

IBC 2018 REQUIRED SPECIAL INSPECTIONS

		FREQUENCY OF INSPECTION	
		CONTINUOUS	PERIODIC
STEEL CONSTRUCTION - STRUCTURAL STEEL (IBC SECTION 1705.2.1)			
1.	SPECIAL INSPECTION AND NONDESTRUCTIVE TESTING OF STRUCTURAL STEEL ELEMENTS IN BUILDINGS, STRUCTURES AND PORTIONS THEREOF SHALL BE IN ACCORDANCE WITH THE QUALITY ASSURANCE INSPECTION REQUIREMENTS OF AISC 360-16.		
CONCRETE CONSTRUCTION (IBC TABLE 1705.3)			
1.	INSPECT REINFORCEMENT, INCLUDING PRESTRESSING TENDONS, AND VERIFY PLACEMENT.	---	X
2.	REINFORCING BAR WELDING:		
A.	VERIFY WELDABILITY OF REINFORCING BARS OTHER THAN ASTM A706;	---	X
B.	INSPECT SINGLE-PASS FILLET WELDS, MAXIMUM 5/16"; AND		X
C.	INSPECT ALL OTHER WELDS	X	
3.	INSPECT ANCHORS CAST IN CONCRETE.	---	X
4.	INSPECT ANCHORS POST-INSTALLED IN HARDENED CONCRETE MEMBERS. (a)		
A.	ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY OR UPWARDLY INCLINED ORIENTATIONS TO RESIST SUSTAINED TENSION LOADS.	X	
B.	MECHANICAL ANCHORS AND ADHESIVE ANCHORS NOT DEFINED IN 4 A.	---	X
5.	VERIFY USE OF REQUIRED DESIGN MIX.	---	X
6.	PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE.	X	---
7.	INSPECT CONCRETE AND SHOTCRETE PLACEMENT OF PROPER APPLICATION TECHNIQUES.	X	---
8.	VERIFY MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES	---	X
9.	INSPECT FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED.	---	X
a.	SPECIFIC REQUIREMENTS FOR SPECIAL INSPECTION SHALL BE INCLUDED IN THE RESEARCH REPORT FOR THE ANCHOR ISSUED BY AN APPROVED SOURCE IN ACCORDANCE WITH 17.8.2 IN ACI 318. WHERE SPECIFIC REQUIREMENTS ARE NOT PROVIDED, CONTACT THE STRUCTURAL ENGINEER-OF-RECORD FOR SPECIAL INSPECTION REQUIREMENTS.		
WOOD CONSTRUCTION - IBC SECTION 1705.5			
1.	SPECIAL INSPECTION OF THE FABRICATION PROCESS OF PREFABRICATED WOOD STRUCTURAL ELEMENTS AND ASSEMBLIES SHALL BE IN ACCORDANCE WITH SECTION 1704.2.5. SPECIAL INSPECTION OF SITE BUILT ASSEMBLIES SHALL BE IN ACCORDANCE WITH SECTION 1705.5		
2.	INSPECTION OF WOOD STRUCTURAL PANEL SHEATHING GRADE AND THICKNESS.	---	X
3.	VERIFICATION OF THE NOMINAL SIZE OF FRAMING MEMBERS AT ADJOINING PANEL EDGES AGREES WITH THE APPROVED CONSTRUCTION DOCUMENTS. (required at wood high load diaphragms designed in accordance with 2306.2.)	---	X
4.	VERIFICATION OF THE NAIL OR STAPLE DIAMETER AND LENGTH, THE NUMBER OF FASTENER LINES AND THE SPACING BETWEEN FASTENERS IN EACH LINE AND AT EDGE MARGINS AGREES WITH THE APPROVED CONSTRUCTION DOCUMENTS.	---	X
5.	VERIFICATION THAT THE INSTALLATION OF THE PERMANENT INDIVIDUAL TRUSS RESTRAINT/BRACING HAS BEEN INSTALLED IN ACCORDANCE WITH THE APPROVED TRUSS SUBMITTAL PACKAGE FOR WOOD TRUSSES WITH OVERALL HEIGHTS OF 60 INCHES OR GREATER.	---	X
6.	VERIFICATION THAT THE TEMPORARY INSTALLATION RESTRAINT/BRACING AND THE PERMANENT INDIVIDUAL TRUSS MEMBER RESTRAINT/BRACING ARE INSTALLED IN ACCORDANCE WITH THE APPROVED TRUSS SUBMITTAL PACKAGE AT METAL-PLATE-CONNECTED WOOD TRUSSES WITH A CLEAR SPAN OF 60'-0" OR GREATER.	---	X
SOILS (IBC TABLE 1705.6)			
1.	VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY.	---	X
2.	VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL.	---	X
3.	PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS.	---	X
4.	VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF COMPACTED FILL.	X	---
5.	PRIOR TO PLACEMENT OF COMPACTED FILL, OBSERVE SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY.	---	X
<p>** CONTINUOUS SPECIAL INSPECTION: SPECIAL INSPECTION BY THE SPECIAL INSPECTOR WHO IS PRESENT WHEN AND WHERE THE WORK TO BE INSPECTED IS BEING PERFORMED.</p> <p>** PERIODIC SPECIAL INSPECTION: SPECIAL INSPECTION BY THE SPECIAL INSPECTOR WHO IS INTERMITTENTLY PRESENT WHERE THE WORK TO BE INSPECTED HAS BEEN OR IS BEING PERFORMED.</p>			

AISC 360-16 SPECIAL INSPECTION REQUIREMENTS

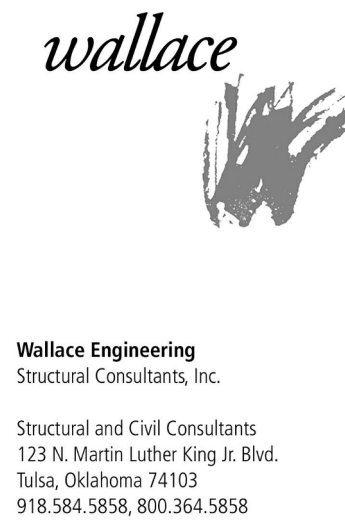
1. QUALITY CONTROL (QC) SHALL BE PROVIDED BY THE FABRICATOR AND ERECTOR.
2. QUALITY ASSURANCE (QA) SHALL BE PROVIDED BY OTHERS.
3. NONDESTRUCTIVE TESTING (NDT) SHALL BE PERFORMED BY THE AGENCY OR FIRM RESPONSIBLE FOR QUALITY ASSURANCE (QA).
4. THE QUALITY ASSURANCE INSPECTOR (QAI) SHALL REVIEW MATERIAL TEST REPORTS AND CERTIFICATIONS AS LISTED IN SECTION N3.2 FOR COMPLIANCE WITH THE CONSTRUCTION DOCUMENTS.
5. FOR WORK PERFORMED BY APPROVED FABRICATORS AND ERECTORS:
 - A. QA INSPECTIONS MAY BE WAIVED WHEN THE WORK IS PERFORMED IN A FABRICATING SHOP OR BY AN ERECTOR APPROVED BY THE AUTHORITY HAVING JURISDICTION (AHJ) TO PERFORM THE WORK WITHOUT QA.
 - B. NDT OF WELDS COMPLETED IN AN APPROVED FABRICATOR'S SHOP MAY BE PERFORMED BY THAT FABRICATOR WHEN APPROVED BY THE AHJ. WHEN THE FABRICATOR PERFORMS THE NDT, THE QA AGENCY SHALL REVIEW THE FABRICATOR'S NDT REPORTS.
 - C. AT COMPLETION OF FABRICATION, THE APPROVED FABRICATOR SHALL SUBMIT A CERTIFICATE OF COMPLIANCE TO THE AHJ STATING THAT THE MATERIALS SUPPLIED AND WORK PERFORMED BY THE FABRICATOR ARE IN ACCORDANCE WITH THE CONSTRUCTION DOCUMENTS.
 - D. AT COMPLETION OF ERECTION, THE APPROVED ERECTOR SHALL SUBMIT A CERTIFICATE OF COMPLIANCE TO THE AHJ STATING THAT THE MATERIALS SUPPLIED AND WORK PERFORMED BY THE ERECTOR ARE IN ACCORDANCE WITH THE CONSTRUCTION DOCUMENTS.

AISC 360-16, CHAPTER N SPECIAL INSPECTION REQUIREMENTS

		FREQUENCY OF INSPECTION	
		PERFORM	OBSERVE
N5.4 - INSPECTION OF WELDING			
AISC 360-16, TABLE N5.4.1 - INSPECTION TASKS PRIOR TO WELDING			
1.	WELDER QUALIFICATION RECORDS AND CONTINUITY RECORDS	---	X
2.	WELDING PROCEDURE SPECIFICATIONS (WPS) AVAILABLE	X	---
3.	MANUFACTURER CERTIFICATIONS FOR WELDING CONSUMABLES AVAILABLE	X	---
4.	MATERIAL IDENTIFICATION (TYPE/GRADE)	---	X
5.	WELDER IDENTIFICATION SYSTEM (a)	---	X
6.	FIT-UP OF GROOVE WELDS (INCLUDING JOINT GEOMETRY)		
A.	JOINT PREPARATION	---	X
B.	DIMENSIONS (ALIGNMENT, ROOT OPENING, ROOT FACE, BEVEL)	---	X
C.	CLEANLINESS (CONDITION OF STEEL SURFACES)	---	X
D.	TACKING (TACK WELD QUALITY AND LOCATION)	---	X
E.	BACKING TYPE AND FIT (IF APPLICABLE)	---	X
7.	FIT-UP OF CJP GROOVE WELDS OF HSS, T-, Y- AND K-JOINTS WITHOUT BACKING (INCLUDING JOINT GEOMETRY)		
A.	JOINT PREPARATIONS	---	X
B.	DIMENSIONS (ALIGNMENT, ROOT OPENING, ROOT FACE, BEVEL)	---	X
C.	CLEANLINESS (CONDITION OF STEEL SURFACES)	---	X
D.	TACKING (TACK WELD QUALITY AND LOCATION)	---	X
8.	CONFIGURATION AND FINISH OF ACCESS HOLES	---	X
9.	FIT-UP OF FILLET WELDS		
A.	DIMENSIONS (ALIGNMENT, GAPS AT ROOT)	---	X
B.	CLEANLINESS (CONDITION OF STEEL SURFACES)	---	X
C.	TACKING (TACK WELD QUALITY AND LOCATION)	---	X
AISC 360-16, TABLE N5.4.2 - INSPECTIONS DURING WELDING			
1.	CONTROL AND HANDLING OF WELDING CONSUMABLES		
A.	PACKAGING	---	X
B.	EXPOSURE CONTROL	---	X
2.	NO WELDING OVER CRACKED TACK WELDS	---	X
3.	ENVIRONMENTAL CONDITIONS		
A.	WIND SPEED WITHIN LIMITS	---	X
B.	PRECIPITATION AND TEMPERATURE	---	X
4.	WELDING PROCEDURE SPECIFICATION (WPS) FOLLOWED		
A.	SETTINGS ON WELDING EQUIPMENT	---	X
B.	TRAVEL SPEED	---	X
C.	SELECTED WELDING MATERIALS	---	X
D.	SHIELDING GAS TYPE / FLOW RATE	---	X
E.	PREHEAT APPLIED	---	X
F.	INTERPASS TEMPERATURE MAINTAINED (MIN./MAX.)	---	X
G.	PROPER POSITION (F, V, H, OH)	---	X
5.	WELDING TECHNIQUES		
A.	INTERPASS AND FINAL CLEANING	---	X
B.	EACH PASS WITHIN PROFILE LIMITATIONS	---	X
C.	EACH PASS MEETS QUALITY REQUIREMENTS	---	X
6.	PLACEMENT AND INSTALLATION OF STEEL HEADED STUD ANCHORS	X	---
AISC 360-16, TABLE N5.4.3 - INSPECTION TASKS AFTER WELDING			
1.	WELDS CLEANED	---	X
2.	SIZE, LENGTH AND LOCATION OF WELDS	X	---
3.	WELDS MEET VISUAL ACCEPTANCE CRITERIA		
A.	CRACK PROHIBITION	X	---
B.	WELD/BASE-METAL FUSION	X	---
C.	CRATER CROSS SECTION	X	---
D.	WELD PROFILES	X	---
E.	WELD SIZE	X	---
F.	UNDERCUT	X	---
G.	POROSITY	X	---
4.	ARC STRIKES	X	---
5.	k-AREA (b)	X	---
6.	WELD ACCESS HOLES IN ROLLED HEAVY SHAPES AND BUILT-UP HEAVY SHAPES (c)	X	---
7.	BACKING REMOVED AND WELD TABS REMOVED (IF REQUIRED)	X	---
8.	REPAIR ACTIVITIES	X	---
9.	DOCUMENT ACCEPTANCE OR REJECTION OF WELDED JOINT OR MEMBER	X	---
10.	NO PROHIBITED WELDS HAVE BEEN ADDED WITHOUT THE APPROVAL OF THE EOR.	---	X
11.	ULTRASONIC TESTING (UT) ON ALL CJP GROOVE WELDS IN BUTT, T- AND CORNER JOINTS, IN MATERIALS 5/16 INCH THICK OR GREATER (required in Risk Category III or IV)	X	---
12.	ULTRASONIC TESTING (UT) ON 10% OF CJP GROOVE WELDS IN BUTT, T- AND CORNER JOINTS, IN MATERIALS 5/16 INCH THICK OR GREATER (required in Risk Category II)	---	X
13.	THERMALLY CUT SURFACES OF ACCESS HOLES SHALL BE TESTED USING MAGNETIC PARTICLE TESTING (MT) OR PENETRANT TESTING (PT), WHEN FLANGE THICKNESS EXCEEDS 2 INCHES FOR ROLLED SHAPES, OR WHEN THE WEB THICKNESS EXCEEDS 2 INCHES FOR BUILT-UP SHAPES	X	---
14.	(see AISC 360-16, section N5-5c for additional special inspections for welded joints subject to fatigue)		
(a)	THE FABRICATOR OR ERECTOR, AS APPLICABLE, SHALL MAINTAIN A SYSTEM BY WHICH A WELDER WHO HAS WELDED A JOINT OR MEMBER CAN BE IDENTIFIED. STAMPS, IF USED, SHALL BE THE LOW STRESS TYPE.		
(b)	WHEN WELDING OF DOUBLER PLATES, CONTINUITY PLATES OR STIFFENERS HAS BEEN PERFORMED IN THE k-AREA, VISUALLY INSPECT THE WEB k-AREA FOR CRACKS WITHIN 3 INCHES OF THE WELD.		
(c)	AFTER ROLLED HEAVY SHAPES AND BUILT-UP HEAVY SHAPES HAVE BEEN WELDED, VISUALLY INSPECT THE WELD ACCESS HOLE PER CRACKS.		
<p>** PERFORM - PERFORM THESE TASKS FOR EACH WELDED JOINT OR MEMBER.</p> <p>** OBSERVE - OBSERVE THESE ITEMS ON A RANDOM BASIS. OPERATIONS NEED NOT BE DELAYED PENDING THESE INSPECTIONS.</p>			
N5.6 - INSPECTION OF HIGH-STRENGTH BOLTS			
AISC 360-16, TABLE N5.6.1 - INSPECTION TASKS PRIOR TO BOLTING			
1.	MANUFACTURER'S CERTIFICATIONS AVAILABLE FOR FASTENER MATERIALS	X	---
2.	FASTENERS MARKED IN ACCORDANCE WITH ASTM REQUIREMENTS	---	X
3.	CORRECT FASTENERS SELECTED FOR THE JOINT DETAIL (GRADE, TYPE, BOLT LENGTH) IF THREADS ARE TO BE EXCLUDED FROM SHEAR PLANE	---	X
4.	CORRECT BOLTING PROCEDURES SELECTED FOR JOINT DETAIL	---	X
5.	CONNECTING ELEMENTS, INCLUDING THE APPROPRIATE FAYING SURFACE CONDITION AND HOLE PREPARATION, IF SPECIFIED, MEET APPLICABLE REQUIREMENTS	---	X
6.	PRE-INSTALLATION VERIFICATION TESTING BY INSTALLATION PERSONNEL OBSERVED AND DOCUMENTED FOR FASTENER ASSEMBLIES AND METHODS USED	X	X
7.	PROTECTED STORAGE PROVIDED FOR BOLTS, NUTS, WASHERS AND OTHER FASTENER COMPONENTS	---	X

AISC 360-16, CHAPTER N SPECIAL INSPECTION REQUIREMENTS

		FREQUENCY OF INSPECTION	
		PERFORM	OBSERVE
AISC 360-16, TABLE N5.6.2 - INSPECTIONS DURING BOLTING			
1.	FASTENER ASSEMBLIES PLACED IN ALL HOLES AND WASHERS AND NUTS ARE POSITIONED AS REQUIRED	---	X
2.	JOINT BROUGHT TO THE SNUG-TIGHT CONDITION PRIOR TO THE PRETENSIONING OPERATION	---	X
3.	FASTENER COMPONENT NOT TURNED BY THE WRENCH PREVENTED FROM ROTATING	---	X
4.	FASTENERS ARE PRETENSIONED IN ACCORDANCE WITH THE RCSC SPECIFICATION, PROGRESSING SYSTEMATICALLY FROM THE MOST RIGID POINT TOWARD THE FREE EDGES	---	X
AISC 360-16, TABLE N5.6.3 - INSPECTIONS AFTER BOLTING			
1.	DOCUMENT ACCEPTANCE OR REJECTION OF BOLTED CONNECTIONS	X	---
<p>** PERFORM - PERFORM THESE TASKS FOR EACH BOLTED CONNECTION.</p> <p>** OBSERVE - OBSERVE THESE ITEMS ON A RANDOM BASIS. OPERATIONS NEED NOT BE DELAYED PENDING THESE INSPECTIONS.</p>			
N5.7 - OTHER INSPECTION TASKS			
1.	INSPECTION OF GALVANIZED STEEL STRUCTURAL MAIN MEMBERS EXPOSED CUT SURFACES OF GALVANIZED MAIN MEMBERS AND EXPOSED CORNERS OF HSS SHALL BE VISUALLY INSPECTED FOR CRACKS SUBSEQUENT TO GALVANIZING.	X	---
N5.8 - OTHER INSPECTION TASKS			
1.	INSPECT THE STEEL TO VERIFY COMPLIANCE WITH THE DETAILS SHOWN ON THE CONSTRUCTION DOCUMENTS.	X	---
2.	INSPECT THE PLACEMENT OF ANCHOR RODS AND OTHER EMBEDMENTS SUPPORTING STRUCTURAL STEEL FOR COMPLIANCE WITH THE CONSTRUCTION DOCUMENTS. THE DIAMETER, GRADE, TYPE AND LENGTH OF THE ANCHOR ROD OR EMBEDDED ITEM, AND THE EXTENT OR DEPTH OF EMBEDMENT INTO THE CONCRETE, SHALL BE VERIFIED AND DOCUMENTED PRIOR TO PLACEMENT OF CONCRETE	X	---
<p>** PERFORM - PERFORM THESE TASKS FOR EACH CONNECTION.</p> <p>** OBSERVE - OBSERVE THESE ITEMS ON A RANDOM BASIS. OPERATIONS NEED NOT BE DELAYED PENDING THESE INSPECTIONS.</p>			



Wallace Engineering
Structural Consultants, Inc.
Structural and Civil Consultants
123 N. Martin Luther King Jr. Blvd.
Tulsa, Oklahoma 74103
918.584.5858, 800.364.5858

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505.302.4472

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MO - CHIPMAN RD

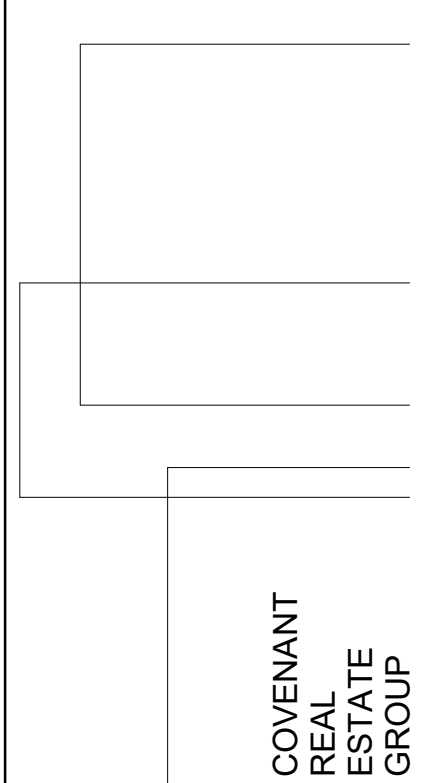
ADDRESS:
400 NW CHIPMAN RD
LEE'S SUMMIT, MO 64806

PROJECT NO: 267

MAIN CONTACT

CHRISTOPHER CLARK, AIA, NCARB
7701 E KELLOGG DR, STE 630
WICHITA, KS 67207
(316) 302-4472
chris@clarkitecture.net

DEVELOPER

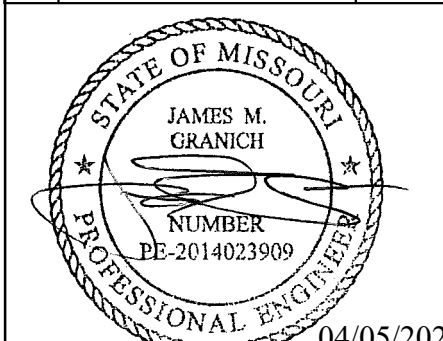


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04/05/2022
Missouri COA #001268

STRUCTURAL
SPECIAL
INSPECTIONS
(IBC 2018)

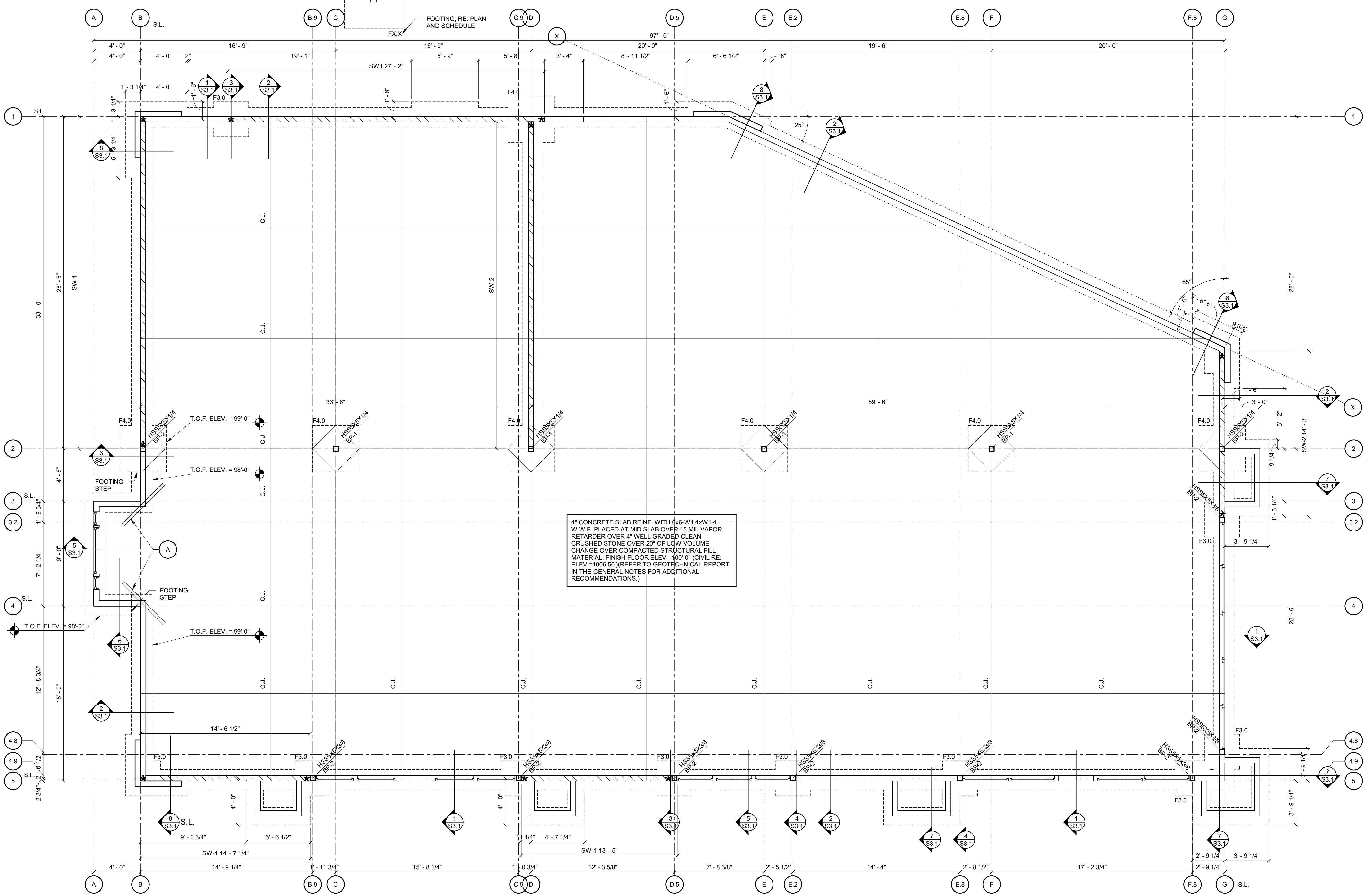
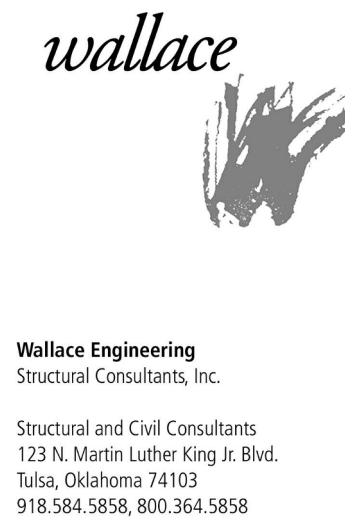
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FOOTING SCHEDULE								NOTES
MARK	LENGTH	WIDTH	THICKNESS	BOTTOM REINFORCING	TOP REINFORCING	LONGITUDINAL TRANSVERSE	LONGITUDINAL TRANSVERSE	
F3.0	3'-0"	3'-0"	2'-6"	(5) #5	(5) #5	(5) #5	(5) #5	
F4.0	4'-0"	4'-0"	2'-6"	(5) #5	(5) #5	(5) #5	(5) #5	

- WALL FRAMING NOTES:**
1. VERIFY ALL WALL OPENING AND INTERIOR WALL DIMENSIONS AND LOCATIONS WITH ARCHITECTURAL DRAWINGS.
 2. ALL EXTERIOR LOAD BEARING WALLS SHALL BE 2x6 AT 16", U.N.O.
 3. TOP PLATES SHALL BE (2) 2x6 AND BE SPLICED.
 4. NON-LOAD BEARING WALLS SHALL BE 2x4 MIN. RE: ARCH FOR WALL TYPES.
 5. RE: GENERAL NOTES FOR EXTERIOR WALL SHEATHING.
 6. RE: 5/S4.1 FOR SHEATHING AT SHEAR WALLS.
 7. RE: 1/S4.0 FOR TYPICAL NAILING SCHEDULE.

