EAGLE METAL

12300 Ford Rd, Suite 110
Dallas, Texas 75234

eaglemetal.com

The truss designs referenced below have been prepared by me or under my direct supervision based on the truss design criteria and requirements ("design criteria") provided by **Vivco Components**.

These truss designs are intended for the fabrication of individual building components that will perform to the design criteria provided. Any variance from the design criteria will render the affected truss designs inapplicable.

Listed below are the truss designs included in this package and covered by this seal.

Job: **500 NW Chipman Rd REVISED -** 1160083
BP15, BP16, BP17, BP18, BP19, BP20, BP21, BP22, BP23, BP24, BP6, BP7, DT10, DT11, DT12, DT13, DT1, DT25, DT2, DT3, DT4, DT5, DT8, DT9, GR10, GR11, GR12, GR1, GR2, GR3, GR4, GR5, GR6, GR7, GR8, GR9, T1, T2, T3, T4, T5, T6, T7

Any location identification is for file reference only. No determination of the appropriateness of design criteria for any specific project has been made in preparing the truss designs.

Please refer to individual truss designs for specific design criteria.

Arturo A. Hernandez (MO, 2006000095)
My license renewal date for the state of MO is 12/31/2022.

IMPORTANT NOTE: The responsibility of the engineer sealing this package, as a Truss Engineer, is solely for design of individual trusses as individual building components based upon design criteria provided by others and set forth in the referenced truss drawings. The truss design criteria for the components have not been verified as appropriate for any particular building, project or use. Adequacy and suitability of design criteria and requirements for the truss designs for any specific project are the responsibility of the building designer, not the Truss Engineer, per ANSI/TPI-1, Chapter 2.

SHOP DRAWING APPROVAL	
This review is only for conformance with the design concept of the project and general compliance with the information given in the Contract Documents. Corrections or comments made on the shop drawings during this review do not relieve contractor from compliance with the requirements of the plans and specifications. Contractor is responsible for: dimensions to be confirmed and correlated at the job site; information that pertains solely to the fabrication processes or to the means, method, techniques, sequences and procedures of construction; coordination of Work of all trades; and for performing all work in a safe and satisfactory manner.	
■ NO EXCEPTION TAKEN ■ APPROVED/REVISE AS NOTED ☐ REVISE AND RESUBMIT	KREHER ENGINEERING, INC. Structural Engineers REVIEWER: NATHAN DREYER DATE: 03-16-2022

DESIGN NOTES

1. The Truss Design Drawing(s) provided with these Design Notes have been prepared under and are subject to ANSI / TPI 1 published by the Truss Plate Institute, www.tpinst.org. Capitalized terms have the meanings provided in ANSI / TPI 1.
2. Copies of each Truss Design Drawing shall be furnished to the installation contractor, Building Designer, Owner and all persons fabricating, handling, installing, bracing, or erecting the trusses.

DESIGN LIMITATIONS

3. The Truss Design Drawing is based upon specifications provided by the Building Designer in accordance with ANS1 / TPI 1. Neither the Truss Designer, Eagle, nor an engineer who seals this design (if any) assumes any responsibility for the adequacy or accuracy of specifications provided by the Building Designer.
4. The Building Designer is solely responsible for the suitability based upon the Truss Design Drawing and shall be responsible for reviewing and verifying that the information shown is in general conformance with the design of the Building.
5. Each Truss Design Drawing is for the individual building component (a truss). A seal on the Truss Design Drawing indicates acceptance of professional engineering responsibility solely for the individual truss.
6. Each Truss Design Drawing assumes trusses will be suitably protected from the environment.

HANDLING, INSTALLING, & BRACING

7. Refer to Building Component Safety Information (BCSI) for handling, installing, restraining and bracing trusses. Copies can be obtained from the Structural Building Components Association, www.sbcindustry.com.
8. Bracing shown on each Truss Design Drawing is for lateral support of individual truss components only to reduce buckling lengths. All temporary and permanent bracing, including lateral load and diagonal or cross bracing, are the responsibility, respectively, of the erector and Building Designer.
9. Eagle is not responsible for improper truss fabrication, handling, erection or bracing.
10. Compression chords shall be laterally braced by the roof or floor sheathing, directly attached, or have purlins provided at spacing shown, unless noted otherwise.
11. Bottom chord required bracing shall be at 1Oft spacing or less, if no structural rated ceiling is installed, unless noted otherwise.
12. Strongbacking shall be installed on all parallel chord trusses, including flooring systems, to limit deflection and reduce vibration. Refer to BCSI-B7.
13. Never exceed the design loading shown. Never stack building or other materials on inadequately braced truss; refer to BCSI.
14. Concentration of construction loads greater than the design loads shall not be applied to the trusses at any time; refer to BCSI.
15. Trusses shall be handled with care prior to erection to avoid damage. Refer to BCSI for recommended truss handling and erection.

MATERIALS & FABRICATION

16. Lumber moisture content shall be 19% or less at the time of fabrication unless noted otherwise.
17. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
18. Unless expressly noted, the truss designs are not applicable for use with fire retardant or preservative treated lumber.
19. Plates shall be applied on both faces of truss at each joint and embedded fully. Knots and wane at joint locations shall be regulated in accordance with ANSI / TPI 1.
20. For a specified plate gauge and grade, the specified size is a minimum.
21. Connections not shown are the responsibility of others.
22. Adequate support shall be provided to resist gravity, lateral and uplift loads.
23. For 4X2 truss orientation, locate plates 0 - 1/16" from outside the edge of the truss.
24. Fabrication of truss shall be in accordance with ANSI / TPI 1.

OTHER NOTES

25. Camber is a non-structural consideration and is the responsibility of truss fabricator.
26. Do not cut or alter any truss member or plate without prior approval from a professional engineer.
27. Lumber design values are in accordance with ANSI / TPI 1; lumber design values are by others.
28. Install specified hangers per manufacturer recommendations.

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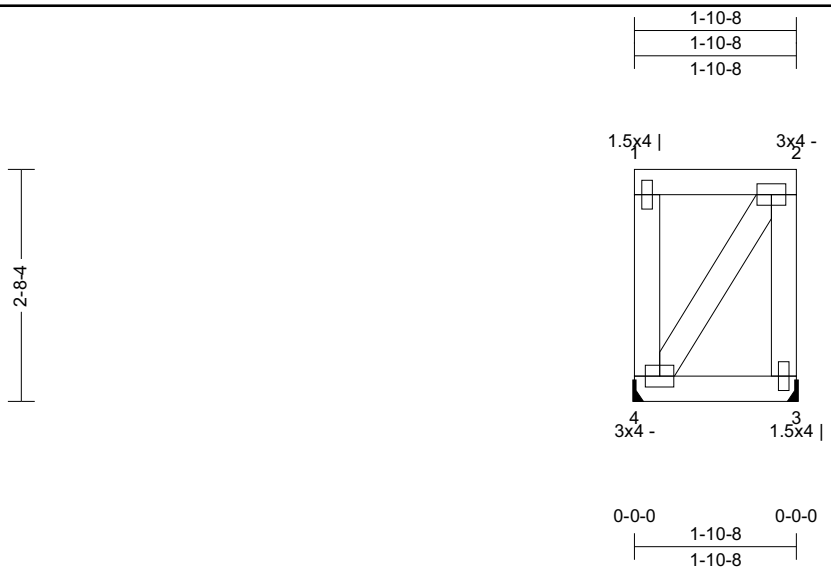
Vivco Components LLC.
2550 Hwy 33 South
P.O. Box 260
Maysville, Missouri 64469

Truss:BP15
Job: 500 NW Chipman Rd REVISED
Date: 02/16/22 10:56:03
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
1-10-8	0/12	2	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	15 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.07 (1-2)	Vert TL: 0 in	L/999	(3-4)	L/240
TCDL: 15	TPI 1-2014	BC: 0.03 (3-4)	Vert LL: 0 in	L/999	(3-4)	L/360
BCLL: 0	Rep Mbr: No	Web: 0.09 (1-4)	Horz TL: 0 in		3	
BCDL: 10	Lumber D.O.L.: 115 %					

02/16/2022

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
4	1	1.5 in	---	137 lbs	·	-48 lbs	-45 lbs	-48 lbs	-90 lbs
3	1	1.5 in	---	137 lbs	·	-48 lbs	-45 lbs	-48 lbs	·

Material

TC: SYP #1 2 x 4
BC: SYP #1 2 x 4
Web: SYP #2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects due to 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has not been designed for the effects of unbalanced snow loads.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
5) Minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	1-10-8	Down	Proj	35.64 plf	35.64 plf	

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC				
BC				
Web				

Notes

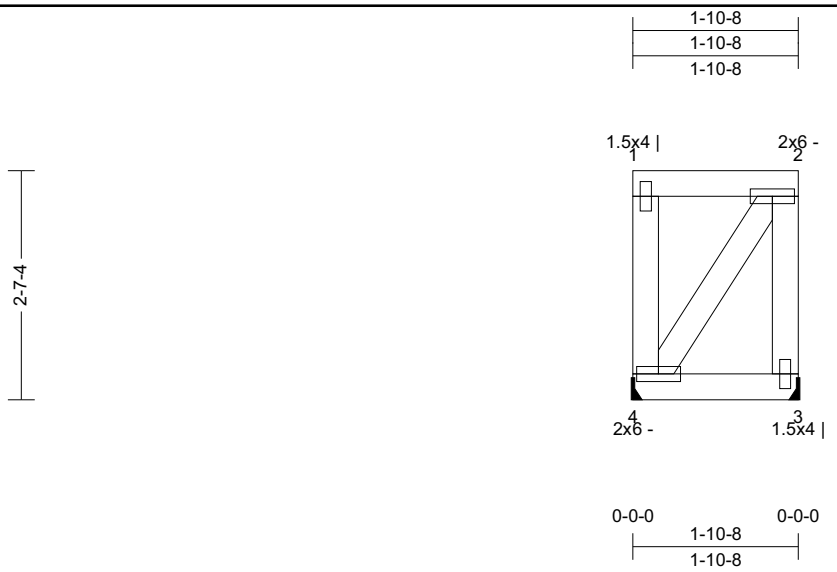
1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 30 % (Cq = 0.70).
3) Hangers are for graphical intrepretation only. Install hangers per manufacturer's recommendations.
4) Provide adequate drainage to prevent ponding.
5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
7) Listed wind uplift reactions based on MWFRS & C&C loading.

TrueBuild® Truss Software v5.6.376
Eagle Metal Products

Vivco Components LLC.
2550 Hwy 33 South
P.O. Box 260
Maysville, Missouri 64469

Truss:BP16
Job: 500 NW Chipman Rd REVISED
Date: 02/16/22 10:56:04
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
1-10-8	0/12	2	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	15 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.05 (1-2)	Vert TL: 0 in	L/999	(3-4)	L/240
TCDL: 15	TPI 1-2014	BC: 0.03 (3-4)	Vert LL: 0 in	L/999	(3-4)	L/360
BCLL: 0	Rep Mbr: No	Web: 0.09 (1-4)	Horz TL: 0 in		3	
BCDL: 10	Lumber D.O.L.: 115 %					

02/15/2022

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
4	1	1.5 in	---	103 lbs	·	-64 lbs	-65 lbs	-65 lbs	-87 lbs
3	1	1.5 in	---	103 lbs	·	-64 lbs	-65 lbs	-65 lbs	·

Material

TC: SYP #1 2 x 4
BC: SYP #1 2 x 4
Web: SYP #2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects due to 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has not been designed for the effects of unbalanced snow loads.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
5) Minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC				
BC				
Web				

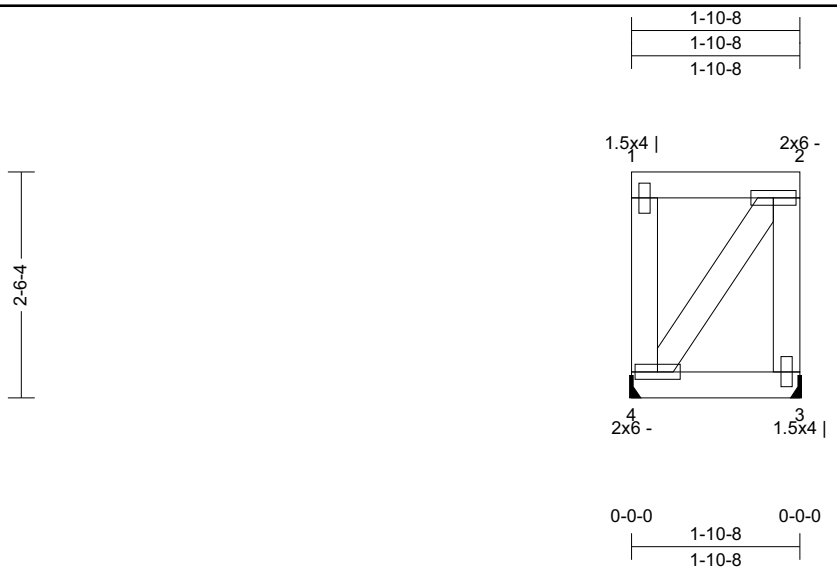
Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 30 % (Cq = 0.70).
3) Hangers are for graphical intrepretation only. Install hangers per manufacturer's recommendations.
4) Provide adequate drainage to prevent ponding.
5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
7) Listed wind uplift reactions based on MWFRS & C&C loading.

Vivco Components LLC.
2550 Hwy 33 South
P.O. Box 260
Maysville, Missouri 64469

Truss:BP17
Job: 500 NW Chipman Rd REVISED
Date: 02/16/22 10:56:05
Page: 1 of 1

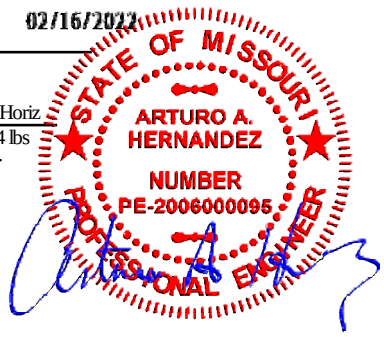
SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
1-10-8	0/12	2	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	14 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.05 (1-2)	Vert TL: 0 in	L/999	(3-4)	L/240
TCDL: 15	TPI 1-2014	BC: 0.03 (3-4)	Vert LL: 0 in	L/999	(3-4)	L/360
BCLL: 0	Rep Mbr: No	Web: 0.09 (1-4)	Horz TL: 0 in		3	
BCDL: 10	Lumber D.O.L.: 115 %					

02/16/2022



Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
4	1	1.5 in	---	103 lbs	·	-60 lbs	-65 lbs	-65 lbs	-84 lbs
3	1	1.5 in	---	103 lbs	·	-60 lbs	-65 lbs	-65 lbs	·

Material

TC: SYP #1 2 x 4
BC: SYP #1 2 x 4
Web: SYP #2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects due to 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has not been designed for the effects of unbalanced snow loads.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
5) Minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC				
BC				
Web				

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 30 % (Cq = 0.70).
3) Hangers are for graphical intrepretation only. Install hangers per manufacturer's recommendations.
4) Provide adequate drainage to prevent ponding.
5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
7) Listed wind uplift reactions based on MWFRS & C&C loading.

