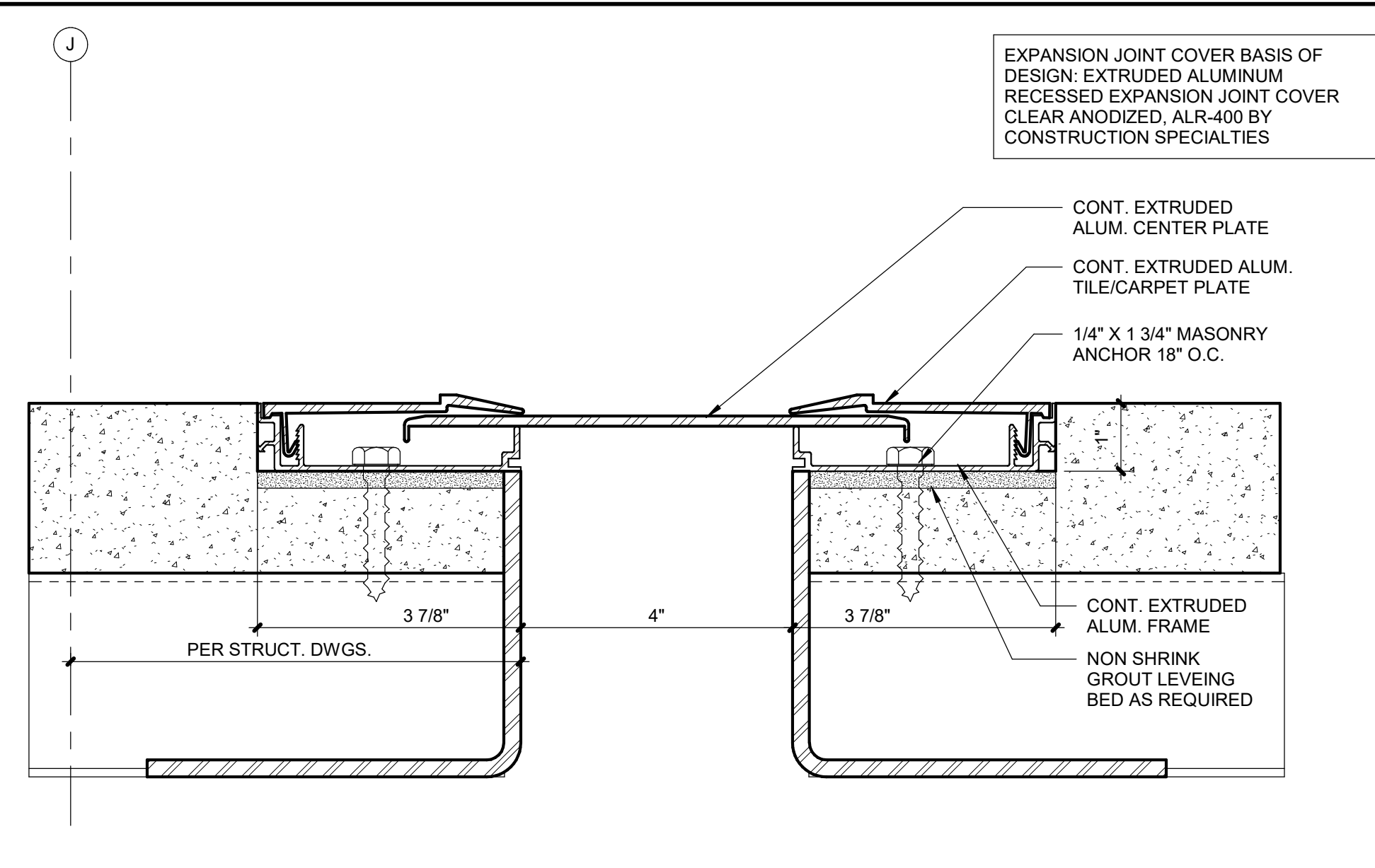


1 WEST STAIR FIRST FLOOR PLAN

A6.05 SCALE: 3/8" = 1'-0"

