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MECHANICAL

ME-001 COVER SHEET

MP1.71G GARAGE FIRST FLOOR- PLUMBING

MP1.72G GARAGE SECOND FLOOR- PLUMBING MP1.73G GARAGE THIRD FLOOR- PLUMBING MP1.74G GARAGE FOURTH FLOOR- PLUMBING MP1.75G GARAGE FIFTH FLOOR- PLUMBING MP2.01 MECHANICAL DETAILS/SCHEDULES

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PROJECT TEAM

ARCHITECTFINKLE + WILLIAMS ARCHITECTURE 8787 Renner Blvd., Suite 100 Lenexa, Kansas 66219 PH. 913.498.1550

9801 Renner Blvd, Suite 300 Lenexa, Kansas 66219

PH. 913.492.0400 LANDSCAPE

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PRECAST CONTRACTOR
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FIRE PROTECTION

PLUMBING LS&A, P.A.

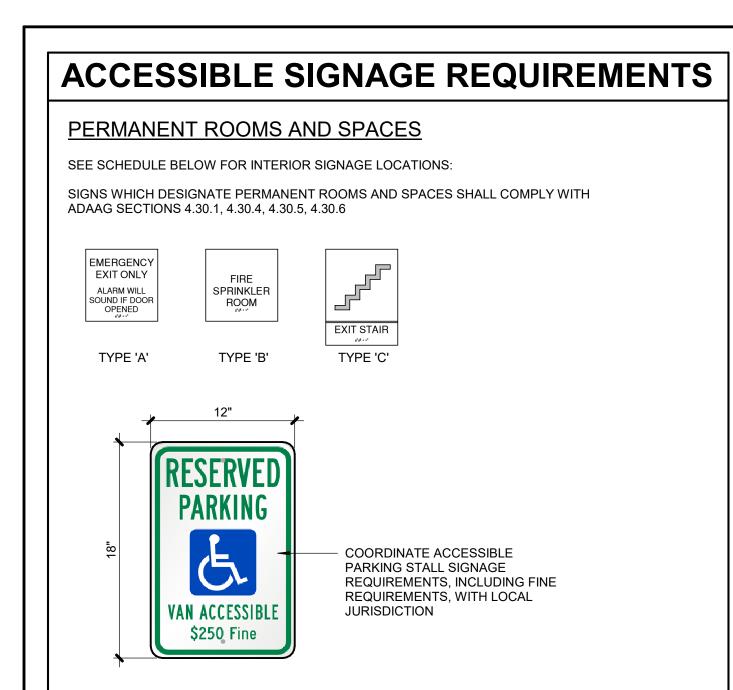
ELECTRICAL

CONTRACTOR
BRINKMANN CONSTRUCTORS

11101 Switzer Rd., Suite 310 Overland Park, Kansas 66210 PH. 913.717.9007 F. 913.717.9407

MECHANICAL LS&A, P.A. 8625 College Blvd., Suite 102 Overland Park, Kansas 66210 PH. 785.233.0647 F. 785.233.0647

8625 College Blvd., Suite 102 Overland Park, Kansas 66210



INTERIOR SIGNAGE SCHEDULE GARAGE

DOOR NO.	ROOM NAME	SIGN TYPE
103.A	WATER SERVICE	TYPE 'B'
105.A	STAIR	TYPE 'A'
206.A	STAIR	TYPE 'C'
206.B	STAIR	TYPE 'C'
306.A	STAIR	TYPE 'C'
406.A	STAIR	TYPE 'C'
506.A	STAIR	TYPE 'C'

- a. CHARACTERS SHALL BE RAISED MINIMUM 1/32"
- CHARATERS SHALL BE ACCOMPANIED BY GRADE 2 BRAILLE
- CHARACTERS SHALL BE UPPER CASE & SANS SERIF OR SERIF TYPESTYLE CHARACTERS SHALL BE A MINIMUM OF 5/8" HIGH AND MAXIMUM 2" HIGH
- PICTOGRAMS SHALL BE ACCOMPANIED BY THE EQUIVALENT VERBAL DESCRIPTION PLACED DIRECTLY BELOW THE PICTORGRAM AS INDICATED. THE BORDER DIMENSION OF THE PICTORGRAM SHALL BE 6" MIN. IN HEIGHT
- CHARACTERS AND BACKGROUND SHALL BE EGGSHELL, MATTE OR OTHER NON-GLARE FINISH AS RECOMMENDED BY THE SIGN MANUFACTURER.
- BACKGROUND SHALL CONSIST OF 1/4" ACRYLIC, COLOR TO MATCH SW 7068
- MOUNT AT 60" ABOVE FINISH FLOOR TO THE CENTER OF SIGN MOUNT ON WALL ADJACENT TO THE LATCH SIDE OF THE DOOR IF NO WALL SPACE EXISTS ON THE LATCH SIDE OF THE DOOR, INCLUDING DOUBLE

CHARACTERS AND SYMBOLS SHALL BE WHITE

- LEAF DOORS, MOUNT ON THE NEAREST ADJACENT WALL
- ROOM" APPLIED TO EXTERIOR SIDE OF DOOR, AS REQUIERD BY LOCAL FIRE DEPT.

DIRECTIONAL INFORMATION

OTHER SIGNS WHICH PROVIDE DIRECTION TO OR INFORMATION ABOUT FUNCTIONAL SPACES OF THE BUILDING SHALL COMPLY WITH ADAAG SECTIONS: 4.30.1, 4.30.2, 3.30.3, 4.30.5

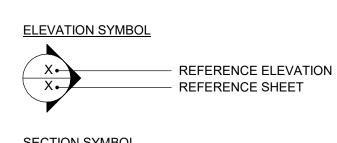
TWO-WAY COMMUNICATION DEVICE SIGNAGE

DIRECTIONS FOR THE USE OF THE TWO-WAY COMMUNICATION SYSTEM, INSTRUCTIONS FOR SUMMONING ASSISTANCE CIA THE TWO-WAY COMMUNICATION SYSTEM AND WRITTEN IDENTIFICATION OF THE LOCATION SHALL BE POSTED ADJACENT TO EACH TWO-WAY COMMUNICATION SYSTEM. EACH SIGN SHALL COMPLY WITH ICC A117.1 FOR VISUAL CHARACTERS. MOUNTING LOCATION OF SIGNAGE AND DEVICE SHALL BE PER DRAWING BELOW.

INCLUDE 4" HIGH VINYL WHITE LETTERS W/ MIN. 0.5" STROKE READING "SPRINKLER

DRAWING SYMBOLS LEGEND

- EXISTING CONSTRUCTION TO REMAIN
- NEW CONSTRUCTION WALL TYPE DESIGNATION -• XX REFERENCE FLOOR PLAN(S) FOR LOCATIONS.
 - **ROOM NAME AND NUMBER -**REFERENCE FLOOR PLAN(S) FOR LOCATIONS. REFERENCE FINISH SCHEDULE FOR FINISHES.
- DOOR AND FRAME DESIGNATION -REFERENCE FLOOR PLAN(S) FOR LOCATIONS. REFERENCE DOOR AND FRAME SCHEDULE FOR REQUIREMENTS.



SECTION SYMBOL - REFERENCE SECTION - REFERENCE SHEET ENLARGED DETAIL / ENLARGED PLAN SYMBOI

/ X• - REFERENCE DETAIL REFERENCE SHEET

INTERIOR ELEVATION SYMBOL



FINISH DESIGNATION SYMBOL

XX-1 WALL FINISH DESIGNATION XX-1- BASE FINISH DESIGNATION LIMITS OF WALL AND BASE FINISHES

> REFERENCE FLOOR PLAN(S) FOR LOCATIONS. REFERENCE FINISH SCHEDULE FOR DESCRIPTIONS.

FLOOR FINISH DESIGNATION -REFERENCE FLOOR PLAN(S) FOR LOCATIONS. REFERENCE FINISH SCHEDULE FOR DESCRIPTIONS.

CONSTRUCTION NOTE

DEMOLITION NOTE

GYPSUM BOARD CONTROL JOINT -REFERENCE FLOOR PLAN(S) FOR LOCATIONS. REFERENCE DETAIL 1,2/A7.01 FOR CONSTRUCTION REQUIREMENTS.

WALL MOUNTED FIRE EXTINGUISHER BY LARSEN'S MANUFACTURING COMPANY, WWW.LARSENMFG.COM, MODEL MP10 W/B2 MOUNTING BRACKET, REFERENCE FLOOR PLAN(S) FOR LOCATIONS. MOUNT SO CENTERLINE OF EXTINGUISHER IS 46" A.F.F.

SEMI-RECESSED FIRE EXTINGUISHER BY LARSEN'S MANUFACTURING COMPANY, WWW.LARSENMFG.COM OR APPROVED EQUAL: ARCHITECTURAL SERIES, MODEL # AL-2409-6R. ALUMINUM, SEMI-RECESSED (2 ½" PROTRUSION FROM WALL WITH ROLLED EDGES). SOLID DOOR WITH RECESSED HANDLE, ENGRAVED VERTICAL LETTERS WITH NO BACKFILL "FIRE EXTINGUISHER" ON DOOR. CABINET TO BE PROVIDED WITH MP10 FIRE EXTINGUISHER AND MANUFACTURER'S STANDARD MOUNTING BRACKET. MOUNT SO CENTERLINE OF CABINET HANDLE IS 46" A.F.F.

GENERAL NOTES

- ALL CONSTRUCTION SHALL CONFORM TO THE MINIMUM STANDARDS OF THE APPLICABLE CODE INDICATED IN THE BUILDING SUMMARY COLUMN AND ALL LOCAL CODES PRESENTLY IN EFFECT UNLESS MORE STRINGENT REQUIREMENTS ARE INDICATED.
- ALL NEW CONSTRUCTION SHALL COMPLY W/THE AMERICANS WITH DISABILITIES ACT ACCESSIBILITY GUIDELINES (ADAAG) AND CHAPTER 11 OF THE INTERNATIONAL BUILDING CODE (INCLUDES ICC A117.1 PER IBC)
- THE GENERAL CONTRACTOR AND SUBCONTRACTORS SHALL OBTAIN AND PAY FOR ALL REQUIRED PERMITS, LICENSES, AND ALL UTILITY CHARGES, AND ARRANGE FOR ALL REQUIRED INSPECTIONS.
- THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR COORDINATING BUILDING & SITE UTILITIES BETWEEN CIVIL & MEP DRAWINGS. THE CONTRACTOR SHALL ALSO CONTACT ALL APPLICABLE UTILITY COMPANIES & PROVIDE CONDUIT & OTHER FACILITIES AS REQUIRED.
- THE GENERAL CONTRACTOR AND ALL SUBCONTRACTORS SHALL VERIFY ALL DIMENSIONS & CONDITIONS ON THE JOB SITE PRIOR TO THE BIDDING OF THE CONTRACT DOCUMENTS. THE CONTRACTOR SHALL NOTIFY THE ARCHITECT IMMEDIATELY OF ANY DISCREPANCIES. IN CASES OF DISCREPANCY CONCERNING DIMENSIONS, QUANTITIES AND LOCATION, THE CONTRACTOR SHALL, IN WRITING, CALL TO THE ATTENTION OF THE ARCHITECT ANY DISCREPANCIES BETWEEN SPECIFICATIONS, PLANS, DETAILS OR SCHEDULES. THE ARCHITECT WILL THEN INFORM THE CONTRACTOR, IN WRITING, WHICH DOCUMENT TAKES PRECEDENCE. THERE SHALL BE NO ADJUSTMENT TO THE COST OR TIME OF THE WORK RESULTING FROM CLARIFICATION OF SUCH DISCREPANCIES.
- DIMENSIONS ON DRAWINGS ARE SHOWN TO FINISHED FACE OF WALLS AND PARTITIONS OF EXISTING OR NEW CONSTRUCTION UNLESS OTHERWISE NOTED. CEILING HEIGHT DIMENSIONS AND ALL OTHER VERTICAL DIMENSIONS ARE TO THE FINISHED FLOOR SURFACE UNLESS OTHERWISE NOTED.
- ALL MATERIALS SPECIFIED OR NOTED SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURERS RECOMMENDATIONS.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR SUBMITTING SHOP DRAWINGS. PRODUCT DATA, OR SAMPLES FOR CASEWORK, FINISHES, DOORS, FRAMES, HARDWARE, MECHANICAL, ELECTRICAL, AND PLUMBING FIXTURES, AND OTHER ITEMS REQUIRING ARCHITECT'S REVIEW FOR CONFORMANCE WITH THE CONTRACT DOCUMENTS, AND FOR ALL ITEMS WHICH ALLOWED CONTRACTOR OPTIONS. PRIOR TO FORWARDING TO THE ARCHITECT FOR REVIEW. THESE SUBMITTALS MUST BE REVIEWED BY THE CONTRACTOR FOR CONFORMANCE WITH THE MEANS, METHODS, TECHNIQUES, SEQUENCES, AND OPERATIONS OF CONSTRUCTION AND SAFETY PRECAUTIONS AND PROGRAMS INCIDENTAL THERETO, ALL OF WHICH ARE THE SOLE RESPONSIBILITY OF THE CONTRACTOR, THE CONTRACTOR SHALL AFFIX A STAMP TO SUBMITTAL INDICATING HIS REVIEW. SUBMITTALS FORWARDED WITHOUT A STAMP WILL BE RETURNED. ALL SUBMITTALS MUST BE REVIEWED BY THE ARCHITECT PRIOR TO CONSTRUCTION.
- CONTRACTOR SHALL GUARANTEE ALL WORK AGAINST FAULT OF ANY MATERIAL OR WORKMANSHIP FOR A PERIOD OF NOT LESS THAN ONE YEAR AFTER COMPLETION OR ACCEPTANCE. FAULTY WORK SHALL BE REPLACED OR REPAIRED AS REQUIRED AT NO COST TO THE OWNER.
- CONTRACTOR SHALL COORDINATE WITH OWNER ALL ITEMS TO BE SALVAGED PRIOR TO SUBMISSION OF BIDS AND START OF CONSTRUCTION. OWNER SHALL HAVE SALVAGE RIGHTS TO RETAIN ALL REMOVED ITEMS.
- ALL CHANGES PROPOSED DURING CONSTRUCTION WHICH RESULT IN A CHANGE TO THE CONTRACT TIME AND/OR SUM SHALL BE SUBMITTED TO THE ARCHITECT IN WRITING AND APPROVED BY THE ARCHITECT AND OWNER BEFORE SUCH WORK SHALL COMMENCE.
- 12. CONTRACTOR SHALL COORDINATE CLEAR OPENINGS FOR ALL APPLIANCES PRIOR TO CONSTRUCTION OF CASEWORK.
- CONTRACTOR SHALL FURNISH AND INSTALL CONCEALED FIRE-RETARDANT TREATED WOOD BLOCKING BEHIND ALL CABINETS. TOILET ACCESSORIES. PLUMBING FIXTURES. AND OTHER WALL MOUNTED ITEMS AS REQUIRED FOR ADEQUATE SUPPORT.
- 14. CONTRACTOR SHALL COORDINATE ALL LOCK AND LATCH SETS AND FINAL KEYING WITH OWNER. DOUBLE KEYED LOCKS ARE NOT PERMITTED ON ANY REQUIRED OR MARKED EXIT.
- 15. ALL DOOR HARDWARE ON EXIT DOORS SHALL BE READILY OPERABLE FROM THE EGRESS

MATCH EXISTING KEYING SYSTEM IF ONE IS EXISTING.

- SIDE WITHOUT THE USE OF A KEY, SPECIAL KNOWLEDGE, OR EFFORT. CONTRACTOR SHALL PREPARE ALL NEW AND EXISTING SURFACES SCHEDULED TO
- RECEIVE NEW FINISHES IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS FOR THE SUBSTRATE & FINISH BEING APPLIED.
- 17. CONTRACTOR SHALL COORDINATE FINAL QUANTITY AND LOCATIONS OF FIRE EXTINGUISHERS WITH THE FIRE DEPARTMENT AND/OR BUILDING DEPARTMENT. SEE SYMBOLS LEGEND FOR TYPE OF EXTINGUISHER.
- 18. ALL CONSTRUCTION MATERIALS EXPOSED WITHIN PLENUMS SHALL BE NON-COMBUSTIBLE OR SHALL HAVE A MAXIMUM FLAME SPREAD RATING OF 25 AND MAXIMUM SMOKE
- 19. ALL PIPING, LOW VOLTAGE WIRE AND CABLE, OPTICAL FIBER, PNEUMATIC TUBING, AND ALL DUCT AND DUCT COVERINGS, LININGS AND CONNECTORS INSTALLED WITHIN PLENUMS MUST BE RATED FOR PLENUM USE.
- 20. TENANT SHALL BE RESPONSIBLE FOR COORDINATION AND INSTALLATION OF VOICE AND DATA CABLING AND EQUIPMENT.
- 21. CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN OF THE AUTOMATIC SPRINKLER SYSTEM. THE DESIGN SHALL BE PER NFPA REQUIREMENTS.
- 22. ALL NEW GLASS AND GLAZING LOCATED IN HAZARDOUS LOCATIONS AS DEFINED IN IBC SECTION 2406.3 SHALL MEET THE REQUIREMENTS FOR SAFETY GLAZING AS DEFINED IN IBC
- 23. IF THE CONTRACTOR FAILS TO SUBMIT A MATERIAL FOR APPROVAL, THE MATERIAL MAY BE REQUIRED TO BE REMOVED BY THE CONTRACTOR EITHER BY DIRECTION OF THE OWNER
- 24. ALL HIGH-PILED STORAGE SHALL COMPLY WITH THE APPLICABLE REQUIREMENTS OF THE

AUTOCAD FILE TO THE OWNER AT THE CONCLUSION OF THE PROJECT.

- APPLICABLE EDITION OF THE INTERNATIONAL FIRE CODE. 25. THE CONTRACTOR IS TO PROVIDE AS BUILT DRAWINGS IN HARD COPY & AN ELECTRONIC
- 26. INSTALL ELASTOMERIC JOINT SEALER AROUND ALL PIPES, DUCTWORK, & STRUCTURE PASSING THRU INTERIOR NON-RATED CONCRETE AND MASONRY WALLS, GYPSUM BOARD PARTITIONS, AND CONCRETE FLOOR/ROOF SLABS. FOR FIRE RATED INTERIOR CONCRETE AND MASONRY WALLS, GYPSUM BOARD PARTITIONS, AND CONCRETE FLOOR/ROOF SLABS SEAL ALL PIPES, DUCTWORK, AND STRUCTURE. INSTALL FIRESTOP MATERIALS IN ALL GAPS PRIOR TO SEALANT APPLICATION. INSTALL SEALER ACCORDING TO MANUFACTURER'S WRITTEN INSTRUCTIONS.
- 27. CONTRACTOR SHALL BE RESPONSIBLE FOR PROTECTION OF ALL EXISTING CONSTRUCTION INDICATED TO REMAIN AND SHALL REPAIR AND/OR REPLACE ALL AREAS AND /OR MATERIAL DAMAGED DURING CONSTRUCTION AT A MINIMUM TO THE CONDITION WHICH EXISTED PRIOR TO CONSTRUCTION.
- 28. CONTRACTOR SHALL BE RESPONSIBLE FOR PRICING RADIO COVERAGE AMPLIFIER FOR EMERGENCY RESPONDERS AS AN ALTERNATE. PRIOR TO CONSTRUCTION COMPLETION, AMPLIFIER SHALL BE PROVIDED ONLY IF REQUIRED BY AHJ.

PROFESSIONAL SERVICES DISCLAIMER

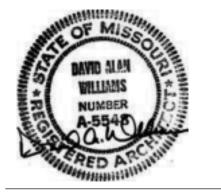
THIS DISCLAMER SERVES NOTICE OF ACCEPTANCE OF RESPONSIBILITY AND DISCLAIMER OF RESPONSIBILITY AS TO THE CONTRACT DOCUMENTS PREPARED FOR

18017,19050.07,19050.08, PARAGON STAR NORTH VILLAGE BY FINKLE + WILLIAMS, INC. THE UNDERSIGNED ARCHITECT, AND FINKLE + WILLIAMS, INC., ARE RESPONSIBLE FOR PREPARATION OF ONLY THE NOTED CONSTRUCTION DRAWINGS BELOW:

NO.	<u>TITLE</u>	<u>DATE</u>
A0.00G	GARAGE COVER	02/04/22
A0.01G	LEGENDS & GEN. NOTES	02/04/22
A0.03	CODE	03.11.22
A1.70G	GARAGE FIRST FLOOR PLAN AT GRADE	02/04/22
A1.71G	GARAGE FIRST FLOOR PLAN	02/04/22
A1.72G	GARAGE SECOND FLOOR PLAN	02/04/22
A1.73G	GARAGE THIRD FLOOR PLAN	02/04/22
A1.74G	GARAGE FOURTH FLOOR PLAN	02/04/22
A1.75G	GARAGE FIFTH FLOOR PLAN	02/04/22
A1.76G	GARAGE SECTIONS	02/04/22
A1.77G	GARAGE STAIRS	02/04/22
A1.78G	GARAGE STAIRS AND SECTIONS	02/04/22
A8.01G	DOOR SCHEDULE AND DETAILS	02/04/22

THE UNDERSIGNED ARCHITECT AND FINKLE + WILLIAMS DISCLAIM RESPONSIBILITY FOR ALL OTHER CONSTRUCTION DOCUMENTS. AND ANY OTHER SPECIFICATIONS. REPORTS. ESTIMATES, SHOP DRAWINGS, ETC. RELATING TO OR INTENDED TO BE USED FOR ANY PART OF THE ARCHITECTURAL OR ENGINEERING PROJECT, INCLUDING ANY GEOTECHNICAL ENGINEERING SERVICES, OR ENVIRONMENTAL REPORTS.

THIS NOTICE IS EXECUTED BY THE UNDERSIGNED AND AUTHENTICATED BY THE ARCHITECTURAL SEAL OF THE PERSON PREPARING THS NOTICE.



ARCHITECT: DAVID A. WILLIAMS

BUILDING SUMMARY

GENERAL BUILDING INFORMATION

PROJECT NAME: PARAGON STAR NORTH VILLAGE ADDRESS: 3200 NW PARAGON PKWY

LEE'S SUMMIT, MO 64081 PROPOSED USE: MIXED USE MULTI-FAMILY RESIDENTIAL AND RETAIL

APPLICABLE CODES

	INTERNATIONAL BUILDING CODE (IBC)	2018 EDITION
	INTERNATIONAL MECHANICAL CODE (IMC)	2018 EDITION
	INTERNATIONAL PLUMBING CODE (IPC)	2018 EDITION
	NATIONAL ELECTRIC CODE (NEC)	2017 EDITION
	INTERNATIONAL FIRE CODE (IFC)	2018 EDITION
	INTERNATIONAL FUEL GAS CODE (IFGC)	2018 EDITION
	ICC/ANSI A117.1-2009, ACCESSIBLE AND USABLE BUILDINGS AND FACILITIES	

GENERAL BUILDING LIMITATIONS

This development is made up of the following multiple adjacent separate and distinct buildings both vertically and horizontally.

Retail Building (SEPARATE PERMIT) Parking Garage Apartment Building A (SEPARATE PERMIT) (SEPARATE PERMIT) Apartment Building B (SEPARATE PERMIT) Apartment Building C Apartment Building D (SEPARATE PERMIT)

Apartment Building E

Apartment Building F

168,460 GSF Parking Garage

(SEPARATE PERMIT)

(SEPARATE PERMIT)

Occupancy: Low-hazard Storage Group S-2 Construction Type: IIA (Fully Sprinklered NFPA 13) 6 Stories – 85' Above Grade Plane Allowable Height: Allowable Area: 117,000 SF/Flr Max. Travel Distance:

PARAGON STAR NORTH VILLAGE

3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081

Project No.: 18017,19050.07,19050.08 02.04.22 Issued For: GARAGE PERMIT

REVISIONS

REGISTRATION

PROJECT TEAM ARCHITECT FINKLE+WILLIAMS ARCHITECTURE **GBA ENGINEERS** LANDSCAPE LAND 3

STRUCTURAL BOB D. CAMPBELL LATIMER SOMMERS PLUMBING

MECHANICAL

LATIMER SOMMERS

LATIMER SOMMERS ELECTRICAL FIRE PROTECTION LATIMER SOMMERS

CONTRACTOR CONSTRUCTORS



 INFILL MTL. DECK W/ FIRE SAFING AS REQUIRED FIRE RATED SEALANT AS

- 20 GA. LONG LEG RUNNER TRACK ANCHORED TO BOTTOM OF STRUCTURE (MAINTAIN 1" CLEAR BET. UPPER & LOWER TRACK)

- CEILING, REF FINISH SCHEDULE FOR TYPE & LOCATION, TYP.

