### DESIGN PARAMETERS

1.		DESIGN CODES AND STANDARDS		
	A.	BUILDING CODE: IBC 2018 RISK CATEGORY		II
	B.	MATERIAL CODES AND STANDARDS DESIGN LOADS: ASCE/SEI 7-16 - MINIMUM DESIGN LOAD STRUCTURES CONCRETE: ACI 318-14 - BUILDING CODE REQUIREM STEEL: AISC 360-16 - SPECIFICATIONS FOR STR WOOD: ANSI/AWC NDS-2018 - NATIONAL DESIGN CONSTRUCTION	S FOR BUILDINGS AND OTHER ENTS FOR STRUCTURAL CONCRETE RUCTURAL STEEL BUILDINGS	
2.	A.	GRAVITY LOADS ROOF DEAD LOADS ROOFING AND INSULATION PLYWOOD DECK MECH., ELEC. AND PLUMBING [AND SPR CEILINGS MISCELLANEOUS TOTAL SUPERIMPOSED ROOF DEAD LOA ROOF STRUCTURE MINIMUM ROOF DEAD LOAD (TO BE USE	NNKLERS] AD ED WITH GROSS UPLIFT)	1.8 PSF 3.2 PSF 6.0 PSF 1.0 PSF 2.0 PSF 14 PSF ACTUAL WEIGHT 20 PSF
	C.	LIVE LOADS (UNIFORM/CONCENTRATED ROOF	))	20 PSF / 300 LB
3.	A. B. C. D. E.	ROOF SNOW LOAD GROUND SNOW LOAD, Pg FLAT ROOF SNOW LOAD, Pf SNOW EXPOSURE FACTOR, Ce SNOW LOAD IMPORTANCE FACTOR, I THERMAL FACTOR, Ct	20 PSF 14 PSF 1.0 1.0 1.0	
4.	А. В. С. D.	WIND DESIGN DATA ULTIMATE DESIGN WIND SPEED (3 SEC WIND EXPOSURE CATEGORY INTERNAL PRESSURE COEFFICIENT, GC DESIGN WIND PRESSURE ON COMPONE	OND GUST), Vult Cpi ENTS AND CLADDING	109 MPH C +/- 0.18
		ROOF PRESSURES (1.0W)	WALL PRESSURES (1.0W)	
		EFFECTIVE WIND AREA	EFFECTIVE WIND AREA	
	ZC ZC ZC ZC ZC NOT 1. 2. 3.	Image: Sign of Sign Free Processing           Image: Sign of Sign of Sign Free Processing           Image: Sign of	ZONE 4 -25.7 PSF -19.8 PSF ZONE 5 -31.6 PSF -19.8 PSF ZONE 4 & 5 23.7 PSF 17.8 PSF JTARY AREAS NOT LISTED S ACTING TOWARD THE NOTED SURFACE S ACTING AWAY FOR THE NOTED SURFACE	AND CE
	E.	WIDTH OF END ZONE		3.0 FT
5.	A. B. C. E. F. G.	EARTHQUAKE DESIGN DATA SEISMIC IMPORTANCE FACTOR, le MAPPED SPECTRAL RESPONSE ACCELE SITE CLASS DESIGN SPECTRAL RESPONSE ACCELE DESIGN SPECTRAL RESPONSE ACCELE SEISMIC DESIGN CATEGORY STRUCTURAL SYSTEM 1.) VERTICAL ELEMENT TYPE	ERATION PARAMETER, Ss ERATION PARAMETER, S1 RATION PARAMETER, Sds RATION PARAMETER, Sd1	1.0 10.6% 6.3% D 11.3% 10.1% B BUILDING FRAME
		2.) BASIC SEISMIC FORCE-RESISTING S	YSTEM TYPE	SYSTEM LIGHT FRAME (WOOD WALLS) WITH SHEAR
	J.	3.) RESPONSE MODIFICATION FACTOR, 4.) SEISMIC RESPONSE COEFFICIENT, C 5.) DESIGN BASE SHEAR, 1.0E ANALYSIS PROCEDURE	R Ss	PANELS - WOOD 6.5 0.016 3.2k EQUIVALENT LATERAL FORCE

### **GENERAL NOTES**

### GENERAL

- STRUCTURAL ELEMENTS ARE NON-SELF SUPPORTING AND REQUIRE INTERACTION WITH OTHER ELEMENTS FOR STABILITY AND RESISTANCE TO LATERAL FORCES. FRAMING AND WALLS SHALL BE TEMPORARILY BRACED BY THE CONTRACTOR UNTIL PERMANENT BRACING, FLOOR AND ROOF DECKS, AND WALLS HAVE BEEN INSTALLED AND CONNECTIONS BETWEEN THESE ELEMENTS HAVE BEEN MADE. THE CONTRACT DOCUMENTS REPRESENT THE FINISHED STRUCTURE AND DO NOT INDICATE THE METHOD OF CONSTRUCTION, UNLESS NOTED OTHERWISE. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR THE MEANS, METHODS, TECHNIQUES, SEQUENCES, AND OPERATION OF CONSTRUCTION AND SAFETY PRECAUTIONS AND PROGRAMS INCIDENTAL THERETO.
- THE STRUCTURE HAS BEEN DESIGNED FOR THE INDICATED LOADS ONLY. USE OF HEAVY EQUIPMENT AND SCAFFOLDING, OR STORAGE OF MATERIALS THAT TRANSFER EXCESSIVE LOADS TO THE STRUCTURE SHALL BE VERIFIED BY THE CONTRACTOR. THE CONTRACTOR SHALL PROVIDE CALCULATIONS SIGN AND SEALED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE WHERE THE PROJECT IS LOCATED TO VERIFY THE ADEQUACY OF THE STRUCTURE FOR ALL APPLIED CONSTRUCTION LOADS THAT EXCEED THE LOADS INDICATED IN THE CONSTRUCTION DOCUMENTS AND SHALL BE APPROVED BY THE ARCHITECT AND ENGINEER-OF-RECORD PRIOR TO ANY CONSTRUCTION ACTIVITY
- THE SPECIFICATIONS ARE AN INTEGRAL PART OF THE CONTRACT DOCUMENTS AND SHALL BE USED IN CONJUCTION WITH THE CONTRACT DRAWINGS. WHERE REQUIREMENTS INDICATED ON THE CONTRACT DRAWINGS DIFFER FROM THE SPECIFICATIONS, NOTIFY THE ARCHITECT AND THE ENGINEER-OF-RECORD.
- STRUCTURAL DRAWINGS ARE NOT STAND-ALONE DOCUMENTS AND ARE INTENDED TO BE USED IN CONJUNCTION WITH CIVIL, ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING AND DRAWINGS FROM OTHER DISCIPLINES. THE CONTRACTOR SHALL COORDINATE ALL REQUIREMENTS OF THE CONTRACT DOCUMENTS INTO SHOP DRAWINGS AND WORK.
- ALL WELDS SHALL BE PERFORMED BY QUALIFIED WELDERS IN ACCORDANCE WITH AMERICAN WELDING SOCIETY (A.W.S) SPECIFICATIONS. THE SIZE AND LOCATION OF EQUIPMENT PADS AND PENETRATIONS THROUGH THE STRUCTURE FOR
- MECHANICAL, ELECTRICAL, AND PLUMBING WORK SHALL BE VERIFIED BY THE CONTRACTOR. PENETRATIONS SHALL BE SUBJECT TO APPROVAL BY THE ARCHITECT AND THE ENGINEER-OF-RECORD. REFERENCE MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS FOR OPENING LOCATIONS NOT SHOWN ON THE STRUCTURAL DRAWINGS.
- USE ONLY DIMENSIONS INDICATED IN THE CONTRACT DOCUMENTS. DO NOT SCALE CONTRACT DOCUMENTS OR USE ANY DIMENSIONS TAKEN FROM ELECTRONIC DRAWING FILES. CONTRACTOR SHALL COORDINATE IN-PLACE DIMENSIONS BASED ON TOLERANCES OF THE RESPECTIVE TRADES.

