

of these plans for your specific site and application.

Care and effort have gone into the creation and design of this plan. However, the designer is not an architect or engineer and construction from these plans should not be undertaken without the assistance of a construction professional, architect or engineer. Because of the mpossibility of any on site consultation and supervision, Viewpoint Residential Design, LLC, and Designer assume no responsibility for any damages, including structural failures, due to any deficiencies, omissions or error in the design or blueprints. Also, site conditions may vary from those illustrated on this plan. Designer does not warrant the suitability of these plans for use on your specific site. Consult your architect to determine the suitability

whosoever believeth in him should not perish, but have everlasting life" (John 3:16). E: Plans@ViewpointDesign.net

The Townhomes of Chapel Ridge - 2nd Plat

Street Address:

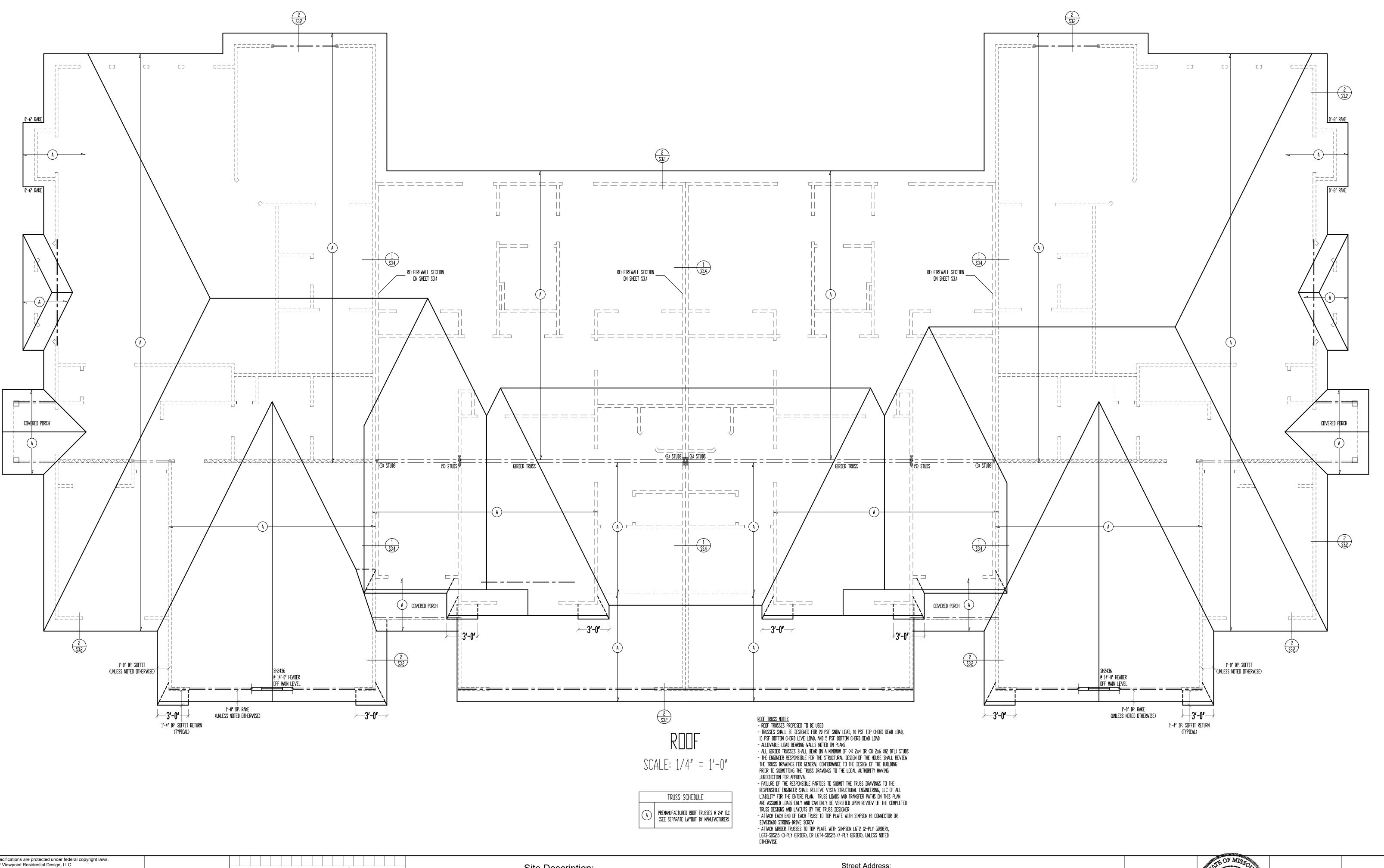
4027, 4025, 4023, & 4021 NE Sagamore Dr.,
Lee's Summit, Missouri
General Contractor:

Kevin Higdon Construction, LLC

Date: 9 - 22 - AD 2021 Rev. 1: 4 - 20 - AD 2022 Rev. 2: Rev. 3:





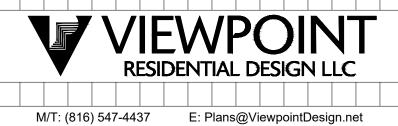


These plans and specifications are protected under federal copyright laws.

of these plans for your specific site and application.

Copyright A.D. 2022 Viewpoint Residential Design, LLC. Care and effort have gone into the creation and design of this plan. However, the designer is not an architect or engineer and construction from these plans should not be undertaken without the assistance of a construction professional, architect or engineer. Because of the mpossibility of any on site consultation and supervision, Viewpoint Residential Design, LLC, and Designer assume no responsibility for any damages, including structural failures, due to any deficiencies, omissions or error in the design or blueprints. Also, site conditions may vary from those illustrated on this plan. Designer does not warrant the suitability of these plans for use on your specific site. Consult your architect to determine the suitability

"For God so loved the world, that he gave his only begotten Son, that whosoever believeth in him should not perish, but have everlasting life" (John 3:16).

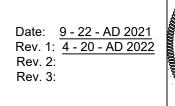


Site Description: Lot 15, The Townhomes of Chapel Ridge - 2nd Plat

Street Address:

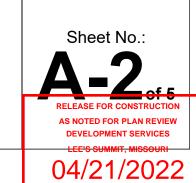
4027, 4025, 4023, & 4021 NE Sagamore Dr.,
Lee's Summit, Missouri

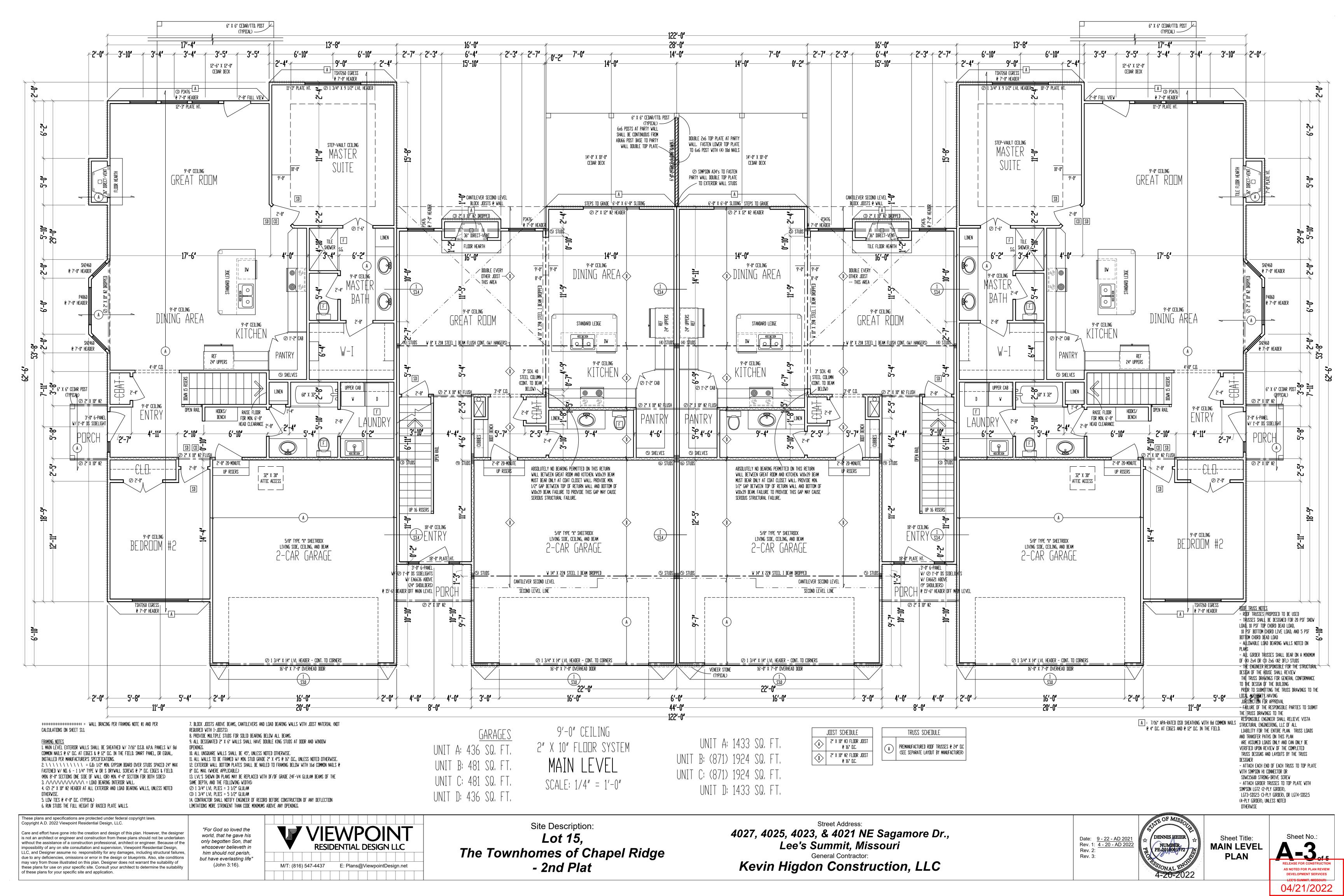
General Contractor: Kevin Higdon Construction, LLC

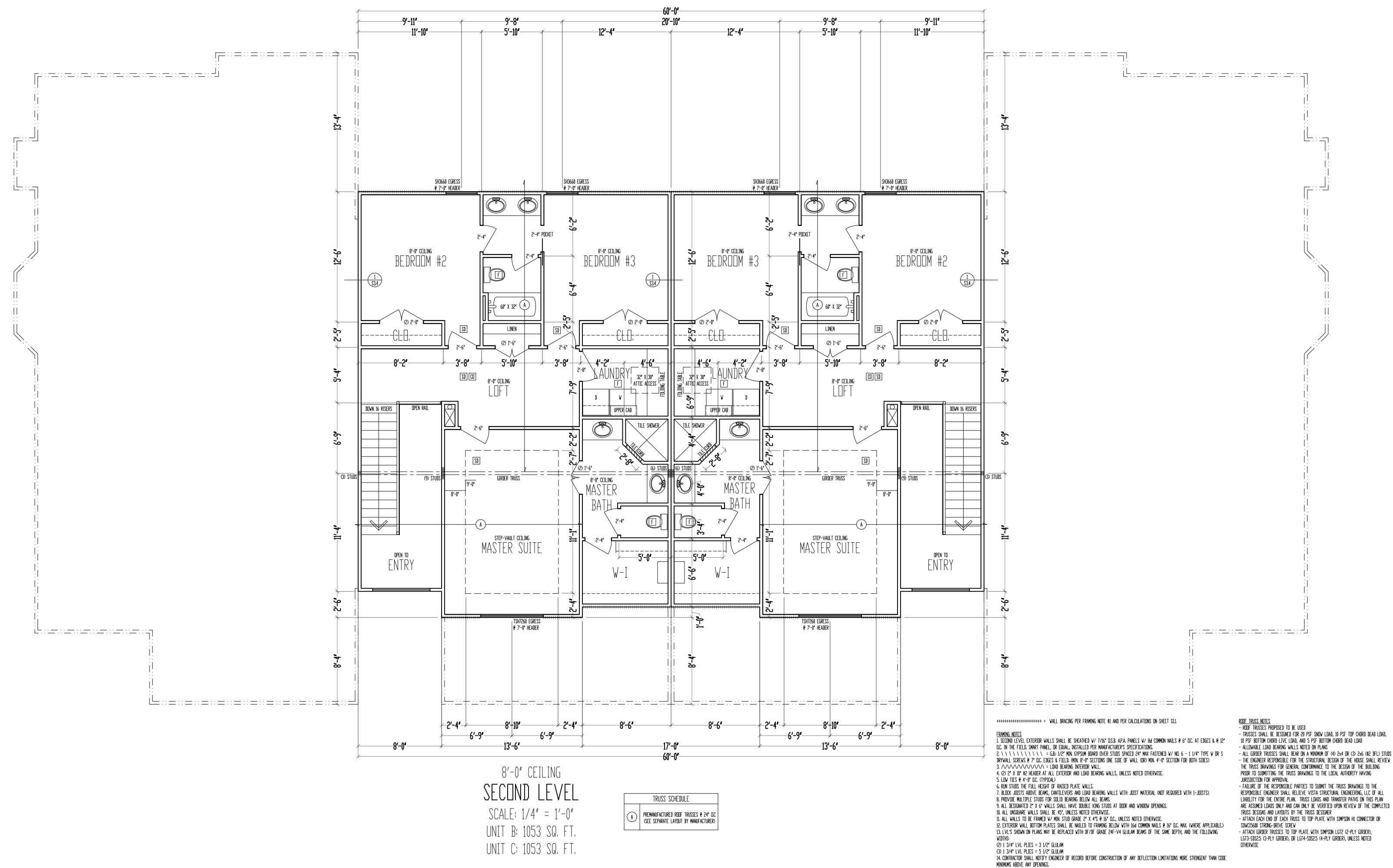




Sheet Title: ROOF **PLAN**





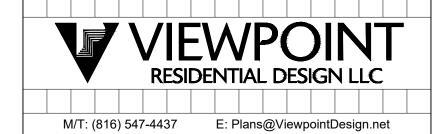


These plans and specifications are protected under federal copyright laws.

Copyright A.D. 2022 Viewpoint Residential Design, LLC.

Care and effort have gone into the creation and design of this plan. However, the designer is not an architect or engineer and construction from these plans should not be undertaken without the assistance of a construction professional, architect or engineer. Because of the impossibility of any on site consultation and supervision, Viewpoint Residential Design, LLC, and Designer assume no responsibility for any damages, including structural failures, due to any deficiencies, omissions or error in the design or blueprints. Also, site conditions may vary from those illustrated on this plan. Designer does not warrant the suitability of these plans for use on your specific site. Consult your architect to determine the suitability of these plans for your specific site and application.

"For God so loved the world, that he gave his only begotten Son, that whosoever believeth in him should not perish, but have everlasting life" (John 3:16).



Site Description:

Lot 15,

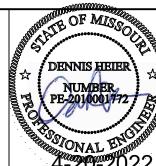
The Townhomes of Chapel Ridge
- 2nd Plat

Street Address:

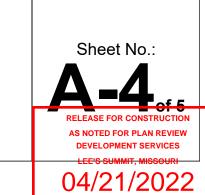
4027, 4025, 4023, & 4021 NE Sagamore Dr.,
Lee's Summit, Missouri
General Contractor:

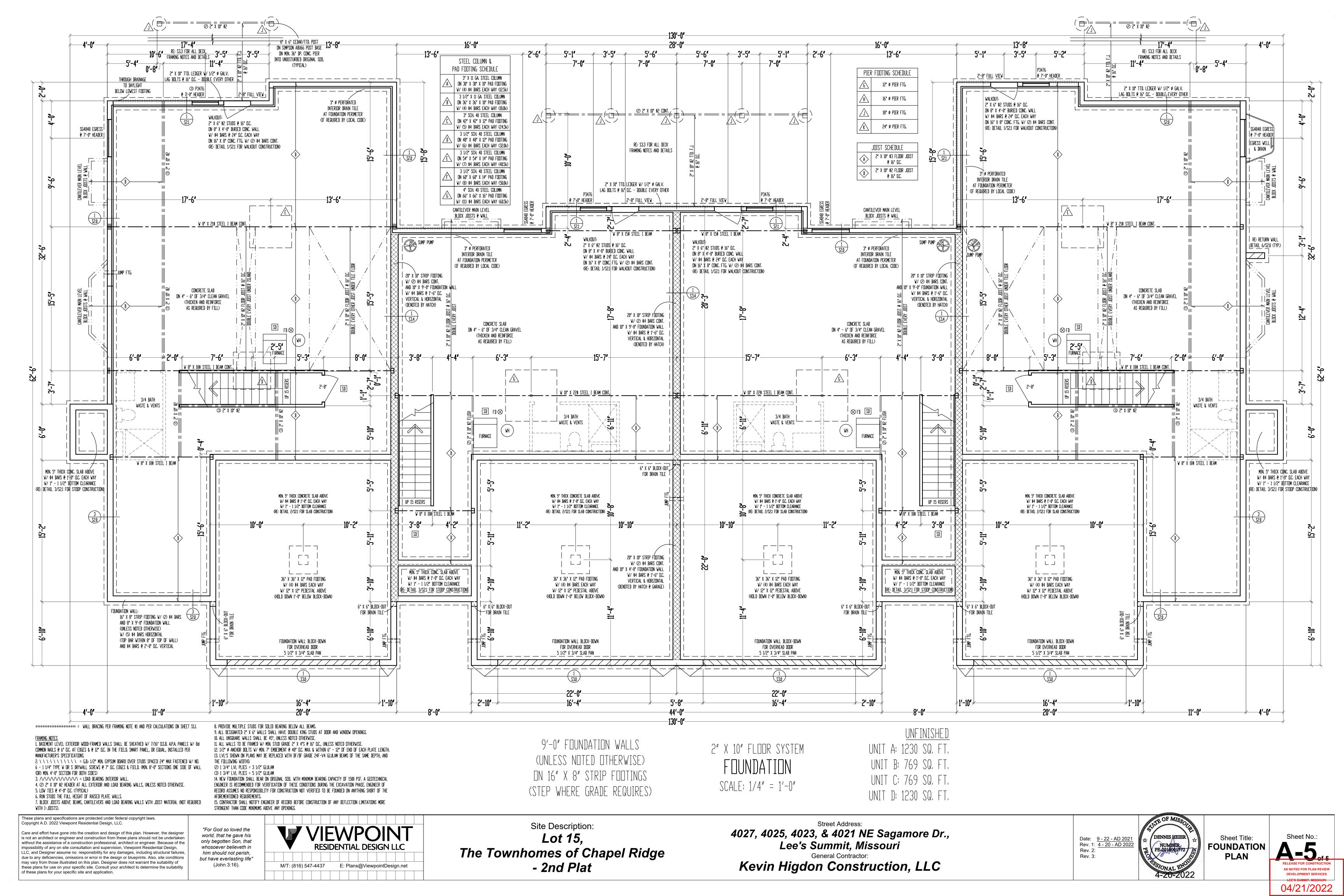
Kevin Higdon Construction, LLC

Date: 9 - 22 - AD 2021 Rev. 1: 4 - 20 - AD 2022 Rev. 2: Rev. 3:



Sheet Title: SECOND LEVEL PLAN





		FASTENER SCHEDULE FO	OR STRUCTURAL MEMBERS		
DESCRIPTION OF BUILDING ELEM	IENTS		PE OF FASTENER OOF 1		SPACING OF FASTENERS
BLOCKING BETWEEN JOISTS OR RAFTE	ERS TO TOP		" x 0.113")		<u>-</u>
PLATE, TOE NAIL CEILING JOISTS TO PLATE, TOE	NAIL	3-8d (2½'	" x 0.113")		
CEILING JOISTS NOT ATTACHED TO F		3-	10d		-
COLLAR TIE TO RAFTER, FACE NAIL O		3-10d (3"	" x 0.128")		_
GAGE RIDGE STRAP RAFTER OR ROOF TRUSS TO PLATE,	TOE NAII	3-16d BOX NAILS (3½" x 0	0.135") OR 3-10d COMMON	2 TOE NAI	ILS ON ONE SIDE AND 1 TOE NAIL ON
ROOF RAFTERS TO RIDGE, VALLEY,			" x 0.148")	OPPOSIT	E SIDE OF EACH RAFTER OR TRUSS
RAFTERS: TOE NAIL FACE NA		, ,	, 3-16d (3½" x 0.135")		-
BUILT-UP STUDS - FACE NAII	<u> </u>	T	ALL ¹ x 0.128")		24" O.C.
ABUTTING STUDS AT INTERSECTIN CORNERS, FACE NAIL		16d (3½"	' x 0.135")		12" O.C.
BUILT-UP HEADER, TWO PIECES WITH	½" SPACER	16d (3½)"	' x 0.135")	1	16" O.C. ALONG EACH EDGE
CONTINUED HEADER, TWO PIEC		16d (3½"	' x 0.135")	1	16" O.C. ALONG EACH EDGE
		4-8d (2½)	" x 0.113")		
CONTINUOUS HEADER TO STUD, TO		,	x 0.128")		24" O.C.
DOUBLE STUDS, FACE NAIL			·		
DOUBLE TOP PLATES, FACE NA		10d (3"	x 0.128")		24" O.C.
DOUBLE TOP PLATES, MINIMUM 24-INC OF END JOINTS, FACE NAIL IN LAPPE		8-16d (3½	<u>′</u> " x 0.135")		-
SOLE PLATE TO JOIST OR BLOCKING,	FACE NAIL	16d (3½"	' x 0.135")		16" O.C.
SOLE PLATE TO JOIST OR BLOCKING A	AT BRACED	3-16d (3½	½" x 0.135")		16" O.C.
STUD TO SOLE PLATE, TOE NA	AIL	3-8d (2½" x 0.113") O	R 2-16d (3½" x 0.135")		-
TOP OR SOLE PLATE TO STUD, EN	ID NAIL	2-16d (3½	(2" x 0.135")		-
TOP PLATES, LAPS AT CORNERS AND		2-10d (3" x 0.128")		_	
1" BRACE TO EACH STUD AND PLATE, FACE NAIL		2-8d (2½" x 0.113")		-	
			" x 0.113")		
1"x6" SHEATHING TO EACH BEARING, I		`	, 		_
1"x8" SHEATHING TO EACH BEARING, FACE NAIL		2-8d (2½" x 0.113")			-
WIDER THAN 1"x8" SHEATHING TO EAC FACE NAIL	H BEARING,	3-8d (2½'	" x 0.113")		<u>-</u>
		FLC	OOR 1		
JOIST TO SILL OR GIRDER, TOE	NAIL	3-8d (2½)	" x 0.113")		-
RIM JOIST TO TOP PLATE, TOE NAIL APPLICATIONS ALSO)	(ROOF	8d (2½" x 0.113"		6" O.C.	
RIM JOIST OR BLOCKING TO SILL PLATE	E, TOE NAIL	8d (2½" x 0.113")		6" O.C.	
1"x6" SUBFLOOR OR LESS TO EACH JO	DIST, FACE	2-8d (2½'	" x 0.113")	-	
NAIL 2" SUBFLOOR TO JOIST OR GIRDER, E	BLIND AND	2-16d (3½" x 0.135")		-	
FACE NAIL		· · ·		AT EACH DEADING	
2" PLANKS (PLANK AND BEAM - FLOOR	AND ROOF)	2-16d (3½" x 0.135")		AT EACH BEARING	
BUILT-UP GIRDERS AND BEAMS, 2-INC LAYERS	H LUMBER	10d (3" x 0.128")		NAIL EACH LAYER AS FOLLOWS: 32" O.C. AT TO AND BOTTOM AND STAGGERED. TWO NAILS A' ENDS AND AT EACH SPLICE	
LEDGER STRIP SUPPORTING JOISTS O	D DAETEDS	3-16d (3½	½" x 0.135")		AT EACH JOIST OR RAFTER
LEDGER STRIP SUPPORTING JUISTS O		FASTNER SCHEDULE FOR	R STRUCTURAL MEMBERS		
DESCRIPTION OF BUILDING MATERIALS WOOD STRUCTURAL PANELS, SUB		IPTION OF FASTENER	EDGE SPACING (INC		INTERMEDIATE SUPPORTS (INCHE RD WALL SHEATHING TO FRAMING
3 ₈ " - ½"		MON (2" x 0.113") NAIL , WALL) 8d COMMON NAIL	6		12
¹⁹ / ₃₂ " - 1"		(ROOF) MON NAIL (2½" x 0.131")	6		12
11/8" - 11/4"	10d COMMO	DN (3" x 0.148") NAIL OR 8d	6		12
178 - 174	(2½" x 0.	131") DEFORMED NAIL	L SHEATHING ¹		1Z
		'ANIZED ROOFING NAIL;			
½" GYPSUM SHEATHING	STAPLE GA	LVANIZED, 1½" LONG; 1¼" EWS, TYPE W OR S	7		7
%" GYPSUM SHEATHING	STAPLE GA	/ANIZED ROOFING NAIL; LVANIZED, 1½" LONG; 1½" EWS, TYPE W OR S	7		7
wo			ON SUBFLOOR UNDERLAYME	ENT TO FRAMI	ING ¹
¾" AND LESS		ED (2" x 0.120") NAIL OR 8d ON (2½" x 0.131") NAIL	6		12
,,,	1				
7/ ₈ " - 1"		N (2½" x 0.131") NAIL OR 8d MED (2½" x 0.120") NAIL	6		12

1. IF INFORMATION LISTED ON PLAN SHEETS CONTRADICTS INFORMATION IN THIS TABLE, INFORMATION ON PLANS TAKES PRECEDENCE OVER INFORMATION LISTED IN THIS TABLE

FOUNDATION NOTES

STANDARDS

- CONCRETE SHALL BE AIR-ENTRAINED BETWEEN 5%-7% WITH A MINIMUM 28-DAY COMPRESSIVE STRENGTH OF 2500 PSI FOR BASEMENT AND INTERIOR FLOOR SLABS-ON-GRADE, 3000 PSI FOR FOUNDATION WALLS, AND 3500 PSI FOR
- PORCHES AND GARAGE FLOOR SLABS THE FOUNDATION DESIGN SHALL COMPLY WITH THE ENFORCING JURISDICTION'S RESIDENTIAL FOUNDATION
- PROVIDE A MINIMUM 4"-DIAMETER PERFORATED DRAIN PIPE ALONG PERIMETER OF USABLE SPACE AT FOOTING LEVEL OR OTHER EQUIVALENT MATERIALS PER IRC SECTION R405.1. THE PIPE SHALL BE COVERED WITH A MINIMUM OF 6" OF GRAVEL OR CRUSHED ROCK. THE DRAIN SHALL DAYLIGHT BELOW FOOTING LEVEL OR TERMINATE IN A
- MINIMUM 20 GALLON SUMP PIT. FOUNDATION SHALL BE DESIGNED FOR A BEARING CAPACITY OF 1500 PSF AND FOUNDED ON COMPETENT ORIGINAL SOIL AS DETERMINED AND CONFIRMED BY A LICENSED GEOTECHNICAL ENGINEER OR ENGINEERING GEOLOGIST. ENGINEER OF RECORD ASSUMES NO RESPONSIBILITY FOR CONSTRUCTION NOT VERIFIED TO BE FOUNDED ON ANY
- SOIL WITH THE AFOREMENTIONED MINIMUM PROPERTIES. FOOTINGS SHALL BE A MINIMUM OF 16" WIDE x 8" DEEP AND SHALL HAVE A MINIMUM OF (2) CONTINUOUS GRADE 40 #4 BARS WITH 3" BOTTOM CLERANCE. BOTTOM OF FOOTING SHALL BE LOCATED A MINIMUM OF 3'-0" BELOW GRADE
- FOR FROST PROTECTION. CONCRETE PADS SUP0PORTING COLUMN LOADS SHALL BE NO SMALLER THAN 2'-0" x 2'-0" x 1'-0" DEEP WITH A
- MINIMUM OF (4) GRADE 40 #4 BARS EACH WAY WITH 3" BOTTOM CLEARANCE FOUNDATION WALLS SHALL BE A MINIMUM OF 8" NOMINAL WIDTH AND SHALL HAVE HOIZONTAL GRADE 40 #4 BARS
- AT 2'-0" O.C. MAX. WITH VERTICAL #4 BARS AS REQUIRED ON FOUNDATION CROSS SECTION ON SHEET S2.0
- REINFORCEMENT SHALL LAP A MINIMUM OF 2'-0" (CLASS B SPLICE) INTERIOR BEARING WALLS AND COLUMNS SHALL BE ISOLATED FROM THE BASEMENT FLOOR SLAB
- GRAVEL OR CRUSHED ROCK. BETWEEN THE BASE COURSE AND FLOOR SLAB SHALL BE PLACED A 6-MIL POLY VAPOR RETARDER WITH MINIMUM OVERLAP OF 6" AT DISCONTINUITIES
- 11. IF A FLOOR IS TO BE SUPPORTED BY A MINIMUM OF 2'-0" OF GRANULAR FILL OR 8" OF EARTH, BASEMENT SLAB SHALL BE DESIGNED BY A LICENSED ENGINEER
- SILL PLATES SHALL BE ANCHORED TO THE FOUNDATION WALL WITH $\frac{1}{2}$ " Ø ANCHOR BOLTS EMBEDDED A MINIMUM OF 7" INTO CENTER OF WALL STEM AND SHALL BE INSTALLED AT A MAXIMUM OF 6'-0" O.C. (OR AS NOTED ON PLANS) AND SHALL BE INSTALLED WITHIN 6" TO 12" OF EACH END OF EACH SILL PLATE LENGTH, PER IRC SECTION R403.1.6 FOUNDATION WINDOW WELLS SHALL BE PROVIDED WITH MINIMUM DIMENSIONS AS SHOWN IN DETAIL ON SHEET

BASEMENT FLOOR SLAB SHALL BE A MINIMUM OF 4" THICK ON A MINIMUM BASE COURSE OF 4" TO 6" OF SAND,

14. THE GARAGE FLOOR SHALL SLOPE TOWARD THE VEHICLE DOORS OR TO A TRENCH OR UNTRAPPED DRAIN THAT DISCHARGES TO THE EXTERIOR, ABOVE GRADE

FRAMING NOTES

- ALL DIMENSIONAL LUMBER SHALL BE DOUGLAS-FIR-LARCH GRADE #2, UNLESS NOTED OTHERWISE ON PLANS
- ALL INTERIOR LOAD-BEARING AND EXTERIOR WALL HEADERS SHALL BE (2) #2 2x10's, UNLESS NOTED OTHERWISE
- BLOCK OVER BEAMS AND AT CANTILEVERS AND DOOR JAMBS
- INTERIOR NON-BEARING WALLS RESTING ON BASEMENT SLAB SHALL BE ISOLATED FROM ABOVE FRAMING BY A
- ALL HEADERS/BEAMS SHALL BEAR ON A MINIMUM OF (2) 2x4 POSTS (KING AND JACK STUDS), UNLESS NOTED
- 20. WHERE JOISTS SPAN PARALLEL TO FOUNDATION, BLOCKING SHALL BE PROVIDED IN THE TWO SPACES MOST ADJACENT TO THE FOUNDATION WALL AT 4'-0" O.C. FOR THE PURPOSE OF TRANSFERRING LATERAL FOUNDATION WALL LOAD TO THE FLOOR DIAPHRAGM. FASTEN JOISTS AND BLOCKING TO SILL PLATE WITH (4) 10d NAILS. IF MECHANICAL DUCTWORK IS INSTALLED IN ONE OF THESE FIRST TWO BAYS, FASTEN 2x4's FLAT AT 4'-0" O.C. BETWEEN JOIST(S) AND/OR SILL AND PROVIDE BLOCKING AS PRESCRIBED ABOVE IN THE NEXT TWO JOIST BAYS. SECURE 2x4's TO JOIST(S)/SILL PLATE WITH (4) 10d NAILS.
- ALL WOOD MATERIAL SUPPORTED ON CONCRETE OR MASONRY SHALL BE TREATED OR OF DECAY-RESISTANT
- JOISTS UNDER BEARING PARTITIONS ON PLANS HAVE BEEN SIZED TO SUPPORT THE DESIGN LOAD.
- 23. JOISTS FRAMING INTO THE FACE OF A STEEL OR WOOD BEAM SHALL BE SUPPORTED WITH APPROPRIATE COLD-FORMED STEEL JOIST HANGERS
- JOISTS FRAMED ON TOP OF STRUCTURAL MEMBER SHALL BE SUPPORTED AT EN DS BY FULL-DEPTH SOLID BLOCKING MIN. 11/8" IN THICKNESS OR BY FASTENING RIM TO JOISTS PER FASTENING TABLE TO LEFT
- ALL WALL COVERINGS SHALL COMPLY WITH IRC SECTION R702.3
- ALL RAFTERS AND COLLAR TIES SHALL COMPLY WITH IRC SECTION R802.3.
- ALL RAFTERS SHALL HAVE 2x4 COLLAR TIES @ 4'-0" O.C. IN UPPER $\frac{1}{3}$ OF VERTICAL DISTANCE BETWEEN CEILING AND
- BLOCKING BETWEEN JOISTS UNDER A LOAD-BEARING WALL IS NOT REQUIRED
- PER IRC SECTION 501.3, BOTTOM OF ALL FLOOR ASSEMBLIES ABOVE UNFINISHED AREAS SHALL BE PROVIDED WITH A 1/8" GYPSUM BOARD MEMBRANE OR RESIDENTIAL FIRE SPRINKLER SYSTEM WHEN FLOOR SYSTEM IS CONSTRUCTED OF OTHER THAN DIMENSION LUMBER OR STRUCTURAL COMPOSITE LUMBER EQUAL TO OR
- GREATER THAN 2x10 NOMINAL DIMENSION(WHERE REQUIRED BY ENFORCING JURISDICTION) 30. ENGINEERED LVL's SHALL HAVE MINIMUM PROPERTIES OF Fb = 2600 psi, E=1900 ksi, AND Fv=285 psi
- ENGINEERED PARALLAMS SHALL HAVE MINIMUM PROPERTIES OF Fb = 2600 psi, E = 2000 ksi, AND Fv = 290 psi COLUMN CONNECTION TO STEEL BEAMS SHALL BE WITH A CLIP POST CAP WITH ALL FOUR TAB EARS BENT AROUND THE BOTTOM FLANGE OF THE BEAM. FOR A BEARING PLATE, FOUR HOLES SHALL BE DRILLED IN THE BOTTOM FLANGE OF THE STEEL BEAM TO MATCH THE HOLE PATTERN OF THE PLATE. 1/2" x 2" BOLTS SHALL THEN BE INSTALLED WITH A FLAT WASHER, LOCK WASHER, AND A NUT IN EACH OF THE HOLES. THE POST CAP MAY BE WELDED TO THE STEEL BEAM IN ACCORDANCE WITH AWS D1.1-92 AS AN ALTERNATIVE, AND WOULD NEED TO BE
- INSPECTED BY AN AWS-CERTIFIED INSPECTOR. 33. WHEN MECHANICAL EQUIPMENT IS LOCATED IN AN ENCLOSED ROOM, THERE SHALL BE (2) 14"x12" VENTS LOCATED IN A WALL COMMON WITH ADDITIONAL LIVING AREA. ONE VENT SHALL BE LOCATED SUCH THAT THE BOTTOM OF THE VENT BEGINS 12" FROM THE FLOOR AND THE OTHER VENT SHALL BE LOCATED SUCH THAT THE TOP OF THE
- VENT BEGINS 12" FROM THE CEILING. 34. ALL ROOF SHEATHING SHALL BE $\frac{7}{16}$ " OSB WITH 8d COMMON NAILS @ 6" O.C. AT PANEL EDGES AND @ 12" O.C. IN FIELD

GLAZING NOTES

- 35. GLAZING IN HAZARDOUS LOCATIONS AS IDENTIFIED IN IRC SECTION R308.4 SHALL BE OF APPROVED SAFETY GLAZING MATERIALS. GLASS IN STORM DOORS, INDIVIDUAL FIXED OR OPENABLE PANELS ADJACENT TO A DOOR WHERE THE NEAREST VERTICAL EDGE IS WITHIN A 2'-0" ARC OF THE DOOR IN A CLOSED POSITION AND FOR WHICH THE BOTTOM EDGE IS WITHIN 5'-0" OF THE FLOOR, WALLS ENCLOSING STAIRWAYS AND LANDINGS WHERE THE GLAZING IS WITHIN 5'-0" OF THE TOP OR BOTTOM OF THE STAIR, ENCLOSURES FOR SPAS, TUBS, SHOWERS, AND WHIRLPOOLS, GLAZING IN FIXED OR OPENABLE PANELS EXCEEDING NINE SQUARE FEET AND FOR WHICH THE
- BOTTOM EDGE IS LESS THAN 1'-6" ABOVE THE FLOOR OR WALKING SURFACE WITHIN 3'-0" 36. ALL OPERABLE WINDOWS SHALL HAVE FALL PROTECTION PER IRC SECTION R612.2

ATTIC VENTILATION

ENCLOSED ATTICS SHALL HAVE CROSS VENTILATION FOR EACH SEPARATE SPACE BY VENTILATING OPENINGS PROTECTED AGAINST THE ENTRANCE OF RAIN OR SNOW. VENTILATING OPENINGS SHALL BE PROVIDED WITH CORROSION-RESISTANT WIRE MESH, WITH $\frac{1}{2}$ " TO $\frac{1}{2}$ " OPENINGS. THE TOTAL FREE VENTILATING AREA SHALL NOT BE LESS THAN $\%_{50}$ OF THE AREA OF SPACE VENTILATED, EXCEPT WHERE THE VENTILATORS ARE LOCATED IN THE UPPER PORTION OF THE SPACE TO BE VENTILATED - THE REQUIRED AREA MAY BE REDUCED TO 1/300.