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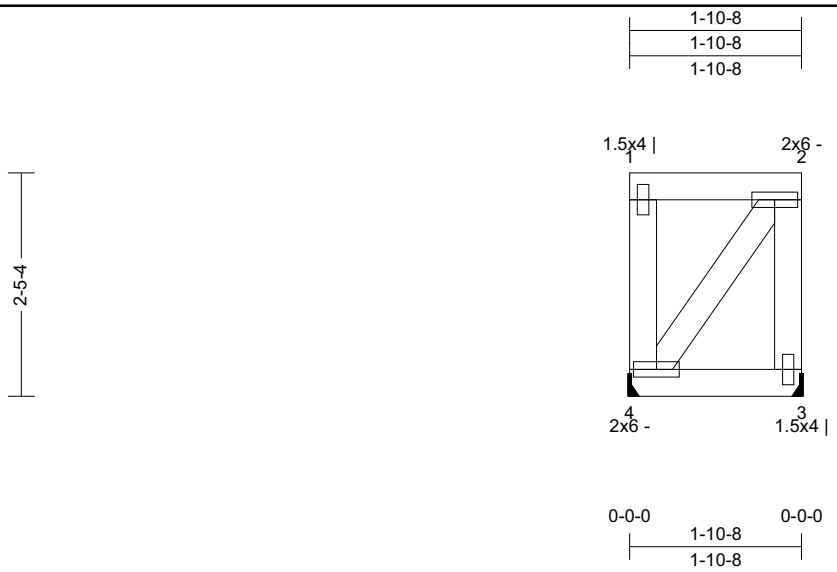
Eagle Metal Products

Vivco Components LLC.
2550 Hwy 33 South
P.O. Box 260
Maysville, Missouri 64469

Truss:BP18
Job: 500 NW Chipman Rd REVISED
Date: 02/16/22 10:56:07
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
1-10-8	0/12	2	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	14 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.05 (1-2)	Vert TL: 0 in	L/999	(3-4)	L/240
TCDL: 15	TPI 1-2014	BC: 0.03 (3-4)	Vert LL: 0 in	L/999	(3-4)	L/360
BCLL: 0	Rep Mbr: No	Web: 0.08 (1-4)	Horz TL: 0 in		3	
BCDL: 10	Lumber D.O.L.: 115 %					

02/16/2022

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
4	1	1.5 in	---	103 lbs	·	-56 lbs	-65 lbs	-65 lbs	-81 lbs
3	1	1.5 in	---	103 lbs	·	-56 lbs	-65 lbs	-65 lbs	·

Material

TC: SYP #1 2 x 4
BC: SYP #1 2 x 4
Web: SYP #2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects due to 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
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5) Minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC				
BC				
Web				

Notes

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2) The fabrication tolerance for this roof truss is 30 % (Cq = 0.70).
3) Hangers are for graphical intrepretation only. Install hangers per manufacturer's recommendations.
4) Provide adequate drainage to prevent ponding.
5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
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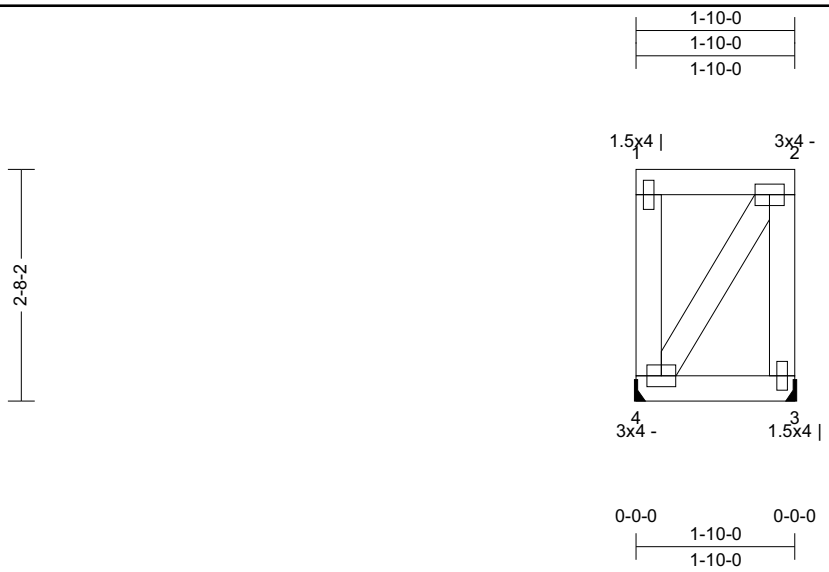
Vivco Components LLC.
2550 Hwy 33 South
P.O. Box 260
Maysville, Missouri 64469

Truss:BP19
Job: 500 NW Chipman Rd REVISED
Date: 02/16/22 10:56:08
Page: 1 of 1

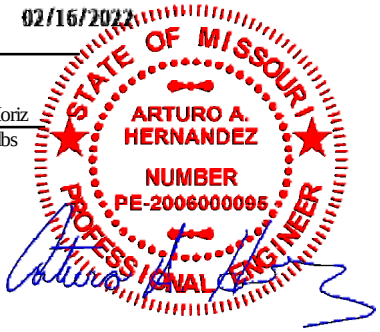
SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
1-10-0	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	15 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.05 (1-2)	Vert TL: 0 in	L/999	(3-4)	L/240
TCDL: 15	TPI 1-2014	BC: 0.03 (3-4)	Vert LL: 0 in	L/999	(3-4)	L/360
BCLL: 0	Rep Mbr: No	Web: 0.10 (1-4)	Horz TL: 0 in		3	
BCDL: 10	Lumber D.O.L.: 115 %					

02/16/2022



Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
4	1	1.5 in	---	101 lbs	·	-70 lbs	-63 lbs	-70 lbs	-89 lbs
3	1	1.5 in	---	101 lbs	·	-70 lbs	-63 lbs	-70 lbs	·

Material

TC: SYP #1 2 x 4
BC: SYP #1 2 x 4
Web: SYP #2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
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3) This truss has not been designed for the effects of unbalanced snow loads.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
5) Minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC				
BC				
Web				

Notes

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2) The fabrication tolerance for this roof truss is 30 % (Cq = 0.70).
3) Hangers are for graphical intrepretation only. Install hangers per manufacturer's recommendations.
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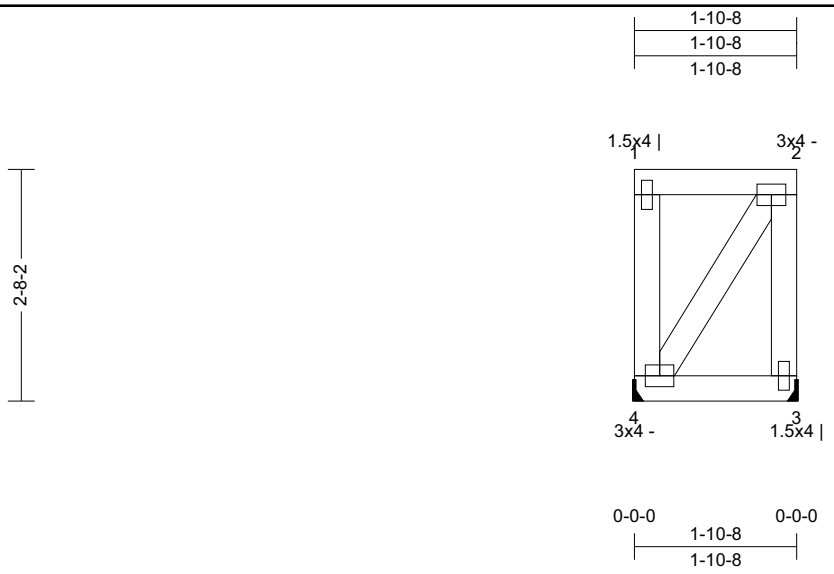
6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
7) Listed wind uplift reactions based on MWFRS & C&C loading.

Vivco Components LLC.
2550 Hwy 33 South
P.O. Box 260
Maysville, Missouri 64469

Truss:BP20
Job: 500 NW Chipman Rd REVISED
Date: 02/16/22 10:56:09
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
1-10-8	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	15 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.05 (1-2)	Vert TL: 0 in	L/999	(3-4)	L/240
TCDL: 15	TPI 1-2014	BC: 0.03 (3-4)	Vert LL: 0 in	L/999	(3-4)	L/360
BCLL: 0	Rep Mbr: No	Web: 0.10 (1-4)	Horz TL: 0 in		3	
BCDL: 10	Lumber D.O.L.: 115 %					

02/16/2022

STATE OF MISSOURI
ARTURO A. HERNANDEZ
NUMBER PE-2006000095
PROFESSIONAL ENGINEER

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
4	1	1.5 in	---	103 lbs	·	-68 lbs	-65 lbs	-68 lbs	-89 lbs
3	1	1.5 in	---	103 lbs	·	-68 lbs	-65 lbs	-68 lbs	·

Material

TC: SYP #1 2 x 4
BC: SYP #1 2 x 4
Web: SYP #2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects due to 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has not been designed for the effects of unbalanced snow loads.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
5) Minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC				
BC				
Web				

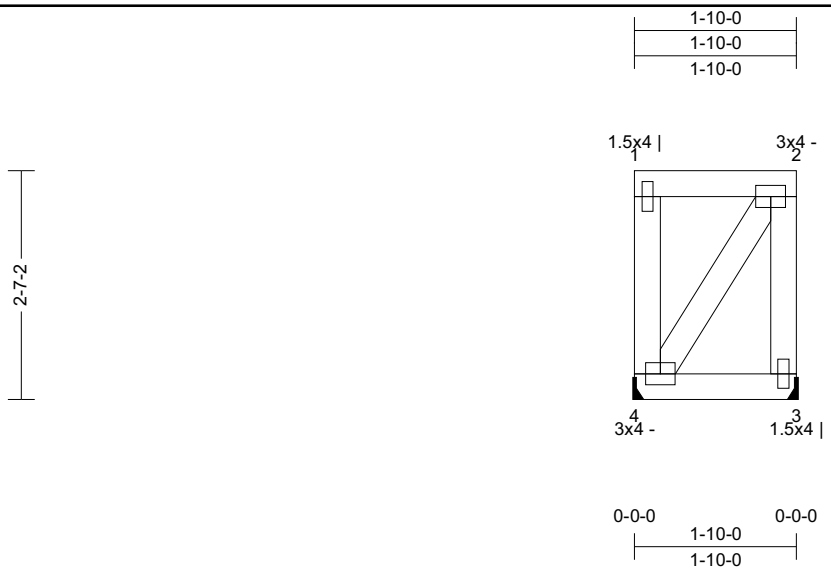
Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 30 % (Cq = 0.70).
3) Hangers are for graphical intrepretation only. Install hangers per manufacturer's recommendations.
4) Provide adequate drainage to prevent ponding.
5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
7) Listed wind uplift reactions based on MWFRS & C&C loading.

Vivco Components LLC.
2550 Hwy 33 South
P.O. Box 260
Maysville, Missouri 64469

Truss:BP21
Job: 500 NW Chipman Rd REVISED
Date: 02/16/22 10:56:10
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
1-10-0	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	15 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.05 (1-2)	Vert TL: 0 in	L/999	(3-4)	L/240
TCDL: 15	TPI 1-2014	BC: 0.03 (3-4)	Vert LL: 0 in	L/999	(3-4)	L/360
BCLL: 0	Rep Mbr: No	Web: 0.09 (1-4)	Horz TL: 0 in		3	
BCDL: 10	Lumber D.O.L.: 115 %					

02/16/2022

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
4	1	1.5 in	---	101 lbs	·	-65 lbs	-63 lbs	-65 lbs	-87 lbs
3	1	1.5 in	---	101 lbs	·	-65 lbs	-63 lbs	-65 lbs	·

Material

TC: SYP #1 2 x 4
BC: SYP #1 2 x 4
Web: SYP #2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects due to 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has not been designed for the effects of unbalanced snow loads.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
5) Minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC				
BC				
Web				

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 30 % (Cq = 0.70).
3) Hangers are for graphical intrepretation only. Install hangers per manufacturer's recommendations.
4) Provide adequate drainage to prevent ponding.
5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
7) Listed wind uplift reactions based on MWFRS & C&C loading.

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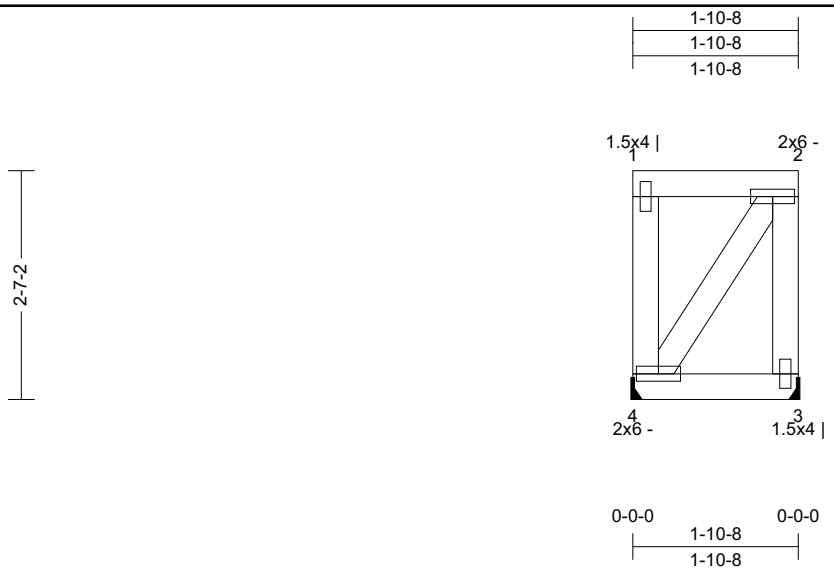
Eagle Metal Products

Vivco Components LLC.
2550 Hwy 33 South
P.O. Box 260
Maysville, Missouri 64469

Truss:BP22
Job: 500 NW Chipman Rd REVISED
Date: 02/16/22 10:56:11
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
1-10-8	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	15 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.05 (1-2)	Vert TL: 0 in	L/999	(3-4)	L/240
TCDL: 15	TPI 1-2014	BC: 0.03 (3-4)	Vert LL: 0 in	L/999	(3-4)	L/360
BCLL: 0	Rep Mbr: No	Web: 0.09 (1-4)	Horz TL: 0 in		3	
BCDL: 10	Lumber D.O.L.: 115 %					

02/16/2022

STATE OF MISSOURI
ARTURO A. HERNANDEZ
NUMBER PE-2006000095
PROFESSIONAL ENGINEER

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
4	1	1.5 in	---	103 lbs	·	-63 lbs	-65 lbs	-65 lbs	-87 lbs
3	1	1.5 in	---	103 lbs	·	-63 lbs	-65 lbs	-65 lbs	·

Material

TC: SYP #1 2 x 4
BC: SYP #1 2 x 4
Web: SYP #2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects due to 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has not been designed for the effects of unbalanced snow loads.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
5) Minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC				
BC				
Web				

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 30 % (Cq = 0.70).
3) Hangers are for graphical intrepretation only. Install hangers per manufacturer's recommendations.
4) Provide adequate drainage to prevent ponding.
5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
7) Listed wind uplift reactions based on MWFRS & C&C loading.