	GENE	RAL NOTES		
	OLIVE			
9. 10.	ASSUME EQUAL SPACING IF NOT INDICATE ARCHITECTURAL. MECHANICAL AND ELEC	ED IN CONTRACT DOCUMENT TRICAL COMPONENTS AND S	S. YSTEMS SHALL BE DESIGNED	3
11.	AND CONSTRUCTED TO RESIST SEISMIC F REFERENCE ARCHITECTURAL DRAWINGS OF NON-LOAD BEARING PARTITION FRAMI	FORCES AS DETERMINED IN C FOR NON-LOAD BEARING PA NG TO THE PRIMARY STRUCT	CHAPTER 13 OF ASCE 7. RTITION FRAMING. CONNECTION TURE SHALL ALLOW FOR	4
12.	CONTRACTOR SHALL COORDINATE ALL DI ANCHOR RODS AND EMBED LOCATIONS P	MENSIONS, OPENING, BLOCK RIOR TO CONSTRUCTION.	G. (OUTS, RECESSES, ELEVATIONS,	5
	FOUNDATIONS			
1.	FOUNDATION DESIGNS AND SUBGRADE P RECOMMENDATIONS PROVIDED IN THE GE ENGINEERING, LLC.DATED: 03/21/2022	REPARATION NOTES ARE BA EOTECHNICAL REPORT NUME	SED ON THE BER 22-5193, BY: CFS	6
2. 3.	FOOTING DESIGNS ARE BASED ON A NET CONTRACTOR AND TESTING LABORATOR REPORT AND BECOME THOROUGHLY FAN THEREIN. THE CONTRACTOR SHALL BE RE AND FILL FOR ESTIMATING AND CONSTRU STRUCTURAL EARTH MOVING SPECIFICAT	[MAX] ALLOWABLE SOIL BEAF Y REPRESENTATIVE SHALL R /IILIAR WITH SITE AND SUBGF ESPONSIBLE FOR DETERMINI CTION. SUBGRADE SHALL BE TION.	RING CAPACITY OF 2500 PSF. EAD THE GEOTECHNICAL RADE INFORMATION GIVEN NG EXACT QUANTITIES OF CUT PREPARED AS NOTED IN THE	7 8 9
4.	A QUALIFIED AND REGISTERED GEOTECH PROJECT IS LOCATED AND WORKING FOR CONFORMANCE OF THE FOUNDATION BEA ABOVE, AND ALL OTHER CONTRACT DOCU ARCHITECT AND ENGINEER-OF-RECORD O DESIGN CRITERIA OR CONTRACT DOCUME	NICAL ENGINEER, LICENSED R THE TESTING LABORATORY ARING STRATA WITH THE FOU JMENTS. TESTING LABORATO OF ANY CONDITIONS NOT IN A ENTS.	IN THE STATE WHERE THE SHALL DETERMINE INDATION DESIGN CRITERIA IRY SHALL NOTIFY CONTRACTOR, CCORDANCE WITH FOUNDATION	1
5.	THE CONTRACTOR SHALL BE RESPONSIBI	LE FOR DETERMINING EXACT	QUANTITIES OF CUT AND FILL	1
6.	AVOID DAMAGE TO UNDERGROUND UTILIT SANITARY SEWERS AND BURIED CABLES	FIES INCLUDING, BUT NOT LIM WHICH MIGHT EXTEND ACRO	IITED TO, WATER MAINS, SS OR ADJOIN SITE.	1
	CONCRETE			1
1.	MINIMUM COMPRESSIVE STRENGTH (fc) A	T THE END OF 28 DAYS SHAL	L BE AS FOLLOWS:	
	A. FOOTINGS B. INTERIOR SLABS-ON-GRADE		4500 PSI 3000 PSI	1
	MAXIMUM WATER/CEMENT RATIOS SHALL	BE 0.45. CONCRETE SHALL B	E NORMAL WEIGHT (145 PCF),	
2.	EXTERIOR CONCRETE AND CONCRETE EX WITH 6% (±1.5%) ENTRAINED AIR BY VOLU FLOOR TO EXCEED 3%.	POSED TO FREEZE-THAW CY ME. DO NOT ALLOW AIR CON	CLES SHALL BE AIR-ENTRAINED	2
3. 4.	MATERIALS OR ADMIXTURES SHALL NOT ( REINFORCING STEEL SHALL MEET THE FC	CONTAIN ANY CALCIUM CHLO DLLOWING:	RIDE.	3
		ASTM	I SPECIFICATION	
	A. DEFORMED BARS B. WELDABLE DEFORMED BARS C. WELDED WIRE REINFORCEMENT	A6 A7	615, GRADE 60 706, GRADE 60 A1064	
5.	PROVIDE MINIMUM CONCRETE CLEAR CO	VER FOR REINFORCEMENT P	ER ACI 318, UNLESS NOTED	
6.	WELDING SHALL MEET ANSI / AWS D1.1, S "STRUCTURAL WELDING CODE FOR REINF DEFORMED BAR ANCHORS SHALL BE 90 K	TRUCTURAL WELDING CODE ORCING STEEL" LATEST REV	AND ANSI / AWS D1.4 ISION. ELECTRODES FOR	
7.	WHERE DOWELS ARE INDICATED BUT NOT OF MAIN REINFORCING STEEL AND LAP SP STEEL SHALL BE SPLICED AS NOTED IN TH	T SIZED, PROVIDE DOWELS T PLICE WITH THE MAIN REINFO HE REINFORCING LAP SCHED	HAT MATCH SIZE AND LOCATION ORCING STEEL. REINFORCING ULE.	4
8.	"C.J." INDICATES SAW CUT CONTRACTION SLAB-ON-GRADE. REFERENCE CAST-IN-PL METHODS. SLAB POURS SHALL BE SEPAR CONTRACTION/CONSTRUCTION JOINTS SH THE ENGINEER-OF-RECORD.	JOINT OR DOWELED CONST ACE CONCRETE SPECIFICAT ATED BY A DOWELED CONST HALL BE LOCATED AS SHOWN	RUCTION JOINT IN ION FOR ACCEPTED SAW CUT RUCTION JOINT. I ON PLANS OR AS DIRECTED BY	6
9. 10.	PROVIDE CORNER BARS THAT MATCH ANI INTERSECTIONS AND CORNERS OF WALLS ANCHOR BOLTS AND EMBED PLATES SHA	D LAP CONTINUOUS REINFOR S AND FOUNDATIONS. LL BE TIED INTO THE REINFO	CEMENT SIZE AND QUANTITY AT	8
	PLACE WITH A RIGID TEMPLATE TO PREVE	ENT MOVEMENT DURING CON	CRETE PLACEMENT.	9
	STRUCTURAL STEEL			
1.	STRUCTURAL STEEL SHALL MEET THE FO	LLOWING MINIMUM YIELD STF YIELD	RESS (Fy): ASTM SPECIFICATION	
	<ul><li>A. BARS, PLATES, CHANNELS, ANGLES:</li><li>B. SQUARE, RECTANGULAR HSS:</li><li>C. ANCHOR RODS:</li></ul>	36 KSI 50 KSI 36 KSI, WELDABLE	A36 A500, GRADE C F1554	1
2.	BOLTS FOR STEEL BEAM AND COLUMN CO	ONNECTIONS SHALL BE 3/4-IN	CH DIAMETER (MIN.) ASTM F3125,	1
3.	GRADE A325-N HIGH-STRENGTH BOLTS UN ALL BOLTED JOINTS SHALL BE SNUG TIGH FOR PRETENSIONED OR SLIP-CRITICAL JC WITH MATCH MARKING, TWIST-OFF-TYPE E1852) OR DIRECT TENSION INDICATORS	VLESS NOTED OTHERWISE IN T UNLESS NOTED OTHERWIS DINTS, THE METHOD OF INSTA TENSION CONTROL BOLT ASS (ASTM F959)	CONTRACT DOCUMENTS. E IN CONTRACT DOCUMENTS . ALLATION SHALL BE TURN-OF-NUT SEMBLIES (ASTM F3125, GRADE	
4.	WELDING SHALL MEET ANSI / AWS D1.1, S SHALL BE 70 KSI, LOW HYDROGEN.	TRUCTURAL WELDING CODE	LATEST REVISION. ELECTRODES	1
5.	WELDS NOT SPECIFICALLY SIZED ON THE THE LATEST AWS D1.1.	STRUCTURAL DRAWINGS SH	ALL BE THE MINIMUM SIZE PER	
6.	PROVIDE DOUBLE NUTS AND DOUBLE WAS ADJUSTMENT IN BASE PLATE ELEVATION. AFTER ERECTION. USE 2 1/2 INCH NON-SH DIAMETER OR LARGER. NON-SHRINK GRO	SHERS FOR STEEL COLUMN / PROVIDE 1 1/2 INCH NON-SHI RINK GROUT WHEN COLUMN UT SHALL BE NON-METALLIC	ANCHOR BOLTS TO ALLOW FOR RINK GROUT UNDER BASE PLATE ANCHOR BOLTS ARE 1 1/4 INCH WITH A MINIMUM COMPRESSIVE	2
7.	LEDGER ANGLES AND LINTELS IN EXTERIO	OR WALL SYSTEMS SHALL BE	HOT DIP GALVANIZED PER ASTM	3
8.	AL23. ALL CONNECTIONS NOT FULLY DETAILED DETAILED BY A PROFESSIONAL ENGINEER THE CONNECTION DESIGN ENGINEER SHA THE DESIGN AND DETAILING SHALL COMP	IN THE CONTRACT DOCUMEN R LICENSED IN THE STATE WH ALL BE EMPLOYED OR RETAIN LY WITH ALL APPLICABLE CO	ITS SHALL BE DESIGNED AND IERE THE PROJECT IS LOCATED. IED BY THE STEEL FABRICATOR. DES AND SPECIFICATION	
9.	THE GENERAL CONTRACTOR SHALL BE RE MISCELLANEOUS STEEL SHOWN IN THE C ARE NOT LIMITED TO, MISCELLANEOUS ST	ESPONSIBLE FOR INCLUDING ONTRACT DOCUMENTS. THE EEL ITEMS SHOWN ON THE S	THE COSTS FOR ALL SE COSTS SHALL INCLUDE, BUT STRUCTURAL, ARCHITECTURAL,	4
10.	AT ALL GALVANIZED OR PAINTED STEEL M GALVANIZING, PAINT OR PRIMER PRIOR TO COMPLETE AND INSPECTOR APPROVED F	INICAL DRAWINGS AND IN TH IEMBERS WITH FIELD WELDE O FIELD WELDING AS REQUIR PREPARE AND REPAINT THE F	IL SECURICATIONS. D CONNECTIONS, REMOVE ED. AFTER WELDING IS FRAMING SURFACES	5