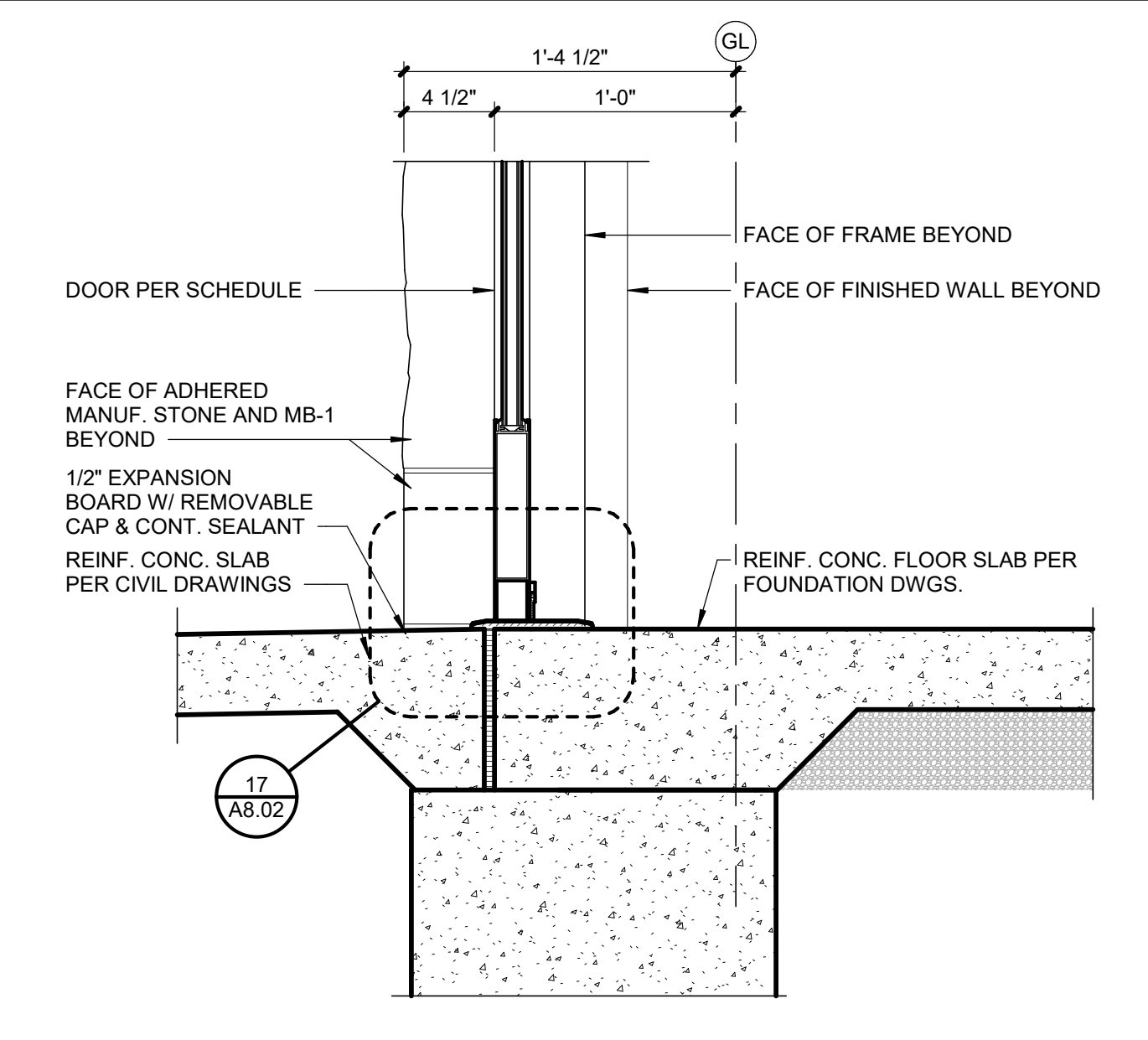






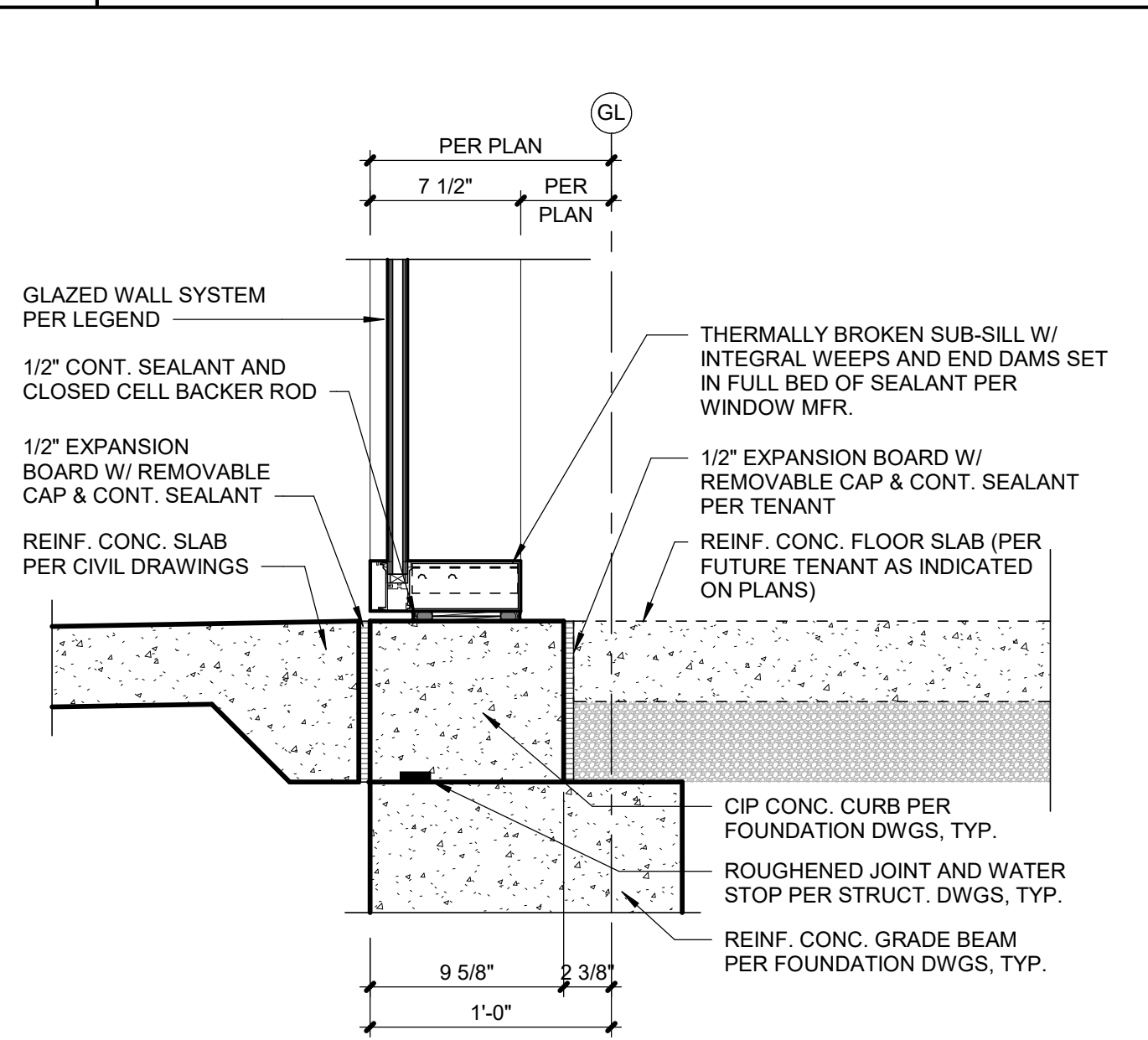
EXP. JOINT FLOOR TO FLOOR

SCALE : 6" = 1'-0"



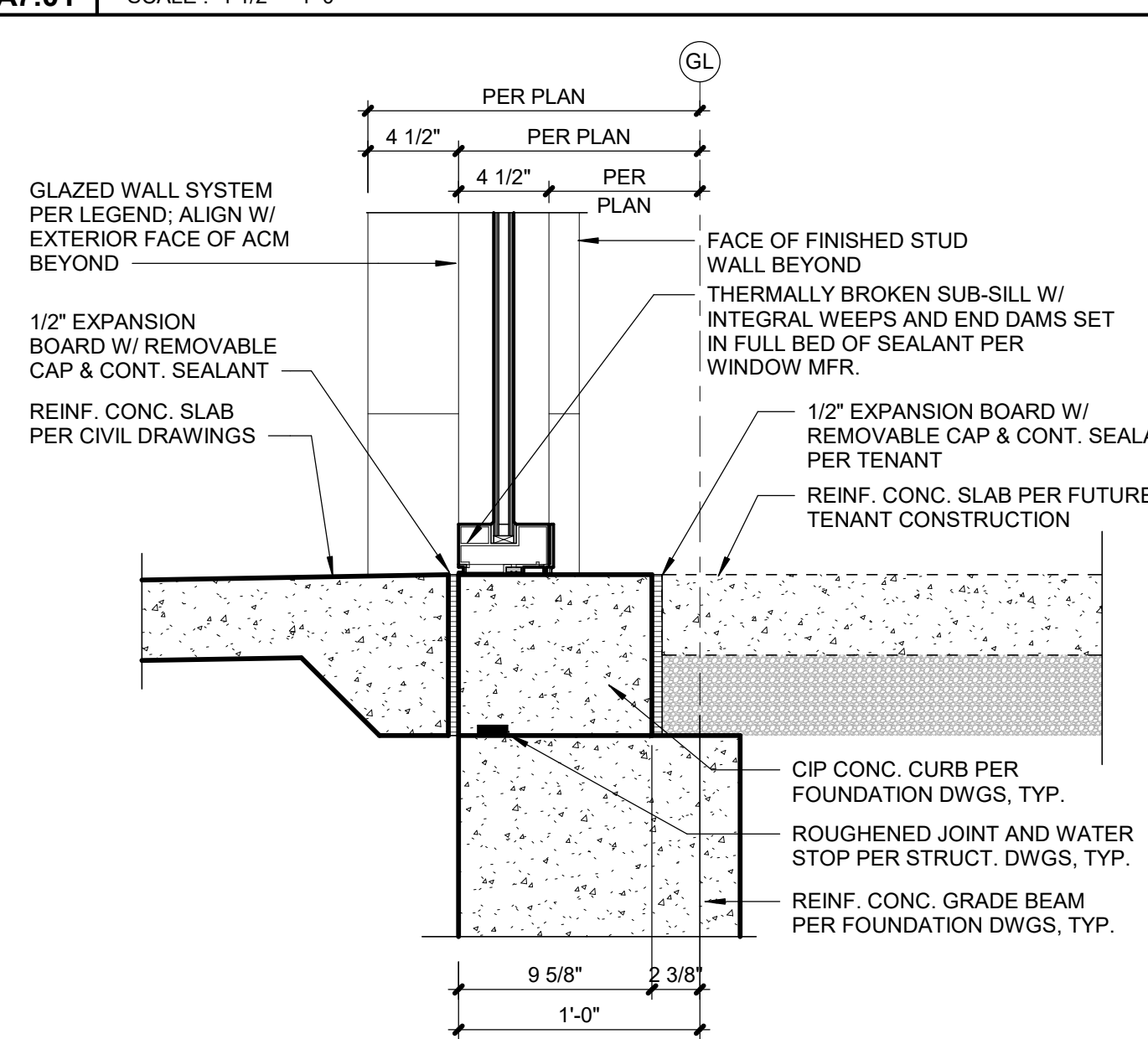
3	FOUNDATION DETAIL
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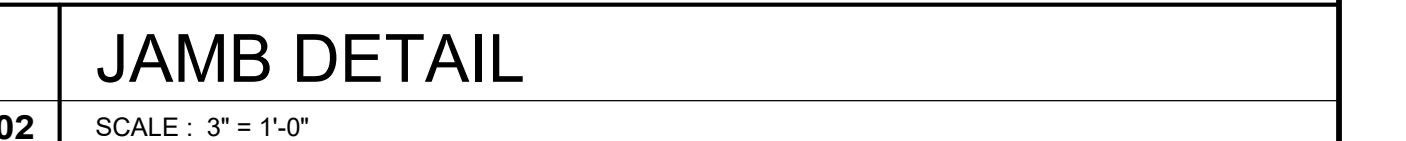
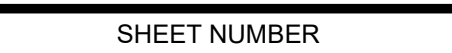
2	FOUNDATION DETAIL
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A7.01	SCALE: 1 1/2" = 1'-0"
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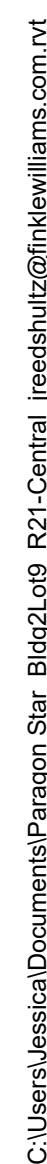


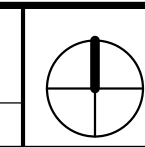
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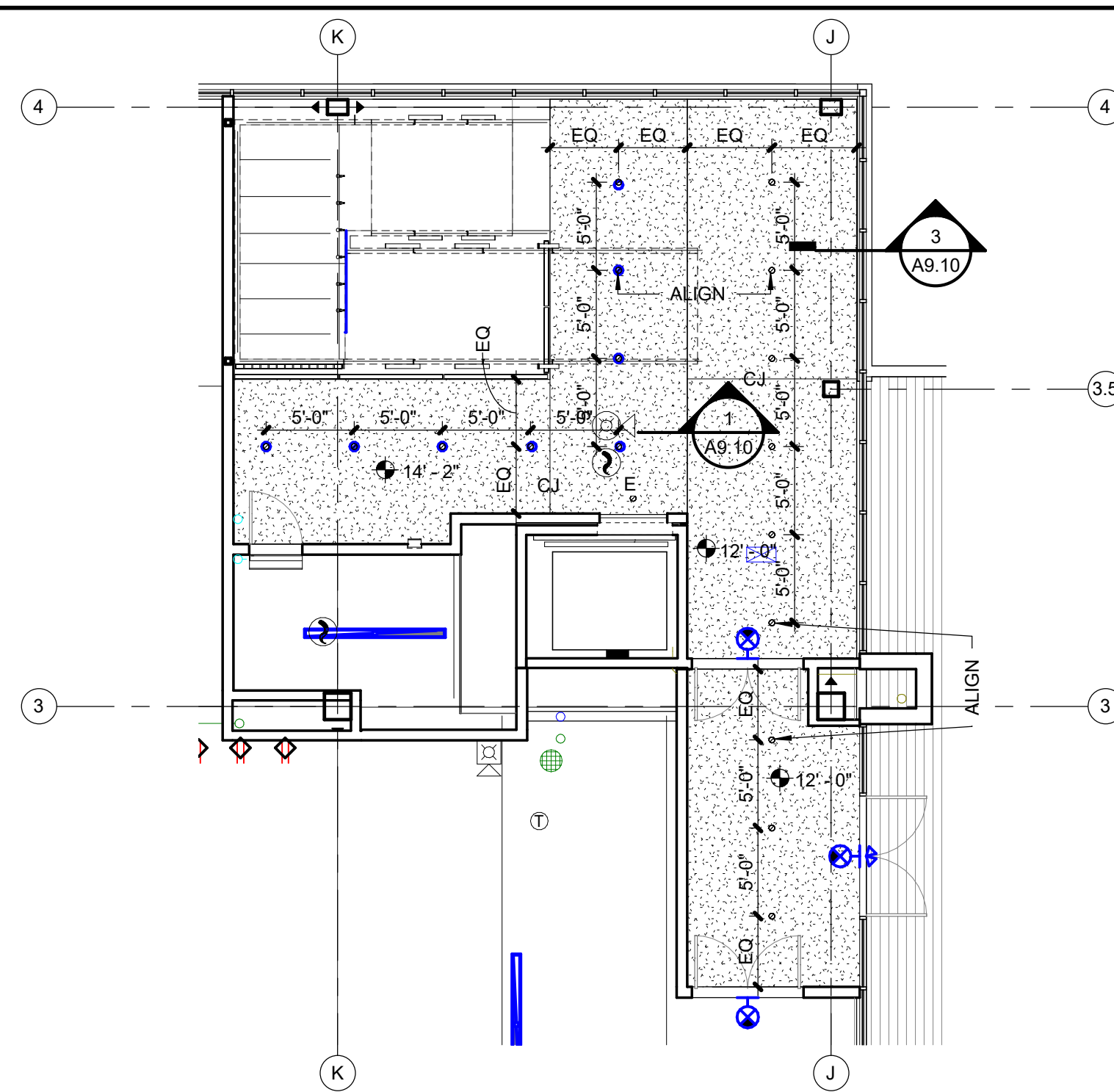
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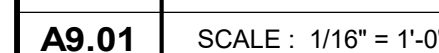
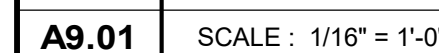
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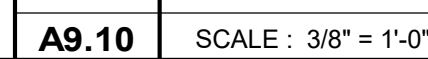
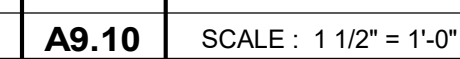