EMERGENCY EGRESS

- PROVIDE A MINIMUM OF ONE WINDOW FOR EACH BEDROOM THAT HAS A MINIMUM OPENABLE AREA OF 5.7 SQUARE FEET WITH A MINIMUM OPENABLE HEIGHT OF 2'-0" AND A MINIMUM WIDTH OF 1'-9". IN ADDITION, THE OPENABLE PORTION OF EGRESS WINDOWS SHALL NOT EXCEED 3'-8" ABOVE THE ADJOINING FLOOR OR PERMANENT STEP.
- PROVIDE SMOKE ALARMS IN EACH SLEEPING ROOM, OUTSIDE OF EACH SLEEPING AREA AND ON EACH FLOOR, INCLUDING BASEMENT (IF APPLICABLE). ALARMS SHALL BE HARDWIRED TOGETHER SO THAT THE ACTIVATION OF ONE SMOKE ALARM WILL ACTIVATE ALL SMOKE ALARMS IN THE DWELLING. PROVIDE CARBON MONOXIDE DETECTORS OUTSIDE EACH SLEEPING AREA.

- 40. MASONRY VENEER SHALL BE ANCHORED TO THE SUPPORTING WALL STUDS WITH CORROSION-RESISTANT METAL TIES EMBEDDED IN MORTAR OR GROUT AND EXTENDING INTO THE VENEER A MINIMUM OF 1½", WITH NOT LESS THAN 5/8" MORTAR OR GROUT COVER TO OUTSIDE FACE.
- 41. VENEER TIES, IF STRAND WIRE, SHALL NOT BE LESS IN THICKNESS THAN NO. 9 U.S. GAGE WIRE AND SHALL HAVE A HOOK EMBEDDED IN THE MORTAR JOINT, OR IF SHEET METAL, SHALL BE NOT LESS THAN NO. 22 U.S. GAGE BY $\frac{7}{3}$ " 42. EACH TIE SHALL SUPPORT NOT MORE THAN 2.67 SQUARE FEET OF WALL AREA AND SHALL BE SPACED NOT MORE

SHALL BE SPACED NOT MORE THAN 3 FEET ON CENTER AND PLACED WITHIN 12 INCHES OF THE WALL OPENING.

THAN 32 INCHES ON CENTER HORIZONTALLY AND 24 INCHES ON CENTER VERTICALLY. 43. VENEER TIES AROUND WALL OPENINGS: ADDITIONAL METAL TIES SHALL BE PROVIDED AROUND ALL WALL OPENINGS GREATER THAN 16 INCHES IN EITHER DIMENSION. METAL TIES AROUND THE PERIMETER OF OPENINGS

- DOOR(S) BETWEEN THE GARAGE AND DWELLING SHALL BE MINIMUM 1%" SOLID CORE OR HONEY-COMBED STEEL DOOR WITH 20-MINUTE FIRE RATING EQUIPPED WITH A SELF-CLOSING DEVICE
- 45. VEHICLE DOORS AND FRAMES SHALL BE DESIGNED AND INSTALLED TO MEET THE 90-MPH 3-SECOND GUST LOADING PER DASMA 108 AND ASTM E 330-96 PER IRC SECTION R301.2.1

GARAGE NOTES (CONTINUED)

- THE GARAGE SHALL BE SEPARATED FROM THE DWELLING AND ITS ATTIC AREAS BY MINIMUM %" GYP. BOARD APPLIED TO THE GARAGE SIDE OF FRAMING. WHERE HABITABLE SPACE OCCURS ABOVE THE GARAGE, THE GARAGE CEILING ASSEMBLY SHALL BE PROTECTED WITH A MINIMUM 5/4" TYPE X GYP. BOARD. WHERE A FLOOR/CEILING SPACE IS PROVIDED ABOVE THE GARAGE COLUMNS AND BEAMS SUPPORTING THE SEPARATION SHALL ALSO BE PROTECTED WITH 5/8" GYP. BOARD. GARAGE DOOR H-FRAME FOR THE ATTACHMENT OF THE TRACK AND COUNTER
- BALANCE SHALL CONSIST OF THE FOLLOWING: 2x6 VERTICAL JAMBS RUNNING FROM FLOOR TO CEILING AND SHALL BE FASTENED WITH $2\frac{1}{2}$ "" x 0.120" NAILS AT 7" O.C. STAGGERED WITH (7) 31/4" x 0.120" NAILS THROUGH THE JAMB INTO THE HEADER. MINIMUM 2x8 HEADER FOR ATTACHMENT OF COUNTER BALANCE SYSTEM.

DESIGN LOADING (PER TABLE R301.5)

MINIMUM UNIFORMLY DISTRIB USE	LIVE LOAD	DEAD LOAD
UNINHABITABLE ATTICS WITHOUT STORAGE	10	10
UNINHABITABLE ATTICS WITH LIMITED STORAGE	20	10
HABITABLE ATTICS AND ATTICS SERVED WITH FIXED STAIRS	30	10
BALCONIES (EXTERIOR) AND DECKS	40	10 ^d
FIRE ESCAPES	40	10
GUARDRAILS AND HANDRAILS ^a	200°	-
GUARDRAIL IN-FILL COMPONENTS ^b	50 ^c	-
PASSENGER VEHICLE GARAGES	50	DEPENDENT UPON SLAB CONSTRUCTION
ROOMS OTHER THAN SLEEPING ROOM	40	10 ^d
SLEEPING ROOM	30	10 ^d
STAIRS	40	10 ^d

a. A single concentrated load applied in any direction at any point along the top.

b. Guard in-fill components (all those except the handrail), ballusters and panel fillers shall be designed to withstand a horizontally applied normal load of 50 pounds on an area equal to one square foot. This load need not be assumed to act concurrently with any other live load requirement. c. Glazing used in handrail assemblies and guards shall be designed with a safety factor of 4. The safety factor shall be applied to each of the concentrated loads applied to the top of the rail, and to the load on the infill components. These loads shall be determined independently of one another, and loads are assumed

not to occur with any other live load. d. An additional dead loading of 10 psf shall be applied where thinset tile floor is to be installed. An additional dead loading of 50 psf shall be applied where mudset tile floor is to be installed.

INSULATION/EFFICIENCY

- BUILDING ENVELOPE INSULATION SHALL COMPLY WITH IRC TABLE N1102.1.1 OR THE 2012 IECC (SEE SHEET S3.1 FOR FRAMING DETAILS AND TABLES ON THIS SHEET FOR MORE INFORMATION)
- CATHEDRAL -VAULTED CEILING FRAMING SHALL BE FRAMED WITH A MINIMUM INSULATION VALUE OF R-38. IF VAULTED RAFTERS DO NOT PROVIDE REQUIRED DEPTH TO ACHIEVE R-38 INSULATION BUILDER SHALL FUR DOWN RAFTERS PER DETAILS PROVIDED ON SHEET S3.1.

INSULATION AND FENESTRATION REQUIRE	1
CLIMATE ZONE	4-A
FENESTRATION U-FACTOR	0.35
SKYLIGHT U-FACTOR	0.55
GLAZED FENSTRATION SHGC	0.40
CEILING R-VALUE	49
WOOD FRAME WALL R-VALUE	13
MASS WALL R-VALUE	8 / 13
FLOOR R-VALUE	19
BASEMENT WALL R-VALUE	10-CONTINUOUS OR 13-CAVITY
SLAB R-VALUE AND DEPTH	10 AT 2'-0"
CRAWL SPACE WALL R-VALUE	10-CONTINUOUS OR 13-CAVITY
DUCTWORK EXPOSED TO OUTSIDE AIR R-VALUE	8
DUCTWORK NOT EXPOSED TO OUTSIDE AIR R-VALUE	6
CATHEDRAL VAULTED CEILING R-VALUE	38

DUCT SEALING

N1103.2.2 (R403.2.2) SEALING (MANDATORY). DUCTS, AIR HANDLERS, AND FILTER BOXES SHALL BE SEALED. JOINTS AND SEAMS SHALL COMPLY WITH SECTION M1601.4.1 OF 2012 IRC.

- 1. AIR-IMPERMEABLE SPRAY FOAM PRODUCTS SHALL BE PERMITTED TO BE APPLIED
- WITHOUT ADDITIONAL JOINT SEALS. WHERE A DUCT CONNECTION IS MADE THAT IS PARTIALLY INACCESSIBLE, THREE SCREWS OR RIVETS SHALL BE EQUALLY SPACED ON THE EXPOSED PORTION OF THE JOINT SO AS TO PREVENT A HINGE EFFECT.
- CONTINUOUSLY WELDED AND LOCKING-TYPE LONGITUDINAL JOINTS AND SEAMS IN DUCTS OPERATING AT STATIC PRESSURES LESS THAN 2 INCHES OF WATER COLUMN PRESSURE CLASSIFICATION SHALL NOT REQUIRE ADDITIONAL CLOSURE SYSTEMS.
- DUCT TIGHTNESS SHALL BE VERIFIED BY EITHER OF THE FOLLOWING:
- POST-CONSTRUCTION TEST: TOTAL LEAKAGE SHALL BE LESS THAN OR EQUAL TO 4 CFM PER 100 SQUARE FEET OF CONDITIONED FLOOR AREA WHEN TESTED AT A PRESSURE DIFFERENTIAL OF 0.1 INCHES W.G. ACROSS THE ENTIRE SYSTEM, INCLUDING THE MANUFACTURER'S AIR HANDLER ENCLOSURE. ALL REGISTER BOOTS SHALL BE TAPED OR OTHERWISE SEALED DURING THE TEST.
- ROUGH-IN TEST: TOTAL LEAKAGE SHALL BE LESS THAN OR EQUAL TO 4 CFM PER 100 SQUARE FEET OF CONDITIONED FLOOR AREA WHEN TESTED AT A PRESSURE DIFFERENTIAL OF 0.1 INCHES W.G. ACROSS THE SYSTEM, INCLUDING THE MANUFACTURER'S AIR HANDLER ENCLOSURE. ALL REGISTERS SHALL BE TAPED OR OTHERWISE SEALED DURING THE TEST. IF THE AIR HANDLER IS NOT INSTALLED AT THE TIME OF THE TEST, TOTAL LEAKAGE SHALL BE LESS THAN OR EQUAL TO 3 CFM PER 100 SQUARE FEET OF CONDITIONED FLOOR AREA.

EXCEPTION: THE TOTAL LEAKAGE TEST IS NOT REQUIRED FOR DUCTS AND AIR HANDLERS LOCATED ENTIRELY WITHIN THE BUILDING THERMAL ENVELOPE.

MF	CHANICAL VENTILATIO	N SYSTEM FAN EFFICA	CY
FAN LOCATION	AIR FLOW RATE MINIMUM (CFM)	MINIMUM EFFICACY (CFM/WATT)	AIR FLOW RATE MAXIMUM (CFM)
RANGE HOODS	ANY	2.8	ANY
IN-LINE FAN	ANY	2.8	ANY
BATHROOM, UTILITY ROOM	10	1.4	90
BATHROOM, UTILITY ROOM	90	2.8	ANY





/					
		· · · · · · · · · · · · · · · · · · ·			
	NO.	DATE	REVI	SION	B
	DRAV	WING TITLE	•		•
		TO	10		ΛΙ
	5	ΙΚι		TUR	AL
		N	OI	ES	
			•		
	=	NICED. DI	411	NIEGKED DV.	- N 41

GINEER: DIVIH | CHECKED BY:DIVIE JOB NO. 3905 DRAWN BY: DMH DATE: **04-20-22**

RESIDENTIAL SEISMIC & WIND ANALYSIS

				INPUT
DETERMINE WEIGHT OF HOUSE:				CALCULATED VALUE
LOCATION		DEAD LOAD (psf)	AREA (ft ²)	WEIGHT (lbs.)
ROOF		10	6349	63490
CEILING		10	6349	63490
FIRST FLOOR		10	6349	63490
	WALL LENGTH (ft)	WALL HEIGHT (ft)	WALL UNIT WT. (psf)	WEIGHT (lbs)
FIRST FLOOR EXT. WALL DL	354.66	10	10	35466
		DEAD LOAD (psf)	AREA (ft2)	WEIGHT (lbs)
FIRST FLOOR INT. PARTITION WALL DL		6	6349	38094

	PROJECTED AREAS (WIND DESIGN PER 115 MPH 3-SECOND GUST, EXPOSURE C AND MEAN ROOF HEIGHT <= 30 FT ASSUMED)								
	FRONT-TO-BACK			SIDE-TO-SIDE					
	AREA	LOAD			AREA	LOAD			
SLOPED ROOF	555	4515		SLOPED ROOF	708	6024			
VERT. ROOF	853	10129	CUMULATIVE	VERT. ROOF	30	373	CUMULATIVE		
1ST	1287	15282	30007	1ST	663.63	8250	14728		
BSMT ^a	0	0	0	BSMT ^a	140	1982	9346		
) - PER ASCE CH. 6					
	SLOPED ROOF	ZONE B		9.7	ZONE C	11.3	2a (FIG. 28.6-1, ASCE7)		
	WALL/VERT. ROOF	ZONE A		14.2	ZONE D	7.7	12.066		
	MEAN ROOF HT., h		24						

a) If there is a walkout wall to be sheathed, determine tributary wind area and enter here. If no walkout, enter 0 for a q_{z10} =0.00256 $K_zK_zK_dV^2$ (ASCE7-10 Velocity Pressure) q_{z10} =0.6 q_{z10} =0.6 q_{z10} (Design Velocity Pressure for

q_{z10_ASD}=0.6q_{z10} (Design Velocity Pressure for ASD analysis under ASCE7-10 and IRC/IBC 2012)

1ST FLOOR TRIBUTARY WEIGHT BASEMENT TRIBUTARY WEIGHT S_S (SITE GROUND MOTION - %g - FROM ASCE7 SEISMIC MAP) F_a (from ASCE7 Table 11.4-1) S_{DS} (= 2/3 * S_S * F_a)

R (from ASCE7 Table 12.2-1)

144713 12.0% 1.6 0.128 6.5

144713

	SEISMIC SHEAR		
LOCATION		From ASCE7 (Eq. 12.8-1):	V (= 1.2 * S _{DS} * W / R) (lbs.)
1ST FLOOR			3420
BASEMENT			3420

Sheathing Location	Min. Sheathing Schedule	Fastening Schedule	Allowable Shear (#/LF)	Code Reference
Exterior (Option #1)	7/16" APA Rated Plywood/OSB	1-1/2" 16ga. Staples w/ 1" penetration@ 6" OC Edges, 6" OC Field For 24" stud spacing, 12" OC Field For 16" stud spacing	155	per IBC, Table 2306.3(1)
Exterior (Option #2)	7/16" APA Rated Plywood/OSB	1-1/2" 16ga. Staples w/ 1" penetration@ 4" OC Edges, 6" OC Field For 24" stud spacing, 12" OC Field For 16" stud spacing	230	per IBC, Table 2306.3(1)
Exterior (Option #3)	7/16" APA Rated Plywood/OSB	1-1/2" 16ga. Staples w/ 1" penetration@ 3" OC Edges, 6" OC Field For 24" stud spacing, 12" OC Field For 16" stud spacing	310	per IBC, Table 2306.3(1)
Exterior (Option #4)	7/16" APA Rated Plywood/OSB or shiplap panel sheathing, or 3/8" shiplap panel sheathing with tighter nail spacing	8d Common Nails w/ 1-3/8" penetration @ 6" O.C. Edges, 12" O.C. Field for 7/16" APA-rated plywood/OSB or shiplap panel sheathing OR @ 4" O.C. Edges, 12" O.C. Field for 3/8" shiplap panel sheathing	220	AF&PA SDPWS Table 4.3A
Exterior <u>(Option #5)</u>	7/16" APA Rated Plywood/OSB or shiplap panel sheathing, or 3/8" shiplap panel sheathing with tighter nail spacing	8d Common Nails w/ 1-3/8" penetration @ 4" O.C. Edges, 12" O.C. Field for 7/16" APA-rated plywood/OSB or shiplap panel sheathing OR @ 3" O.C. Edges, 12" O.C. Field for 3/8" shiplap panel sheathing	320	AF&PA SDPWS Table 4.3A
Exterior <u>(Option #6)</u>	7/16" APA Rated Plywood/OSB or shiplap panel sheathing, or 3/8" shiplap panel sheathing with tighter nail spacing and double studs at each panel edge	8d Common Nails w/ 1-3/8" penetration @ 3" O.C. Edges, 12" O.C. Field	410	AF&PA SDPWS Table 4.3A
Interior	1/2" Gypsum Board	No. 6- 1 ¹ / ₄ " Type W or S Screws @ 8" O.C. Edges, 12" O.C. Field	60	per IBC, Table 2306.4.4
Interior	16 Ga. Simpson/USP Type WB Steel X-Brace (or equal)	(3) 16d @ end studs & (1) 8d @ intermediate studs (per manufacture specifications - see detail on sheet S3)	325	