- LEGEND:**
- BP-X = BASE PLATE.
 - C.J. = CONTROL JOINT.
 - FX = FOOTING MARK RE: PLAN AND SCHEDULE.
 - S.L. = OUTSIDE FACE OF STUD.
 - ★ = HOLD DOWN ANCHOR, RE: 3/S4.1
 - SW-X = SHEAR WALL, RE: 5/S4.1
 - (A) = PROVIDE (2) #4 x5'-0" BARS AT MID DEPTH AT ALL RE-ENTRANT CORNERS

- FOUNDATION NOTES:**
1. THE CONCRETE SLABS SHOWN ON THE STRUCTURAL DRAWINGS HAVE BEEN DESIGNED FOR THE FINISHED STRUCTURE AND HAVE NOT BEEN DESIGNED FOR CONSTRUCTION CONSIDERATIONS. CONTRACTOR SHALL COORDINATE SLAB DESIGN WITH CONSTRUCTION NEEDS. THE SLAB DESIGN INDICATED ON THESE DRAWINGS IS CONSIDERED A MINIMUM. SUBMIT CHANGES TO THE SLAB DESIGN TO E.O.R. FOR REVIEW.
 2. CONTRACTOR SHALL SUBMIT ANY LIFT, CRANE OR OTHER CONSTRUCTION EQUIPMENT CUT SHEETS TO THE E.O.R. FOR INDEPENDENT ANALYSIS AT NO COST TO THE OWNER FOR THE USE OF SUCH EQUIPMENT ON THE SLAB.
 3. TOP OF FOOTING ELEVATION = 1'-0" BELOW T.O. SLAB UNLESS NOTED OTHERWISE.
 4. ALL PIPING OR CONDUITS THAT OCCUR THROUGH OR UNDER A GRADE BEAM OR FOOTING SHALL BE APPROVED BY THE ENGINEER PRIOR TO PLACEMENT (RE: 7/S3.0 AND RE: 8/S3.0).
 5. RE: 1/S3.0 FOR REINFORCING LAP SCHEDULE.
 6. PROVIDE CORNER BARS IN FOUNDATIONS, RE: 9/S3.0.
 7. CONTRACTOR SHALL COORDINATE SHEARWALL POST ANCHOR BOLT EMBEDMENTS WITH THE FOUNDATIONS PRIOR TO POURING.
 8. RE: ARCH/MEP DRAWINGS FOR LOCATIONS OF FLOOR DRAINS.
 9. FOOTING STEP, RE: 6/S3.0



1 FOUNDATION PLAN
1/4" = 1'-0"

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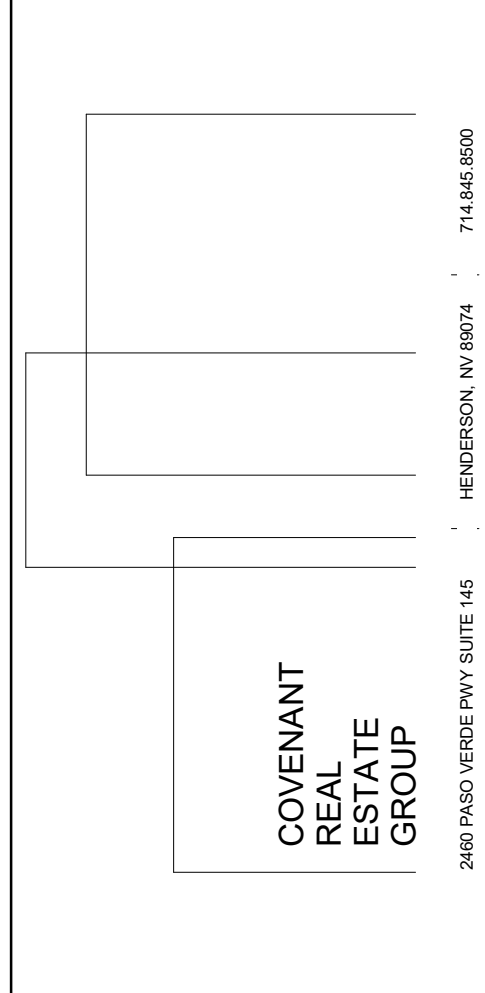
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400 NW CHIPMAN RD
LEE'S SUMMIT, MO 64806

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MAIN CONTACT

CHRISTOPHER CLARK, AIA, NCARB
7701 E KELLOGG DR, STE 630
WICHITA, KS 67207
(316) 302-4472
chris@clarkitcture.net

DEVELOPER



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FOUNDATION PLAN

S1.0

ROOF FRAMING NOTES:

1. RE: ARCH/MEP DRAWINGS FOR LOCATIONS OF ROOFTOP OPENINGS AND EQUIPMENT.
2. TRUSS BEARING ELEVATION VARIES, RE: PLAN
3. ACTUAL TRUSS LAYOUT TO BE DETERMINED BY THE TRUSS MFR. PROVIDE LAYOUT AS REQUIRED TO MATCH ARCH. ROOF LINES. TRUSS SPACING SHALL NOT EXCEED 24" O.C., RE: S001 FOR PREFABRICATED WOOD TRUSS INFORMATION AND DESIGN LOADINGS AND RE: 6/14/1 FOR SNOW DRIFT.
4. RE: GENERAL NOTES FOR ROOF SHEATHING, RE: 4/S4.1 FOR NAILING DIAGRAM.
5. ROOF ACCESS LADDER SHALL BE PER THE GENERAL CONTRACTOR

WALL FRAMING NOTES:

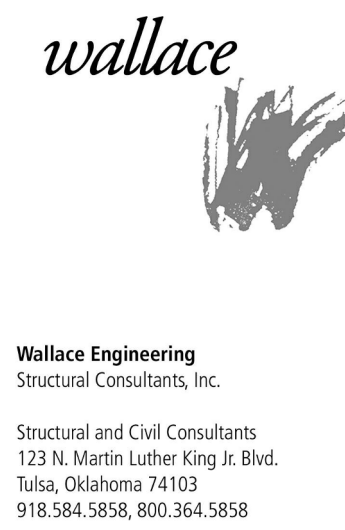
1. VERIFY ALL WALL OPENING AND WALL DIMENSIONS AND LOCATIONS WITH ARCHITECTURAL DRAWINGS
2. ALL EXTERIOR LOAD BEARING WALLS SHALL BE 2x6 AT 16", U.N.O. CENTER STUDS UNDER EA. TRUSS, RE: 14/S4.0
3. TOP PLATES SHALL BE (2) 2x6 AND BE SPLICED.
4. NON-LOAD BEARING WALLS SHALL BE 2x4, RE: ARCH FOR WALL TYPES
5. RE: GENERAL NOTES FOR EXTERIOR WALL SHEATHING
6. RE: 5/S4.1 FOR SHEATHING AT SHEAR WALLS
7. RE: 1/S4.0 FOR TYPICAL NAILING SCHEDULE
8. TRUSS SUPPLIER SHALL COORDINATE TRUSS DESIGN WITH MEP EQUIPMENT

PLAN REFERENCE NOTES:

- (A) PROVIDE FLAT 2x BLOCKING BETWEEN TRUSSES AND ATTACH ROOF SHEATHING WITH 10d NAILS AT 6" O.C. INSTALL CONT SIMPSON CMST14 COIL STRAP OVER BLOCKING.
- (B) TRUSS SUPPLIER TO DESIGN SHEAR PANEL BLOCKING TO TRANSFER AXIAL TENSION/COMPRESSION (PLF) OF 305 PLF (0.6W/0.7E) FOR THE LENGTH OF THE SHEAR WALL BELOW.
- (C) TRUSS SUPPLIER TO DESIGN SHEAR PANEL BLOCKING TO TRANSFER AXIAL TENSION/COMPRESSION (PLF) OF 255 PLF (0.6W/0.7E) FOR THE LENGTH OF THE SHEAR WALL BELOW
- (D) TRUSS SUPPLIER TO DESIGN SHEAR PANEL BLOCKING TO TRANSFER AXIAL TENSION/COMPRESSION (PLF) OF 90 PLF (0.6W/0.7E) FOR THE LENGTH OF THE SHEAR WALL BELOW
- (E) CANOPY, RE: ARCH FOR EXTENTS, RE: GC FOR DESIGN AND ATTACHMENT
- (F) PARAPET CANOPY SHALL BE INTEGRAL WITH THE ROOF TRUSS FRAMING
- (G) TRUSS SUPPLIER SHALL PROVIDE EXTERIOR TRUSSES ATTACHED TO OUTSIDE ENDS OF MAIN TRUSSES AND CANTILEVERED OUT TO SUPPORT CORNER TRUSSES

LEGEND:

- H# = HEADER MARK, RE: 13/S4.0
- T# = TRUSS MARK, RE: 6/S4.1
- SW-x = SHEAR WALL, RE: 5/S4.1



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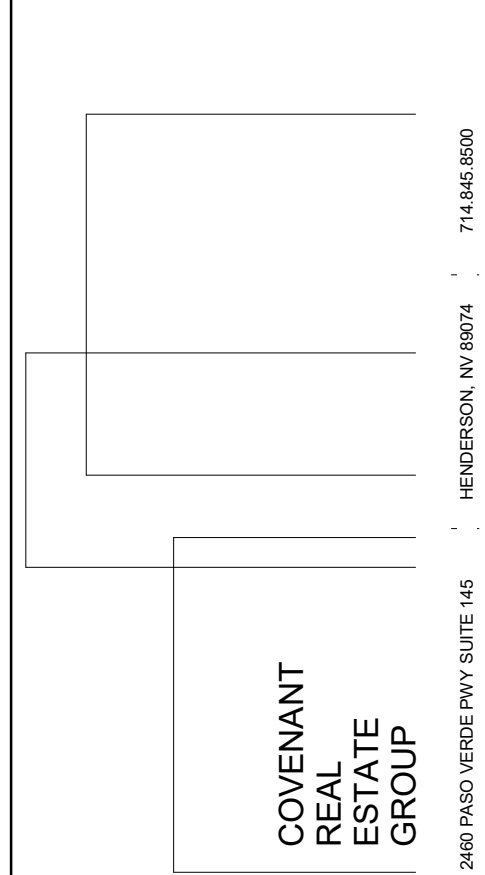
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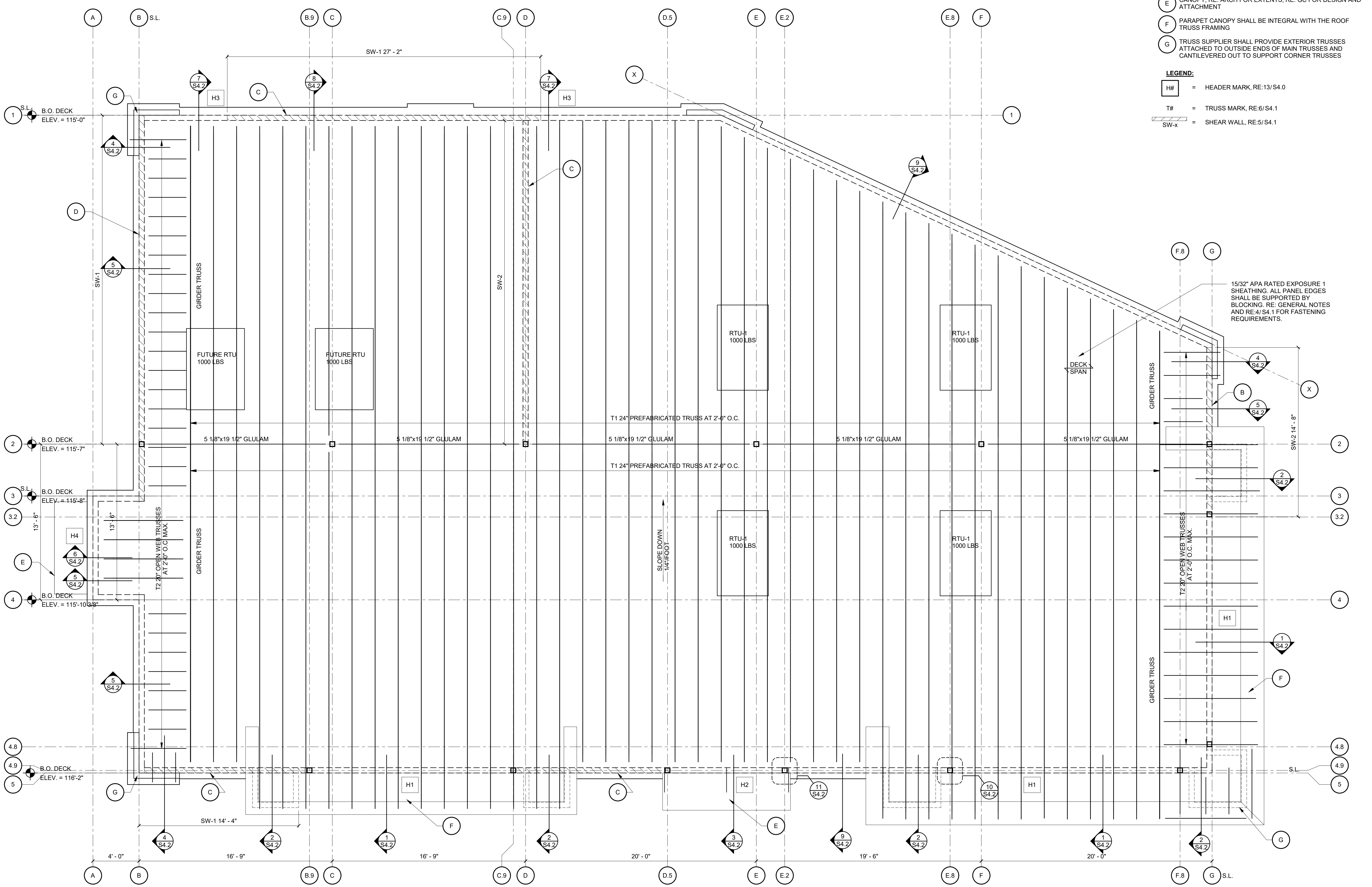
REVISION SCHEDULE

NO	DESCRIPTION	DATE

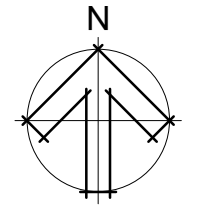


FRAMING PLAN

S2.0



1 ROOF FRAMING PLAN
1/4" = 1'-0"



PROJECT INFO

CLIENT:
COVENANT GROUP, LLC

PROJECT:
BUILDING SHELL - LEE'S SUMMIT,
MO - CHIPMAN RD

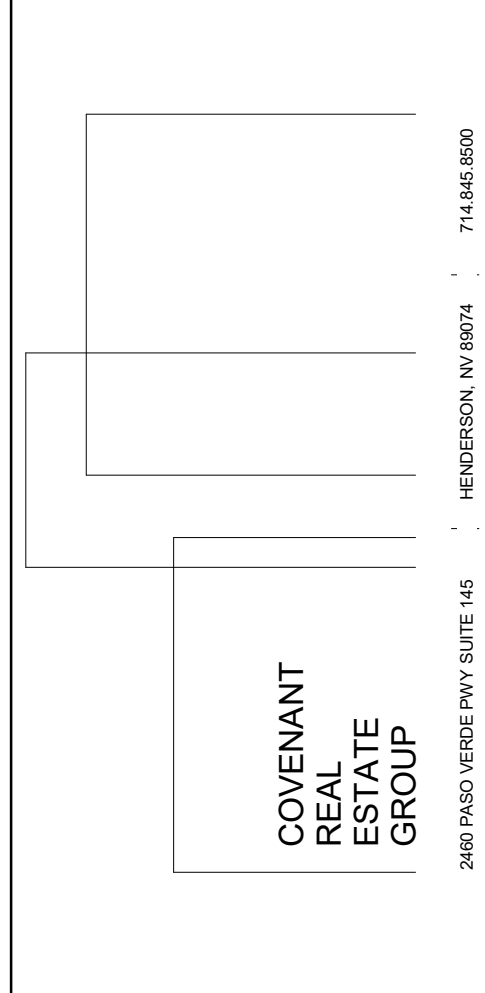
ADDRESS:
400 NW CHIPMAN RD
LEE'S SUMMIT, MO 64806

PROJECT NO: 287

MAIN CONTACT

CHRISTOPHER CLARK, AIA, NCARB
7701 E KELLOGG DR, STE 630
WICHITA, KS 67207
(316) 302-4472
chris@clarkitecture.net