### WOOD FRAMING

- WOOD FRAMING SHALL MEET THE NDS MINIMUM STRESS PROPERTIES UNLESS NOTED OTHERWISE IN CONTRACT DOCUMENTS
- A. DOUGLAS FIR SOUTH #2 OR BETTER, PER THE NDS B. GLULAM BEAMS: 24F-V4 DF/DF, PER THE NDS
- 1. E = 1,800,000 PSI
- C. LAMINATED VENEER LUMBER (LVL): 1. Fb = 2,600 PSI
- 2. Ft = 1,555 PSI
- . 3. Fc = 2,510 PSI (PARALLEL TO GRAIN)
- . 4. Fc = 750 PSI (PERPENDICULAR TO GRAIN)
- 5. Fv = 285 PSI (PARALLEL TO GRAIN)

6. E = 2,000,000 PSI 2. PROVIDE SIMPSON STRONG-TIE CONNECTORS OR EQUIVALENT FOR WOOD FRAMING CONNECTING TO SUPPORTING MEMBERS. INSTALL WITH MANUFACTURER'S SPECIFIED FASTENERS ACCORDING TO THE MANUFACTURER'S SPECIFICATIONS. SUBSTITUTION REQUESTS FOR CONNECTORS SHALL BE SUBMITTED BY THE CONTRACTOR TO THE STRUCTURAL ENGINEER-OF-RECORD ALONG WITH CALCULATIONS THAT ARE SIGNED AND SEALED BY A QUALIFIED PROFESSIONAL ENGINEER RESPONSIBLE FOR THEIR PREPARATION AND LICENSED IN THE STATE WHERE THE PROJECT IS LOCATED. THE CALCULATIONS SHALL DEMONSTRATE THAT THE SUBSTITUTED PRODUCT IS CAPABLE OF ACHIEVING THE PERTINENT EQUIVALENT PERFORMANCE VALUES OF THE SPECIFIED PRODUCT USING THE APPROPRIATE DESIGN PROCEDURE AND/OR STANDARDS AS REQUIRED BY THE BUILDING CODE.

# GENERAL NOTES

- ALL ROOF, FLOOR AND EXTERIOR WALL SHEATHING SHALL BE APA RATED EXPOSURE 1 SHEATHING [U.N.O.] AND SHALL BEAR THE STAMP OF AN APPROVED TESTING AGENCY.
- INSTALL ROOF AND FLOOR SHEATHING WITH THE LONG DIMENSION OF THE PANEL PERPENDICULAR TO SUPPORTS UNLESS NOTED OTHERWISE IN CONTRACT DOCUMENTS, AND WITH PANEL CONTINUOUS OVER TWO OR MORE SPANS. STAGGER END JOINTS, UNLESS NOTED OTHERWISE. SPACE PANELS 1/8" APART AT EDGES AND ENDS.
- ALL ROOF SHEATHING SHALL HAVE A MINIMUM THICKNESS OF 15/32 INCH WITH A SPAN RATING OF AT LEAST 32/16 AND BE FASTENED TO ROOF FRAMING NAILED WITH 8d GALVANIZED COMMON NAILS AT 6" O.C. AT PANEL EDGES AND 12 INCHES ON CENTER AT INTERMEDIATE SUPPORTS. 8d COMMON NAILS SHALL HAVE A MINIMUM 0.131 INCH DIAMETER AND 1 3/8 INCH MINIMUM PENETRATION INTO SUPPORTING FRAMING.
- ALL EXTERIOR WALL SHEATHING SHALL HAVE A MINIMUM THICKNESS OF 5/16 INCH AND BE FASTENED TO WALL STUDS WITH 6d GALVANIZED COMMON NAILS AT 6 INCHES ON CENTER AT EDGES AND 12 INCHES ON CENTER AT INTERMEDIATE SUPPORTS. [PROVIDE BLOCKING AT UNSUPPORTED PANEL EDGES]. 6d COMMON NAILS SHALL HAVE A MINIMUM 0.113 INCH DIAMETER AND 1 1/4 INCH MINIMUM PENETRATION INTO SUPPORTING FRAMING.
- INSTALL ALL JOISTS, RAFTERS, HEADERS AND BEAMS CROWN UP.
- REFERENCE DETAILS FOR CUTTING, BORING OR NOTCHING OF FRAMING MEMBERS, WALL STUDS AND TOP PLATES. FASTEN PLIES OF ENGINEERED WOOD PRODUCTS TOGETHER PER THE MANUFACTURER'S
- RECOMMENDATIONS OR AS DETAILED IN THE CONSTRUCTION DOCUMENTS. REFERENCE BUILT-UP COLUMN AND BEAM DETAILS FOR NAILED BUILT-UP COLUMN AND BEAM REQUIREMENTS. SPLICES IN MULTIPLE BUILT-UP MEMBERS ARE NOT PERMITTED, U.N.O. PROVIDE SLIP CONNECTION AT TOP OF ALL NON-LOAD BEARING WALLS TO ALLOW FOR 1 1/2"
- DEFLECTION OF FRAMING ABOVE. ALL WOOD IN CONTACT WITH CONCRETE AND EXTERIOR MASONRY SHALL BE PRESERVATIVE
- TREATED
- ALL STEEL CONNECTORS AND FASTENERS USED WITH PRESERVATIVE TREATED WOOD SHALL BE GALVANIZED (G90).
- NAILING SHALL COMPLY WITH REQUIREMENTS OF NAILING SCHEDULE UNLESS NOTED OTHERWISE IN CONTRACT DOCUMENTS.
- ALL NAILS SHOWN ON PLAN ARE 'COMMON', UNLESS NOTED OTHERWISE. REFERENCE NAIL SIZE SCHEDULE FOR REQUIRED COMMON NAIL SIZES.