EXTERIOR SHEATHING OPTION FOR FIRST FLOOR	4
EXTERIOR SHEATHING OPTION FOR BASEMENT WALLS	4

WIDTH OF 1ST STORY (FT.)	117
DEPTH OF 1ST STORY (FT.)	60.33
BACK WALL OF GARAGE (FT.)	0
GAR WALL 1=F-B 2=S-S	2

WIDTH OF 2ND STORY (FT.) 1
DEPTH OF 2ND STORY (FT.) 1

	EXTERIOR STRUCTURAL WALL LENGTHS (ft.) & RESISTANCES										
		SE	ISMIC		WIND						
	FRONT-TO-BACK	RESISTANCE (lbs.)	SIDE-TO-SIDE	RESISTANCE (lbs.)	FRONT-TO-BACK	RESISTANCE (lbs.)	SIDE-TO-SIDE	RESISTANCE (lbs.)			
1ST FLOOR	114	31920	49.5	13860	114	44688	49.5	19404			
BASEMENT	0	0	75	21000		0	75	29400			
-				_							
		ADDITIONAL RESIS	TANCE REQUIRED		Anchor Bolt Spacing	(in.)	16d Nail Spacing req'd at	bottom plate (in			
		SEISMIC	WIND		diameter (in.	0.5	1st Floor F-B	11			
1ST FLOOR FRONT-TO-BACK		0	0	1	Shear value (per NDS)	944	1st Floor S-S	43			
1ST FLOOR SIDE-TO-SIDE		0	0	1	Spacing F-B (inches)	72.9					
BASEMENT FRONT-T	O-BACK	0	0	1	spacing S-S (inches)	288.0					

BASEMENT SIDE-TO-SIDE	0	0		g = (manag,			
		RESISTANCE REQUIR	RED IN ADDITION TO RES	ISTANCE PROVIDED BY EXTERIOR W	ALLS**		
	ADDITIONAL RESISTANCE REQUIRED (POUNDS)	PORTAL FRAMES OR PERF. SHEAR WALL RESISTANCE	INTERIOR X-BRACES (325#/BRACE)	INTERIOR WALL LENGTH W/ 1/2" GYPSUM BOARD PER TABLE (FT.)	INT. WALL LENGTH SHEATHED W/ OSB (TOTAL LENGTH, ONE SIDE, FT.)	RESISTANCE PROVIDED BY ADDITIONAL METHODS (POUNDS)	OK?
1ST FLOOR FRONT-TO-BACK	0					0	YES
1ST FLOOR SIDE-TO-SIDE	0					0	YES
BASEMENT FRONT-TO-BACK	0					0	YES
BASEMENT SIDE-TO-SIDE	0					0	YES

**NOTES: 1) SEE ATTACHED CALCULATIONS FOR PORTAL FRAME OR PERFORATED SHEAR WALL RESISTANCE CAPACITIES (IF APPLICABLE),
2) SEE SHEET S1 FOR INTERIOR STEEL X-BRACE INSTALLATION, 3) INTERIOR WALLS SHEATHED WITH OSB SHALL BE ATTACHED WITH SAME STAPLE/NAILING

PATTERN AS EXTERIOR OSB ON SAME FLOOR (SEE TABLE ABOVE) AND ARE ONLY APPLICABLE FOR FULL-HEIGHT SECTIONS OF 2'-8" OR LONGER

ALL LATERAL BRACI	ALL LATERAL BRACING ACHIEVED AT EXTERIOR WALLS AND WALLS DIRECTLY ON FOUNDATIONS; THEREFORE, NO INTERIOR BRACING PER 2012 IRC SECTION R502.2.1 IS REQUIRED								
	WIND UPLIFT ANALYSIS								
	X/12 DEGREES								
ROOF PITCH (MAX)	12	45.0	PITCH OF 6 OR LESS:	OF 6 OR LESS: EOH -13.3, E -7.2, G -5.2					
	ASCE 7								
	LENGTH (FT.)	PRESSURE (PSF)	LINEAL FT. OF OH	UPLIFT PER FT* (LBS)					
OVERHANG	1	-1.08	356.66	-1.08					
	TOTAL AREA (FT ²)	ZONE E AREA (FT ²)	ZONE G AREA (FT ²)	PRESSURE ZN. E (PSF)	PRESSURE ZN. G (PSF)	TOTAL FORCE (LBS)	FORCE PER LINEAL FT @ PERIMETER (LBS)		
MAIN ROOF**	7058.61	-534.089424	7592.699424	-1.08	-0.36	-2157	-6.1		
*ALONG PERIMETER	*ALONG PERIMETER TOTAL UPLIFT PER LINEAL FOOT ALONG EXTERIOR (POUNDS)				-7.2	UPLIFT OK			
**INSIDE EXTERIOR V	VALLS	RESISTANCE DUE TO DEAD	WEIGHT & (3) 10d TOENAILS		251.6				

NOTE FOR CONSTRUCTION:

THE CONTINUOUS STRUCTURAL PANEL SHEATHING BRACING METHOD REQUIRES USE OF THE ABOVE TABLE FOR SHEATHING OF THE ENTIRE STRUCTURE. IN ADDITION, FRAMING MEMBERS SHALL BE @ 16" O.C. MAX., UNBLOCKED, AND W/ SHEATHING APPLIED DIRECTLY TO FRAMING MEMBERS

NOTE FOR DESIGN:

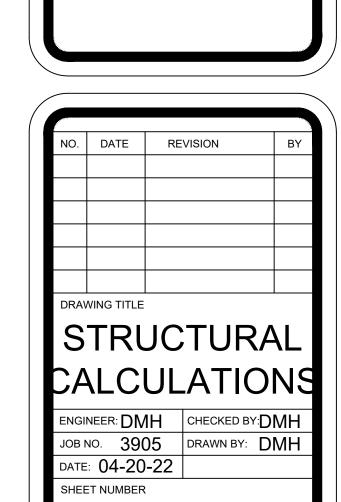
ALL WALLS USED IN THE CALCULATION OF THE RESISTANCE FOR THIS STRUCTURE SHALL HAVE A MINIMUM UNINTERRUPTED HEIGHT OF 8'-0" AND LENGTH OF 2'-8". ALLOWABLE RESISTANCES HAVE BEEN #/FT AND INCREASED BY 40% FOR WIND LOADS, PER VALUES IN 2012 IBC SECTION 2306 AND AF&PA SDPWS TABLE 4.3A. FOR EXAMPLE, 7/16" APA-RATED SHEATHING WITH 8d @ 6" & 12" HAS A SEISMIC SHEAR VALUE OF 240 A WIND SHEAR VALUE OF 335#/FT - 40% GREATER THAN THAT OF SEISMIC)

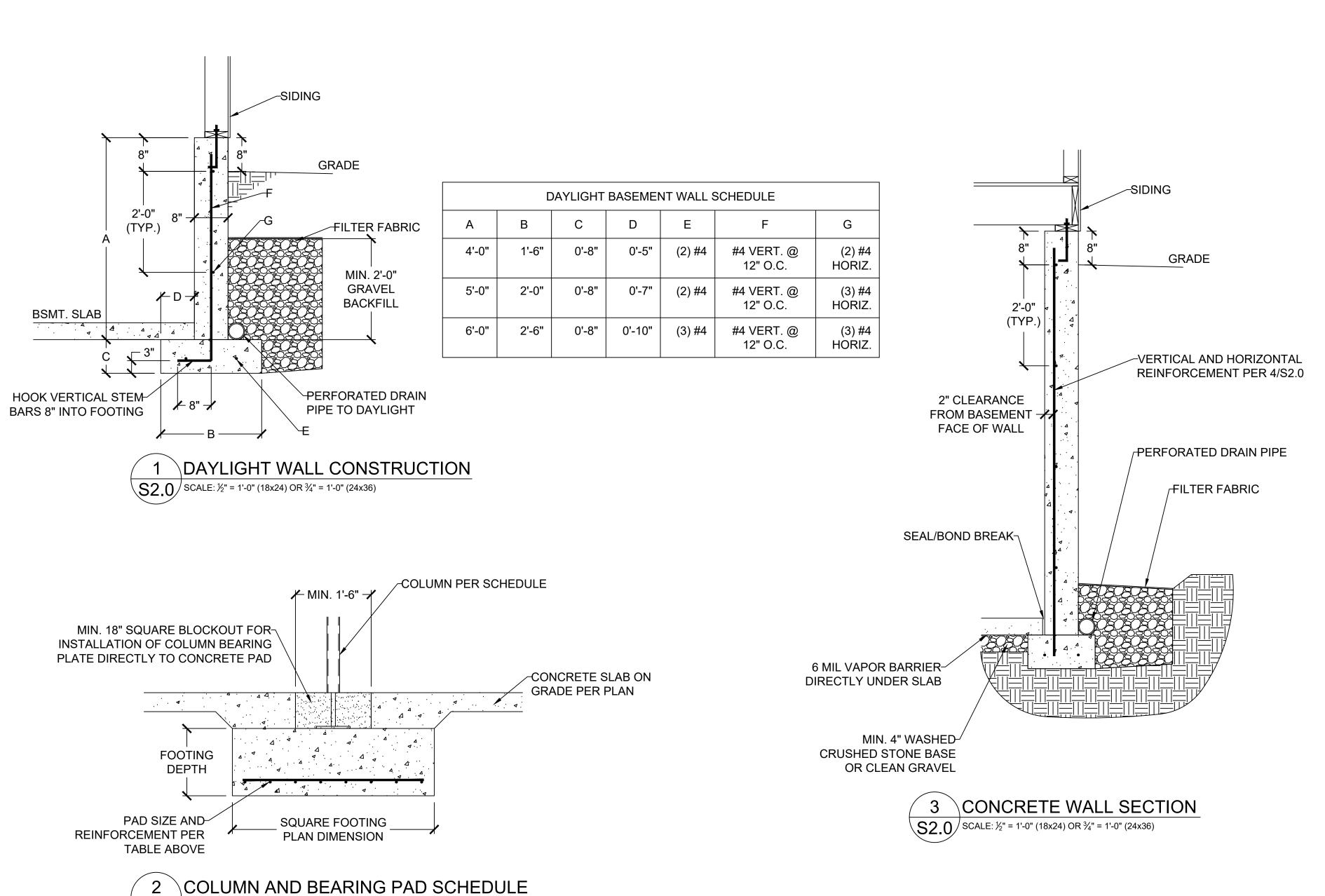
NOTE: SOIL SITE CLASS ASSUMED TO BE CLASS D. IF SITE CONDITIONS ARE DETERMINED TO BE CLASS E OR F, CONSULT ENGINEER BEFORE PROCEEDING WITH CONSTRUCTION



CLIENT: KEVIN HIGDON CONSTRUCTION, LLC

LOT 15, THE TC LOCATION: 4027, 4025, 402





TYPICAL CORNER REINFORCEMENT!

AT LEAST (1) #4 BAR 48" LONG @

AS CLOSE AS PRACTICAL TO THE CORNER

EACH INSIDE CORNER

NOTE: WHERE OPENINGS OR ABRUPT ELEVATION

CHANGES OCCUR IN THE TOP OR BOTTOM OF THE WALL

6 REINFORCEMENT AT OPENING CORNERS

S2.0/AND STEP CORNERS @ INSIDE CORNERS

SCALE: ½" = 1'-0" (18x24) OR ¾" = 1'-0" (24x36)

AT LEAST ONE #4 BAR 48" LONG SHALL BE DIAGONALLY

SCALE: $\frac{1}{2}$ " = 1'-0" (18x24) OR $\frac{3}{4}$ " = 1'-0" (24x36)

/MIN. (2) #4 BARS EXTENDING 24"

PAST OVER-EXCAVATION AND

INTO INTERSECTING WALL

-CONTINUOUS FOOTING

AND REBAR THROUGH

6'-0" MAX.

SOLID JUMP

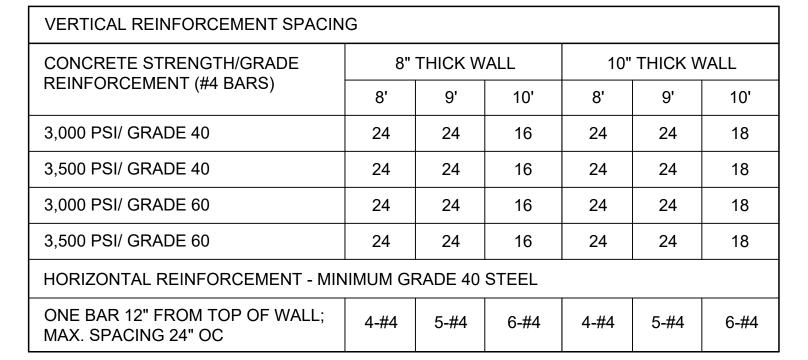
MAX. 12" BLOCKOUT FOR

FORM PLACEMENT AND

TO EXTEND DRAIN TILE

5 SOLID JUMP

S2.0 SCALE: $\frac{1}{2}$ " = 1'-0" (18x24) OR $\frac{3}{4}$ " = 1'-0" (24x36)



FOOTNOTES:

1) WALL HEIGHT IS MEASURED FROM THE TOP OF THE WALL TO THE TOP OF THE FLOOR SLAB 2) VERTICAL REINFORCEMENT FOR CONCRETE WALLS THAT ARE NOT FULL HEIGHT, AND FOR REINFORCEMENT SPACING 24" OC, REINFORCEMENT MAY BE PLACED IN THE MIDDLE OF THE WALL. OTHER WALLS SHALL HAVE VERTICAL REINFORCEMENT AS FOLLOWS:

A) 8" WALL - MINIMUM 5" FROM THE OUTSIDE FACE

B) 10" WALL - MINIMUM 63/4" FROM THE OUTSIDE FACE C) EXTEND BARS TO WITHIN 8" OF THE TOP OF THE WALL

3) REINFORCEMENT CLEARANCES:

A) CONCRETE EXPOSED TO EARTH - MINIMUM 11/2"

B) NOT EXPOSED TO WEATHER (INTERIOR SIDE OF WALLS) -3/4"

C) CONCRETE EXPOSED TO WEATHER (TOP CLEARANCE IN GARAGE AND DRIVEWAY

SLABS) - 1½"

4) HORIZONTAL REINFORCEMENT: A) ONE BAR SHALL BE PLACED WITHIN 12" OF THE TOP OF THE WALL

B) OTHER BARS SHALL BE EQUALLY SPACED WITH SPACING NOT TO EXCEED 24" OC

C) HORIZONTAL BARS SHOULD BE AS CLOSE TO THE TENSION FACE AS POSSIBLE (INTERIOR) AND BEHIND THE VERTICAL REINFORCEMENT (I.E. 2" TOWARD THE

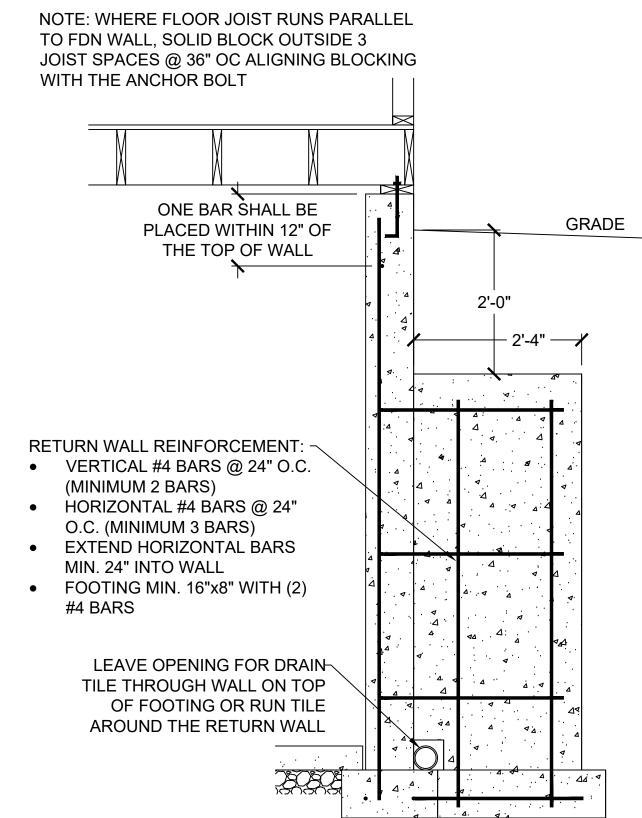
D) SUPPLEMENTAL REINFORCEMENT AT CORNERS - PLACE (1) #4 BAR 48" LONG AT 45 DEGREE ANGLE AT CORNERS OF OPENINGS. PLACE REINFORCEMENT WITHIN 6" OF THE EDGE OF INSIDE CORNERS.