6" BATT INSULATION,

WHERE SCHEDULED

- (2) LAYERS 5/8" TYPE

- FLOOR SLAB

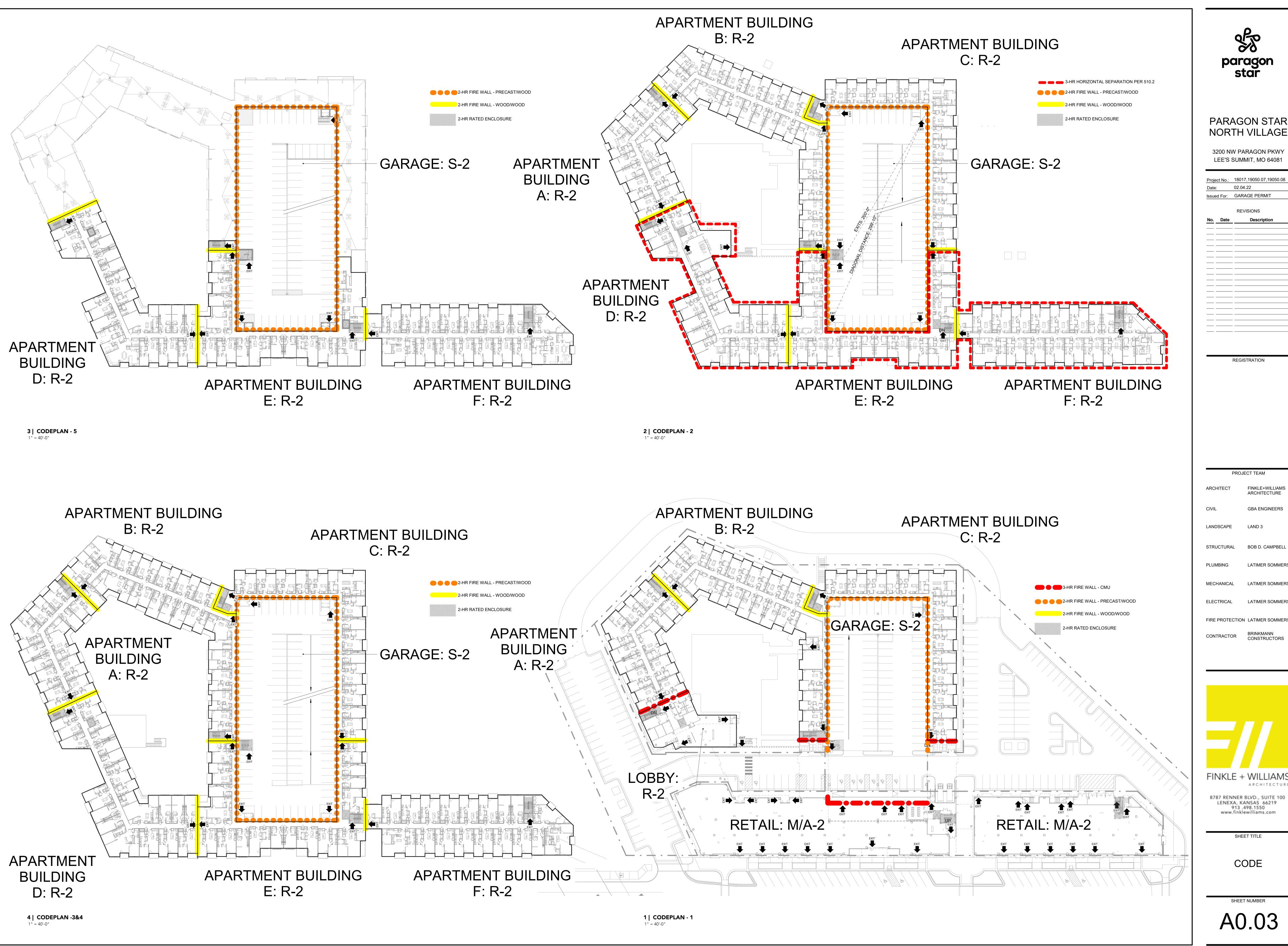
- 20 GA. 6" MTL. STUDS @

REQUIRED

SHEET TITLE LEGENDS &

GEN. NOTES SHEET NUMBER

Seal w/ acoustical sealant, top and botton





PROJECT TEAM

LATIMER SOMMERS LATIMER SOMMERS

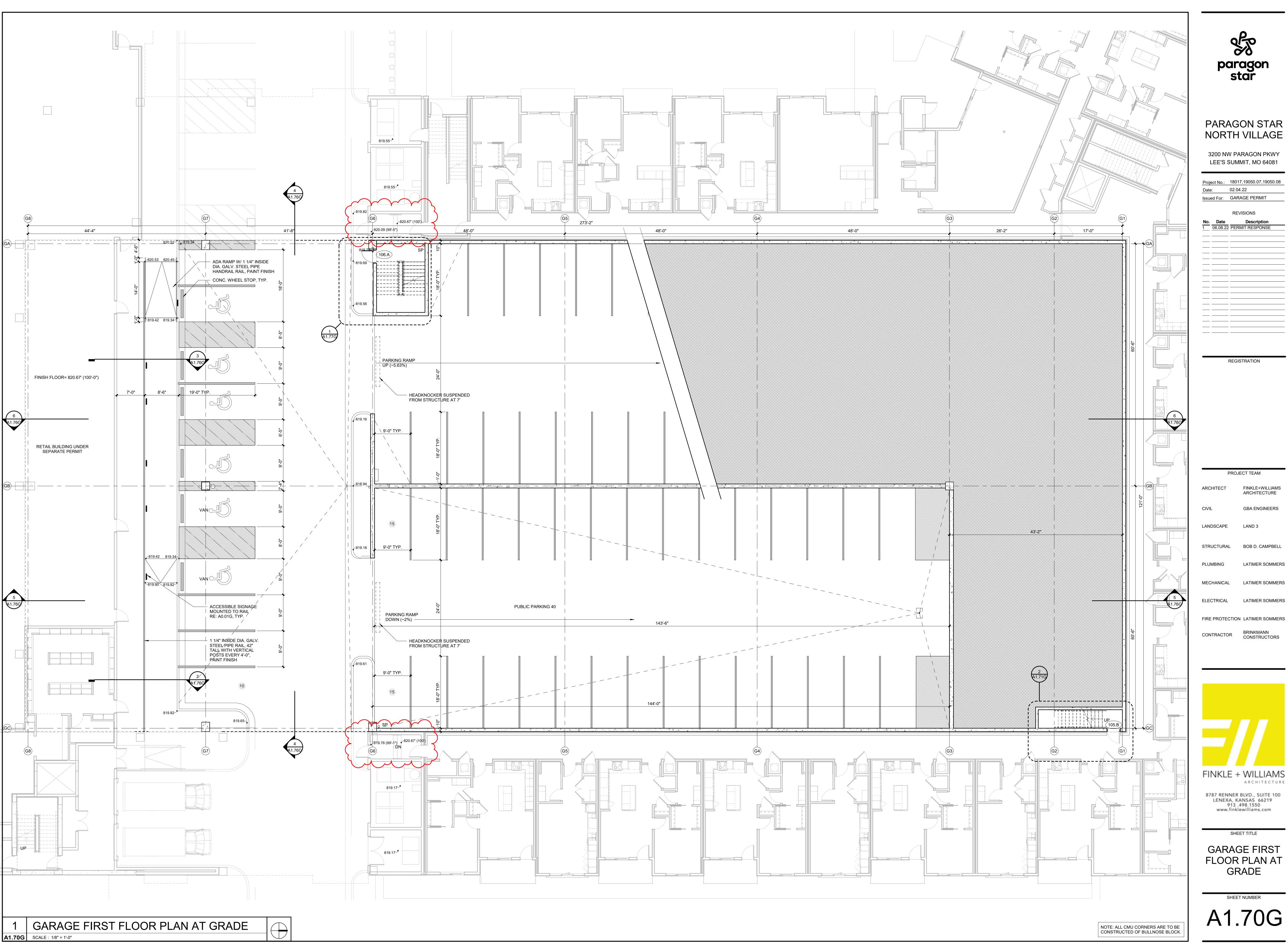


8787 RENNER BLVD., SUITE 100 LENEXA, KANSAS 66219 913 .498.1550 www.finklewilliams.com

SHEET TITLE

CODE

SHEET NUMBER A0.03





3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081

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MECHANICAL LATIMER SOMMERS

ELECTRICAL LATIMER SOMMERS

BRINKMANN CONSTRUCTORS CONTRACTOR

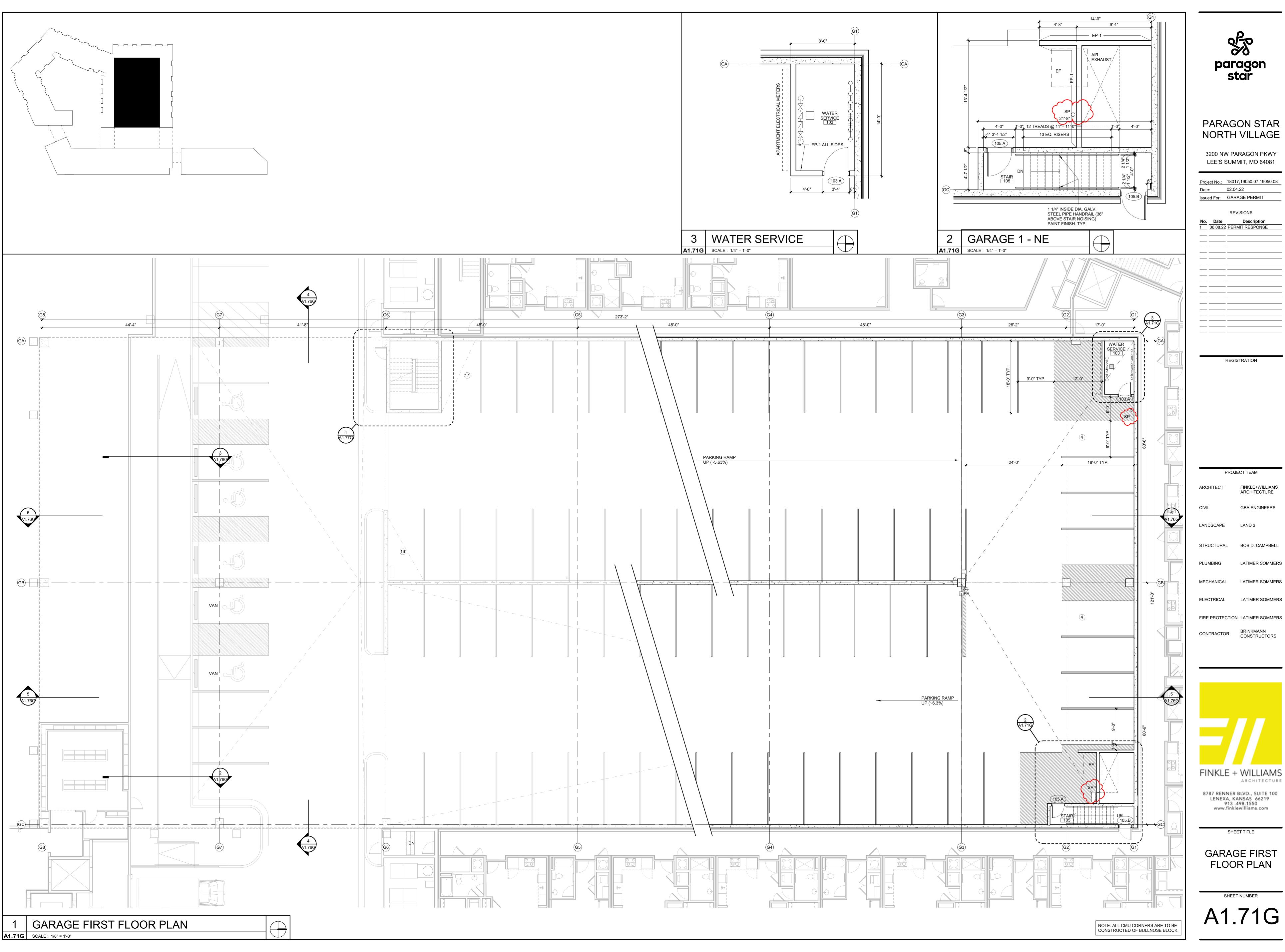
ARCHITECTURE

8787 RENNER BLVD., SUITE 100 LENEXA, KANSAS 66219 913 .498.1550 www.finklewilliams.com

SHEET TITLE

GARAGE FIRST FLOOR PLAN AT GRADE

SHEET NUMBER A1.70G





3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081

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LATIMER SOMMERS

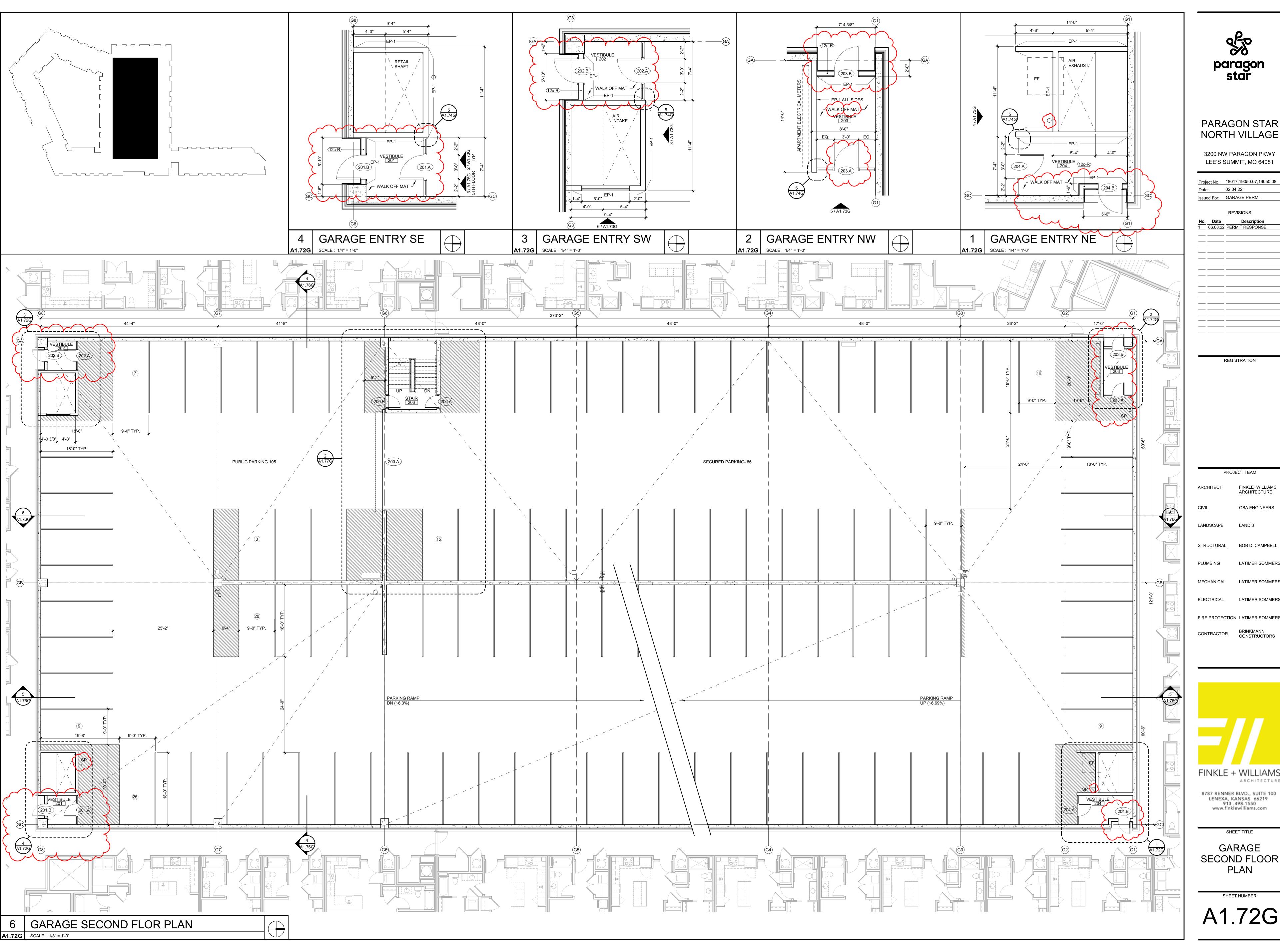
CONTRACTOR BRINKMANN CONSTRUCTORS

ARCHITECTURE

LENEXA, KANSAS 66219 913 .498.1550 www.finklewilliams.com

GARAGE FIRST FLOOR PLAN

SHEET TITLE





3200 NW PARAGON PKWY

Project No.: 18017,19050.07,19050.08

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LATIMER SOMMERS

MECHANICAL LATIMER SOMMERS

ELECTRICAL LATIMER SOMMERS

FIRE PROTECTION LATIMER SOMMERS

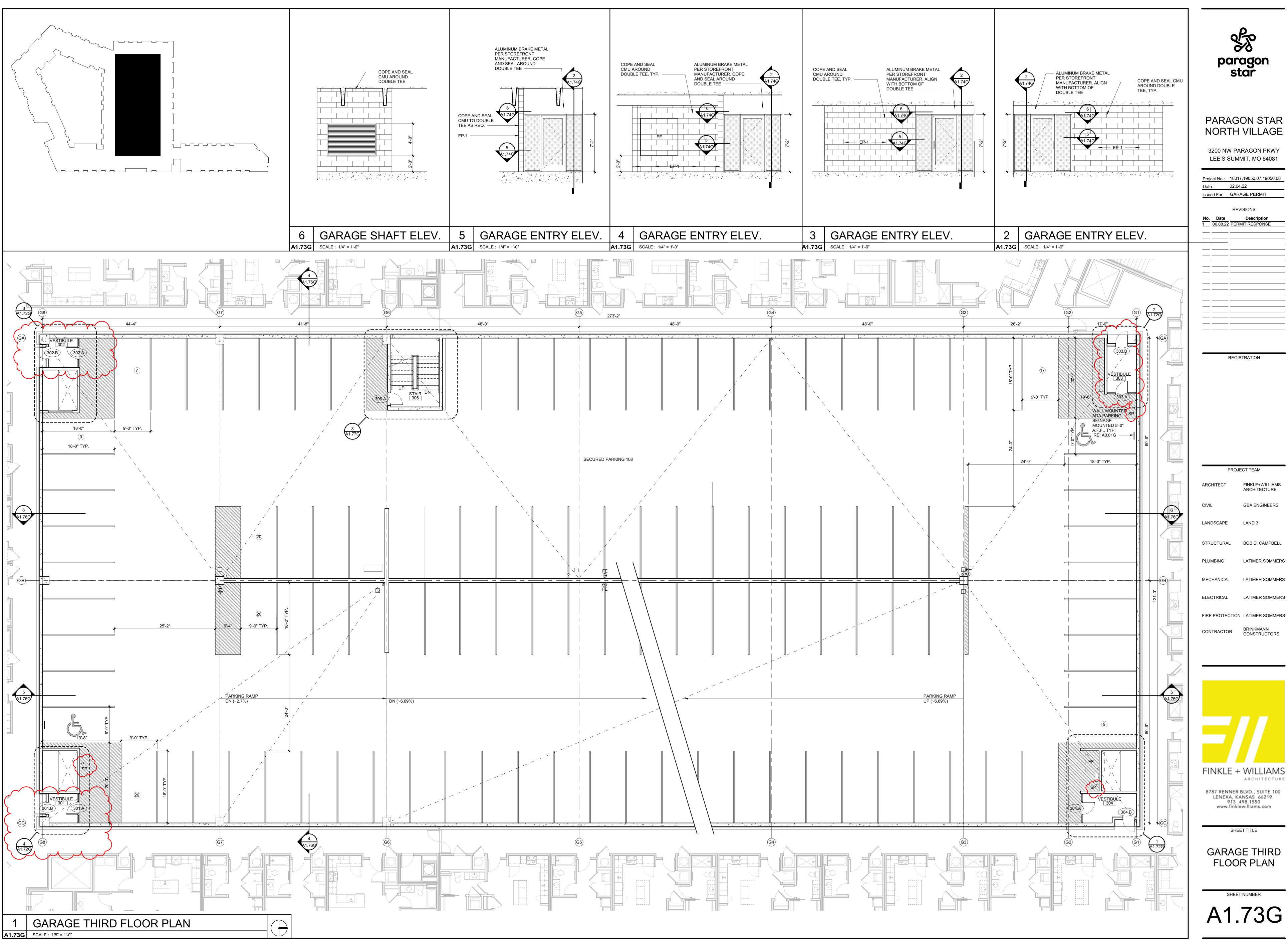
ARCHITECTURE

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SHEET TITLE

GARAGE SECOND FLOOR PLAN

SHEET NUMBER A1.72G





3200 NW PARAGON PKWY

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REVISIONS

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PROJECT TEAM FINKLE+WILLIAMS

LANDSCAPE LAND 3

GBA ENGINEERS

STRUCTURAL BOB D. CAMPBELL

LATIMER SOMMERS

CONTRACTOR

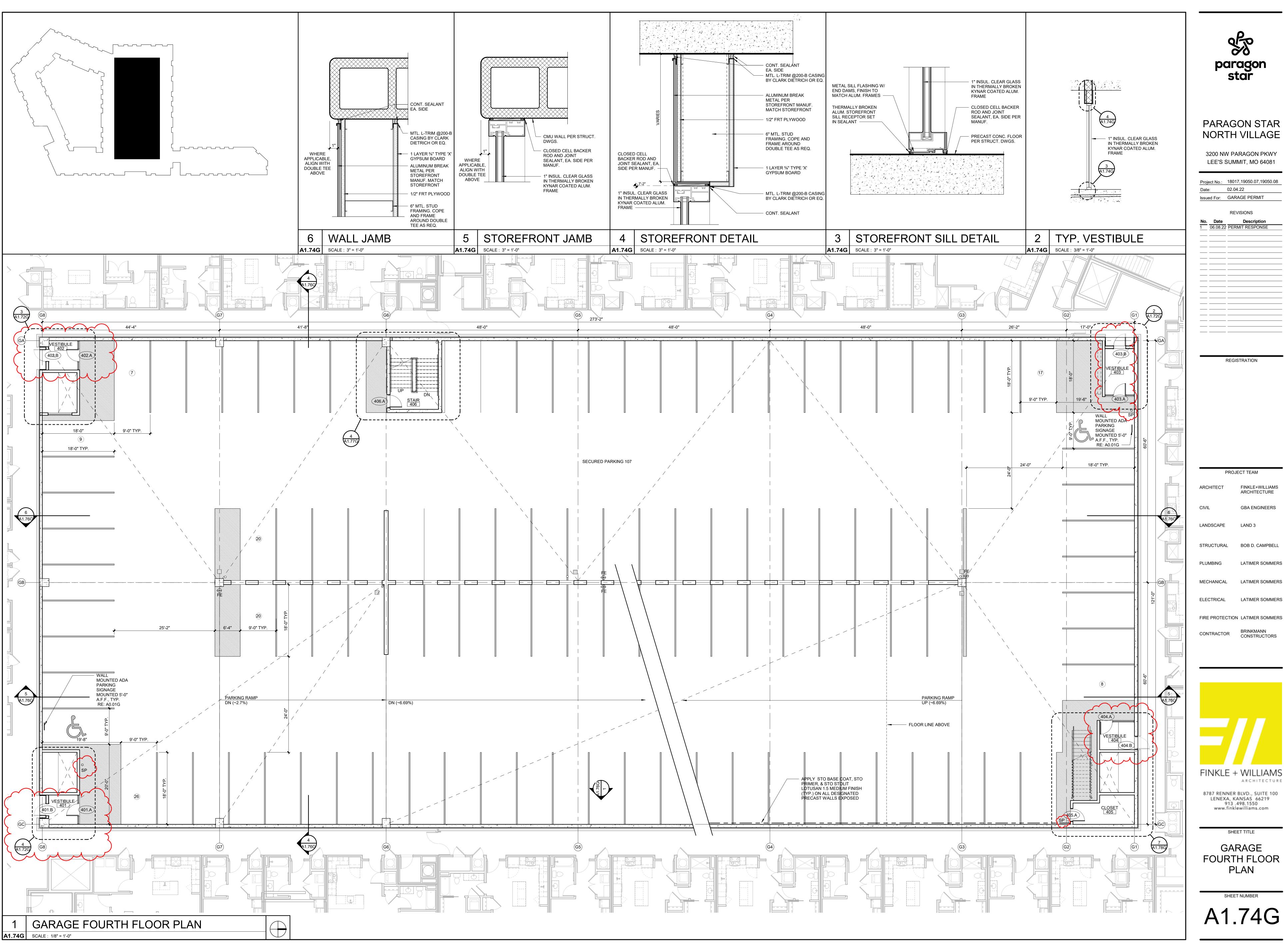
ARCHITECTURE 8787 RENNER BLVD., SUITE 100

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SHEET TITLE

GARAGE THIRD FLOOR PLAN

SHEET NUMBER A1.73G





3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081

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LATIMER SOMMERS

BRINKMANN CONSTRUCTORS CONTRACTOR



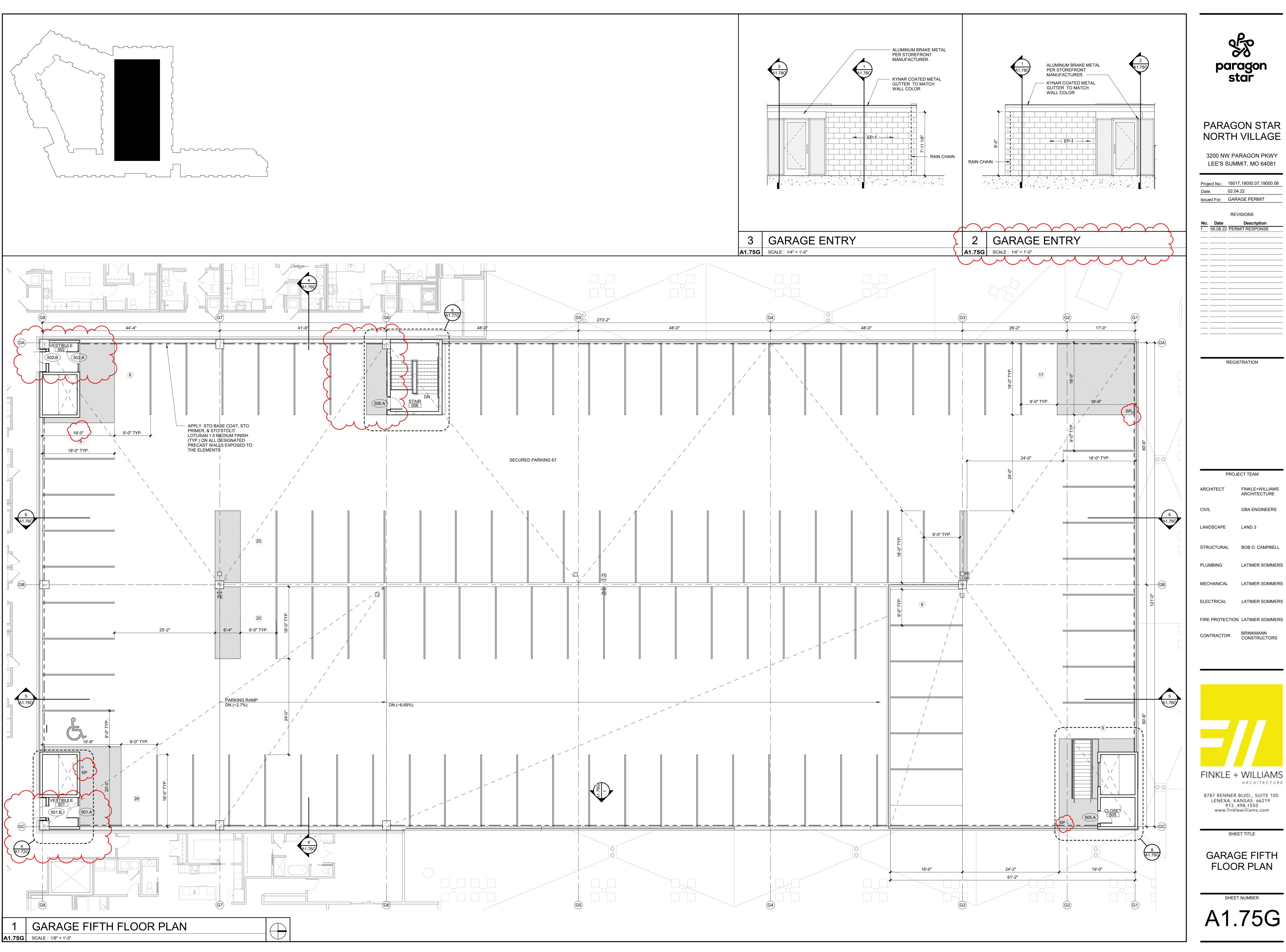
FINKLE + WILLIAMS ARCHITECTURE 8787 RENNER BLVD., SUITE 100

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SHEET TITLE

GARAGE FOURTH FLOOR PLAN

SHEET NUMBER A1.74G





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STRUCTURAL BOB D. CAMPBELL

LATIMER SOMMERS

MECHANICAL LATIMER SOMMERS

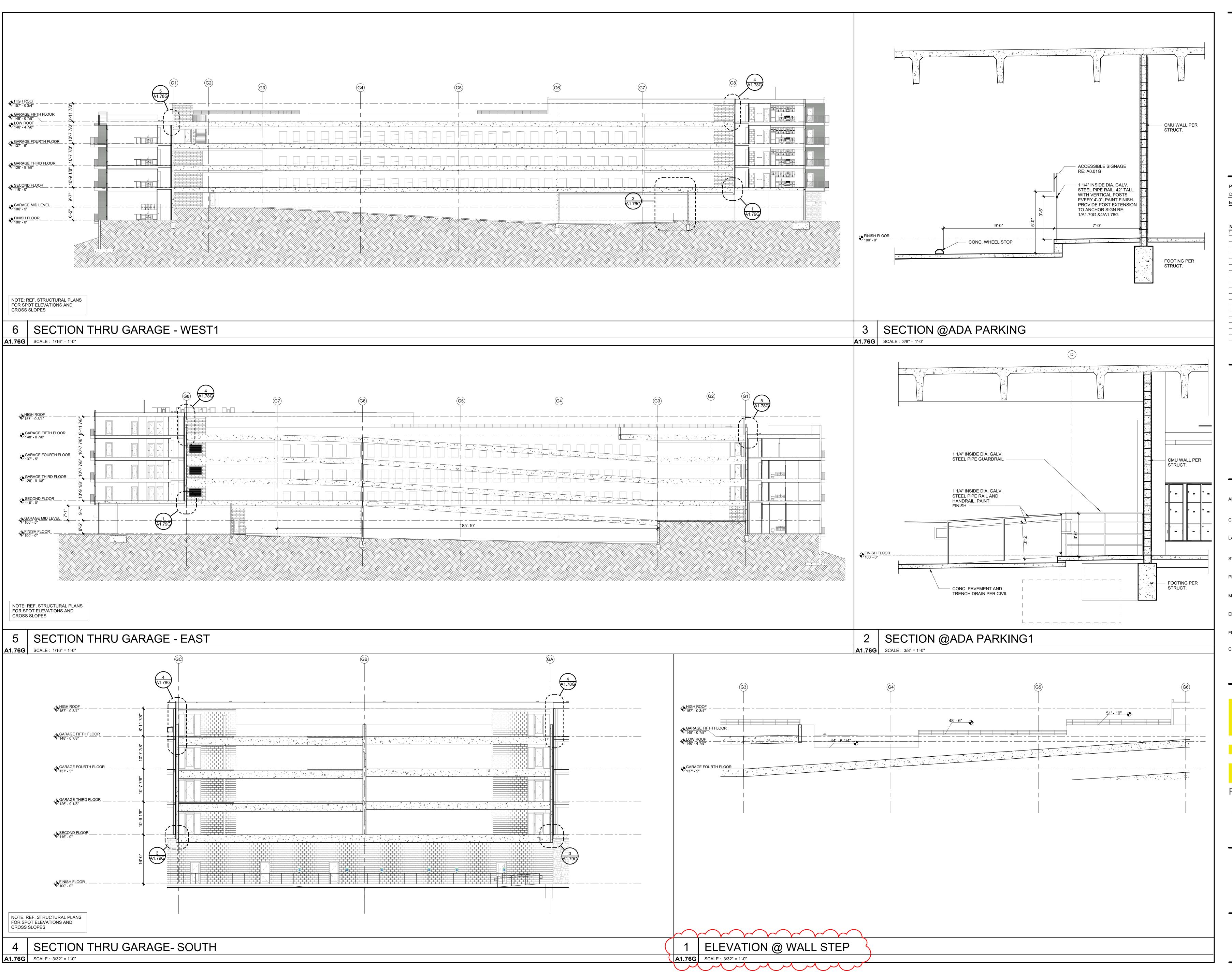
FIRE PROTECTION LATIMER SOMMERS

FINKLE + WILLIAMS

ARCHITECTURE 8787 RENNER BLVD., SUITE 100 LENEXA, KANSAS 66219 913 .498.1550 www.finklewilliams.com

SHEET TITLE

GARAGE FIFTH FLOOR PLAN





3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081

 Project No.:
 18017,19050.07,19050.08

 Date:
 02.04.22

 Issued For:
 GARAGE PERMIT

DESCRIPTIONS

DESCRIPTION

DESC

PROJECT TEAM

ARCHITECT FINKLE+WILLIAMS ARCHITECTURE

CIVIL GBA ENGINEERS

LANDSCAPE LAND 3

STRUCTURAL BOB D. CAMPBELL

PLUMBING LATIMER SOMMERS

MECHANICAL LATIMER SOMMERS

ELECTRICAL LATIMER SOMMERS
FIRE PROTECTION LATIMER SOMMERS

CONTRACTOR BRINKMANN
CONSTRUCTORS

INKLE + WILLIAMS

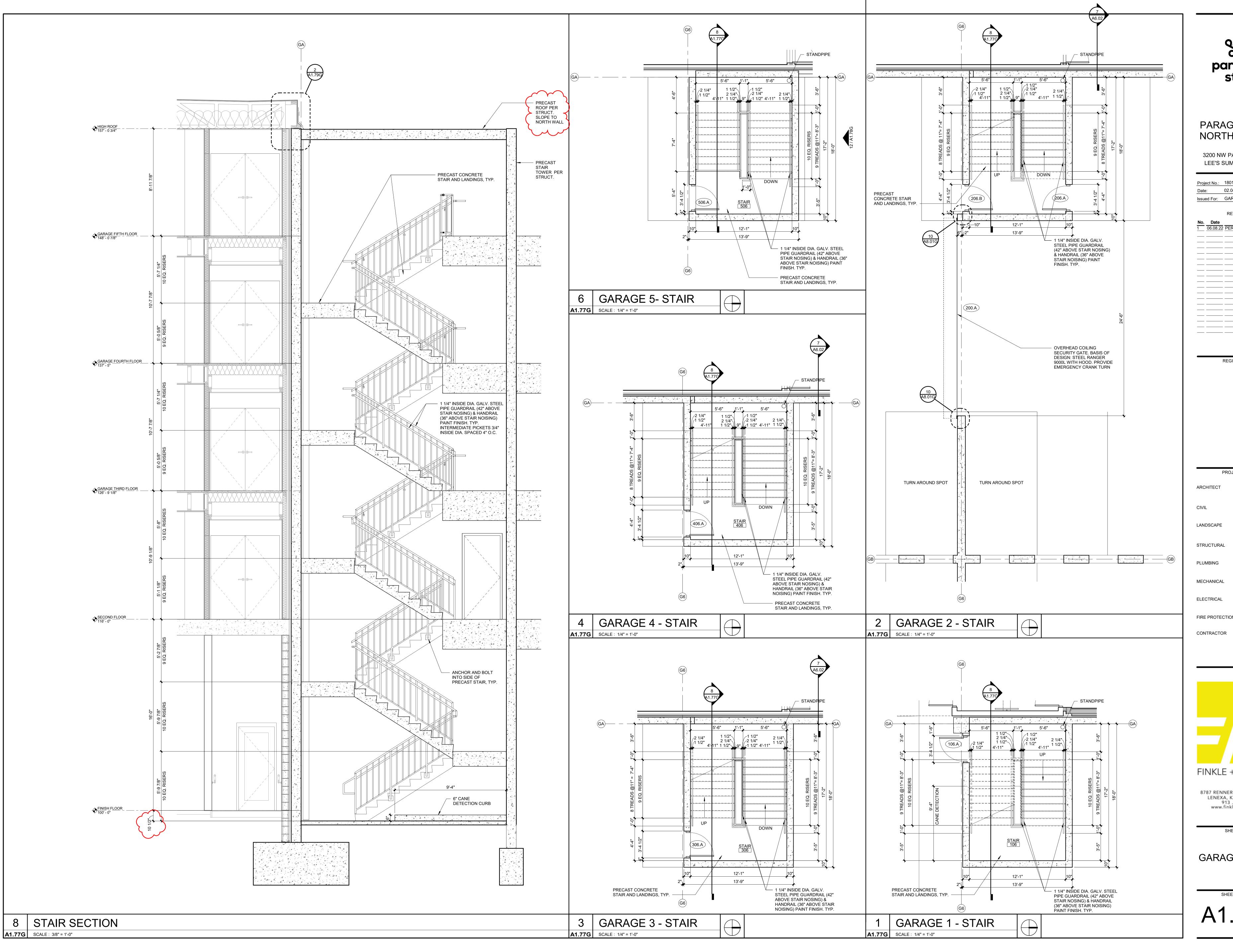
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SHEET TITLE