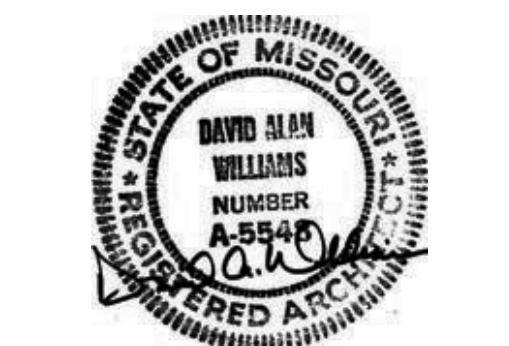




A9.01	SCALE : 1/8" = 1'-0"
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PROJECT TEAM

ARCHITECT	FINKLE+WILLIAMS ARCHITECTURE
CIVIL	GBA
LANDSCAPE	LAND 3
FOUNDATIONS	BSE STRUCTURAL ENGINEERS
STRUCTURAL	BSE STRUCTURAL ENGINEERS
PLUMBING	HENDERSON ENGINEERS
MECHANICAL	HENDERSON ENGINEERS
ELECTRICAL	HENDERSON ENGINEERS
FIRE PROTECTION	HENDERSON ENGINEERS
CONTRACTOR	FOGEL-ANDERSON



SHEET TITLE

PROJECT SPECIFICATIONS

SHEET NUMBER

A11.12

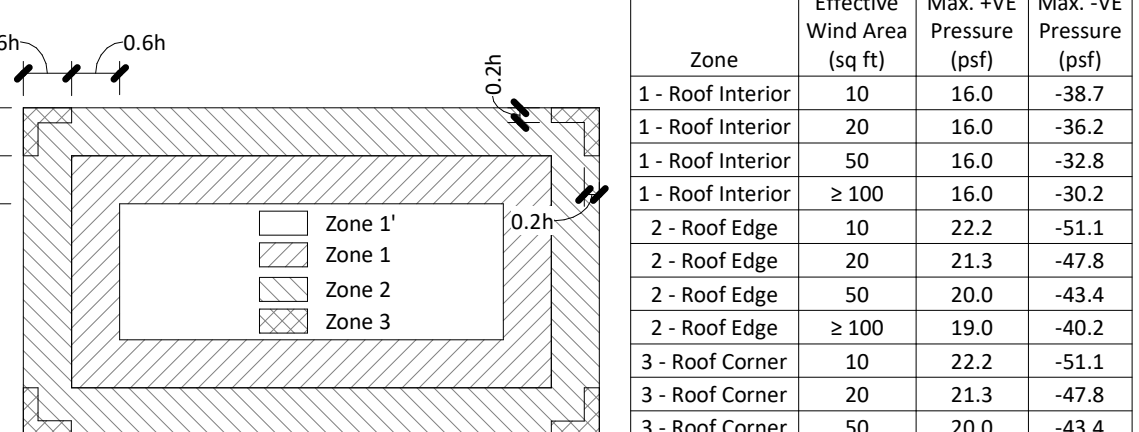
Design Specifications:

- ASCE 7-16
- ACI 318-16
- ACI 530-16
- AISI C300-15
- AISI S100-16
- ANSI / AWC NDS-18

Roof Loads:	Wind Loads:	Seismic Loads:
• Dead Load: 20 psf	• Occupancy: III	• I _{ms} : 1.25
• Live Load: 20 psf	• Velocity: 117 mph	• S _s : 0.088 g
• D _o : 0	• Exposure: B	• S ₁ : 0.068 g
M.E.P. Zone:	• Iw: 1.0	• Site Class: D
• Dead Load: 65 psf	• Snow Loads:	• S _{DS} : 0.105 g
• Floor Loads:	• Pg: 20 psf	• S ₁ : 0.109 g
• Dead Load: 65 psf	• PF: 22 psf	• Seismic Design Category: B
• Office Live: 80 psf	• C _e : 0.9	• Seismic Force-Resisting System: S.O.M.F.
	• I _s : 1.1	• Design Base Shear: C.W.
	• C _s : 1.0	• Cs: 0.0373
	• Analysis Procedure Used: E.L.F.P.	

Design Load Notes:

- Dead load shown includes collateral load of 4 psf and solar load of 6 psf.
- See components and cladding table for design wind pressures.
- See net uplift diagram for roof framing due to wind pressures.



Components & Cladding Wind Zone Diagram

- The components & cladding (C&C) wind pressures shown assume a mean roof height of 32'-0" above finished floor elevation. All components shall be designed to resist the provided pressures, which shall be clearly defined on all shop drawings. Refer to wind zone diagram for zone locations. Plus and minus signs signify pressures acting toward and away from surfaces, respectively.
- The components & cladding wind zone diagram is generalized to show all possible conditions. The diagram shape may not match the specific layout of this project.
- a = 17'-6"
- Internal Pressure Coefficient = +0.18

General:

- The structural systems shown on these documents have been designed for the final, in place usage of the structure based on the intended occupancy and code requirements. While general constructability has been considered, the structural systems have not been designed to accommodate specific construction methods and methods that might be utilized by the Contractor.
- The Contractor shall field verify all existing dimensions prior to fabrication.
- The Contractor shall notify the Engineer of any observed discrepancies in dimensions, detailing, or other items as shown on the plans or specified prior to proceeding with work relating to said discrepancies.
- The Contractor shall not alter or modify work shown on the structural drawings without receiving written approval from the Engineer.
- The Contractor shall be responsible for supplying shop drawings for joist girders, bar joists, structural steel, metal deck, reinforcing steel and concrete mix designs. Shop drawings must be reviewed for conformance with the means, methods, techniques, sequences, and operations of construction, and safety precautions and programs indicated thereto, all of which are the sole responsibility of the Contractor, and shall be stamped "approved" by the Contractor prior to submittal. Shop drawings submitted without the Contractor's stamped approval will be returned "rejected". All shop drawings shall be reviewed by the Structural Engineer prior to construction.
- See architectural, mechanical, and electrical drawings for other pertinent information related to the structural work and coordinate as required. These structural drawings are intended to be included in a complete set of construction documents, including but not limited to, architectural drawings, civil drawings, and mechanical/electrical/plumbing drawings. Contractor shall verify coordination of these drawings with contents of above drawing sets specified and only proceed with bidding and construction after such has taken place.
- The building and the independent structural components shown in these documents are not structurally stable until all connections, framing, shear walls, diaphragms, permanent bracing, metal decking, interior and exterior concrete slabs on grade, and exterior or interior load-bearing walls are complete and have achieved their design strength. Contractor is solely responsible for maintaining structural stability during erection and construction. Temporary bracing systems shall remain in place until all structural work is complete.
- The Contractor is responsible for verifying all existing dimensions and conditions of the existing building and reporting discrepancies from the assumed conditions shown on the structural drawings to the Engineer of record prior to fabrication and erection of any member.
- The Contractor shall coordinate the roof drainage system with the Architect as required to ensure that no more than 3 1/2" of water can accumulate before entering an overflow drainage system.