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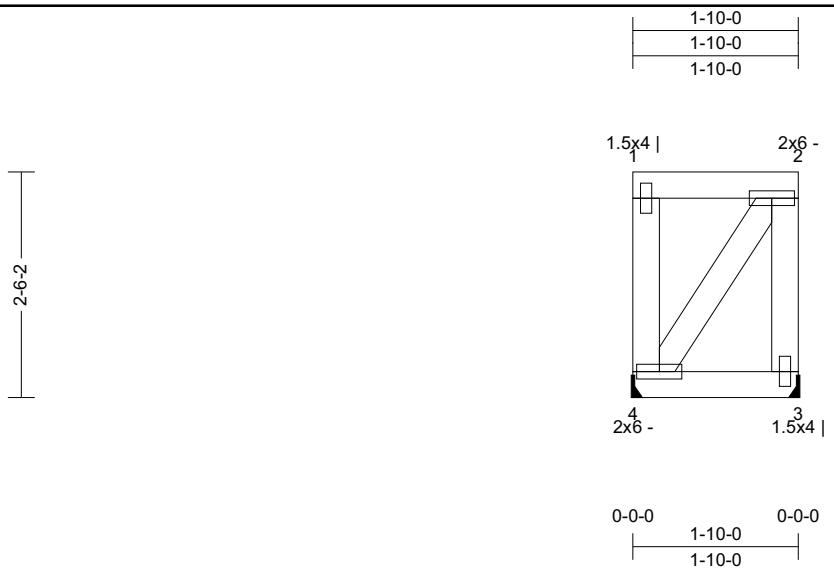
Eagle Metal Products

Vivco Components LLC.
2550 Hwy 33 South
P.O. Box 260
Maysville, Missouri 64469

Truss:BP23
Job: 500 NW Chipman Rd REVISED
Date: 02/16/22 10:56:12
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
1-10-0	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	14 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.05 (1-2)	Vert TL: 0 in	L/999	(3-4)	L/240
TCDL: 15	TPI 1-2014	BC: 0.03 (3-4)	Vert LL: 0 in	L/999	(3-4)	L/360
BCLL: 0	Rep Mbr: No	Web: 0.08 (1-4)	Horz TL: 0 in		3	
BCDL: 10	Lumber D.O.L.: 115 %					

02/16/2022

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
4	1	1.5 in	---	101 lbs	·	-61 lbs	-63 lbs	-63 lbs	-84 lbs
3	1	1.5 in	---	101 lbs	·	-61 lbs	-63 lbs	-63 lbs	·

Material

TC: SYP #1 2 x 4
BC: SYP #1 2 x 4
Web: SYP #2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects due to 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has not been designed for the effects of unbalanced snow loads.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
5) Minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC				
BC				
Web				

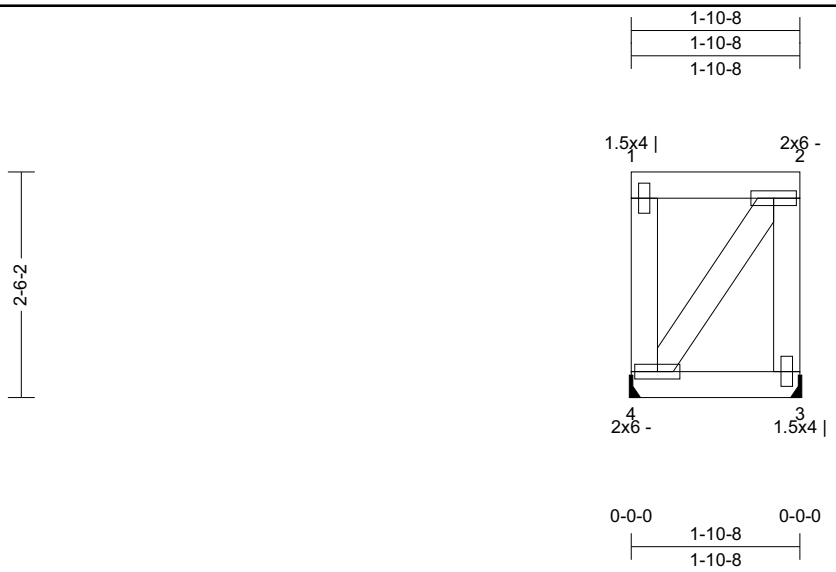
Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 30 % (Cq = 0.70).
3) Hangers are for graphical intrepretation only. Install hangers per manufacturer's recommendations.
4) Provide adequate drainage to prevent ponding.
5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
7) Listed wind uplift reactions based on MWFRS & C&C loading.

Vivco Components LLC.
2550 Hwy 33 South
P.O. Box 260
Maysville, Missouri 64469

Truss:BP24
Job: 500 NW Chipman Rd REVISED
Date: 02/16/22 10:56:14
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
1-10-8	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	14 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.05 (1-2)	Vert TL: 0 in	L/999	(3-4)	L/240
TCDL: 15	TPI 1-2014	BC: 0.03 (3-4)	Vert LL: 0 in	L/999	(3-4)	L/360
BCLL: 0	Rep Mbr: No	Web: 0.09 (1-4)	Horz TL: 0 in		3	
BCDL: 10	Lumber D.O.L.: 115 %					

02/16/2022

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
4	1	1.5 in	---	103 lbs	·	-59 lbs	-65 lbs	-65 lbs	-84 lbs
3	1	1.5 in	---	103 lbs	·	-59 lbs	-65 lbs	-65 lbs	·

Material

TC: SYP #1 2 x 4
BC: SYP #1 2 x 4
Web: SYP #2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects due to 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has not been designed for the effects of unbalanced snow loads.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
5) Minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC				
BC				
Web				

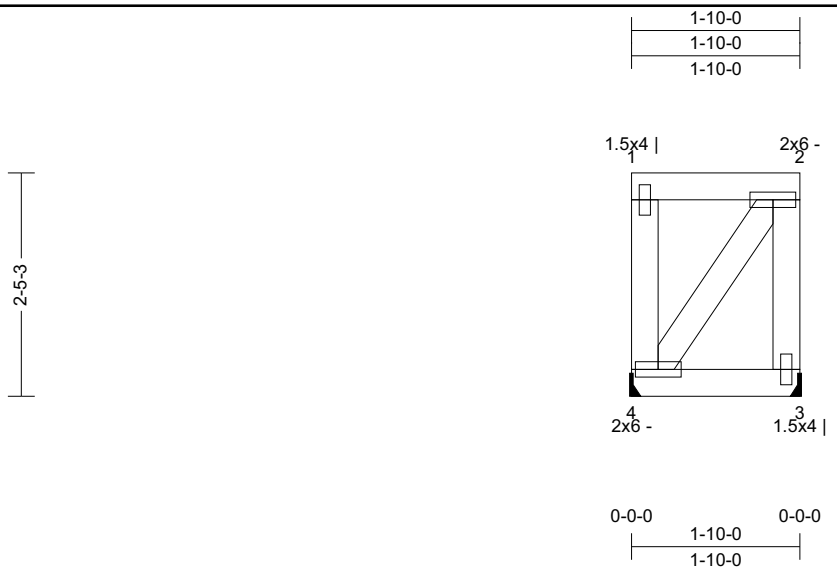
Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 30 % (Cq = 0.70).
3) Hangers are for graphical intrepretation only. Install hangers per manufacturer's recommendations.
4) Provide adequate drainage to prevent ponding.
5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
7) Listed wind uplift reactions based on MWFRS & C&C loading.

Vivco Components LLC.
2550 Hwy 33 South
P.O. Box 260
Maysville, Missouri 64469

Truss:BP6
Job: 500 NW Chipman Rd REVISED
Date: 02/16/22 10:56:15

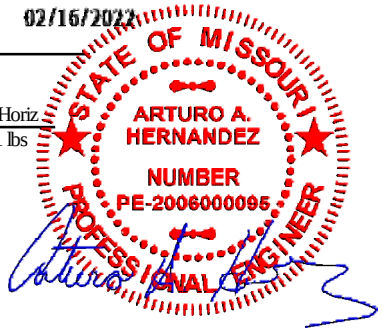
SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
1-10-0	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	2	24 in	14 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.13 (1-2)	Vert TL: 0 in	L/999	(3-4)	L/240
TCDL: 15	TPI 1-2014	BC: 0.01 (3-4)	Vert LL: 0 in	L/999	(3-4)	L/360
BCLL: 0	Rep Mbr: No	Web: 0.03 (1-4)	Horz TL: 0 in		3	
BCDL: 10	Lumber D.O.L.: 115 %					

02/16/2022



Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
4	1	1.5 in	---	412 lbs	·	·	·	·	81 lbs
3	1	1.5 in	---	412 lbs	·	·	·	·	·

Material

TC: SYP #1 2 x 4
BC: SYP #1 2 x 4
Web: SYP #2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects due to 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has not been designed for the effects of unbalanced snow loads.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
5) Minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case Lr1: Std Live Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	1-10-0	Down	Proj	90 plf	90 plf	

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	1-10-0	Down	Proj	160 plf	160 plf	

User-defined Load Case S2: Snow Drift

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	1-10-0	Down	Proj	20 psf	20 psf	24 in
Top	0-0-0	1-10-0	Down	Proj	180 plf	180 plf	

Point Loads

Member	Location	Direction	Load	Trib Width

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
1-10-0	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	2	24 in	14 lbs

Load Combinations

#	Load Combo	Factor
1	D1	1.000
2	D1 + Lr1	1.000
3	D1 + S1	1.000
4	D1 + S2	1.000
5	0.60 D1 + 0.60 W1 [Uplift]	1.000
6	0.60 D1 + 0.60 W2 [Uplift]	1.000
7	0.60 D1 + 0.60 W4 [Uplift]	1.000
8	0.60 D1 + 0.60 W8 [Uplift]	1.000
9	D1 + L10*1	1.000
10	D1 + Lr1 + L10*1	1.000
11	D1 + S1 + L10*1	1.000
12	D1 + S2 + L10*1	1.000
13	D1 + UR1	1.000
14	D1 + UR2	1.000
15	D1 + AS10*1	1.000
16	D1 + Lr1 + AS10*1	1.000
17	D1 + S1 + AS10*1	1.000
18	D1 + S2 + AS10*1	1.000

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

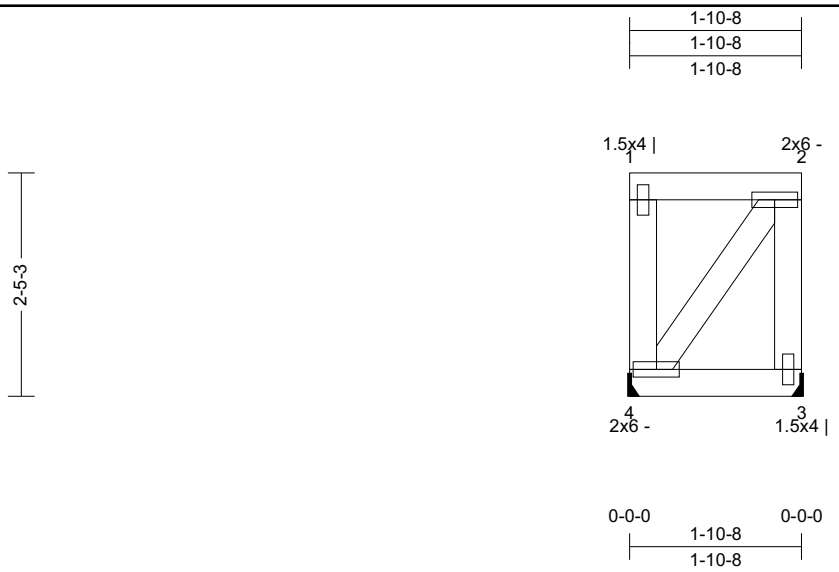
TC				
BC				
Web				

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 30 % (Cq = 0.70).
3) Hangers are for graphical intrepretation only. Install hangers per manufacturer's recommendations.
4) Provide adequate drainage to prevent ponding.
5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
7) The forces shown for this multi-ply truss are per ply and the reactions are for all plies. Two identical trusses shall be built and attached as follows: SDS Simpson 0.25"x3" (2 - ply) Screws TC - 1 row @ 13.5 in oc, BC - 1 row @ 24 in oc , Webs - 1 row 10d Nails or Gun Nails[min .12"x2] @ 24 in oc.
8) When applied loads are on one side of girder, do not flip girder during girder connector installation, install connectors on the girder side where supported loads are applied. When applied loads are on both sides of girder, double the spacing and install half of the connectors on one side of girder and then flip the girder to install the other half of the connectors on the opposite side (at double the connector spacing). Connectors on opposite sides of the girder shall be offset.
9) Lateral bracing shall be attached to each ply.
10) Install screws per manufacturer recommendations.
11) Listed wind uplift reactions based on MWFRS & C&C loading.

	Vivco Components LLC. 2550 Hwy 33 South P.O. Box 260 Maysville, Missouri 64469	Truss:BP7 Job: 500 NW Chipman Rd REVISED Date: 02/16/22 10:56:17 Page: 1 of 2

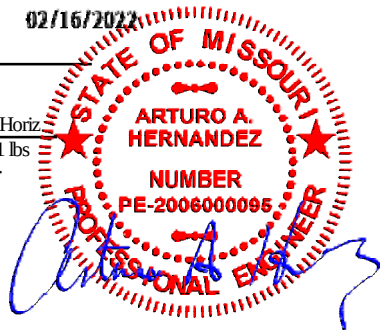
SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
1-10-8	0/12	3	0-0-0	0-0-0	0-0-0	0-0-0	2	24 in	14 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	**General**	**CSI**	**Deflection**	**L/**	**(loc)**	**Allowed**
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.12 (1-2)	Vert TL: 0 in	L/999	(3-4)	L/240
TCDL: 15	TPI 1-2014	BC: 0.01 (3-4)	Vert LL: 0 in	L/999	(3-4)	L/360
BCLL: 0	Rep Mbr: Yes	Web: 0.03 (1-4)	Horz TL: 0 in		3	
BCDL: 10	Lumber D.O.L.: 115 %					

02/16/2022



Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
4	1	1.5 in	---	422 lbs	·	·	·	·	81 lbs
3	1	1.5 in	---	422 lbs	·	·	·	·	·

Material

TC: SYP #1 2 x 4
BC: SYP #1 2 x 4
Web: SYP #2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects due to 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has not been designed for the effects of unbalanced snow loads.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
5) Minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case Lr1: Std Live Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	1-10-8	Down	Proj	90 plf	90 plf	

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	1-10-8	Down	Proj	160 plf	160 plf	

User-defined Load Case S2: Snow Drift

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	1-10-8	Down	Proj	20 psf	20 psf	24 in
Top	0-0-0	1-10-8	Down	Proj	180 plf	180 plf	

Point Loads

Member	Location	Direction	Load	Trib Width

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
1-10-8	0/12	3	0-0-0	0-0-0	0-0-0	0-0-0	2	24 in	14 lbs

Load Combinations

#	Load Combo	Factor
1	D1	1.000
2	D1 + Lr1	1.000
3	D1 + S1	1.000
4	D1 + S2	1.000
5	0.60 D1 + 0.60 W1 [Uplift]	1.000
6	0.60 D1 + 0.60 W2 [Uplift]	1.000
7	0.60 D1 + 0.60 W4 [Uplift]	1.000
8	0.60 D1 + 0.60 W8 [Uplift]	1.000
9	D1 + L10*1	1.000
10	D1 + Lr1 + L10*1	1.000
11	D1 + S1 + L10*1	1.000
12	D1 + S2 + L10*1	1.000
13	D1 + UR1	1.000
14	D1 + UR2	1.000
15	D1 + AS10*1	1.000
16	D1 + Lr1 + AS10*1	1.000
17	D1 + S1 + AS10*1	1.000
18	D1 + S2 + AS10*1	1.000

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC				
BC				
Web				

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 30 % (Cq = 0.70).
3) Hangers are for graphical intrepretation only. Install hangers per manufacturer's recommendations.
4) Provide adequate drainage to prevent ponding.
5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
7) The forces shown for this multi-ply truss are per ply and the reactions are for all plies. Two identical trusses shall be built and attached as follows: SDS Simpson 0.25"x3" (2 - ply) Screws TC - 1 row @ 13.5 in oc, BC - 1 row @ 24 in oc , Webs - 1 row 10d Nails or Gun Nails[min .12"x2] @ 24 in oc.
8) When applied loads are on one side of girder, do not flip girder during girder connector installation, install connectors on the girder side where supported loads are applied. When applied loads are on both sides of girder, double the spacing and install half of the connectors on one side of girder and then flip the girder to install the other half of the connectors on the opposite side (at double the connector spacing). Connectors on opposite sides of the girder shall be offset.
9) Lateral bracing shall be attached to each ply.
10) Install screws per manufacturer recommendations.
11) Listed wind uplift reactions based on MWFRS & C&C loading.