GARAGE SECTIONS

A1.76G





3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081

Project No.: 18017,19050.07,19050.08 02.04.22 Issued For: GARAGE PERMIT

REVISIONS

REGISTRATION

PROJECT TEAM FINKLE+WILLIAMS

ARCHITECT ARCHITECTURE **GBA ENGINEERS**

LANDSCAPE LAND 3

STRUCTURAL BOB D. CAMPBELL

LATIMER SOMMERS PLUMBING LATIMER SOMMERS MECHANICAL

ELECTRICAL LATIMER SOMMERS

FIRE PROTECTION LATIMER SOMMERS BRINKMANN CONSTRUCTORS



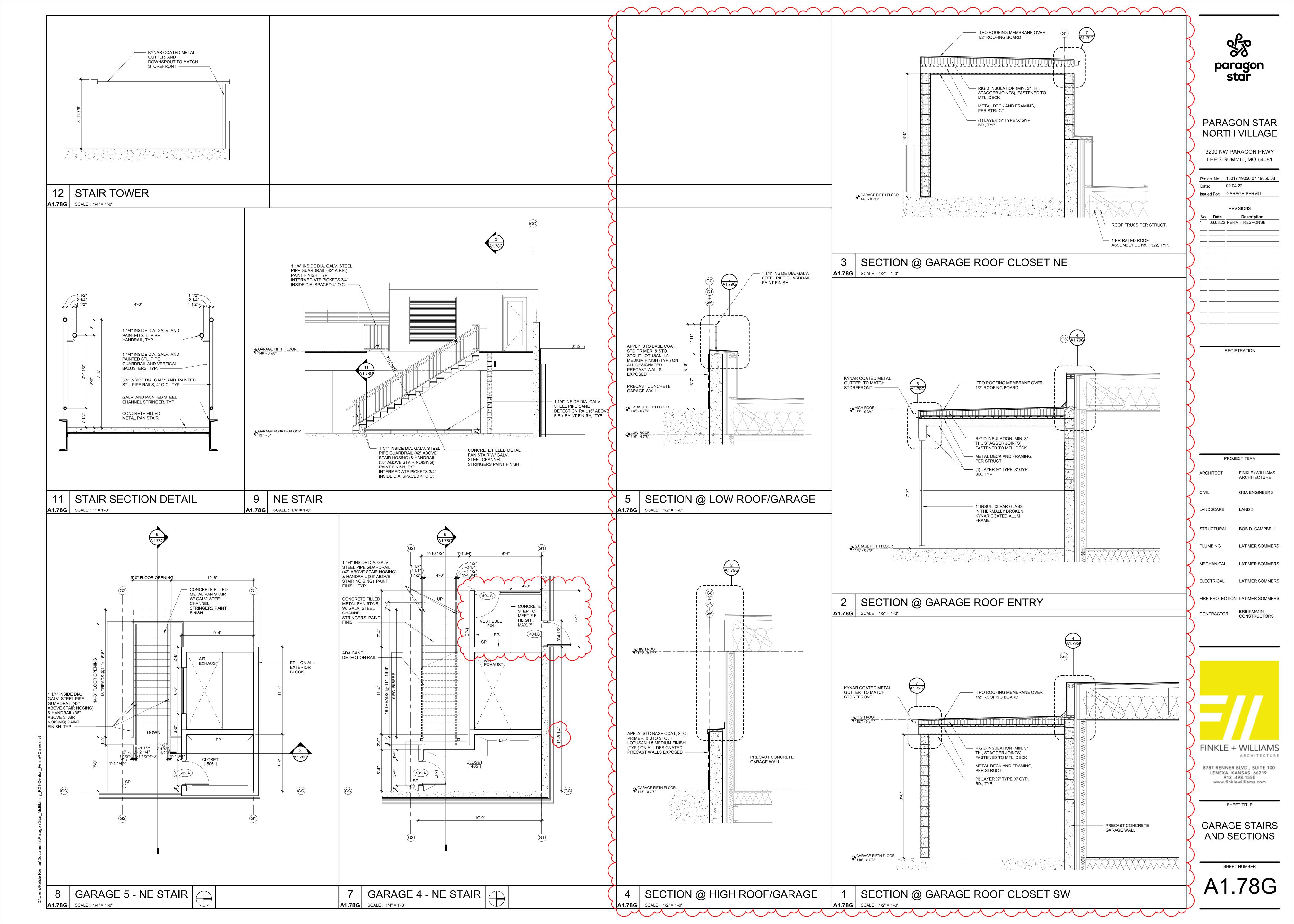
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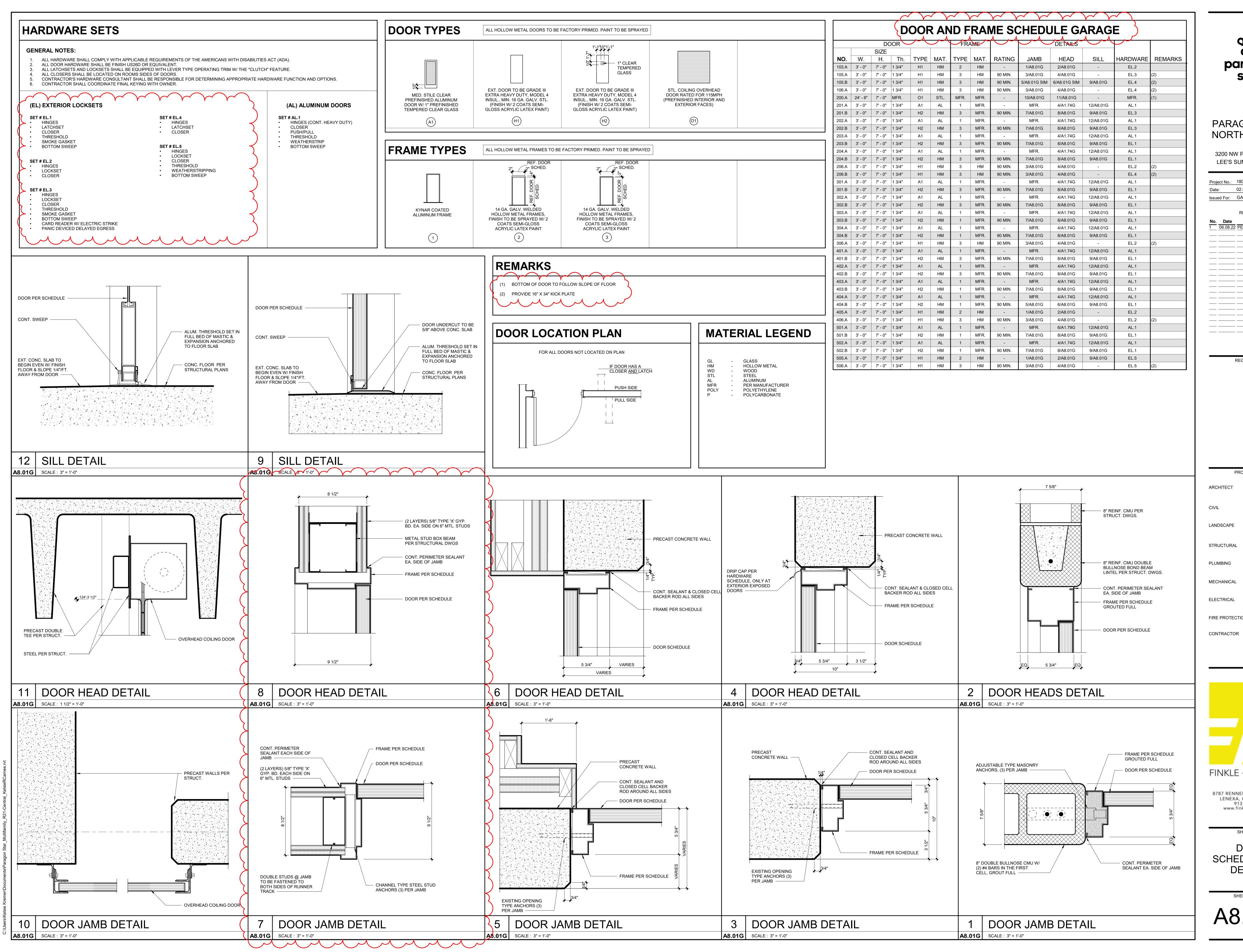
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SHEET TITLE

GARAGE STAIRS

SHEET NUMBER A1.77G





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SHEET TITLE DOOR

SCHEDULE AND DETAILS

SHEET NUMBER

A8.01G

before proceeding. All design and construction work for this project shall conform to the requirements of the following governing design codes: 1. International Building Code (IBC 2018) as amended by the city of Lee's Summit,

Minimum Design Loads for Buildings and Other Structures (ASCE7-16)

Member Design Basis is Allowable Stress Design (ASD) Connection Design Basis is Allowable Stress Design (ASD) 4. Structural Welding Code (AWS D1.4-17)

Specification for Structural Steel Buildings (AISC 360-16)

5. Building Code Requirements for Structural Concrete (ACI 318-14) Building Code Requirements for Masonry Structures (TMS 402-16)

. North American Specification for the Design of Cold-Formed Steel Structural Members (AISI S100-16) 8. National Design Specification (NDS) for Wood Constriction with 2018 Supplements (ANSI/AWC NDS-2018)

9. Special Design Provisions for Wind and Seismic (AWC SDPWS-2015) D. These drawings are for this specific project and no other use is authorized.

2. Structural Load Design Criteria

A. Dead Load: Floor, Apartment = 35 psf = 15 psf Floor, Balony Floor, Corridor (Above Podium) = 25 psf Garage Collateral = 5 psf = 25 psf Stair, Metal Pan = 60 psf Stair, Wood = 25 psf B. Live Load: = 40 psf Floor, Apartment = 60 psf Floor, Balcony Floor, Corridor (Serving Apartment) = 40 psf Floor, Corridor)Serving Public) = 100 psf Floor, Public (Clubhouse) = 100 psfFloor, Storage = 125 psf= 40 psf Garage = 20 psf Roof, MEP Equipment Zone = 45 psf = 100 psf

C. Snow: Pg = 20psf, Ce = 1.0

Pf = 14 psf (Apt) & 16.8 psf (Garage), Pm = 20 psf Is = 1.0, Cs = 1.0, Ct = 1.0 (Apt) & 1.2 (Garage) Drift & unbalanced snow loads per ASCE/SEI 7-16 D. Lateral Loads:

1.) Wind V(ult) = 109 mph, exposure B. lw=1.0 GCpi=+/-0.18 Design wind pressures to be used for the design of exterior component and cladding materials on the designated zones of wall and roof surfaces shall be per section 30.7 and Table 30.7-2 of ASCE/SEI 7-16. Tabulated pressures shall be multiplied by effective area reduction factors, exposure adjustment factors, and topographic factors where applicable

2.) Seismic: Ss = 0.099, S1 = 0.068, le=1.0, Site Classification D Seismic Design Category B Basic Seismic Force-resisting System:

A.2 - Ordinary Reinforced Concrete Shear Walls R = 4, Omega = 4, Cd = 2 1/2, V = 0.053W

At Apartments Above Podium: A.17 - Light-Framed Walls with Shear Panels of All Other Materials R = 2, Omega = 2 1/2, Cd = 2, V = 0.053W

At Precast Garage: A.6 - Ordinary Precast Shear Walls (N/S Direction) R = 3, Omega = 2 1/2, Cd = 3, V = 0.035W B.9 - Ordinary Precast Shear Walls (E/W Direction)

R = 4. Omega = 2 1/2. Cd = 4. V = 0.026WE. This project is designed to resist the most critical effects resulting from the load combinations of section 1605.3 of the International Building Code.

3. Concrete

A. All concrete for foundations (walls, grade beams, footings and piers) shall develop minimum ultimate compressive design strength of 3500 psi in 28 days, but not less than 500 pounds of cement shall be used per cubic vard of concrete regardless of strengths obtained, not over 6 gallons of water per 100 pounds of cement and not over 4 inches of slump. B. All concrete for interior flatwork (without floor covering) shall develop minimum

ultimate compressive design strength of 4000 psi in 28 days, but not less than 525 pounds of cement shall be used per cubic yard of concrete regardless of strengths obtained, not over 5.75 gallons of water per 100 pounds of cement and not over 4 inches of slump. Concrete mix shop drawing shall contain testing data proving concrete design mix shrinkage is less than 0.034% at 28 days when tested according to ASTM C157 (air drying method only).

All concrete for interior flatwork (with floor covering) shall develop minimum ultimate compressive design strength of 4000 psi in 28 days, but not less than 540 pounds of cement shall be used per cubic yard of concrete regardless of strengths obtained, not over 5.40 gallons of water per 100 pounds of cement and not over 4 inches of slump. Concrete mix shop drawing shall contain testing data proving concrete design mix shrinkage is less than 0.034% at 28 days when tested according to ASTM C157 (air drying method only).

All concrete for exterior flatwork shall have a minimum design compressive strength of 4500 psi in 28 days, with not less than 560 pounds of cement per cubic yard of concrete, not over 5 gallons of water per 100 pounds of cement, with 6% +/- 1% air entrainment, and a maximum of 4 inches of slump. All concrete for columns shall develop a minimum ultimate compressive design strength of 4000 psi in 28 days, but not less than 560 pounds of cement shall be

used per cubic yard of concrete regardless of strengths obtained, not over 5 gallons of water per 100 pounds of cement and not over 4 inches of slump. The preceding minimum mix requirements may have water-reducing admixtures conforming to ASTM C494 added to the mix at manufacturer's dosage rates for improved workability.

G. The preceding minimum mix requirements may have up to 15% maximum of the cement content replaced with an approved ASTM C618 Class C fly ash, provided the total minimum cementitious content is not reduced.

H. Combined aggregate (coarse plus fine) for all concrete shall be well graded from coarsest to finest with no more than 18 percent and not less than 8 percent retained on an individual sieve, except that less than 8 percent may be retained on coarsest sieve and on No. 50 and finer sieves. Submit this gradation report with the concrete mix design shop drawings. All interior concrete slabs on grade shall be placed over 15 mil, Class A Vapor

Barrier per ASTM E1745 with less than 0.01 perms, tested after mandatory conditioning. All joints shall be lapped and sealed per manufacturer's recommendations. All penetrations, as well as damaged vapor barrier material shall also be sealed per manufacturer's recommendation prior to concrete placement. Install barrier per manufacturer recommended details at all discontinuous edges (at interior columns, exterior edge of slab, etc.) to ensure terms of warranty are followed. The vapor barrier shall be placed over freedraining granular material as prescribed by the project soils report.

Basement foundation walls shall be braced at the base and top of wall by the contractor until the slab on grade at the base and the floor framing/slab at the top of wall is complete and the concrete has achieved 75% of the design strength. The contractor is responsible for engineering and design of the wall bracing, if K. All concrete is reinforced concrete unless specifically called out as unreinforced.

Reinforce all concrete not otherwise shown with same steel as in similar sections or areas. Any details not shown shall be detailed per ACI 315 and meet requirements of ACI 318, current editions. Control joints in dirt formed slab to be as shown on plans. Where not shown, limit

controlled areas to not more than 144 square feet, or 12 feet on any side. Slab panel side ratio shall not exceed 1 1/2 to 1. M. Contractor shall verify that all concrete inserts, reinforcing and embedded items

are correctly located and rigidly secured prior to concrete placement. N. Construction joints in beams, slabs, and grade beams shall occur at midspan (middle third) unless noted otherwise. Provide 2 x 4 horizontal keys at construction joints for shear transfer. O. No aluminum items shall be embedded in any concrete.

4. Reinforcing Steel

A. All reinforcing steel shall conform to the requirements of ASTM A615 or A706 grade 60 steel. Welded plain wire fabric shall be supplied in sheets and conform

to the requirements of ASTM A185. B. Clear minimum coverage of concrete over reinforcing steel shall be as follows: Concrete placed against earth: 3"

Formed concrete against earth: 2' Slabs: 1-1/2" Beams or Columns:

Other All coverage shall be nominal bar diameter minimum.

All dowels shall be the same size and spacing as adjoining main bars (splice lap 48 bar diameters or 24" minimum unless noted otherwise). . At corners of all walls, beams, and grade beams supply corner bars (minimum 2'-0" in each direction or 48 bar diameters) in outside face of wall, matching size and spacing of horizontal bars. Where there are no vertical bars in outside face of wall,

supply 3 - #4 vertical support bars for corner bars. Bars marked continuous and all vertical steel shall be lapped 48 bar diameters (2'-0" minimum) at splices and embedments, unless shown otherwise. Splice top bars near midspan and splice bottom bars over supports, unless noted otherwise. At all holes in concrete walls and slabs, add 2 - #5 bars (opening dimension plus 96 diameters long) at each of four sides and add 2 - #5 x 5'-0" diagonally at each of four corners of hole. Openings in 8" thick walls are reinforced similar, but with 1 - # 5 instead of 2 - #5, respectively.

G. Unless otherwise covered on architectural plans or specifications, vertical control joints in concrete wall shall be spaced at a maximum of 20'-0" on center and coordinated with the architect. Every other horizontal wall reinforcing bar shall be discontinuous at control joints except heavy top and bottom bars unless noted otherwise. Provide base seal waterstop style number 772 (by Greenstreak Inc. or approved equal) on dirt face side of wall at all walls below grade.

H. Accessories shall be as specified in latest edition of the ACI Detailing Handbook and the concrete Reinforcing Steel Institute Design Handbook. Maximum accessory spacing shall be 4'-0" on center, and all accessories on exposed surfaces are to have plastic coated feet.

I. All slabs and stairs not shown otherwise shall be 6" thick with #4 bars at 12" on center each way. All exterior porches and stoops not otherwise detailed may be constructed in any standard manner, solid or hollow, but must be reinforced with #4 bars at 12" on center each way minimum. Porches shall be doweled to adjacent walls or grade beams with #4 bars at 12" on center, hooked or embedded 48 diameters into both members. Slope porches 1/8" per foot for drainage unless noted otherwise.

J. Allow 2 tons of reinforcing bars #4 or larger to be used as directed in the field for special conditions by the engineer of record (labor for placing same to be included).

5. Structural Steel

A. All structural steel beams and columns shall be ASTM A992, grade 50 steel and all miscellaneous steel shall be ASTM A36 grade steel (except at moment connections where plates shall be ASTM A572, grade 50). Hollow Structural Sections (HSS) shall be ASTM A500, grade C. Fabrication and erection shall be in accordance with AISC 303-05 "Code of Standard Practice for Steel Buildings and Bridges" in the 13th Edition of the AISC Steel Construction Manual.

B. All welding shall conform to the recommendations of the AWS. C. All exterior steel and connections, and brick relief angles shall be hot-dip galvanized. All bolts not otherwise specified shall be 3/4" diameter high strength (ASTM A325-N).

All bolts shall be fully pretensioned. All beam connections shall be designed per the AISC Manual of Steel Construction "Framed Beam Connections" for the indicated reactions or at least 0.4 x beam total shear capacity, Vn/Omega, shown in the maximum total uniform load tables, whichever is greater; and, shall account for eccentricity when the bolt line is more than 2" from the center of the support. All connections must be two bolt minimum. Additional connection elements may not be specifically shown in the conceptual details in this set but may be required by the final connection design, such as stiffener plates, doubler plates, supplement/reinforcing plates or other connection material. Connection design and shop drawing preparation shall be completed under the direct supervision of a professional engineer licensed in the state the project is located and shop drawings and connection calculations shall bear his/her seal.

E. All anchor bolts shall be 3/4" diameter, ASTM F1554, Grade 36 unless noted otherwise. Washers of minimum size and thickness for the given anchor diameter in Table 14-2 of the AISC Steel Construction Manual shall be provided at every column anchor bolt. Washers shall have a standard size hole for the anchor bolt. At braced frames washers shall be welded all around to the column base plate with 3/16" fillet weld.

F. Allow 2.0 tons structural steel to be used as directed in field for special conditions by the engineer of record. Cost for shop drawings, fabrication, delivery, detailing, and erection to be included. 50% of structural steel allowance shall be bid as miscellaneous galvanized angle and plate.

6. Post Installed Anchors

A. Post-installed anchors shall be used only where specified on the drawings unless approved in writing by the engineer of record. See drawings for anchor diameter spacing and embedment. Performance values of the anchors shall be obtained for specified products using appropriate design procedures and/or standards as required by the governing building code. Anchors installed in concrete shall have an ICC-ES Evaluation Service Report. Special inspection is required for all post installed anchors. The contractor shall coordinate an on-site meeting with the post installed anchor manufacturer field representative to educate the construction team on the anchor

installation guidelines and requirements. B. Mechanical anchors used in cracked and uncracked concrete shall have been tested and qualified for use in accordance with ACI 355.2 and ICC-ES AC193. All anchors shall be installed per the anchor manufacturer's written instructions.

C. Adhesive anchors used in cracked and uncracked concrete shall have been tested and qualified for use in accordance with ICC-ES AC308. All anchors shall be installed per the anchor manufacturer's written instructions. D. Mechanical anchors used in solid grouted masonry shall have been tested and

qualified for use in accordance with ICC-ES AC01. All anchors shall be installed per the anchor manufacturer's written instructions. E. Adhesive anchors used in solid grouted masonry shall have been tested and qualified

for use in accordance with ICC-ES AC58. All anchors shall be installed per the anchor manufacturer's written instructions. F. Anchors used in hollow concrete masonry shall have been tested and qualified in accordance with ICC-ES AC106 or ICC-ES AC58 as appropriate. All anchors shall be installed per the anchor manufacturer's written instructions with appropriate screen

7. Foundations

tubes used for adhesives

A. The soil investigation was prepared by Terracon, the report number is 02215169 and the telephone number is 913-492-7777 B. Structural foundations consist of a network of stright shaft auger pressure grouted piles established on moderatly weathered shale capable of safely supporting 40ksf end bearing. Each pile shall penetrate 5'-0" minimum into the moderately weathered shale. Spread footing and shallow foundations for ancillary structures are designed to bear on

engineered fill or undisturbed soil capable of safely supporting 1,500 psf. Retaining walls are designed for an active lateral load of 50 pcf equivalent fluid pressure. D. Basement walls are designed for an at rest lateral load of 70 pcf equivalent fluid

pressure. See General Note 3.J for wall bracing requirements. E. Contractor shall provide for dewatering at excavations from either surface water or F. All foundation excavations shall be inspected by a qualified soil engineer, approved by

the architect and/or structural engineer, prior to placement of steel or concrete. This inspection shall be at the owner's expense. G. All concrete in the structural portion retaining the backfill shall have attained its design strength prior to being backfilled.

H. Moisture content in soils beneath building locations should not be allowed to change after footing excavations and after grading for slabs on grade are completed. If subgrade materials become desiccated or softened by water or other conditions, recompact materials to the density and water content specified for engineered fill. Do not place concrete on frozen ground.

8. Drilled Auger Pressure-Grouted Piers

A. Piers not otherwise indicated shall be 30" diameter.

B. All piers shall have (4) #7x6'-0" hooked dowels unless otherwise indicated. C. Pier dowels shall extend 40 diameters above top of pier. Driving dowels into concrete after initial set is not allowed

D. Refer to the specifications (sections for excavation and concrete) for other detailed requirements

E. Pier concrete to have 6" slump.

9. Concrete Masonry Units

A. Concrete block used in exterior walls or load bearing walls shall meet the requirements of ASTM C90 and have a minimum net compressive strength of 2650 psi and laid up using type N mortar such that f'm equals 2000 psi. Mortar shall be volume proportion based cement lime mortar. Proportioning shall be completed by box measure. Any block in contact with earth shall be normal weight units, laid using type "S" mortar and arouted solid.

B. The contractor shall provide adequate temporary bracing for all masonry walls during construction. C. All concrete block shall have 9 gage (or larger) horizontal joint reinforcing (ladder

or truss) per architectural drawings and specifications (16" maximum vertical spacing). D. Cavity wall construction shall be reinforced as designed for specific concrete block used. The horizontal joint reinforcing shall be of the ladder or truss style per specification and continuous between brick and block, as prescribed by the architectural drawings.

E. Concrete block shall be reinforced as follows in 6", 8", 10", and 12" walls: . Vertical reinforcing shall be a minimum of 1 - #4 bar in 6" and 8" walls and 2 - #4 bars in 10" and 12" walls at 4'-0" on center, at each corner, at each door and window jamb, each side of control joints and in the end void of each length of wall. Lap splices for masonry vertical reinforcing shall be 48 bar diameters or 24" Horizontal reinforcing:

A. Horizontal joint reinforcing as noted above. B. Continuous horizontal bars shall be included per section or detail in bond beam or optional running bond beam where noted. Where bond beams are continuous at corners of walls, supply corner bars matching size of horizontal bars (minimum 2'-0" or 40 bar diameters in each direction). F. Grout, where noted above, shall have a minimum design ultimate compressive strength

of 2500 psi at 28 day test and 3/8" maximum aggregate size. G. Non-load bearing concrete block walls shall be isolated from adjacent structural elements with vertical 3/8" control joints and at the top of the wall with 1" air space or compressible material and support per architectural detail. H. Unless otherwise covered on architectural plans or specifications, vertical control joints

in masonry construction shall be 3/8" wide, full height of wall. Joints shall be spaced at a maximum of 24'-0" on center and coordinated with the architect. All horizontal joint reinforcing shall be discontinuous at control joints in masonry. All bond beam horizontal reinforcing shall be continuous through control joints. I. Lintels over all openings up to 8'-0" wide in new and existing masonry walls not

otherwise covered shall be one 6x3 1/2x5/16 angle for each 4" width of masonry. All exterior lintels to be galvanized. J. Walls shall be anchored top and bottom by dowels matching wall vertical reinforcing(unless noted otherwise) from floor slab bottom and bracing angles at the

10. Light Gage Metal Structural Framing

top, per details on the drawings.

A. All load bearing, light gage structural studs, track, and bridging shall be of the

type, size, gage, and spacing as shown on the plans, minimum. B. All materials shall be 33,000 psi minimum yield, except studs of 16 gage or heavier shall have a minimum yield of 50,000 psi. C. All properties, fabrication, and erection shall be in accordance with latest editions of

the AISI "Specifications for the Design of Cold-Formed Structural Members." D. All framing components shall be cut squarely or at an angle to fit squarely against abutting members. Splicing of axially loaded members is not permitted. Members shall be held firmly in place until properly fastened. Attachments of similar components shall be by welding, screw attachment, or bolting. Wire

tying of components is not permitted. E. Tracks shall be securely anchored to floor and overhead members. Special anchorage requirements required for wind bracing shall be as shown on the plans. F. Prior to fabrication and/or erection, the contractor shall submit shop drawings complete with detail of erection, fabrication, attachments, anchorages, lintels, etc., for review by the architect/engineer.

11. Timber and Wood Framing

A. Quality and construction of wood framing members and their fasteners for load supporting purposes not otherwise indicated on the drawings shall be in accordance with the 2018

International Building Code. B. All studs and top and bottom plates shall be Douglas Fir No. 2 grade visually graded lumber, with an allowable fiber stress in bending of 900 psi minimum and an elastic modulus of 1,600,000psi unless noted otherwise. All joist, truss members, and headers to be No. 2 grade (min.) unless noted otherwise. All lumber for exterior decks and balconies shall be treated Southern Yellow Pine No. 2 grade.

C. Bridging of stud bearing walls and shear walls shall be solid, matching sheathing joints. D. Joist blocking and bridging shall be solid wood or cross bridging of either wood or metal

straps. Spacing, in any case, shall not exceed 8'-0". E. Wood members and sheathing shall be fastened with number and size of fasteners not less than that set forth in Table 2304.9.1 of the 2018 International Building Code. Floor sheathing shall be APA rated tongue and groove Sturd-I-Floor, exposure 1, glued and nailed with 10d nails or # 10 screws at 12" on center field. Sheathing of shear walls or roof diaphragms shall be edge nailed with 8d common nails at 6" on center and nailed to intermediate framing and/or blocking members with 8d common nails at 12" on center unless otherwise noted on the drawings.

re: shearwall schedule.) Provide plate washers at sill plate anchors for shearwalls per shearwall schedule. Plates in direct contact with concrete or masonry shall be preservative-G. All hangers, ties and connections shown are based on Simpson Strong Tie as the basis of design, provide Simpson Strong Tie or an approved equal. Joist hangers shall be equal to "LUS" for wood application and "LB" for steel weld-on application. Roof truss ties shall be

F. Sill plates shall be bolted to concrete slabs with 1/2" diameter bolts at 32" on center (UNO,

equal to "H2.5A" and tie the roof truss to the top plate (provide (2) "H2.5A" diagonally across from each other when uplift load shown in truss shop submittal exceeds 600lbs). Roof girder ties shall be equal to a "LGT2", "LTG3" or "LGT4" tie (dependent on number of plies) and tie the truss girder to the top plate. Provide "H2.5A" at the top of each stud to top track when the top track has roof truss attached. H. Service condition - dry with moisture content at or below 19% in service.

I. Laminated strand lumber (LSL) shall have an allowable flexural stress (Fb) of 1,700 psi (reduced by size factor) and an elastic modulus (E) of 1,300,000 psi. J. Laminated veneer lumber (LVL) shall have an allowable flexural stress (Fb) of

2,600 psi (reduced by size factor) and an elastic modulus (E) of 2,000,000 psi. K. Parallel Strand Lumber (PSL) shall have an allowable flexural stress (Fb) of 2,900 psi (reduced by size factor) and an elastic modulus (E) of 2,000,000 psi. ((E) = 2,200,000 psi

L. Glulams shall be 24F-V8 or better with an allowable flexural stress (Fb) of 2,400 psi and an elastic modulus (E) of 1,800,000 psi. Exterior glulams shall be moisture-resistant treated. M. Pre-engineered wood trusses shall be designed in accordance with the Truss Plate Institute's national design standard for metal-plate connected wood truss construction (ANSI/TPI-1 latest edition). Trusses shall be designed and manufactured by an authorized member of the Wood Truss Council of America (WTCA). Truss design shall conform to specified codes, allowable stress increases, deflection limitations and other applicable criteria of the governing code.

N. Truss shop drawings showing complete erection and fabrication details and calculations (including connections) shall be submitted to the project architect/engineer for review prior to fabrication and/or erection. Calculations and layout plan shall bear the seal of a professional engineer, registered in the state of the project location. Layout plan shall incldue truss locations, spacing and all hanger designations used to support trusses to beams or other trusses. Calculations shall indicate max reactions in all directions, number of plies for the truss and dead, live and total load deflections along with a list or key of all standard and nonstandard utilized load combination. Shop drawings shall also be submitted to the local government controlling agency when requested by that agency.

O. All trusses shall be securely braced both during erection and permanently, as indicated on the approved truss design drawings and in accordance with TPI's commentary and recommendations for handling, installing and bracing metal-plate connected wood trusses (HIB-91, booklet) and the latest edition of ANSI/TPI-1. P. The truss manufacturer shall supply all hardware and fasteners for joining truss

members together and fastening truss members to their supports. Metal connector plates shall be manufactured by a member of the Wood Truss Council of America (WTCA) and shall be 20 gauge minimum. Connector plates shall meet or exceed ASTM A653, grade 33, with ASTM A924 galvanized coating designation G60. Q. Provide truss space directly above and centered over HVAC closets. Refer to Architectural and MEP drawings for exact locations. R. Shipment, handling, and erection of trusses shall be by experienced, qualified persons and

shall be performed in a manner so as not to endanger life or property. Apparent truss

damage shall be reported to the truss manufacturer for evaluation prior to erection. Cutting or alteration of trusses is not permitted. S. Pre-Engineered Floor Trusses: Top Chord Dead Load = 30 psf= Per General Note 5B Top Chord Live Load = 10 psf Bottom Chord Dead Load Live Load Deflection = L/480; (1/2" max) Total Load Deflection = L/360

Roof Truss Design Criteria: = 15 psf (TPO Roof) Top Chord Dead Load = 20 psf (Plus Rooftop Equipment) Top Chord Live Load Top Chord Snow Load = 20 psf or 14 psf plus Drift Bottom Chord Dead Load Bottom Chord Live Load = 5 psf Live Load Deflection = L/360

Total Load Deflection = L/300U. Roof trusses shall be designed per IBC 2018 for net uplift resulting from wind loading as calculated using components and cladding loading. Top and bottom chord dead load used in combination with wind uplift shall be 5psf for each chord.