DEVELOPER



SHEET INFO

ISSUE DATE : 03/31/2022
ISSUED FOR: PERMIT SET

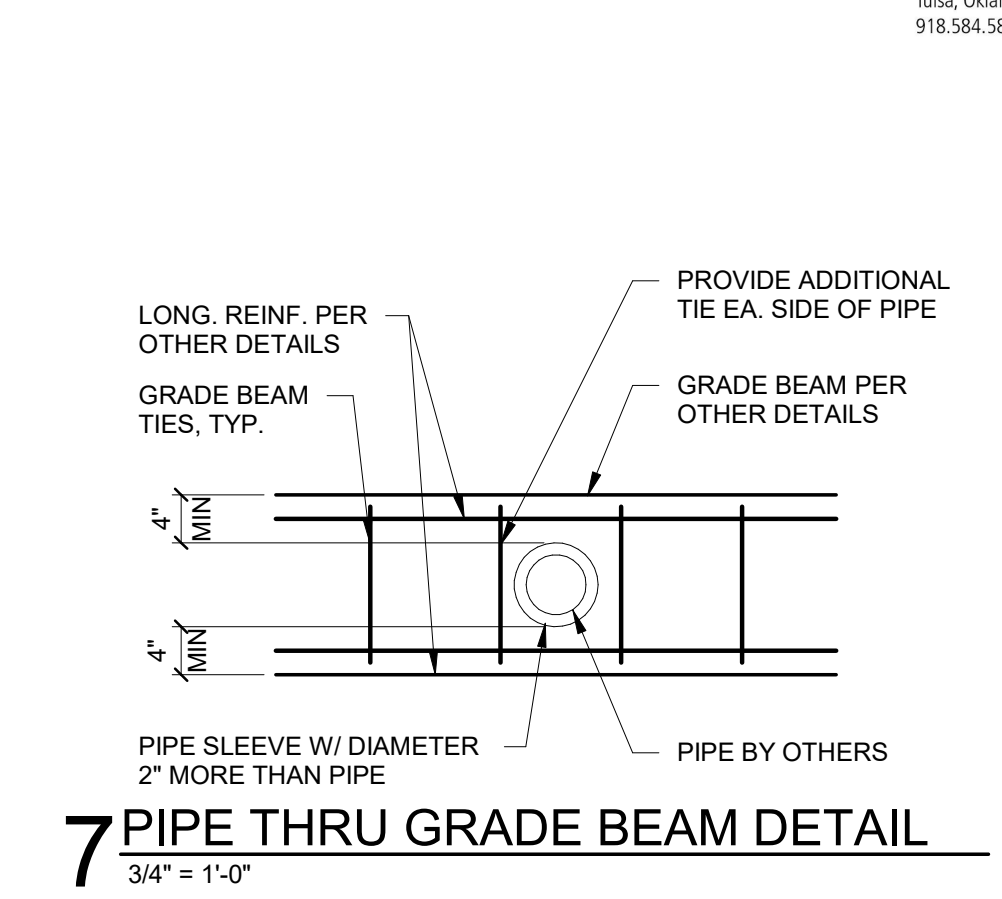
REVISION SCHEDULE

NO	DESCRIPTION	DATE

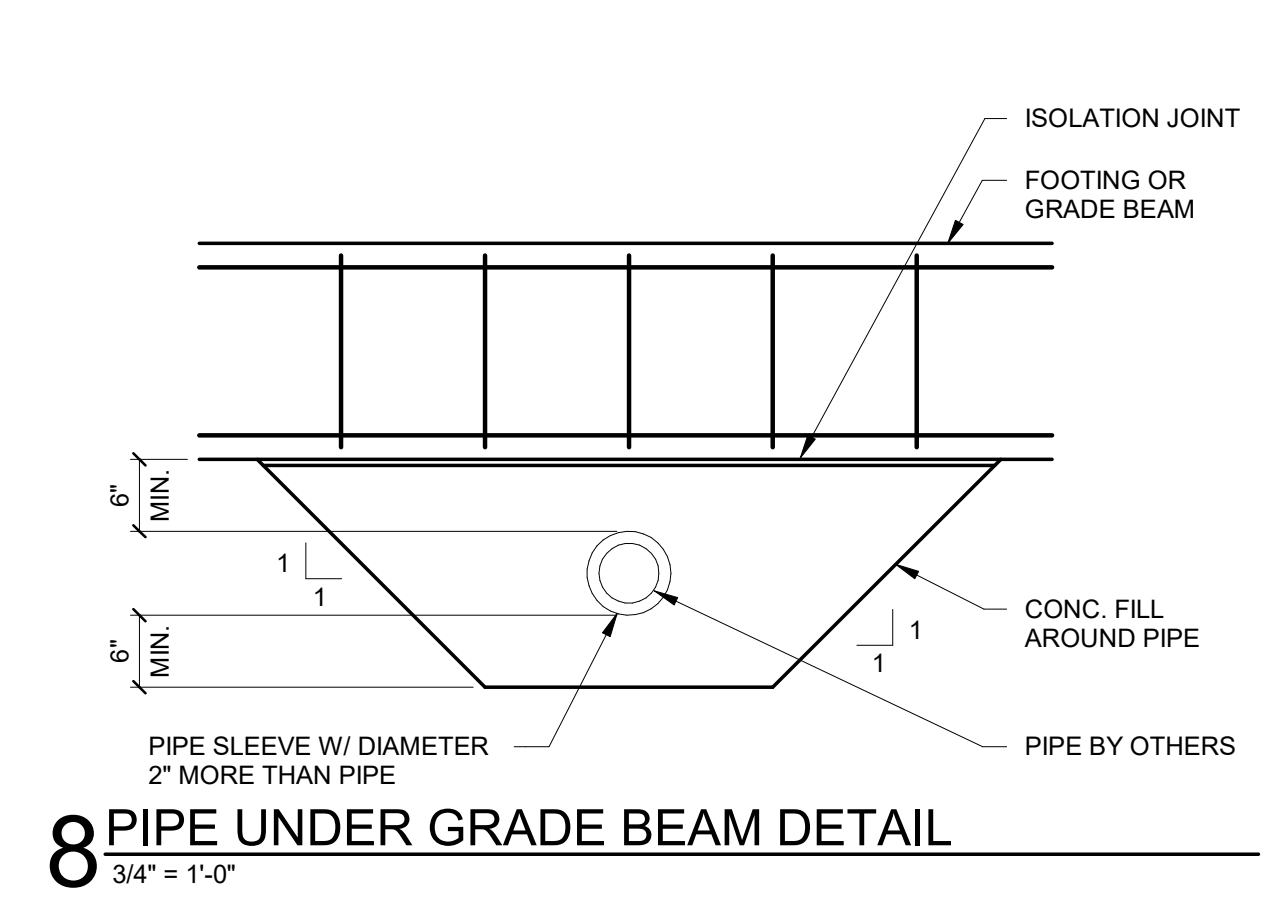


FOUNDATION DETAILS

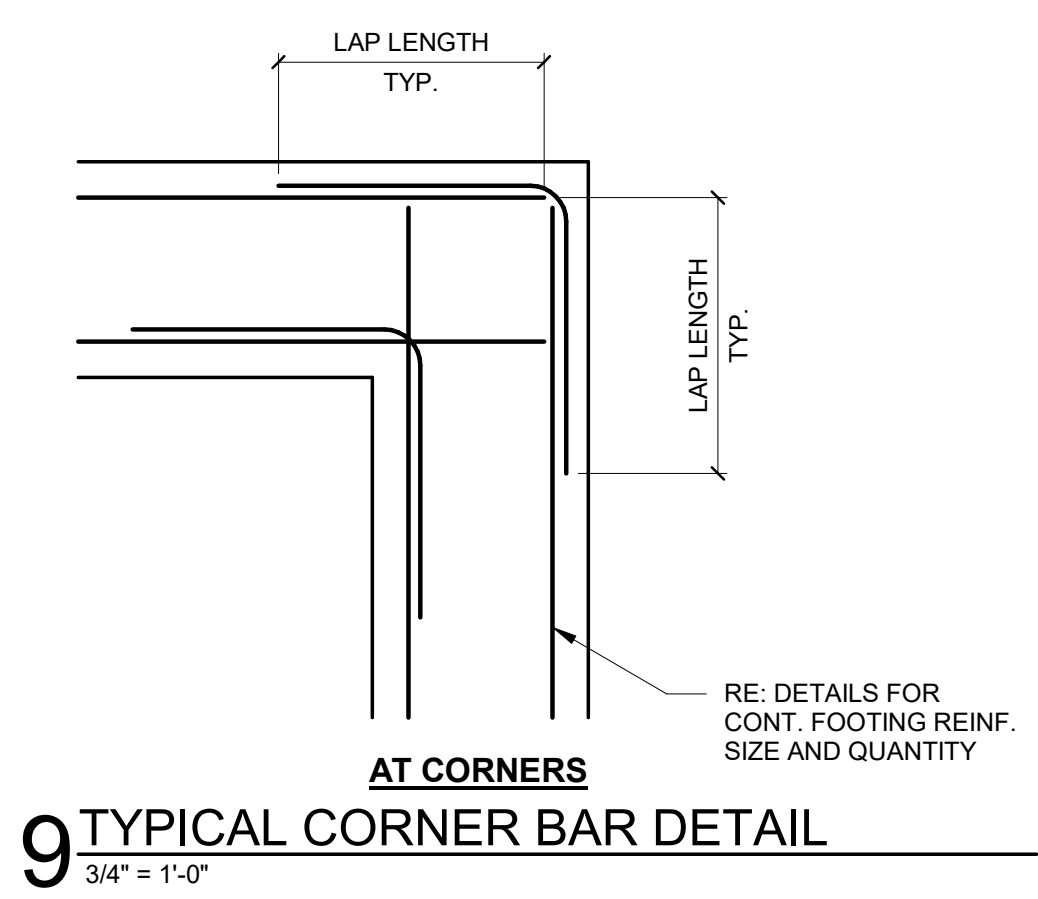
S3.0



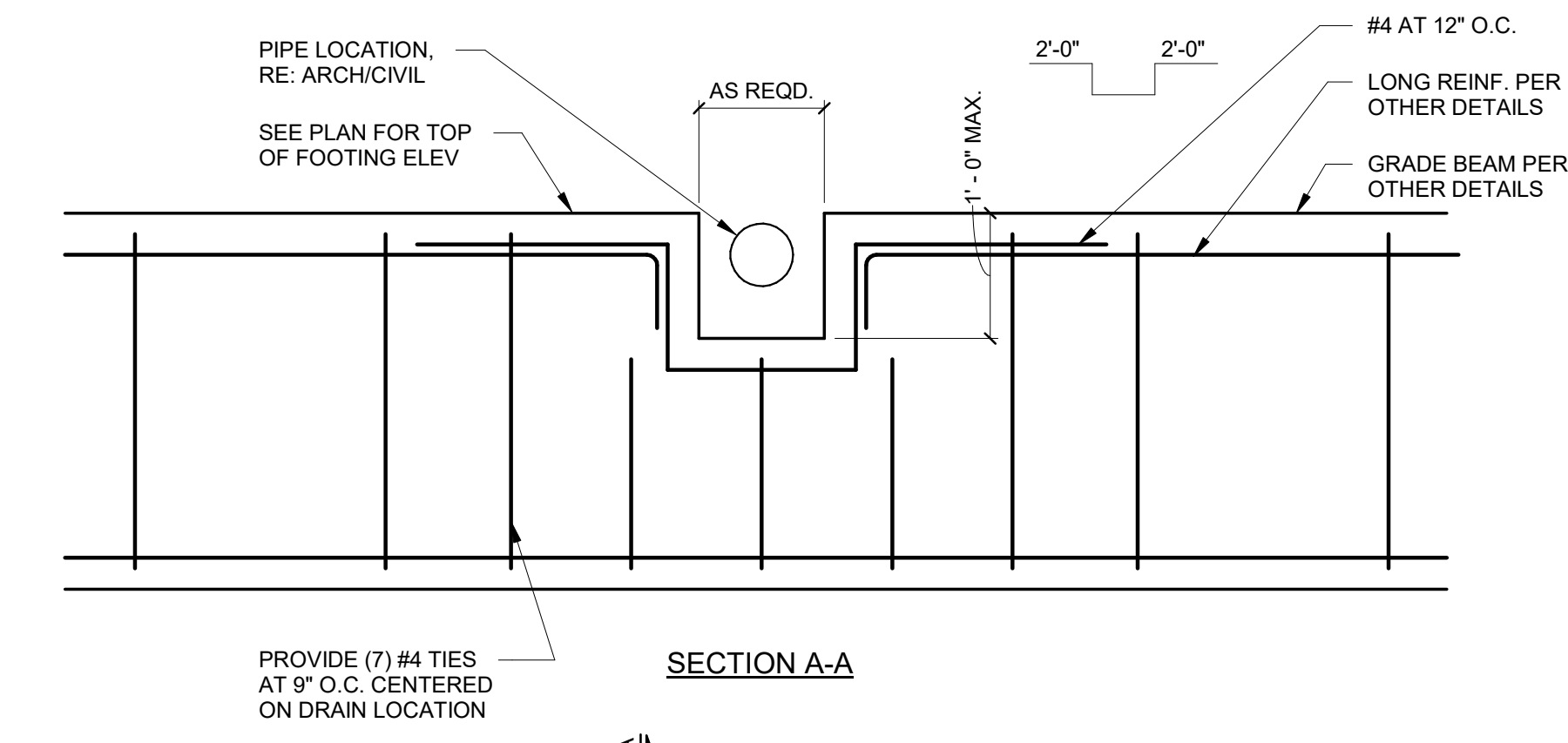
7 PIPE THRU GRADE BEAM DETAIL
3/4" = 1'-0"



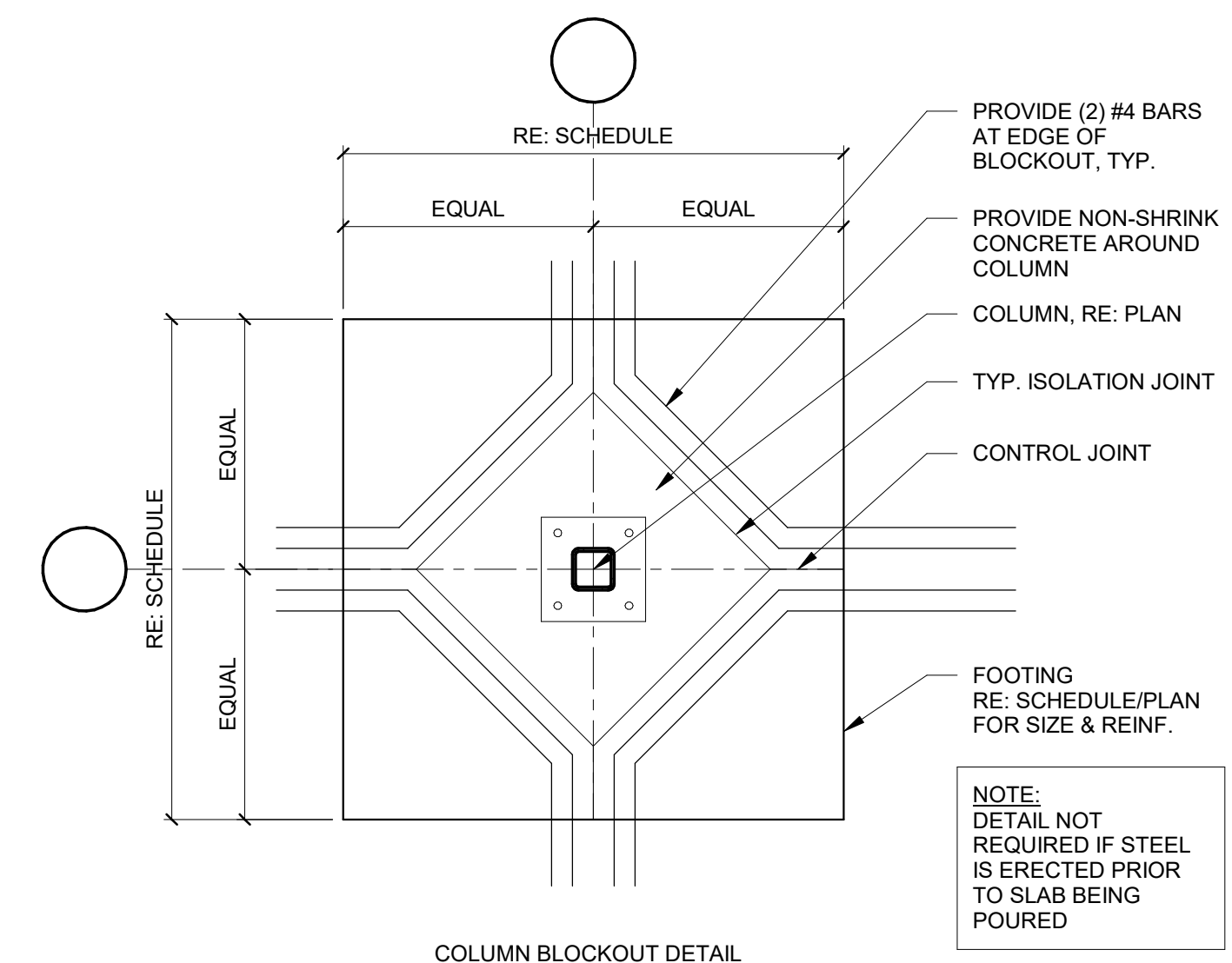
8 PIPE UNDER GRADE BEAM DETAIL
3/4" = 1'-0"



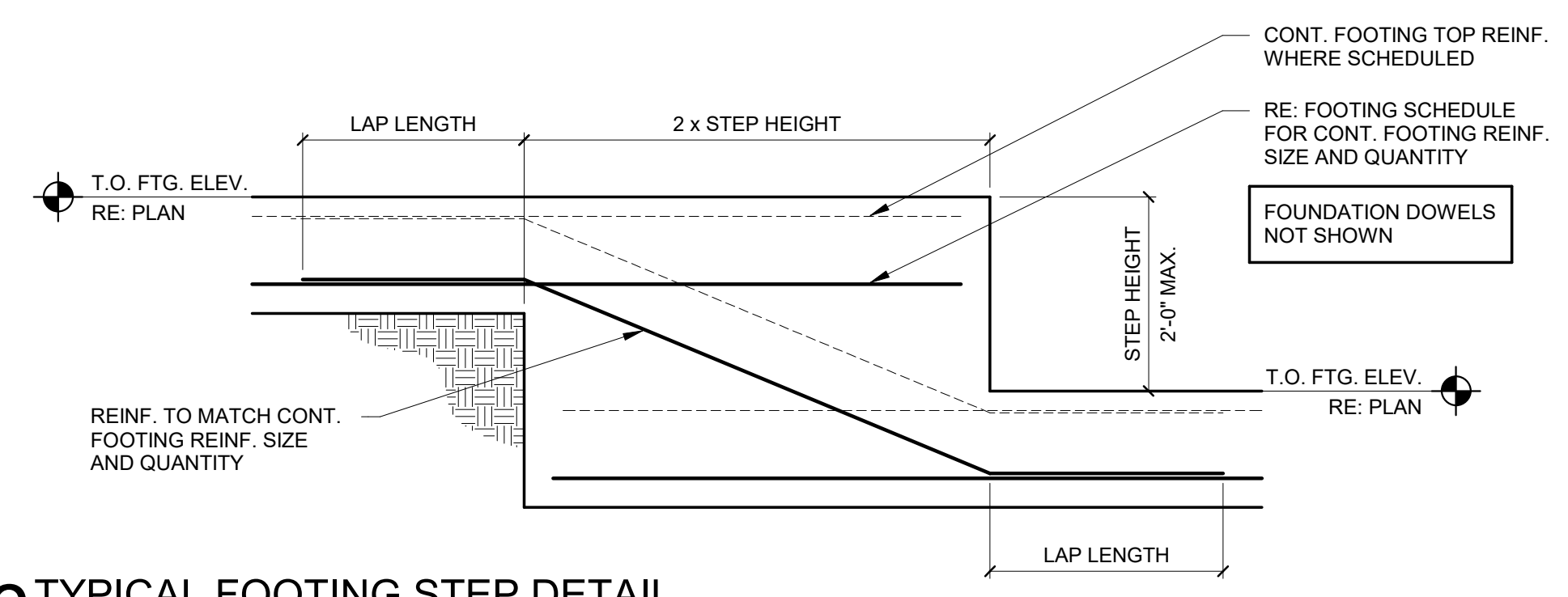
9 TYPICAL CORNER BAR DETAIL
3/4" = 1'-0"



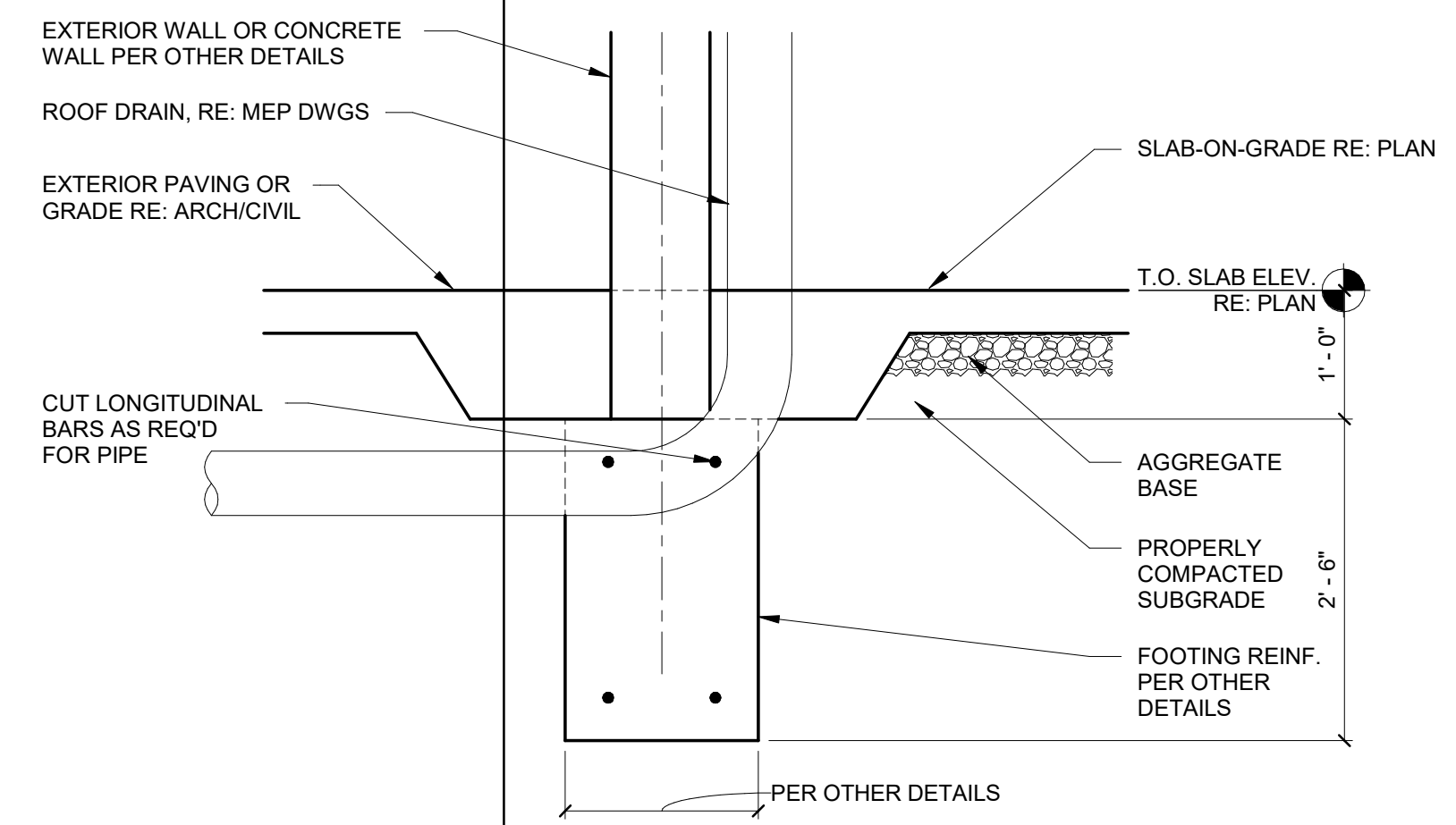
4 FOUNDATION SECTION AT DRAIN
3/4" = 1'-0"



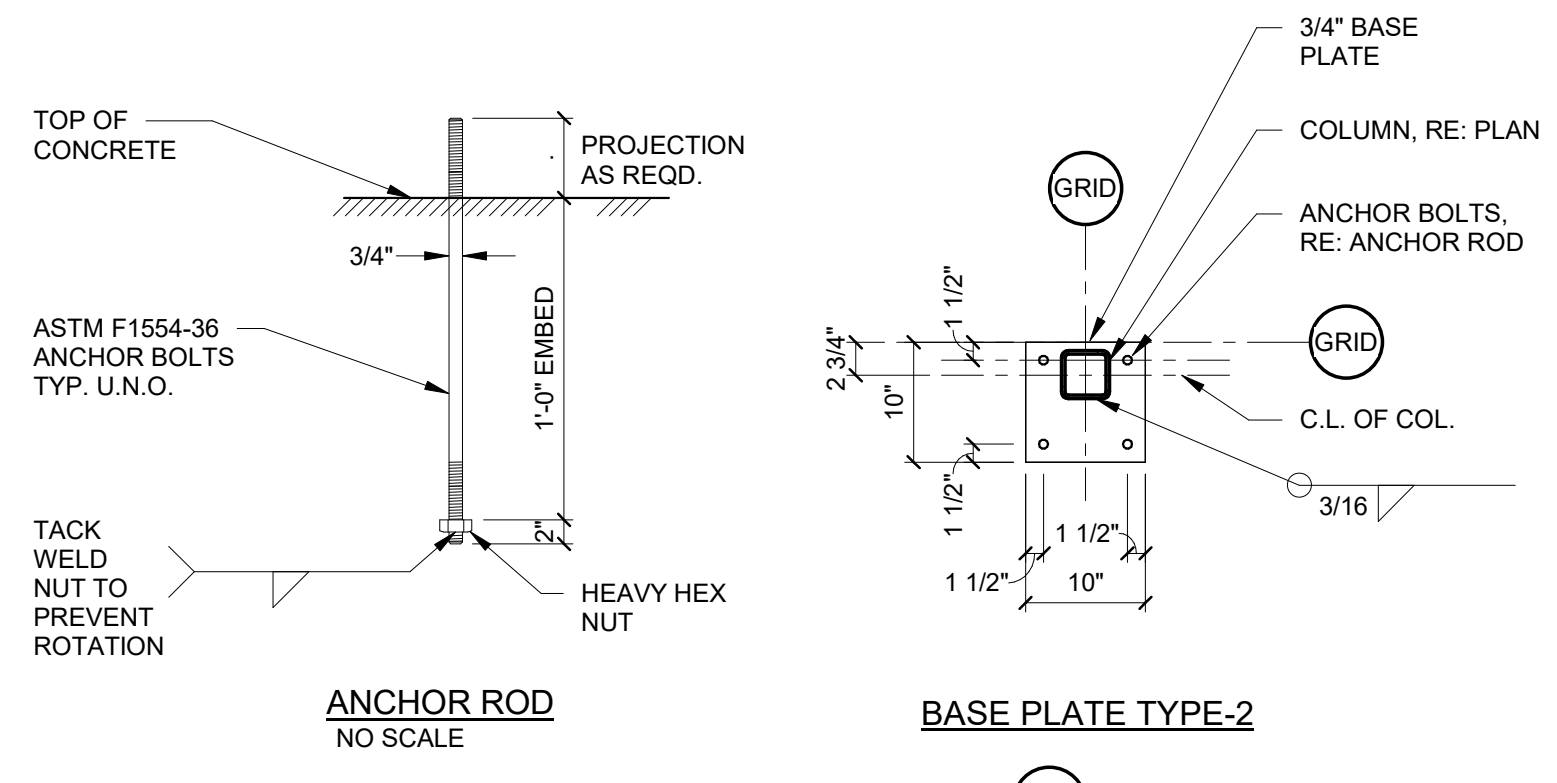
5 COLUMN BLOCKOUT DETAIL
3/4" = 1'-0"



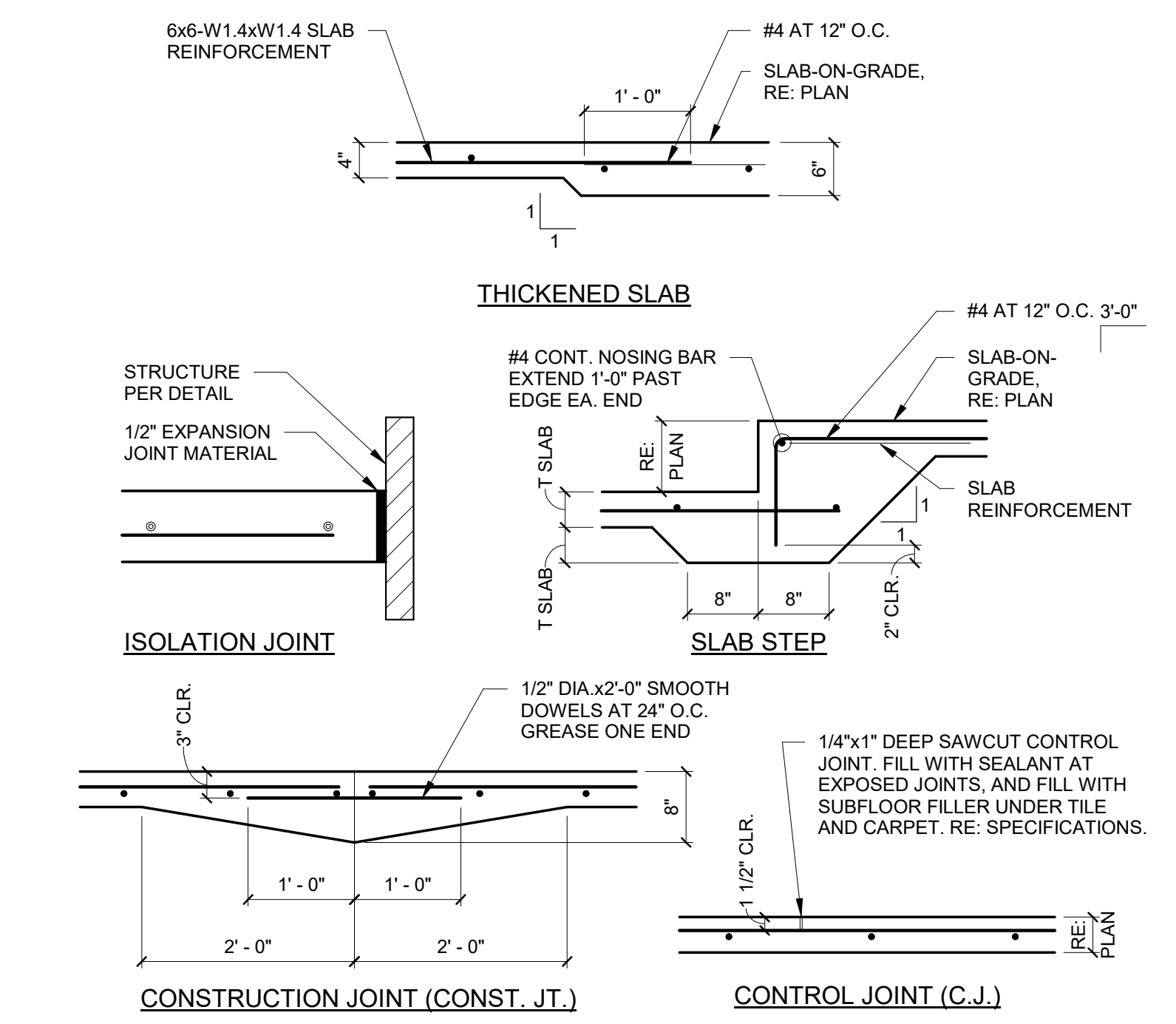
6 TYPICAL FOOTING STEP DETAIL
3/4" = 1'-0"



4 FOUNDATION SECTION AT DRAIN
3/4" = 1'-0"



2 ANCHOR ROD AND BASE PLATE DIAGRAMS
3/4" = 1'-0"

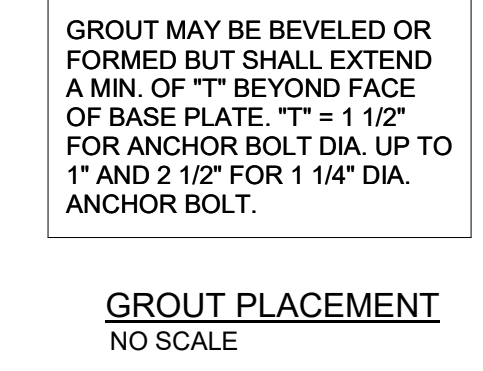


3 SLAB-ON-GRADE JOINT DETAILS
3/4" = 1'-0"

BAR SIZE	CONCRETE REINFORCING LAP LENGTH SCHEDULE					
	STRUCTURAL ELEMENT MINIMUM COMPRESSIVE STRENGTH (f _c)					
	3000psi		4000psi		4500psi	
	TOP BARS	OTHER	TOP BARS	OTHER	TOP BARS	OTHER
#3	28"	22"	25"	19"	23"	18"
#4	38"	29"	33"	25"	31"	24"
#5	47"	36"	41"	31"	38"	30"
#6	56"	43"	49"	37"	46"	35"
#7	81"	63"	71"	54"	67"	51"
#8	93"	72"	81"	62"	76"	59"
#9	105"	81"	91"	70"	86"	66"
#10	118"	91"	102"	79"	96"	74"

NOTES:
1. LAP LENGTH FOR TOP BARS SHALL BE USED WHEN MORE THAN 12 INCHES OF FRESH CONCRETE IS PLACED BELOW HORIZONTAL REINFORCEMENT.