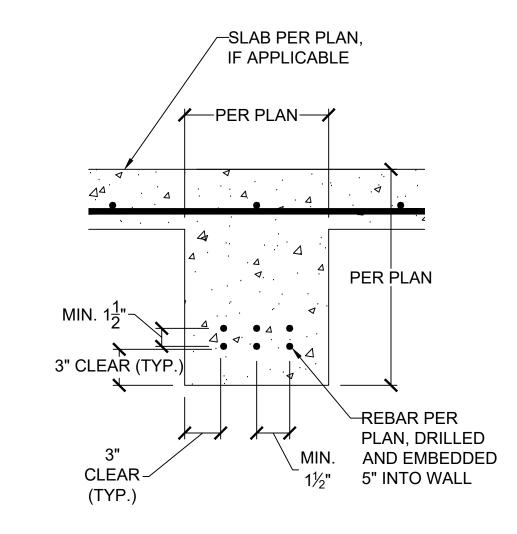
5) REINFORCEMENT SHALL BE LAPPED A MINIMUM 24" AT ENDS, SPLICES, AND AROUND CORNERS.

6) AT MASONRY LEDGES THE MINIMUM WALL THICKNESS SHALL BE 3½". LEDGES SHALL NOT EXCEED A DEPTH OF MORE THAN 24" BELOW THE TOP OF THE WALL. FOR WALL THICKNESSES LESS THAN 4" PROVIDE #4 BARS AT MAX. 24" OC TO WITHIN 8" OF THE TOP OF THE WALL

7) STRAIGHT WALLS MORE THAN 5' TALL AND MORE THAN 16 FEET LONG SHALL BE PROVIDED WITH EXTERIOR BRACED RETURN WALLS. WALL LENGTH SHALL BE MEASURED USING INSIDE THE SHORTEST DIMENSION BETWEEN INTERSECTING WALLS

4 \FOUNDATION WALL REINFORCEMENT TABLE S2.0 NO SCALE

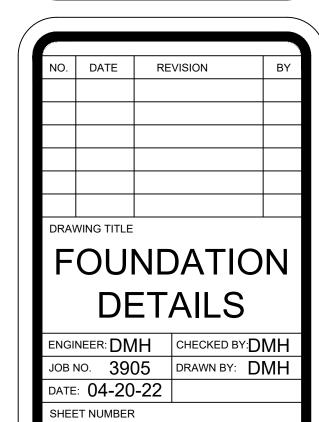


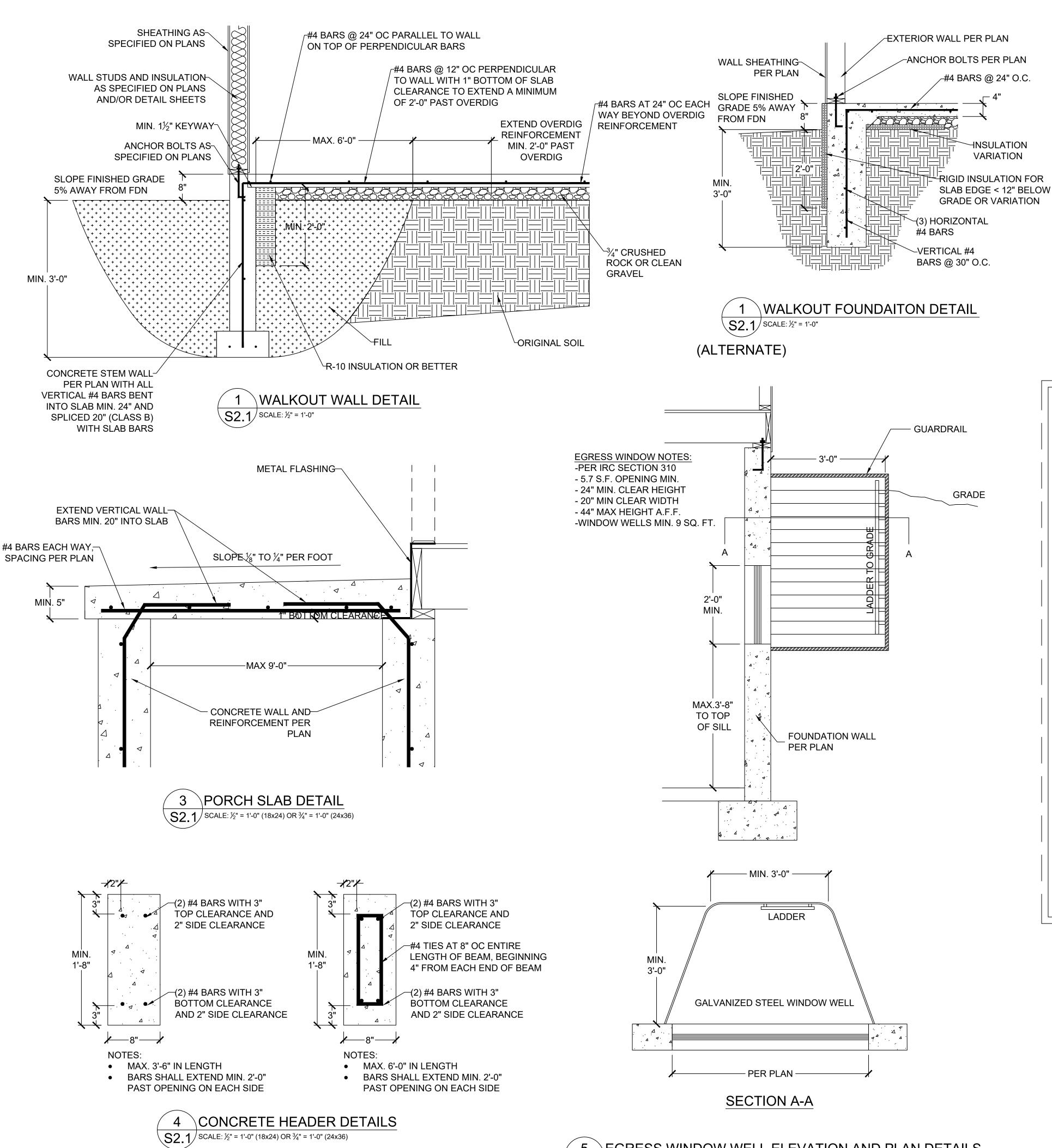


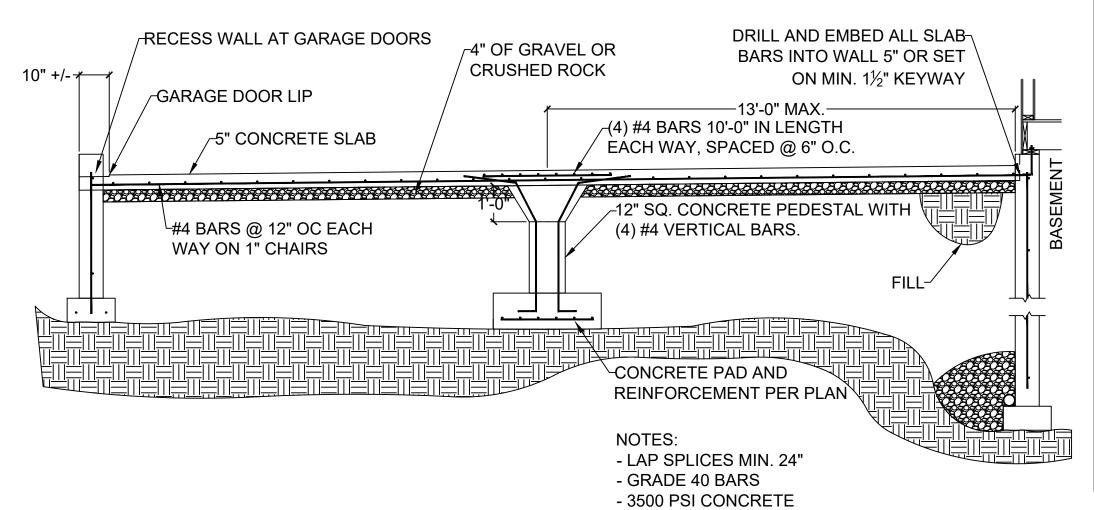
CONCRETE GRADE BEAM S2.0 SCALE: 1" = 1'-0" (18x24) OR $1\frac{1}{2}$ " = 1'-0" (24x36)

RETURN WALL DETAIL S2.0 SCALE: $\frac{1}{2}$ " = 1'-0" (18x24) OR $\frac{3}{4}$ " = 1'-0" (24x36)