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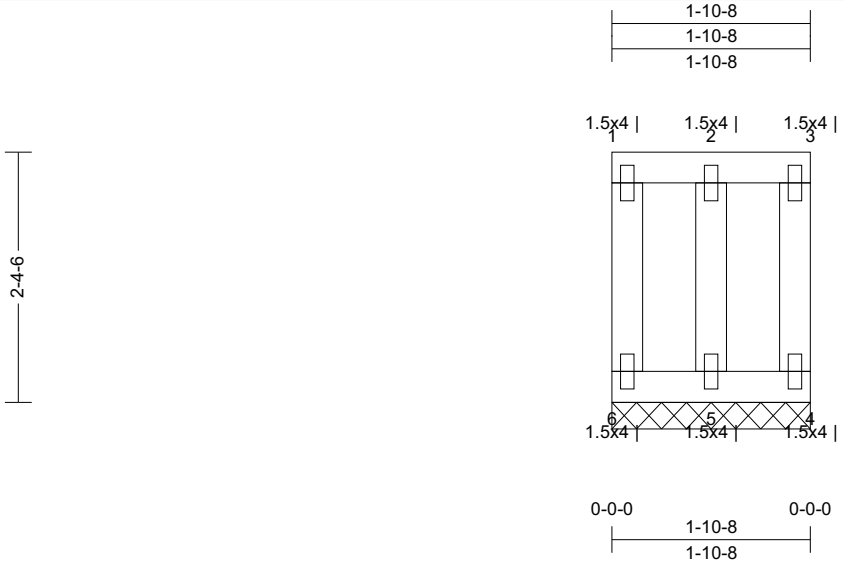
SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
1-10-8	0/12	13	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	13 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	**General**	**CSI**	**Deflection**	**L/**	**(loc)**	**Allowed**
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.06 (1-2)	Vert TL: 0 in UP	L/999	4	L/240
TCDL: 15	TPI 1-2014	BC: 0.00 (4-5)	Vert LL: 0 in	L/999	4	L/360
BCLL: 0	Rep Mbr: Yes	Web: 0.08 (1-6)	Horz TL: 0 in			
BCDL: 10	Lumber D.O.L.: 115 %					

02/16/2022

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	-	93 lbs	110 plf	-	-53 lbs	-55 lbs	-55 lbs	46 lbs

Material

TC: SYP #1 2 x 4
BC: SYP #1 2 x 4
Web: SYP #1 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has not been designed for the effects of unbalanced snow loads.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC			
BC			
Web			

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Gable requires continuous bottom chord bearing.
3) Gable webs placed at 16 " OC, U.N.O.
4) Attach gable webs with 1.5x4 20ga plates, U.N.O.
5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
6) The fabrication tolerance for this roof truss is 30 % (Cq = 0.70).
7) Provide adequate drainage to prevent ponding.
8) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
9) Listed wind uplift reactions based on MWFRS & C&C loading.

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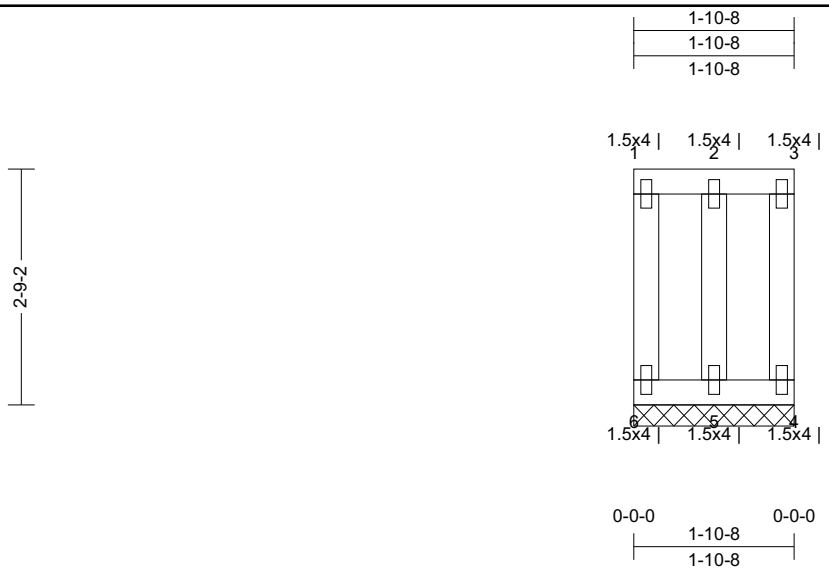
SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
1-10-8	0/12	2	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	15 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.09 (1-2)	Vert TL: 0 in UP	L/999	4	L/240
TCDL: 15	TPI 1-2014	BC: 0.00 (4-5)	Vert LL: 0 in	L/999	4	L/360
BCLL: 0	Rep Mbr: No	Web: 0.10 (1-6)	Horz TL: 0 in			
BCDL: 10	Lumber D.O.L.: 115 %					

02/16/2022

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	-	93 lbs	124 plf	-	-73 lbs	-63 lbs	-73 lbs	-53 lbs

Material

TC: SYP #1 2 x 4
BC: SYP #1 2 x 4
Web: SYP #1 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has not been designed for the effects of unbalanced snow loads.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC			
BC			
Web			

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Gable requires continuous bottom chord bearing.
3) Gable webs placed at 16 " OC, U.N.O.
4) Attach gable webs with 1.5x4 20ga plates, U.N.O.
5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
6) The fabrication tolerance for this roof truss is 30 % (Cq = 0.70).
7) Provide adequate drainage to prevent ponding.
8) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
9) Listed wind uplift reactions based on MWFRS & C&C loading.

Vivco Components LLC.
2550 Hwy 33 South
P.O. Box 260
Maysville, Missouri 64469

Truss:DT11
Job: 500 NW Chipman Rd REVISED
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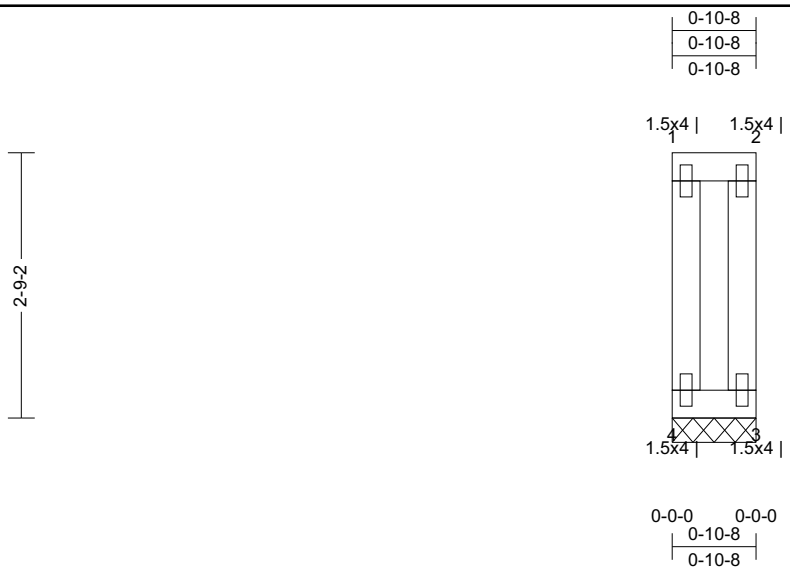
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
0-10-8	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	9 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.09 (1-2)	Vert TL: 0 in	L/999	3	L/240
TCDL: 15	TPI 1-2014	BC: 0.00 (3-4)	Vert LL: 0 in	L/999	3	L/360
BCLL: 0	Rep Mbr: No	Web: 0.12 (1-4)	Horz TL: 0 in			
BCDL: 10	Lumber D.O.L.: 115 %					

02/16/2022

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	-	195 lbs	445 plf	-	-195 lbs	-30 lbs	-195 lbs	-52 lbs

Material

TC: SYP #1 2 x 4
BC: SYP #1 2 x 4
Web: SYP #1 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (20 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Risk Category II (I = 1.00), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC				
BC				
Web				

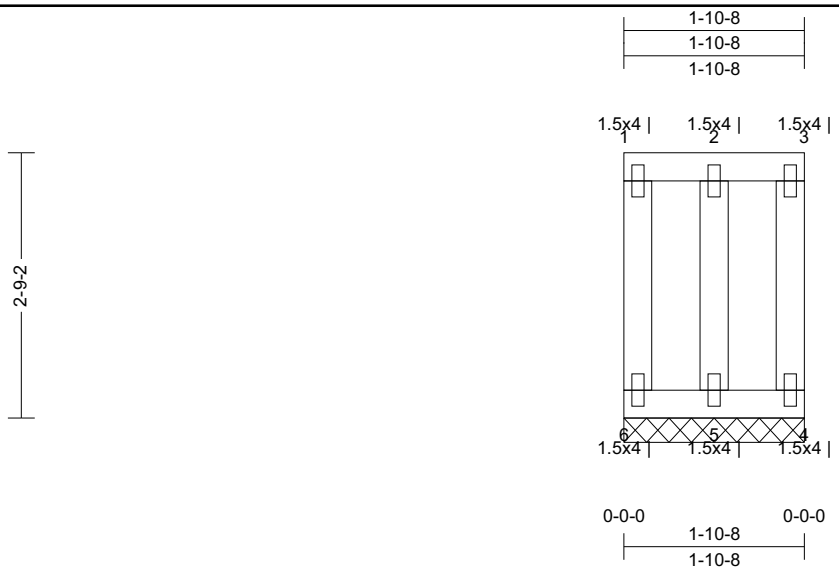
Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Gable requires continuous bottom chord bearing.
3) Gable webs placed at 16 " OC, U.N.O.
4) Attach gable webs with 1.5x4 20ga plates, U.N.O.
5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
6) The fabrication tolerance for this roof truss is 30 % (Cq = 0.70).
7) Provide adequate drainage to prevent ponding.
8) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
9) Listed wind uplift reactions based on MWFRS & C&C loading.

Vivco Components LLC.
2550 Hwy 33 South
P.O. Box 260
Maysville, Missouri 64469

Truss:DT12
Job: 500 NW Chipman Rd REVISED
Date: 02/16/22 10:56:21
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
1-10-8	0/12	11	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	15 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.08 (1-2)	Vert TL: 0 in UP	L/999	4	L/240
TCDL: 15	TPI 1-2014	BC: 0.00 (4-5)	Vert LL: 0 in	L/999	4	L/360
BCLL: 0	Rep Mbr: Yes	Web: 0.10 (1-6)	Horz TL: 0 in			
BCDL: 10	Lumber D.O.L.: 115 %					

02/16/2022

STATE OF MISSOURI
ARTURO A. HERNANDEZ
NUMBER PE-2006000095
PROFESSIONAL ENGINEER

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	-	93 lbs	124 plf	-	-73 lbs	-63 lbs	-73 lbs	-53 lbs

Material

TC: SYP #1 2 x 4
BC: SYP #1 2 x 4
Web: SYP #1 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has not been designed for the effects of unbalanced snow loads.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC			
BC			
Web			

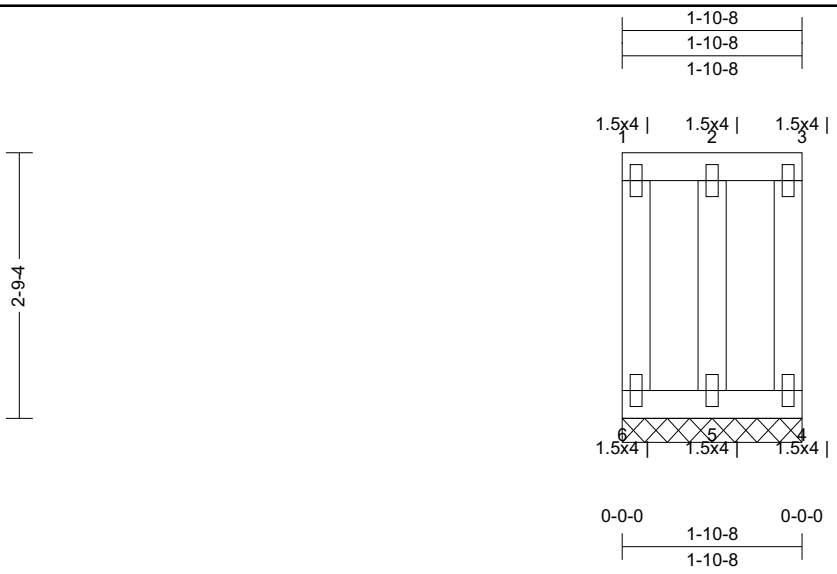
Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Gable requires continuous bottom chord bearing.
3) Gable webs placed at 16 " OC, U.N.O.
4) Attach gable webs with 1.5x4 20ga plates, U.N.O.
5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
6) The fabrication tolerance for this roof truss is 30 % (Cq = 0.70).
7) Provide adequate drainage to prevent ponding.
8) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
9) Listed wind uplift reactions based on MWFRS & C&C loading.

Vivco Components LLC.
2550 Hwy 33 South
P.O. Box 260
Maysville, Missouri 64469

Truss:DT13
Job: 500 NW Chipman Rd REVISED
Date: 02/16/22 10:56:23
Page: 1 of 1

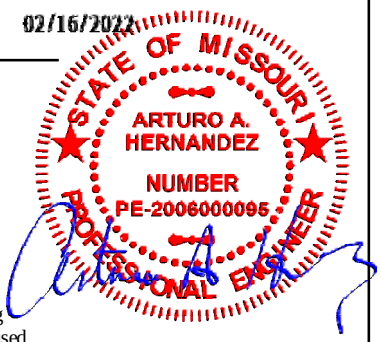
SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
1-10-8	0/12	5	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	15 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.08 (2-3)	Vert TL: 0 in UP	L/999	4	L/240
TCDL: 15	TPI 1-2014	BC: 0.00 (4-5)	Vert LL: 0 in	L/999	4	L/360
BCLL: 0	Rep Mbr: Yes	Web: 0.10 (1-6)	Horz TL: 0 in			
BCDL: 10	Lumber D.O.L.: 115 %					

02/16/2022



Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	-	93 lbs	125 plf	-	-74 lbs	-64 lbs	-74 lbs	-54 lbs

Material

TC: SYP #1 2 x 4
BC: SYP #1 2 x 4
Web: SYP #1 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has not been designed for the effects of unbalanced snow loads.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC			
BC			
Web			

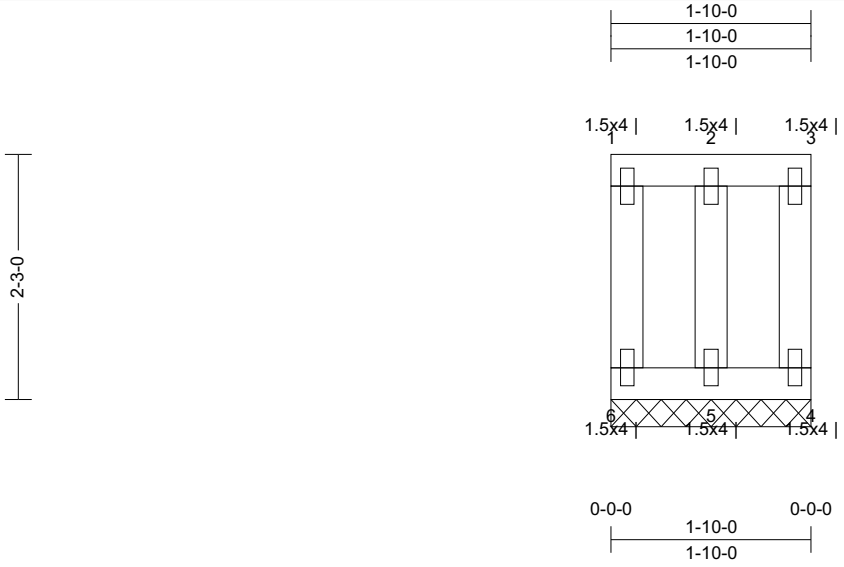
Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Gable requires continuous bottom chord bearing.
3) Gable webs placed at 16 " OC, U.N.O.
4) Attach gable webs with 1.5x4 20ga plates, U.N.O.
5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
6) The fabrication tolerance for this roof truss is 30 % (Cq = 0.70).
7) Provide adequate drainage to prevent ponding.
8) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
9) Listed wind uplift reactions based on MWFRS & C&C loading.