1 CONCRETE REINFORCING LAP SCHEDULE
3/4" = 1'-0"



2 ANCHOR ROD AND BASE PLATE DIAGRAMS
3/4" = 1'-0"

PROJECT INFO

CLIENT:
COVENANT GROUP, LLC

PROJECT:
BUILDING SHELL - LEE'S SUMMIT,
MO - CHIPMAN RD

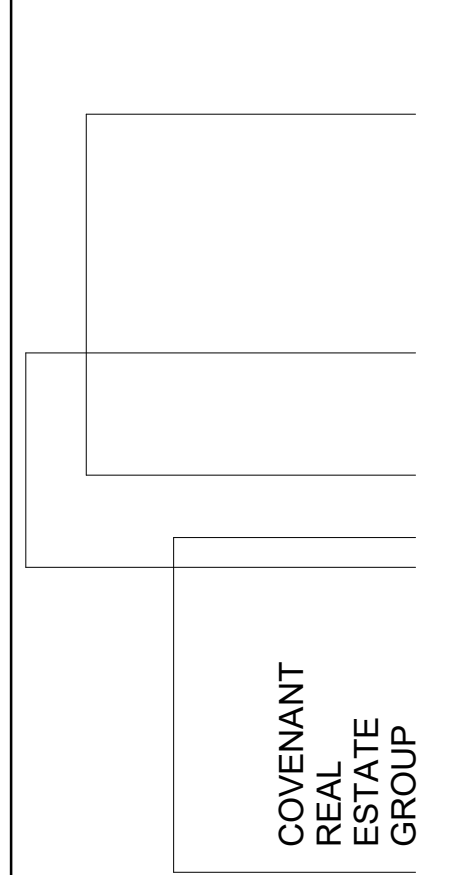
ADDRESS:
400 NW CHIPMAN RD
LEE'S SUMMIT, MO 64896

PROJECT NO: 287

MAIN CONTACT

CHRISTOPHER CLARK, AIA, NCARB
7701 E KELLOGG DR, STE 630
WICHITA, KS 67207
(316) 302-4472
chris@clarkitecture.net

DEVELOPER



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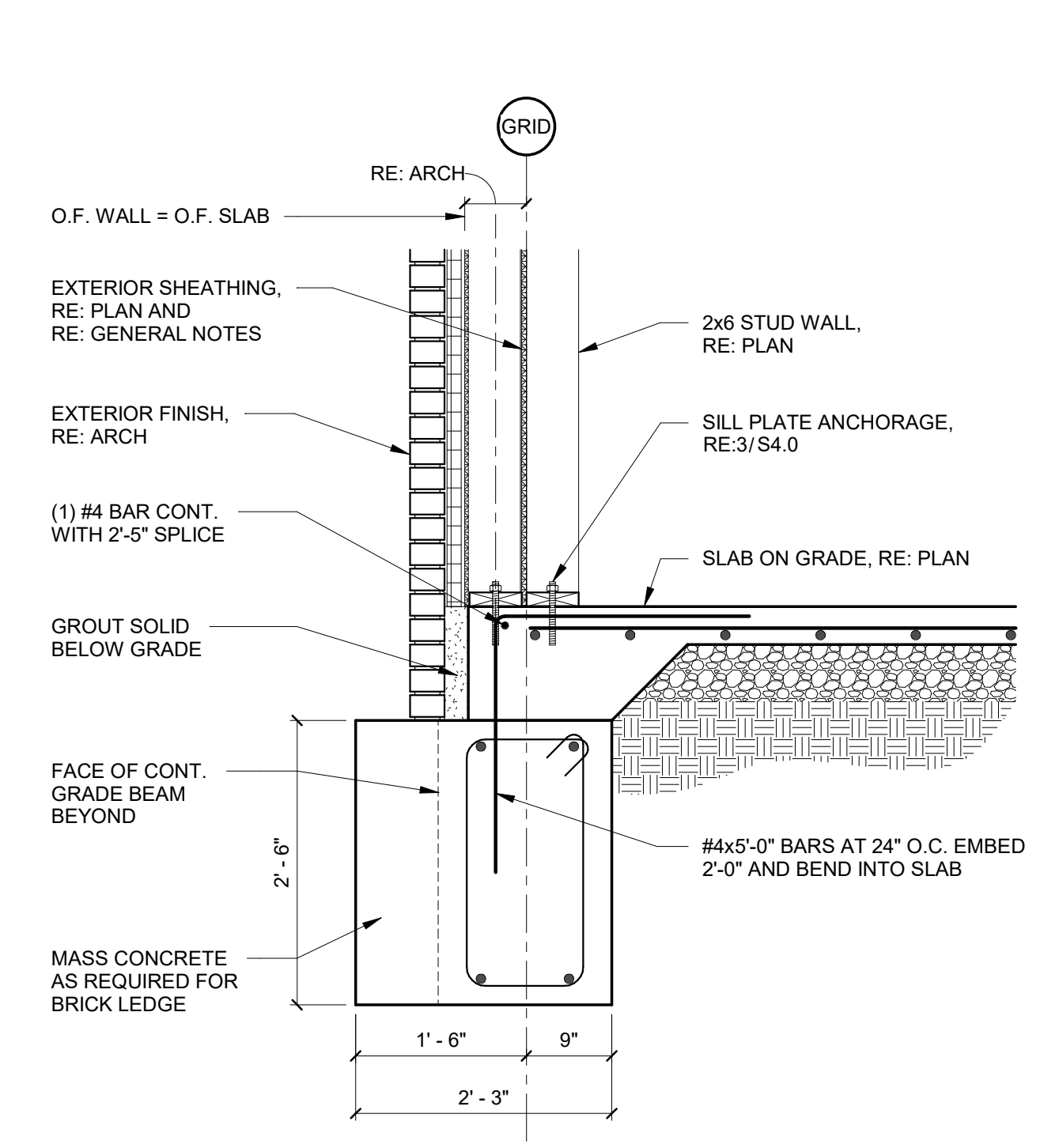
REVISION SCHEDULE

NO	DESCRIPTION	DATE

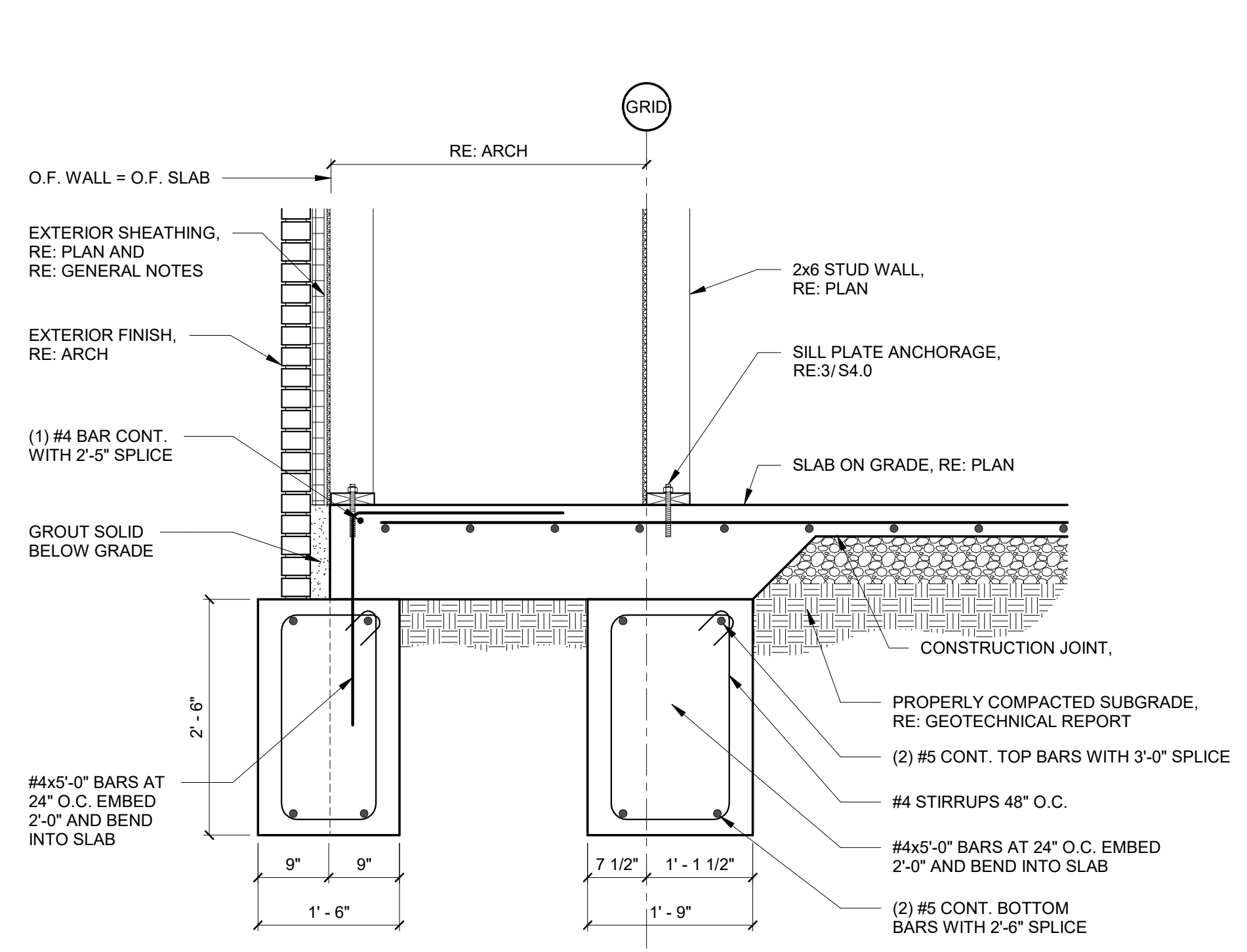


FOUNDATION DETAILS

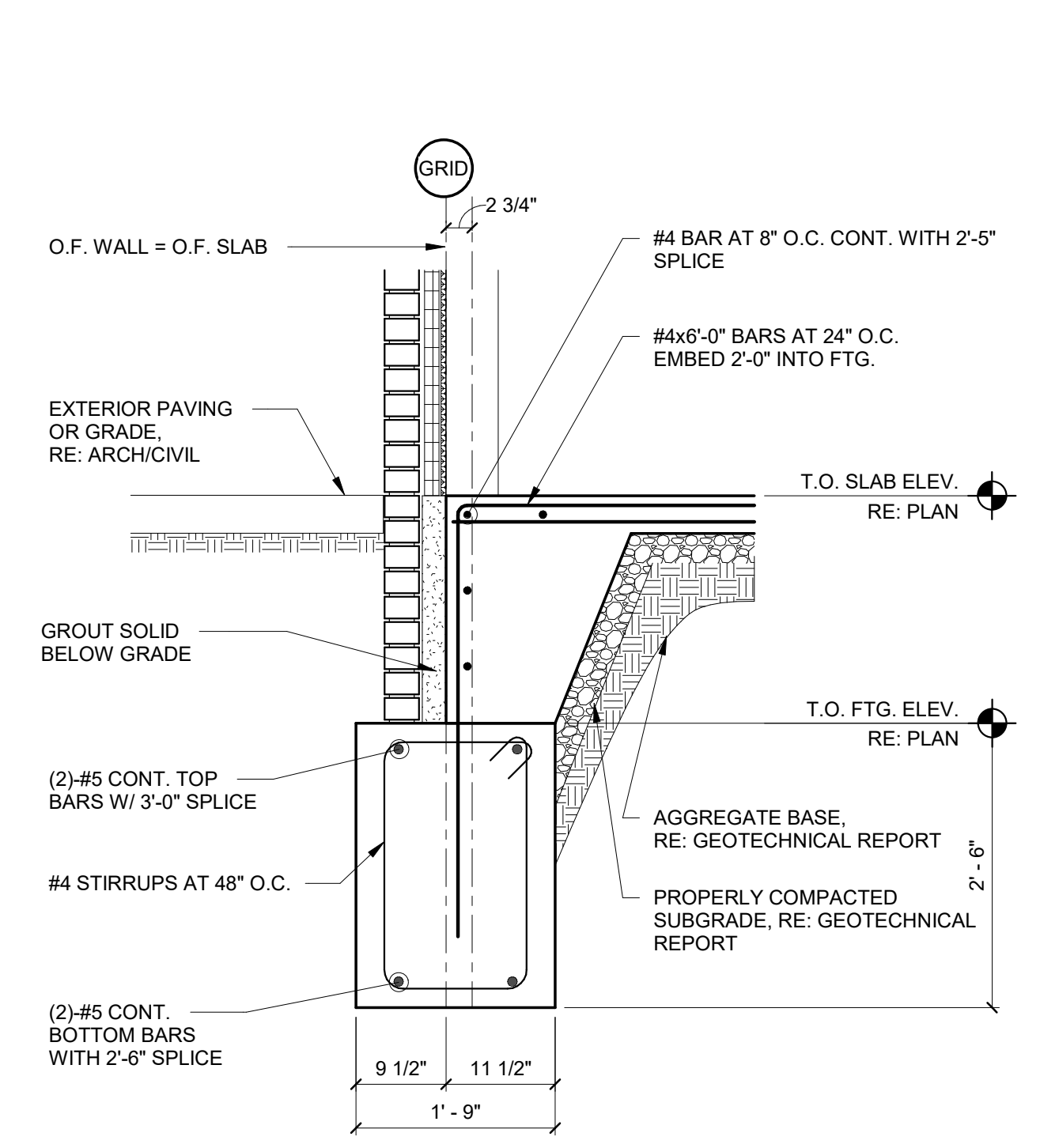
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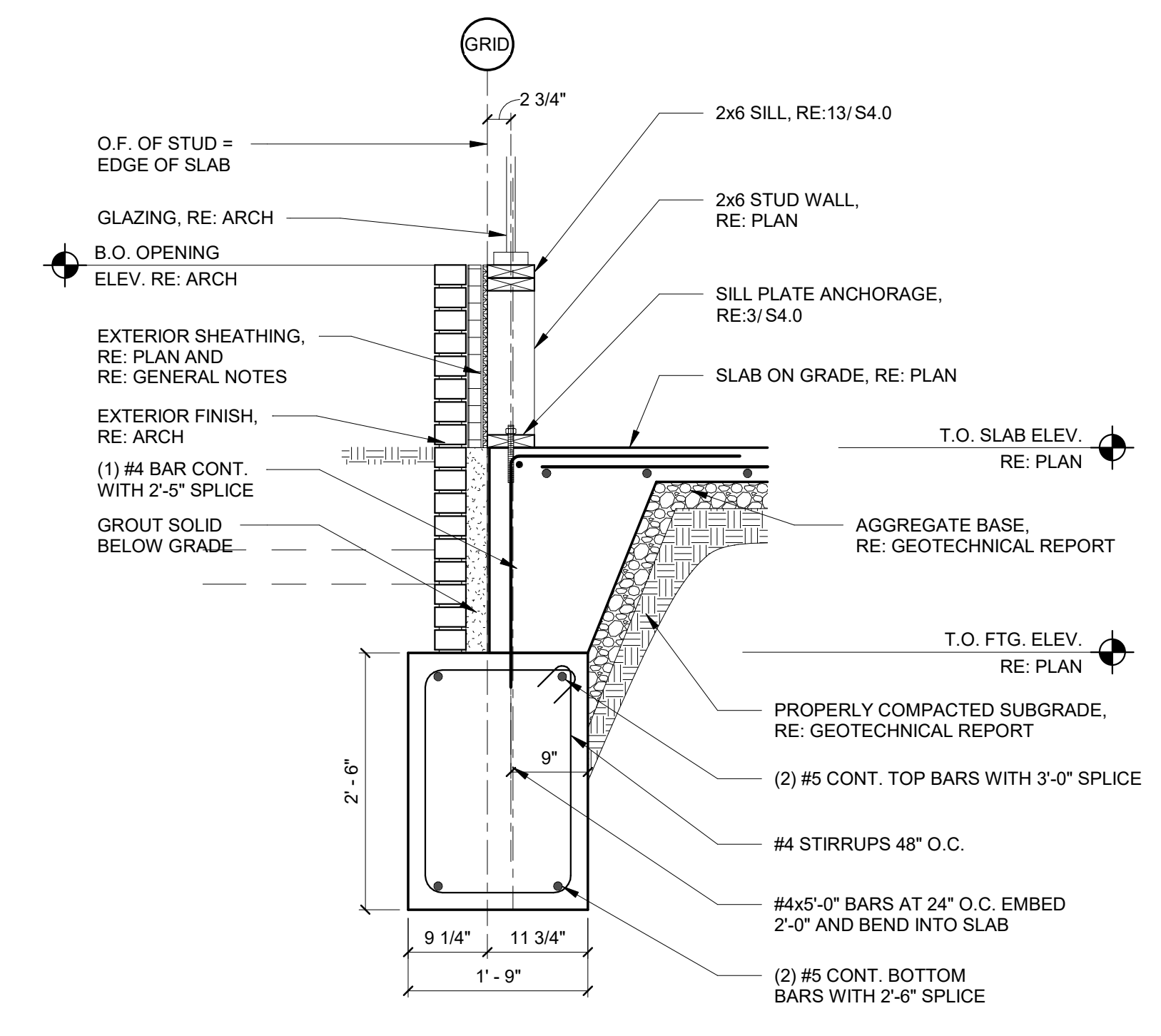
8 THICKENED FOOTING SECTION
3/4" = 1'-0"



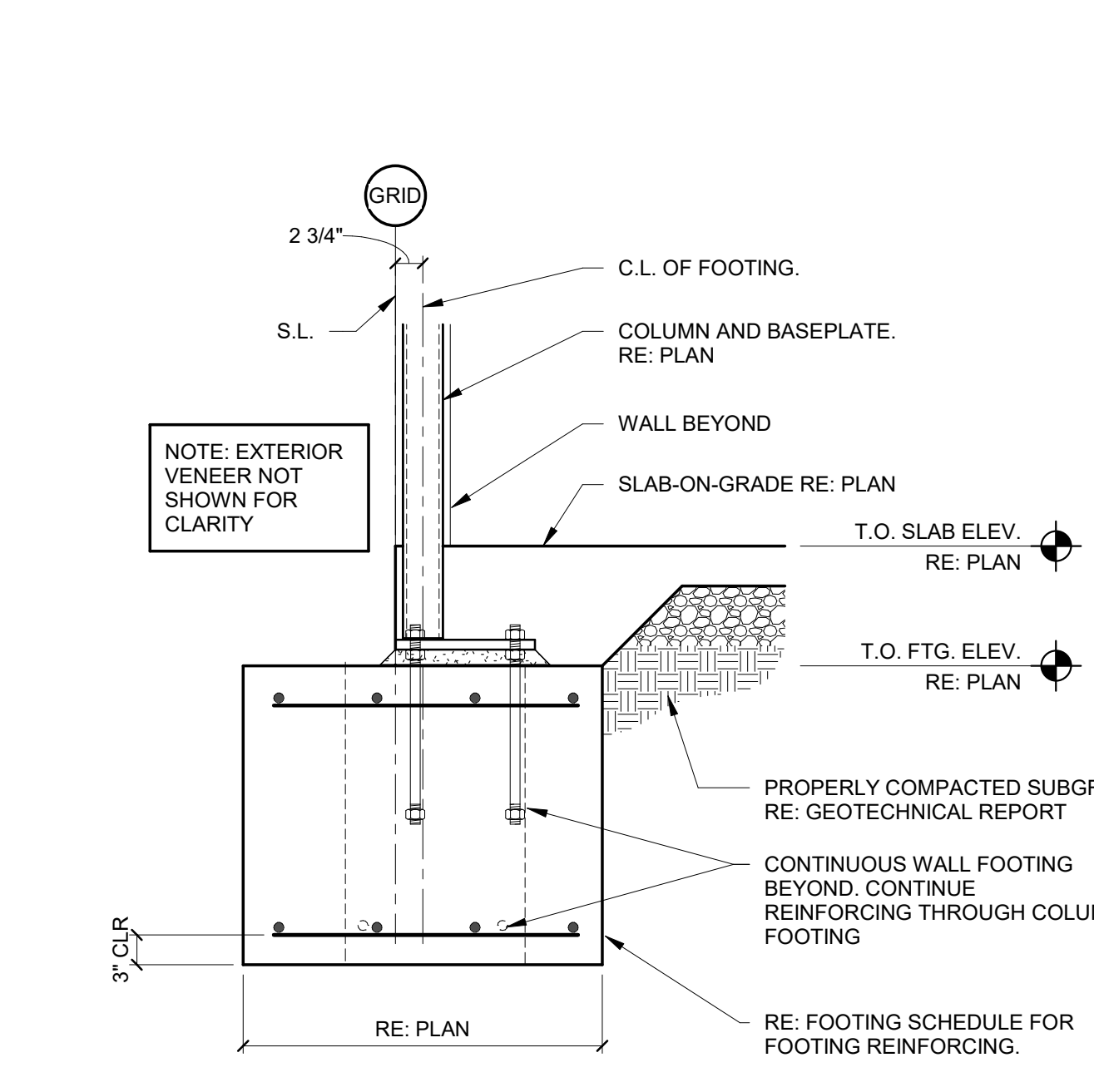
7 PILASTER SECTION
3/4" = 1'-0"



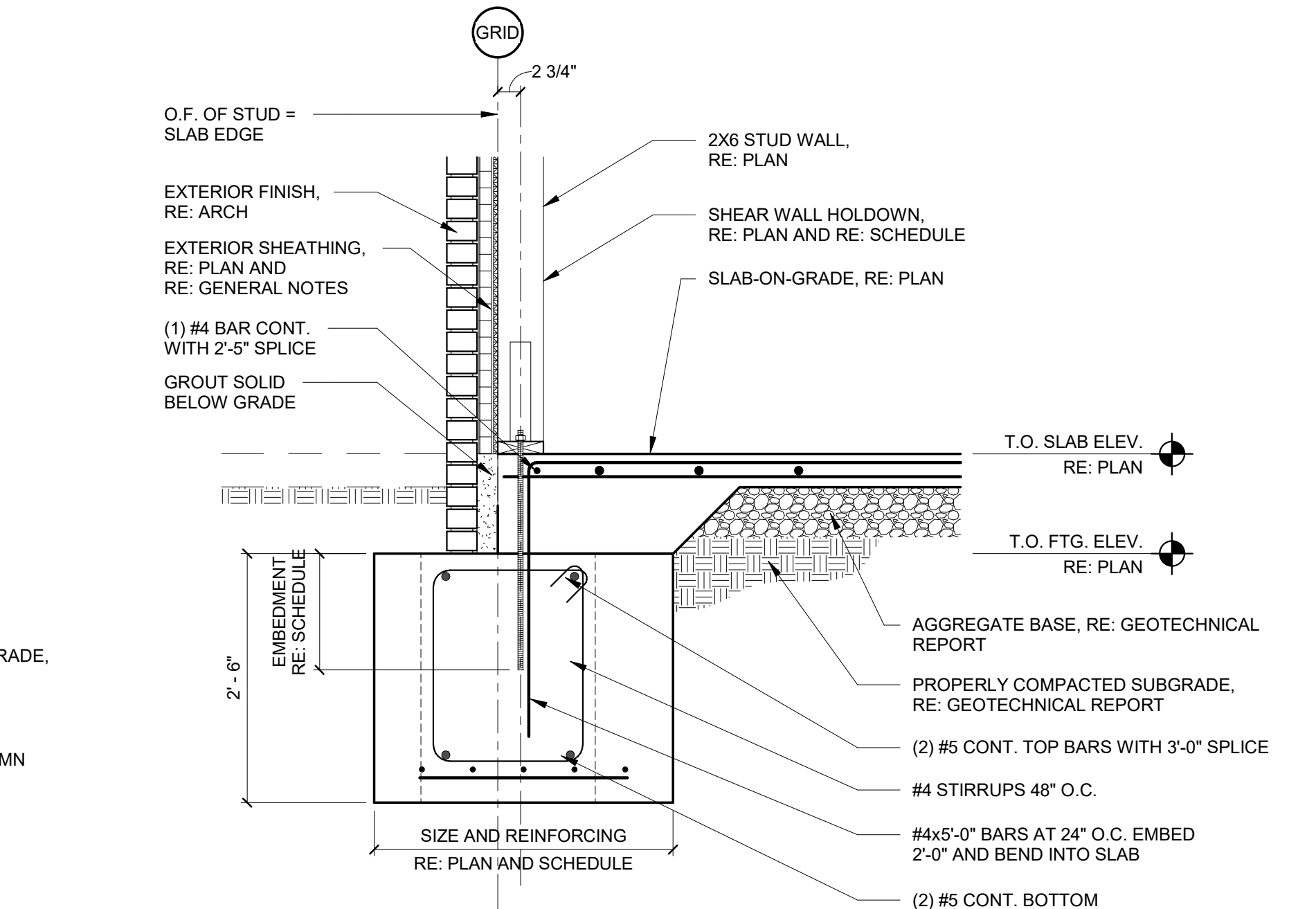
6 FOUNDATION SECTION
3/4" = 1'-0"