SHOP-FABRICATED WOOD TRUSSES

- ALL TRUSSES SHALL BE PRE-ENGINEERED AND SHOP FABRICATED. TRUSSES AND CONNECTIONS SHALL BE DESIGNED IN ACCORDANCE WITH THE LATEST REVISION OF THE AF&PA N.D.S. TRUSSES SHALL CONFORM TO THE SPACING. DIMENSIONS AND CONFIGURATIONS SPECIFIED IN THESE NOTES AND ON THE PLANS AND SHALL BE DESIGNED FOR ALL SPECIFIED LOADS. FRAMING PLANS INDICATE THE REQUIRED BASIC TRUSS LAYOUT. SIGNIFICANT DEVIATIONS FROM THESE PLANS WILL NOT BE PERMITTED.
- TRUSS MANUFACTURER SHALL SUBMIT STRUCTURAL CALCULATIONS AND SHOP DRAWINGS, INCLUDING AN ERECTION DRAWING SHOWING TRUSS LAYOUT PREPARED AS NOTED IN THE DEFERERED SUBMITTAL NOTE.
- ROOF TRUSS DESIGN LOADS (ASD):
- A. TOP CHORD
- . 1. ROOF LIVE LOAD = 20 PSF . 2. DEAD LOAD = 9 PSF
- . 3. SNOW LOAD = JOIST LOADING DIAGRAMS.
- B. BOTTOM CHORD
- . 1. DEAD LOAD = 8 PSF . 2. LIVE LOAD = AS REQUIRED PER GOVERNING BUILDING CODE
- C. SELF WEIGHT OF THE TRUSSES SHALL BE ADDED TO THE ABOVE LOADS.
- D. WIND LOADS = RE: DESIGN PARAMETERS
- E. ROOF TRUSS SHALL BE DESIGNED TO LIMIT THE MAXIMUM LIVE LOAD DEFLECTION TO SPAN/240 AND MAXIMUM TOTAL LOAD DEFLECTION TO SPAN/180 F. REFERENCE PLANS AND DETAILS FOR ADDITIONAL DESIGN LOADS OR SHEAR TRUSS/BLOCKING
- REQUIREMENTS
- TRUSS TOP CHORD SHALL BE DOUGLAS FIR SOUTH OR SOUTHERN PINE.
- TRUSSES SHALL BE FABRICATED WITH MINIMUM 20 GAUGE TRUSS PLATES HAVING A MINIMUM WOOD PENETRATION OF 0.37 INCH.
- TRUSSES AND CONNECTOR PLATES SHALL BE DESIGNED IN ACCORDANCE WITH THE LATEST REVISION OF ANSI/TPI 1: NATIONAL DESIGN STANDARD FOR METAL PLATE CONNECTED WOOD TRUSS CONSTRUCTION.
- TRUSS BEARING POINTS SHALL BE PINNED FOR THE DESIGN OF THE TRUSSES.
- CEILINGS WILL NOT BRACE BOTTOM CHORDS. TRUSS MANUFACTURER SHALL DESIGN PERMANENT BOTTOM CHORD BRIDGING TO BE SUPPLIED AND INSTALLED BY THE CONTRACTOR.
- TRUSS DESIGNER SHALL DESIGN AND SPECIFY THE TRUSS TO TRUSS AND THE TRUSS TO SUPPORT CONNECTIONS, U.N.O. ON THE DETAILS. PROPER ERECTION BRACING SHALL BE INSTALLED TO HOLD THE TRUSSES TRUE AND PLUMB AND IN
- SAFE CONDITION UNTIL PERMANENT TRUSS BRACING AND BRIDGING HAVE BEEN INSTALLED TO FORM A STRUCTURALLY SOUND FRAMING SYSTEM. ALL ERECTION AND PERMANENT BRACING SHALL BE INSTALLED AND ALL COMPONENTS PERMANENTLY FASTENED BEFORE THE APPLICATION OF ANY LOADS TO THE TRUSSES. ALL BRACING SHALL BE DESIGNED BY MANUFACTURER AND INDICATED ON SHOP DRAWINGS. ALL PREFABRICATED WOOD TRUSSES ARE TO BE INSTALLED IN ACCORDANCE WITH THE BUILDING COMPONENT SAFETY INFORMATION (BCSI), "GUIDE TO GOOD PRACTICE FOR HANDLING, INSTALLING & BRACING OF METAL PLATE CONNECTED WOOD TRUSSES".
- TRUSS MANUFACTURER SHALL ARRANGE TRUSS WEB MEMBERS AS REQUIRED BY DESIGN. REFERENCE ARCH. AND MECH. DRAWINGS FOR DUCT LAYOUT. PROVIDE CHASES IN TRUSSES TO ACCOMODATE DUCTS AS REQUIRED.
- DO NOT CUT, NOTCH OR OTHERWISE ALTER THE TRUSSES WITHOUT WRITTEN PERMISSION FROM THE FABRICATOR AND THE STRUCTURAL ENGINEER OR RECORD.