Vivco Components LLC.
2550 Hwy 33 South
P.O. Box 260
Maysville, Missouri 64469

Truss:DT2
Job: 500 NW Chipman Rd REVISED
Date: 02/16/22 10:56:24
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
1-10-0	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	13 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 20	Bldg Code : IBC 2018/	TC : 0.06 (2-3)	Vert TL: 0 in UP	L / 999	4	L / 240
TCDL : 15	TPI 1-2014	BC : 0.00 (4-5)	Vert LL: 0 in	L / 999	4	L / 360
BCLL : 0	Rep Mbr : No	Web : 0.07 (1-6)	Horz TL: 0 in			
BCDL : 10	Lumber D.O.L. : 115 %					

02/16/2022

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	-	90 lbs	110 plf	-	-49 lbs	-53 lbs	-53 lbs	-44 lbs

Material

TC: SYP #1 2 x 4
BC: SYP #1 2 x 4
Web: SYP #1 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has not been designed for the effects of unbalanced snow loads.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

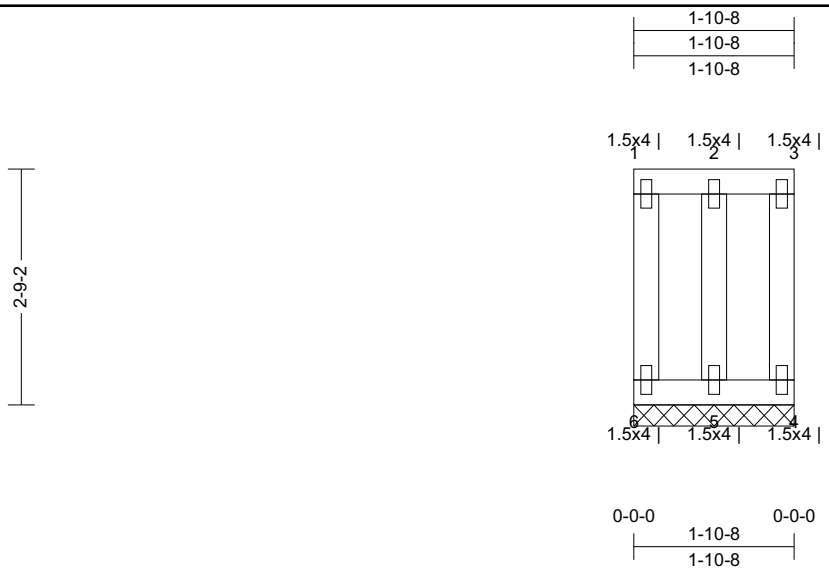
TC			
BC			
Web			

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Gable requires continuous bottom chord bearing.
3) Gable webs placed at 16 " OC, U.N.O.
4) Attach gable webs with 1.5x4 20ga plates, U.N.O.
5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
6) The fabrication tolerance for this roof truss is 30 % (Cq = 0.70).
7) Provide adequate drainage to prevent ponding.
8) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
9) Listed wind uplift reactions based on MWFRS & C&C loading.

	Vivco Components LLC. 2550 Hwy 33 South P.O. Box 260 Maysville, Missouri 64469	Truss:DT25 Job: 500 NW Chipman Rd REVISED Date: 02/16/22 10:56:25 Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
1-10-8	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	15 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.09 (1-2)	Vert TL: 0 in UP	L/999	4	L/240
TCDL: 15	TPI 1-2014	BC: 0.00 (4-5)	Vert LL: 0 in	L/999	4	L/360
BCLL: 0	Rep Mbr: No	Web: 0.10 (1-6)	Horz TL: 0 in			
BCDL: 10	Lumber D.O.L.: 115 %					

02/16/2022

STATE OF MISSOURI
ARTURO A. HERNANDEZ
NUMBER PE-2006000095
PROFESSIONAL ENGINEER

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	-	93 lbs	124 plf	-	-73 lbs	-63 lbs	-73 lbs	-53 lbs

Material

TC: SYP #1 2 x 4
BC: SYP #1 2 x 4
Web: SYP #1 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has not been designed for the effects of unbalanced snow loads.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC			
BC			
Web			

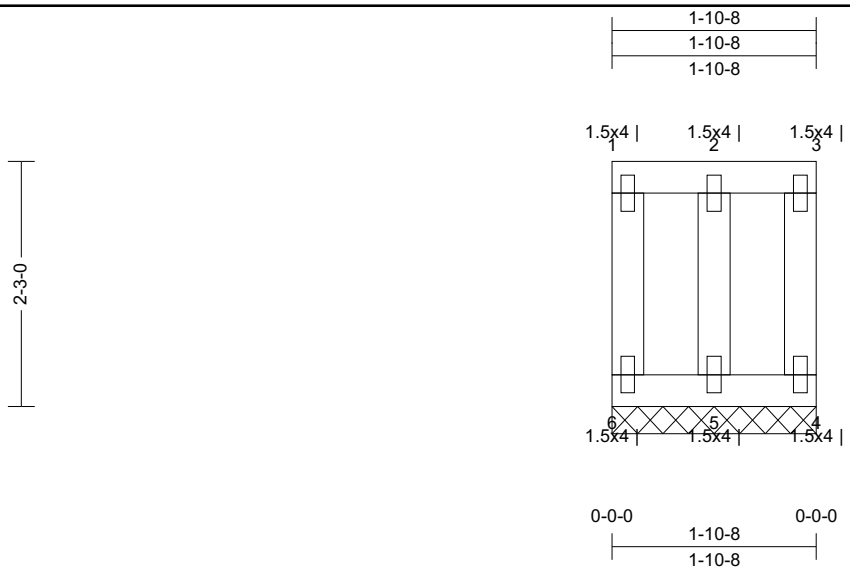
Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Gable requires continuous bottom chord bearing.
3) Gable webs placed at 16 " OC, U.N.O.
4) Attach gable webs with 1.5x4 20ga plates, U.N.O.
5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
6) The fabrication tolerance for this roof truss is 30 % (Cq = 0.70).
7) Provide adequate drainage to prevent ponding.
8) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
9) Listed wind uplift reactions based on MWFRS & C&C loading.

Vivco Components LLC.
2550 Hwy 33 South
P.O. Box 260
Maysville, Missouri 64469

Truss:DT3
Job: 500 NW Chipman Rd REVISED
Date: 02/16/22 10:56:26
Page: 1 of 1

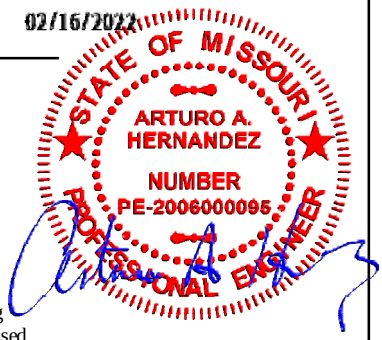
SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
1-10-8	0/12	2	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	13 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.06 (1-2)	Vert TL: 0 in UP	L/999	4	L/240
TCDL: 15	TPI 1-2014	BC: 0.00 (4-5)	Vert LL: 0 in	L/999	4	L/360
BCLL: 0	Rep Mbr: No	Web: 0.07 (1-6)	Horz TL: 0 in			
BCDL: 10	Lumber D.O.L.: 115 %					

02/16/2022



Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	-	93 lbs	110 plf	-	-48 lbs	-53 lbs	-53 lbs	-44 lbs

Material

TC: SYP #1 2 x 4
BC: SYP #1 2 x 4
Web: SYP #1 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has not been designed for the effects of unbalanced snow loads.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC			
BC			
Web			

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Gable requires continuous bottom chord bearing.
3) Gable webs placed at 16 " OC, U.N.O.
4) Attach gable webs with 1.5x4 20ga plates, U.N.O.
5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
6) The fabrication tolerance for this roof truss is 30 % (Cq = 0.70).
7) Provide adequate drainage to prevent ponding.
8) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
9) Listed wind uplift reactions based on MWFRS & C&C loading.

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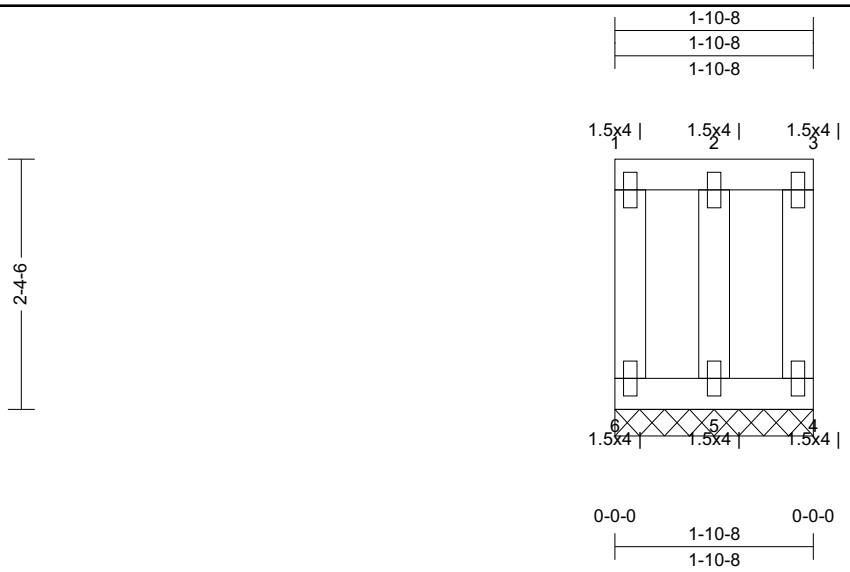
Eagle Metal Products

Vivco Components LLC.
2550 Hwy 33 South
P.O. Box 260
Maysville, Missouri 64469

Truss:DT4
Job: 500 NW Chipman Rd REVISED
Date: 02/16/22 10:56:27
Page: 1 of 1

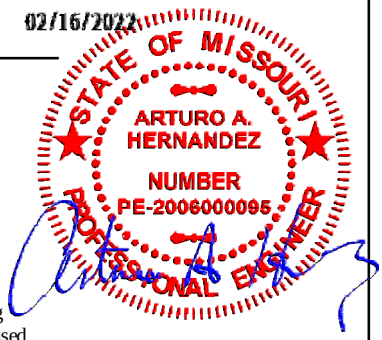
SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
1-10-8	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	13 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.07 (1-2)	Vert TL: 0 in UP	L/999	4	L/240
TCDL: 15	TPI 1-2014	BC: 0.00 (4-5)	Vert LL: 0 in	L/999	4	L/360
BCLL: 0	Rep Mbr: No	Web: 0.08 (1-6)	Horz TL: 0 in			
BCDL: 10	Lumber D.O.L.: 115 %					

02/16/2022



Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	-	93 lbs	110 plf	-	-53 lbs	-55 lbs	-55 lbs	46 lbs

Material

TC: SYP #1 2 x 4
BC: SYP #1 2 x 4
Web: SYP #1 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has not been designed for the effects of unbalanced snow loads.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC			
BC			
Web			

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Gable requires continuous bottom chord bearing.
3) Gable webs placed at 16 " OC, U.N.O.
4) Attach gable webs with 1.5x4 20ga plates, U.N.O.
5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
6) The fabrication tolerance for this roof truss is 30 % (Cq = 0.70).
7) Provide adequate drainage to prevent ponding.
8) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
9) Listed wind uplift reactions based on MWFRS & C&C loading.

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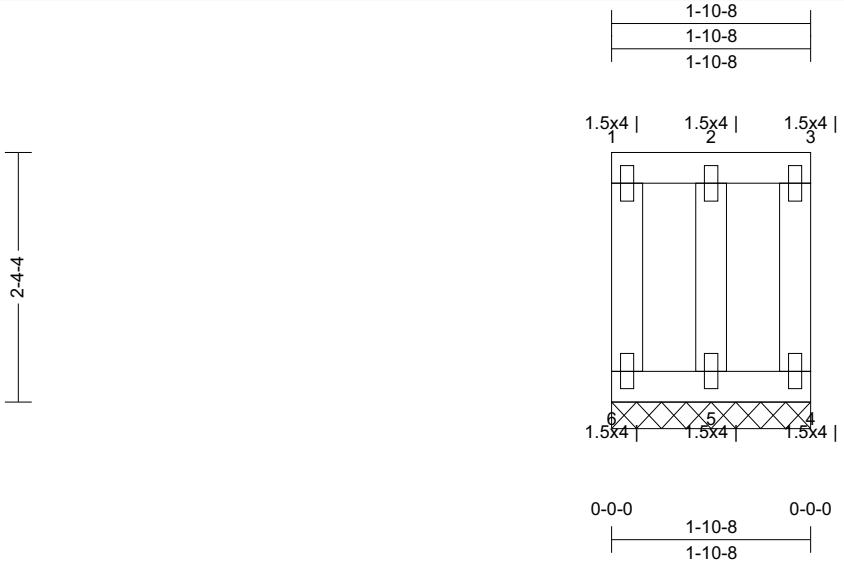
Eagle Metal Products

	Vivco Components LLC. 2550 Hwy 33 South P.O. Box 260 Maysville, Missouri 64469	Truss:DT5 Job: 500 NW Chipman Rd REVISED Date: 02/16/22 10:56:28 Page: 1 of 1

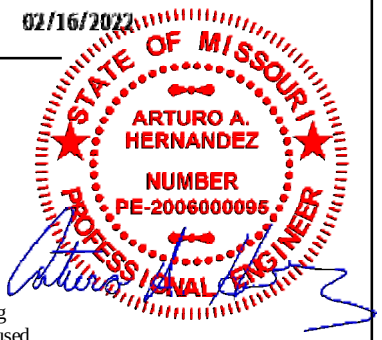
SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
1-10-8	0/12	3	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	13 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.06 (2-3)	Vert TL: 0 in UP	L/999	4	L/240
TCDL: 15	TPI 1-2014	BC: 0.00 (4-5)	Vert LL: 0 in	L/999	4	L/360
BCLL: 0	Rep Mbr: Yes	Web: 0.07 (1-6)	Horz TL: 0 in			
BCDL: 10	Lumber D.O.L.: 115 %					

02/16/2022



Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	-	93 lbs	110 plf	-	-53 lbs	-55 lbs	-55 lbs	-46 lbs

Material

TC: SYP #1 2 x 4
BC: SYP #1 2 x 4
Web: SYP #1 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has not been designed for the effects of unbalanced snow loads.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC			
BC			
Web			

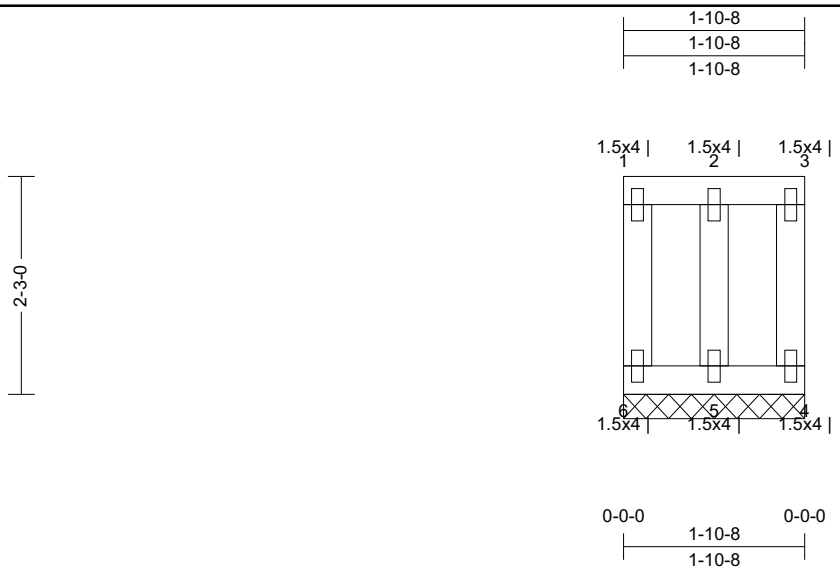
Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Gable requires continuous bottom chord bearing.
3) Gable webs placed at 16 " OC, U.N.O.
4) Attach gable webs with 1.5x4 20ga plates, U.N.O.
5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
6) The fabrication tolerance for this roof truss is 30 % (Cq = 0.70).
7) Provide adequate drainage to prevent ponding.
8) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
9) Listed wind uplift reactions based on MWFRS & C&C loading.