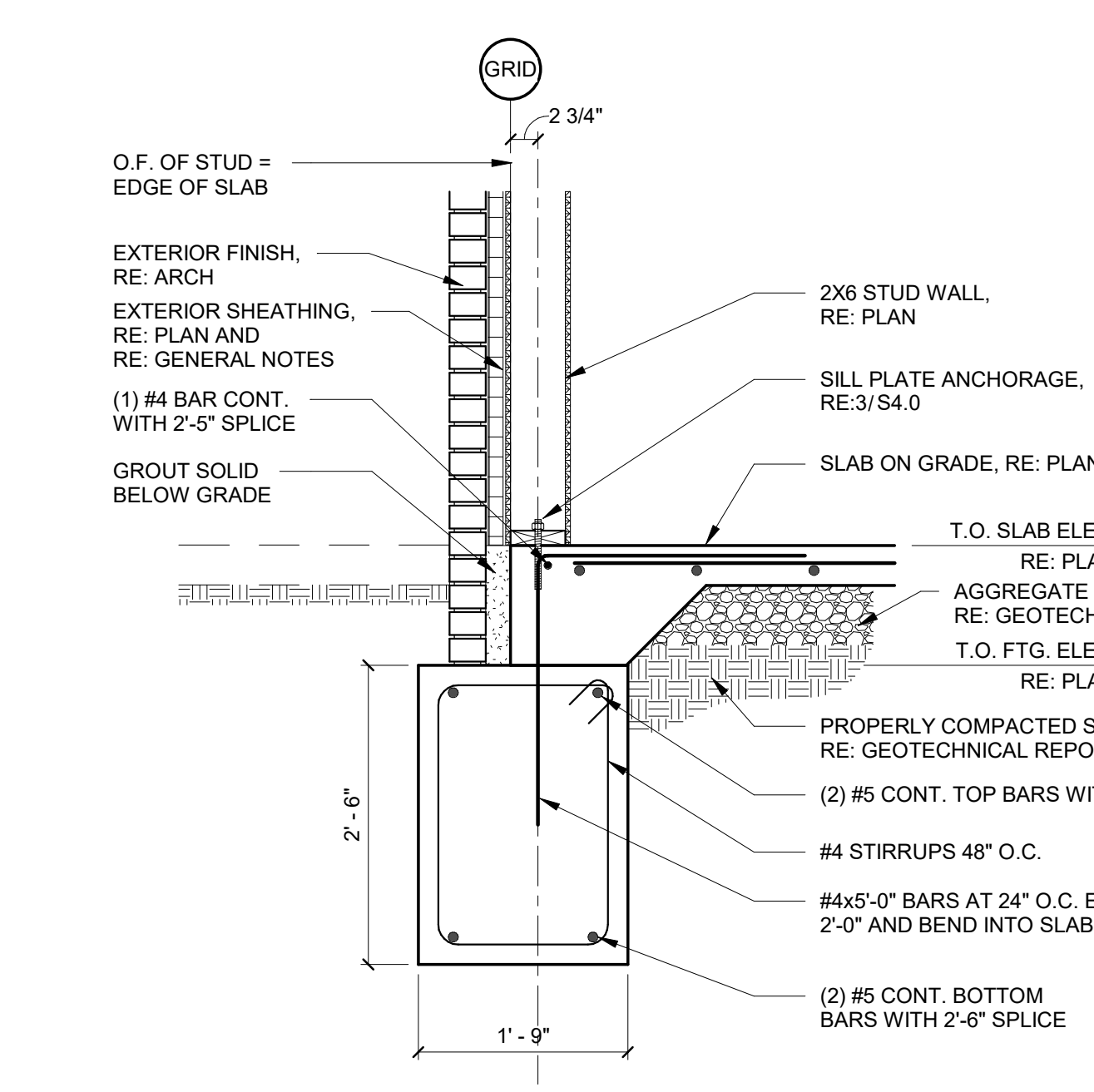
5 GRADE BEAM SECTION AT WINDOW
3/4" = 1'-0"



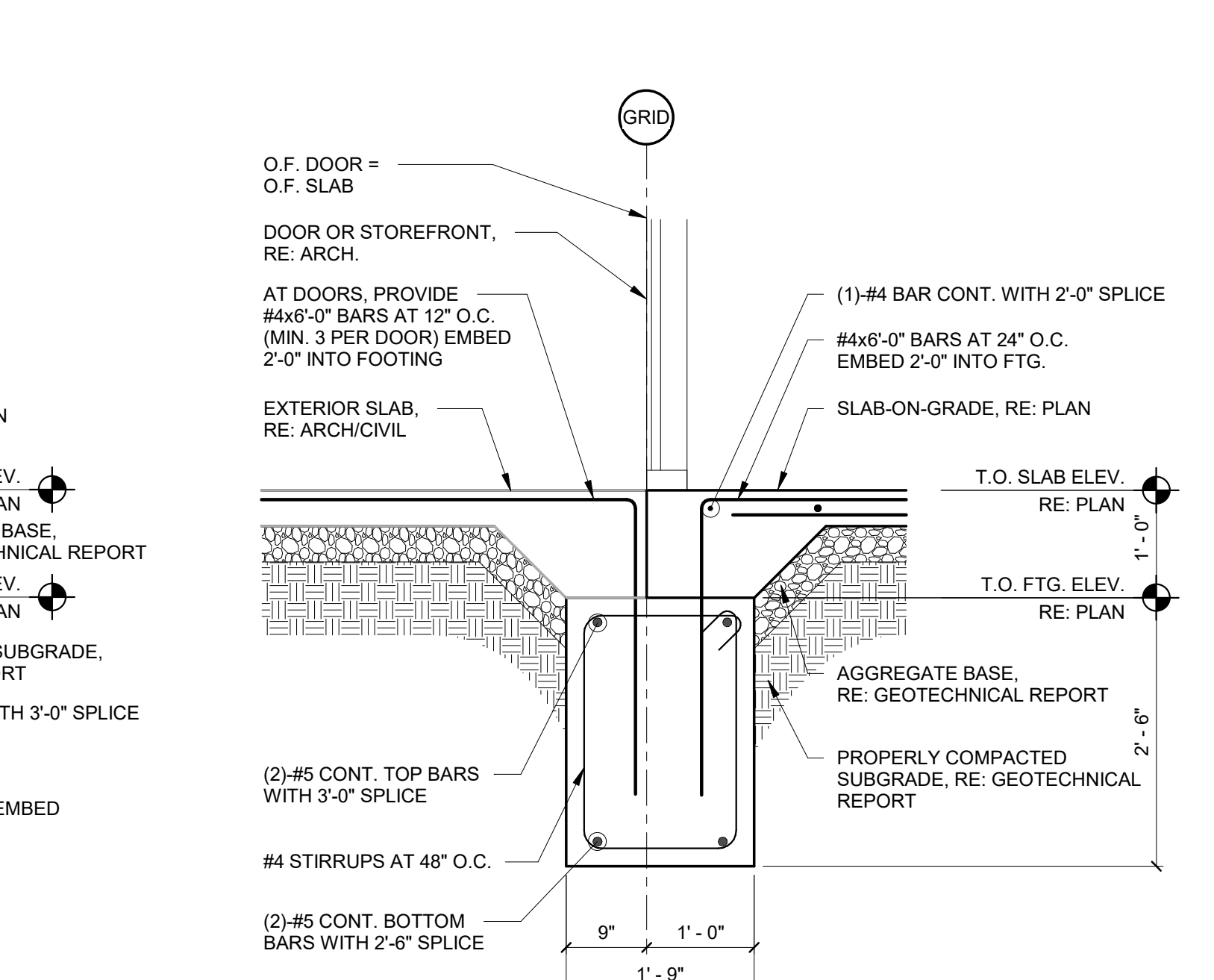
4 EXTERIOR WALL AT COLUMN
3/4" = 1'-0"



3 SHEAR WALL FOUNDATION SECTION
3/4" = 1'-0"



2 GRADE BEAM SECTION AT WALL
3/4" = 1'-0"



1 FOUNDATION SECTION
3/4" = 1'-0"

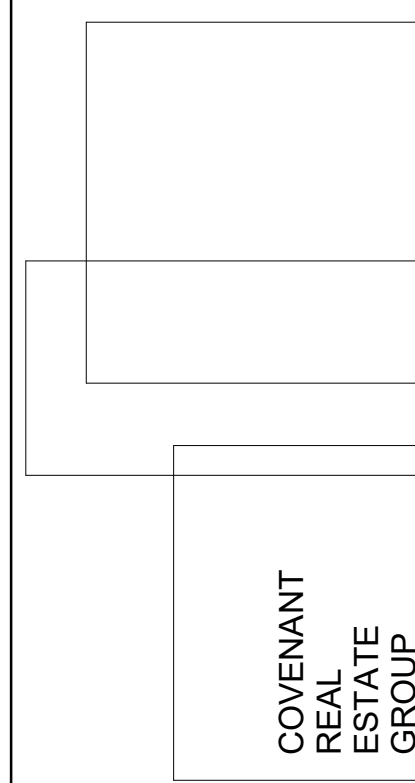
PROJECT INFO

CLIENT: COVENANT GROUP, LLC
PROJECT: BUILDING SHELL - LEE'S SUMMIT, MO - CHIPMAN RD
ADDRESS: 400 NW CHIPMAN RD, LEE'S SUMMIT, MO 64066
PROJECT NO: 287

MAIN CONTACT

CHRISTOPHER CLARK, AIA, NCARB
7701 E KELLOGG DR, STE 630
WICHITA, KS 67207
(316) 302-4472
chris@clarkitecture.net

DEVELOPER

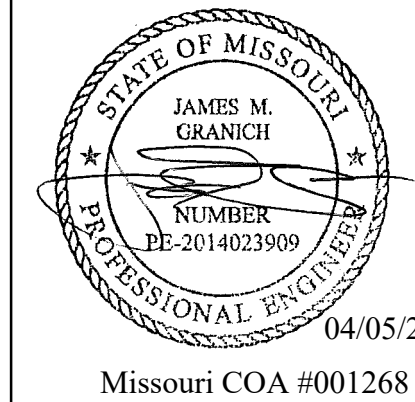


SHEET INFO

ISSUE DATE: 03/31/2022
ISSUED FOR: PERMIT SET

REVISION SCHEDULE

NO	DESCRIPTION	DATE

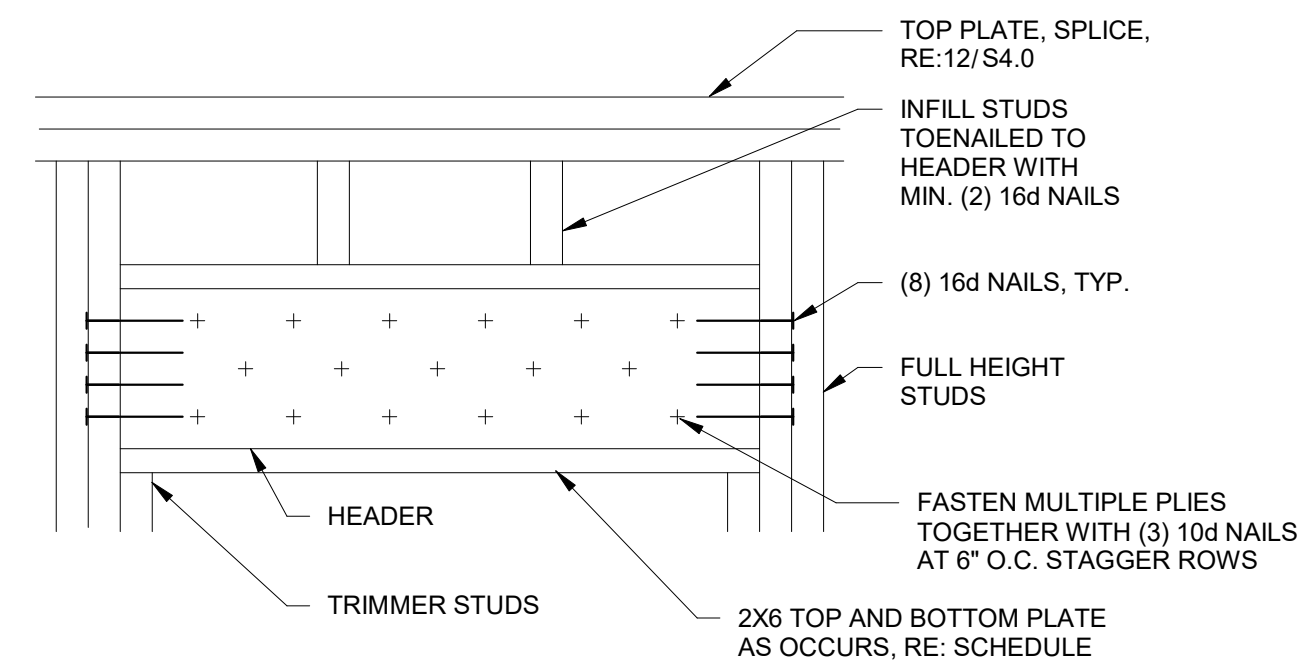


FRAMING DETAILS

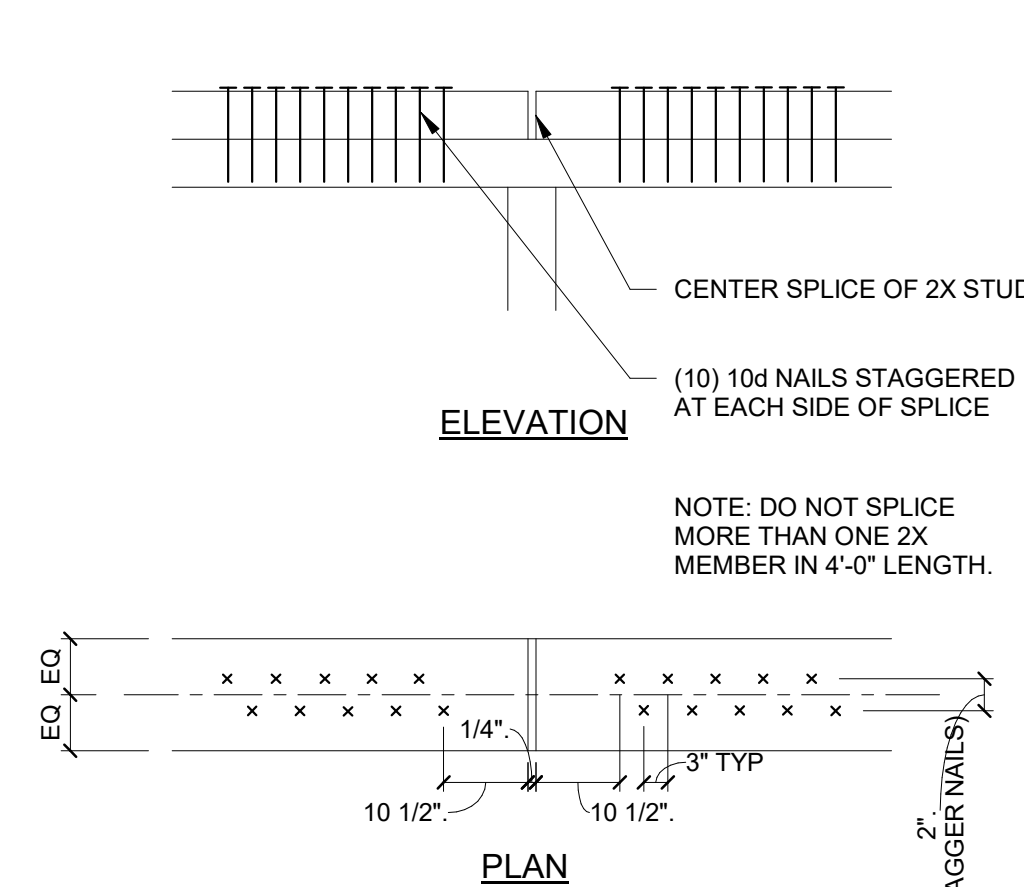
S4.0

TAG	MAX OPENING WIDTH	HEADER	TRIMMER STUDS	FULL HEIGHT STUDS	SILL
H1	19'-6"	(3) 1 1/2x18 LVL	(2) 2X6	HSS5x5x3/8	(2) 2X6
H2	10'-0"	(3) 1 1/2x14 LVL	(1) 2X6	HSS5x5x3/8	(2) 2X6
H3	4'-0"	(3) 2X6	(1) 2X6	(2) 2X6	(1) 2X6
H4	7'-0"	(3) 2X10	(1) 2X6	(2) 2X6	(2) 2X6

NOTES: RE: 10/S4.2 FOR HEADER ATTACHMENT TO STEEL COLUMN

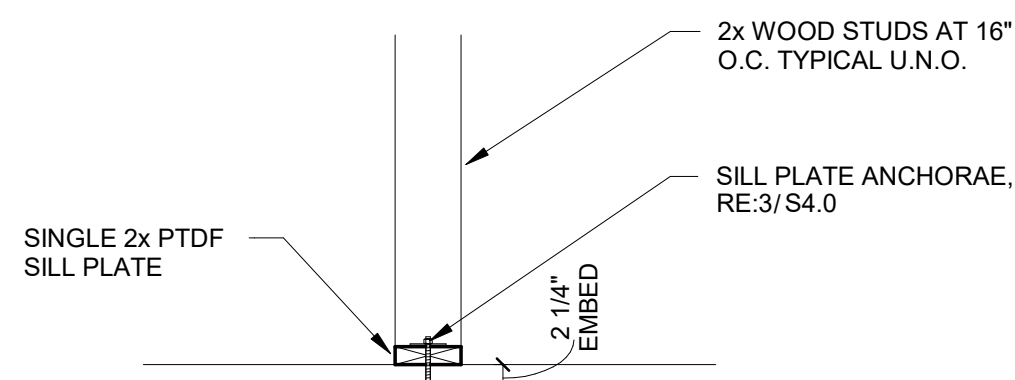


13 BEARING WALL HEADER SCHEDULE
1" = 1'-0"



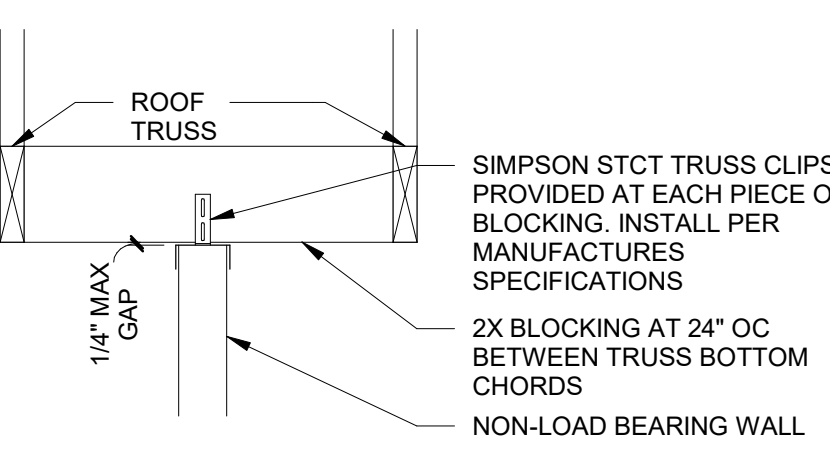
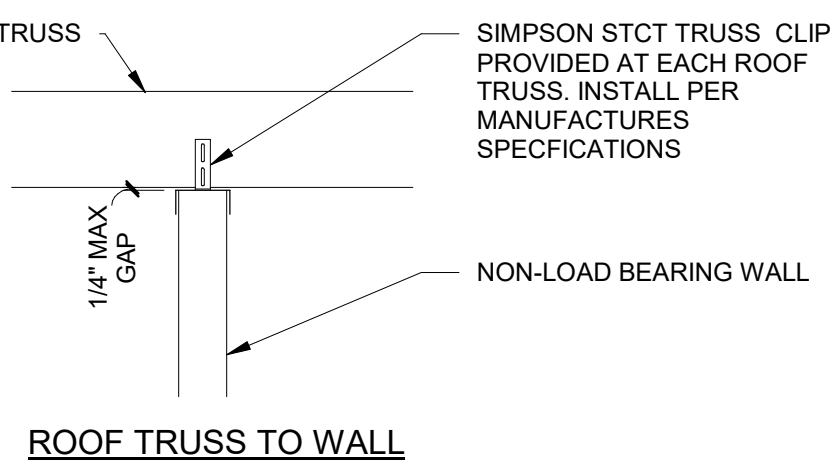
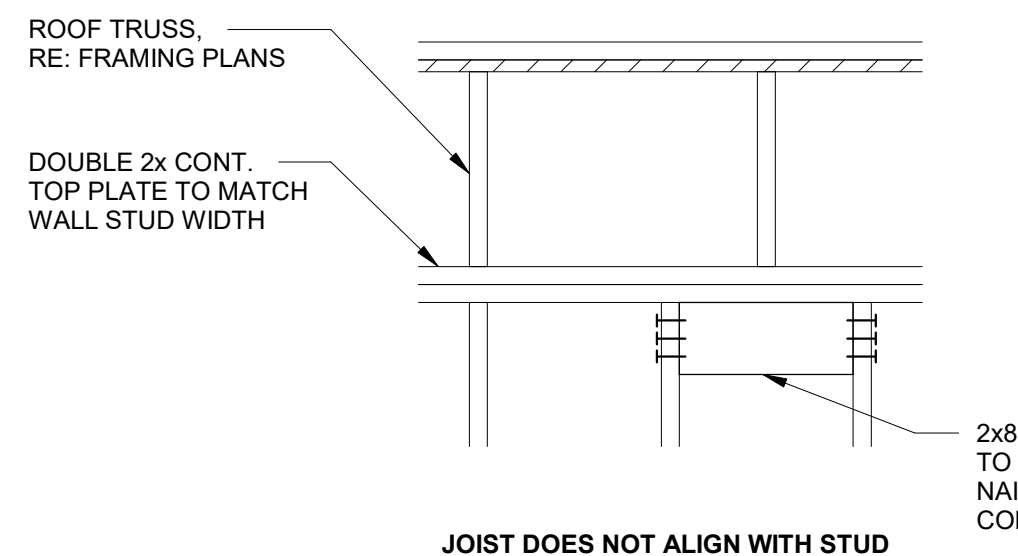
12 TOP PLATE SPLICE DETAIL
1" = 1'-0"

NOTES:
1. PLACE FIRST ANCHOR 8" AND 16" FROM ENDS OF SILL PLATE.
2. SILL PLATES SHALL HAVE A MINIMUM OF 2 ANCHORS.

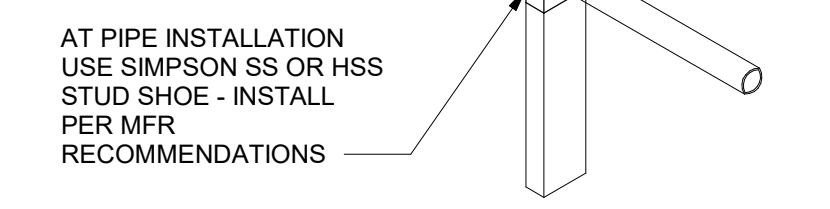
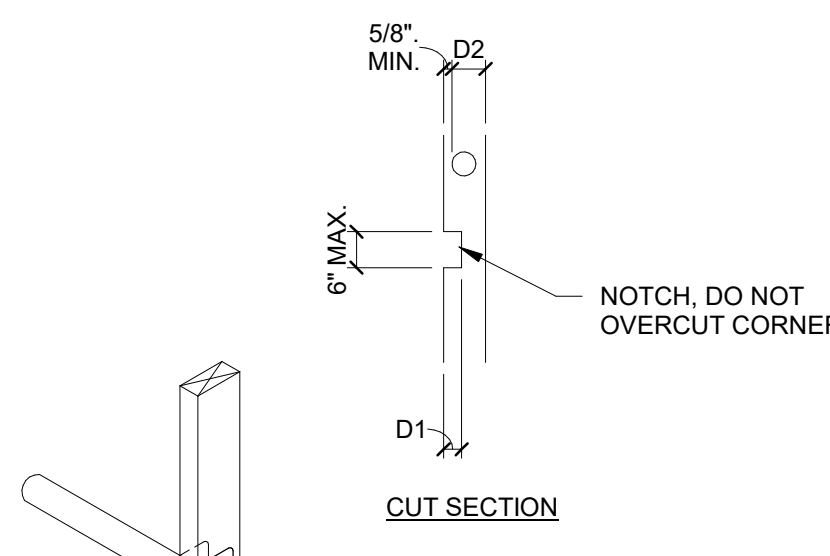


8 NON-LOAD BEARING WALL CONNECTION
3/4" = 1'-0"

14 BEARING WALL FRAMING ALIGNMENT
NTS



11 INTERIOR WALL BRACING DETAIL
3/4" = 1'-0"



STUD SIZE	BEARING WALLS		NON-BEARIGN WALLS	
	MAX D1 NOTCH	MAX D2 HOLE DIA.	MAX D1 NOTCH	MAX D2 HOLE DIA.
2x4 AND 3x4	7/8"	1 3/8"	1 3/8"	2"
2x6	1 3/8"	2 3/16"	2 3/16"	3 1/4"
2x8	1 3/4"	2 7/8"	2 7/8"	4 1/4"

6 TYPICAL NOTCH AT WOOD STUD
3/4" = 1'-0"

ATTACHMENT ANCHOR TYPE	EMBEDMENT	LOCATION AND SPACING		
		EXTERIOR WALLS	INTERIOR BEARING WALLS	INTERIOR NON LOAD BEARING WALLS
5/8" DIA. CONCRETE SCREW ANCHORS	6"	48" O.C.	72" O.C.	N/A
5/8" DIA. HEADED ANCHOR BOLTS	7"	48" O.C.	72" O.C.	N/A
1/2" DIA. EXPANSION ANCHORS	2 1/4"	N/A	72" O.C.	N/A
0.177" DIA P.A.F.	1 1/2"	N/A	24" O.C. STAGGERED	48" O.C. STAGGERED
0.099" DIA P.A.F.	1"	N/A	12" O.C. STAGGERED	12" O.C. STAGGERED

NOTES:
1. EACH SILL PLATE SHALL HAVE (2) ANCHORS MIN.
2. LOCATE ANCHOR BOLTS 12" MAX. FROM SILL PLATE ENDS. LOCATE P.A.F. 6" MAX. FROM SILL PLATE ENDS.
3. DO NOT OVERSIZE HOLES IN SILL PLATE.
4. EXPANSION ANCHORS SHALL NOT BE ALLOWED WITHIN 6 INCHES OF SLAB EDGE.
5. ALL HARDWARE IN CONTACT WITH PRESSURE TREATED FRAMING SHALL BE HOT-DIPPED ZINC-COATED GALVANIZED STEEL OR STAINLESS STEEL.
6. RE: SHEAR WALL SCHEDULE FOR SILL PLATE ANCHORS AT SHEAR WALLS.