POST INSTALLED ANCHORS

- ANCHORS SHALL ONLY BE INSTALLED WHERE SPECIFIED ON THE CONTRACT DRAWINGS. THE CONTRACTOR SHALL OBTAIN APPROVAL FROM THE STRUCTURAL ENGINEER PRIOR TO INSTALLING POST INSTALLED ANCHORS IN PLACE OF MISSING OR MIS-PLACED CAST-IN-PLACE ANCHORS. CARE SHALL BE TAKEN IN PLACING POST-INSTALLED ANCHORS TO AVOID CONFLICTS WITH EXISTING REINFORCING. ANY CONFLICTS SHALL BE BROUGHT TO THE ATTENTION OF THE EOR PRIOR TO COMPLETION OF WORK.
- THE CONTRACTOR SHALL SUBMIT PRODUCT DATA WITH DESIGN VALUES AND PHYSICAL PROPERTIES FOR ALL POST INSTALLED ANCHORS. ADDITIONALLY, THE CONTRACTOR SHALL SUBMIT CERTIFIED ICC ES OR ESR REPORTS WHICH VERIFY COMPLIANCE WITH THE SPECIFIED CRITERIA.
- SUBSTITUTION REQUESTS FOR PRODUCTS OTHER THAN THOSE SPECIFIED ON THE CONTRACT DRAWINGS SHALL BE SUBMITTED BY THE CONTRACTOR TO THE STRUCTURAL ENGINEER ALONG WITH CALCULATIONS THAT ARE SIGNED AND SEALED BY A QUALIFIED PROFESSIONAL ENGINEER RESPONSIBLE FOR THEIR PREPARATION AND LICENSED IN THE STATE WHERE THE PROJECT IS LOCATED. THE CALCULATIONS SHALL DEMONSTRATE THAT THE SUBSTITUTED PRODUCT IS CAPABLE OF ACHIEVING THE PERTINENT EQUIVALENT PERFORMANCE VALUES OF THE SPECIFIED PRODUCT USING THE APPROPRIATE DESIGN PROCEDURE AND/OR STANDARDS AS REQUIRED BY THE BUILDING CODE
- ALL HOLES SHALL BE DRILLED, DRY AND CLEANED AND ANCHORS SHALL BE INSTALLED IN ACCORDANCE PER ANCHOR MANUFACTURER'S WRITTEN SPECIFICATIONS. THE LATEST VERSION OF THE WRITTEN SPECIFICATION SHALL BE ON-SITE AND FOLLOWED DURING THE INSTALLATION OF THE ANCHORS.
- THE ANCHOR EMBEDMENT DEPTH SHALL BE DEFINED AS THE DEPTH FROM THE SURFACE FACE OF THE LOAD BEARING BASE MATERIAL TO THE DEEPEST PART OF THE ANCHOR AFTER THE ANCHOR HAS BEEN FULLY INSTALLED INTO THE HOLE PER MANUFACTURER'S SPECIFICATIONS.
- ANCHORS EXPOSED TO WEATHER SHALL BE STAINLESS STEEL. CONTRACTOR SHALL FOLLOW THE LATEST VERSION OF MANUFACTURER'S SPECIFICATION DURING INSTALLATION OF ANCHORS.
- OVERHEAD ADHESIVE ANCHORS MUST BE INSTALLED BY PERSONNEL CERTIFIED BY THE ACI/CRSI ADHESIVE ANCHOR INSTALLER CERTIFICATION PROGRAM.

DEFERRED STRUCTURAL SUBMITTALS (IBC 2018 SECTION 107.3.4.1)

THE FOLLOWING STRUCTURAL COMPONENTS SHALL BE DESIGNED AND SUBMITTED BY OTHERS FOR APPROVAL IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.

A. STRUCTURAL STEEL CONNECTIONS OF FRAMING AND BRACING ELEMENTS. B. STEEL, SELF-SUPPORTING STAIRS.

DOCUMENTS FOR DEFERRED STRUCTURAL SUBMITTAL ITEMS SHALL BE DESIGNED, SEALED AND SIGNED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE WHERE THE PROJECT IS LOCATED. THE DEFERRED SUBMITTAL DOCUMENTS SHALL BE SUBMITTED TO THE ARCHITECT OR ENGINEER-OF-RECORD WHO SHALL REVIEW THEM AND FORWARD THEM TO THE BUILDING OFFICIAL AS REQUESTED WITH A NOTATION INDICATING THAT THE DEFERRED SUBMITTAL DOCUMENTS HAVE BEEN REVIEWED FOR DESIGN LOADS AND BEEN FOUND TO BE IN GENERAL CONFORMANCE TO THE DESIGN CRITERIA OF THE BUILDING. THE DEFERRED SUBMITTAL ITEMS SHALL NOT BE INSTALLED UNTIL THE DESIGN AND SUBMITTAL DOCUMENTS HAVE BEEN APPROVED BY THE BUILDING OFFICIAL.



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CLIENT: COVENANT GROUP, LLC
PROJECT:
BUILDING SHELL - LEE'S SUMMIT, MO - CHIPMAN RD
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SHEET INFO ISSUE DATE : 03/31/2022 ISSUED FOR:PERMIT SET REVISION SCHEDULE
NO DESCRIPTION DATE
NUMBER PE-2014023909 Missouri COA #001268
GENERAL NOTES

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IBC 2018	REQUIRED	SPECIAL	<b>INSPECTIO</b>	NS

			FREQUENCY O	F INSPECT
			CONTINUOUS	PERIODI
	0.T.F			
1		EL CONSTRUCTION - STRUCTURAL STEEL (IBC SECTION 1705.2.1)	1	
1.		STRUCTURES AND PORTIONS THEREOF SHALL BE IN ACCORDANCE WITH THE QUALITY ASSURANCE INSPECTION REQUIREMENTS OF AISC 360-16.		
	CO	NCRETE CONSTRUCTION (IBC TABLE 1705.3)		
1.		INSPECT REINFORCEMENT, INCLUDING PRESTRESSING TENDONS, AND VERIFY PLACEMENT.		Х
2.		REINFORCING BAR WELDING:		
	Α.	VERIFY WELDABILITY OF REINFORCING BARS OTHER THAN ASTM A706;		Х
	В.	INSPECT SINGLE-PASS FILLET WELDS, MAXIMUM 5/16"; AND		Х
	C.	INSPECT ALL OTHER WELDS	X	
3.		INSPECT ANCHORS CAST IN CONCRETE.		Х
4.		INSPECT ANCHORS POST-INSTALLED IN HARDENED CONCRETE MEMBERS. (a)		
	Α.	ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY OR UPWARDLY INCLINED ORIENTATIONS TO RESIST SUSTAINED TENSION LOADS.	X	
	В.	MECHANICAL ANCHORS AND ADHESIVE ANCHORS NOT DEFINED IN 4 A.		Χ
5.		VERIFY USE OF REQUIRED DESIGN MIX.		Χ
6.		PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE.	Х	
7.		INSPECT CONCRETE AND SHOTCRETE PLACEMENT OF PROPER APPLICATION TECHNIQUES.	X	
8.		VERIFY MAINTENANCE OF SPECIFIED CUREING TEMPERATURE AND TECHNIQUES.		Х
9.		INSPECT FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED.		Χ
		a. SPECIFIC REQUIREMENTS FOR SPECIAL INSPECTION SHALL BE INCLUDED IN THE RESEARCH REPORT FOR THE ANCHOR ISSUED BY AN APPROVED SOURCE IN ACCORDANCE WITH 17.8.2 IN ACI 318. WHERE SPECIFIC REQUIREMENTS ARE NOT PROVIDED, CONTACT THE STRUCTURAL ENGINEER-OF-RECORD FOR SPECIAL INSPECTION REQUIREMENTS.		
	wo	OD CONSTRUCTION - IBC SECTION 1705 5		
1.		SPECIAL INSPECTION OF THE FABRICATION PROCESS OF PREFABRICATED WOOD STRUCTURAL ELEMENTS AND ASSEMBLIES SHALL BE IN ACCORDANCE WITH SECTION 1704.2.5. SPECIAL INSPECTION OF SITE BUILT ASSEMBLIES SHALL BE IN ACCORDANCE WITH SECTION 1705.5		
2		INSPECTION OF WOOD STRUCTURAL PANEL SHEATHING GRADE AND THICKNESS.		X
3.		VERIFICATION OF THE NOMINAL SIZE OF FRAMING MEMBERS AT ADJOINING PANEL EDGES AGREES WITH THE		X
		APPROVED CONSTRUCTION DOCUMENTS. (required at wood high load diaphragms designed in accordance with 2306.2.)		
4.		VERIFICATION OF THE NAIL OR STAPLE DIAMETER AND LENGTH, THE NUMBER OF FASTENER LINES AND THE SPACING BETWEEN FASTENERS IN EACH LINE AND AT EDGE MARGINS AGREES WITH THE APPROVED CONSTRUCTION DOCUMENTS.		X
5.		VERIFICATION THAT THE INSTALLATION OF THE PERMANENT INDIVIDUAL TRUSS RESTRAINT/BACKING HAS BEEN INSTALLED IN ACCORDANCE WITH THE APPROVED TRUSS SUBMITTAL PACKAGE FOR WOOD TRUSSES WITH OVERALL HEIGHTS OF 60 INCHES OR GREATER.		Х
6.		VERIFICATION THAT THE TEMPORARY INSTALLATION RESTRAINT/BRACING AND THE PERMANENT INDIVIDUAL TRUSS MEMBER RESTRAINT/BRACING ARE INSTALLED IN ACCORDANCE WITH THE APPROVED TRUSS SUBMITTAL PACKAGE AT METAL-PLATE-CONNECTED WOOD TRUSSES WITH A CLEAR SPAN OF 60'-0" OR GREATER.		Х
	SO	II S (IBC TABLE 1705.6)		
1.		VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY.		Х
2.		VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL.		Х
3.		PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS.		X
4.		VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF COMPACTED FILL.	X	
5.		PRIOR TO PLACEMENT OF COMPACTED FILL, OBSERVE SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY.		Х
		** CONTINUOUS SPECIAL INSPECTION: SPECIAL INSPECTION BY THE SPECIAL INSPECTOR WHO IS PRESENT WHEN AND WHERE THE WORK TO BE INSPECTED IS BEING PERFORMED.		
		** PERIODIC SPECIAL INSPECTION: SPECIAL INSPECTION BY THE SPECIAL INSPECTOR WHO IS INTERMITTENTLY PRESENT WHERE THE WORK TO BE INSPECTED HAS BEEN OR IS BEING PERFORMED.		