Vivco Components LLC.
2550 Hwy 33 South
P.O. Box 260
Maysville, Missouri 64469

Truss:DT8
Job: 500 NW Chipman Rd REVISED
Date: 02/16/22 10:56:29
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
1-10-8	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	13 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI	Deflection		L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2018/ TPI 1-2014	TC: 0.06 (1-2)	Vert TL:	0 in UP	L/999	4	L/240
TCDL: 15	Rep Mbr:	No	BC: 0.00 (4-5)	Vert LL:	0 in	L/999	4	L/360
BCLL: 0	Lumber D.O.L.:	115 %	Web: 0.07 (1-6)	Horz TL:	0 in			
BCDL: 10								

02/16/2022
STATE OF MISSOURI
ARTURO A. HERNANDEZ
NUMBER PE-2006000095
PROFESSIONAL ENGINEER

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	-	93 lbs	110 plf	-	-48 lbs	-53 lbs	-53 lbs	-44 lbs

Material

TC: SYP #1 2 x 4
BC: SYP #1 2 x 4
Web: SYP #1 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has not been designed for the effects of unbalanced snow loads.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

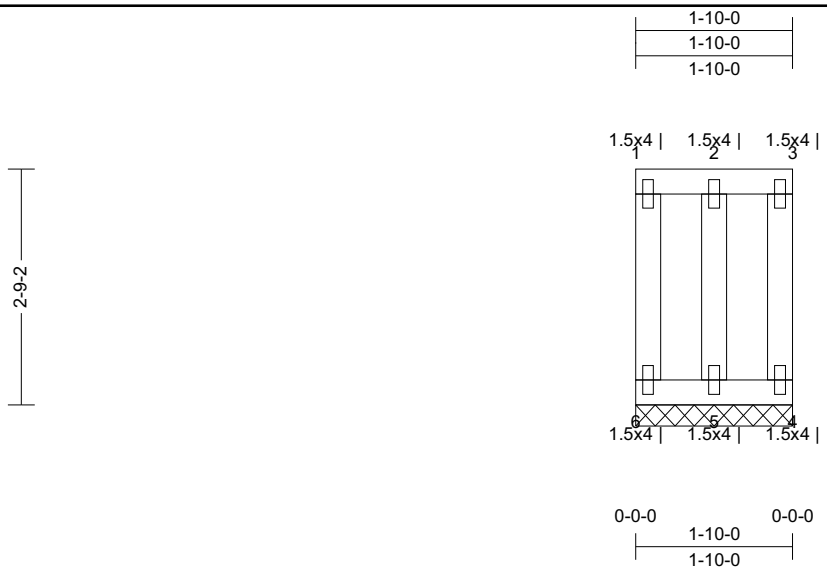
TC			
BC			
Web			

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Gable requires continuous bottom chord bearing.
3) Gable webs placed at 16 " OC, U.N.O.
4) Attach gable webs with 1.5x4 20ga plates, U.N.O.
5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
6) The fabrication tolerance for this roof truss is 30 % (Cq = 0.70).
7) Provide adequate drainage to prevent ponding.
8) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
9) Listed wind uplift reactions based on MWFRS & C&C loading.

	Vivco Components LLC. 2550 Hwy 33 South P.O. Box 260 Maysville, Missouri 64469	Truss:DT9 Job: 500 NW Chipman Rd REVISED Date: 02/16/22 10:56:30 Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
1-10-0	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	15 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.09 (1-2)	Vert TL: 0 in UP	L/999	4	L/240
TCDL: 15	TPI 1-2014	BC: 0.00 (4-5)	Vert LL: 0 in	L/999	4	L/360
BCLL: 0	Rep Mbr: No	Web: 0.10 (1-6)	Horz TL: 0 in			
BCDL: 10	Lumber D.O.L.: 115 %					

02/16/2022

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	-	90 lbs	128 plf	-	-75 lbs	-63 lbs	-75 lbs	-53 lbs

Material

TC: SYP #1 2 x 4
BC: SYP #1 2 x 4
Web: SYP #1 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has not been designed for the effects of unbalanced snow loads.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC			
BC			
Web			

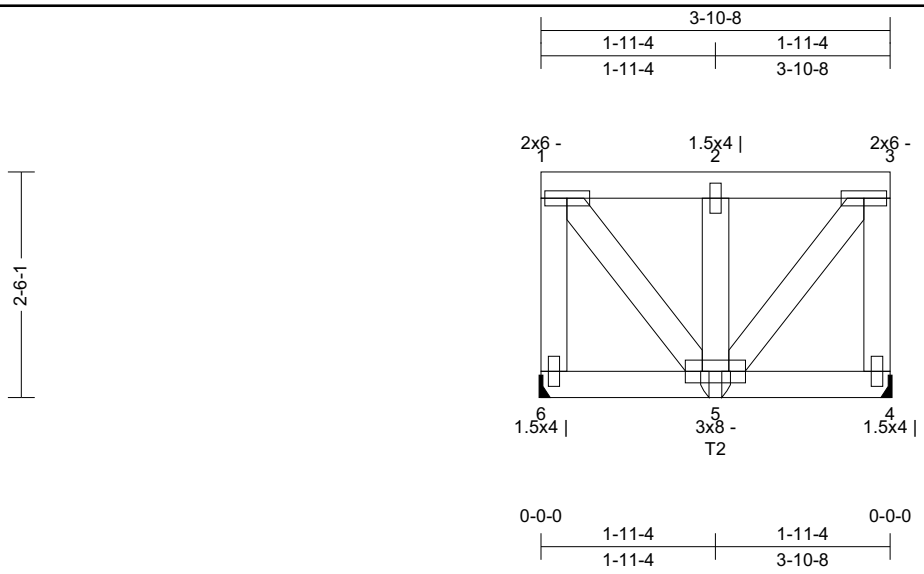
Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Gable requires continuous bottom chord bearing.
3) Gable webs placed at 16 " OC, U.N.O.
4) Attach gable webs with 1.5x4 20ga plates, U.N.O.
5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
6) The fabrication tolerance for this roof truss is 30 % (Cq = 0.70).
7) Provide adequate drainage to prevent ponding.
8) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
9) Listed wind uplift reactions based on MWFRS & C&C loading.

Vivco Components LLC.
2550 Hwy 33 South
P.O. Box 260
Maysville, Missouri 64469

Truss:GR1
Job: 500 NW Chipman Rd REVISED
Date: 02/16/22 10:56:32

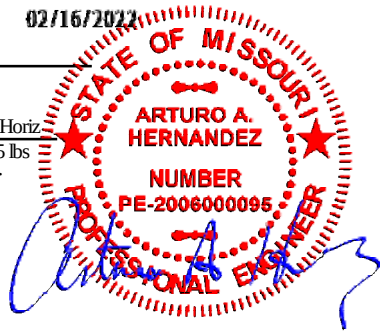
SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
3-10-8	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	15.75 in	27 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code : IBC 2018/	TC : 0.03 (1-2)	Vert TL: 0 in	L / 999	(4-5)	L / 240
TCLL : 20	TPI 1-2014	BC : 0.03 (5-6)	Vert LL: 0 in UP	L / 999	5	L / 360
TCDL : 15	Rep Mbr : No	Web : 0.07 (1-5)	Horz TL: 0 in		4	
BCLL : 0	Lumber D.O.L. : 115 %					
BCDL : 10						

02/16/2022



Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
6	1	1.5 in	---	293 lbs	·	-56 lbs	-36 lbs	-56 lbs	-55 lbs
4	1	1.5 in	---	293 lbs	·	-90 lbs	-23 lbs	-90 lbs	·

Material

TC: SYP #1 2 x 4
BC: SYP #1 2 x 4
Web: SYP #2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects due to 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has not been designed for the effects of unbalanced snow loads.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
5) Minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case Lr1: Std Live Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	3-10-8	Down	Proj	26.25 plf	26.25 plf	

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	3-10-8	Down	Proj	19.69 plf	19.69 plf	
Bot	0-0-0	3-10-8	Down	Proj	13.13 plf	13.13 plf	

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC			
BC			
Web			

Truss to Truss Connection Summary

Carried Truss	Carrying Chord	Carrying Offset
T2	BC	1-11-4

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
3-10-8	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	15.75 in	27 lbs

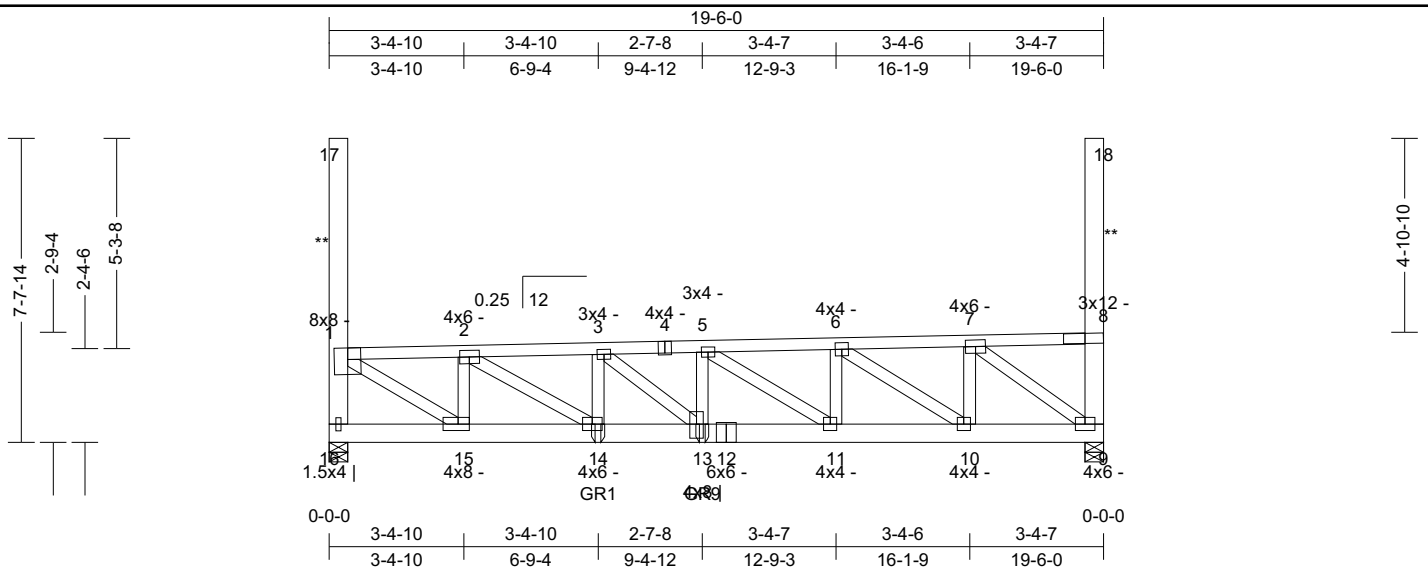
Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 30 % (Cq = 0.70).
3) Hangers are for graphical intrepretation only. Install hangers per manufacturer's recommendations.
4) Provide adequate drainage to prevent ponding.
5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
7) Listed wind uplift reactions based on MWFRS & C&C loading.

Vivco Components LLC.
2550 Hwy 33 South
P.O. Box 260
Maysville, Missouri 64469

Truss:GR10
Job: 500 NW Chipman Rd REVISED
Date: 02/16/22 10:56:34
Page: 1 of 3

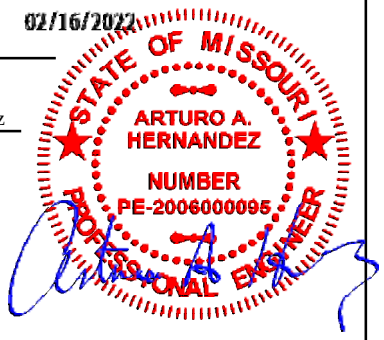
SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
19-6-0	0.25/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	147 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code: IBC 2018/	TC: 0.40 (3-5)	Vert TL: 0.21 in	L/999	13	L/240
TCLL: 20	TPI 1-2014	BC: 0.33 (11-13)	Vert LL: 0.09 in	L/999	(13-14)	L/360
TCDL: 15	Rep Mbr: No	Web: 0.92 (17-1)	Horz TL: 0.04 in		9	
BCLL: 0	Lumber D.O.L.: 115 %					
BCDL: 10						

02/16/2022



Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
16	1	5.5 in	1.65 in	1,637 lbs	·	·	-105 lbs	-105 lbs	490 lbs
9	1	5.5 in	1.59 in	1,572 lbs	·	-103 lbs	-94 lbs	-103 lbs	·

Material

TC: SYP #1 2 x 4
BC: SYP SSDense 2 x 6
Web: SYP #2 2 x 4 except:
SYP SSDense 2 x 6: 17-16, 18-9

Bracing

TC: Sheathed or Purlins at 2-11-0, Purlin design by Others.
BC: Sheathed or Purlins at 9-5-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects due to 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has not been designed for the effects of unbalanced snow loads.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
5) Minimum storage attic loading has been applied in accordance with IBC 1607.1
6) ** - Indicates parapet wind loading has been applied to members 17-1 & 18-8

Load Case Lr1: Std Live Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	6-9-4	Down	Proj	20 plf	20 plf	
Top	6-9-4	9-4-12	Down	Proj	20 plf	20 plf	
Top	9-4-12	19-6-0	Down	Proj	20 plf	20 plf	
Top	0-0-0	19-6-0	Down	Proj	20 plf	20 plf	

TrueBuild® Truss Software v5.6.390
Eagle Metal Products

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
19-6-0	0.25 /12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	147 lbs

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	13-10-0	16-0-0	Down	Proj	46.15 plf	46.15 plf	
Top	0-0-0	6-9-4	Down	Proj	15 plf	15 plf	
Top	6-9-4	9-4-12	Down	Proj	15 plf	15 plf	
Top	9-4-12	19-6-0	Down	Proj	15 plf	15 plf	
Top	0-0-0	19-6-0	Down	Proj	15 plf	15 plf	
Bot	0-0-0	6-9-4	Down	Proj	10 plf	10 plf	
Bot	6-9-4	9-4-12	Down	Proj	10 plf	10 plf	
Bot	9-4-12	19-6-0	Down	Proj	10 plf	10 plf	
Bot	0-0-0	19-6-0	Down	Proj	10 plf	10 plf	

User-defined Load Case S2: Drift

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	1-0-12	6-9-4	Down	Proj	20 plf	20 plf	
Top	1-0-12	6-9-4	Down	Proj	20 plf	20 plf	
Top	6-9-4	9-4-12	Down	Proj	20 plf	20 plf	
Top	9-4-12	18-5-4	Down	Proj	20 plf	20 plf	
Top	9-4-12	18-5-4	Down	Proj	20 plf	20 plf	
Top	0-0-0	4-6-0	Down	Proj	32 psf	0 psf	24 in
Top	15-0-0	19-6-0	Down	Proj	0 psf	32 psf	24 in

Point Loads

Member	Location	Direction	Load	Trib Width
Bot	6-9-4	Down	120 lbs	
Bot	9-4-12	Down	153 lbs	

Load Combinations

#	Load Combo	Factor
1	D1	1.000
2	D1 + Lr1	1.000
3	D1 + S1	1.000
4	D1 + S2	1.000
5	0.60 D1 + 0.60 W1 [Uplift]	1.000
6	0.60 D1 + 0.60 W2 [Uplift]	1.000
7	0.60 D1 + 0.60 W4 [Uplift]	1.000
8	0.60 D1 + 0.60 W8 [Uplift]	1.000
9	D1 + L10*1	1.000
10	D1 + Lr1 + L10*1	1.000
11	D1 + S1 + L10*1	1.000
12	D1 + S2 + L10*1	1.000
13	D1 + UR1	1.000
14	D1 + UR2	1.000
15	D1 + AS10*1	1.000
16	D1 + Lr1 + AS10*1	1.000
17	D1 + S1 + AS10*1	1.000
18	D1 + S2 + AS10*1	1.000

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.247	-2,187 lbs		3-5	0.401	-3,917 lbs		6-7	0.223	-1,850 lbs		
	2-3	0.375	-3,608 lbs		5-6	0.329	-3,167 lbs		7-8	0.191	1,066 lbs	(-500 lbs)	
BC	9-10	0.154	1,847 lbs	(-598 lbs)	11-13	0.329	3,915 lbs	(-667 lbs)	14-15	0.216	2,184 lbs	(-802 lbs)	
	10-11	0.256	3,164 lbs	(-673 lbs)	13-14	0.312	3,604 lbs	(-711 lbs)	15-16	0.044	804 lbs	(-747 lbs)	
Web	1-16	0.639	-1,557 lbs		3-14	0.058	-442 lbs		6-11	0.155	634 lbs	(-129 lbs)	
	1-15	0.634	2,583 lbs	(-66 lbs)	3-13	0.098	401 lbs	(-143 lbs)	6-10	0.438	-1,586 lbs		
	2-15	0.160	-1,231 lbs		5-13	0.081	329 lbs	(-133 lbs)	7-10	0.248	1,010 lbs	(-103 lbs)	
	2-14	0.408	1,663 lbs		5-11	0.243	-895 lbs		7-9	0.572	-2,298 lbs		

Truss to Truss Connection Summary

Carried Truss	Carrying Chord	Carrying Offset
GR1	BC	6-9-4
GR9	BC	9-4-12

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 30 % (Cq = 0.70).
3) Provide adequate drainage to prevent ponding.
4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.