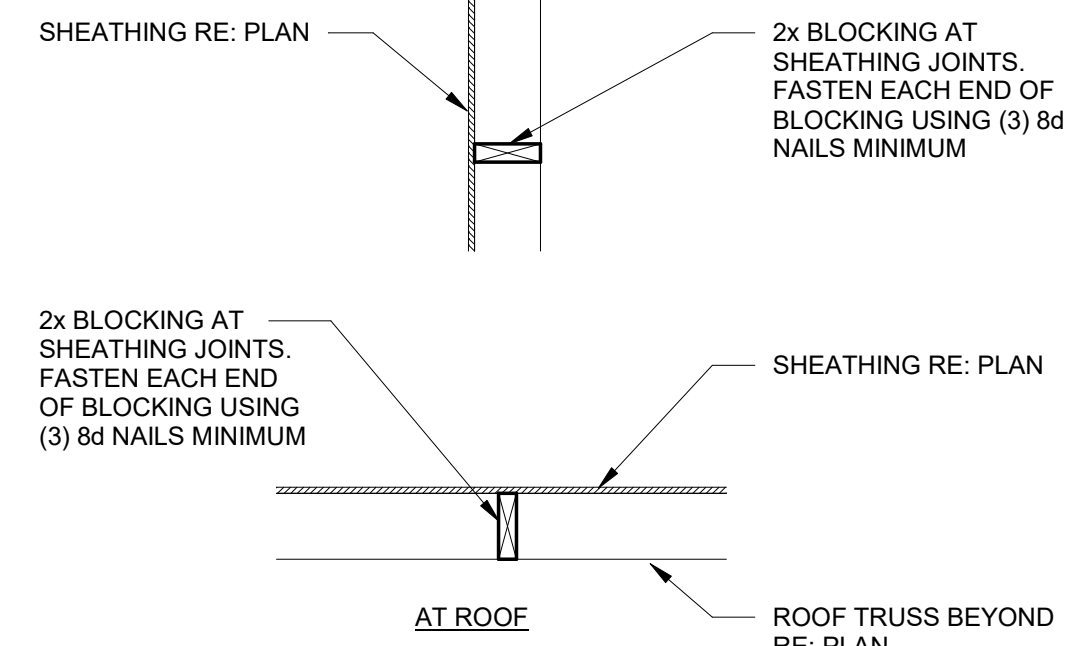
EQUIVALENT PNEUMATIC OR "POWER" DRIVEN NAIL SIZES	
6d COMMON	0.113" DIA. x 2"
8d COMMON	0.131" DIA. x 2 1/2"
10d COMMON	0.148" DIA. x 3"
12d COMMON	0.148" DIA. x 3 1/4"
16d COMMON	0.162" DIA. x 3 1/2"
20d COMMON	0.192" DIA. x 4"
30d COMMON	0.207" DIA. x 4 1/2"

2 EQUIVALENT NAIL SIZE SCHEDULE
3/4" = 1'-0"

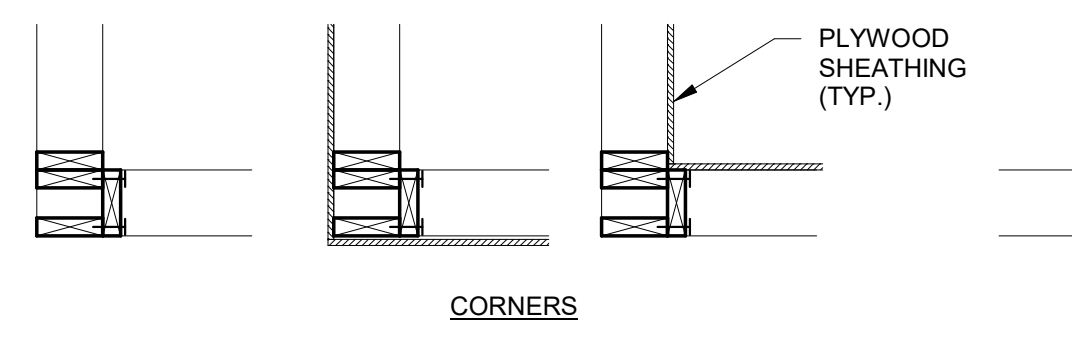
JOIST SIZE	FACE MOUNT HANGER	TOP FLANGE HANGER
2x4	LUS24	PF24
2x6	LUS26	JB26
2x8	LUS26	JB28
2x10	LUS28	JB210A
2x12	LUS210	JB212A
(2) 2x8	LUS28-2	HUS28-2TF
(2) 2x10	LUS28-2	HUS210-2TF
(2) 2x12	LUS28-2	HUS212-2TF

NOTES:
1. THE ABOVE ARE MIN. HANGER REQUIREMENTS. REFER TO DETAILS FOR FRAMING MEMBERS NOT SHOWN.
2. ALL JOIST HANGERS BY SIMPSON STRONG-TIE CO. INC. ONLY USE FACE MOUNTED HANGERS WHERE ALL FACE NAILING CAN BE INSTALLED PER THE MANUFACTURER'S REQUIREMENTS.
3. PLY 2x4: (1) ROW OF STAGGERED 10d COMMON NAILS AT 12" O.C. SPACED AT 11 1/2" HORIZ. AND 6" VERT. CENTERED ON MEMBER, WITH ADJACENT NAILS DRIVEN FROM OPPOSITE SIDES OF THE BUILT-UP MEMBER.
4. PLY 2x6: (1) ROW OF STAGGERED 10d COMMON NAILS AT 12" O.C. SPACED AT 21 1/2" HORIZ. AND 6" VERT. CENTERED ON MEMBER, WITH ADJACENT NAILS DRIVEN FROM OPPOSITE SIDES OF THE BUILT-UP MEMBER.
5. PLY 2x8: (1) ROW OF STAGGERED 30d COMMON NAILS AT 16" O.C. SPACED AT 11 1/2" HORIZ. AND 8" VERT. CENTERED ON MEMBER, WITH ADJACENT NAILS DRIVEN FROM OPPOSITE SIDES OF THE BUILT-UP MEMBER.
6. PLY 2x6: (2) ROWS OF 30d COMMON NAILS AT 8" O.C. EA. SIDE, SPACED AT 11 1/2" HORIZ. FROM EDGE.
7. PLY 2x8: (2) ROWS OF SIMPSON SDS25600 SCREWS AT 8" O.C. EA. SIDE, SPACED AT 11 1/2" HORIZ. FROM EDGE.
8. "H" DENOTES NAIL LOCATIONS DRIVEN FROM BACK FACE OF ADDITIONAL NAILS WHERE (2) ROWS OF NAILS ARE REQUIRED.

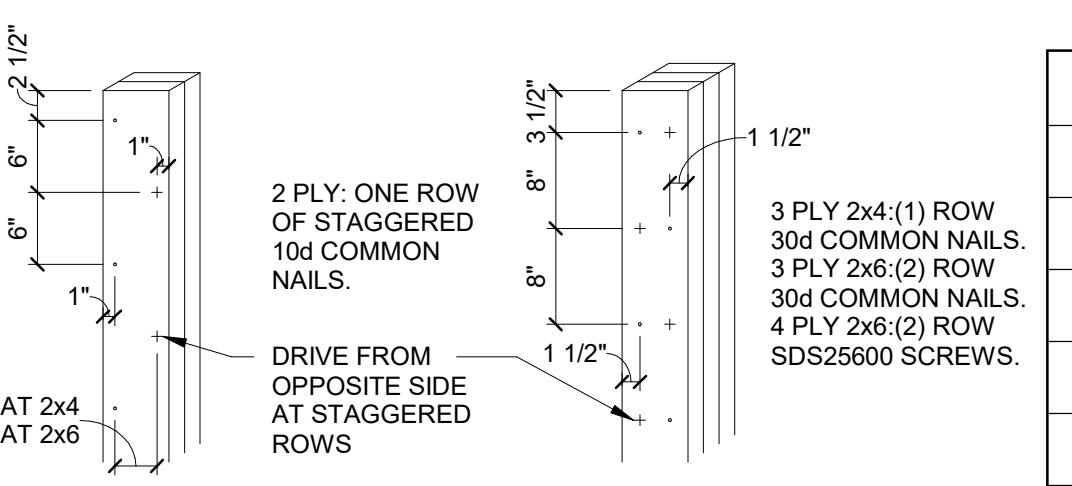
4 FRAMING HANGER SCHEDULE
3/4" = 1'-0"



9 TYPICAL WALL FRAMING BLOCKING
3/4" = 1'-0"



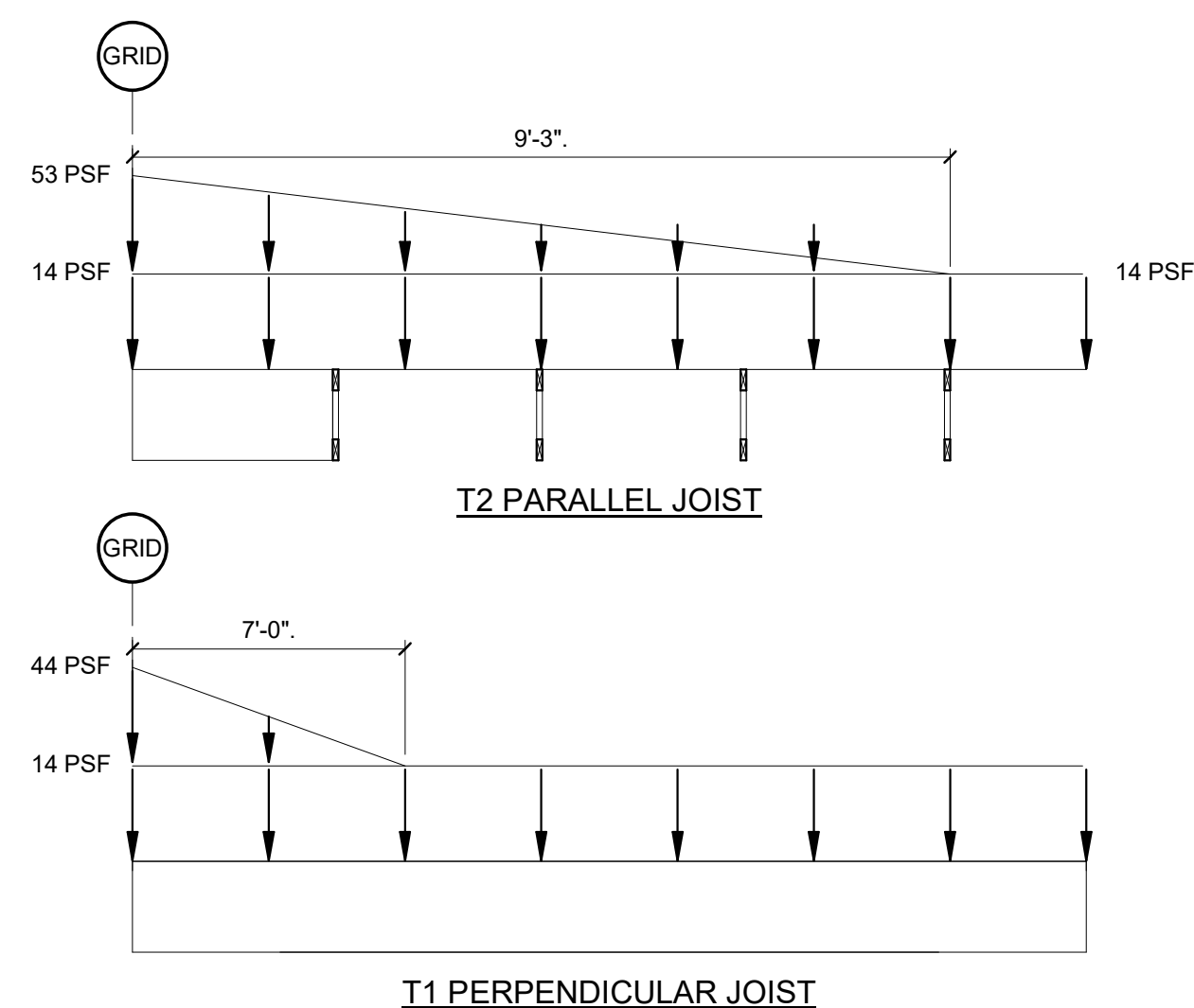
7 TYPICAL WALL FRAMING CONNECTION
3/4" = 1'-0"



NOTE: ALL STUDS SHOWN ARE 2x
5 BUILT-UP POSTS
3/4" = 1'-0"

3 SILL PLATE ANCHOR SCHEDULE AT NON-SHEAR WALLS
3/4" = 1'-0"

1 TYPICAL NAILING SCHEDULE
3/4" = 1'-0"



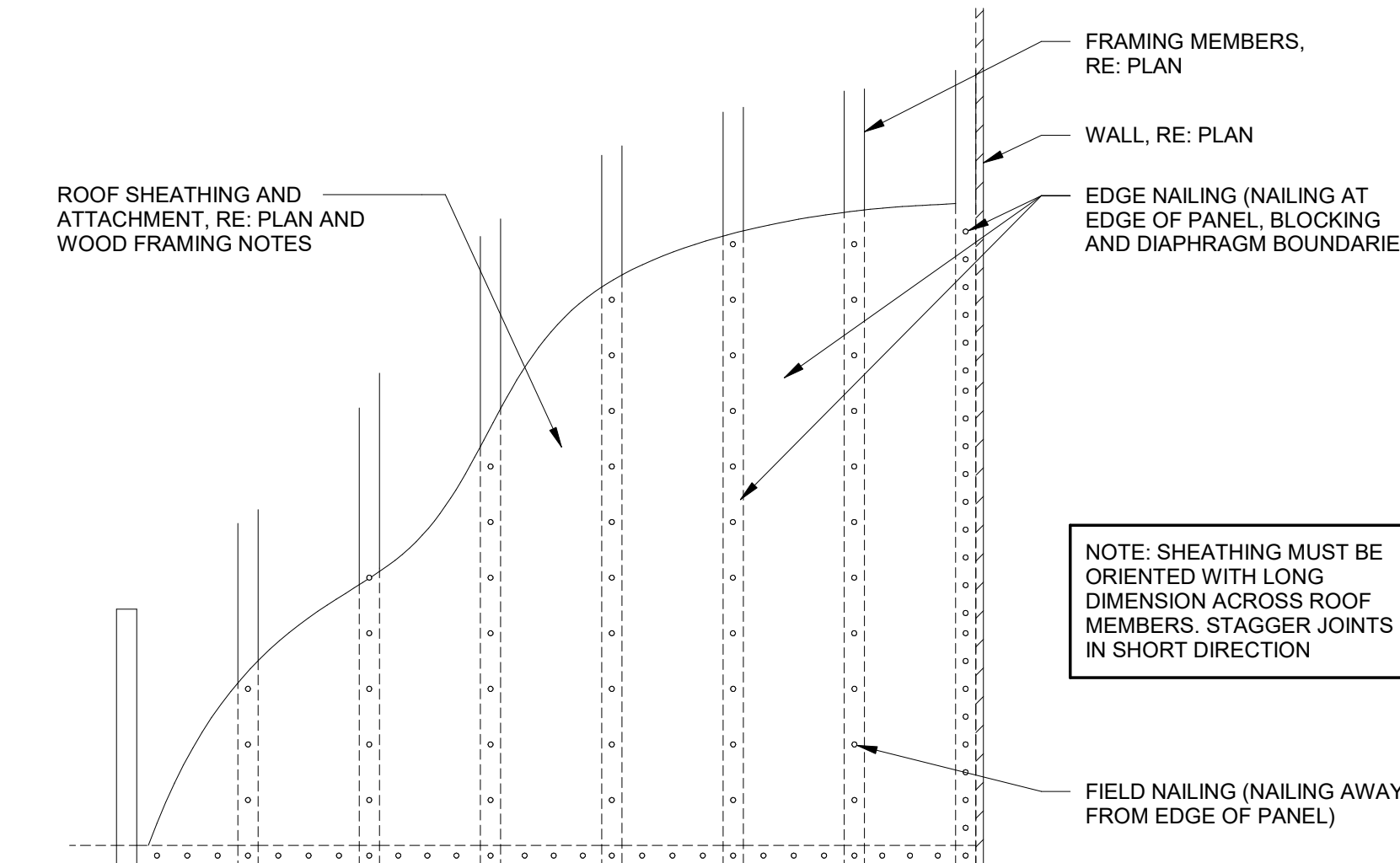
NOTES:
1. ROOF TRUSSES SHALL BE DESIGNED FOR THE CODE LOADING SPECIFIED WITHIN THE GENERAL NOTES. SNOW DRIFT SHALL BE PER THE LOADING SPECIFIED ABOVE.

6 JOIST SNOW DRIFT
1/4" = 1'-0"

MARK	SHEATHING PANEL	NAILING		ANCHORS		
		AT PANEL EDGES AND BOUNDARIES	AT INTERMEDIATE FRAMING MEMBERS	SILL PLATE TO CONCRETE	HOLDOWN ANCHORS (RE: PLANS FOR LOCATIONS)	BUILT-UP END STUDS
SW1	5/16" WOOD STRUCTURAL PANEL ONE SIDE	6d AT 6" O.C.	6d AT 12" O.C.	5/8" DIA. SIMPSON TITEN HD ANCHOR AT 48" O.C. WITH 6" EMBEDMENT	(1) SIMPSON HDU4-SDS2.5 HOLDOWN WITH 5/8" DIA. HILTI HIT-HY 200 ADHESIVE WITH 12" EMBEDMENT	(2) 2x6
SW2	5/16" WOOD STRUCTURAL PANEL ONE SIDE	6d AT 4" O.C.	6d AT 12" O.C.	5/8" DIA. SIMPSON TITEN HD ANCHOR AT 48" O.C. WITH 6" EMBEDMENT	(1) SIMPSON HDU5-SDS2.5 HOLDOWN WITH 5/8" DIA. HILTI HIT-HY 200 ADHESIVE WITH 12" EMBEDMENT	(2) 2x6

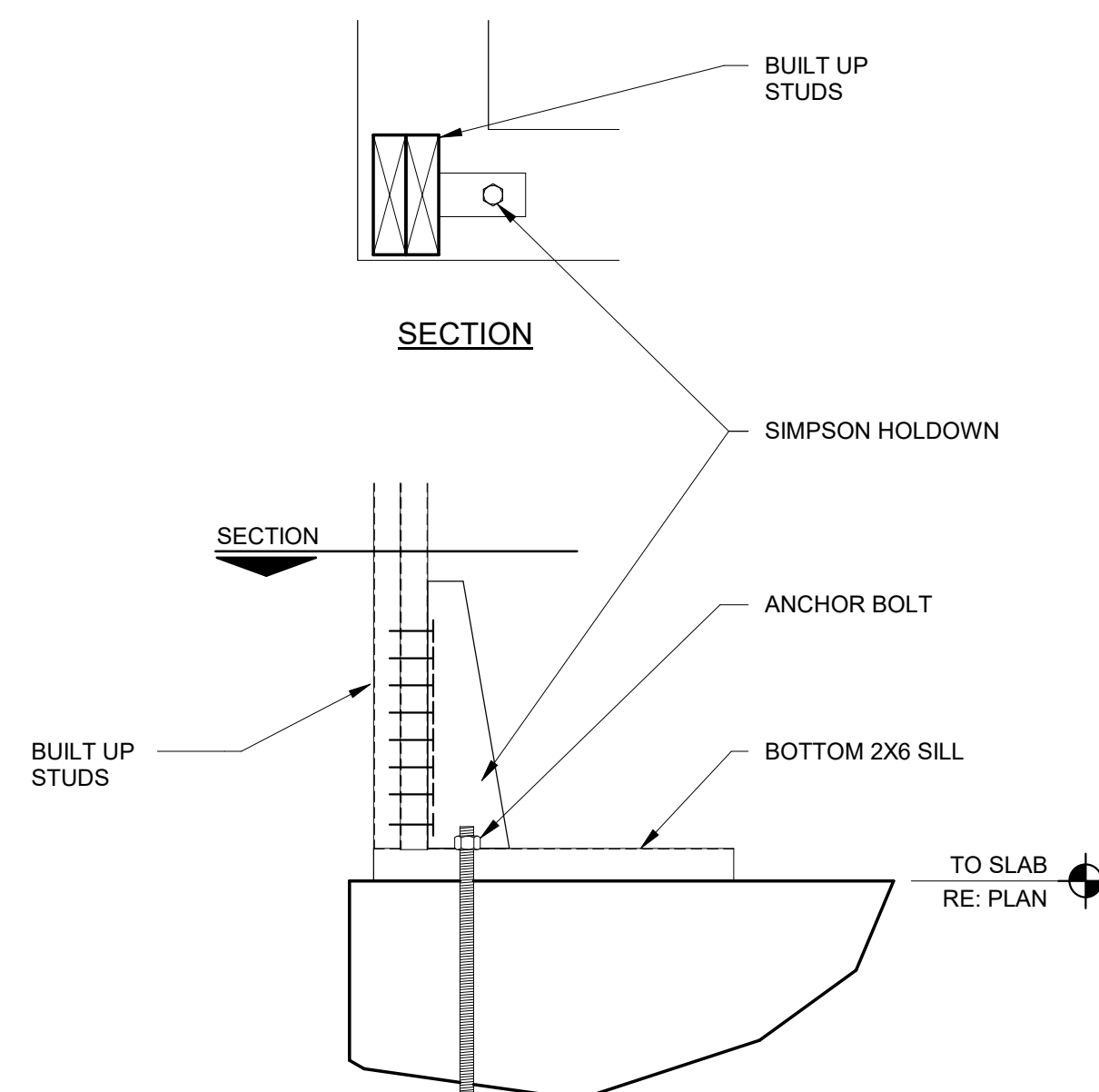
NOTES:
1. RE: PLANS FOR ANCHOR BOLT AND HOLDOWN LOCATIONS.
2. ALL SHEATHING TO BE APA RATED, EXPOSURE I.
3. HOLDOWN EMBEDMENT DOES NOT INCLUDE SLAB-ON-GRADE THICKNESS

5 SHEAR WALL SCHEDULE
3/4" = 1'-0"

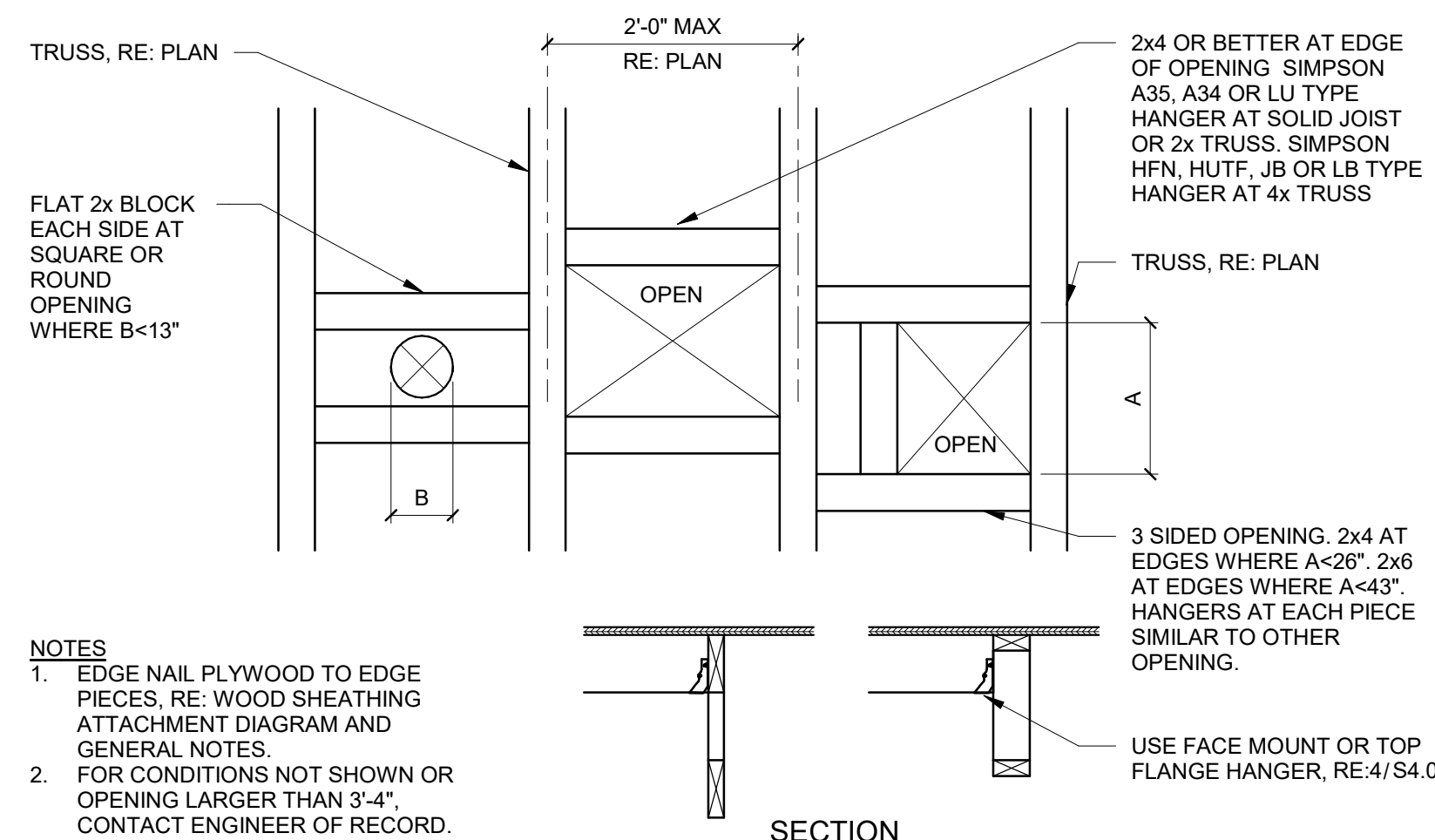


NOTE: SHEATHING MUST BE ORIENTED WITH LONG DIMENSION ACROSS ROOF MEMBERS. STAGGER JOINTS IN SHORT DIRECTION

4 WOOD SHEATHING ATTACHMENT DIAGRAM
3/4" = 1'-0"

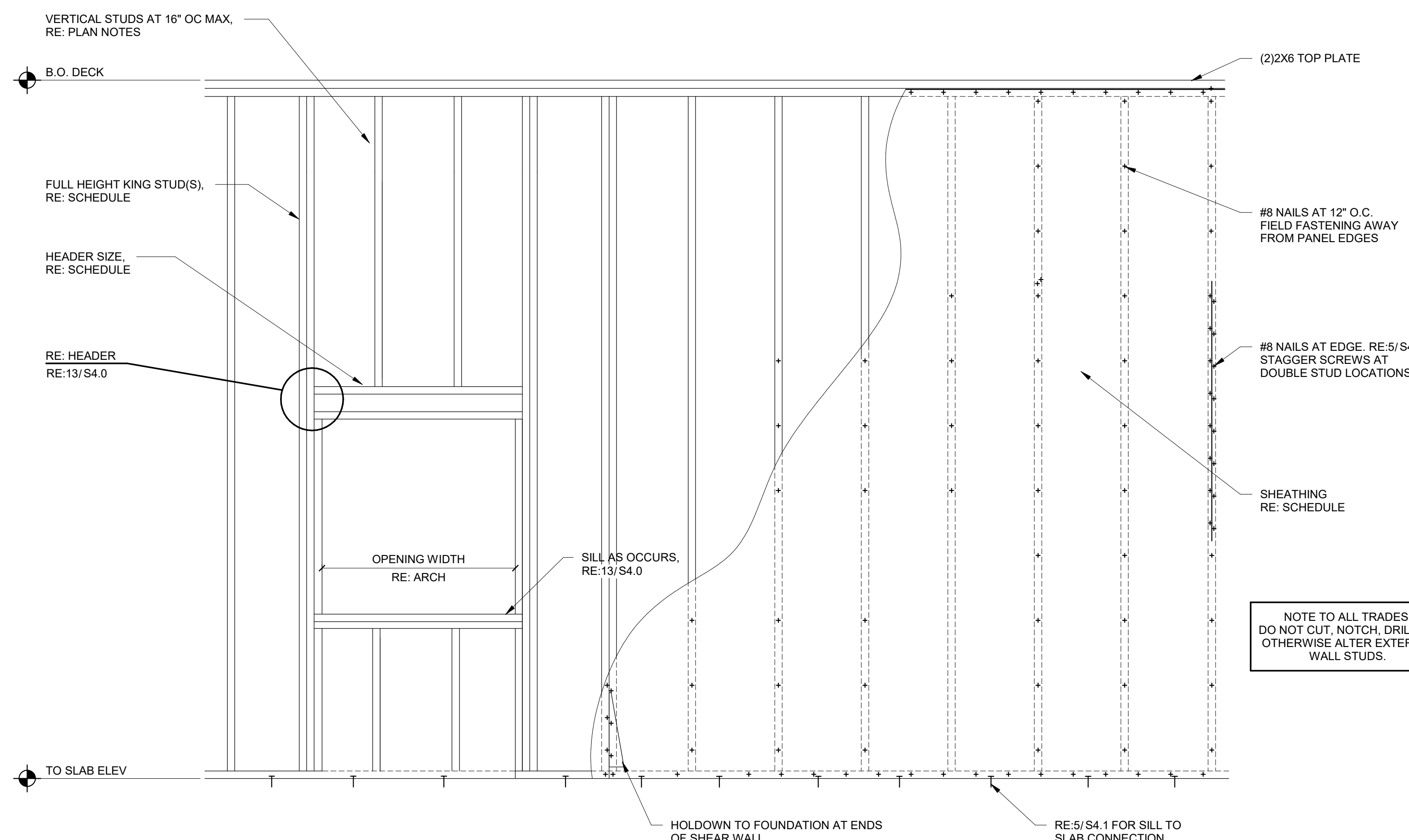


3 SHEAR WALL HOLDDOWN
1 1/2" = 1'-0"



NOTES:
1. EDGE NAIL PLYWOOD TO EDGE PIECES, RE: WOOD SHEATHING ATTACHMENT DIAGRAM AND GENERAL NOTES.
2. FOR CONDITIONS NOT SHOWN OR OPENING LARGER THAN 3'-4", CONTACT ENGINEER OF RECORD.