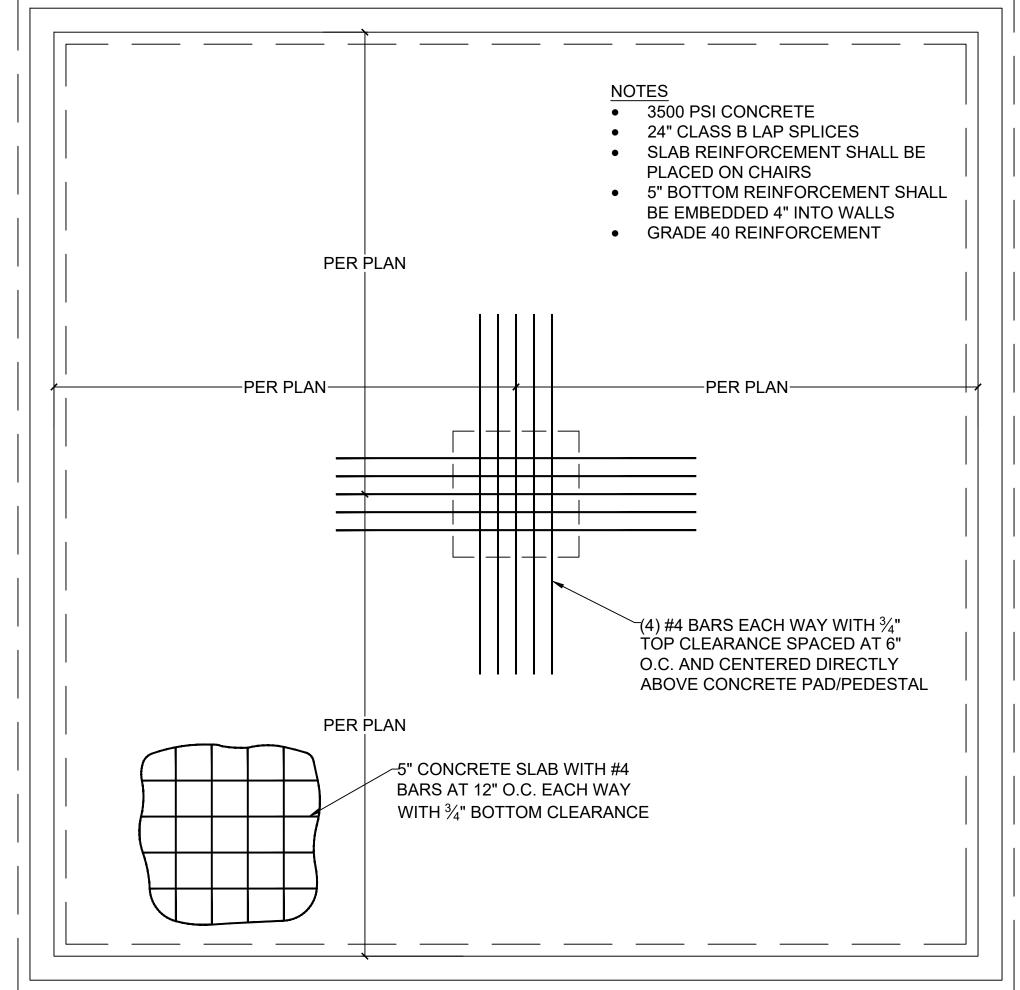








2 GARAGE SLAB ON FILL S2.1 SCALE: 1/4" = 1'-0" (18x24) OR 3/8" = 1'-0" (24x36)



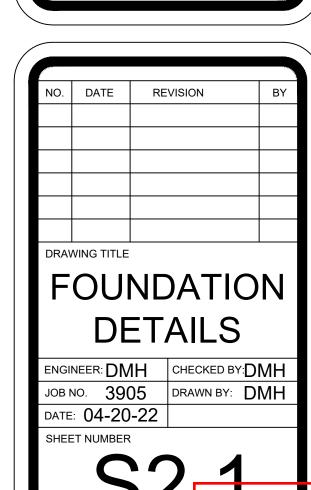


JOB TITLE: TCR015 TOWNHOME

LOT 15, THE TOWNHOMES OF CHAPEL RI

LOCATION: 4027, 4025, 4023, & 4021 NE SAGAMORE D

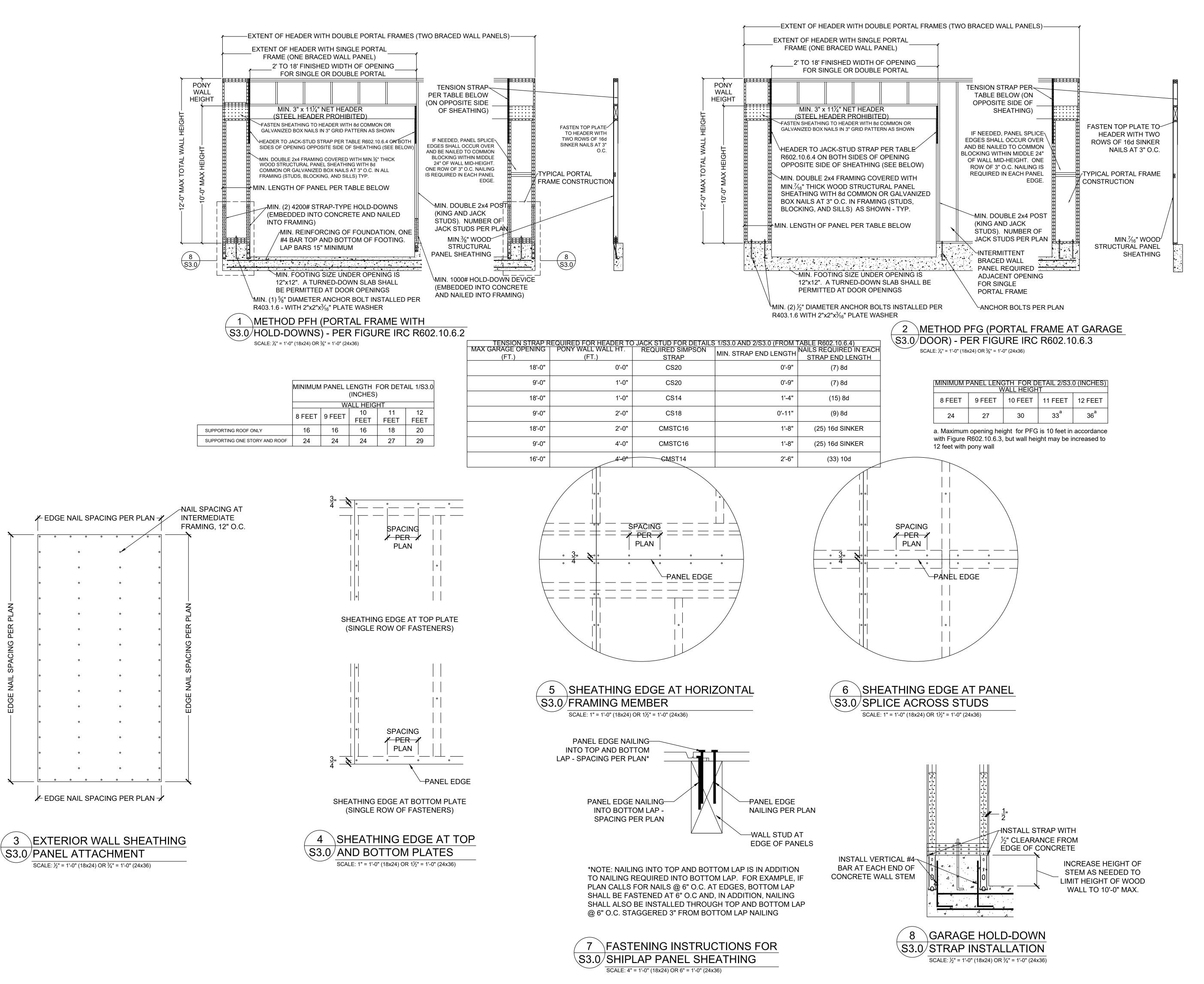




5 EGRESS WINDOW WELL ELEVATION AND PLAN DETAILS S2.1 SCALE: ½" = 1'-0" (18x24) OR ¾" = 1'-0" (24x36)

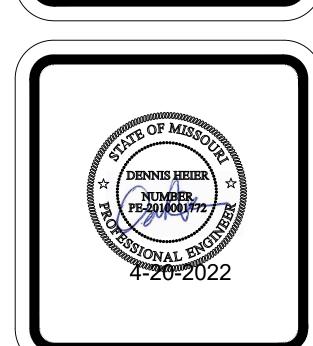
LEE'S SUMMIT MISSOURI

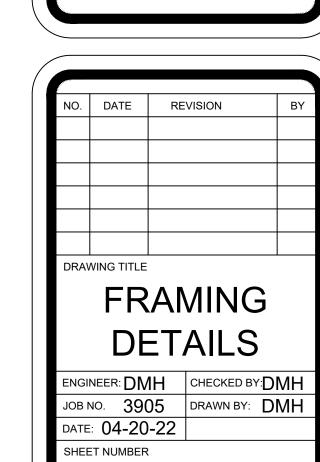
04/21/2022

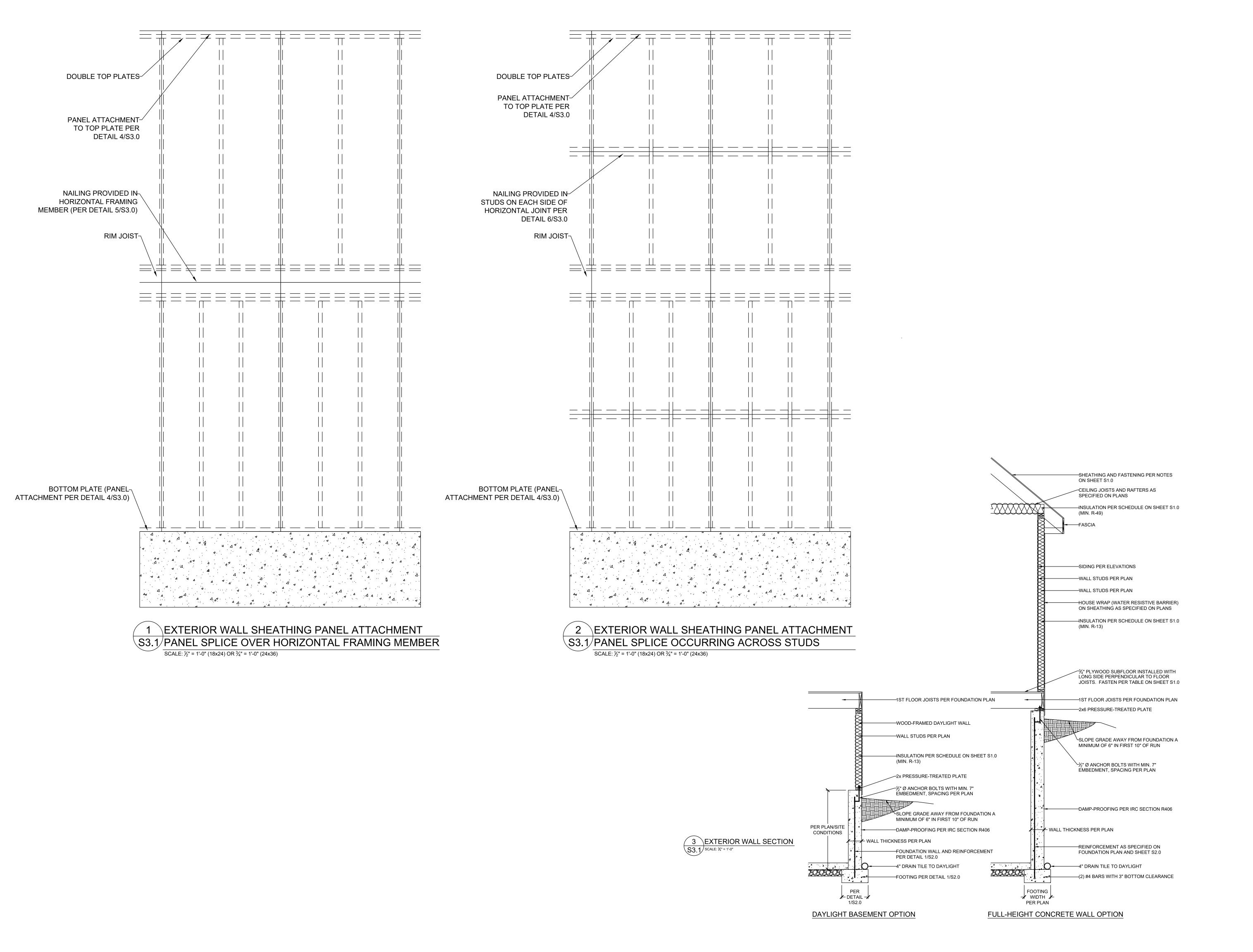




CLIENT: KEVIN HIGDON CONSTRUCTION, LLC
JOB TITLE: TCR015 TOWNHOME
LOT 15, THE TOWNHOMES OF CHAPE

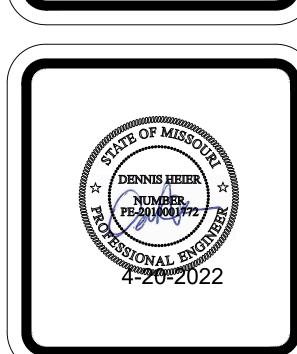


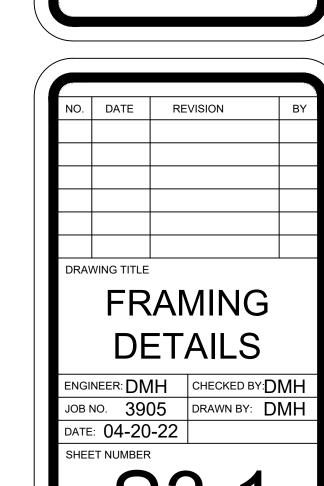


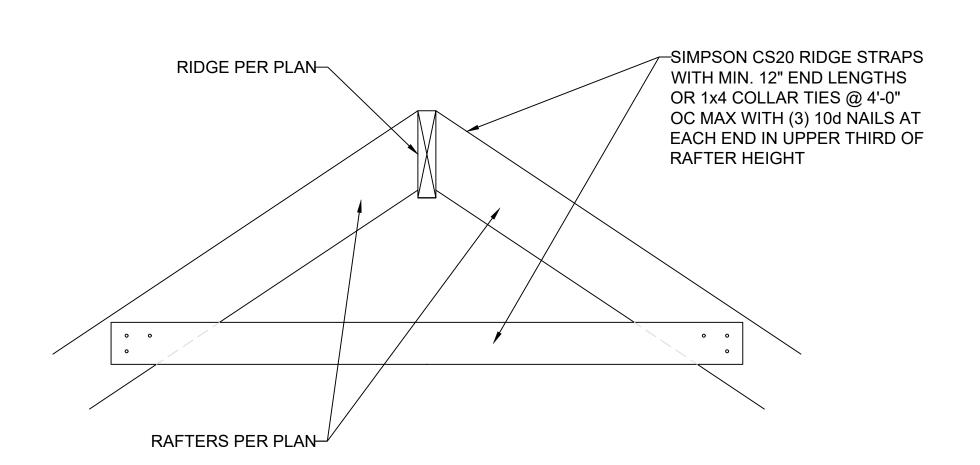




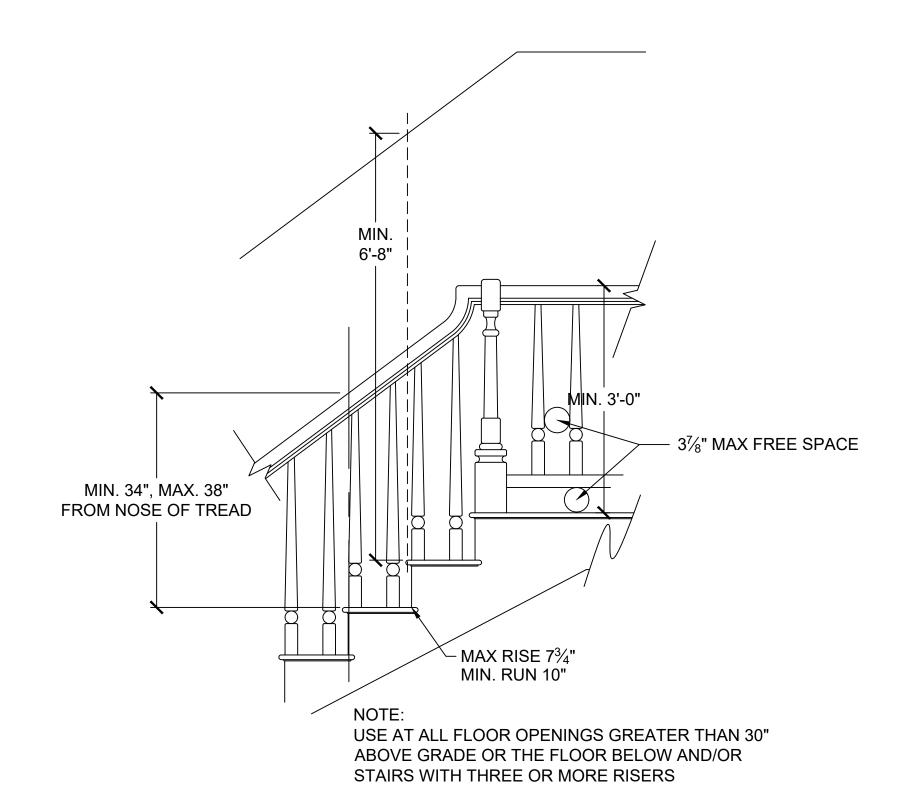
CLIENT: KEVIN HIGDON CONSTRUCTION, LLC JOB TITLE: TCR015 TOWNHOME LOT 15, THE TOWNHOMES OF CH



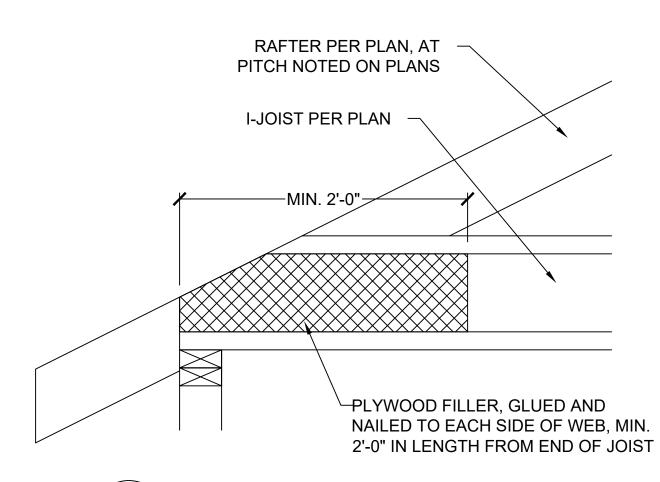




\RIDGE FRAMING DETAIL S3.2 SCALE: 1" = 1'-0" (18x24) OR $1\frac{1}{2}$ " = 1'-0" (24x36)

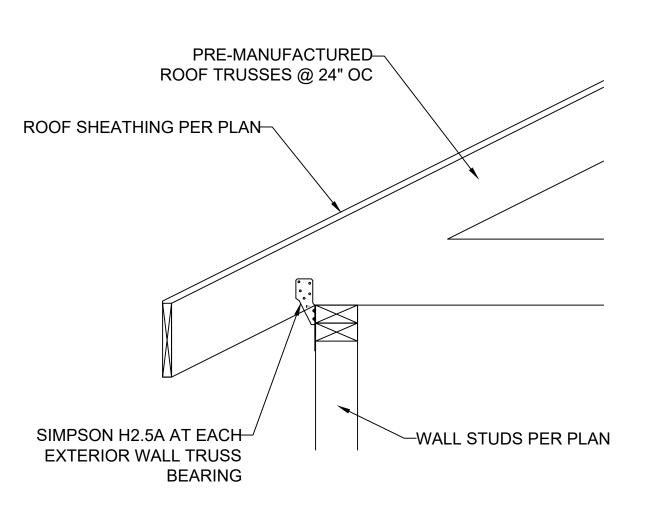


4 \STAIR AND HANDRAIL/GUARDRAIL DETAIL SCALE: $\frac{1}{2}$ " = 1'-0" (18x24) OR $\frac{3}{4}$ " = 1'-0" (24x36)

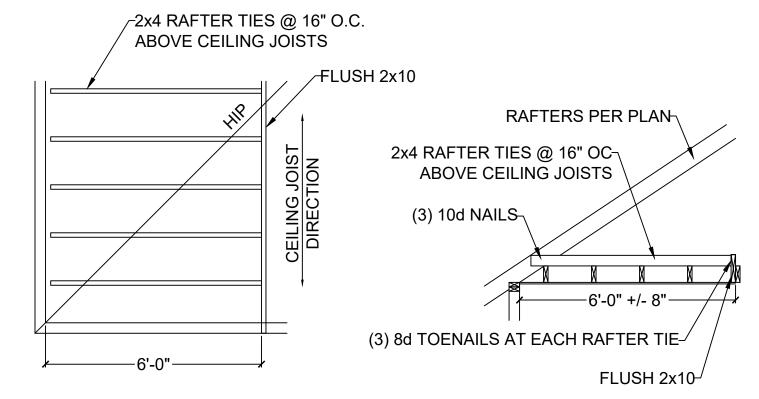


9 \COPED I-JOIST REINFORCEMENT

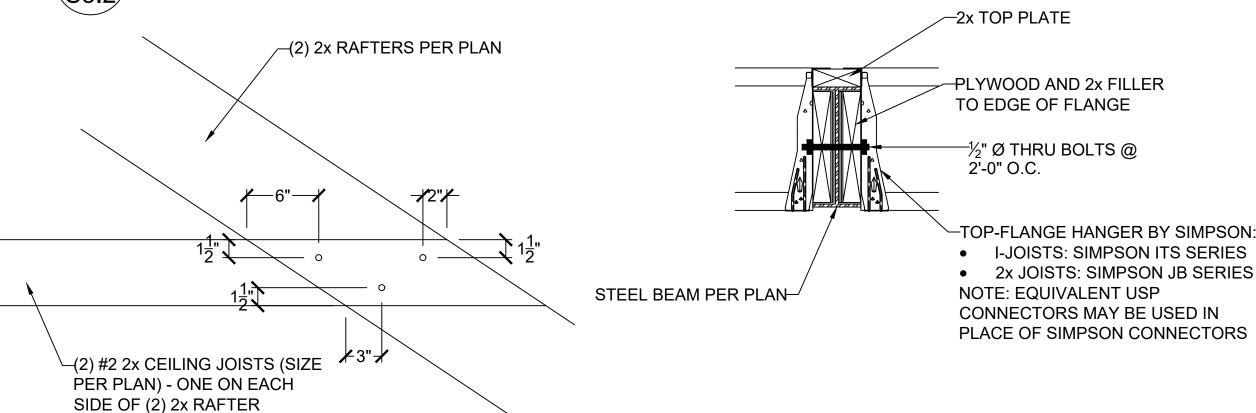
 $\overline{S3.2}$ SCALE: 1" = 1'-0" (18x24) OR $1\frac{1}{2}$ " = 1'-0" (24x36)



TRUSS CONNECTION TO EXT. WALL BEARING SCALE: 1" = 1'-0" (18x24) OR 1½" = 1'-0" (24x36)

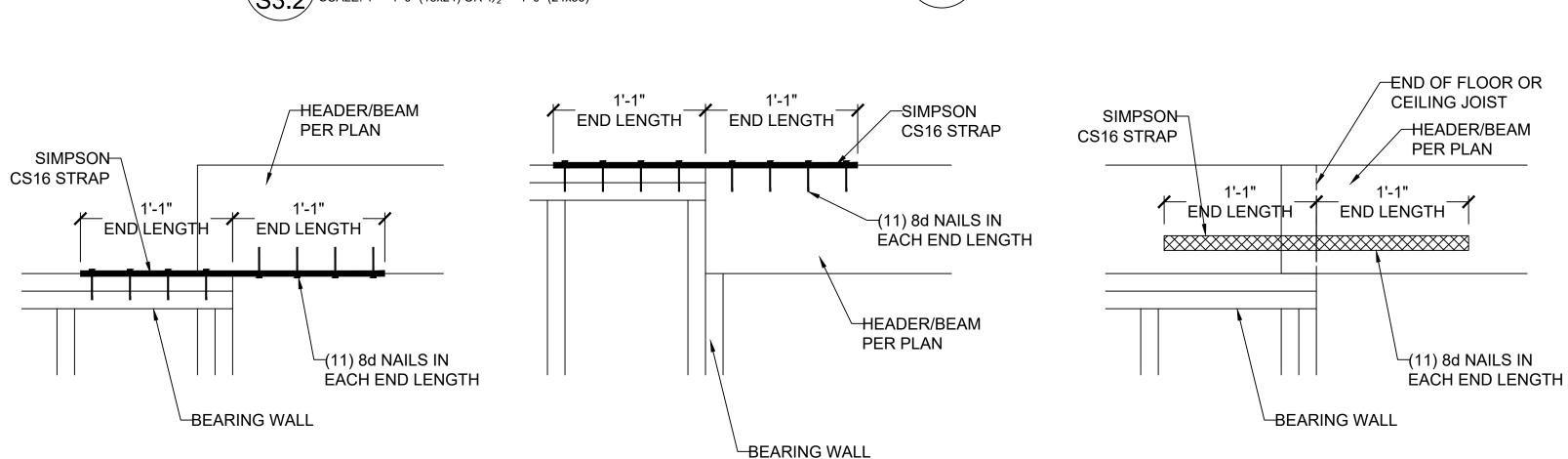




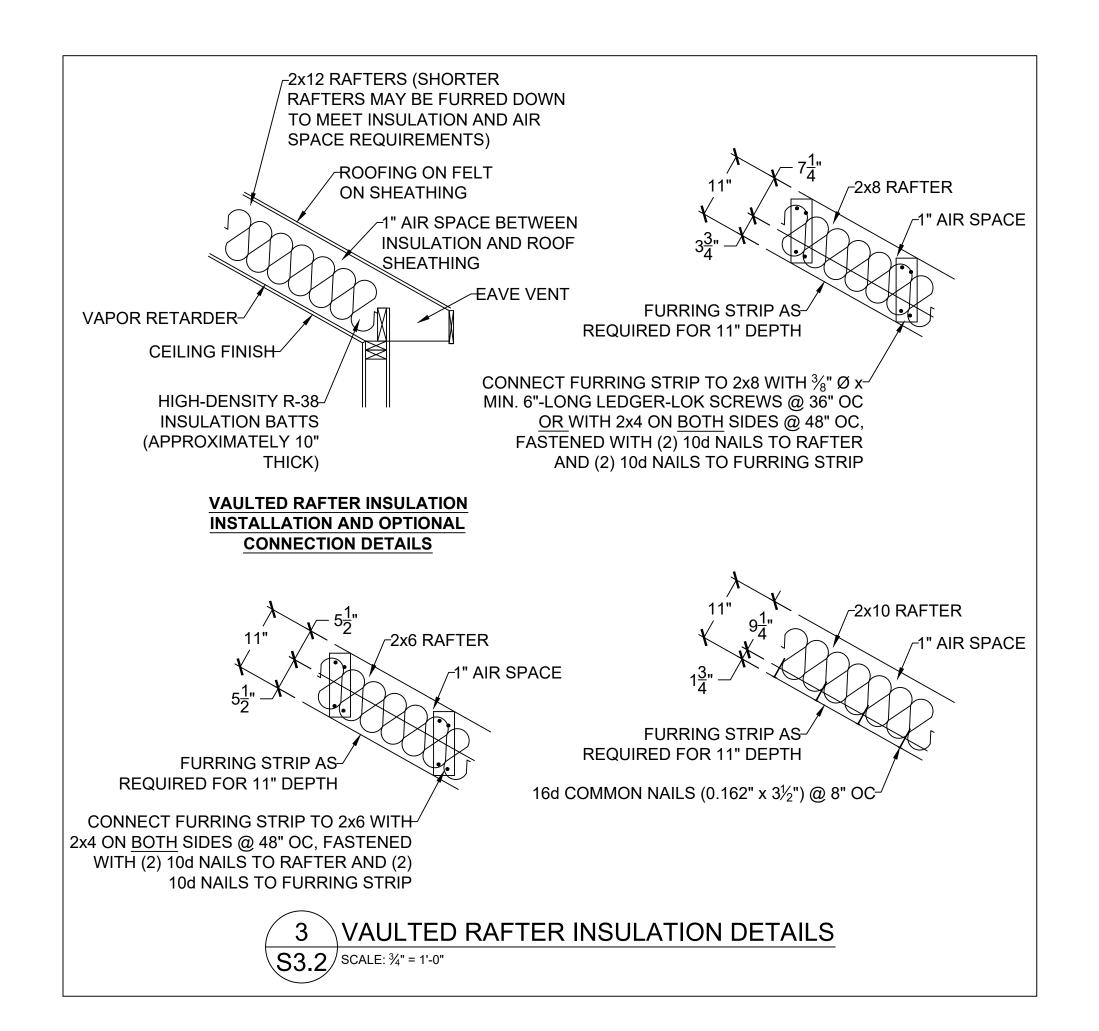


6 \FIELD-CONSTRUCTED A-FRAME DETAIL S3.2 SCALE: 1" = 1'-0" (18x24) OR $1\frac{1}{2}$ " = 1'-0" (24x36)