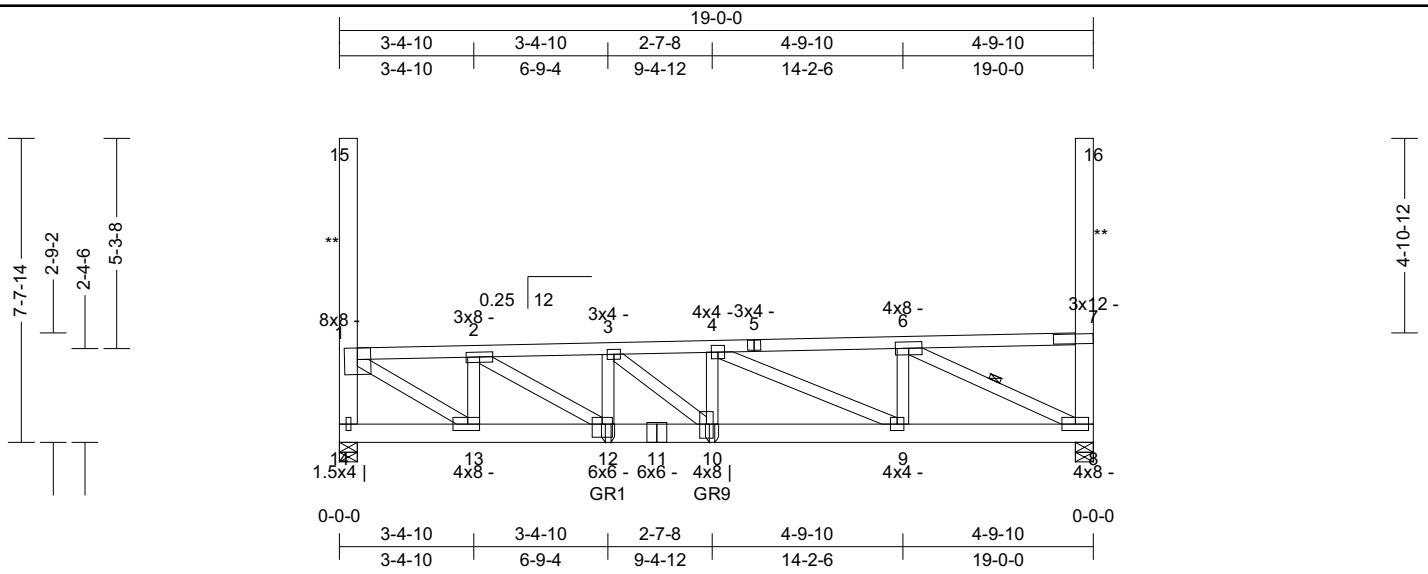
SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
19-6-0	0.25 /12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	147 lbs

7) Listed wind uplift reactions based on MWFRS & C&C loading.
8) Parapet TL: 0.83 in, 2L/158 (17-1), Allowable 2L/120.

Vivco Components LLC.
2550 Hwy 33 South
P.O. Box 260
Maysville, Missouri 64469

Truss:GR11
Job: 500 NW Chipman Rd REVISED
Date: 02/16/22 10:56:36

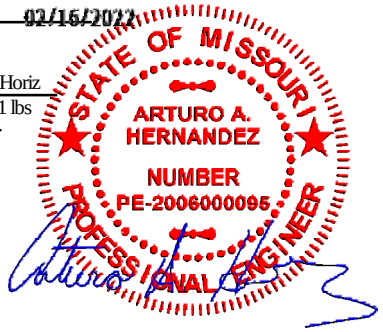
SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
19-0-0	0.25 /12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	141 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code : IBC 2018/	TC : 0.44 (4-6)	Vert TL: 0.21 in	L / 999	(9-10)	L / 240
TCLL : 20	TPI 1-2014	BC : 0.33 (9-10)	Vert LL: 0.09 in	L / 999	10	L / 360
TCDL : 15	Rep Mbr : No	Web : 0.92 (15-1)	Horz TL: 0.04 in		8	
BCLL : 0	Lumber D.O.L. : 115 %					
BCDL : 10						

02/16/2022



Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
14	1	5.5 in	1.62 in	1,599 lbs	·	·	-106 lbs	-106 lbs	491 lbs
8	1	5.5 in	1.59 in	1,577 lbs	·	-78 lbs	-87 lbs	-87 lbs	·

Material

TC: SYP #1 2 x 4
BC: SYP SSDense 2 x 6
Web: SYP #2 2 x 4 except:
SYP SSDense 2 x 6: 15-14, 16-8

Bracing

TC: Sheathed or Purlins at 3-0-0, Purlin design by Others.
BC: Sheathed or Purlins at 9-5-0, Purlin design by Others.
Web: One Midpoint Row: 6-8

Loads

1) This truss has been designed for the effects due to 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has not been designed for the effects of unbalanced snow loads.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
5) Minimum storage attic loading has been applied in accordance with IBC 1607.1
6) ** - Indicates parapet wind loading has been applied to members 15-1 & 16-7

Load Case Lr1: Std Live Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	19-0-0	Down	Proj	20 plf	20 plf	
Top	0-0-0	6-9-4	Down	Proj	20 plf	20 plf	
Top	6-9-4	9-4-12	Down	Proj	20 plf	20 plf	
Top	9-4-12	19-0-0	Down	Proj	20 plf	20 plf	

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
19-0-0	0.25 /12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	141 lbs

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	13-10-0	16-0-0	Down	Proj	46.15 plf	46.15 plf	
Top	0-0-0	19-0-0	Down	Proj	15 plf	15 plf	
Top	0-0-0	6-9-4	Down	Proj	15 plf	15 plf	
Top	6-9-4	9-4-12	Down	Proj	15 plf	15 plf	
Top	9-4-12	19-0-0	Down	Proj	15 plf	15 plf	
Bot	0-0-0	19-0-0	Down	Proj	10 plf	10 plf	
Bot	0-0-0	6-9-4	Down	Proj	10 plf	10 plf	
Bot	6-9-4	9-4-12	Down	Proj	10 plf	10 plf	
Bot	9-4-12	19-0-0	Down	Proj	10 plf	10 plf	

User-defined Load Case S2: Drift

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	1-0-12	6-9-4	Down	Proj	20 plf	20 plf	
Top	1-0-12	6-9-4	Down	Proj	20 plf	20 plf	
Top	6-9-4	9-4-12	Down	Proj	20 plf	20 plf	
Top	9-4-12	17-11-4	Down	Proj	20 plf	20 plf	
Top	9-4-12	17-11-4	Down	Proj	20 plf	20 plf	
Top	17-11-4	19-0-0	Down	Proj	20 plf	20 plf	
Top	0-0-0	4-6-0	Down	Proj	32 psf	0 psf	24 in
Top	14-6-0	19-0-0	Down	Proj	0 psf	32 psf	24 in

Point Loads

Member	Location	Direction	Load	Trib Width
Bot	6-9-4	Down	120 lbs	
Bot	9-4-12	Down	153 lbs	

Load Combinations

#	Load Combo	Factor
1	D1	1.000
2	D1 + Lr1	1.000
3	D1 + S1	1.000
4	D1 + S2	1.000
5	0.60 D1 + 0.60 W1 [Uplift]	1.000
6	0.60 D1 + 0.60 W2 [Uplift]	1.000
7	0.60 D1 + 0.60 W4 [Uplift]	1.000
8	0.60 D1 + 0.60 W8 [Uplift]	1.000
9	D1 + L10*1	1.000
10	D1 + Lr1 + L10*1	1.000
11	D1 + S1 + L10*1	1.000
12	D1 + S2 + L10*1	1.000
13	D1 + UR1	1.000
14	D1 + UR2	1.000
15	D1 + AS10*1	1.000
16	D1 + Lr1 + AS10*1	1.000
17	D1 + S1 + AS10*1	1.000
18	D1 + S2 + AS10*1	1.000

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.243	-2,130 lbs		3-4	0.387	-3,777 lbs		6-7	0.425	1,071 lbs	(-504 lbs)
	2-3	0.362	-3,491 lbs		4-6	0.440	-2,528 lbs					
BC	8-9	0.221	2,525 lbs	(-641 lbs)	10-12	0.299	3,487 lbs	(-707 lbs)	13-14	0.044	805 lbs	(-745 lbs)
	9-10	0.334	3,775 lbs	(-670 lbs)	12-13	0.211	2,126 lbs	(-802 lbs)				
Web	1-14	0.639	-1,519 lbs		3-12	0.054	-411 lbs		6-9	0.190	775 lbs	(-81 lbs)
	1-13	0.617	2,515 lbs	(-67 lbs)	3-10	0.091	371 lbs	(-166 lbs)	6-8	0.437	-2,822 lbs	
	2-13	0.155	-1,195 lbs		4-10	0.094	382 lbs	(-95 lbs)				
	2-12	0.391	1,592 lbs	(-39 lbs)	4-9	0.642	-1,374 lbs					

Truss to Truss Connection Summary

Carried Truss	Carrying Chord	Carrying Offset
GR1	BC	6-9-4
GR9	BC	9-4-12

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 30 % (Cq = 0.70).
3) Provide adequate drainage to prevent ponding.

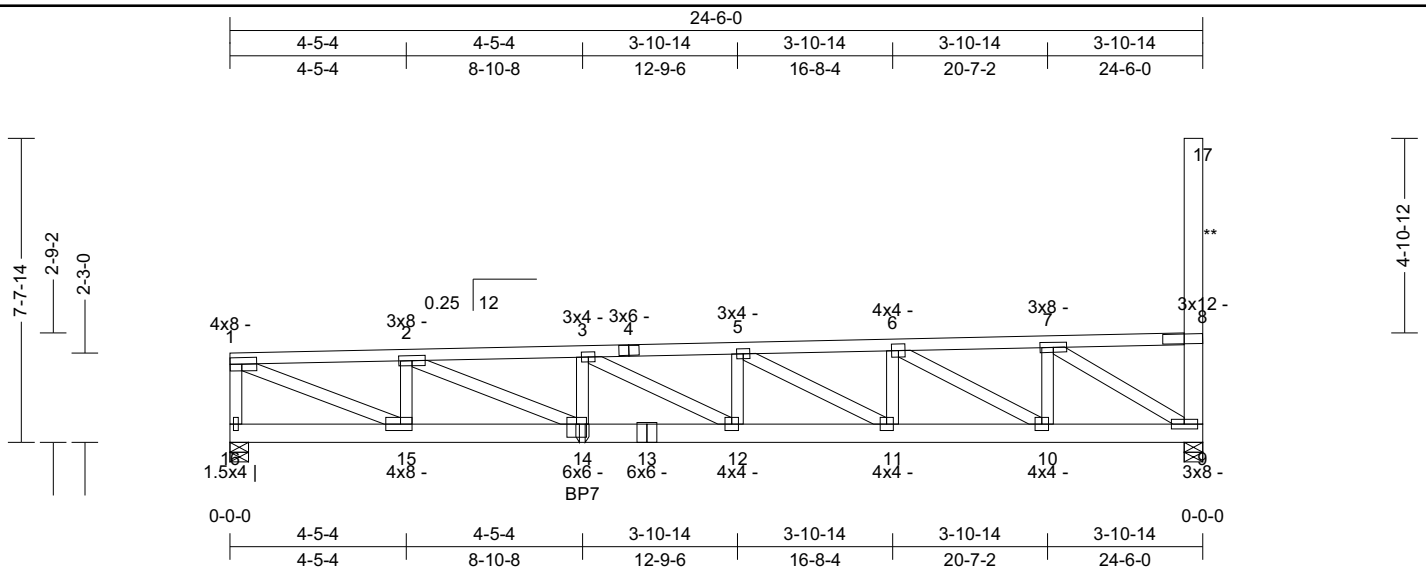
SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
19-0-0	0.25 /12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	141 lbs

4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
7) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
8) ⊠ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
9) Listed wind uplift reactions based on MWFRS & C&C loading.
10) Parapet TL: 0.82 in, 2L/158 (15-1), Allowable 2L/120.