## AISC 360-16 SPECIAL INSPECTION REQUIREMENTS

1. QUALITY CONTROL (QC) SHALL BE PROVIDED BY THE FABRICATOR AND ERECTOR.

PROTECTED STORAGE PROVIDED FOR BOLTS, NUTS, WASHERS AND OTHER FASTENER COMPONENTS

2. QUALITY ASSURANCE (QA) SHALL BE PROVIDED BY OTHERS.

3. NONDESTRUCTIVE TESTIING (NDT) SHALL BE PERFORMED BY THE AGENCY OR FIRM RESPONSIBLE FOR QUALITY ASSURANCE (QA). 4. THE QUALITY ASSURANCE INSPECTOR (QAI) SHALL REVIEW MATERIAL TEST REPORTS AND CERTIFICATIONS AS LISTED IN SECTION N3.2 FOR COMPLIANCE

WITH THE CONSTRUCTION DOCUMENTS.

5. FOR WORK PERFORMED BY APPROVED FABRICATORS AND ERECTORS: A. QA INSPECTIONS, MAY BE WAIVED WHEN THE WORK IS PERFORMED IN A FABRICATING SHOP OR BY AN ERECTOR APPROVED BY THE AUTHORITY HAVING JURISDICTION (AHJ) TO PERFORM THE WORK WITHOUT QA.

B. NDT OF WELDS COMPLETED IN AN APPROVED FABRICATOR'S SHOP MAY BE PERFORMED BY THAT FABRICATOR WHEN APPROVED BY THE AHJ. WHEN THE FABRICATOR PERFORMS THE NDT, THE QA AGENCY SHALL REVIEW THE FABRICATOR'S NDT REPORTS.

C. AT COMPLETION OF FABRICATION, THE APPROVED FABRICATOR SHALL SUBMIT A CERTIFICATE OF COMPLIANCE TO THE AHJ STATING THAT THE MATERIALS SUPPLIED AND WORK PERFORMED BY THE FABRICATOR ARE IN ACCORDANCE WITH THE CONSTRUCTION DOCUMENTS.

D. AT COMPLETION OF ERECTION, THE APPROVED ERECTOR SHALL SUBMIT A CERTIFICATE OF COMPLIANCE TO THE AHJ STATING THAT THE MATERIALS SUPPLIED AND WORK PERFORMED BY THE ERECTOR ARE IN ACCORDANCE WITH THE CONSTRUCTION DOCUMENTS.