Structural Engineer Site Observations:

- The contract structural drawings & specifications represent the finished structure, and, except where specifically shown, do not indicate the method or means of construction. The Contractor shall supervise and direct the work and shall be solely responsible for all construction means, methods, procedures, techniques, and sequence.
- The Engineer shall not have control or charge of and shall not be responsible for construction means, methods, techniques, sequences, or procedures, for safety precautions & programs in connection with the work, for the acts or omission of the Contractor, subcontractor, or any other persons performing any of the work, or for the failure of any of them to carry out the work in accordance with the contract documents.
- Periodic site observation by field representatives of BSE Structural Engineers LLC is solely for the purpose of determining if the work of the Contractor is proceeding in general accordance with the structural contract documents. This limited site observation should not be construed as exhaustive or continuous to check the quality or quantity of work, but rather periodic in an effort to guard the Client against defects or deficiencies in the work of the Contractor.

Slab On Grade:

- Welded wire fabric shall be supplied in sheets only. Rolls will not be permitted. (As required on construction documents.)
- Welded wire fabric shall be supported on chairs or blocks prior to concrete placement. Mesh shall not be hooked and pulled up during concrete placement. (As required on construction documents.)
- Welded wire fabric shall have end and edge laps of one full mesh plus 2" between cross wires. Wire all laps securely together.
- Welded wire fabric shall conform to ASTM A1064.
- Floor finish requirements: Slab-on-grade shall be finished to overall floor flatness, overall floor levelness, local floor flatness, and local floor levelness requirements as defined by the Owner. Coordinate requirements as required with G.C. prior to slab-on-grade placement. Floor finish requirements to be determined in accordance with ASTM E 1155.

Foundations:

- Foundations for this project have been designed in accordance with requirements set forth in a geotechnical addendum prepared by Terracon Consultants (Project #02259318), ACP pile foundations dated June 28, 2021. This is an addendum to geotechnical report (Project #022195181) Drilled Shafts dated August 2, 2019). Augered, cast in place (ACP) piles have been designed for an allowable soil bearing value of 40,000 psf. The Contractor shall refer to the Geotechnical Report for all requirements and recommendations pertinent to this project.
- Anchor rods shall conform to ASTM F1554 Gr. 36 (U.N.O.) and shall be located by means of a template. Provide a not above and below template to assure proper vertical alignment.
- All foundations shall be square and level.
- Grout shall be dry and stiff to prevent shrinkage, with a minimum compressive strength of 4000 psi. Grout below column base plates and precast panels as required. Thoroughly compact grout beneath base plates.

Concrete and Reinforcing Steel:

- Concrete mix designs shall meet the following requirements:

Location	Minimum Compressive Strength (psi)	Max. Aggregate Size	Water/Cement Ratio	Slump (in.)	Air Entrainment (%)
Interior Slabs	4000	3/4"	0.50	4 ± 1	0
Exterior Slabs	3500	3/4"	0.50	4 ± 1	6 ± 1
Interior Foundations	3000	1"	0.50	4 ± 1	0
Perimeter Foundations	3000	1"	0.50	4 ± 1	6 ± 1
Exterior Walls & Prestalls	4000	3/4"	0.50	4 ± 1	6 ± 1
Composite Floor Slab	4000	1/2"	0.48	4 ± 1	0
Interior Pier Caps	5000	1"	0.50	4 ± 1	0

- Fly ash shall not be used unless approved in writing by the Engineer. Fly ash, if approved, shall conform to ASTM C618 and ACI 232.2R-96. Fly ash shall be limited to types C & F and shall not exceed 15% of the total cement wt.
- The use of admixtures to increase the slump shall not be used unless approved in writing by the Engineer.
- All concrete is reinforced unless specifically called out as unreinforced. Reinforce all concrete not otherwise shown with same steel as in similar sections or areas.
- Construction joints in beam girders shall be at midspan unless noted otherwise. Reinforcing steel shall be continuous through construction joints unless noted otherwise.
- No aluminum items shall be embedded in any concrete or placed in contact with concrete.

- Reinforcing bars #4 and larger (except ties and stirrups) shall meet ASTM A615 with Supplementary Requirements (S1), Grade 60. Smaller bars shall be Grade 40.

- Concrete coverage of reinforcement shall have the following clear distances unless noted otherwise on the drawings:

Cast against earth: 3"

Formed concrete exposed to earth or weather: 2"

Not exposed to earth or weather: 1" Slabs, 1 1/2" Beams and columns

- Embedded and all reinforcing bars marked marionous shall be embedded to develop the full tensile capacity of the bars. Laps shall be Class B tension laps unless specified otherwise on the drawings. Unless shown otherwise, splice top bars near reinforcement and splice bottom bars over supports.

- Supply corner bars 4'-0" long (min. 2'-0" in each direction) in outside face of wall at corners of all walls and grade beams, matching size and spacing of horizontal bars. Where there are no vertical bars at corner of wall, supply three (3) #4 vertical support bars for corner bars.

- All bars are to be supported in forms and spaced with wire bar supports per ACI "Manual of Standard Practice for Detailing Concrete Structures" (latest edition). Bars shall be securely wired per the latest edition of CRSI's "Recommended Practice for Placing Reinforcing Bars". Accessories for exposed concrete shall be plastic or shall have plastic-tipped feet.

- Vertical control joints in walls shall be placed at 30'-0" maximum spacing unless noted otherwise. Locate joints beside piers monolithic with walls, near corners, and in concealed locations where possible. Construction joints may be placed in lieu of control joints at contractor's discretion. Coordinate location of control joints with Architect.

- Hot weather is defined as that combination of air temperature, concrete temperature, relative humidity and wind speed that will cause a rate of evaporation of 0.2 lb/sq.ft./hr. or more as defined by Figure 2.1.5 of ACI 308R.

- Do not add water to concrete during delivery, at Project Site, or during placement, unless approved by the Engineer.

- Provide 3/4" chamfer on all exposed corners unless noted otherwise on architectural or structural construction documents.

- All cold joints shall be roughened and cleaned unless noted otherwise.

- Vertical control joints in walls shall be placed at 30'-0" maximum spacing unless noted otherwise. Locate joints beside piers monolithic with walls, near corners, and in concealed locations where possible. Construction joints may be placed in lieu of control joints at contractor's discretion. Coordinate location of control joints with Architect.

Post-Installed Anchors:

- Post-installed anchors shall only be used where specified in the construction documents or approved by the engineer.
- The Contractor shall obtain written approval from the Engineer prior to installing post-installed anchors for misplaced-placed anchors.
- The Contractor shall not place with placing post-installed anchors to avoid damaging existing reinforcement.
- The holes shall be drilled and cleaned in accordance with the manufacturer's specifications.
- Post-installed anchors shall meet ACI 318 Appendix C criteria. The following are acceptable post-installed anchors:

- All adhesive anchoring systems referred to in these drawings shall be one of the following:

- a. Hilti HIT HY 200 V3
- b. Powers AC108 Gold
- c. Simpson Strong-Tie SET-3G
- d. Or Approved Equivalent

- All screw anchors referred to in these drawings shall be one of the following:

- a. Hilti KH-EZ
- b. Powers Wedge Bolt
- c. Simpson Strong-Tie Titan HD
- d. Or Approved Equivalent

Masonry:

- Mortar shall be Type S for all masonry work and must achieve a minimum compressive strength of 1800 psi at the 28-day test. Masonry units shall have a minimum strength of f'm = 1900 psi.
- Masonry grout shall be a coarse type grout and must achieve a minimum compressive strength of 2000 psi at the 28-day test. Slump shall range from 8" minimum to 10" maximum. Grout materials and proportions shall conform to ASTM C476.
- All masonry shall be reinforced with horizontal 9 gauge truss type reinforcement at 16" o.c. vertical or as shown on the drawings.
- Vertical reinforcing shall be installed as noted on the drawings. Reinforcing bars shall be lapped as specified on the design drawings. If no lap length is shown, conform to the Engineer.

- Vertical control joints in masonry shall be 3/8" wide, full height of wall at locations shown on the Architectural drawings. Joints shall be spaced at a maximum of 25'-0" apart and coordinated with the Architect. All horizontal joint reinforcing shall be discontinuous at masonry control joints. Refer to typical details for additional information.

- Unless over openings shall be installed as indicated on the drawings. If no inlets are indicated, notify the Engineer.

- Provide at least (1) vertical rebar at each end of each wall, side of control joints, jambs, corner, and intersection of all reinforced masonry walls. Size of rebar to match the size of typical vertical reinforcing shown.

- Provide (1) corner bar at each horizontal bond beam. Size of rebar to match typical bond beam reinforcing shown.

- Shop submit shop drawings including plan and elevation views of reinforced masonry walls including bond beams, control joints, expansion joints, and inlets.

- All steel beams bearing on masonry shall have (3) cores minimum grouted full directly below the bearing locations unless noted otherwise.

- All bond beam reinforcing shall continue through control joints.

- All cells containing reinforcement, bolts, or other metal anchors shall be grouted solid. Any cells below grade shall be grouted solid whether reinforced or not.

Structural Steel:

- All structural steel shall conform to the following (U.N.O.):

Structural Steel Wide Flanges:	ASTM A992
Miscellaneous Steel:	ASTM A36
Structural Tubing:	ASTM A500, Grade C (Fy = 50 ksi)
Steel Pipe:	ASTM A53, Type E or S, Grade B

- Bolts shall be as follows (U.N.O.):

Connection Bolts:	ASTM A325
Anchor Rods:	ASTM F1554, Grade 36
Shear Studs:	ASTM A108, Grade 10LS through 1020

- Welding shall conform to the latest publication of applicable codes set forth by the American Welding Society. Welding electrodes shall be E70XX.

- All exterior steel exposed to weather shall be hot-dipped galvanized and/or painted per Architect unless noted other wise.