V. Construction bracing shall be provided by the contractor as required to keep the building and W. Structural members shall not be cut for pipes, etc., unless specifically detailed. Nothing and boring of studs and top of plates shall conform to the provisions of section 2308.9.10 and 2308.9.11 of the IBC. Where top plates or sole plates are cut for pipes, a metal tension tie with minimum 0.058 inches thick and 1 1/2" inches wide shall be fastened to each plate across and to each side of the opening with not less than (6) 16d nails, in accordance with

section 2308.9.8 of the IBC. X. All fasteners for wood to wood connections and wood connectors shall be as indicated in structural drawings or manufacturer literature to achieve full capacity of connector. Alternate fasteners may be submitted as a substitution request. Submittal must show that alternative fasteners will not reduce the capacity of the connection.

12. Precast Concrete Members

A. The contractor/supplier is responsible for the design of all the precast members and connection between them and other structural members. Submit design calculations, sealed by an engineer licensed in the state of the project location, for review by the

architect/engineer of record. B. All precast members are to be designed in accordance with ACI 318-11, 2012 IBC and other applicable codes, standards (see specs) and design criteria shown on design C. Precast concrete members shall conform to the 2012 IBC for the required fire ratings

(refer to architects documents). D. All wall panels should be designed for building wind loads, seismic loads, gravity loads, and transmit these loads to the foundation through properly designed connections. E. Provide blockouts and openings for mechanical/electrical equipment. Refer to

mechanical/electrical documents. Shop drawings shall be complete and shall include a layout plan, fabrication details, estimated camber, connection and anchorage details and member identification marks. Identification marks shall appear on manufactured units to facilitate correct field

13. Deferred Submittal and Shop Drawing

or submissions without GC approval stamp.

need not be submitted.

A. Bob D. Campbell and Company, Inc. will review the General Contractor's (GC) shop drawings and related submittals (as indicated below) with respect to the ability of the detailed work, when complete, to be a properly functioning integral element of the

overall structural system designed by Bob D. Campbell and Company, Inc. B. Deferred submittals shall be submitted to the architect of record for review who shall forward to the building official for review and approval. Design calculations for deferred sub mittals shall be submitted at the same time as the shop drawings for review. Design calculations shall be prepared and sealed by a Professional Engineer licensed in the

state of the project. The deferred submittal items shall not be installed until the deferred submittal documents have been approved by the building official. Prior to submittal of a shop drawing or any related material to Bob D. Campbell and Company, Inc., the GC shall: 1. Review each submission for conformance with the means, methods, techniques, sequences and operations of construction and safety precautions and programs incidental thereto, all of which are the sole responsibility of the GC.

2. Review and approve each submission. . Stamp each submission as approved. D. Bob D. Campbell and Company, Inc. shall assume that no submission comprises a

variation unless the GC advises Bob D. Campbell and Company, Inc. with written E. Bob D. Campbell and Company, Inc. shall review shop drawings and related materials with comments provided that each submission has met the above requirements. Bob D. Campbell and Company, Inc. shall return without comment unrequired material

F. Shop drawings and related material (if any) required are indicated below. Should Bob D. Campbell and Company, Inc. require more than ten (10) working days to perform the review, Bob D. Campbell and Company, Inc. shall so notify the GC. 1. Concrete mix designs and material certificates including admixtures and compounds applied to the concrete after placement.

Reinforcing steel shop drawings including erection drawings and bending details.Bar list will not be reviewed for correct quantities. 3. Elevations of all reinforced concrete masonry walls at a scale no smaller than 3/8" = 1'-0" showing all required reinforcing. Grout mix designs (for CMU).

5. Construction and control joint plans and/or elevations. 6. Structural steel shop drawings including erection drawings and piece details. Include joist, decking and connector submittals. Include miscellaneous framing specified on the structural drawings, but do not submit framing specified on nonstructural drawings for Bob D. Campbell and Company, Inc. review. Defferred Submittal: Structural steel connections

8. Defferred Submittal: Railings and guardrails 9. Defferred Submittal: Metal stair framing 10. Defferred Submittal: Exterior cold-formed metal framing 11. Defferred Submittal: Exterior curtain wall 12. Deferred Submittal: Structural steel connection design calculations submitted

concurrently with structural steel shop drawings. 13. Miscellaneous anchors shown on the structural drawings. 14. Deferred Submittal: Wood truss design calculations and detailed erection and fabrication drawings. Standard stick framing shop drawings need not be submitted. 15. Standard details and bridging information for light gage metal framing. Erection plans and details for light gage metal joists and lintels spanning more than 6'-0" shall be submitted. Standard wall framing

16. Deferred Submittal: Augured pile foundation plans and details. 17. Deferred Submittal: Precast concrete shop drawings including erection drawings and connection details. 18. Deferred Submittal: Precast concrete connection design calculations. 19. Deferred Submittal: Cold-Formed metal framing for exterior walls.

14. Statement of Structural Special Inspections

A. The structural design for this project is based on completion of special inspections during construction in accordance with section 1704 of the International Building Code. The owner shall employ one or more qualified special inspectors to provide

the required special inspections. B. The special inspector shall furnish inspection reports to the building official, owner, architect and structural engineer, and any other designated person.

C. All discrepancies shall be brought to the immediate attention of the contractor for correction, then, if uncorrected, to the proper design authority, building official and D. The special inspector shall submit a final signed report stating that the work requiring

special inspection was, to the best of the inspector's knowledge, in conformance with the approved plans and specifications and the applicable workmanship provisions of the building code. E. The following inspections and tests are required with the frequency (continuous or periodic) as defined within the referenced section or standard listed below. The General Contractor shall provide notification to the inspector when items requiring inspection are ready to be inspected and provide access for those inspections.

1. Shop Fabrication – structural steel per Section 1704.2.5 unless AISC certified 2. Shop Fabrication – pre-engineered wood trusses per Section 1704.2.5 unless TPI certified shop

3. Shop Fabrication – precast concrete per Section 1704.2.5 unless PC certified 4. Steel Construction per Section 1705.2 and the quality assurance requirements

of AISC 341 Chapter J (as referenced by AISC 360) 5. Concrete Construction per Section 1705.3 and Table 1705.3 Reinforcing Steel Placement Reinforcing Steel Welding . Cast in Place Anchors

d. Post Installed Anchors

e. Design Mix Verification Concrete Sampling and Testing Concrete Placement

Concrete Curing Prestressed Concrete Stressing and Grouting Erection of Precast

Verification of In-situ Concrete Strength Prior to Stressing Post-Tensioned Formwork Shape, Location and Dimensions

6. Masonry Construction per Section 1705.4 and the quality assurance requirements of TMS 402/ACI530/ASCE5 and TMS602/A530.1/ASCE6 Level B Verification of Soils per Table 1705.6 Inspections and Tests of Cast-In-Place Deep Foundation per Table 1705.8 Wood Lateral System (periodic)

a. Wood shearwalls (include sheathing, rim board and bottom plate

attachments) b. Portal frames Shear wall and portal frame holdowns

e. Floor and roof trusses (random sampling)

d. Shear wall tension rod system 10. Wood Gravity Framing and Placement (adjust frequency of random sampling where indicated as required)

a. Heavy timber/SCL/glulam beams and supports (periodic) b. Headers and jambs (random sampling) Bearing walls (random sampling) Connector/hardware installation (random sampling)

15. Copyright and Disclaimer

A. All drawings in the structural set (S-series drawings) are the copyrighted work of Bob D. Campbell and company, Inc. These drawings may not be photographed, traced, or copies in any manner without the written permission of Bob D. Campbell and Company, Inc. Exception: Original drawings may be printed for distribution to the owner, architect, and general contractor for coordination, bidding, and construction. Subcontractors may not reproduce these drawings for any purpose

B. I, Christopher A. Beverlin, P.E., registered engineer and a representative of Bob D. Campbell and Company, Inc., do hereby accept professional responsibility as required by the professional registration laws of this state for the structural design drawings consisting of S-series drawings. I hereby disclaim responsibility for all other drawings in the construction document package, they being the responsibility of other design professionals whose seals and signed s elsewhere in the construction document package.

STRUCTURAL ABBREVIATIONS

D. S.	ADDITIONAL ABOVE FINISHED FLOOR ALTERNATE ARCHITECTURAL BUILDING BOTTOM OF BEAM BOTTOM BEARING CAMBER CONCRETE DECK TYPE CONSTRUCTION/CONTROL JOINT COMPLETE JOINT PENETRATION CENTERLINE CONCRETE MASONRY UNIT COLUMN CONCRETE CONNECTION CONTINUOUS COORDINATE COVER DOUBLE DETAIL DIAMETER DIMENSION DEAD LOAD DRAWING EACH EACH FACE EXPANSION JOINT ELEVATION EMBEDMENT, EMBEDDED ENGINEER EDGE OF DECK ENGINEER OF RECORD EDGE OF SLAB EQUAL EQUIPMENT EACH WAY EXPANSION EXTERIOR EXISTING	FS FTG FV GA GALV GEN GR GRBM HORIZ HSS IF INFO INT JT K KSF LBS, # LL LLH LLV LONG LSLT LTWT M MAX MECH MFGR MIN MISC MSRY MTL NF NS NTS NW OC OPP OVS P	FLOOR FAR SIDE FOOTING FIELD VERIFY GAGE GALVANIZE(D) GENERAL GRADE GRADE BEAM HORIZONTAL HOLLOW STRUCTURAL SECTION INSIDE FACE INFORMATION INTERIOR JOIST JOINT KIPS (1000 LBS) KIPS PER SQUARE FOOT KIPS PER SQUARE INCH POUNDS DEVELOPMENT LENGTH LIVE LOAD LONG LEG HORIZONTAL LONG LEG VERTICAL LONG-SLOTTED HOLE TRANSVERSE LIGHTWEIGHT MOMENT FORCE MAXIMUM MECHANICAL MANUFACTURER MINIMUM MISCELLANEOUS MASONRY METAL NEAR FACE NEAR SIDE NOT TO SCALE NORMAL WEIGHT ON CENTER OUTSIDE FACE OPENING OPPOSITE OVERSIZED HOLE AXIAL FORCE POWDER ACTUATED FASTENER
XT	EXTERIOR	OVS	OVERSIZED HOLE
D-#	FLOOR DECK TYPE	PAF	POWDER ACTUATED FASTENER PRECAST / PILE CAP
DN	FOUNDATION	PC	
F	FAR FACE	PCF	POUNDS PER CUBIC FOOT PRE-ENGINEERED METAL BUILDING
IN	FINISH	PEMB	

STRUCTURAL DECK & SLAB SCHEDULE

(SLOPE PER ARCH.) RE: NOTE 5

1" TO 1 1/4" GYPCRETE ATOP 23/32" APA RATED T&G STURD-I-FLOOR.

SHANK NAILS OR #10 SCREWS AT 6"o.c. AT EDGES & 12"o.c. AT FIELD.

REINFORCE WITH CELLULOSE FIBER AT 1.5lb/cu, vd. ATOP PRECAST

ARCH ATOP PODIUM SLAB. SLOPE TO DRAIN PER ARCH. RE: NOTE 5

3/4" CLEAN GRANULAR LEVELING COURSE ATOP SUITABLE SUBGRADE MATERIAL

SPECIFICATIONS T/SLAB EL. = PER PLAN, SLOPE TO DRAIN

SPECIFICATIONS T/SLAB EL. = PER PLAN, SLOPE TO DRAIN

3" NORMAL WEIGHT CONC. SLAB (4500psi. AIR-ENTRAINED)

3" NORMAL WEIGHT CONC. SLAB (4500psi, AIR-ENTRAINED)

3" NORMAL WEIGHT CONC. SLAB (4500psi. AIR-ENTRAINED)

EXP 1 SHEATHING. SHEATHING SHALL BE GLUED AND NAILED W/ 8d RING

REINFORCE WITH CELLULOSE FIBER AT 1.5 LBS/CU. YD. ATOP WATERPROOFING

REINFORCE WITH CELLULOSE FIBER AT 1.5lb/cu. vd. ATOP WATERPROOFING PER

REINFORCE WITH 6x6-W2.9xW2.9 WWF ATOP 15 MIL VAPOR BARRIER ATOP 4" OF

PER GEOTECH SPECIFICATIONS. T/SLAB EL. = PER PLAN, SLOPE TO DRAIN

REINFORCE WITH 6x6-W2.9xW2.9 WWF ATOP 4" OF 3/4" CLEAN GRANULAR

LEVELING COURSE, ATOP SUITABLE SUBGRADE MATERIAL PER GEOTECH

REINFORCE WITH #4 @ 12"oc EACH WAY BOTTOM ATOP 4" OF 3/4" CLEAN

GEOTECH SPECIFICATIONS. T/SLAB EL. = PER PLAN, SLOPE TO DRAIN

GRANULAR LEVELING COURSE, ATOP SUITABLE SUBGRADE MATERIAL PER

REINFORCE WITH #4 @ 12"o.c. EACH WAY ATOP 4" OF 3/4" CLEAN GRANULAR

LEVELING COURSE, ATOP SUITABLE SUBGRADE MATERIAL PER GEOTECH

19/32" APA RATED, EXP 1 SHEATHING ATTACHED WITH #10 SCREWS AT

6"o.c. AT EDGES & 12"o.c. AT FIELD. (PROVIDE FRT TREATED PLYWOOD AT

FIREWALLS - REFER TO ARCH DRAWINGS FOR LOCATION AND EXTENTS)

MEMBRANE (RE: ARCH.) ATOP 15/32" EXTERIOR GRADE PLYWOOD SHEATHING

MARK

DESCRIPTION

STAIR LANDING

1. FD = FLOOR DECK TYPE.

4. RD = ROOF DECK TYP.

2. CD = CONCRETE DECK TYP.

3. SOG = SLAB-ON-GRADE TYP.

4" CONC. SLAB (4000psi)

4" CONC. SLAB (4500psi, AIR-ENTRAINED)

8" CONC. SLAB (4500psi, AIR-ENTRAINED)

6" CONC. SLAB (4500psi, AIR-ENTRAINED)

5. PROVIDE 1" DEEP TOOLED CONTROL JOINT (TRANSVERSE DIRECTION) @

MID-SPAN OF BALCONY (8'-0" MAX SPACING) FILL JOINT w/ SEALANT.

-O-F	DOLLARDO DER COLLARE FOOT	
PSF	POUNDS PER SQUARE FOOT	
PSI	POUNDS PER SQUARE INCH	7
QTY	QUANTITY	``
RAD	RADIUS	(
RD-#	ROOF DECK TYPE	/
REF	REFERENCE	(
REINF	REINFORCEMENT	/
REQD	REQUIRED	(
		\
REV	REVISION	(
RLL	ROOF LIVE LOAD	1
RTU	ROOF TOP UNIT	(
SC	SLIP CRITICAL	7
SCHED	SCHEDULE(D)	
SECT	SECTION	1
SHT	SHEET	(
		(
SIM	SIMILAR	1
SJ	SAW JOINT	(
SL	SNOW LOAD	\
SOG	SLAB-ON-GRADE	(
SOG-#	SLAB-ON-GRADE TYPE	1
SPCG	SPACING	
SPEC	SPECIFICATION	1
SPRT	SUPPORT	Ì
		(
SQ	SQUARE	/
SS	STAINLESS STEEL	(
SSLT	SHORT-SLOTTED HOLE TRANS	√Ę/
STD	STANDARD	(
STIFF	STIFFENER	
STIR	STIRRUP	'
STL	STEEL	1
STRUCT	STRUCTURE, STRUCTURAL	
Γ/	TOP OF	ì
THRU	THROUGH	(
ΓOS	TOP OF STEEL, TOP OF SLAB	(
ΓRANS	TRANSVERSE	(
ΓΥΡ	TYPICAL	1
JNO	UNLESS NOTED OTHERWISE	(
/	SHEAR FORCE	
/ERT	VERTICAL	
N/	WITH	1
N/0	WITHOUT	Ì
ΝF	WIDE FLANGE	
ΝL	WIND LOAD	(
NΡ	WORK POINT	
ΛWF	WELDED WIRE FABRIC	,
		(
		(

S1.56E BUILDING E ROOF FRAMING PLAN 31.57E BUILDING E SHEARWALL PLAN 81.61F BUILDING F FOUNDATION PLAN 1.62Fa BUILDING F PODIUM SLAB REINFORCEMENT PLAN S1.62Fb BUILDING F PODIUM STUD RAIL & SLAB GEOMETRY PLAN PERPENDICULAR S1.63F BUILDING F THIRD FLOOR FRAMING PLAN POUNDS PER LINEAR FOOT PARTIAL JOINT PENETRATION > S1.64F BUILDING F FOURTH FLOOR FRAMING PLAN 81.65F BUILDING F FIFTH FLOOR FRAMING PLAN S1.66F BUILDING F ROOF FRAMING PLAN 1.67F BUILDING F SHEARWALL PLAN S1.71G GARAGE FOUNDATION PLAN 06.08.22 S1.72G GARAGE SECOND FLOOR FRAMING PLAN 06.08.22 S1.73G GARAGE THIRD FLOOR FRAMING PLAN 06.08.22 S1.74G GARAGE FOURTH FLOOR FRAMING PLAN 06.08.22 S1.75G GARAGE FIFTH FLOOR FRAMING PLAN 06.08.22 S1.76G GARAGE SNOW LOADING PLAN STAIR 1 & 2 FRAMING PLANS STAIR 3 FRAMING PLANS S2.02 STAIR 4 FRAMING PLANS .03 STAIR 5 FRAMING PLANS S2.04 STAIR 6 FRAMING PLANS STAIR FRAMING DETAILS **ELEVATOR FRAMING DETAILS** BALCONY FRAMING PLANS 2.30 BALCONY FRAMING DETAILS TYPICAL FOUNDATION DETAILS S3.01 GARAGE FOUNDATION DETAILS 06.08.22 S3.02 PILE & PODIUM FOUNDATION DETAILS 03 APARTMENT FOUNDATION DETAILS 3.04 APARTMENT FOUNDATION DETAILS S3.05 | FOUNDATION DETAILS 3.11 | CONCRETE FRAMING DETAILS S3.30 WOOD FLOOR FRAMING DETAILS 3.31 WOOD FLOOR FRAMING DETAILS S3.41 WOOD ROOF FRAMING DETAILS WOOD FIREWALL DETAILS PRECAST GARAGE FRAMING DETAILS PRECAST GARAGE FRAMING DETAILS

STRUCTURAL SHEET LIST

Sheet Name

WOOD SCHEDULES & TYPICAL DETAILS

WOOD SCHEDULES & TYPICAL DETAILS

WOOD SHRINKAGE & MOVEMENT

SECOND FLOOR FRAMING PLAN

1.12A BUILDING A SECOND FLOOR FRAMING PLAN

1.14A BUILDING A FOURTH FLOOR FRAMING PLAN

B1.22B BUILDING B SECOND FLOOR FRAMING PLAN

1.23B BUILDING B THIRD FLOOR FRAMING PLAN

81.24B BUILDING B FOURTH FLOOR FRAMING PLAN

1.32C BUILDING C SECOND FLOOR FRAMING PLAN

1.34C BUILDING C FOURTH FLOOR FRAMING PLAN

1.35C BUILDING CROOF FIND TLANCLUDED WITH

1.41D BUILDING D FOUNDATION ARAGE PERMIT

1.42Db BUILDING D PODIUM STUD RAIL REAL PROPERTY CAS

1.33C BUILDING C THIRD FLOOR FRAMING PLAN

1.43D BUILDING D THIRD FLOOR FRAMING PL

1.46D BUILDING D ROOF FRAMING PLAN

81.47D BUILDING D SHEARWALL PLAN

1.51E BUILDING E FOUNDATION PLAN

1.44D BUILDING D FOURTH FLOOR FRAMING PLAN

31.52Ea BUILDING E PODIUM SLAB REINFORCEMENT PLAN

1.52Eb BUILDING E PODIUM STUD RAIL & SLAB GEOMETRY PLAN

1.45D BUILDING D FIFTH FLOOR FRAMING PLAN

31.53E BUILDING E THIRD FLOOR FRAMING PLAN

S1.55E BUILDING E FIFTH FLOOR FRAMING PLAN

1.54E BUILDING E FOURTH FLOOR FRAMING PLAN

1.13A BUILDING A THIRD FLOOR FRAMING PLAN

THIRD FLOOR FRAMING PLAN

CONCRETE SCHEDULE

S1.04 FOURTH FLOOR FRAMING PLAN

S1.11A BUILDING A FOUNDATION PLAN

S1.15A BUILDING A ROOF FRAMING PLAN

1.25B BUILDING B ROOF FRAMING PLAN

S1.26B BUILDING B SHEARWALL PLAN

1.31C BUILDING C FOUNDATION PLAN

S1.16A BUILDING A SHEARWALL PLAN

S1.21B BUILDING B FOUNDATION PLAN

S1.05 FIFTH FLOOR FRAMING PLAN

FOUNDATION PLAN

Sheet

Number

GENERAL NOTES

S0.04 STEEL SCHEDULES

S0.05 CMU DETAILS

Current

Revision

Revision

Date

06.08.22

LEGEND:

SPAN DIRECTION OF DECK - DECK TYPE PER SCHEULE ON \$0.01 √ HSS 6x6x1/4 COLUMN SIZE ---- BASE PLATE MARK - SEE SCHEDULE ON SHEET S0.04

BEAM OR HEADER PER SCHEDULE ON S0.02 (A#-#u) UPSET BEAM OR HEADER PER SCHEDULE ON S0.02 BEARING WALL TYPE PER SCHEDULE ON S0.02

SHEARWALL HOLDDOWN TYPE PER SCHEDULE ON S0.03 NUMBER OF WALL STUDS IN STUD PACK NUMBER OF JACK STUDS/KING STUDS IN STUD PACK

PLAN NOTE PER SCHEDULE ON S0.02

AMOUNT OF UPWARD POSITIVE CAMBER

SHEARWALL TYPE PER SCHEDULE ON S0.03 CONCRETE SHEARWALL TYPE PER SCHED ON S0.10

PILE CAP SIZE PER SCHEDULE ON S3.02

PARAGON STAR NORTH VILLAGE

3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081 Project No.: 18017,19050.07,19050.08

02.23.2022 Issued For: BLDG 1 & 2 75% SET **REVISIONS**

REGISTRATION



PROJECT TEAM ARCHITECT FINKLE+WILLIAMS ARCHITECTURE CIVIL **GBA ENGINEERS** LANDSCAPE LAND 3 STRUCTURAL BOB D. CAMPBELL LATIMER SOMMERS LATIMER SOMMERS **MECHANICAL** ELECTRICAL LATIMER SOMMERS FIRE PROTECTION LATIMER SOMMERS

CONSTRUCTORS

CONTRACTOR

GENERAL NOTES

SHEET TITLE

TYPICAL CMU WALL REINFORCING AT OPENINGS

LEGEND:

- TULL HEIGHT VERTICAL BARS AS JAMB REINFORCING IN FIRST 2 CELLS ADJACENT TO OPENING. REINFORCE EACH CELL WITH SIZE & QUANTITY OF BAR TO MATCH WALL REINFORCING (1 BAR TYPICAL IN 8" WALLS AND 2 BARS TYPICAL IN 12" WALLS).
- 2 LINTEL REINFORCING PER SECTION C. EXTEND 2'-0" PAST EDGE OF OPENING ON EACH SIDE (TYPICAL).
- 2-#5 CONTINUOUS HORIZONTAL BARS AS SILL REINFORCING IN 8" COURSE BELOW OPENING (U.N.O.). EXTEND 2'-0" PAST EDGE OF OPENING ON EACH SIDE (TYPICAL).
- FULL HEIGHT VERTICAL BARS PER MASONRY VERTICAL REINFORCING SCHEDULE LOCATED IN END CELL AT EACH SIDE OF VERTICAL WALL CONTROL JOINTS.

GENERAL CRITERIA: (SECTION A CONTINUED):

- VERTICAL REINFORCING BARS SHALL BE DOWELED TO FOUNDATION WITH A DOWEL OF MATCHING SIZE

 AND ORACING.
- CONTRACTOR SHALL COORDINATE AND VERIFY OPENINGS IN MASONRY WALLS. OPENINGS SHALL BE DETAILED ON REINFORCING STEEL SHOP DRAWING ELEVATIONS.
 VERTICAL CONTROL JOINTS IN MASONRY WALLS SHALL BE 3/8" WIDE, FULL HEIGHT OF WALL. JOINTS SHALL
- 3. VERTICAL CONTROL JOINTS IN MASONRY WALLS SHALL BE 3/8" WIDE, FULL HEIGHT OF WALL. JOINTS SHALL BE SPACED AT A MAXIMUM OF 24'-0" ON CENTER AND NOT LESS THAN 2'-0" FROM THE EDGE OF ANY OPENING. ALL HORIZONTAL JOINT REINFORCING SHALL BE DISCONTINUOUS AT CONTROL JOINTS. ALL BOND BEAM HORIZONTAL REINFORCING SHALL BE CONTINUOUS THROUGH CONTROL JOINTS. CONTRACTOR SHALL COORDINATE AND VERIFY ALL CONTROL JOINT LOCATIONS.

MASONRY VERTICAL REINFORCING SCHEDULE FOR LOAD BEARING MASONRY (CMU) WALLS										
WALL THICKNESS	LOCATION	VERTICAL REINF. (IN GROUTED CELLS)	SPACING							
8"	GARAGE ATOP PRECAST	1- #5	48"oc							
8"	BTWN GARAGE & RETAIL	1- #5	32"oc							
8"	ELEVATOR	1- #5	32"oc							

- NOTES:

 1. IN ADDITION TO SPACING SHOWN IN SCHEDULE, VERTICAL REINFORCING SHALL BE PROVIDED IN GROUTED CELLS AT THE FOLLOWING LOCATIONS
- A.) IN THE FIRST 2 CELLS ADJACENT TO EACH OPENING
 B.) IN THE END CELLS ON EACH SIDE OF VERTICAL CONTROL JOINTS
 C.) IN THE END CELLS OF EACH LENGTH OF WALL
 D.) AT EACH CORNER OF WALLS
- 2. ALL MASONRY VOIDS AND BOND BEAMS TO BE GROUTED SHALL BE FREE OF DEBRIS AND MORTAR DROPPINGS PRIOR TO GROUTING. ANY MASONRY W/

- 8" CMU REINF

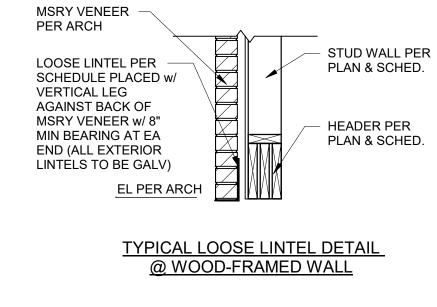
- CMU LINTEL

PER C/S0.05

PER GEN NOTES

DROPPINGS OR DEBRIS OBSERVED IN VOIDS SHALL BE REJECTED.

A CMU WALL ELEVATION



3 **SECTION**

TYPICAL LOOSE LINTEL DETAIL

@ CMU-FRAMED WALL

MSRY VENEER

LOOSE LINTEL PER

SCHEDULE PLACED w/

VERTICAL LEG AGAINST

w/ 8" MIN BEARING AT

EACH END (ALL

GALV)

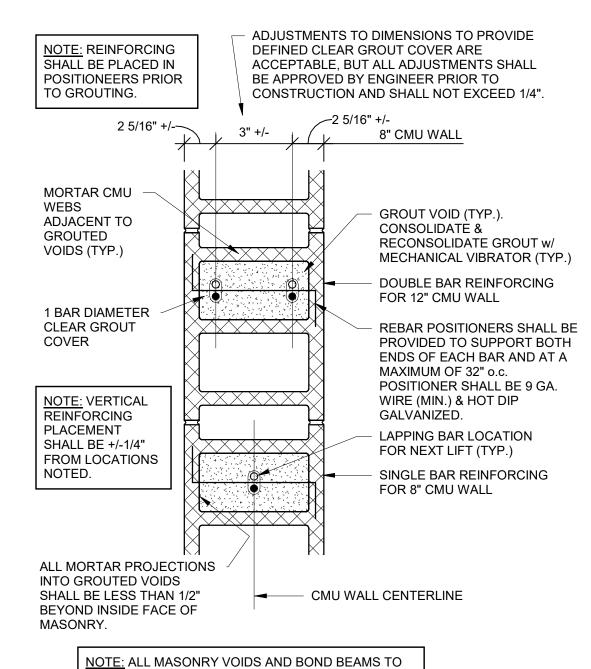
BACK OF MSRY VENEET

EXTERIOR LINTELS TO BE

EL. PER ARCH.

PER ARCH

3A <u>SECTION</u>
3/4" = 1'-0"



NOTE: ALL MASONRY VOIDS AND BOND BEAMS TO BE GROUTED SHALL BE FREE OF DEBRIS AND MORTAR DROPPINGS PRIOR TO GROUTING. ANY MASONRY W/ DROPPINGS OR DEBRIS OBSERVED IN VOIDS SHALL BE REJECTED.

TYPICAL REBAR POSITIONING DETAIL

CMU REINF PER

GENERAL NOTES

DOWELS TO MATCH SIZE

& SPACING OF VERTICAL

T + 8"

TYPICAL THICKENED SLAB (UNDER NON-LOAD-BEARING MASONRY)

1 **SECTION**

REINFORCING. DRILL &

EPOXY 4" INTO SLAB.

B SECTION 1 1/2" = 1'-0"

REFER TO TYPICAL

DETAILS FOR LATERAL

BEARING MASONRY

O ARCHITECTURAL

─ (3) #4 CONT.

(BOTTOM)

BRACING AT TOP OF WALL

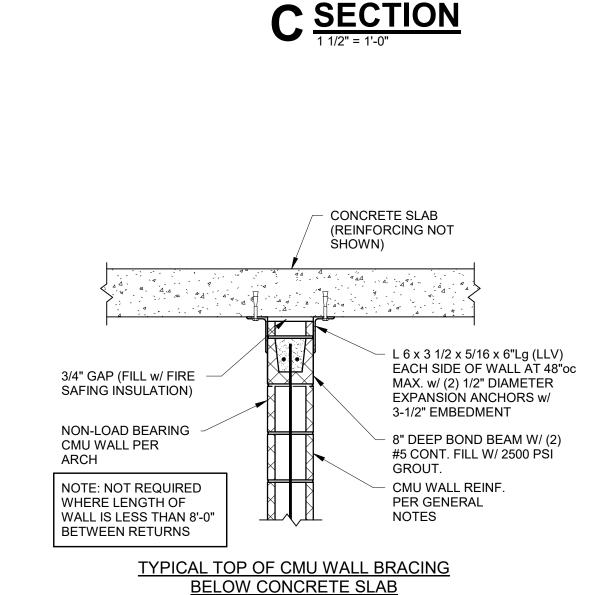
NOTE: PROVIDE THICKENED

SLAB UNDER ALL NON-LOAD-

PARTITION WALLS. REFER

DRAWINGS FOR LOCATIONS.

SLAB PER PLAN



TYPICAL MASONRY REINFORCING NOTE:

ALL INTERIOR & EXTERIOR MASONRY WALLS SHOWN ON ARCHITECTURAL

BOND BEAMS (2 - #5 BOTTOM) AT BOTTOM COURSE, TOP COURSE, JOIST

BEARING ELEVATION AND AT 8'-0" MAXIMUM O.C. AND VERTICALLY AS

RE: DETAILS "A" THROUGH "E" ON THIS SHEET.

REBAR POSITIONER

OR SUPPORT @ 48"

(2) #4 CONT. TOP

48"oc

(2) #4 CONT.

WALL

THICKNESS

PER PLAN ²

OPENINGS UP TO 4'-0"

REINF. CHAIR

SUPPORT @

o.c. MAX.