2 SMALL OPENING IN PLYWOOD DECK
3/4" = 1'-0"



SHEAR WALL NOTES:
1. SEE HOLDOWN ANCHOR DETAIL, RE:3/S4.1 FOR ADDITIONAL INFORMATION.
2. WALL STUDS SHALL BE CONTINUOUS FROM BOTTOM TO TOP.
3. SHEAR WALL HOLDOWNS LOCATED AT EACH WALL CORNER. RE: FOUNDATION PLAN

1 WOOD STRUCTURAL WALL ELEVATION
3/4" = 1'-0"

PROJECT INFO

CLIENT:
COVENANT GROUP, LLC

PROJECT:
BUILDING SHELL - LEE'S SUMMIT, MO - CHIPMAN RD

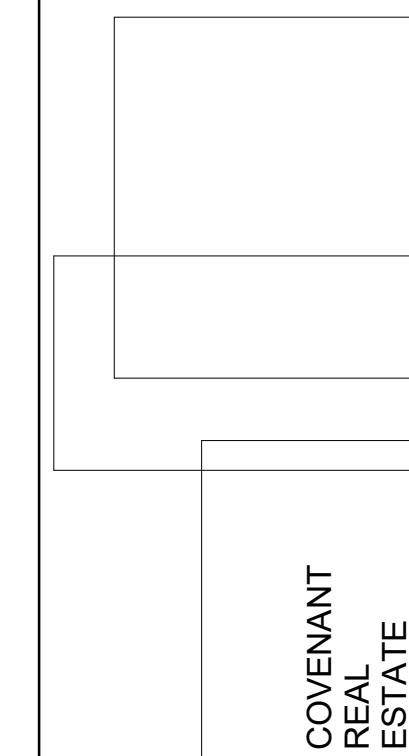
ADDRESS:
400 NW CHIPMAN RD
LEE'S SUMMIT, MO 64006

PROJECT NO: 287

MAIN CONTACT

CHRISTOPHER CLARK, AIA, NCARB
7701 E KELLOGG DR, STE 630
WICHITA, KS 67207
(316) 302-4472
chris@clarkitecture.net

DEVELOPER

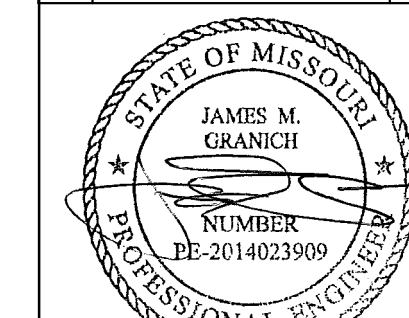


SHEET INFO

ISSUE DATE : 03/31/2022
ISSUED FOR: PERMIT SET

REVISION SCHEDULE

NO	DESCRIPTION	DATE



04/05/2022
Missouri COA #001268

FRAMING DETAILS

S4.1

PROJECT INFO

CLIENT:
COVENANT GROUP, LLC

PROJECT:
BUILDING SHELL - LEE'S SUMMIT,
MO - CHIPMAN RD

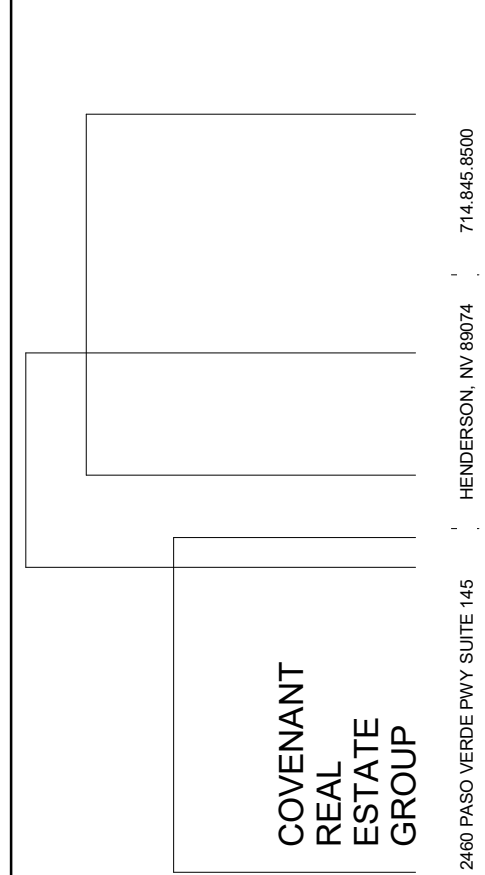
ADDRESS:
400 NW CHIPMAN RD
LEE'S SUMMIT, MO 64806

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WICHITA, KS 67207
(316) 302-4472
chris@clarkitecture.net

DEVELOPER



SHEET INFO

ISSUE DATE : 03/31/2022
ISSUED FOR PERMIT SET

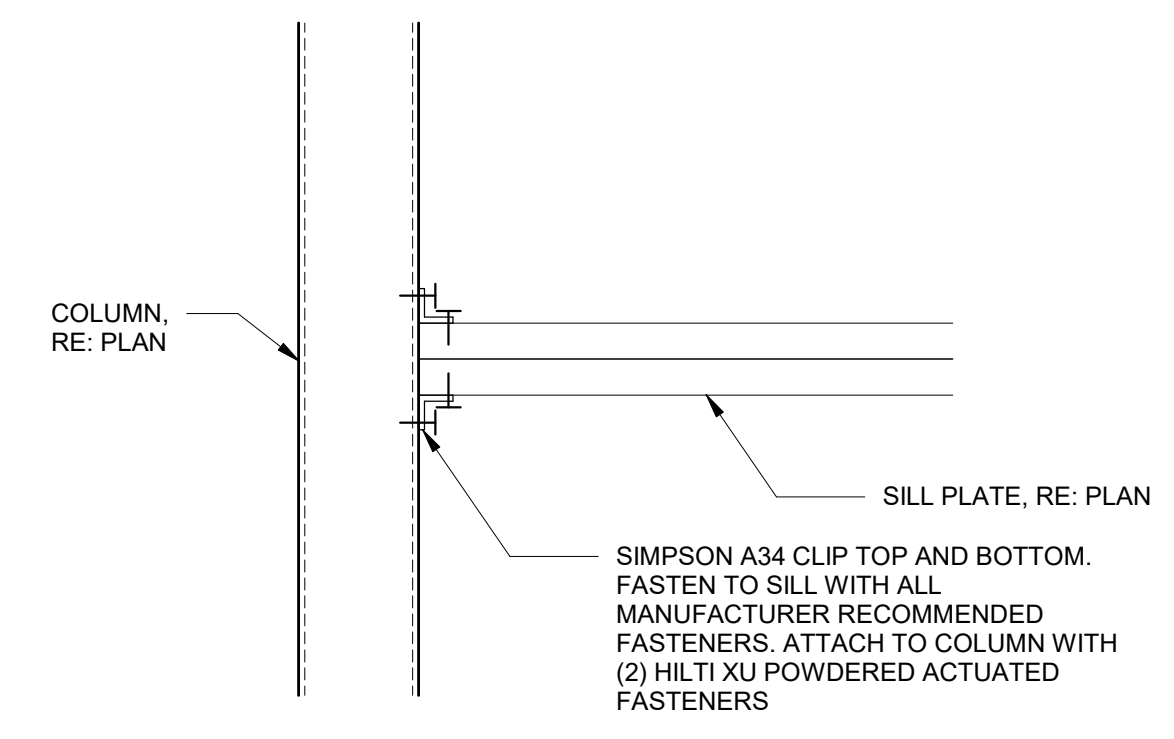
REVISION SCHEDULE

NO	DESCRIPTION	DATE

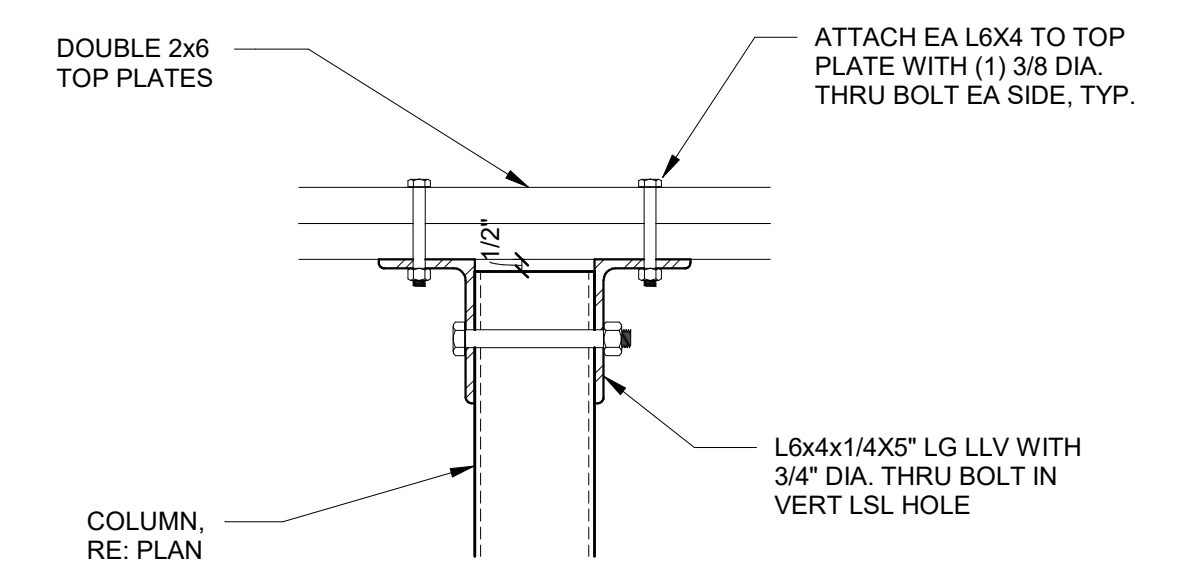


FRAMING DETAILS

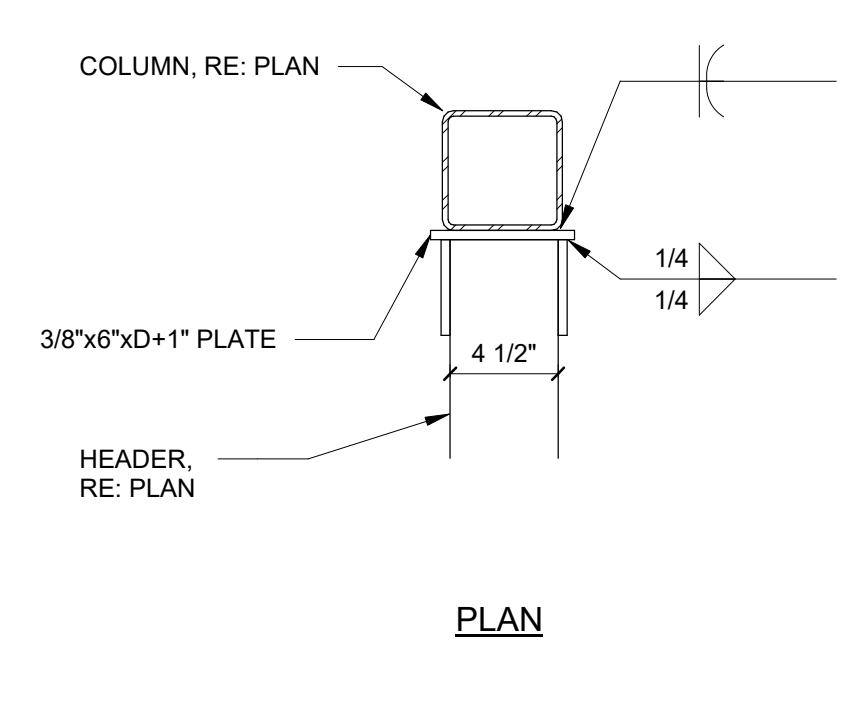
S4.2



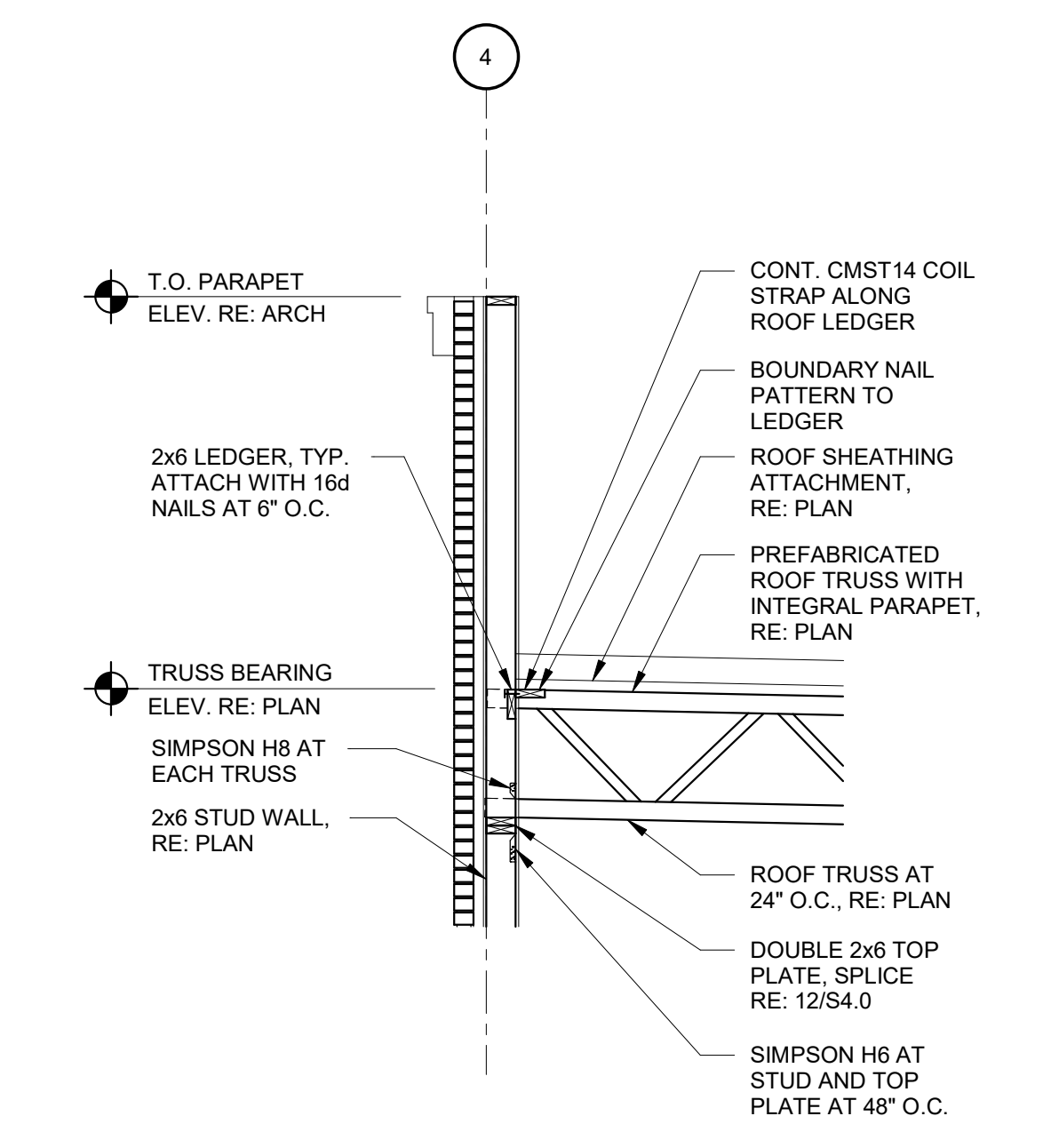
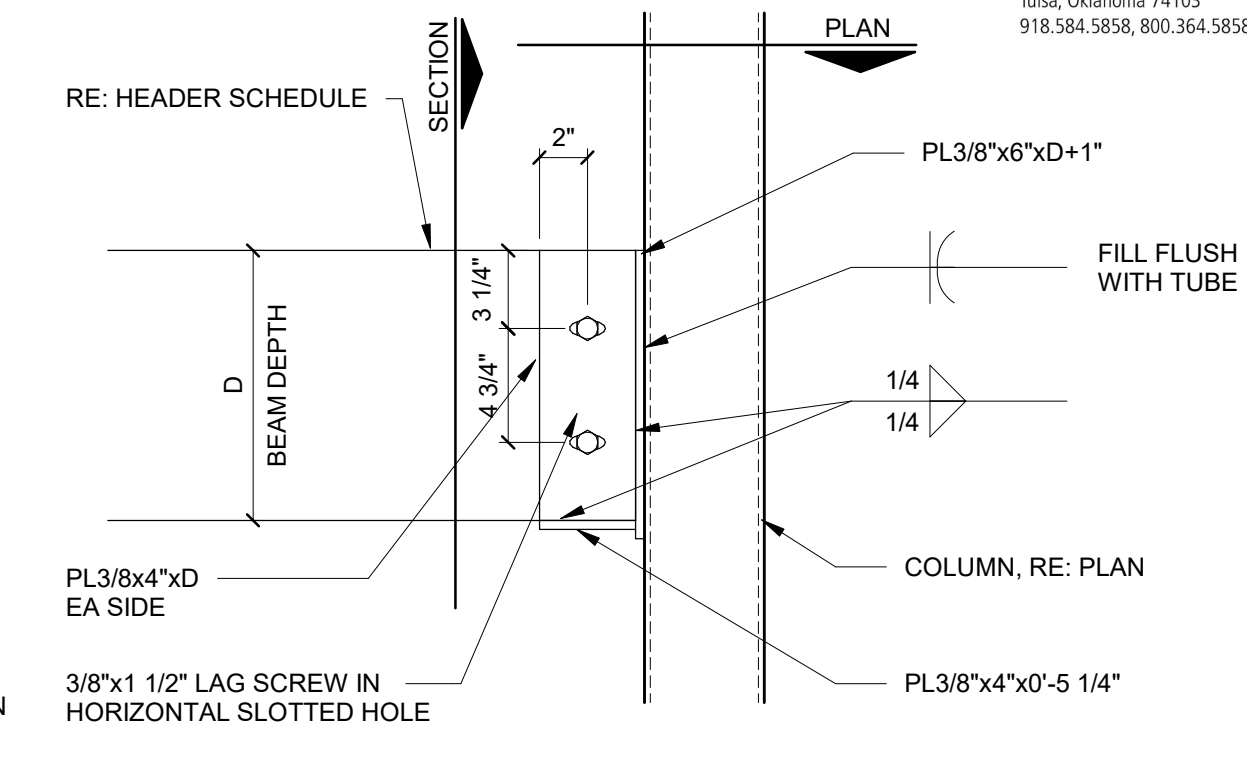
12 COLUMN TO SILL PLATE CONNECTION
1 1/2" = 1'-0"



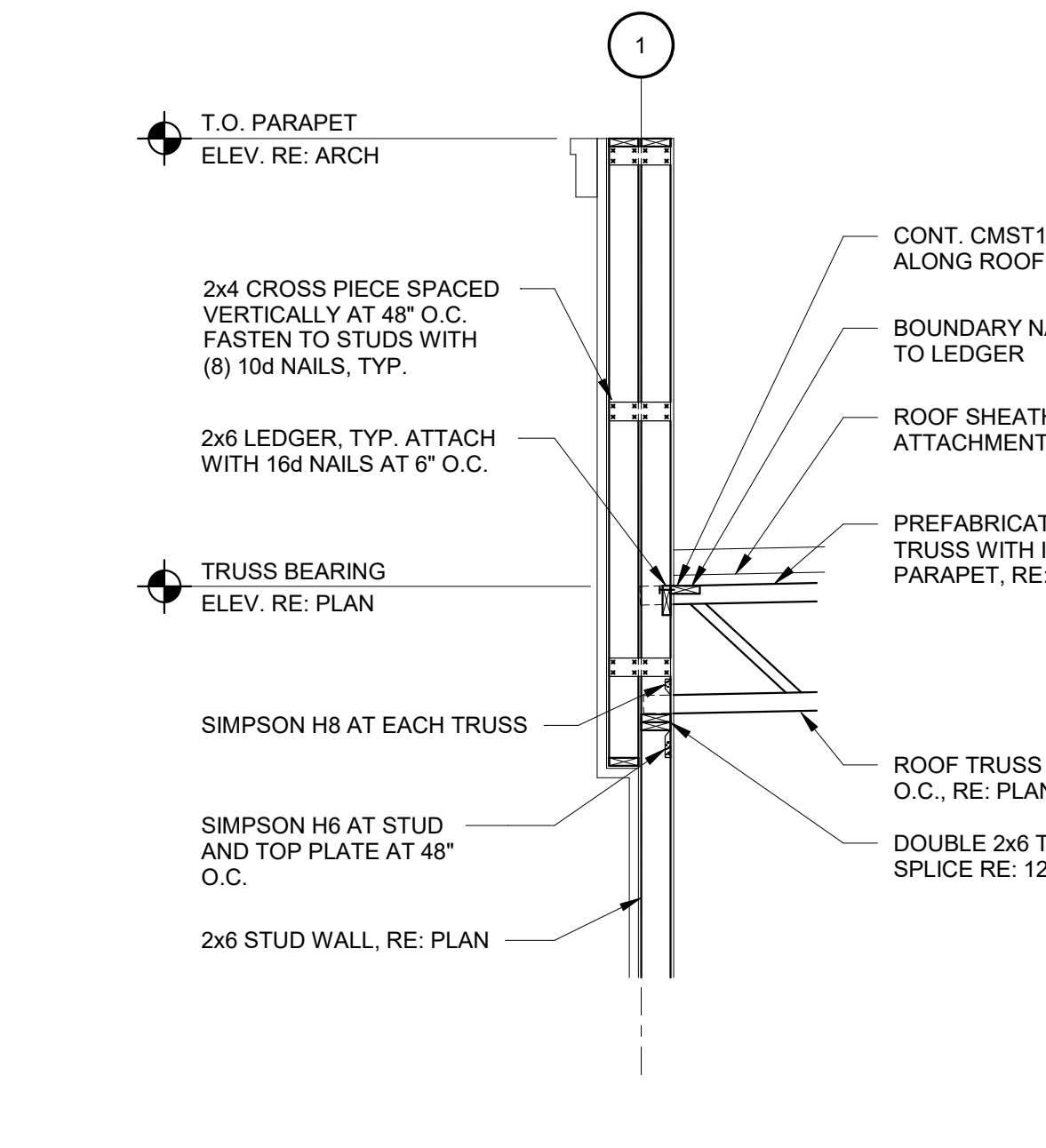
11 COLUMN TO TOP PLATE CONNECTION
1 1/2" = 1'-0"