- Weld all joints to supporting members with 1/8" x 2" long fillet welds on each side of the joint. In steel frames, where columns are not framed in at least two directions with structural steel members, joints at column lines shall be field-bolted at the columns to provide lateral stability during construction.

- All roof bar joists shall be designed for uplift as stipulated by the applicable building code. Extra bracing shall be added as required, and the joist manufacturer shall certify that the joists have been designed for reverse bending due to uplift.

- All pipe hangers supporting more than 100 lbs. and being supported from steel bar joists or joist girders shall be hung from top chords and within 2" of web panels. If interferences exist that will not allow pipe to be hung in this manner, the Contractor shall notify the Engineer for required modifications.

- All openings in the roof shall be framed with a 4 x 4 x 1/4 angle minimum, unless noted otherwise. Mechanical units shall be supported with structural steel frames as required. If framing is not shown for mechanical units, the design of the steel joist institute regarding additional bolted "X" bridging required for erection stability.

- All pipe hangers supporting more than 100 lbs. and being supported from steel bar joists or joist girders shall be hung from top chords and within 2" of web panels. If interferences exist that will not allow pipe to be hung in this manner, the Contractor shall notify the Engineer for required modifications.

- All openings in the roof shall be framed with a 4 x 4 x 1/4 angle minimum, unless noted otherwise. Mechanical units shall be supported with structural steel frames as required. If framing is not shown for mechanical units, the design of the steel joist institute regarding additional bolted "X" bridging required for erection stability.

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Special Inspector:

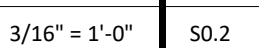
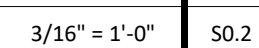
- The following items require special inspection in accordance with the building code.
 - Reinforced masonry construction - level 1 inspection
 - Concrete & masonry grout design mix
 - Placing of concrete & reinforcing steel
 - Bolts & anchors embedded in concrete & masonry
 - Concrete formwork
 - Structural steel fabrication
 - Structural steel bolting & welding
 - Inspection of roof & deck attachment
 - Post installed anchors in masonry & concrete
 - In-situ soils, excavations, filling & compaction
- The Contractor shall request special inspection of the items listed above prior to those items becoming inaccessible & unobservable due to progression of the work.

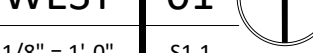
- The Special Inspector shall be a qualified person who shall demonstrate competence, to the satisfaction of the building official, for inspection of the particular type of construction or operation requiring special inspection.

- The Special Inspector shall observe the work assigned for conformance with the approved design drawings and specifications.

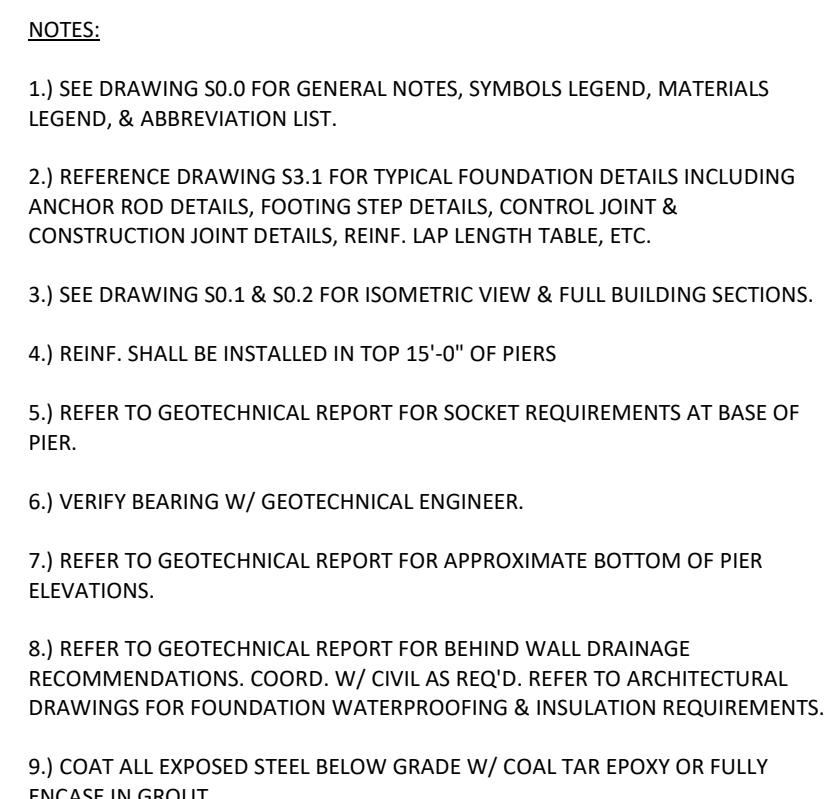
- The Special Inspector shall furnish inspection reports to the Building Official, the Engineer and Architect of record, and other designated persons. All discrepancies shall be brought to the immediate attention of the Contractor for correction, then-if uncorrected, to the proper design authority and to the Building Official.

- The Special Inspector shall submit a final signed report stating whether the work requiring special inspection was, to the best of the inspector's knowledge, in conformance with the approved plans and specifications and the applicable workmanship provisions of the governing building codes.





NOTE:
- EMBED DRILLED PIER INTO ROCK PER THE GEOTECHNICAL REPORT RECOMMENDATIONS

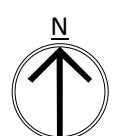


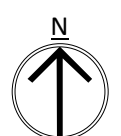
PIER CAP SCHEDULE									
MARK	DIMENSIONS	TYPE	REINFORCEMENT	NUMBER OF PILES	T.O.C.	T.O.P.	PILE Ø	PILE REINFORCEMENT	COMMENT
P1	PER 53.3		#6 @ 8" SPA. EA. WAY TOP & BOTT.						PER 53.3
PC1	8'-0" x 8'-0" x 3'-0"	PER 53.3	#6 @ 8" SPA. EA. WAY TOP & BOTT.	4	99.33	96.33	16"	PER 53.3	
PC2	11'-0" x 13'-0" x 3'-0"	PER 53.3	#6 @ 7" SPA. EA. WAY TOP & BOTT.	5	99.33	96.33	20"	PER 53.3	
PC3	9'-2" x 10'-0" x 3'-0"	PER 53.3	#6 @ 8" SPA. EA. WAY TOP & BOTT.	3	99.33	96.33	24"	PER 53.3	
PC4	9'-0" x 9'-0" x 3'-0"	PER 53.3	#6 @ 7" SPA. EA. WAY TOP & BOTT.	4	99.33	96.33	20"	PER 53.3	
PC5	8'-0" x 13'-0" x 3'-0"	PER 53.3	#6 @ 8" SPA. EA. WAY TOP & BOTT.	6	99.33	96.33	16"	PER 53.3	
PC6	8'-0" x 11'-8" x 3'-0"	PER 53.3	#6 @ 8" SPA. EA. WAY TOP & BOTT.	6	99.33	96.33	16"	PER 53.3	

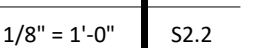
NOTE:
- EMBED DRILLED PIER INTO ROCK PER THE GEOTECHNICAL REPORT RECOMMENDATIONS