AND STRUCTURAL DRAWINGS ARE TO BE REINFORCED HORIZONTALLY WITH

INDICATED ON DRAWINGS. THESE WALLS ARE TO BE ANCHORED TOP AND

BOTTOM TO THE FOUNDATION, FLOOR, OR ROOF PER TYPICAL DETAILS. THE

VERTICAL REINFORCING IS CONTINUOUS (IN 6'-6" MAXIMUM LENGTHS, LAPPED 2'-6" MINIMUM). FILL BLOCK CELLS AND BOND BEAMS WITH 2,500psi GROUT.

REBAR POSITIONER

2-#6 CONT.

(TOP)

OR SUPPORT @ 48"

o.c. MAX.

CONT. TOP

GROUT COURSES

SIMULTANEOUSLY

#3 @8"o.c. EACH

48"oc

(2) #5 CONT.

BOTTOM

WALL

THICKNESS

PER PLAN

OPENINGS 4'-0" TO 7'-4"

TYPICAL LINTELS AT ALL CMU WALLS (U.N.O.)

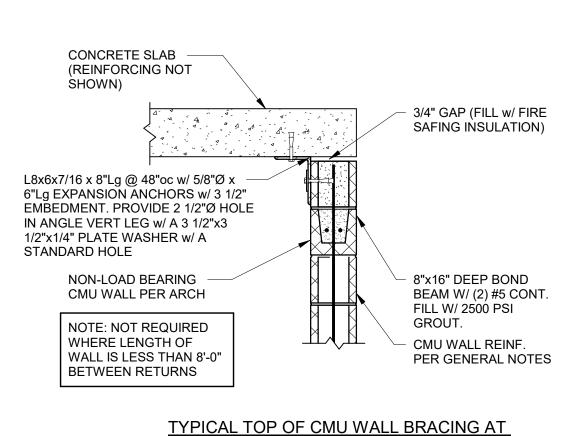
FACE w/ 90 DEGREE

HOOK @ EACH END

REINF. CHAIR

SUPPORT @

2 **SECTION**3/4" = 1'-0"



ALL VOIDS IN

BE GROUTED

COLUMN SHALL

(2) #6 CONT.

ВОТТОМ

WALL

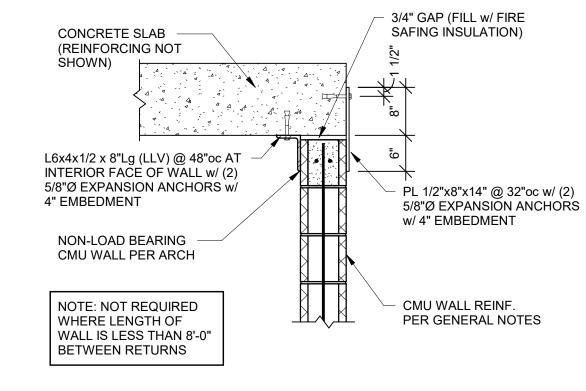
PER PLAN

OPENINGS 7'-4" TO 12'-0"

THICKNESS

2B <u>SECTION</u>

EDGE ON CONCRETE SLAB @ INTERIOR WALI



- #2 TIES @8"oc THROUGH

OPENING. TIES SHALL BE

REINFORCING WITHIN THE

BLOCK AS REQUIRED TO

SINGLE LAYER OF TIE

HORIZONTAL MORTAR

JOINT. CUT WEBS OF

RECEIVE TIES WHERE

CONFLICTS OCCUR.

(2) TYPICAL VERTICAL

BARS PER VOID (FULI

HEIGHT OF WALL)

COLUMN HEIGHT PLUS 2'-0" ABOVE AND BELOW

FABRICATED TO MAINTAIN A

"KNOCKOUT " (K.O.) or TROUGH BOND

BEAM BLOCK (TYPICAL UNIT EXCEPT @

DOOR OPENINGS; SOLID BOTTOM BOND

STOP (RE: SPECS.) UNDER K.O. BOND

BE REINFORCED AND GROUTED.

SLOTS

BEAM SHALL BE USED). PROVIDE GROUT

BEAMS OVER CELLS WHICH ARE NOT TO

TOP BOND BEAM

OR K.O. BLOCK

SHOWN FOR

CLARITY)

(REINFORCING NOT

- SAW CUT OR

SPECIAL BLOCK

TYPICAL BOND BEAM DETAIL AT CORNER OF CMU WALL

 $D \frac{DETAIL}{3/4" = 1'-0"}$

COLUMN DIMENSIONAL RANGE

16" MIN. TO 40" MAX.

<u>TYPICAL MASONRY COLUMN</u>

E SECTION1 1/2" = 1'-0"

PROVIDE CORNER

CONTINUOUS BOND

BEAM REINFORCING

BARS TO MATCH

2C <u>SECTION</u>

TYPICAL TOP OF CMU WALL BRACING AT EDGE

OF CONCRETE SLAB @ EXTERIOR WALL

LOOSE LINTEL SCHEDULE

FOR OPENINGS:	GALV. ANGLE
LESS THAN 6'-0"	L5"x3 1/2"x5/16" (LLV)
6'-0" < OPENING < 9'-0"	L6"x3 1/2"x5/16" (LLV)
9'-0" < OPENING < 9'-6"	L8"x4"x1/2" (LLV)

paragon star

PARAGON STAR NORTH VILLAGE

3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081

Projec	ct No.:	18017,19050.07,19050.08	
Date:		18017,19050.07,19050.08 02.23.2022 BLDG 1 & 2 75% SET REVISIONS Description	
Date: 02.23.2022 Issued For: BLDG 1 & 2 75% SET REVISIONS			
		REVISIONS	
No.	Date	Description	

REGISTRATION

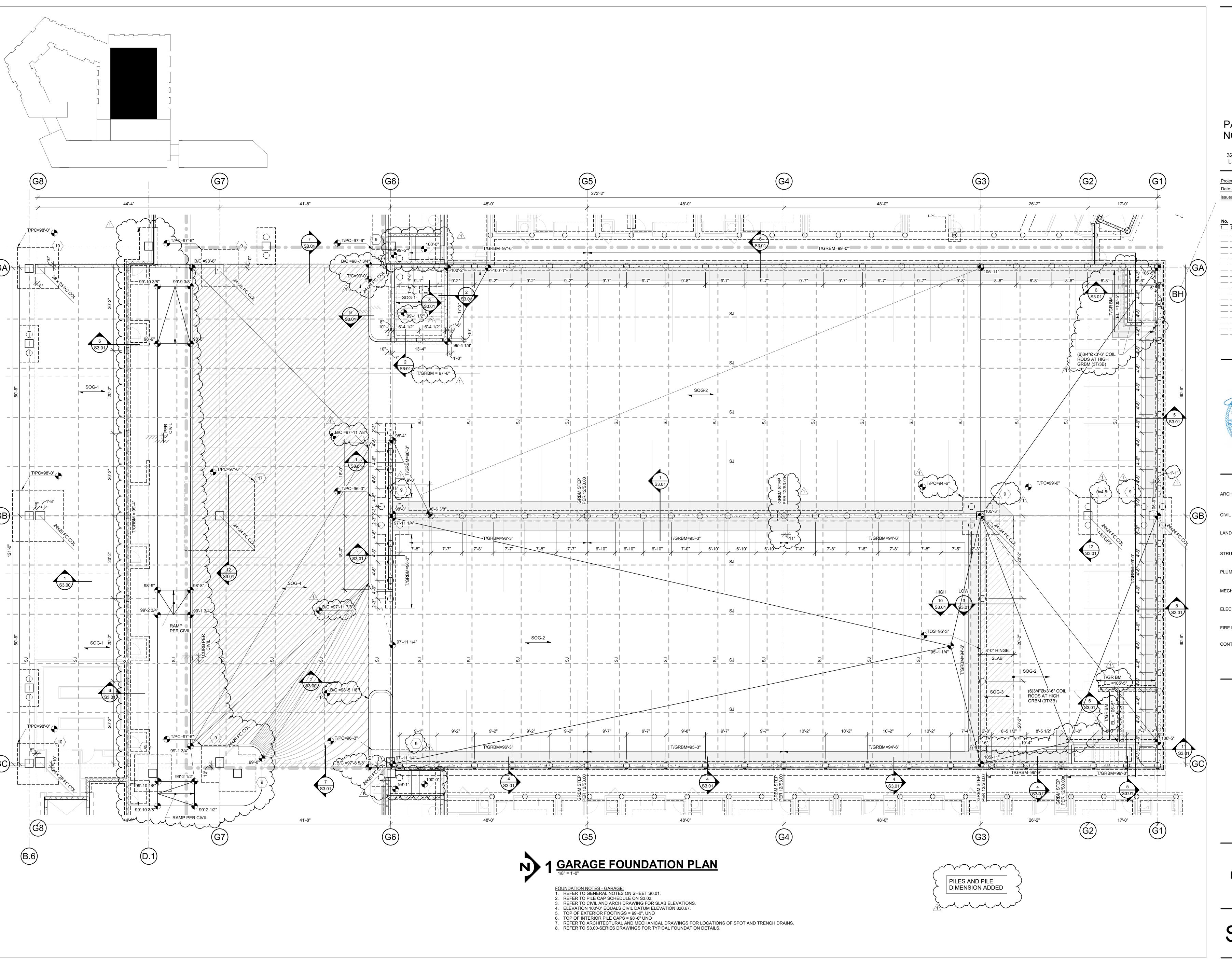


PROJECT TEAM ARCHITECT FINKLE+WILLIAMS ARCHITECTURE CIVIL **GBA ENGINEERS** LANDSCAPE LAND 3 STRUCTURAL BOB D. CAMPBELL PLUMBING LATIMER SOMMERS LATIMER SOMMERS **MECHANICAL** LATIMER SOMMERS ELECTRICAL FIRE PROTECTION LATIMER SOMMERS BRINKMANN CONTRACTOR CONSTRUCTORS

OB D. CAMPBELL & CO.
Tuctural Engineers Since 1957
38 Belleview Ave. 816.531.4144
nsas City, MO 64111 www.bdc-engrs.com

CMU DETAILS

SO.05





3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081

Project No.: 18017,19050.07,19050.00

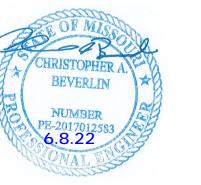
Date: 02.23.2022

Issued For: BLDG 1 & 2 75% SET

REVISIONS

Date Description

REGISTRATION



PROJECT TEAM

HITECT FINKLE+WILLIAMS
ARCHITECTURE

CIVIL GBA ENGINEERS

LANDSCAPE LAND 3

STRUCTURAL BOB D. CAMPBELL
PLUMBING LATIMER SOMMERS

PLUMBING LATIMER SOMMERS

MECHANICAL LATIMER SOMMERS

ELECTRICAL LATIMER SOMMERS

FIRE PROTECTION LATIMER SOMMERS

CONTRACTOR

BRINKMANN

CONSTRUCTORS

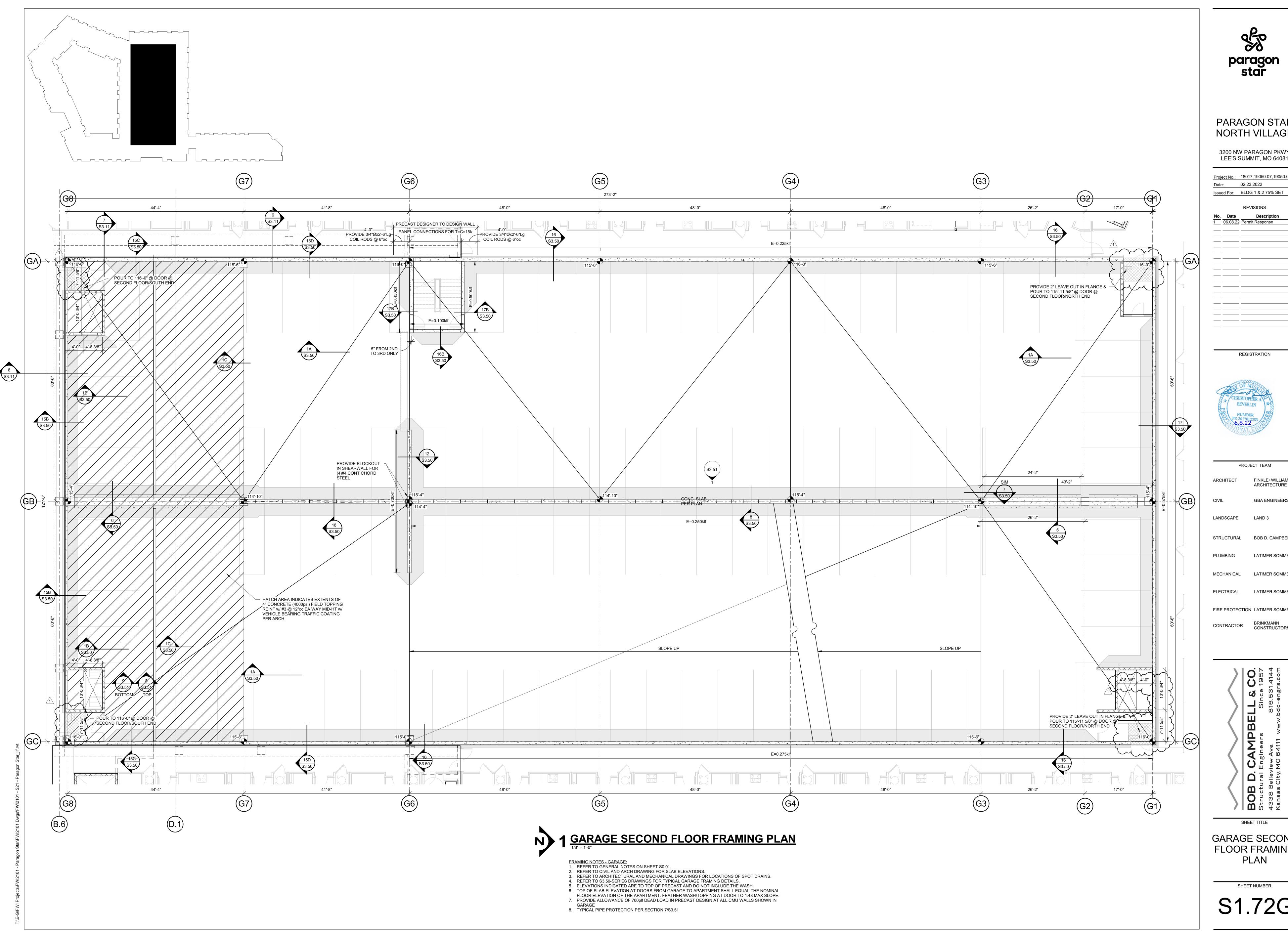
BOB D. CAMPBELL & CO.
Structural Engineers Since 1957
4338 Belleview Ave. 816.531.4144
Kansas City, MO 64111 www.bdc-engrs.com

SHEET TITLE

GARAGE

FOUNDATION PLAN

SHEET NUMBER
S1.71G





3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081

Project No.: 18017,19050.07,19050.08 Date: 02.23.2022

REVISIONS

REGISTRATION



PROJECT TEAM FINKLE+WILLIAMS ARCHITECTURE **GBA ENGINEERS** LANDSCAPE STRUCTURAL BOB D. CAMPBELL

LATIMER SOMMERS MECHANICAL LATIMER SOMMERS

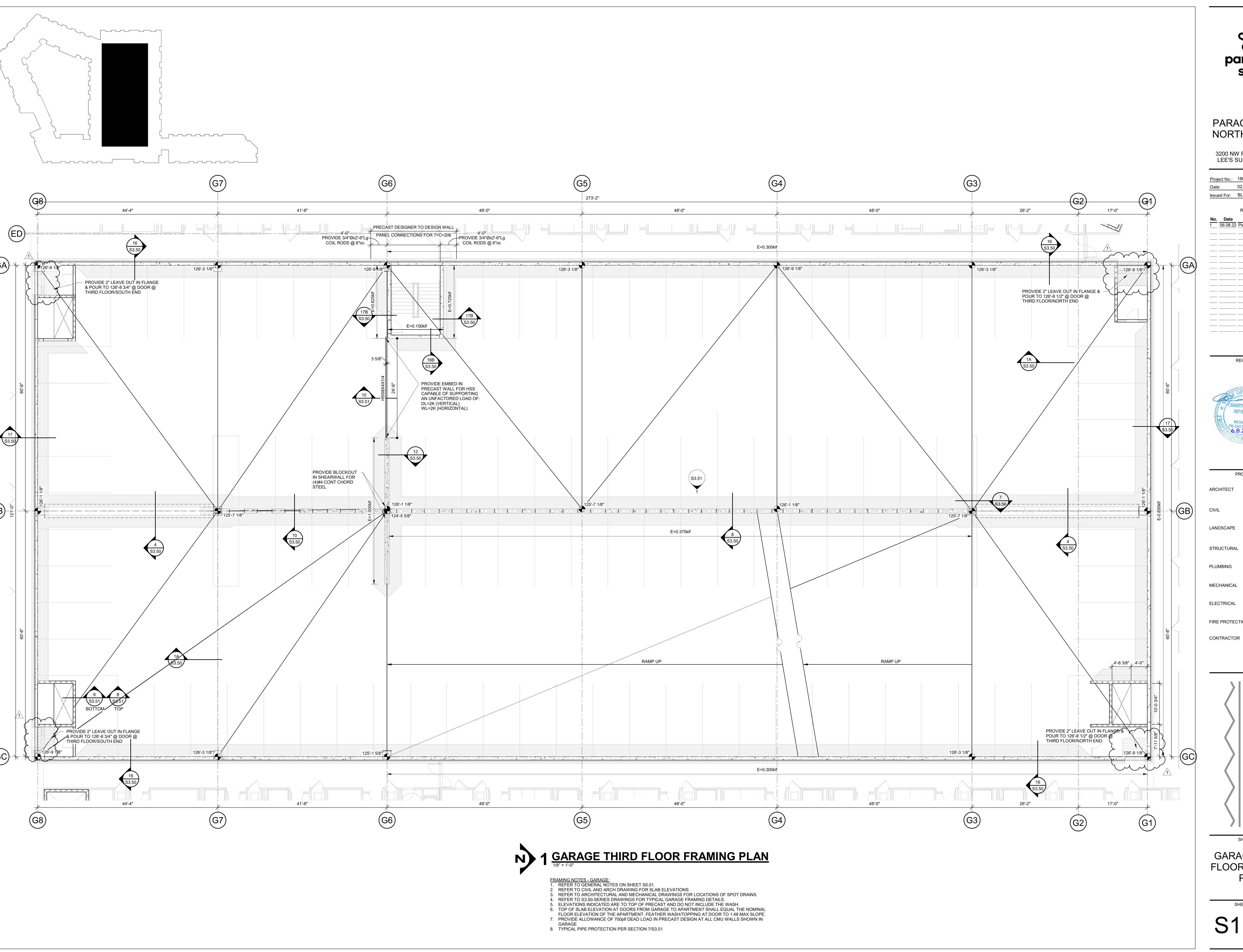
LATIMER SOMMERS

FIRE PROTECTION LATIMER SOMMERS

BOB Structu 4338 B Kansas SHEET TITLE

GARAGE SECOND FLOOR FRAMING PLAN

SHEET NUMBER S1.72G





3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081

Project No.: 18017,19050.07,19050.08 Date: 02.23.2022 Issued For: BLDG 1 & 2 75% SET

REVISIONS

REGISTRATION



PROJECT TEAM FINKLE+WILLIAMS **GBA ENGINEERS** LANDSCAPE LAND 3 STRUCTURAL BOB D. CAMPBELL LATIMER SOMMERS

LATIMER SOMMERS FIRE PROTECTION LATIMER SOMMERS

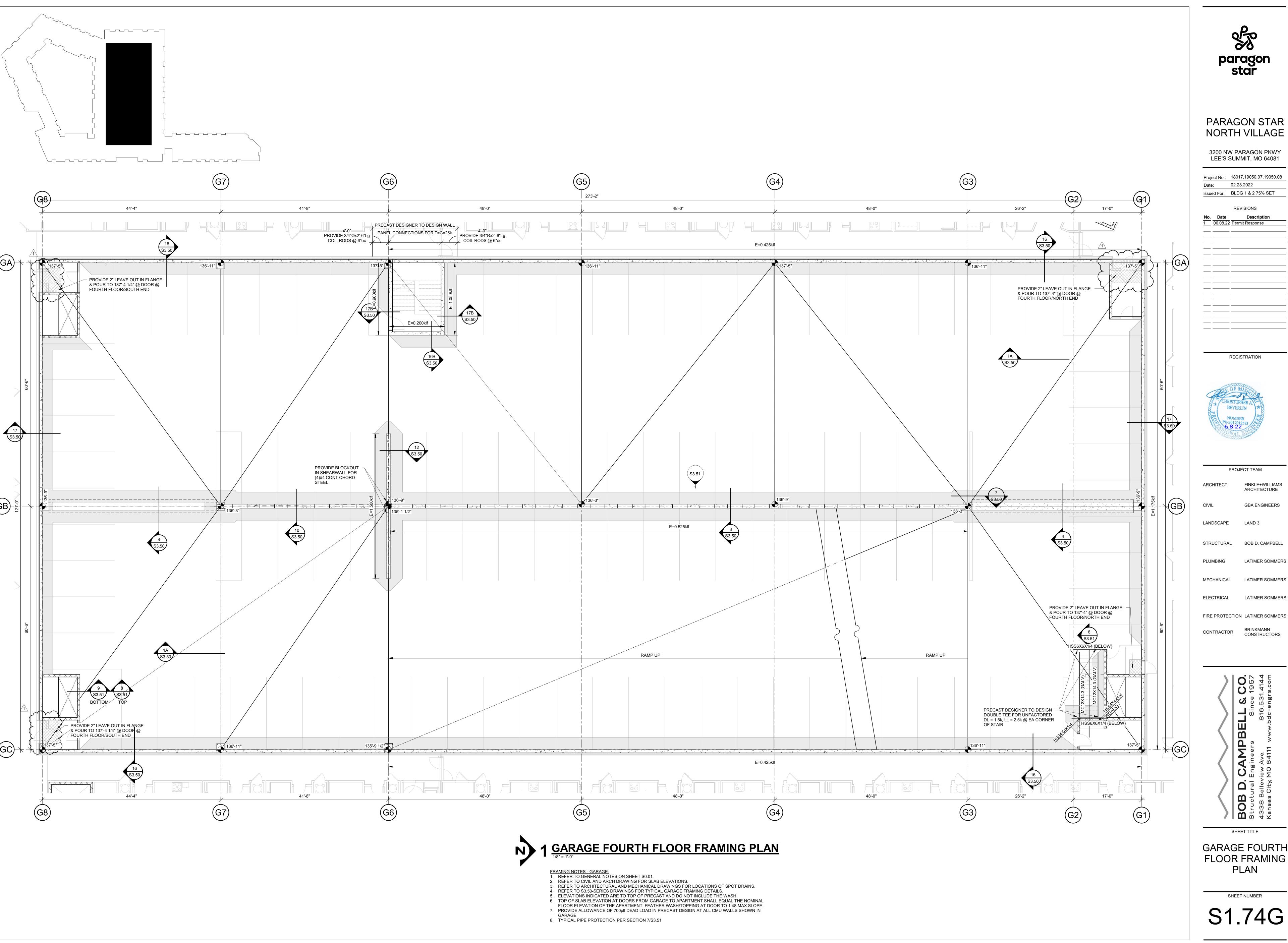
LATIMER SOMMERS

CONTRACTOR BRINKMANN CONSTRUCTORS

SHEET TITLE

GARAGE THIRD FLOOR FRAMING PLAN

SHEET NUMBER S1.73G





3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081

Project No.: 18017,19050.07,19050.08 Date: 02.23.2022 Issued For: BLDG 1 & 2 75% SET

REVISIONS

REGISTRATION



PROJECT TEAM FINKLE+WILLIAMS ARCHITECTURE **GBA ENGINEERS**

LANDSCAPE LAND 3 STRUCTURAL BOB D. CAMPBELL

LATIMER SOMMERS MECHANICAL LATIMER SOMMERS

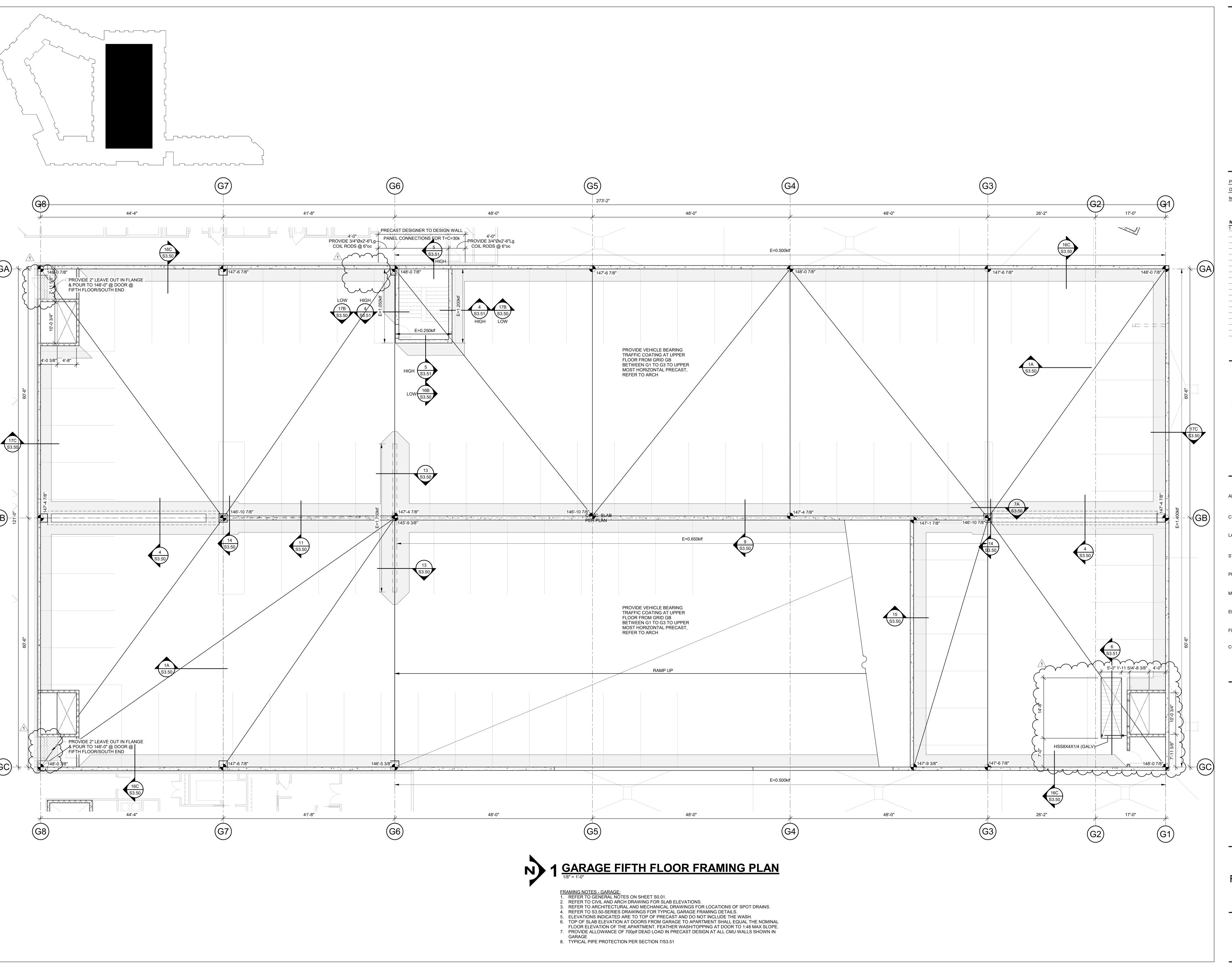
FIRE PROTECTION LATIMER SOMMERS CONTRACTOR BRINKMANN CONSTRUCTORS

GARAGE FOURTH FLOOR FRAMING PLAN

SHEET TITLE

SHEET NUMBER

S1.74G





3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081

Project No.: 18017,19050.07,19050.08

Date: 02.23.2022

Issued For: BLDG 1 & 2 75% SET

 No.
 Date
 Description

 1
 06.08.22
 Permit Response

REGISTRATION



PROJECT TEAM

ARCHITECT FINKLE+WILLIAMS ARCHITECTURE

CIVIL GBA ENGINEERS

LANDSCAPE LAND 3

STRUCTURAL BOB D. CAMPBELL

PLUMBING LATIMER SOMMERS

MECHANICAL LATIMER SOMMERS

ELECTRICAL LATIMER SOMMERS

FIRE PROTECTION LATIMER SOMMERS

CONTRACTOR

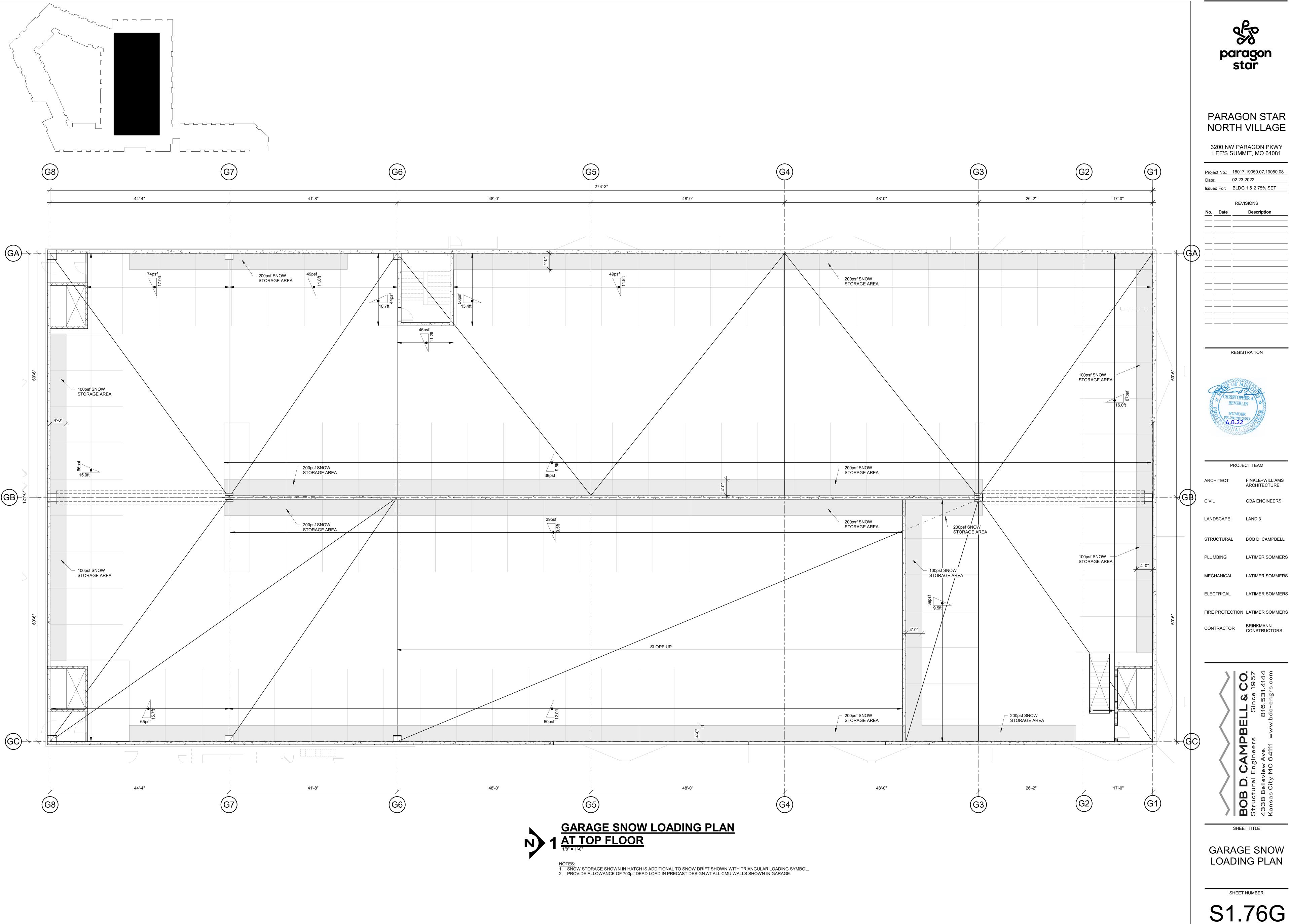
BRINKMANN

CONSTRUCTORS

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tructural Engineers Since 1957
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ansas City, MO 64111 www.bdc-engrs.com