		AISC 360-16, CHAPTER N SPECIAL INSPECTION REQUIR		6			AISC 360-16, CHA
			FREQUENCY O				
N5.4	 4 -	L NSPECTION OF WELDING	F EIXI OIXIM	OBSERVE		A	ISC 360-16, TABLE N5.6-2 - INSPECTION
	A	NSC 360-16, TABLE N5.4-1 - INSPECTION TASKS PRIOR TO WELDING	1	V	1.		FASTENER ASSEMBLIES PLACED IN ALL
1. 2.		WELDER QUALIFICATION RECORDS AND CONTINUITY RECORDS	 X	× 	2. 3.	·	FASTENER COMPONENT NOT TURNED B
3.		MANUFACTURER CERTIFICATIONS FOR WELDING CONSUMABLES AVAILABLE	Х		4.		FASTENERS ARE PRETENSIONED IN ACC
4. 5.		WELDER IDENTIFICATION (TYPE/GRADE)		X X			
6.		FIT-UP OF GROOVE WELDS (INCLUDING JOINT GEOMETRY)			1	A	ISC 360-16, TABLE N5.6-3 - INSPECTION
	A. B.	JOINT PREPARATION DIMENSIONS (ALIGNMENT, ROOT OPENING, ROOT FACE, BEVEL)		X	1.		DOCUMENT ACCEPTANCE OR REJECTIC
	С.	CLEANLINESS (CONDITION OF STEEL SURFACES)		X			** PERFORM - PERFORM THESE TASKS F
	D. F	TACKING (TACK WELD QUALITY AND LOCATION) BACKING TYPE AND FIT (IF APPLICABLE)		X			INSPECTIONS.
7.		FIT-UP OF CJP GROOVE WELDS OF HSS, T-, Y- AND K-JOINTS WITHOUT BACKING (INCLUDING JOINT GEOMETRY)			NI	E 7 C	
	A.	JOINT PREPARATIONS		X	IN	5.7 - C	THER INSPECTION TASKS
	В. С.	CLEANLINESS (CONDITION OF STEEL SURFACES).		X	1.		INSPECTION OF GALVANIZED STEEL ST
	D.	TACKING (TACK WELD QUALITY AND LOCATION)		X			GALVANIZING.
8. 9.		FIT-UP OF FILLET WELDS		X	NI		
	Α.	DIMENSIONS (ALIGNMENT, GAPS AT ROOT)		Х		5.0 - C	THER INSPECTION TASKS
	B. C.	CLEANLINESS (CONDITION OF STEEL SURFACES) TACKING (TACK WELD QUALITY AND LOCATION)		X	1.		INSPECT THE STEEL TO VERIFY COMPLI
	10.		I	X	2.		INSPECT THE PLACEMENT OF ANCHOR COMPLIANCE WITH THE CONSTRUCTION
1	A 	NISC 360-16, TABLE N5.4-2 - INSPECTIONS DURING WELDING					ANCHOR ROD OR EMBEDDED ITEM, AND VERIFIED AND DOCUMENTED PRIOR TO
<u> '.</u>	А.	PACKAGING		X			
	В.			X			** PERFORM - PERFORM THESE TASKS F
∠. 3.		ENVIRONMENTAL CONDITIONS		X			INSPECTIONS.
	A.			X			
4.	<u>в.</u>	WELDING PROCEDURE SPECIFICATION (WPS) FOLLOWED		Х			
	Α.	SETTINGS ON WELDING EQUIPMENT		Х			
	B.	TRAVEL SPEED SELECTED WELDING MATERIALS		X			
	D.	SHIELDING GAS TYPE / FLOW RATE		X			
	E.	PREHEAT APPLIED		X			
	F.	INTERPASS TEMPERTURE MAINTAINED (MIN./MAX.)		X			
5.	0.	WELDING TECHNIQUES					
	A.	INTERPASS AND FINAL CLEANING		X			
	В. С.	EACH PASS WITHIN PROFILE LIMITATIONS EACH PASS MEETS QUALITY REQUIREMENTS		X			
6.		PLACEMENT AND INSTALLATION OF STEEL HEADED STUD ANCHORS	Х				
	Δ	NSC 360-16 TABLE N5 4-3 - INSPECTION TASKS AFTER WELDING					
1.		WELDS CLEANED		Х			
2.		SIZE, LENGTH AND LOCATION OF WELDS	Х				
3.	A.	CRACK PROHIBITION	X				
	В.	WELD/BASE-METAL FUSION	Х				
	C.	CRATER CROSS SECTION	X				
	E.	WELD SIZE	X				
	F.		X				
4.	<u> </u> .	ARC STRIKES	× ×				
5.			X				
6. 7	-	WELD ACCESS HOLES IN ROLLED HEAVY SHAPES AND BUILT-UP HEAVY SHAPES (c) BACKING REMOVED AND WELD TABS REMOVED (IF REQUIRED)	X X				
8.		REPAIR ACTIVITIES	X				
9.	$\vdash$		X	 V			
11.	-	ULTRASONIC TESTING (UT) ON ALL CJP GROOVE WELDS IN BUTT, T- AND CORNER JOINTS, IN MATERIALS 5/16 INCH	 X				
10	_	THICK OR GREATER (required in Risk Catgory III or IV)		~			
12.		INCH THICK OR GREATER (required in Risk Catgory II)		Α			
13.		THERMALLY CUT SURFACES OF ACCESS HOLES SHALL BE TESTED USING MAGNETIC PARTICLE TESTING (MT) OR	Х				
		WEB THICKNESS EXCEEDS 2 INCHES FOR BUILT-UP SHAPES					
14.		(see AISC 360-16, section N5-5c for additional special inspections for welded joints subject to fatigue)					
	(a)	THE FABRICATOR OR ERECTOR, AS APPLICABLE, SHALL MAINTAIN A SYSTEM BY WHICH A WELDER WHO HAS					
	()  /	WELDED A JOINT OR MEMBER CAN BE IDENTIFIED. STAMPS, IF USED, SHALL BE THE LOW STRESS TYPE.					
	(a)	K-AREA, VISUALLY INSPECT THE WEB K-AREA FOR CRACKS WITHIN 3 INCHES OF THE WELD.					
	(c)	AFTER ROLLED HEAVY SHAPES AND BUILT-UP HEAVY SHAPES HAVE BEEN WELDED, VISUALLY INSPECT THE WELD					
		** PERFORM - PERFORM THESE TASKS FOR EACH WELDED JOINT OR MEMBER.					
1		INSPECTIONS.					
	· · · ·						
	6 - II	NSPECTION OF HIGH-STRENGTH BOLTS					
N5.6	۵	NOU 300-10. TABLE NO.0-1 - INSPECTION TAGKS PRIOR TO BOLTING					
N5.(	A	MANUFACTURER'S CERTIFICATIONS AVAILABLE FOR FASTENER MATERIALS	Х				
N5.( 1. 2.		MANUFACTURER'S CERTIFICATIONS AVAILABLE FOR FASTENER MATERIALS FASTENERS MARKED IN ACCORDANCE WITH ASTM REQUIREMENTS	X	 X v			
N5.( 1. 2. 3.		MANUFACTURER'S CERTIFICATIONS AVAILABLE FOR FASTENER MATERIALS FASTENERS MARKED IN ACCORDANCE WITH ASTM REQUIREMENTS CORRECT FASTENERS SELECTED FOR THE JOINT DETAIL (GRADE, TYPE, BOLT LENGTH) IF THREADS ARE TO BE EXCLUDED FROM SHEAR PLANE	X  	 X X			
N5.0 1. 2. 3. 4.		MANUFACTURER'S CERTIFICATIONS AVAILABLE FOR FASTENER MATERIALS FASTENERS MARKED IN ACCORDANCE WITH ASTM REQUIREMENTS CORRECT FASTENERS SELECTED FOR THE JOINT DETAIL (GRADE, TYPE, BOLT LENGTH) IF THREADS ARE TO BE EXCLUDED FROM SHEAR PLANE CORRECT BOLTING PROCEDURES SELECTED FOR JOINT DETAIL	X  	 X X X			
N5.( <u>1.</u> <u>2.</u> <u>3.</u> <u>4.</u> <u>5.</u>		MANUFACTURER'S CERTIFICATIONS AVAILABLE FOR FASTENER MATERIALS FASTENERS MARKED IN ACCORDANCE WITH ASTM REQUIREMENTS CORRECT FASTENERS SELECTED FOR THE JOINT DETAIL (GRADE, TYPE, BOLT LENGTH) IF THREADS ARE TO BE EXCLUDED FROM SHEAR PLANE CORRECT BOLTING PROCEDURES SELECTED FOR JOINT DETAIL CONNECTING ELEMENTS, INCLUDING THE APPROPRIATE FAYING SURFACE CONDITION AND HOLE PREPARATION, IF SPECIFIED, MEET APPLICABLE REQUIREMENTS	X  	 X X X X X			

--- X



## Wallace Engineering Structural Consultants, Inc.

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PTER N SPECIAL INSPECTION REQUIREMENTS					
	FREQUENCY O	F INSPECTION			
	PERFORM	OBSERVE			
NS DURING BOLTING		<u> </u>			
HOLES AND WASHERS AND NUTS ARE POSITIONED AS REQUIRED		Х			
ONDITION PRIOR TO THE PRETENSIONING OPERATION		Х			
3Y THE WRENCH PREVENTED FROM ROTATING		Х			
CORDANCE WITH THE RCSC SPECIFICATION, PROGRESSING ID POINT TOWARD THE FREE EDGES		Х			
NS AFTER BOLTING					
ON OF BOLTED CONNECTIONS	Х				
FOR EACH BOLTED CONNECTION.					
N A RANDOM BASIS. OPERATIONS NEED NOT BE DELAYED PENDING THESE					
RUCTURAL MAIN MEMBERS EXPOSED CUT SURFACES OF GALVANIZED RS OF HSS SHALL BE VISUALLY INSPECTED FOR CRACKS SUBSEQUENT TO	Х				
ANCE WITH THE DETAILS SHOWN ON THE CONSTRUCTION DOCUMENTS.	Х				
RODS AND OTHER EMBEDMENTS SUPPORTING STRUCTURAL STEEL FOR N DOCUMENTS. THE DIAMETER, GRADE, TYPE AND LENGTH OF THE D THE EXTENT OR DEPTH OF EMBEDMENT INTO THE CONCRETE, SHALL BE PLACEMENT OF CONCRETE	Х				
FOR EACH CONNECTION.					
N A RANDOM BASIS. OPERATIONS NEED NOT BE DELAYED PENDING THESE					