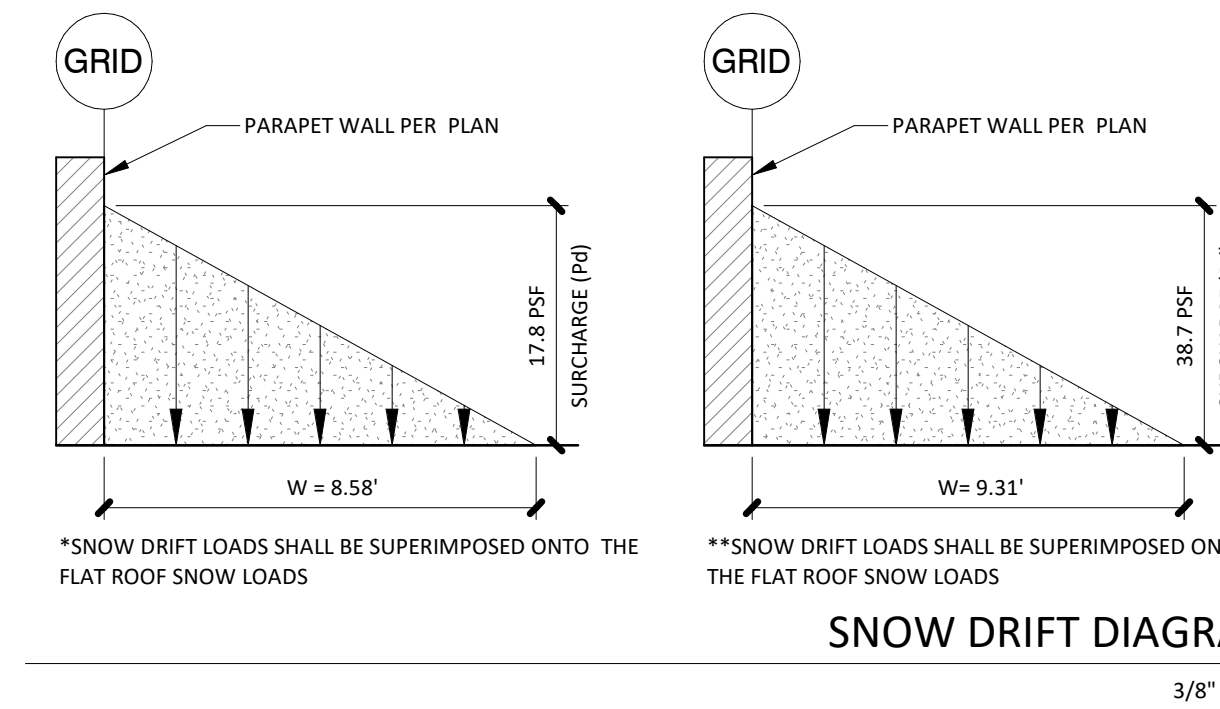
WEST
AREA

FOUNDATION PLAN - EAST | 01

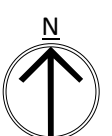


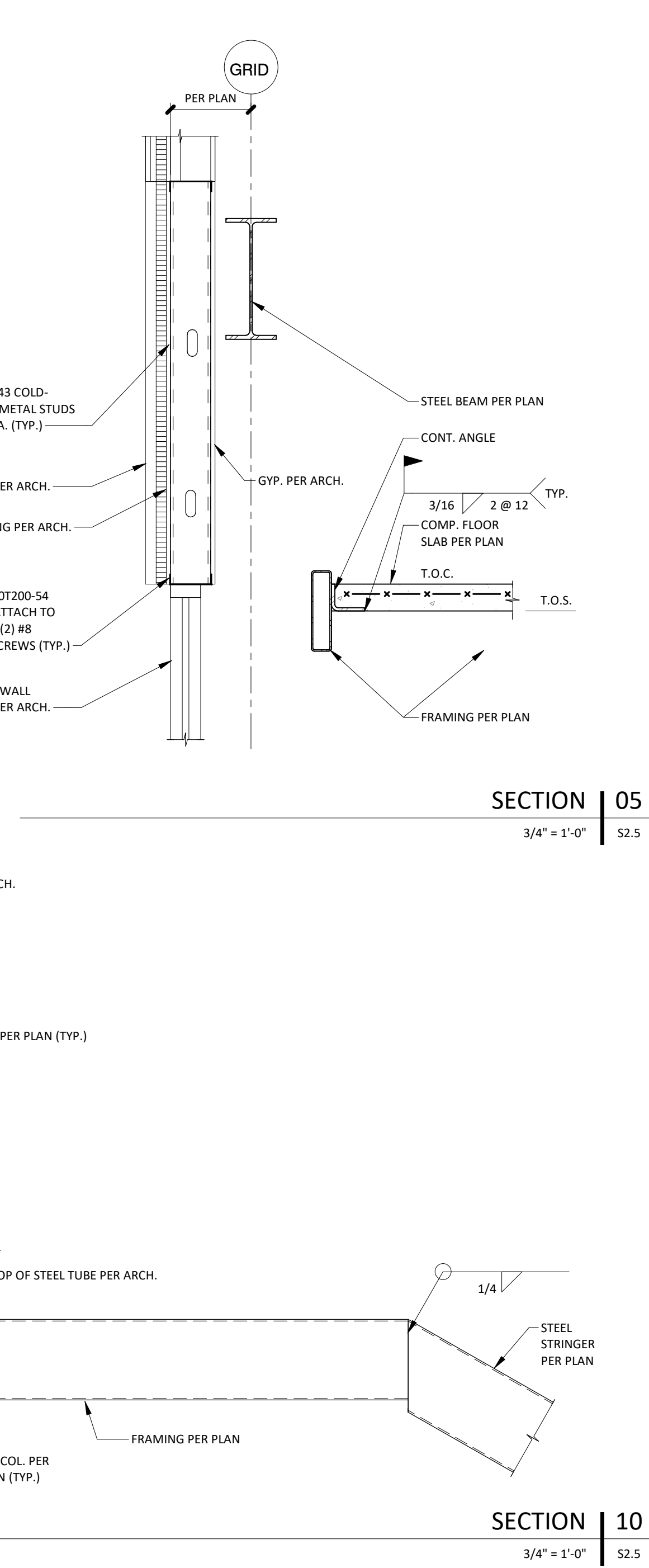






- | | |
|--------------------------|------|
| ROOF FRAMING PLAN - WEST | 01 |
| 1/8" = 1'-0" | 52.3 |








Project No.: 19050.01a

Date: 06.02.22

Issued For: ADDENDUM 1