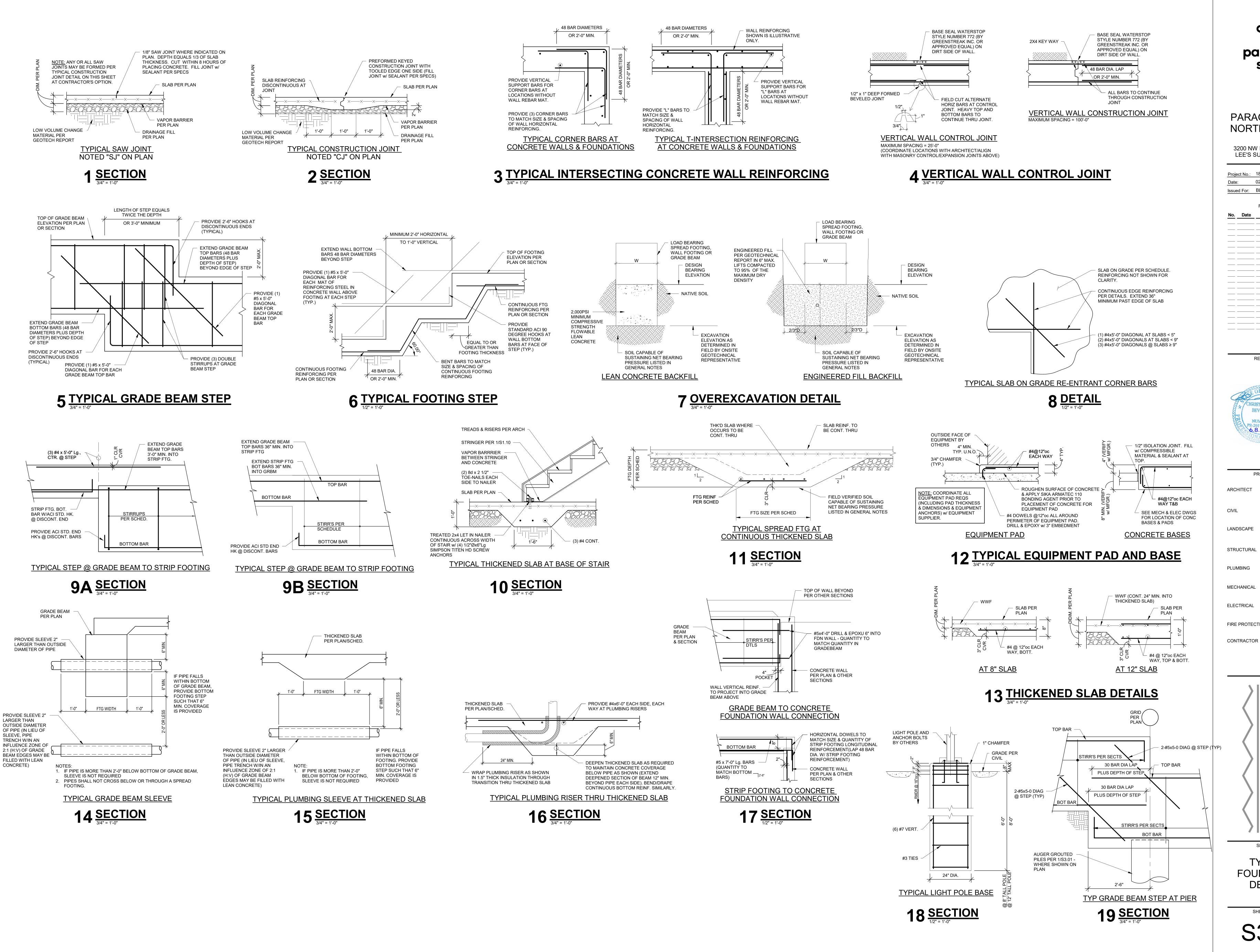
SHEET TITLE

GARAGE FIFTH FLOOR FRAMING PLAN



Project No.: 18017,19050.07,19050.08

LOADING PLAN



3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081

Project No.: 18017,19050.07,19050.08 02.23.2022

Issued For: BLDG 1 & 2 75% SET

REVISIONS

REGISTRATION



PROJECT TEAM FINKLE+WILLIAMS ARCHITECTURE GBA ENGINEERS

LAND 3 LANDSCAPE

BOB D. CAMPBELL

LATIMER SOMMERS LATIMER SOMMERS

LATIMER SOMMERS ELECTRICAL

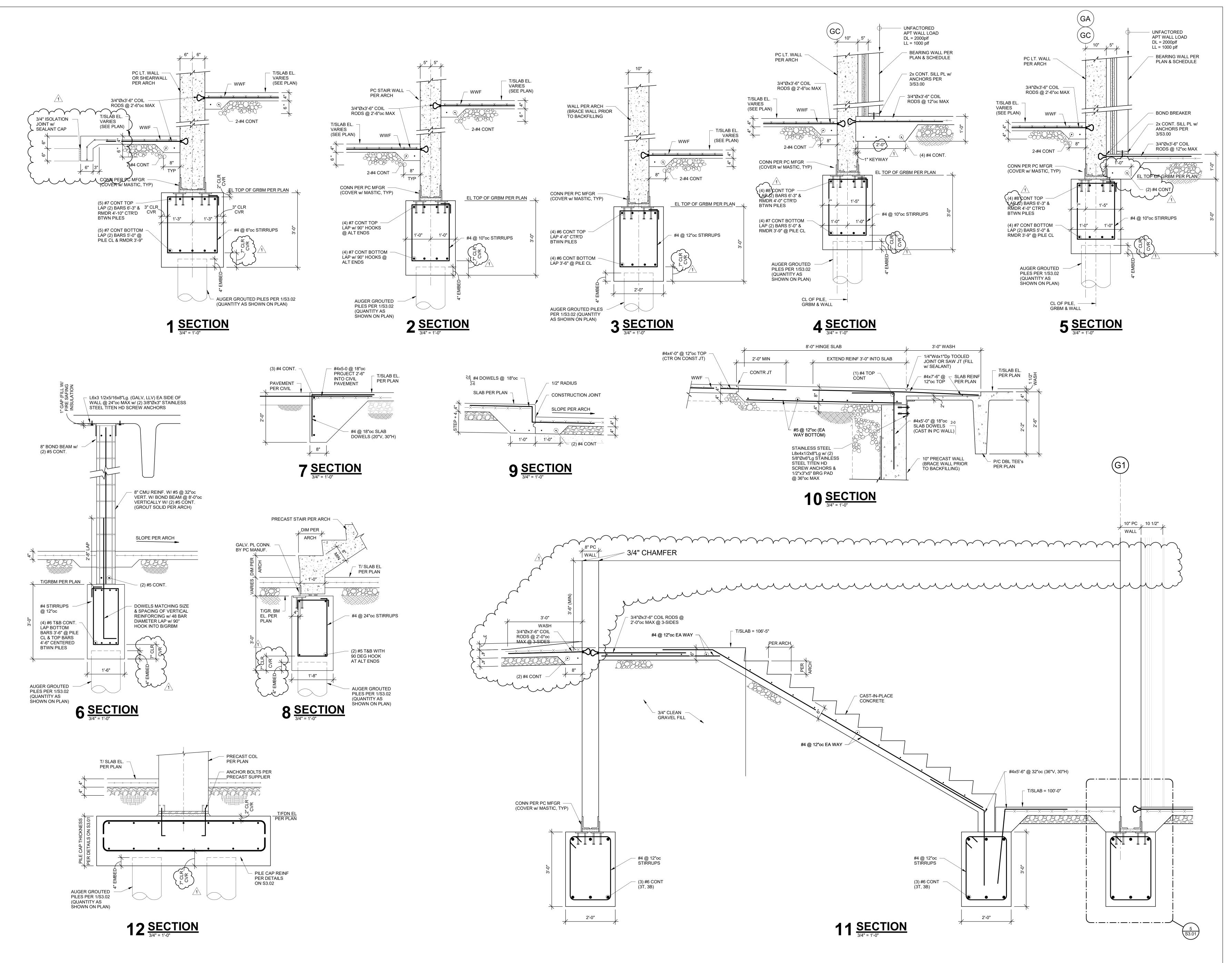
FIRE PROTECTION LATIMER SOMMERS BRINKMANN CONSTRUCTORS CONTRACTOR

Σ Seers BOB Structu 4338 B Kansas

SHEET TITLE **TYPICAL** FOUNDATION

DETAILS SHEET NUMBER

S3.00





3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081

Project No.: 18017,19050.07,19050.08

Date: 02.23.2022

Issued For: BLDG 1 & 2 75% SET

No. Date Description
1 06.08.22 Permit Response

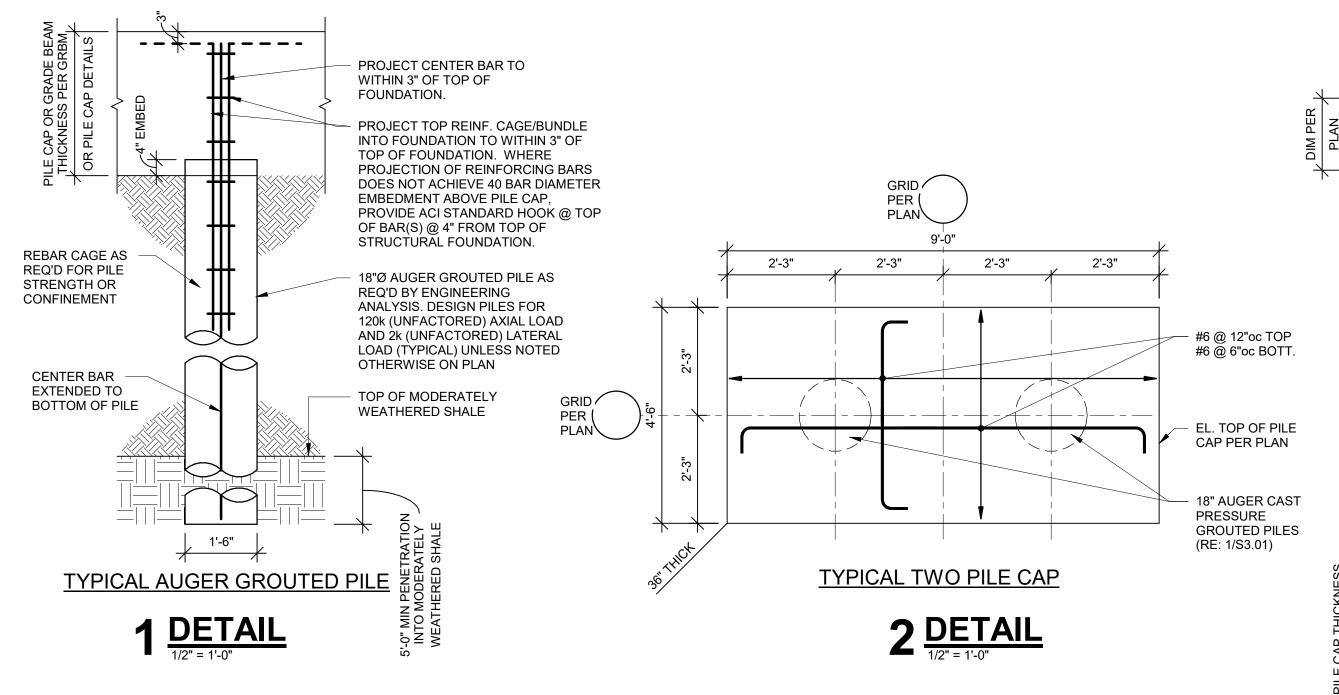
REGISTRATION

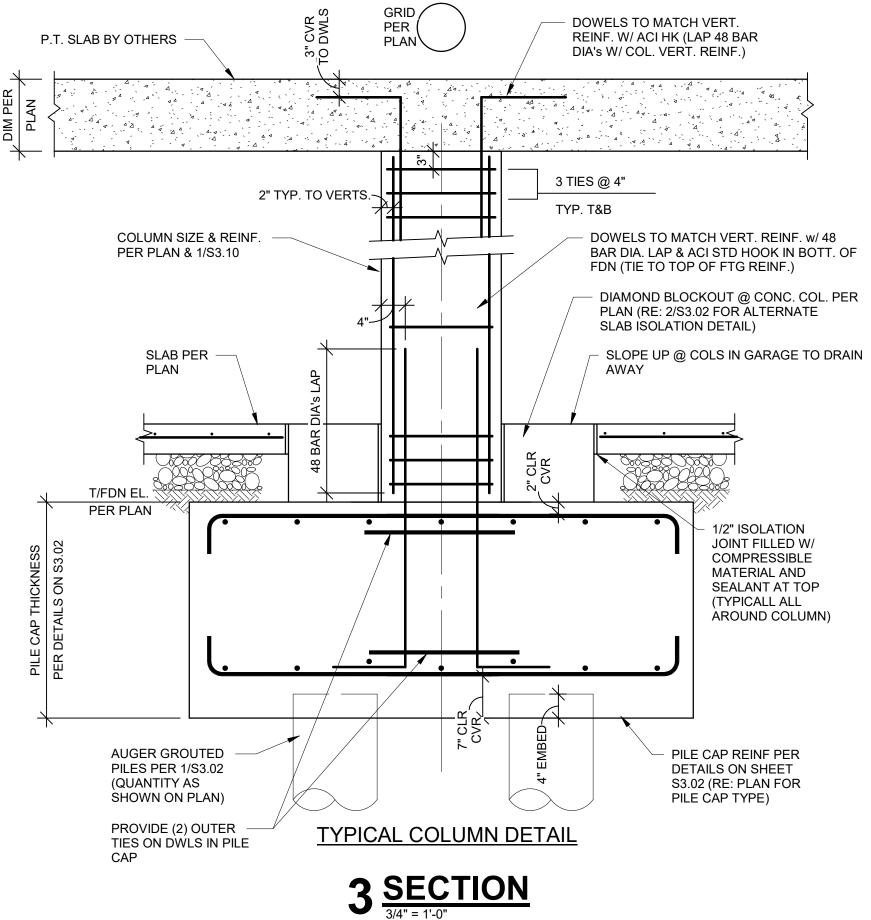


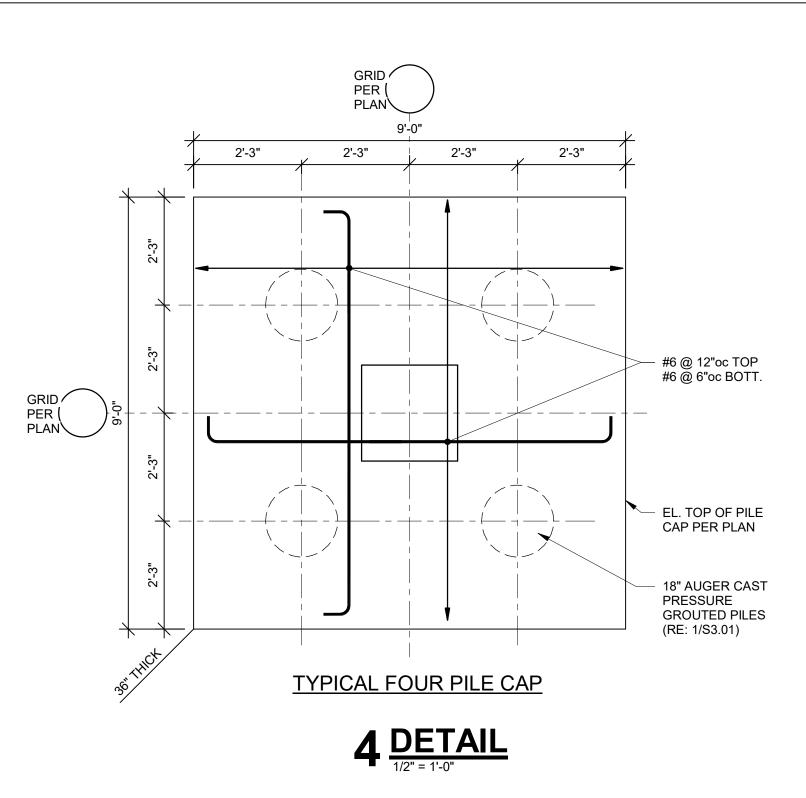
PROJECT TEAM FINKLE+WILLIAMS ARCHITECT ARCHITECTURE CIVIL **GBA ENGINEERS** LAND 3 LANDSCAPE STRUCTURAL BOB D. CAMPBELL PLUMBING LATIMER SOMMERS LATIMER SOMMERS MECHANICAL LATIMER SOMMERS ELECTRICAL FIRE PROTECTION LATIMER SOMMERS BRINKMANN CONSTRUCTORS CONTRACTOR

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GARAGE FOUNDATION DETAILS







	PILE	CAP SO	CHE	DULE				
ROCK BRG	PRESSURE (PSF): 25000	CONCRETE 3500	(PSI):	REBAR (KSI): 60				
TYPE	FOOTING S THICKNES	SIZE (FT.) SS (IN.)	C	TY/SIZE OF BARS EACH WAY				
3.0	3'-0" x 3'-0	" x 16"	(6) #4 BOTTOM					
3.5	3'-6" x 3'-6	" x 18"	18" (7) #5 BOTTOM					
(3.5A)	3'-6" x 3'-6	" x 18"	(7	(7) #5 TOP & BOTTOM				
4.0	4'-0" x 4'-0	" x 18"		(7) #5 BOTTOM				
(5.0)	5'-0" x 5'-0	" x 28"	(6) #4 BOTTOM					
(x4.5)	9'-0" x 4'-6	" x 36"	#6 @ 6"o	c BOTTOM / #6 @ 12"oc TOP				
9	9'-0" x 9'-0	" x 36"	#6 @ 6"o	c BOTTOM / #6 @ 12"oc TOP				
(12.5)	12'-6" x 12'-	6" x 48"						
(17)	17'-0" x 17'-	0" x 54"						
9 (12.5)	9'-0" x 9'-0 12'-6" x 12'-	" x 36" 6" x 48"						

1.) EXTERIOR FOOTINGS OR FOOTING AT GRADE BEAM SHALL MATCH GRADE BEAM DEPTH AND BE PLACE WITH GRADE BEAM. PROVIDE SPECIFIED REBAR TOP AND BOTTOM WITH 4 STANDEES TO SUPPORT MATS.

2.) PROVIDE #4 @ 12"o.c. EACH WAY IN TOP OF FTG. AT ALL MOMENT FRAMES AND AT BRACE BAY COLUMNS.3.) CENTER FOOTINGS ON COLUMNS AND/OR WALL CENTER LINES PER PLAN,

UNLESS OTHERWISE NOTED.
4.) PROVIDE ACI STANDARD HOOK AT EACH END OF BARS.

paragon star

PARAGON STAR NORTH VILLAGE

3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081

Project No.: 18017,19050.07,19050.08

Date: 02.23.2022

Issued For: BLDG 1 & 2 75% SET

REVISIONS

No. Date Description

REGISTRATION



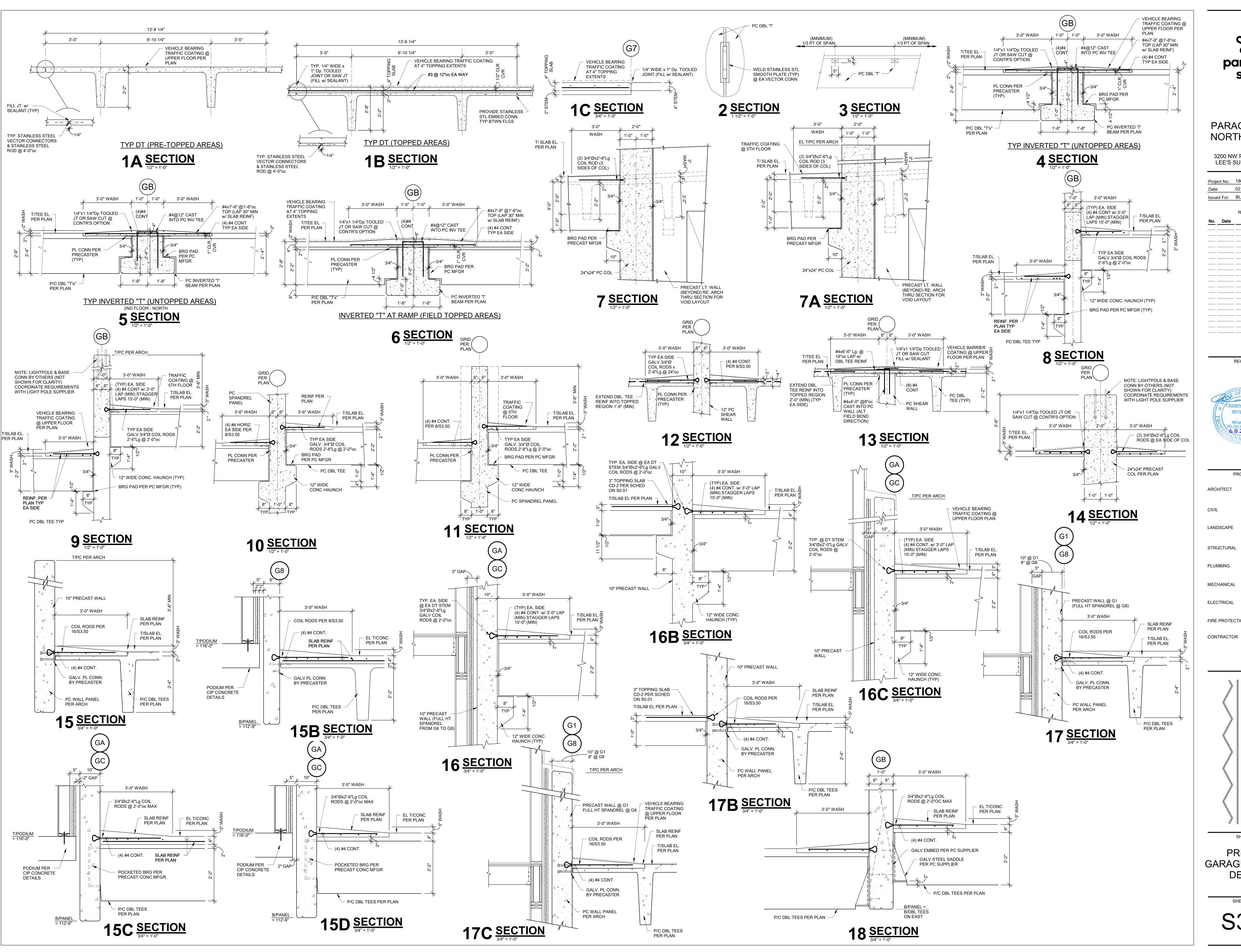
PROJECT TEAM ARCHITECT FINKLE+WILLIAMS ARCHITECTURE CIVIL **GBA ENGINEERS** LANDSCAPE LAND 3 STRUCTURAL BOB D. CAMPBELL PLUMBING LATIMER SOMMERS LATIMER SOMMERS MECHANICAL ELECTRICAL LATIMER SOMMERS FIRE PROTECTION LATIMER SOMMERS BRINKMANN CONSTRUCTORS CONTRACTOR

> BOB D. CAMPBELL & CO Structural Engineers Since 1953 4338 Belleview Ave. 816.531.414 Kansas City, MO 64111 www.bdc-engrs.cor

SHEET TITLE

PILE & PODIUM FOUNDATION DETAILS

S3 02



paragon star

PARAGON STAR NORTH VILLAGE

3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081

Project No.: 18017,19050.07,19050.08 02.23.2022 Issued For: BLDG 1 & 2 75% SET

REVISIONS

REGISTRATION



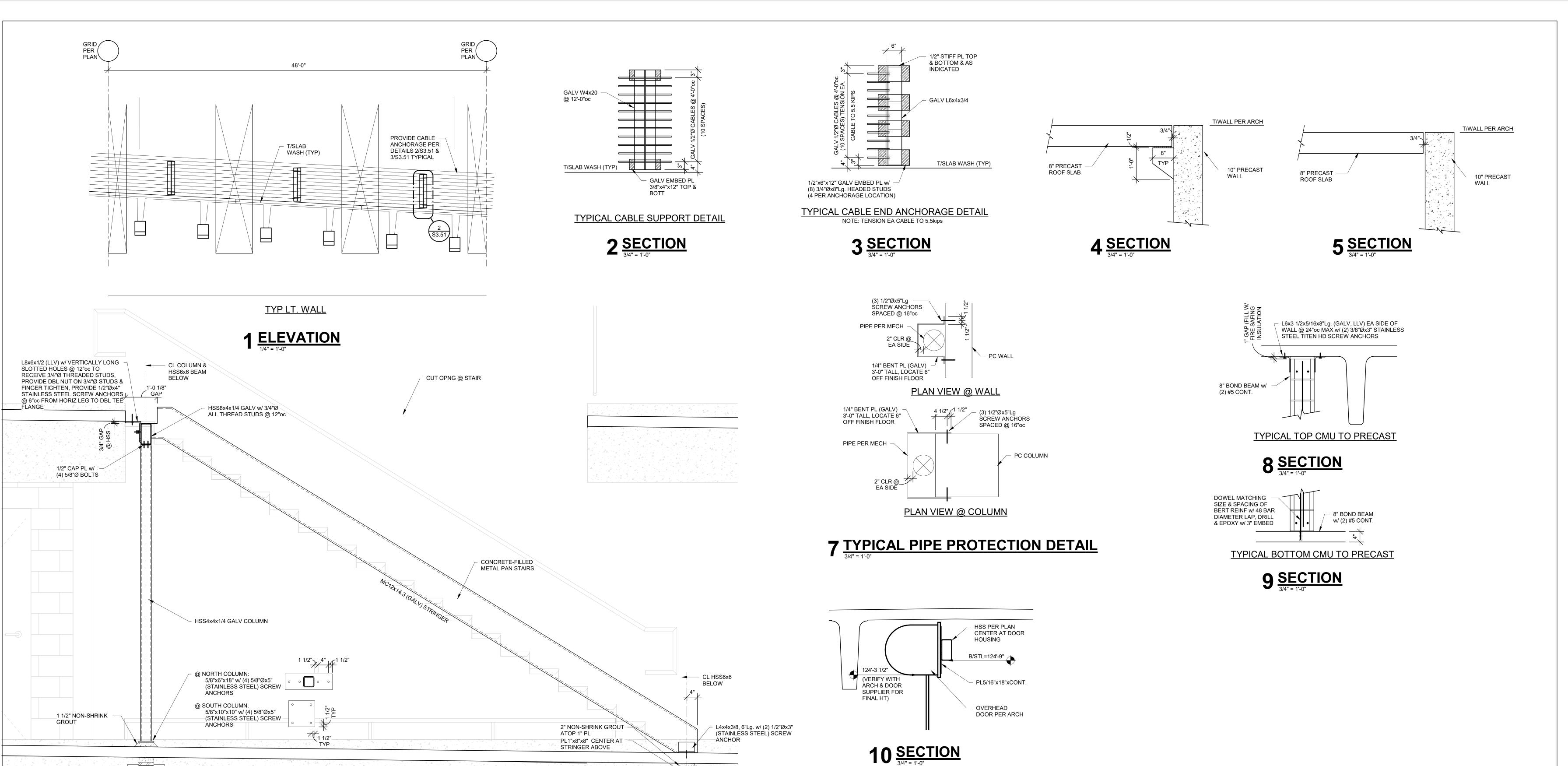
PROJECT TEAM FINKLE+WILLIAMS ARCHITECTURE GBA ENGINEERS LANDSCAPE STRUCTURAL BOB D. CAMPBELL LATIMER SOMMERS PLUMBING LATIMER SOMMERS **MECHANICAL** LATIMER SOMMERS ELECTRICAL FIRE PROTECTION LATIMER SOMMERS

BOB Structu 4338 B

BRINKMANN CONSTRUCTORS

SHEET TITLE **PRECAST** GARAGE FRAMING **DETAILS**

> SHEET NUMBER S3.50



HSS6x6x1/4 (GALV) w/ 1/2"x14"x8" -

END PLATES, ATTACH TO DBL TEE

STEM w/ (4) 5/8"Øx5" (STAINLESS STEEL) SCREW ANCHORS

HSS6x6x1/4 (GALV) w/ 1/2"x14"x8" END PLATES

 $6\frac{\text{SECTION}}{3/4" = 1'-0"}$

ATTACH TO DBL TEE STEM w/ (4) 5/8"Øx5"

(STAINLESS STEEL) SCREW ANCHORS

paragon star

PARAGON STAR NORTH VILLAGE

3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081

Project No.: 18017,19050.07,19050.08

Date: 02.23.2022

Issued For: BLDG 1 & 2 75% SET

REVISIONS

No. Date Description

REGISTRATION



PROJECT TEAM FINKLE+WILLIAMS ARCHITECT ARCHITECTURE CIVIL **GBA ENGINEERS** LAND 3 LANDSCAPE STRUCTURAL BOB D. CAMPBELL LATIMER SOMMERS PLUMBING MECHANICAL LATIMER SOMMERS ELECTRICAL LATIMER SOMMERS FIRE PROTECTION LATIMER SOMMERS

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nsas City, MO 64111 www.bdc-engrs.com