7 \FLOOR JOIST TO FLUSH STEEL BEAM DETAIL $\frac{\text{S3.2}}{\text{SCALE: 1"}} = 1'-0" (18x24) \text{ OR } 1\frac{1}{2}" = 1'-0" (24x36)$



[10	HEADER/BEAM CONNECTION OPTIONS AT OUTDOOR/OPEN SPACE
7	S3.2/	SCALE: 1" = 1'-0" (18x24) OR $1\frac{1}{2}$ " = 1'-0" (24x36)



	SPACING (INCHES O.C.)					
HEIGHT (FT.)	24	16	12	8		
	SUPPORT	ING A ROOI	ONLY	•		
10 OR LESS	2x4	2x4	2x4	2x4		
12	2x6	2x4	2x4	2x4		
14	2x6	2x6	2x6	2x4		
16	2x6	2x6	2x6	2x4		
18	DR	2x6	2x6	2x6		
20	DR	DR	2x6	2x6		
SUPP	ORTING O	NE FLOOR	AND A ROO	F		
10 OR LESS	2x6	2x4	2x4	2x4		
12	2x6	2x6	2x6	2x4		
14	2x6	2x6	2x6	2x6		
16	DR	2x6	2x6	2x6		
18	DR	2x6	2x6	2x6		
20	DR	DR	2x6	2x6		
SUPPO	DRTING TV	VO FLOORS	AND A ROC)F		
10 OR LESS	2x6	2x6	2x4	2x4		
12	2x6	2x6	2x6	2x6		
14	2x6	2x6	2x6	2x6		
16	DR	2x6	2x6	2x6		
18	DR	DR	2x6	2x6		
20	DR	DR	DR	2x6		

1) DR = DESIGN REQUIRED 2) UTILITY, STANDARD, STUD AND #3 GRADE LUMBER OF ANY SPECIES ARE NOT PERMITTED 3) THIS TABLE DOES NOT APPLY FOR STUDS SUPPORTING MEMBERS WITH A TRIB. LENGTH GREATER THAN 6'-0"

8 MAXIMUM ALLOWABLE LENGTH OF S3.2/WOOD WALL STUDS (IRC TABLE 602.3.1)



R0,



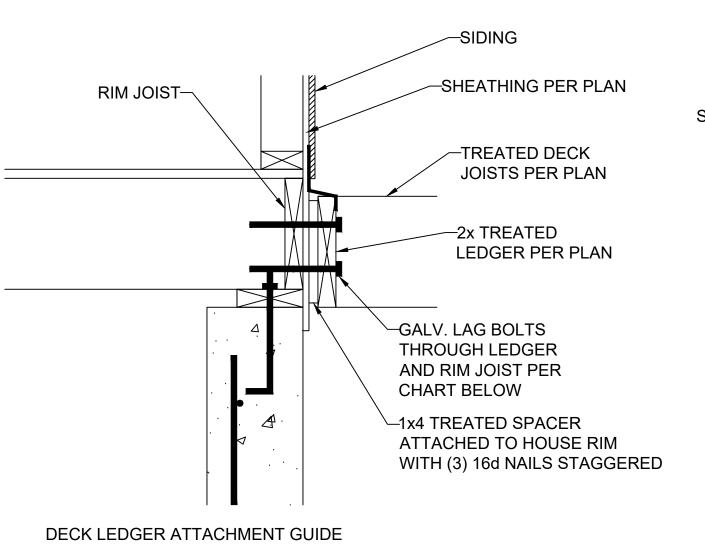
NO.	DATE	RI	EVISION		BY
DRAV	VING TITLE				
	FR	Al	MIN	1G	
	DE	ΞΤ	AIL	.S	
ENGI	NEER: DIV	1H	CHECKE	ED BY:D	MH

JOB NO. 3905 DRAWN BY: DMH

04/21/2022

DATE: 04-20-22

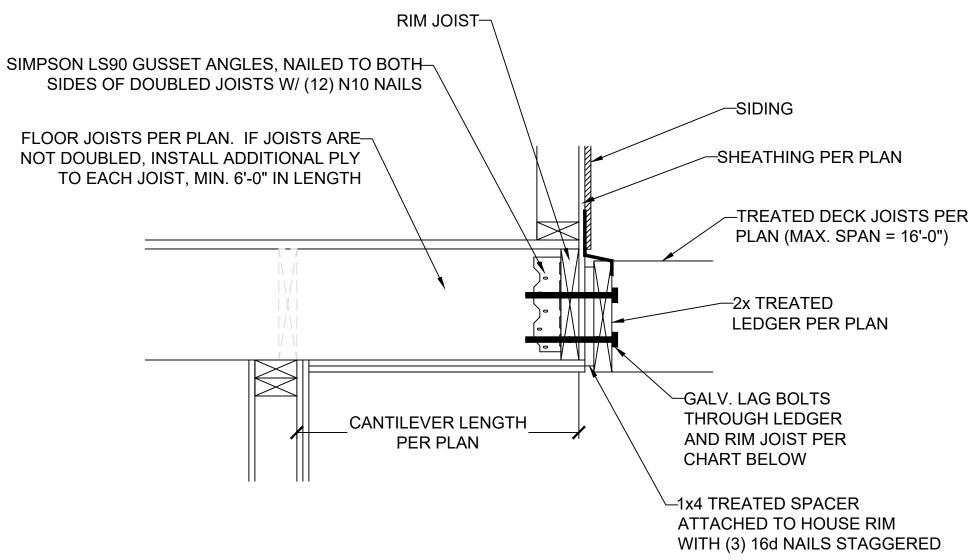
SHEET NUMBER

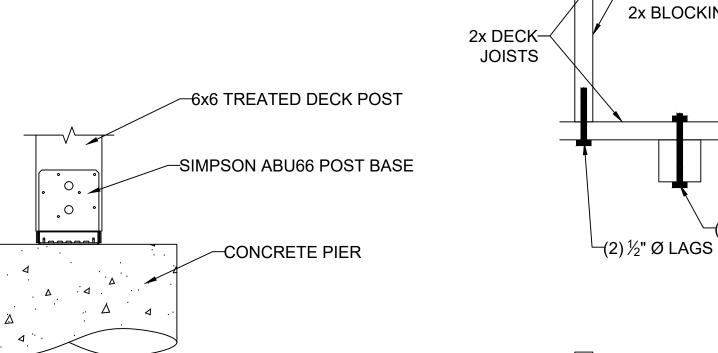


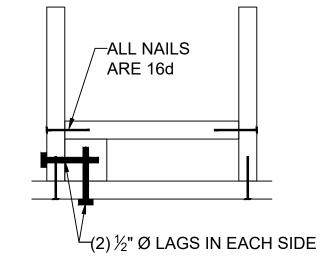
DECK JOIST SPAN	$\frac{1}{2}$ " Ø GALV. LAG OR $\frac{3}{8}$ " Ø LEDGER-LOK SPACING
10'-0" OR LESS	16" OC
10'-0" - 13'-11"	12" OC OR @ 16" OC DOUBLED EVERY OTHER
14'-0" - 18'-0"	8" OC OR @ 16" OC DOUBLED

LEDGER ATTACHMENT

S3.3 | SCALE: 1" = 1'-0" (18x24) OR 1½" = 1'-0" (24x36)







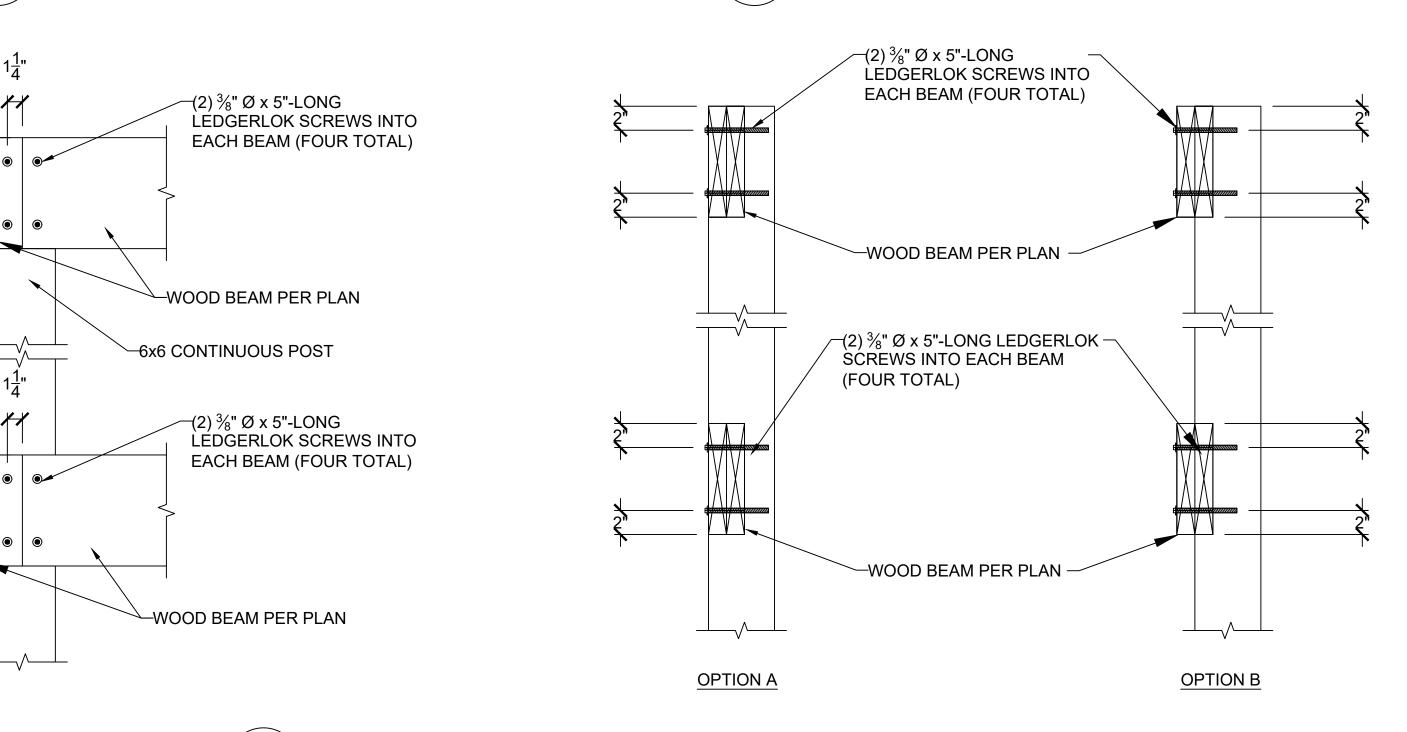
-FULL DEPTH

2x BLOCKING

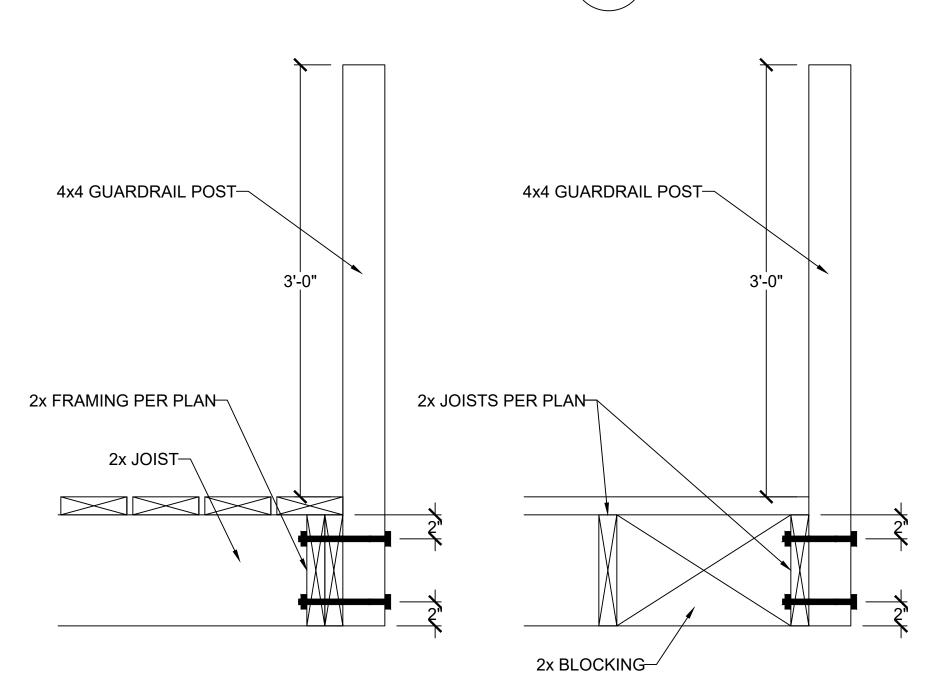
–(2) ½" Ø BOLTS

3 DECK POST BASE S3.3 SCALE: 1" = 1'-0" (18x24) OR $1\frac{1}{2}$ " = 1'-0" (24x36)

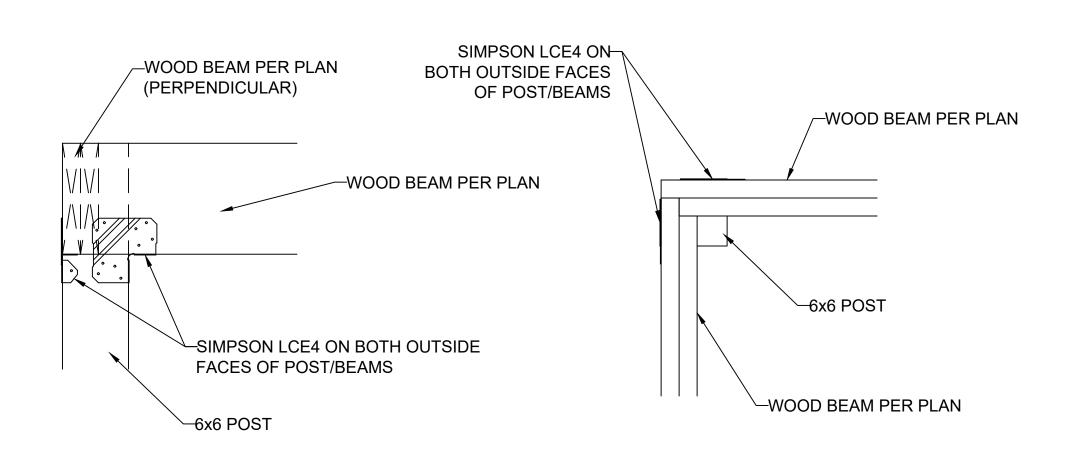
REINF. POST CONNECTIONS 2 CANTILEVER WITH DECK ATTACHMENT S3.3 SCALE: 1" = 1'-0" (18x24) OR $1\frac{1}{2}$ " = 1'-0" (24x36) S3.3 SCALE: 1" = 1'-0" (18x24) OR 1½" = 1'-0" (24x36)



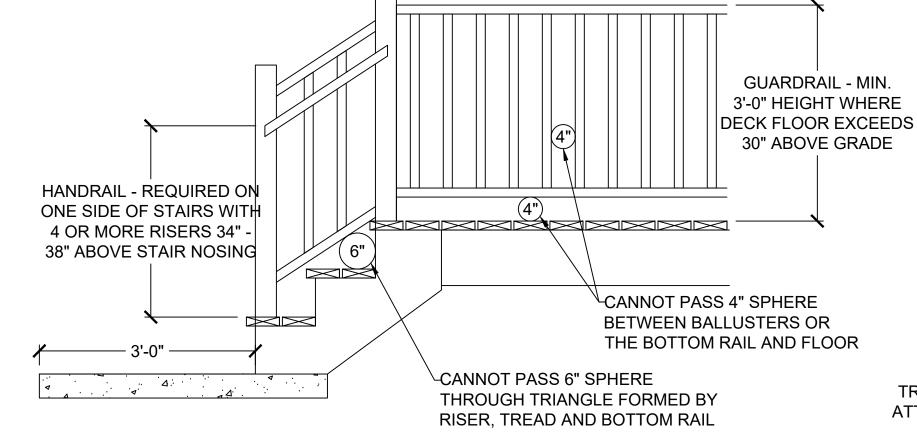
5 \LET-IN (COVERED) DECK BEAM CONNECTION