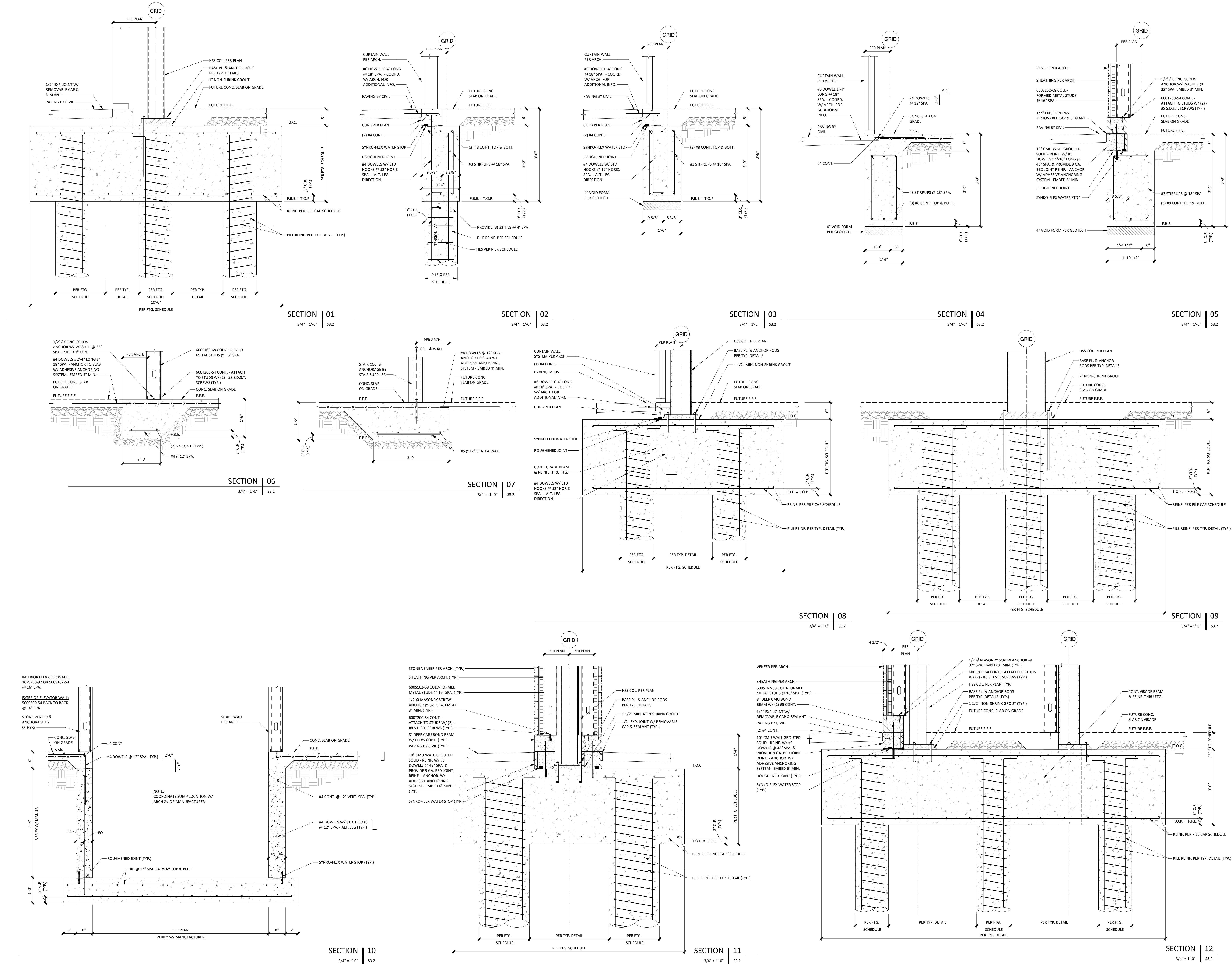
REGISTRATION

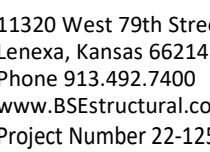
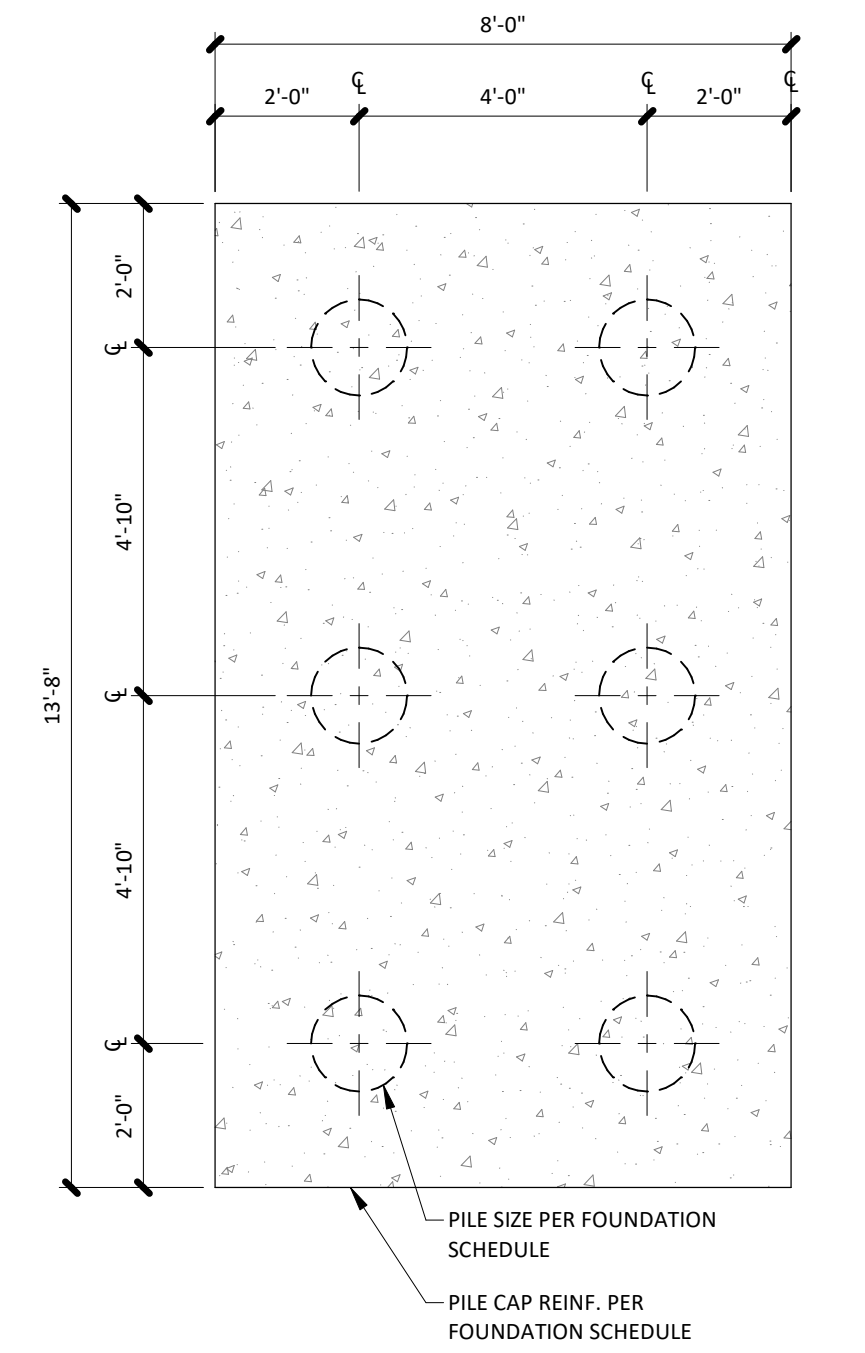
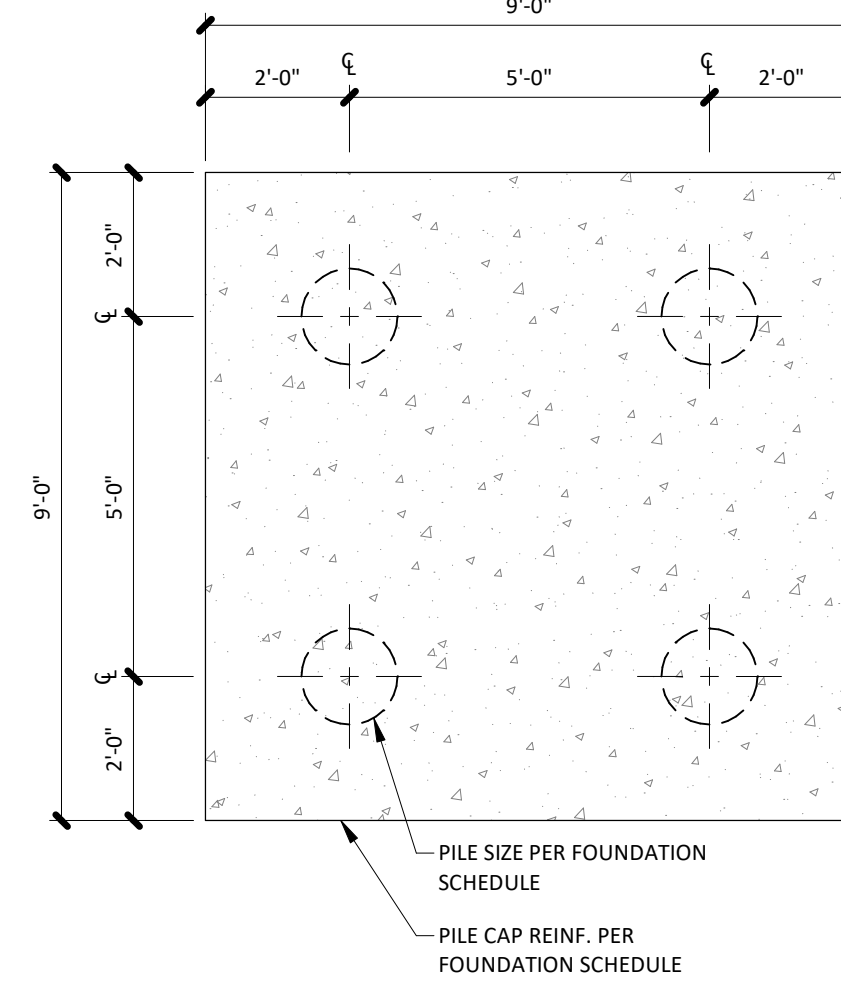
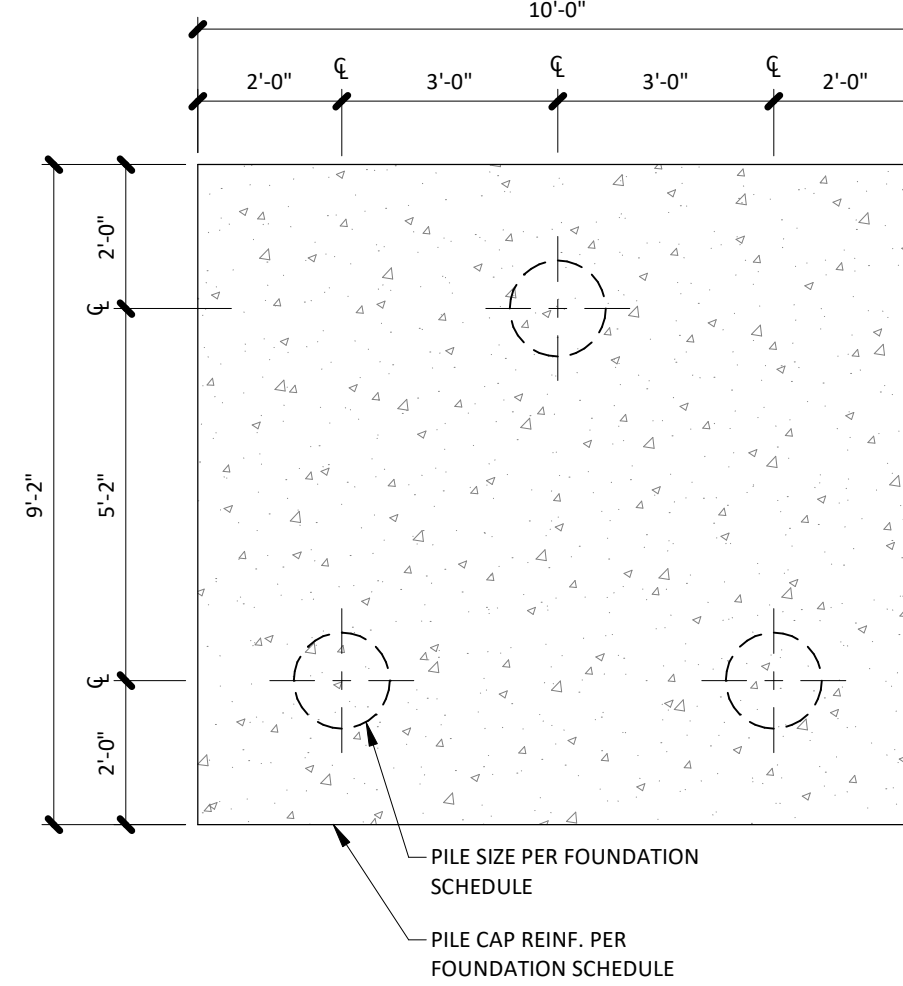
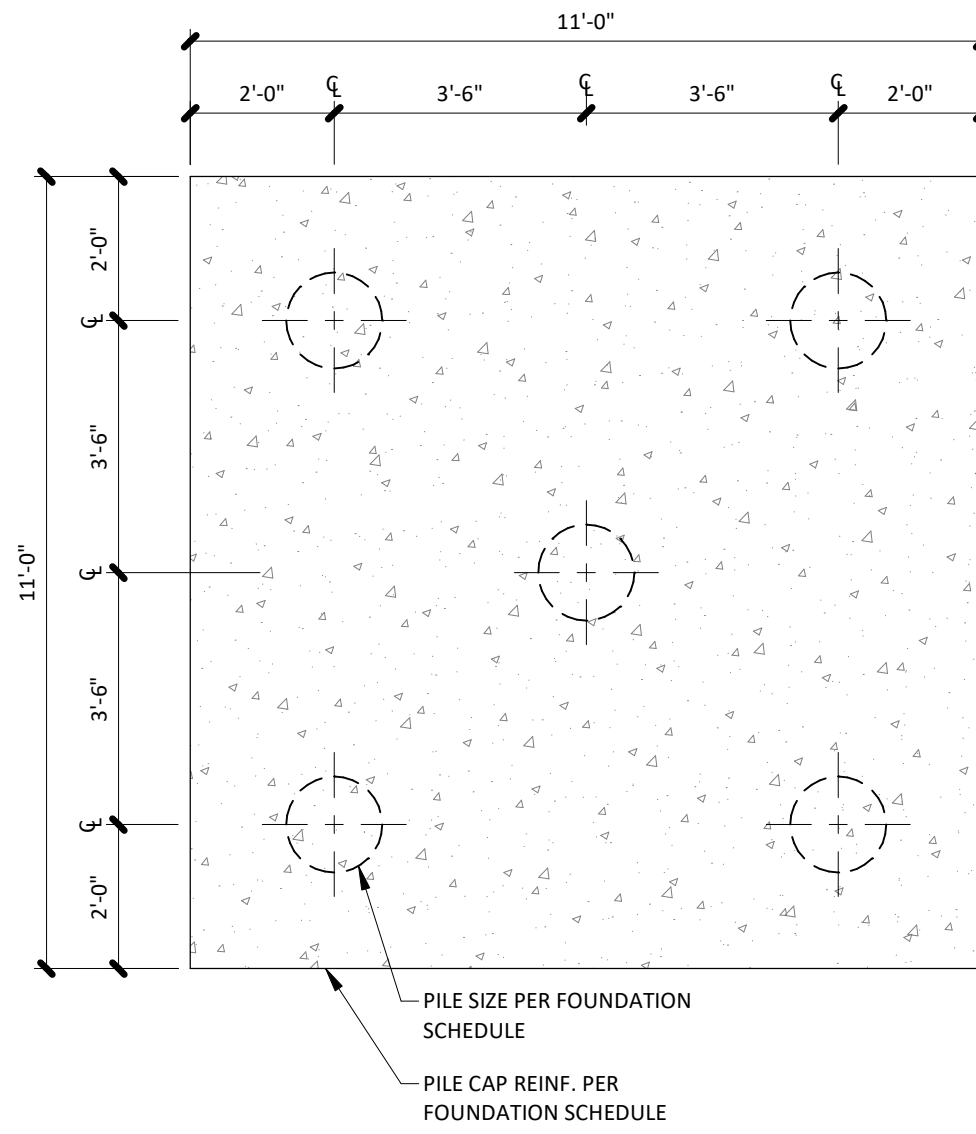
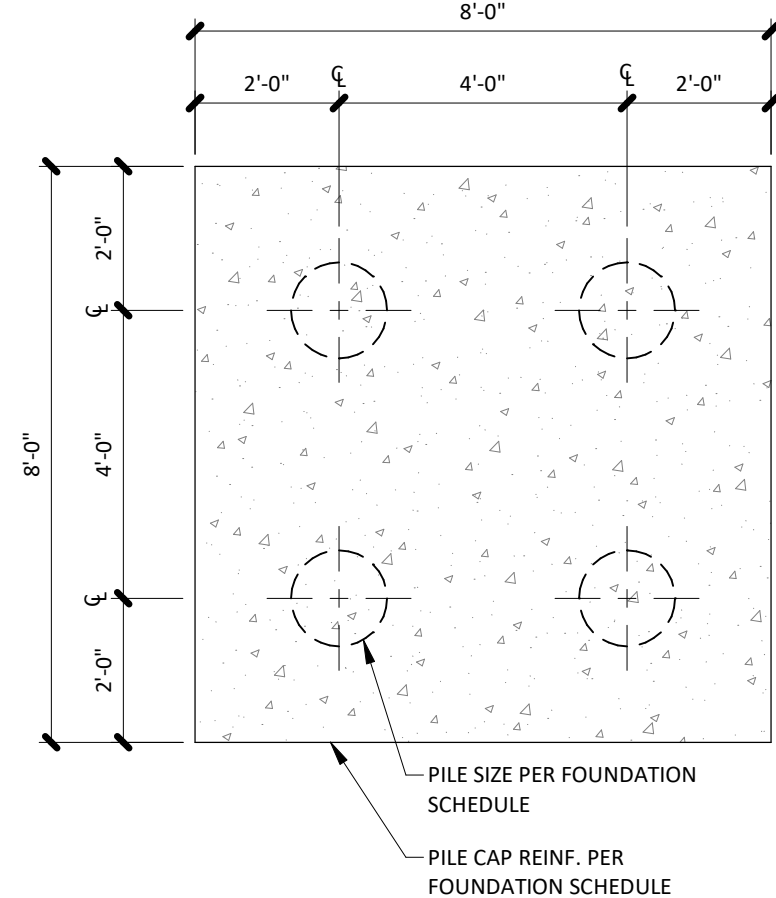
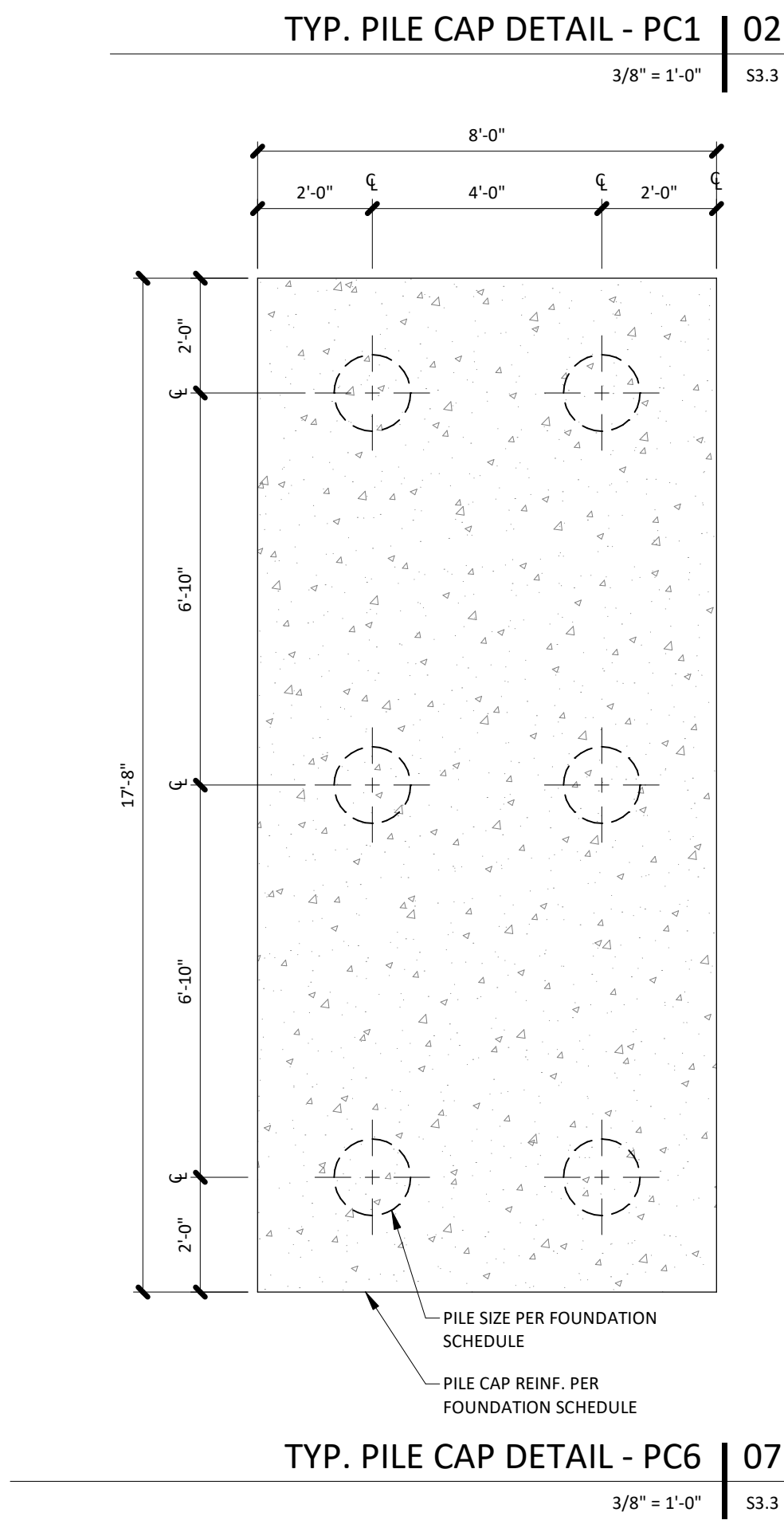