BRINKMANN CONSTRUCTORS

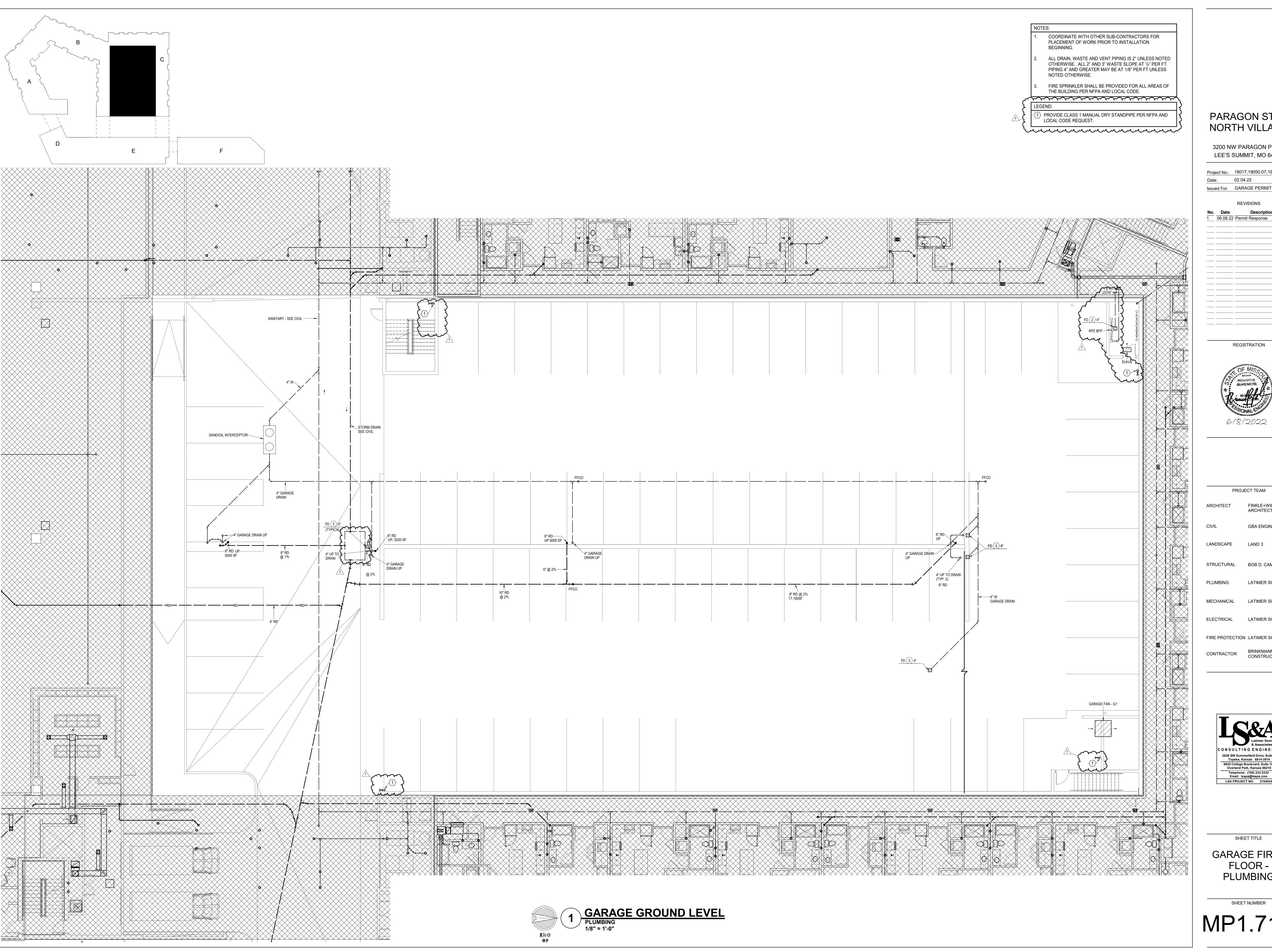
CONTRACTOR

SHEET TITLE

PRECAST

PRECAST GARAGE FRAMING DETAILS

S3.51



3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081

REGISTRATION



PROJECT TEAM

FINKLE+WILLIAMS ARCHITECTURE

LATIMER SOMMERS

LATIMER SOMMERS

LATIMER SOMMERS

FIRE PROTECTION LATIMER SOMMERS

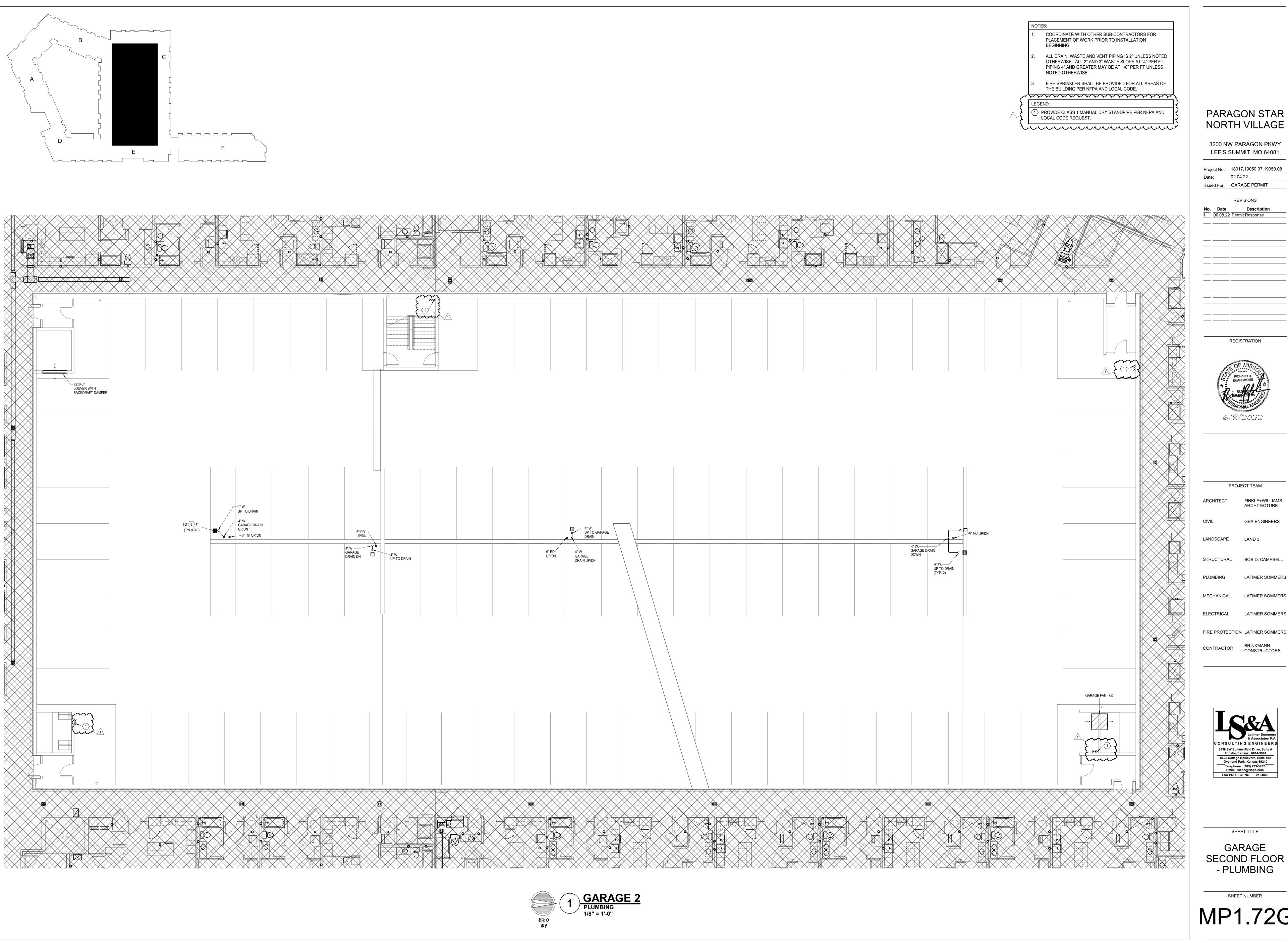


SHEET TITLE

GARAGE FIRST FLOOR -PLUMBING

SHEET NUMBER

MP1.71G



3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081

Project No.: 18017,19050.07,19050.08

REGISTRATION

PROJECT TEAM

FINKLE+WILLIAMS ARCHITECTURE

GBA ENGINEERS

STRUCTURAL BOB D. CAMPBELL

LATIMER SOMMERS

LATIMER SOMMERS MECHANICAL

LATIMER SOMMERS

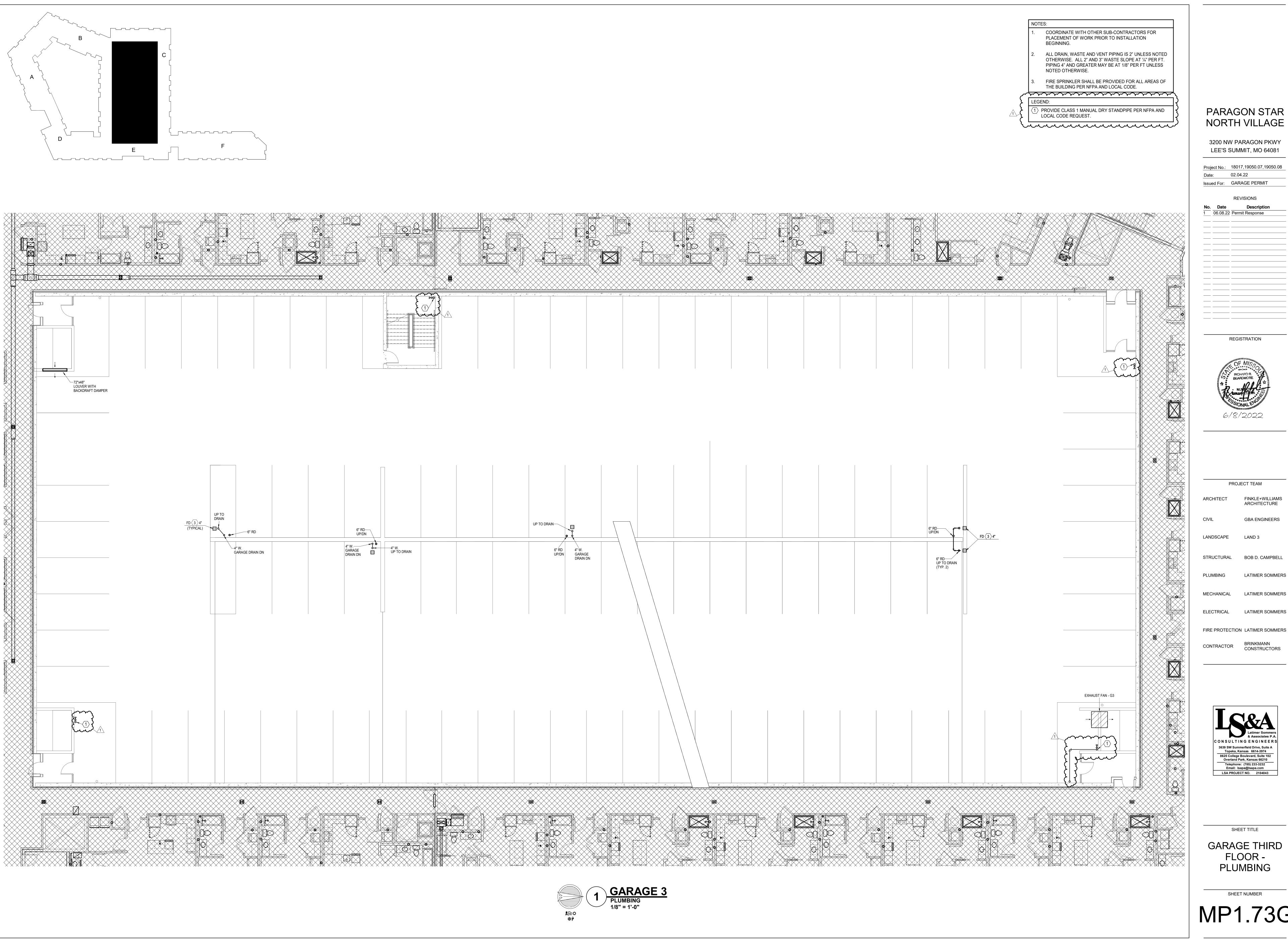
BRINKMANN CONSTRUCTORS

3639 SW Summerfield Drive, Suite A Topeka, Kansas 6614-3974 8625 College Boulevard, Suite 102 Overland Park, Kansas 66210

SHEET TITLE

GARAGE SECOND FLOOR - PLUMBING

MP1.72G



3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081

Project No.: 18017,19050.07,19050.08

REGISTRATION



FINKLE+WILLIAMS ARCHITECTURE

GBA ENGINEERS

STRUCTURAL BOB D. CAMPBELL

LATIMER SOMMERS

LATIMER SOMMERS

LATIMER SOMMERS

BRINKMANN CONSTRUCTORS

CONSULTING ENGINEERS 3639 SW Summerfield Drive, Suite A
Topeka, Kansas 6614-3974

8625 College Boulevard, Suite 102
Overland Park, Kansas 66210

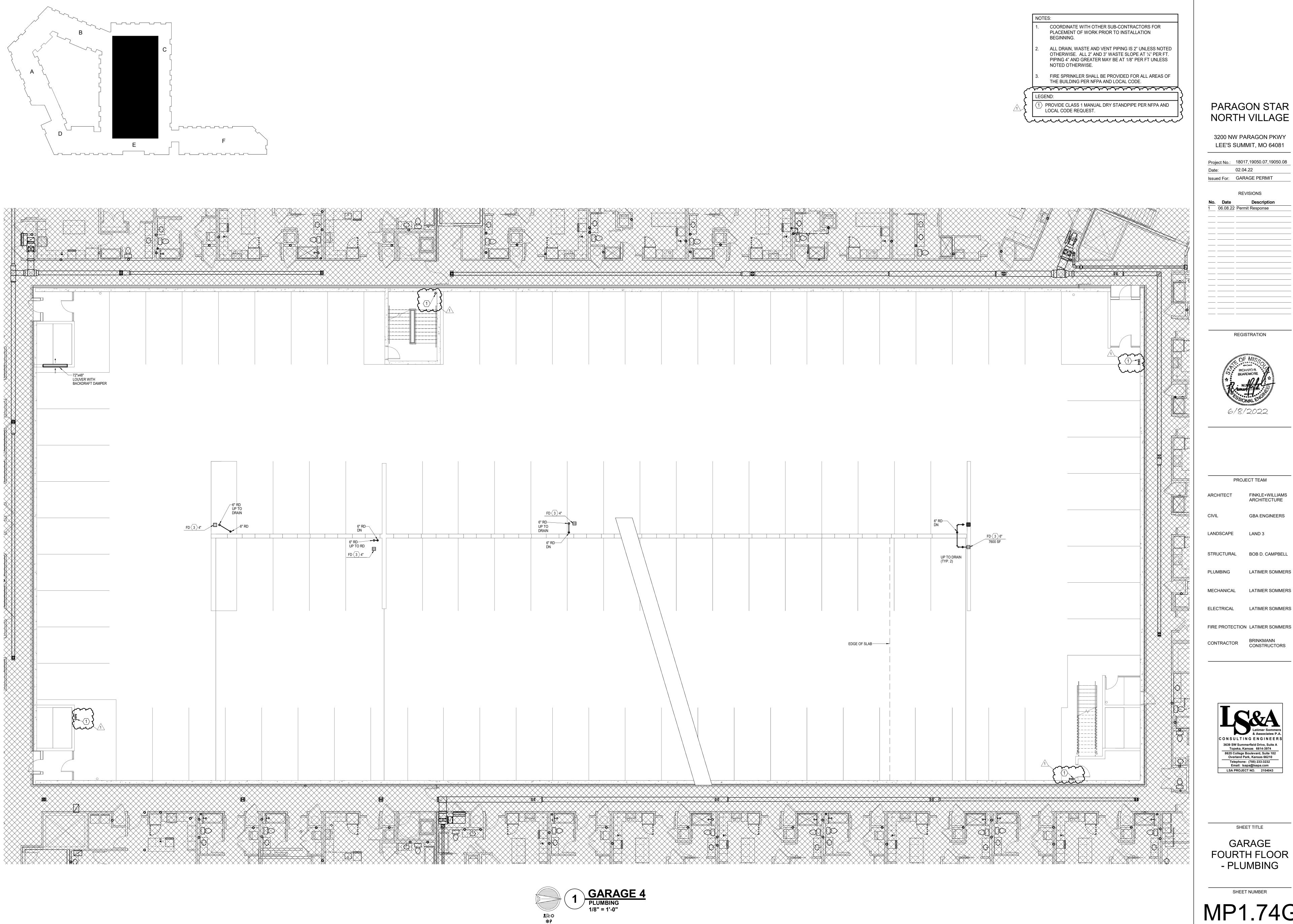
Telephone: (785) 233-3232
Email: Isapa@Isapa.com

LSA PROJECT NO. 2104043

SHEET TITLE

GARAGE THIRD FLOOR -PLUMBING

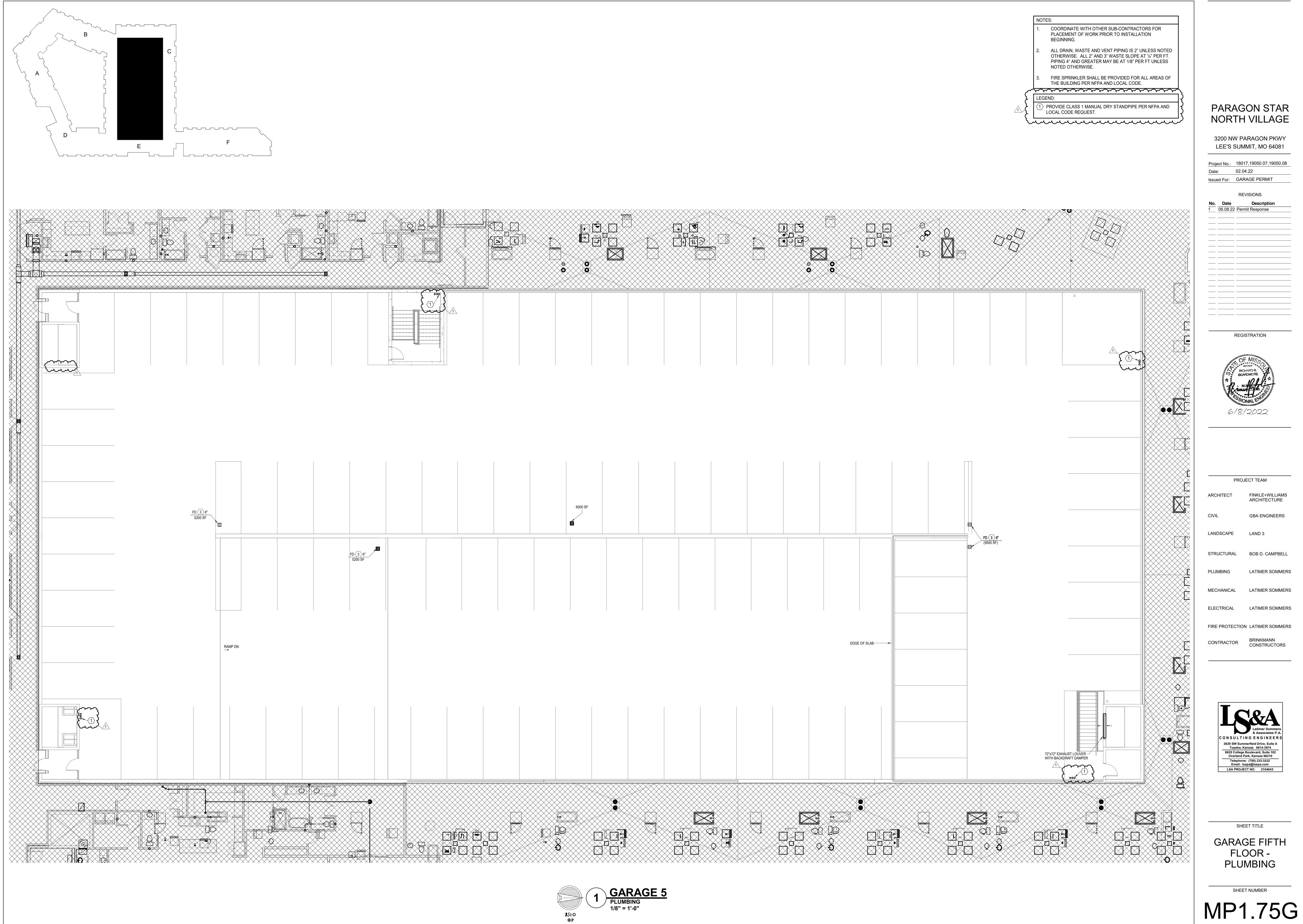
MP1.73G



PARAGON STAR

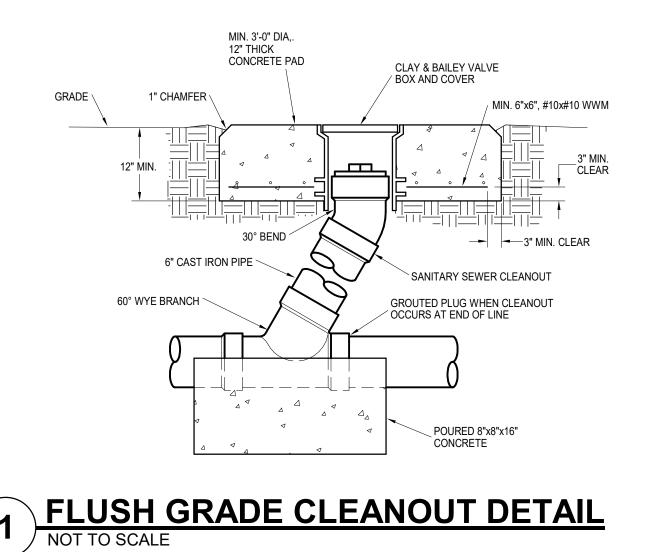
GBA ENGINEERS

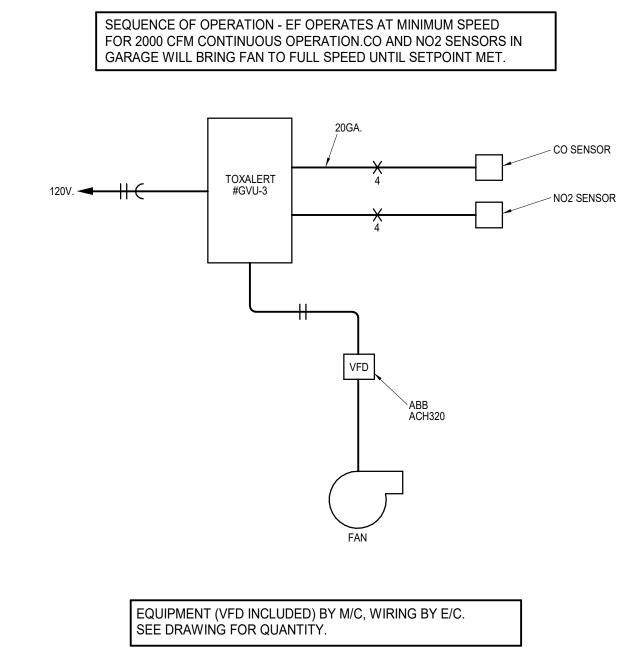
LATIMER SOMMERS



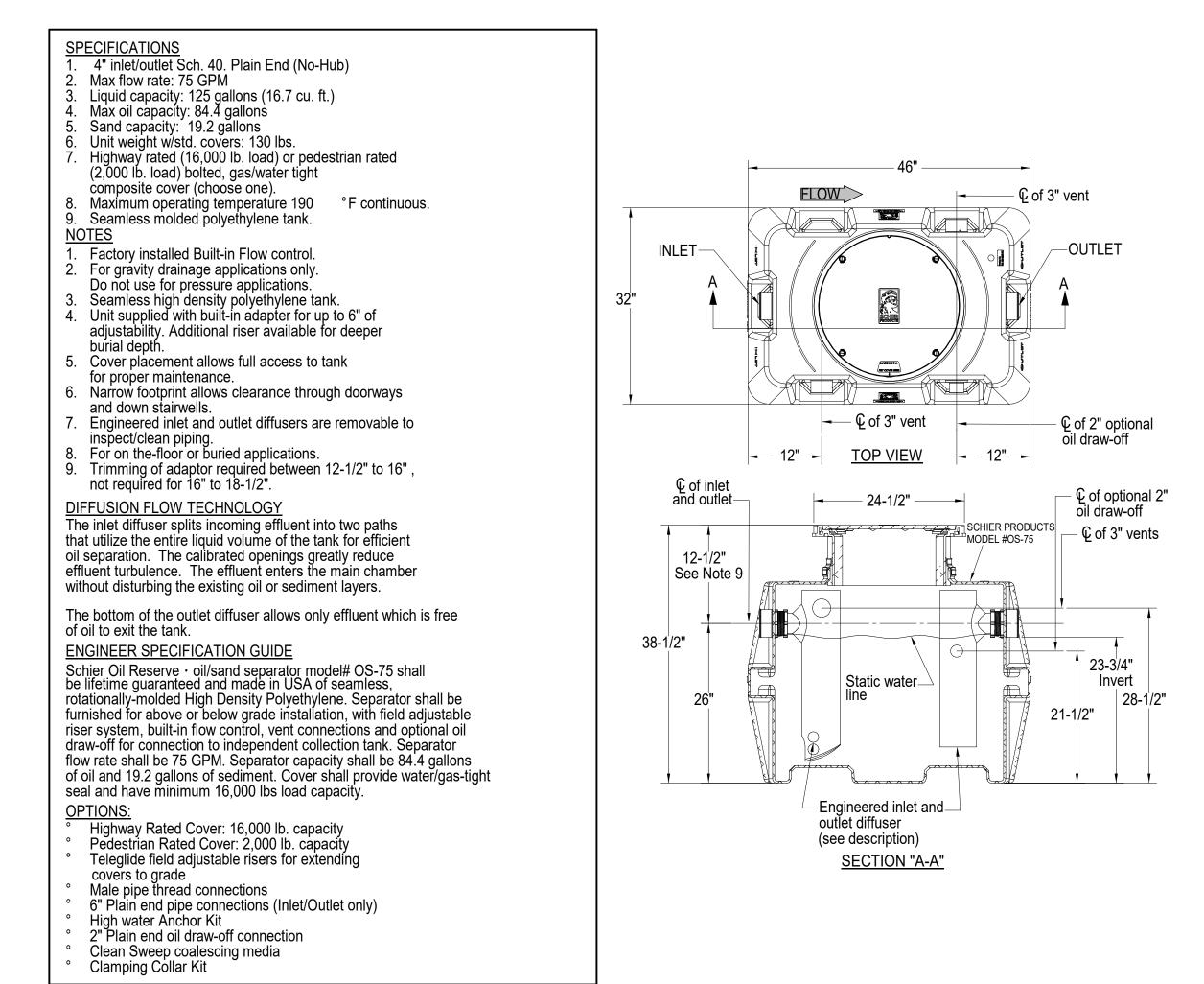
PARAGON STAR







2 PARKING GARAGE FAN CONTROL DETAIL
NOT TO SCALE



3 SAND/OIL SEPERATOR DETAIL NOT TO SCALE

2-CEILING GRILLE, DS, WALL/ROOF CAP

3-WITH CRD

		EXHAUST F	AN SC	HED	ULL	ı				I
ELECTRICAL										
MARK	MFGR	MODEL	CFM	ESP	FAN HP	VOLTS/ PH	FLA	ОСР	WIRING	CONFIGURATION NOTES
EF-G1, G2, G3	GREENHECK	SB-2L48-30	25000	0.25	3.00	208/3	8	15	(3) #12	sidewall, 1
APT EF	BROAN	LP50100DC	80	0.1	Fr.	120/1	1	15	(3) #12	ceiling, 2, 3