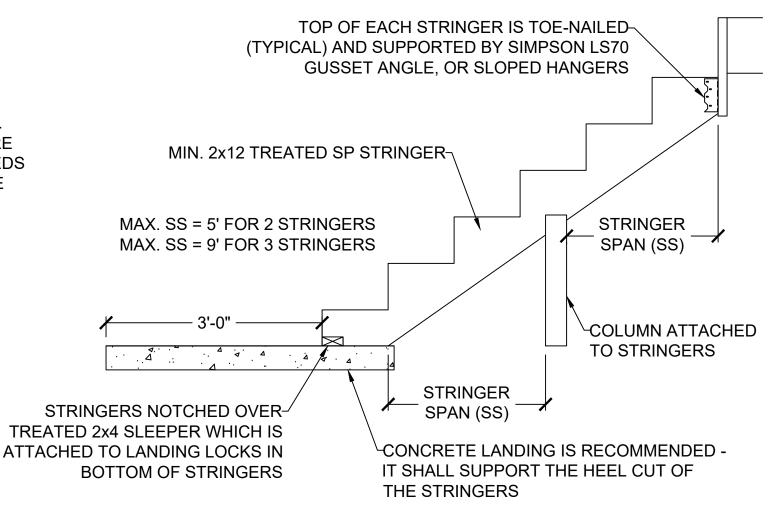


6 \GUARDRAIL CONNECTION S3.3 SCALE: 1" = 1'-0" (18x24) OR 1½" = 1'-0" (24x36)



S3.3 SCALE: 1" = 1'-0" (18x24) OR 1½" = 1'-0" (24x36)





SCALE: $\frac{1}{2}$ " = 1'-0" (18x24) OR $\frac{3}{4}$ " = 1'-0" (24x36)

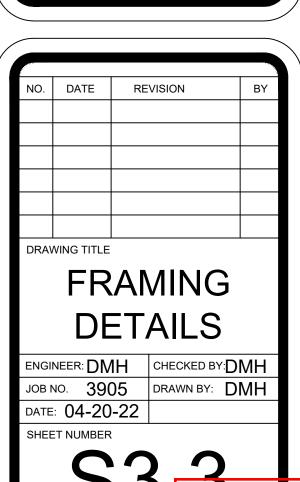
8 GUARDRAIL DETAIL S3.3 SCALE: $\frac{1}{2}$ " = 1'-0" (18x24) OR $\frac{3}{4}$ " = 1'-0" (24x36)

9 \STAIR STRINGER DETAIL (MAX. 5' STAIR WIDTH)



WNHOME E TOWNHOMES (R015 T 15, ⁻





7 \ALTERNATE COVERED DECK/PORCH INTERSECTION S3.3/CORNER BEAM CONNECTION

SCALE: 1" = 1'-0" (18x24) OR 1½" = 1'-0" (24x36)

DESIGN NO. U366 NON-BEARING WALL RATING - 2HR FINISH RATING - 120 MIN $\frac{3}{4}$ " AIR SPACE 3" AIR SPACE 5 4)-**SECTION B-B -**(5) **CONFIGURATION B** EXPOSED TO FIRE FROM EITHER SIDE NTS COMPONENT 2" WIDE CHANNEL AT FLOOR, INTERMEDIATE OR OF TOP WALL 2" DEEP x 13" H-SHAPED STEEL STUDS @ 24" OC

(2) LAYERS OF 1" THICK GYPSUM BOARD LINER PANELS IN 24" WIDTHS

AREA SEPARATION WALL: (MAX HEIGHT - 44 FT)

(JAMES HARDIE GYPSUM INC-TYPE HARDILINER)

BETWEEN WOOD FRAMING AND "H" STUDS.

*BEARING THE UL CLASSIFICATION MARK

AREA SEPARATION WALL.

COMPOUND.

PROTECTED WALL: (BEARING OR NON-BEARING WALL)

MIN 1 THICK x 4' WIDE GYPSUM BOARD APPLIED HORIZONTAL OR VERTICAL

ALUMINUM ANGLE ATTACHMENT CLIPS- MIN 2" WIDE WITH MIN 2" AND 21 LEGS

1. FLOOR, INTERMEDIATE OR TOP OF WALL - 2 IN. WIDE CHANNEL SHAPED WITH 1-IN LONG LEGS

2. STEEL STUDS - STEEL MEMBERS FORMED FROM NO. 25 MSG GALV STEEL HAVING "H" SHAPED

4. WOOD STUDS - NOM 2 BY 4 IN. MAX SPACING @ 24 IN. OC. STUDS CROSS-BRACED AT MIDHEIGHT

5. GYPSUM BOARD - CLASSIFIED OR UNCLASSIFIED - MIN. ½ IN. THICK, 4FT WIDE, APPLIED EITHER

COVERED WITH PAPER TAPE AND JOINT COMPOUND. NAIL HEADS COVERED WITH JOINT

HORIZONTALLY OR VERTICALLY. WALLBOARD ATTACHED TO STUDS WITH 1\frac{1}{4} IN. LONG STEEL

WHERE NECESSARY FOR CLIP ATTACHMENT. MIN. $\frac{3}{4}$ " SEPARATION BETWEEN WOOD FRAMING AND

DRYWALL NAILS SPACED @ 8 IN. OC. VERTICAL JOINTS LOCATED OVER STUDS. (OPTIONAL) JOINTS

ATTACHMENT CLIPS - ALUMINUM ANGLE, 0.063 IN. THICK, MIN 2 IN. WIDE WITH MIN 2 IN. AND $2\frac{1}{4}$ IN.

LEGS. CLIPS SECURED WITH TYPE S SCREWS 3 IN. LONG TO "H" STUDS AND WITH TYPE W SCREWS

 $1\frac{1}{4}$ IN. LONG TO WOOD FRAMING THROUGH HOLES PROVIDED IN CLIP. CLIPS SPACED A MAX OF 10

HIGH. FOR SEPARATION WALLS UP TO 44FT HIGH, CLIPS SPACED AS DESCRIBED ABOVE FOR THE

FT OC VERTICALLY BETWEEN WOOD FRAMING AND "H" STUDS FOR SEPARATION WALLS UP TO 23 FT

UPPER 24 FT AND THE REMAINING WALL AREA BELOW REQUIRES CLIPS A MAX 5 FT OC VERTICALLY

24 IN WIDTHS. VERTICAL EDGES OF PANELS FRICTION FITTED INTO "H" SHAPED STUDS.

FLANGE SPACED @ 24 IN OC; OVERALL DEPTH 2 IN AND FLANGE WIDTH 1-3/8 IN.

FORMED FROM NO. 25 MSG GALV STEEL, SECURED WITH SUITABLE FASTENERS SPACED @ 24 IN OC

GYPSUM BOARD* - 2 LAYERS OF 1 IN THICK GYPSUM WALLBOARD LINER PANELS, SUPPLIED IN NOM

2x4 WOOD STUDS @24" OC MAX, MIN $\frac{3}{4}$ " SEPARATION BETWEEN WOOD FRAMING & AREA SEPARATION WALL

SEPARATION WALL AND AJOINING WALL NOTES:

- TWO HOUR FIRE WALL PER UL DESIGN # U366 SHOWN IN THE UL FIRE RESISTANCE DIRECTORY
- INSULATE STUD CAVITIES WITH 3¹/₂" BATT INSULATION

-(2) 1" TYPE X SHEETROCK

 $-\frac{3}{4}$ " T AND G PLYWOOD (GLUED AND NAILED)

I-JOISTS OR #2-2x10 JOISTS @16" OC

-(2) 2x4 TOP PLATE

-(2) 1" TYPE X SHEETROCK

" SHEETROCK (5" SHEETROCK FOR ONE

STORY STRUCTURES)

- ANY SHAFT WALL PENETRATIONS IN EXCESS OF 1 BUT LESS THAN 1 TO BE FILLED WITH APPROVED
- LAYER OF 5" TYPE X SHEET ROCK, PROPERLY NAILED AND GLUED. SEAL ADDITIONAL DRYWALL PATCH COMPLETELY WITH FIRE CAULK

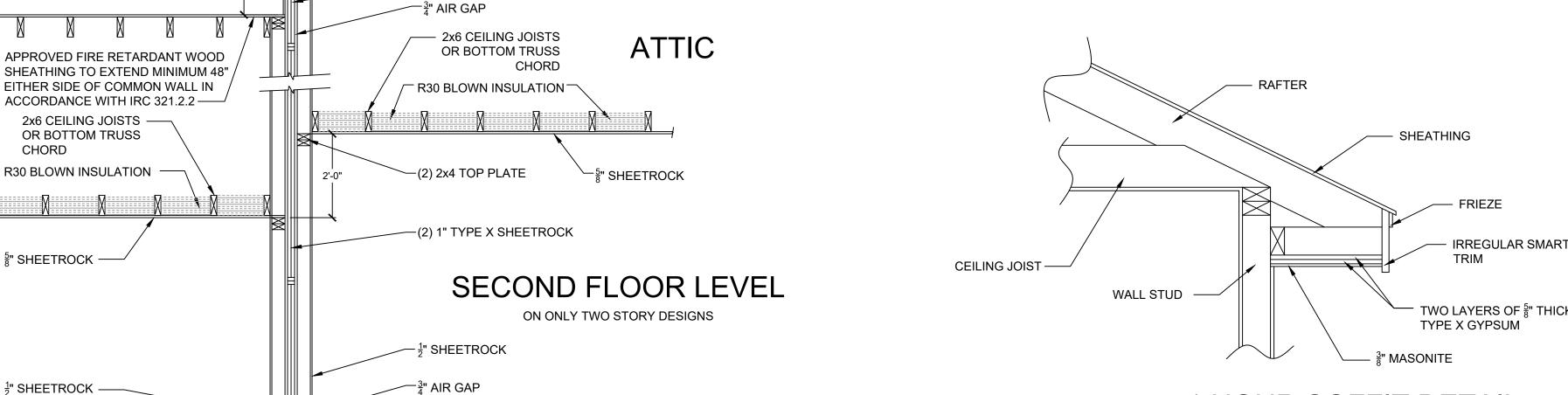
2'-0"

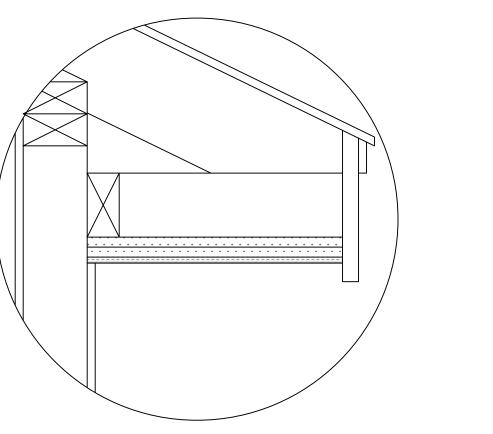
 $\frac{3}{4}$ " AIR GAP

2x4 PLATE

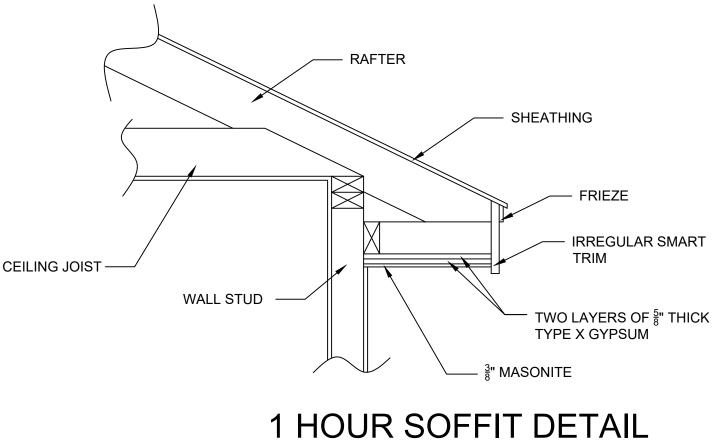
I-JOISTS OR #2-2x10 JOISTS @16" OC

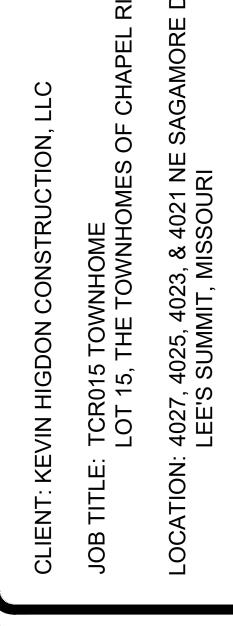


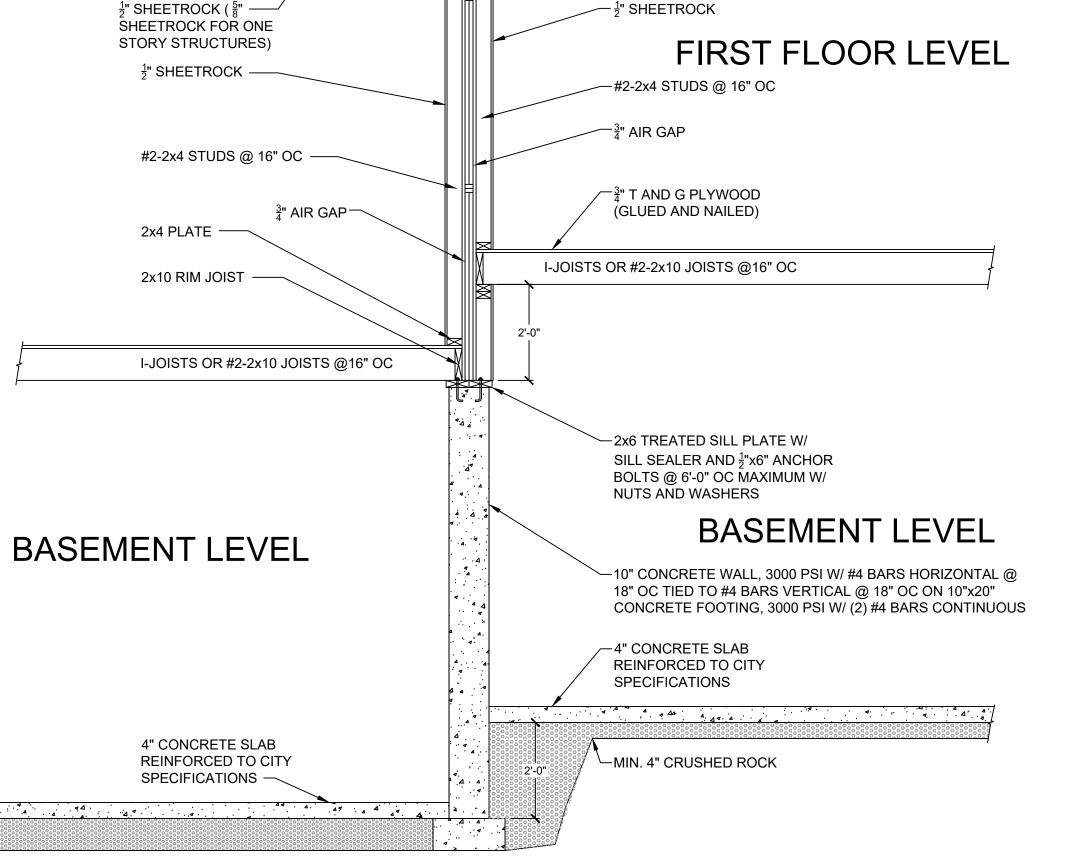


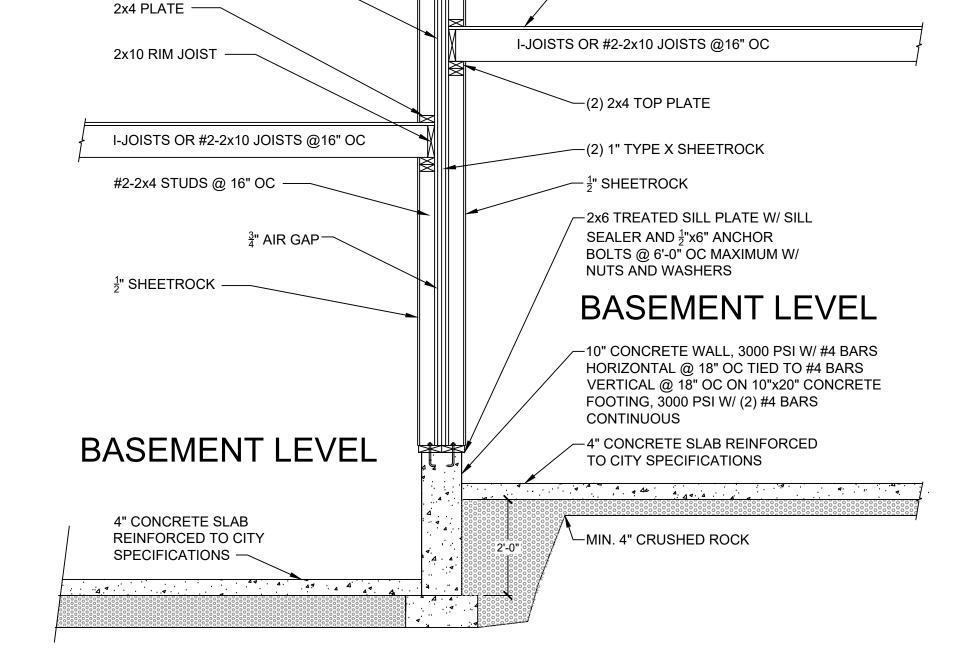


TWO LAYERS OF 5" THICK TYPE X GYPSUM BOARD COVERED BY 3" MASONITE APPLIED AT RIGHT ANGLES W/3" LONG TYPE W SCREWS @ 8" OC









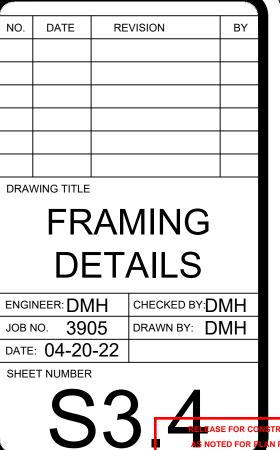
3" AIR GAP

 $-\frac{3}{4}$ " T AND G PLYWOOD

(GLUED AND NAILED)

ALTERNATE FIRE WALL SECTION AT BASEMENT





FIRE WALL SECTION