BS E STRUCTURAL ENGINEERS
11320 West 79th Street
Lenexa, Kansas 66214
Phone 913.492.7400
www.BSEstructural.com
Project Number 22-125

SHEET NUMBER

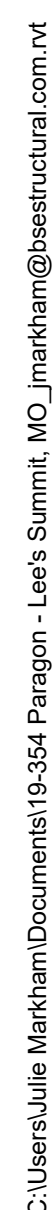
S3.2





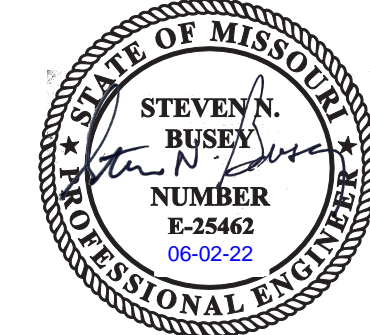
S3.3





REVISIONS		
No.	Date	Description

REGISTRATION



PROJECT TEAM

ARCHITECT	FINKLE-WILLIAMS ARCHITECTURE
CIVIL	GBA
LANDSCAPE	LAND 3
FOUNDATIONS	BSE STRUCTURAL ENGINEERS
STRUCTURAL	BSE STRUCTURAL ENGINEERS
PLUMBING	HENDERSON ENGINEERS
MECHANICAL	HENDERSON ENGINEERS
ELECTRICAL	HENDERSON ENGINEERS
FIRE PROTECTION	HENDERSON ENGINEERS
CONTRACTOR	GC

SHEET TITLE

FRAMING
DETAILS

SHEET NUMBER

S4.6