3200 NW PARAGON PKWY
LEE'S SUMMIT, MO 64081

Project No.: 18017,19050.07,19050.08

Date: 02.04.22

Issued For: GARAGE PERMIT

REVISIONS

PARAGON STAR

NORTH VILLAGE

No. Date Description

REGISTRATION



PROJECT TEAM

ARCHITECT FINKLE+WILLIAMS ARCHITECTURE

CIVIL GBA ENGINEERS

LANDSCAPE LAND 3

STRUCTURAL BOB D. CAMPBELL

PLUMBING LATIMER SOMMERS

MECHANICAL LATIMER SOMMERS

ELECTRICAL LATIMER SOMMERS

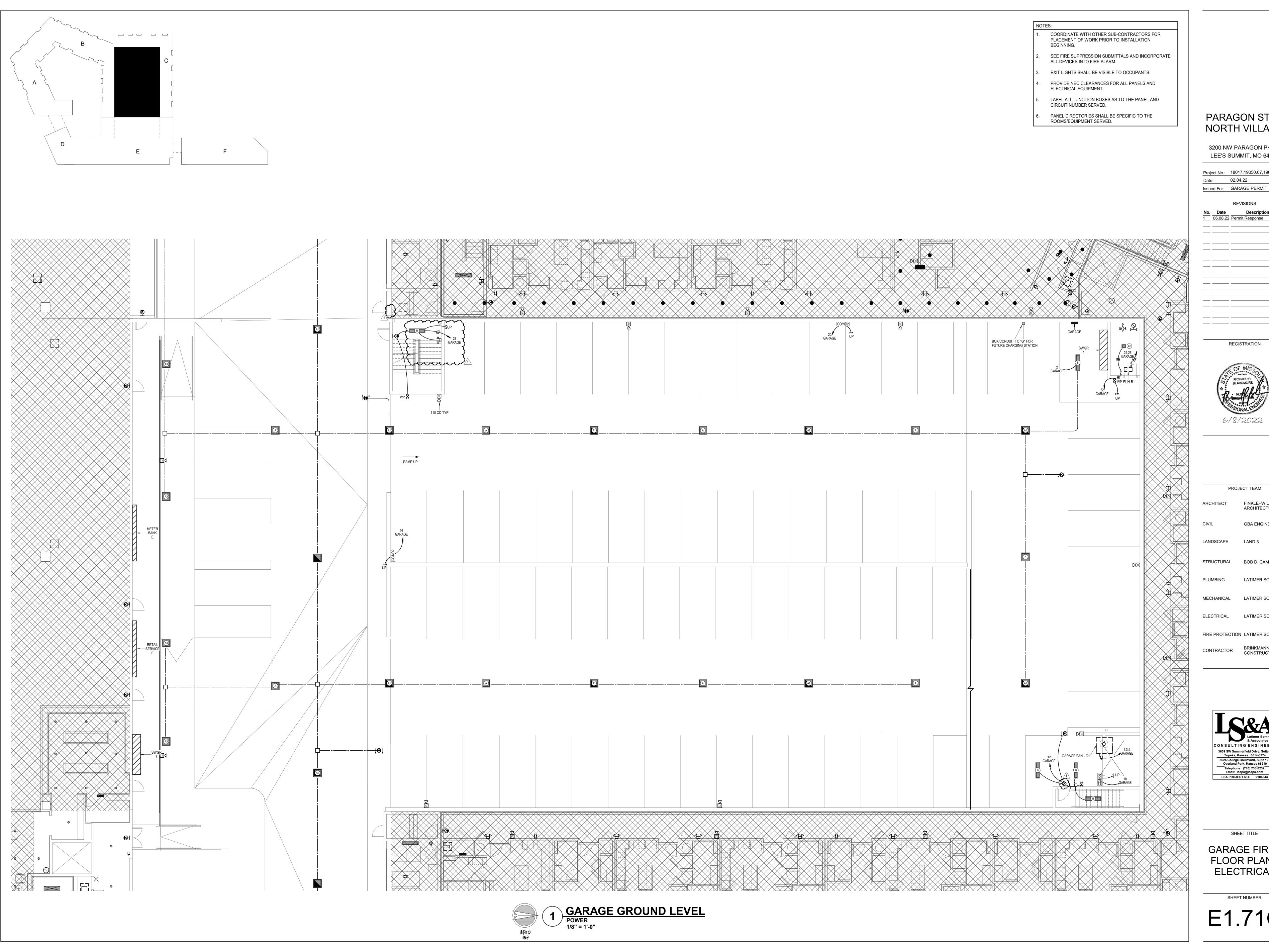
FIRE PROTECTION LATIMER SOMMERS

CONTRACTOR BRINKMANN CONSTRUCTORS



SHEET TITLE

MECHANICAL DETAILS/SCHEDULES



3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081

Project No.: 18017,19050.07,19050.08

REGISTRATION



PROJECT TEAM

FINKLE+WILLIAMS ARCHITECTURE

LATIMER SOMMERS

LATIMER SOMMERS

ELECTRICAL LATIMER SOMMERS

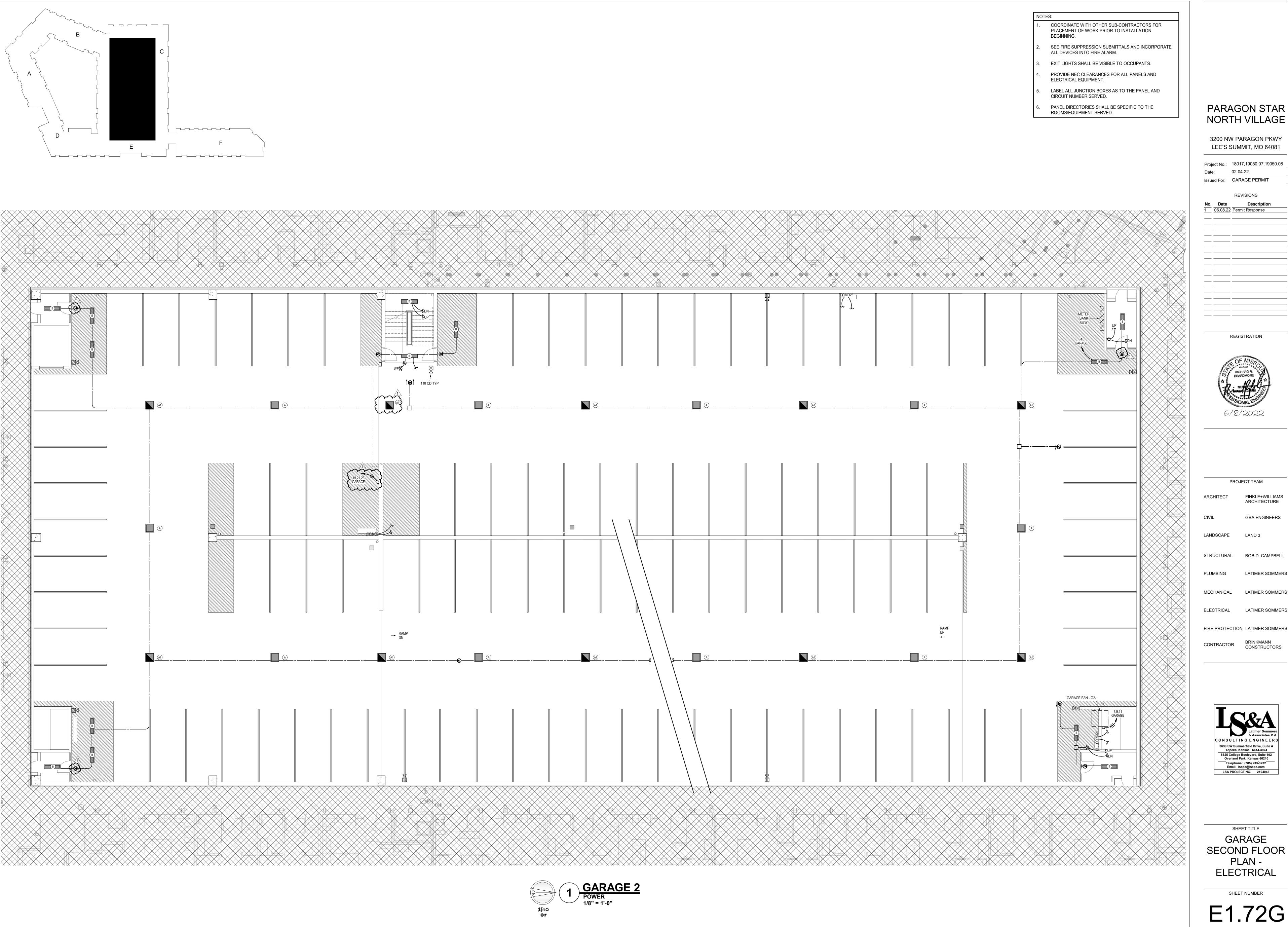
FIRE PROTECTION LATIMER SOMMERS



SHEET TITLE

GARAGE FIRST FLOOR PLAN -ELECTRICAL

E1.71G



3200 NW PARAGON PKWY

REGISTRATION



PROJECT TEAM FINKLE+WILLIAMS ARCHITECTURE

GBA ENGINEERS

STRUCTURAL BOB D. CAMPBELI

LATIMER SOMMERS

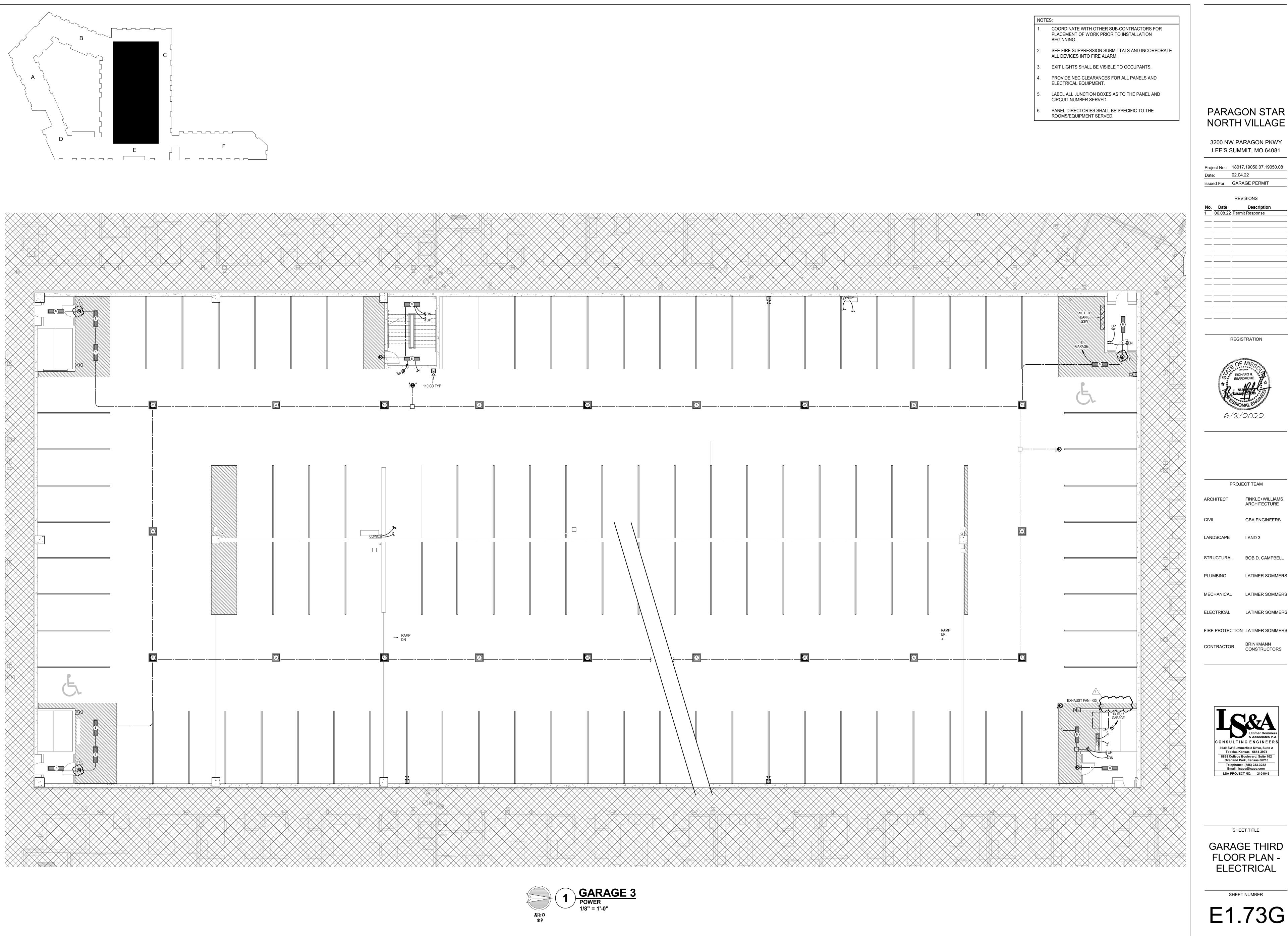
LATIMER SOMMERS

LATIMER SOMMERS

BRINKMANN CONSTRUCTORS CONTRACTOR

> 3639 SW Summerfield Drive, Suite A Topeka, Kansas 6614-3974 8625 College Boulevard, Suite 102 Overland Park, Kansas 66210 Telephone: (785) 233-3232 Email: Isapa@Isapa.com

SHEET TITLE



3200 NW PARAGON PKWY



PROJECT TEAM

FINKLE+WILLIAMS ARCHITECTURE

BOB D. CAMPBELI

LATIMER SOMMERS

LATIMER SOMMERS

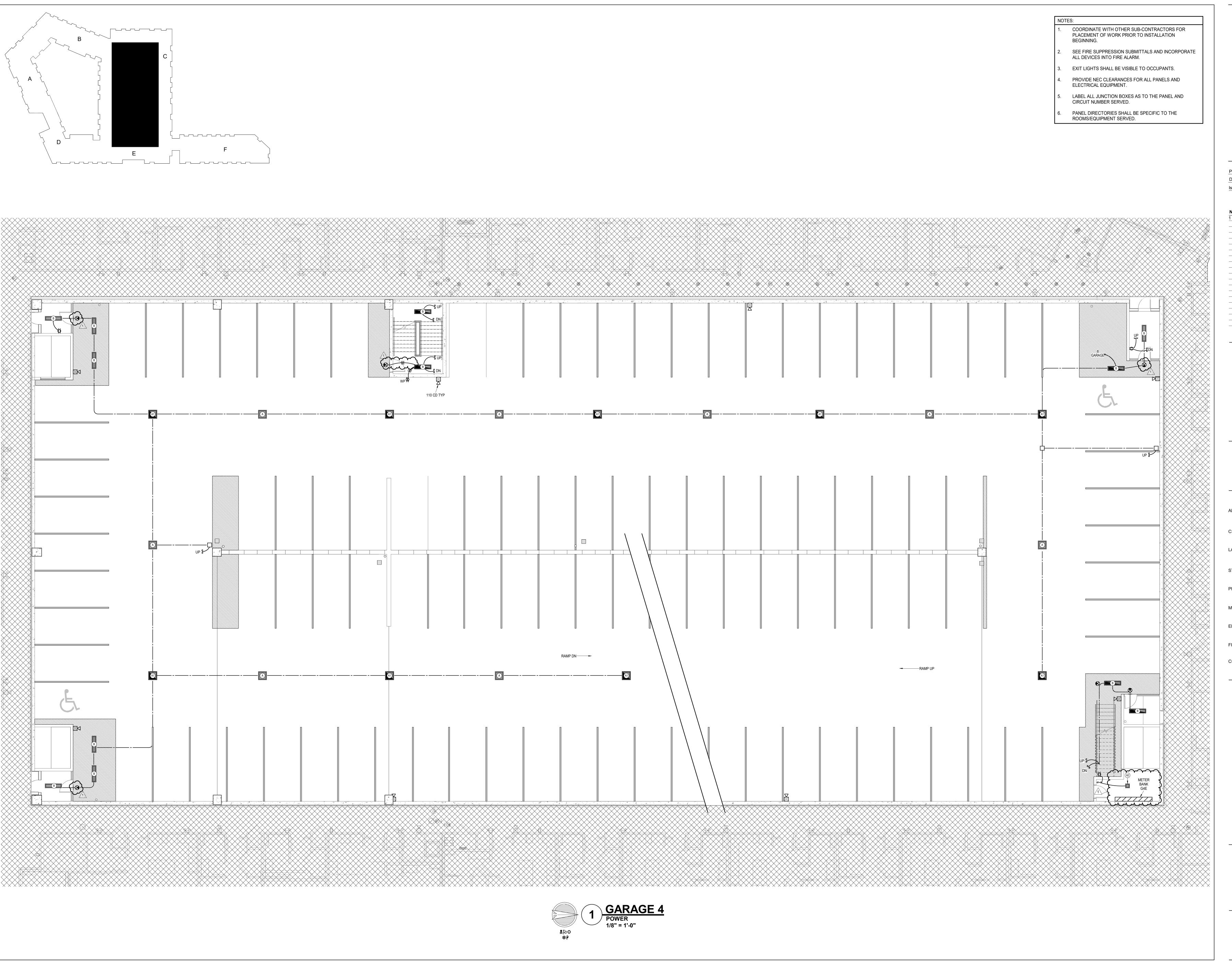
LATIMER SOMMERS

BRINKMANN CONSTRUCTORS

8625 College Boulevard, Suite 102 Overland Park, Kansas 66210 Telephone: (785) 233-3232 Email: Isapa@Isapa.com

SHEET TITLE

GARAGE THIRD



3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081

oject No.: 18017,19050.07,19050.0

For: GARAGE PERMIT

No. Date Description
1 06.08.22 Permit Response

REGISTRATION



6/8/2022

PROJECT TEAM

FINKLE+WILLIAMS ARCHITECTURE

GBA ENGINEE

DSCAPE LAND 3

STRUCTURAL BOB D. CAMPBELI

BING LATIMER SOMMERS

FIRE PROTECTION LATIMER SOMMERS

LATIMER SOMMERS

LATIMER SOMMERS

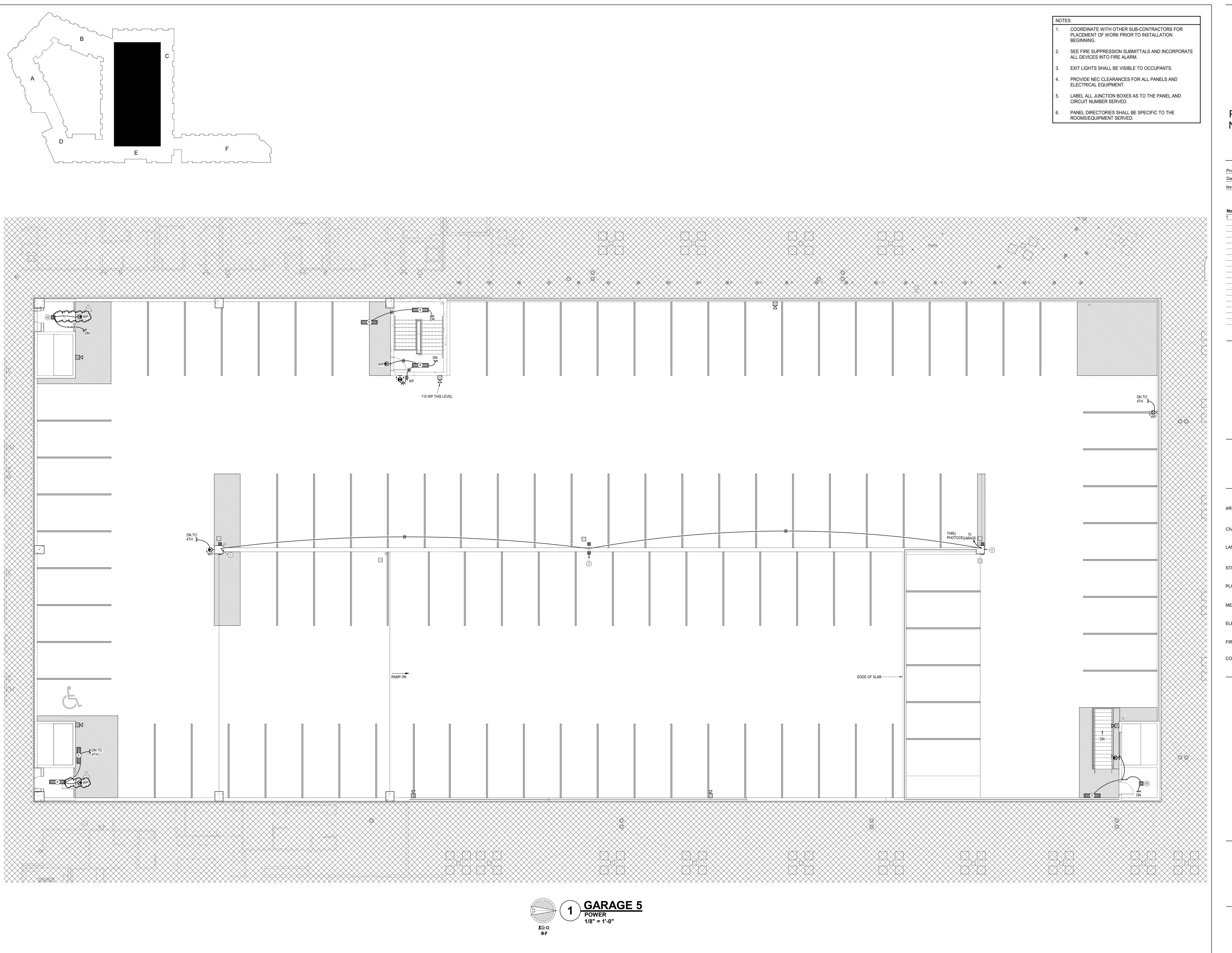
CONTRACTOR BRINKMANN
CONSTRUCTORS



GARAGE
FOURTH FLOOF
PLAN FLECTRICAL

SHEET NUMBER

E1.74G



3200 NW PARAGON PKWY LEE'S SUMMIT, MO 64081

et No.: 18017,19050.07,19050.08

Issued For: GARAGE PERMIT

No. Date Description

06.08.22 Permit Response

REGISTRATION



6/8/2022

PROJECT TEAM

CT FINKLE+WILLIAMS ARCHITECTURE

GBA ENGINEERS

SCAPE LAND 3

STRUCTURAL BOB D. CAMPBELL

NG LATIMER SOMMERS

MECHANICAL LATIMER SOMMERS

CAL LATIMER SOMMERS

FIRE PROTECTION LATIMER SOMMERS

RACTOR BRINKMANN
CONSTRUCTORS

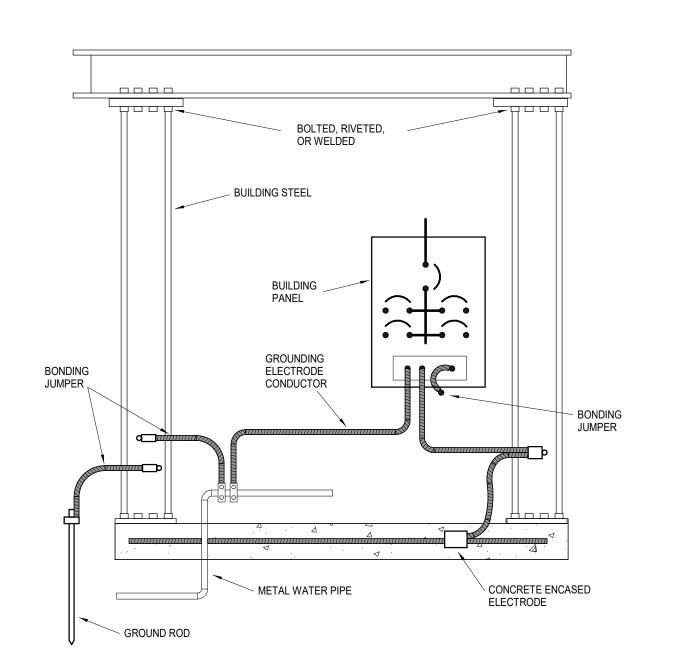


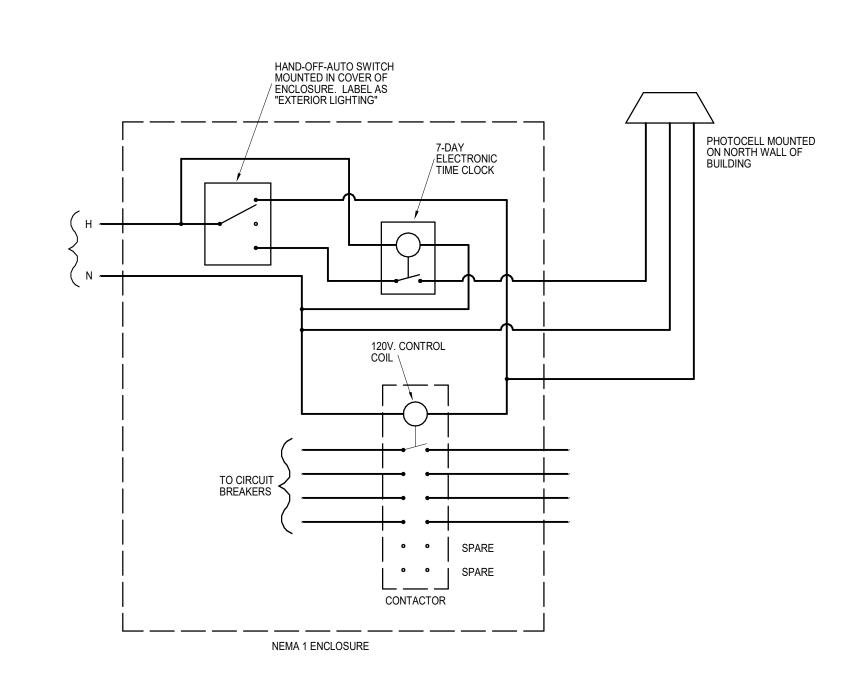
SHEET TITLE

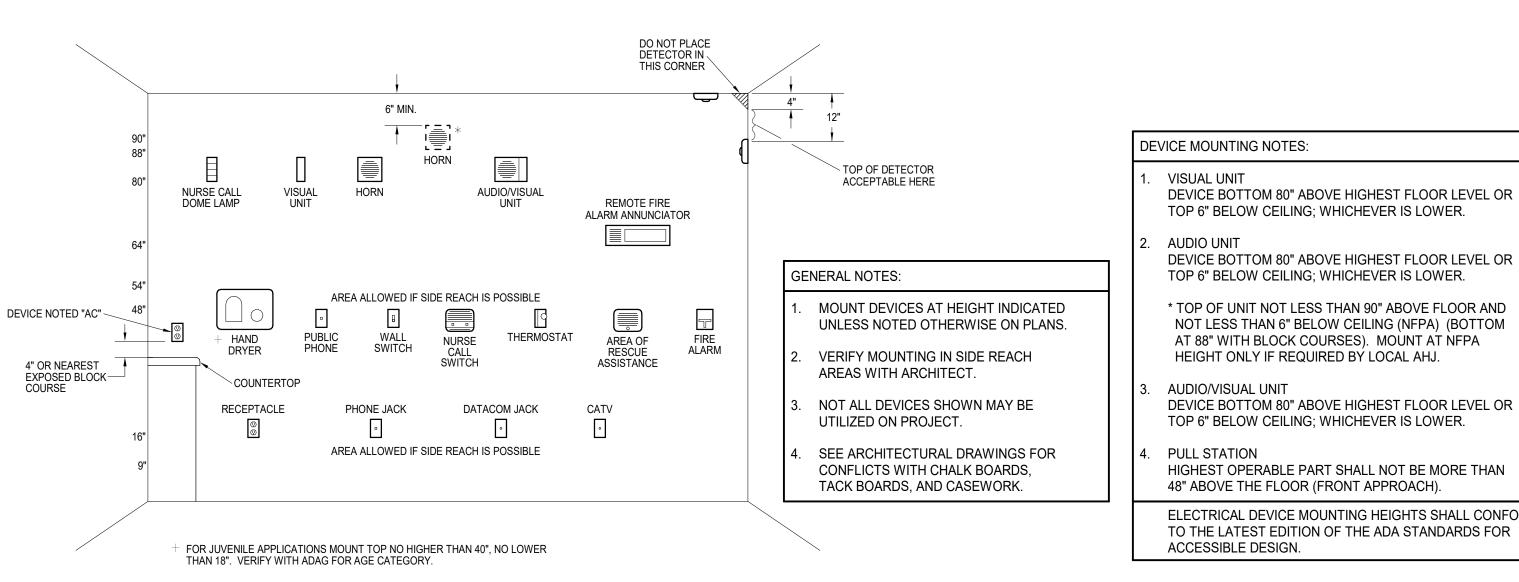
GARAGE FIFTH FLOOR PLAN -FLECTRICAL

SHEET NUMBER

E1.75G







> LEE'S SUMMIT, MO 64081 Project No.: 18017,19050.07,19050.08

Issued For: GARAGE PERMIT

3200 NW PARAGON PKWY

REVISIONS

REGISTRATION

PROJECT TEAM

ARCHITECT

LANDSCAPE

STRUCTURAL

PLUMBING

MECHANICAL

ELECTRICAL

CONTRACTOR

CIVIL

FINKLE+WILLIAMS

ARCHITECTURE

GBA ENGINEERS

BOB D. CAMPBELL

LATIMER SOMMERS

LATIMER SOMMERS

LATIMER SOMMERS

BRINKMANN CONSTRUCTORS

FIRE PROTECTION LATIMER SOMMERS

LAND 3

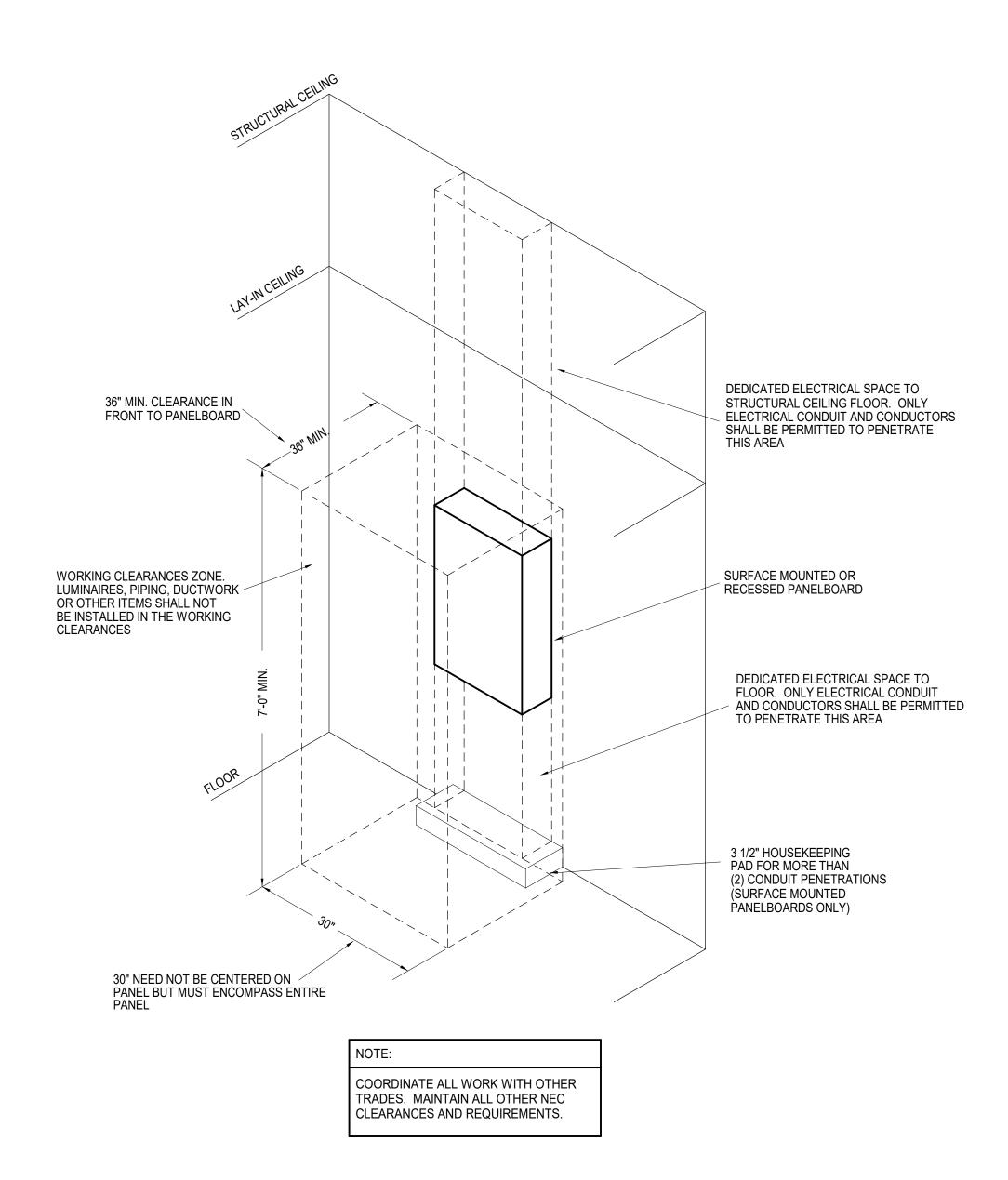
TOP 6" BELOW CEILING; WHICHEVER IS LOWER. HIGHEST OPERABLE PART SHALL NOT BE MORE THAN 48" ABOVE THE FLOOR (FRONT APPROACH).

ELECTRICAL DEVICE MOUNTING HEIGHTS SHALL CONFORM TO THE LATEST EDITION OF THE ADA STANDARDS FOR

GROUNDING ELECTRODE SYSTEM DETAIL
NOT TO SCALE

2 EXTERIOR LIGHTING CONTROL PANEL SCHEMATIC NOT TO SCALE

3 ELECTRICAL DEVICE MOUNTING HEIGHTS NOT TO SCALE



MARK	DESCRIPTION	MFGR	MODEL	MOUNTING	FINISH	LAMPS	NOTES
Α	Garage Light	Royal Pacific	4431OB-60-MS	surface	standard	60W 4000K	with 50% motion dimming
A1	Garage Light	Royal Pacific	4431OB-60-MS	surface	standard	60W 4000K	with 50% motion dimming/battery
A2	Surface Globe	Lithonia	LDN4CYL-40/LO4/AR/LSS/fcm	surface	white	1000 Lumen 4000K 10W	
В	Strip	Lithonia	MNSL-L46 1LL MVOLt 40k	surface	white	20W 4000K LED	with battery
С	Double head Pole	McGraw Edison	(2) GLEON-SA2C-740-U-5WQ	jb/surface	white	15,200 Lumen 4000K 115W LED	16 ft pole
X1	Exit	Lithonia	LQM SW3R	surface	white/red	LED	
X2	Exit - waterproof	Lithonia	WLTE-W-1R-EL	surface	white/red	LED	

	Location: GA Supply From: Mounting: SUI Enclosure:			Volts: 120/208 Wye Phases: 3 Wires: 4							A.I.C. Rating: 10000 Mains Type: MLO Mains Rating: 225 A MCB Rating:					
Notes	::															
СКТ	Circuit Description	Trip	Wire Size		A		В		<u> </u>	Wire Size	Trip	Circui	t Description	CI		
1	•			1321	2060					12	20 A	Lighting	•	2		
3	GARAGE EF	15 A	12			1321	1600			12	20 A	Lighting		4		
5			<u>L</u> _					1321	1680	12	20 A	Lighting		6		
7				1321	1752					12	20 A	Lighting		3		
9	GARAGE EF	15 A	12			1321	600			12	20 A	Lighting		1		
11								1321	737	12	20 A	Lighting		1		
13				1321										1		
15	GARAGE EF	15 A	12			1321	800			12	20 A	CO SENSO		10		
17					000			1321	800	12	20 A	CO SENSO		1		
19	Daws 0 4005	00.4		0	800		000			12	20 A	CO SENSO	K	2		
21	Power Space 1305	20 A				0	296		0000	12	20 A	Receptacle		2		
23 25					2000			0	2808	8	35 A	EUH-B		2		
25 27					2808		1995				20 A	Lighting STA	VIR 106	2		
29							1990				20 A	Lighting STF	AII \ 100	3		
31														3:		
33														3		
35														3		
37														3		
39														4		
41														4		
	I	Total	Load:	1126	66 VA	918	4 VA	9919	VA							
		Total	Amps:	98	5 A	77	7 A	84	Α							
Leger			.	411	d D		4	F-4i	-4- J			Daniel	Tatala			
Load HVAC	Classification			ted Loa 05 VA	iu De	mand Fa			ated 05 VA			Panel	Totals			
Lightir				2 VA		125.009			00 VA		Total	Conn. Load:	30362 \/Δ			
Other				0 VA		100.009			0 VA 0 VA			st. Demand:				
Power				O VA		100.009) VA			Total Conn.:				
Recep				0 VA		100.00%			0 VA			st. Demand:				
							-		•							
Notes);						I									

4 TYPICAL PANELBOARD INSTALLATION DETAIL
NOT TO SCALE

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SHEET TITLE

ELECTRICAL DETAILS/SCHEDULES