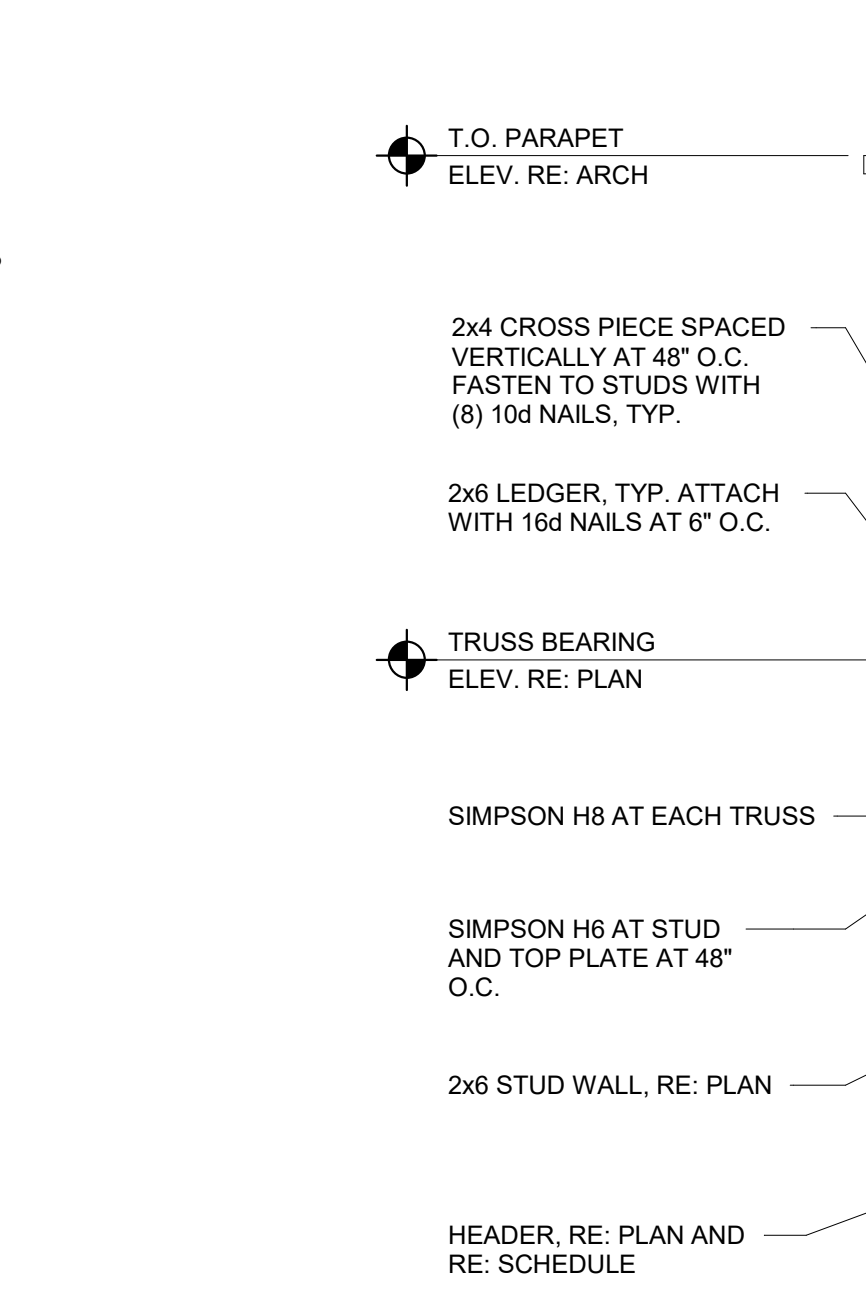
10 HEADER TO STEEL CONNECTION
1 1/2" = 1'-0"



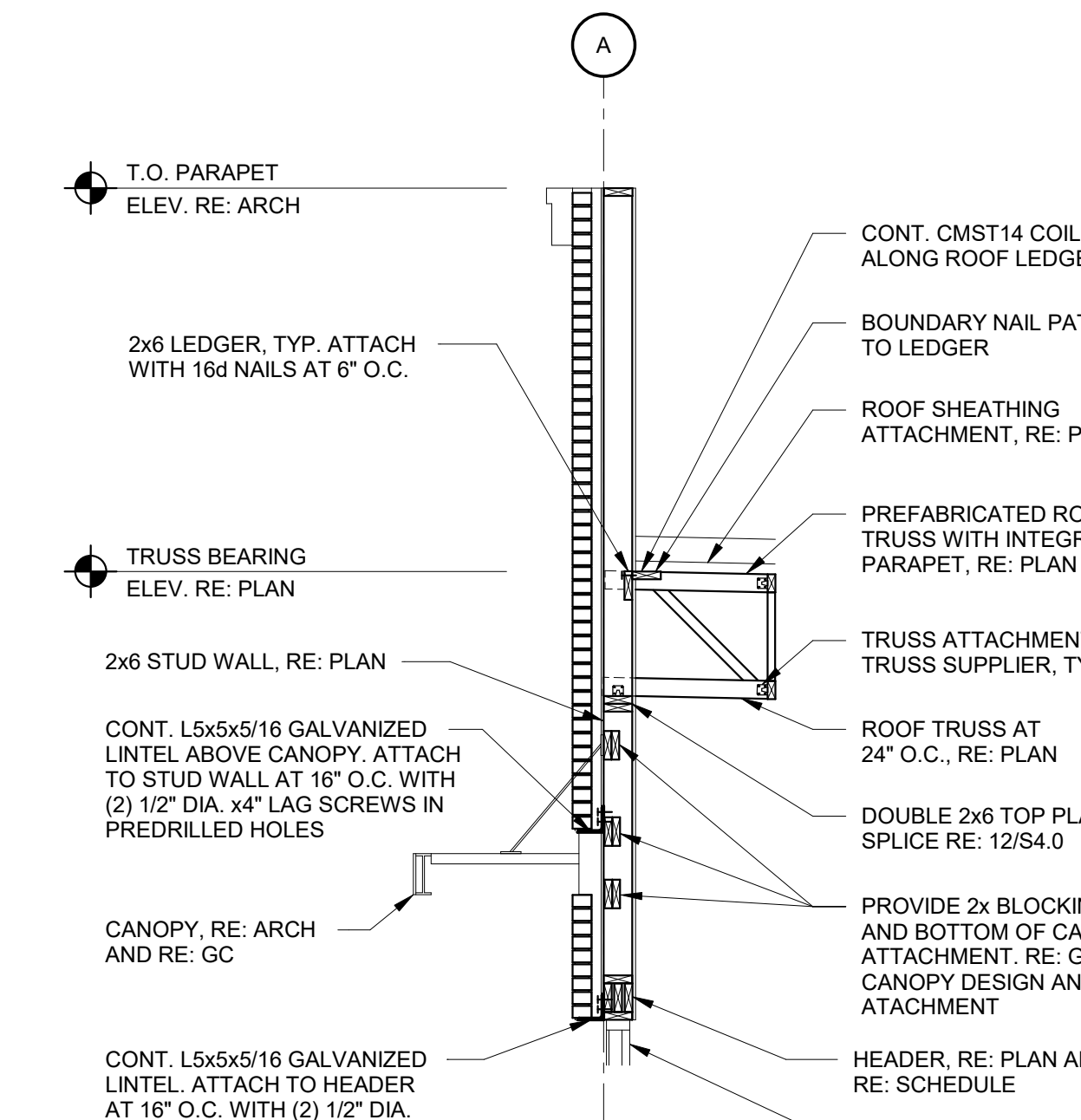
9 WALL SECTION
3/8" = 1'-0"



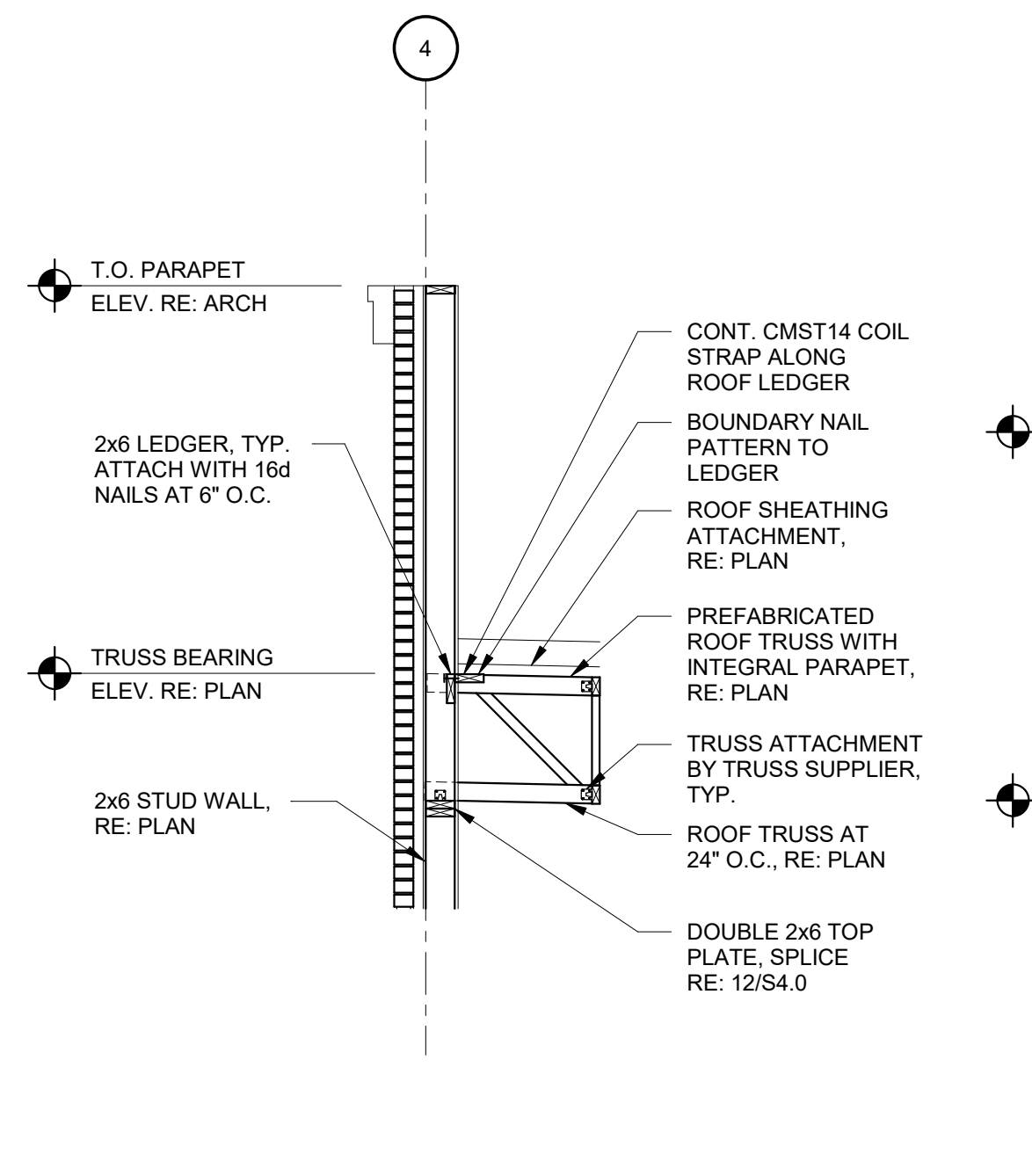
8 WALL SECTION
3/8" = 1'-0"



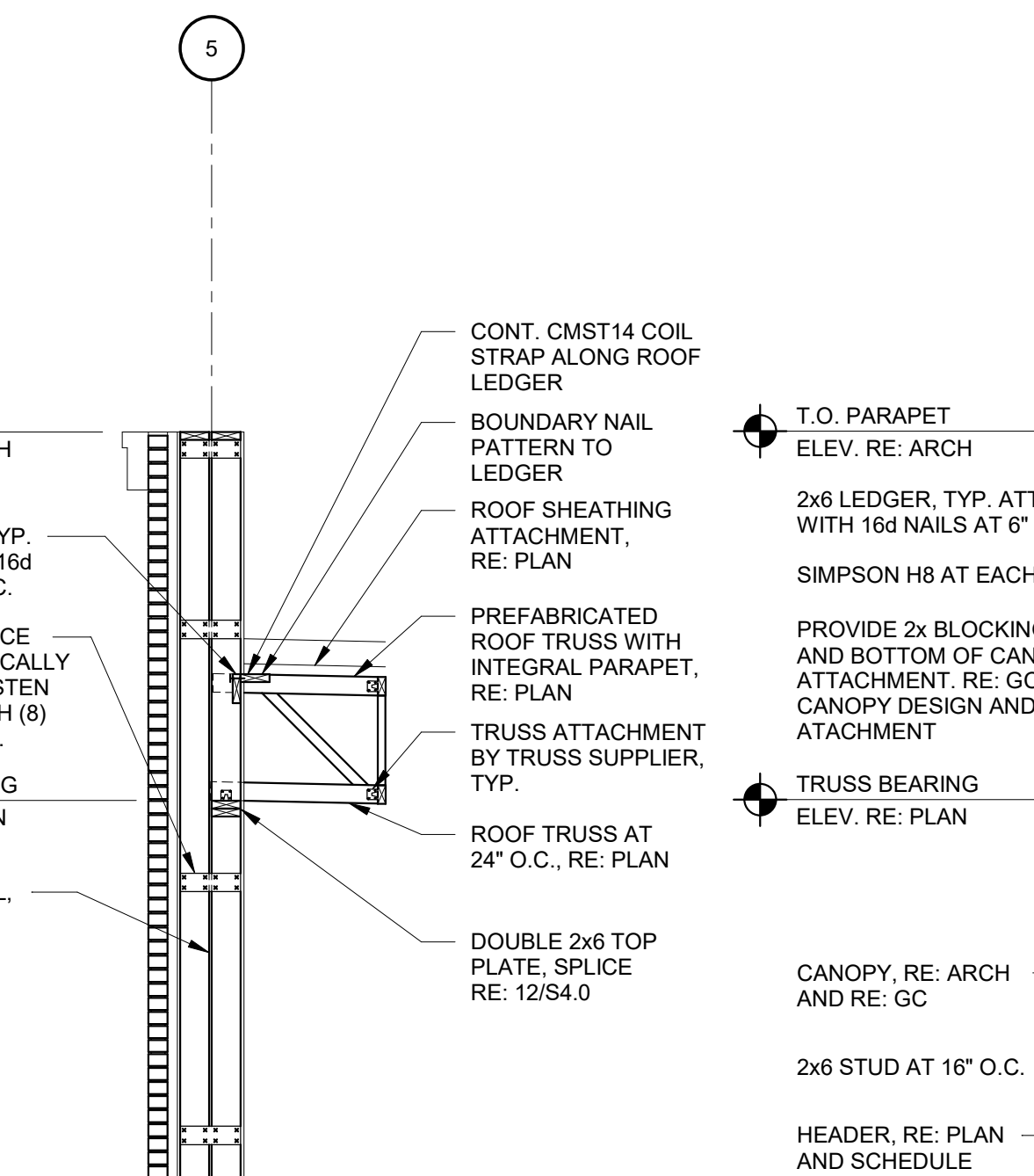
7 WALL SECTION
3/8" = 1'-0"



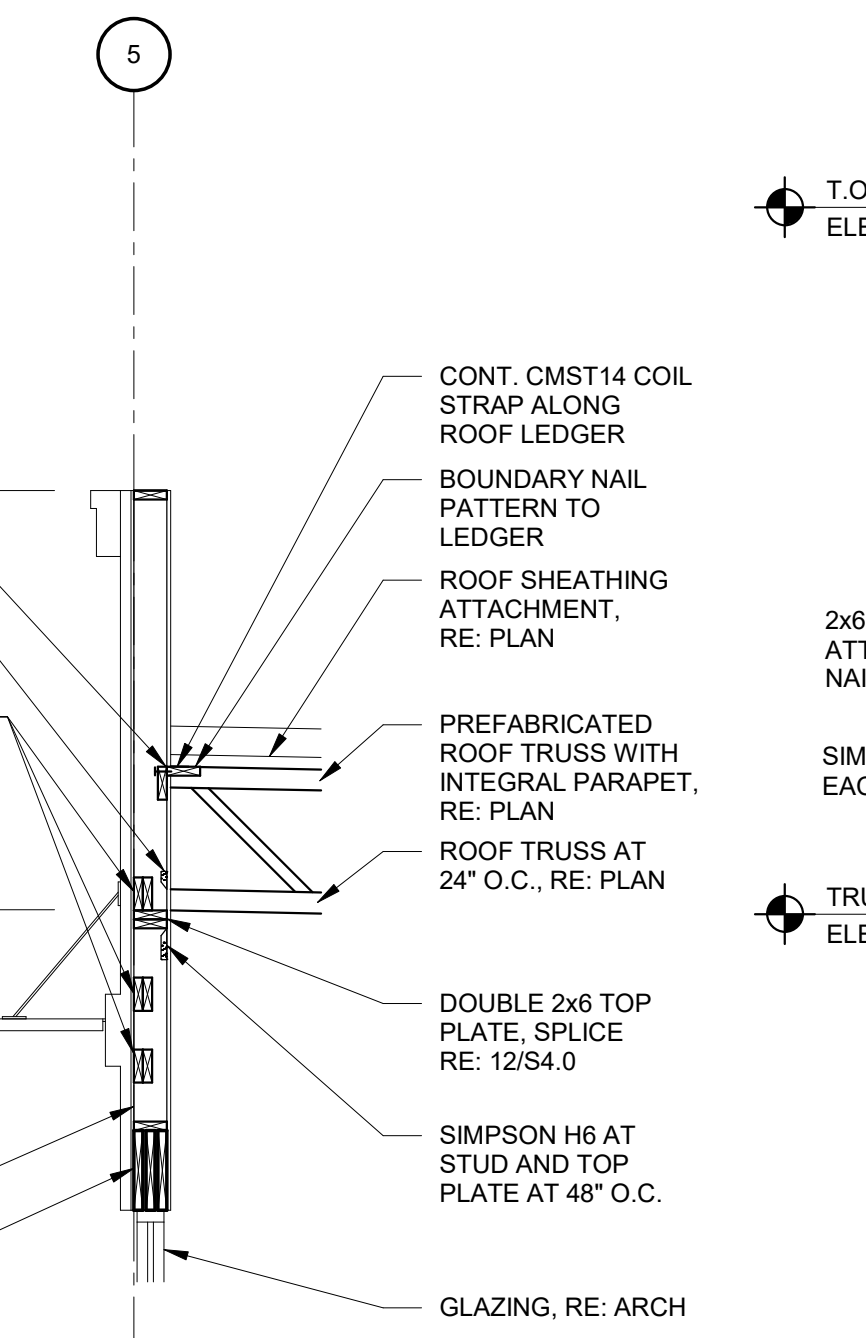
6 WALL SECTION
3/8" = 1'-0"



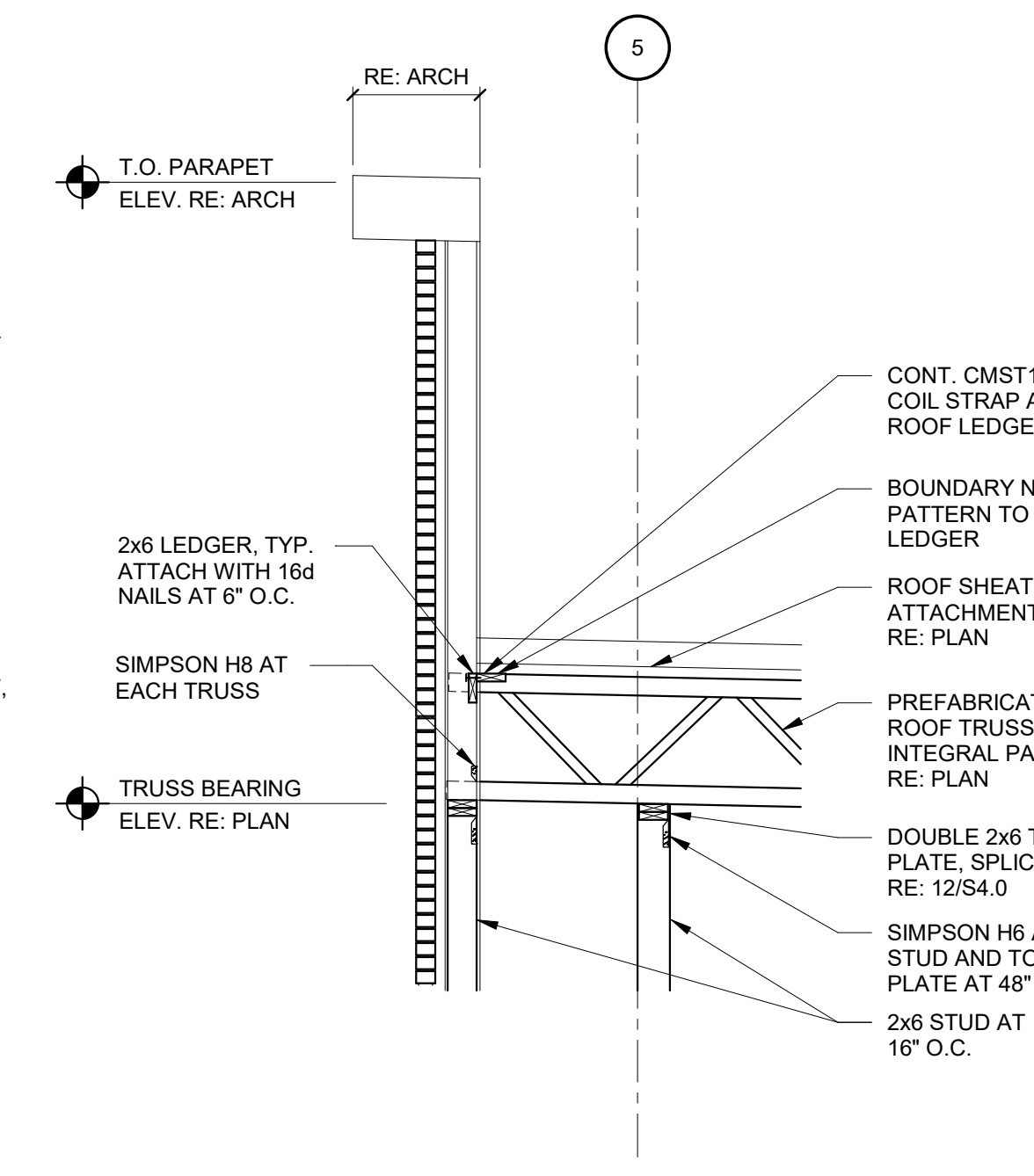
5 WALL SECTION
3/8" = 1'-0"



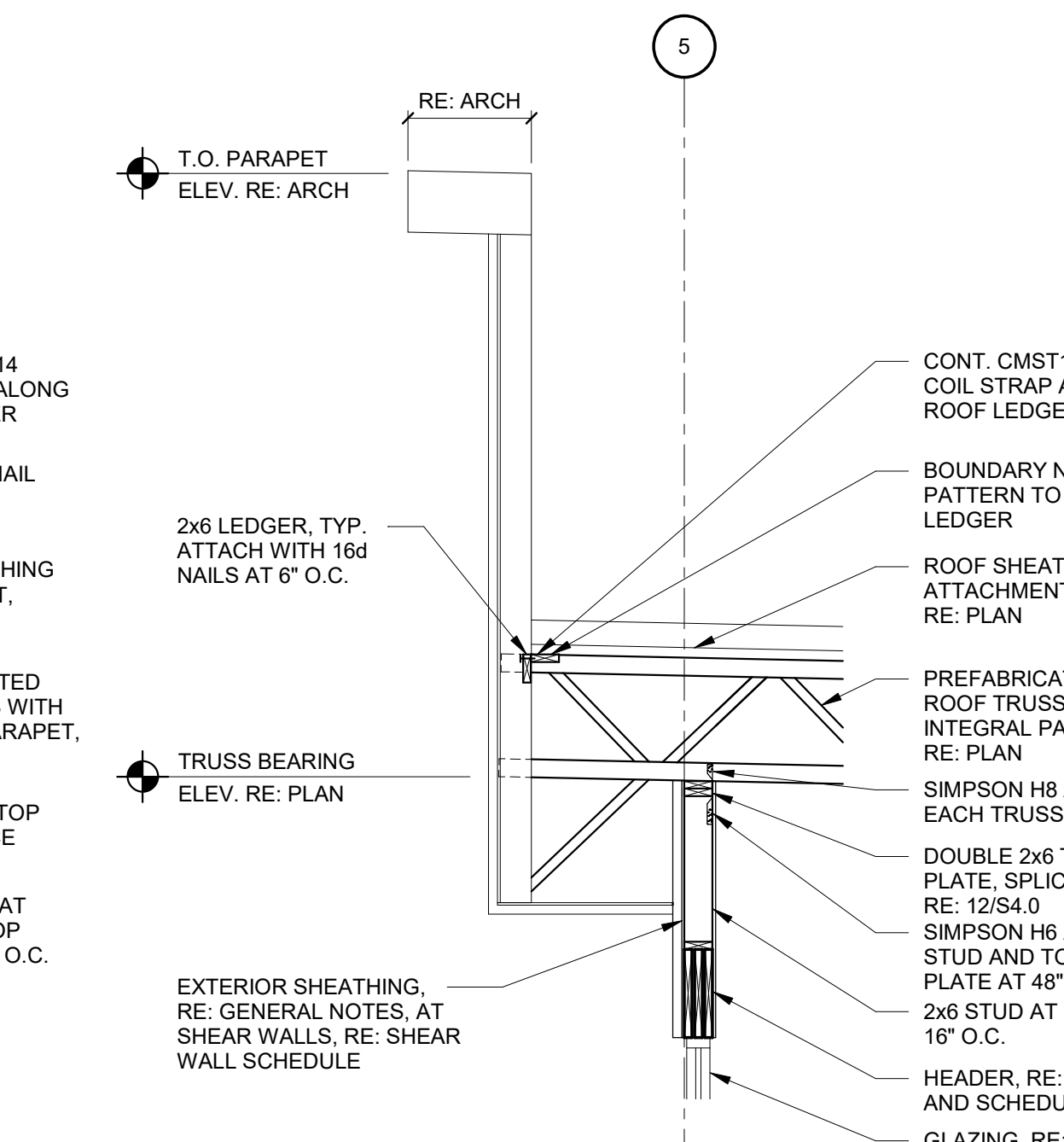
4 WALL SECTION
3/8" = 1'-0"



3 WALL SECTION
3/8" = 1'-0"



2 WALL SECTION
3/8" = 1'-0"



1 WALL SECTION
3/8" = 1'-0"