STRUCTU	TRUCTURAL ELEMENT MINIMUM COMPRESSIVE STRENGTH (fc)								
300	0psi	400	0psi	4500psi					
P BARS	OTHER	TOP BARS	OTHER	TOP BARS	OTHER				
28"	22"	25"	19"	23"	18"				
38"	29"	33"	25"	31"	24"				
47"	36"	41"	31"	38"	30"				
56"	43"	49"	37"	46"	35"				
81"	63"	71"	54"	67"	51"				
93"	72"	81"	62"	76"	59"				
105"	81"	91"	70"	86"	66"				
118"	91"	102"	79"	96"	74"				



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TYPICAL NAILING SCHEDULE	
ONNECTION	NAILING
RUSS/JOIST/RAFTER TO SILL, TOP PLATE, OR GIRDER	(3)-8d TOENAIL
RIDGING OR BLOCKING TO JOIST	(2)-8d TOE NAIL EA. END
RIDGING OR BLOCKING BETWEEN JOIST TO TOP PLATE	(3)-8d TOE NAIL EA. END
OTTOM PLATE TO JOIST OR BLOCKING	16d AT 24" O.C.
TUD TO TOP OR BOTTOM PLATE	(2)-16d END NAIL / (4)-8d TOE NAIL
TUD TO STUD AND BUILT-UP CORNER STUDS	10d AT 16" O.C. FACE NAIL OR 16d AT 24" O.C., FACE NAIL
OUBLE TOP PLATES, MIN. 24" OFFSET OF END JOISTS	10d AT 12" O.C. FACE NAIL 16d AT 16" O.C. FACE NAIL
OUBLE TOP PLATES AT END JOISTS, MIN. 24" OFFSET	(8)-16d EA. SIDE OF JOINT FACE NAIL
OP PLATES, LAPS AND INTERSECTIONS	(2)-16d FACE NAIL
IM JOISTS TO TOP PLATE	8d AT 6" O.C. TOE NAIL
DISTS TO BAND JOIST OR RIM JOIST	(3)-16d END NAIL
UILT-UP HEADER (2x TO 2x WITH 1/2" MAX. SPACER)	16d AT 16" O.C., FACE NAIL EA. EDGE
OUBLE TRUSSES (2x TO 2x CHORD MEMBERS)	16d AT 16" O.C., FACE NAIL EA. CHORD
UILT-UP BEAMS, (2x LAYERS WITH 3 OR MORE PLYS) RE: NOTE 4)	20d AT 32" O.C. FACE NAIL AT TOP AND BOTTOM AND STAGGERED ON OPPOSITE SIDES. TWO NAILS AT EA. END AND AT EA. SPLICE
ONTINUOUS HEADER TO STUD	(4)-8d TOE NAIL
EILING JOIST TO TOP PLATE	(3)-8d TOE NAIL
EILING JOIST LAP OF PARTITION	(3)-16d FACE NAIL
EILING JOIST TO PARALLEL RAFTERS	AS REQUIRED PER IBC
OLLAR TIE TO PARALLEL RAFTERS	(3)-10d FACE NAIL
AFTER TO 2x RIDGE BEAM	(2) 16d END NAIL/(3) 10d TOENAIL
AFTER TO VALLEY OR HIP RAFTER	(2) 16d END NAIL/(3) 10d TOENAIL
' BRACE TO EA. STUD PLATE	(2)-8d FACE NAIL
EDGER STRIP	(3) 16d FACE NAIL AT EA. JOIST
'x6" SUBFLOOR OR LESS	(2)-8d FACE NAIL, EA. JOIST
/IDER THAN 1"x6" SUBFLOOR	(2)-8d FACE NAIL, EA. JOIST
' SUBFLOOR TO JOIST OR GIRDER	(2)-16d BLIND AND FACE NAIL
' PLANKS	(2)-16d FACE NAIL EA. BEARING
'x6" SHEATHING	(2)-8d FACE NAIL EA. BEARING
'x8" AND WIDER SHEATHING	(3)-8d FACE NAIL EA. BEARING

INSTALL SIMPSON

ST2215 STRAP

BOTH SIDES

THE ABOVE ARE MIN. NAILING REQUIREMENTS. REFER TO GENERAL NOTES, DETAILS, AND SCHEDULES FOR MORE STRINGENT REQUIREMENTS. RE: IBC FASTENING SCHEDULE FOR MINIMUM WOOD FASTENING REQUIREMENTS NOT SHOWN. PROVIDE ADDITIONAL ROW OF NAILS WHEN DEPTH IS 14" OR GREATER. PROVIDE HOT-DIPPED ZINC-COATED GALVANIZED NAILS AT EXTERIOR FACE OF WALLS. 5. RE: GENERAL NOTES AND SHEAR WALL SCHEDULE FOR SHEATHING ATTACHMENT.

RKI PROJECT INFO CLIENT: COVENANT GROUP, LLC PROJECT: BUILDING SHELL - LEE'S SUMMIT, MO - CHIPMAN RD ADDRESS: 400 NW CHIPMAN RD LEE'S SUMMIT, MO 64806 PROJECT NO: MAIN CONTACT CHRISTOPHER CLARK, AIA, NCARB 7701 E KELLOGG DR, STE 630 WICHITA, KS 67207 (316) 302-4472 chris@clarkitecture.net DEVELOPER SHEET INFO ISSUE DATE : 03/31/2022 ISSUED FOR:PERMIT SET **REVISION SCHEDULE** NO DESCRIPTION DATE F MI JAMES M. GRANICH S NUMBER \_PE-2014023909 JNAT -04/05/2022 Missouri COA #001268 FRAMING DETAILS

S4.0



1 WOOD STRUCTURAL WALL ELEVATION 3/4" = 1'-0"

3. SHEAR WALL HOLDOWNS LOCATED AT EACH WALL CORNER. RE: FOUNDATION PLAN

![](_page_7_Figure_3.jpeg)

NOTES: 1. RE: PLANS FOR ANCHOR BOLT AND HOLDOWN LOCATIONS. 2. ALL SHEATHING TO BE APA RATED, EXPOSURE I. HOLDDOWN EMBEDMENT DOES NOT INCLUDE SLAB-ON-GRADE THICKNESS

SHEAR WALL SCHEDULE								
	NA	ILING	ANCHORS					
HING PANEL	AT PANEL EDGES AND BOUNDARIES	AT INTERMEDIATE FRAMING MEMBERS	SILL PLATE TO CONCRETE	HOLDOWN ANCHORS (RE: PLANS FOR LOCATIONS)	BUILT-UP END STUDS			
6" WOOD TURAL PANEL NE SIDE	6d AT 6" O.C.	6d AT 12" O.C.	5/8" DIA. SIMPSON TITEN HD ANCHOR AT 48" O.C. WITH 6" EMBEDMENT	(1) SIMPSON HDU4-SDS2.5 HOLDOWN WITH 5/8" DIA. HILTI HIT-HY 200 ADHESIVE WITH 12" EMBEDMENT	(2) 2x6			
6" WOOD TURAL PANEL NE SIDE	6d AT 4" O.C.	6d AT 12" O.C.	5/8" DIA. SIMPSON TITEN HD ANCHOR AT 48" O.C. WITH 6" EMBEDMENT	(1) SIMPSON HDU5-SDS2.5 HOLDOWN WITH 5/8" DIA. HILTI HIT-HY 200 ADHESIVE WITH 12" EMBEDMENT	(2) 2x6			

VERTICAL STUDS AT 16" OC MAX,

![](_page_7_Figure_9.jpeg)

ROOF SHEATHING AND ATTACHMENT, RE: PLAN AND WOOD FRAMING NOTES

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![](_page_7_Figure_12.jpeg)

FIELD NAILING (NAILING AWAY FROM EDGE OF PANEL)

![](_page_7_Figure_14.jpeg)

![](_page_7_Figure_15.jpeg)

FRAMING

DETAILS

S4.1

![](_page_8_Figure_0.jpeg)