Vivco Components LLC.
2550 Hwy 33 South
P.O. Box 260
Maysville, Missouri 64469

Truss:GR12
Job: 500 NW Chipman Rd REVISED
Date: 02/16/22 10:56:38

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
24-6-0	0.25 /12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	18.25 in	156 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code : IBC 2018/	TC : 0.37 (3-5)	Vert TL: 0.29 in	L / 970	(12-13)	L / 240
TCLL : 20	TPI 1-2014	BC : 0.30 (12-14)	Vert LL: 0.11 in	L / 999	(12-13)	L / 360
TCDL : 15	Rep Mbr : No	Web : 0.59 (1-15)	Horz TL: 0.04 in		9	
BCLL : 0	Lumber D.O.L. : 115 %					
BCDL : 10						

02/16/2022

STATE OF MISSOURI
ARTURO A. HERNANDEZ
NUMBER PE-2006000095
PROFESSIONAL ENGINEER

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
16	1	5.5 in	1.50 in	1,184 lbs	·	·	-211 lbs	-211 lbs	390 lbs
9	1	5.5 in	1.50 in	1,173 lbs	·	-9 lbs	-70 lbs	-70 lbs	·

Material

TC: SYP #1 2 x 4
BC: SYP SSDense 2 x 6
Web: SYP #2 2 x 4 except:
SYP SSDense 2 x 6: 17-9

Bracing

TC: Sheathed or Purlins at 2-11-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects due to 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has not been designed for the effects of unbalanced snow loads.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
5) Minimum storage attic loading has been applied in accordance with IBC 1607.1
6) ** - Indicates parapet wind loading has been applied to member 17-8

Load Case Lr1: Std Live Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	7-10-8	Down	Proj	20 plf	20 plf	
Top	8-10-8	24-6-0	Down	Proj	20 plf	20 plf	
Top	0-0-0	5-6-0	Down	Proj	10.42 plf	10.42 plf	
Top	5-6-0	24-6-0	Down	Proj	10 plf	10 plf	

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
24-6-0	0.25 /12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	18.25 in	156 lbs

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	7-10-8	Down	Proj	15 plf	15 plf	
Top	8-10-8	24-6-0	Down	Proj	15 plf	15 plf	
Top	0-0-0	5-6-0	Down	Proj	7.81 plf	7.81 plf	
Top	5-6-0	24-6-0	Down	Proj	7.5 plf	7.5 plf	
Bot	0-0-0	7-10-8	Down	Proj	10 plf	10 plf	
Bot	8-10-8	24-6-0	Down	Proj	10 plf	10 plf	
Bot	0-0-0	5-6-0	Down	Proj	5.21 plf	5.21 plf	
Bot	5-6-0	24-6-0	Down	Proj	5 plf	5 plf	

User-defined Load Case S2: Snow Drift

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	9-0-0	13-6-0	Down	Proj	32 psf	32 psf	24 in
Top	20-0-0	24-6-0	Down	Proj	32 psf	32 psf	24 in

Point Loads

Member	Location	Direction	Load	Trib Width

User-defined Load Case W12: W1 (DOL = 1.60)

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	1-0-12	N Up	Rake	25.39 plf	25.39 plf	
Top	1-0-12	5-6-0	N Up	Rake	25.39 plf	25.39 plf	
Top	1-0-12	5-6-0	N Up	Rake	25.39 plf	25.39 plf	
Top	5-6-0	7-6-0	N Up	Rake	25.39 plf	25.39 plf	
Top	5-6-0	7-6-0	N Up	Rake	12.7 plf	12.7 plf	
Top	7-6-0	7-10-8	N Up	Rake	25.39 plf	25.39 plf	
Top	7-6-0	7-10-8	N Up	Rake	12.7 plf	12.7 plf	
Top	7-10-8	9-10-8	N Up	Rake	12.7 plf	12.7 plf	
Top	9-10-8	15-0-0	N Up	Rake	25.39 plf	25.39 plf	
Top	9-10-8	15-0-0	N Up	Rake	12.7 plf	12.7 plf	
Top	15-0-0	23-5-4	N Up	Rake	15.97 plf	15.97 plf	
Top	15-0-0	23-5-4	N Up	Rake	7.98 plf	7.98 plf	
Web 1-16	0-0-0	2-3-0	Right	Rake	12.74 plf	12.74 plf	

Point Loads

Member	Location	Direction	Load	Trib Width
Bot	8-10-8	Down	44 lbs	
Top	0-0-0	Right	600 lbs	

User-defined Load Case W13: W2 (DOL = 1.60)

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	1-0-12	N Up	Rake	15.97 plf	15.97 plf	
Top	1-0-12	5-6-0	N Up	Rake	15.97 plf	15.97 plf	
Top	1-0-12	5-6-0	N Up	Rake	15.97 plf	15.97 plf	
Top	5-6-0	7-10-8	N Up	Rake	15.97 plf	15.97 plf	
Top	5-6-0	7-10-8	N Up	Rake	7.98 plf	7.98 plf	
Top	7-10-8	9-6-0	N Up	Rake	7.98 plf	7.98 plf	
Top	9-6-0	9-10-8	N Up	Rake	12.7 plf	12.7 plf	
Top	9-10-8	17-0-0	N Up	Rake	25.39 plf	25.39 plf	
Top	9-10-8	17-0-0	N Up	Rake	12.7 plf	12.7 plf	
Top	17-0-0	23-5-4	N Up	Rake	25.39 plf	25.39 plf	
Top	17-0-0	23-5-4	N Up	Rake	12.7 plf	12.7 plf	
Web 1-16	0-0-0	2-3-0	Left	Rake	15.11 plf	15.11 plf	

Point Loads

Member	Location	Direction	Load	Trib Width
Bot	8-10-8	Up	139 lbs	
Top	0-0-0	Left	600 lbs	

User-defined Load Case W14: W3 (DOL = 1.60)

Point Loads

Member	Location	Direction	Load	Trib Width
Top	0-0-0	Left	600 lbs	

Load Combinations

#	Load Combo	Factor
1	D1	1.000
2	D1 + Lr1	1.000

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
24-6-0	0.25 /12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	18.25 in	156 lbs

3	D1 + S1	1.000
4	D1 + S2	1.000
5	0.60 D1 + 0.60 W1 [Uplift]	1.000
6	0.60 D1 + 0.60 W2 [Uplift]	1.000
7	0.60 D1 + 0.60 W4 [Uplift]	1.000
8	0.60 D1 + 0.60 W8 [Uplift]	1.000
9	0.60 D1 + 0.60 W0 [Uplift]	1.000
10	0.60 D1 + 0.60 W0 [Uplift]	1.000
11	0.60 D1 + 0.60 W0 [Uplift]	1.000
12	D1 + L10*1	1.000
13	D1 + Lr1 + L10*1	1.000
14	D1 + S1 + L10*1	1.000
15	D1 + S2 + L10*1	1.000
16	D1 + UR1	1.000
17	D1 + UR2	1.000
18	D1 + AS10*1	1.000
19	D1 + Lr1 + AS10*1	1.000
20	D1 + S1 + AS10*1	1.000
21	D1 + S2 + AS10*1	1.000

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.248	-2,197 lbs		3-5	0.374	-3,372 lbs		6-7	0.178	-1,541 lbs	
	2-3	0.370	-3,460 lbs		5-6	0.282	-2,683 lbs		7-8	0.232	798 lbs	(-376 lbs)
BC	9-10	0.131	1,541 lbs	(-470 lbs)	11-12	0.273	3,368 lbs	(-372 lbs)	14-15	0.216	2,194 lbs	(-162 lbs)
	10-11	0.218	2,680 lbs	(-480 lbs)	12-14	0.303	3,458 lbs	(-140 lbs)	15-16	0.048	386 lbs	(-380 lbs)
Web	1-16	0.139	-1,108 lbs		5-11	0.301	-906 lbs		7-9	0.554	-1,818 lbs	
	1-15	0.588	2,395 lbs	(-397 lbs)	6-11	0.137	558 lbs					
	2-15	0.105	-821 lbs		6-10	0.443	-1,313 lbs					
	2-14	0.377	1,535 lbs	(-178 lbs)	7-10	0.189	770 lbs					

Truss to Truss Connection Summary

Carried Truss	Carrying Chord	Carrying Offset
BP7	BC	8-10-8

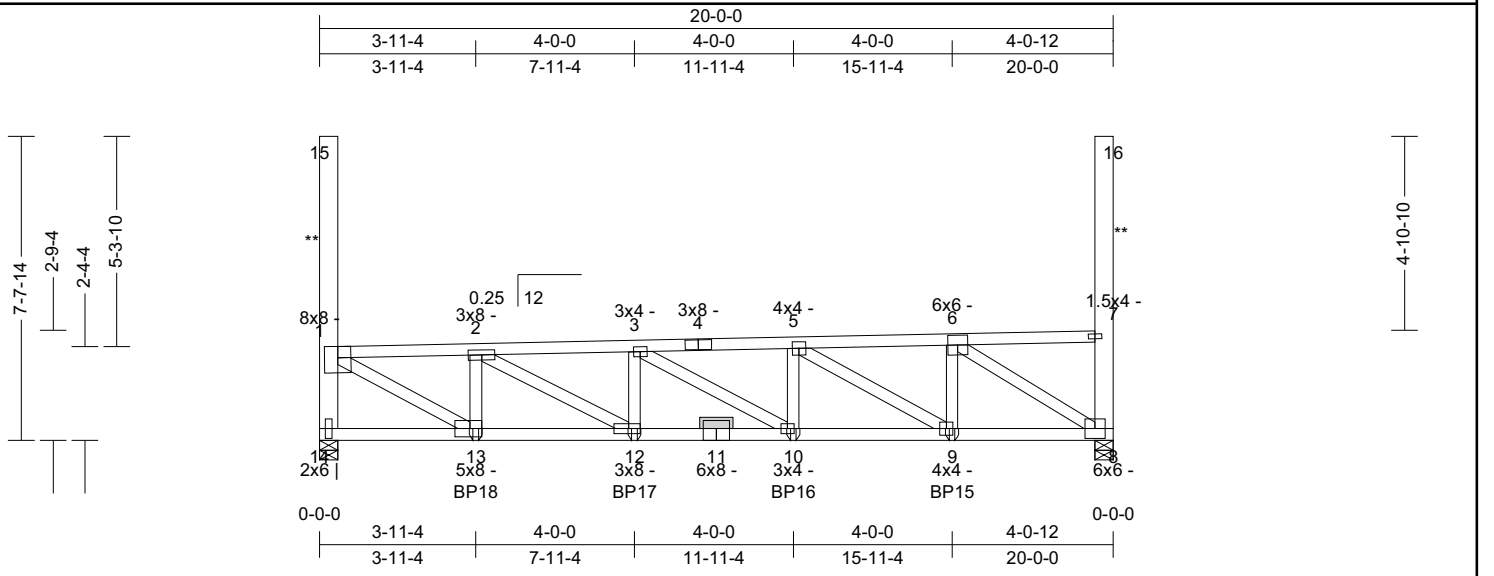
Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 30 % (Cq = 0.70).
3) Provide adequate drainage to prevent ponding.
4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
7) Listed wind uplift reactions based on MWFRS & C&C loading.
8) Parapet TL: 0.51 in, 2L/238 (17-8), Allowable 2L/120.

Vivco Components LLC.
2550 Hwy 33 South
P.O. Box 260
Maysville, Missouri 64469

Truss:GR2
Job: 500 NW Chipman Rd REVISED
Date: 02/16/22 10:56:40

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
20-0-0	0.25 /12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	133 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection		L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code : IBC 2018/	TC : 0.48 (2-3)	Vert TL:	0.29 in	L / 788	11	L / 240
TCLL : 20	TPI 1-2014	BC : 0.79 (10-12)	Vert LL:	0.18 in	L / 999	11	L / 360
TCDL : 15	Rep Mbr : No	Web : 0.99 (6-8)	Horz TL:	0.07 in		8	
BCLL : 0	Lumber D.O.L. : 115 %						
BCDL : 10							

02/16/2022

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
14	1	5.5 in	2.14 in	1,817 lbs	·	-31 lbs	-144 lbs	-144 lbs	248 lbs
8	1	5.5 in	2.17 in	1,837 lbs	·	-164 lbs	-122 lbs	-164 lbs	·

Material

TC: SYP #1 2 x 4
BC: SYP #1 2 x 4
Web: SYP #2 2 x 4 except:
SYP SSDense 2 x 6: 15-14, 16-8

Bracing

TC: Sheathed or Purlins at 2-8-0, Purlin design by Others.
BC: Sheathed or Purlins at 8-4-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects due to 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has not been designed for the effects of unbalanced snow loads.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
5) Minimum storage attic loading has been applied in accordance with IBC 1607.1
6) ** - Indicates parapet wind loading has been applied to members 15-1 & 16-7

Load Case Lr1: Std Live Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	1-0-12	2-11-4	Down	Proj	20 plf	20 plf	
Top	4-11-4	6-11-4	Down	Proj	20 plf	20 plf	
Top	8-11-4	10-11-4	Down	Proj	20 plf	20 plf	
Top	12-11-4	14-11-4	Down	Proj	20 plf	20 plf	
Top	16-11-4	18-11-4	Down	Proj	20 plf	20 plf	
Top	0-0-0	20-0-0	Down	Proj	20 plf	20 plf	

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
20-0-0	0.25/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	133 lbs

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	1-0-12	2-11-4	Down	Proj	15 plf	15 plf	
Top	4-11-4	6-11-4	Down	Proj	15 plf	15 plf	
Top	8-11-4	10-11-4	Down	Proj	15 plf	15 plf	
Top	12-11-4	14-11-4	Down	Proj	15 plf	15 plf	
Top	16-11-4	18-11-4	Down	Proj	15 plf	15 plf	
Top	0-0-0	20-0-0	Down	Proj	15 plf	15 plf	
Bot	1-0-12	2-11-4	Down	Proj	10 plf	10 plf	
Bot	4-11-4	6-11-4	Down	Proj	10 plf	10 plf	
Bot	8-11-4	10-11-4	Down	Proj	10 plf	10 plf	
Bot	12-11-4	14-11-4	Down	Proj	10 plf	10 plf	
Bot	16-11-4	18-11-4	Down	Proj	10 plf	10 plf	
Bot	0-0-0	20-0-0	Down	Proj	10 plf	10 plf	

User-defined Load Case S2: Drift

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	1-0-12	Down	Proj	20 plf	20 plf	
Top	1-0-12	2-11-4	Down	Proj	20 plf	20 plf	
Top	1-0-12	2-11-4	Down	Proj	20 plf	20 plf	
Top	2-11-4	4-11-4	Down	Proj	20 plf	20 plf	
Top	4-11-4	6-11-4	Down	Proj	20 plf	20 plf	
Top	4-11-4	6-11-4	Down	Proj	20 plf	20 plf	
Top	6-11-4	8-11-4	Down	Proj	20 plf	20 plf	
Top	8-11-4	10-11-4	Down	Proj	20 plf	20 plf	
Top	8-11-4	10-11-4	Down	Proj	20 plf	20 plf	
Top	10-11-4	12-11-4	Down	Proj	20 plf	20 plf	
Top	12-11-4	14-11-4	Down	Proj	20 plf	20 plf	
Top	12-11-4	14-11-4	Down	Proj	20 plf	20 plf	
Top	14-11-4	16-11-4	Down	Proj	20 plf	20 plf	
Top	16-11-4	18-11-4	Down	Proj	20 plf	20 plf	
Top	16-11-4	18-11-4	Down	Proj	20 plf	20 plf	
Top	18-11-4	20-0-0	Down	Proj	20 plf	20 plf	
Top	0-0-0	20-0-0	Down	Proj	41 psf	41 psf	24 in

Point Loads

Member	Location	Direction	Load	Trib Width
Bot	3-11-4	Down	38 lbs	
Bot	7-11-4	Down	38 lbs	
Bot	11-11-4	Down	38 lbs	
Bot	15-11-4	Down	38 lbs	

Load Combinations

#	Load Combo	Factor
1	D1	1.000
2	D1 + Lr1	1.000
3	D1 + S1	1.000
4	D1 + S2	1.000
5	0.60 D1 + 0.60 W1 [Uplift]	1.000
6	0.60 D1 + 0.60 W2 [Uplift]	1.000
7	0.60 D1 + 0.60 W4 [Uplift]	1.000
8	0.60 D1 + 0.60 W8 [Uplift]	1.000
9	D1 + L10*1	1.000
10	D1 + Lr1 + L10*1	1.000
11	D1 + S1 + L10*1	1.000
12	D1 + S2 + L10*1	1.000
13	D1 + UR1	1.000
14	D1 + UR2	1.000
15	D1 + AS10*1	1.000
16	D1 + Lr1 + AS10*1	1.000
17	D1 + S1 + AS10*1	1.000
18	D1 + S2 + AS10*1	1.000

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.419	-2,689 lbs		3-5	0.454	-3,841 lbs		6-7	0.373	523 lbs	(-247 lbs)		
	2-3	0.484	-3,951 lbs		5-6	0.422	-2,512 lbs							
BC	8-9	0.532	2,506 lbs	(-390 lbs)	10-12	0.789	3,944 lbs	(-550 lbs)	13-14	0.112	394 lbs	(-369 lbs)		
	9-10	0.749	3,835 lbs	(-506 lbs)	12-13	0.567	2,684 lbs	(-516 lbs)						

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
20-0-0	0.25 /12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	133 lbs

Web	1-14	0.319	-1,761 lbs		3-12	0.066	-481 lbs	
	1-13	0.754	3,073 lbs	(-168 lbs)	5-9	0.556	-1,535 lbs	
	2-13	0.173	-1,287 lbs		6-9	0.249	1,014 lbs	(-120 lbs)
	2-12	0.351	1,430 lbs	(-39 lbs)	6-8	0.992	-2,954 lbs	

Truss to Truss Connection Summary

Carried Truss	Carrying Chord	Carrying Offset
BP18	BC	3-11-4
BP17	BC	7-11-4
BP16	BC	11-11-4
BP15	BC	15-11-4

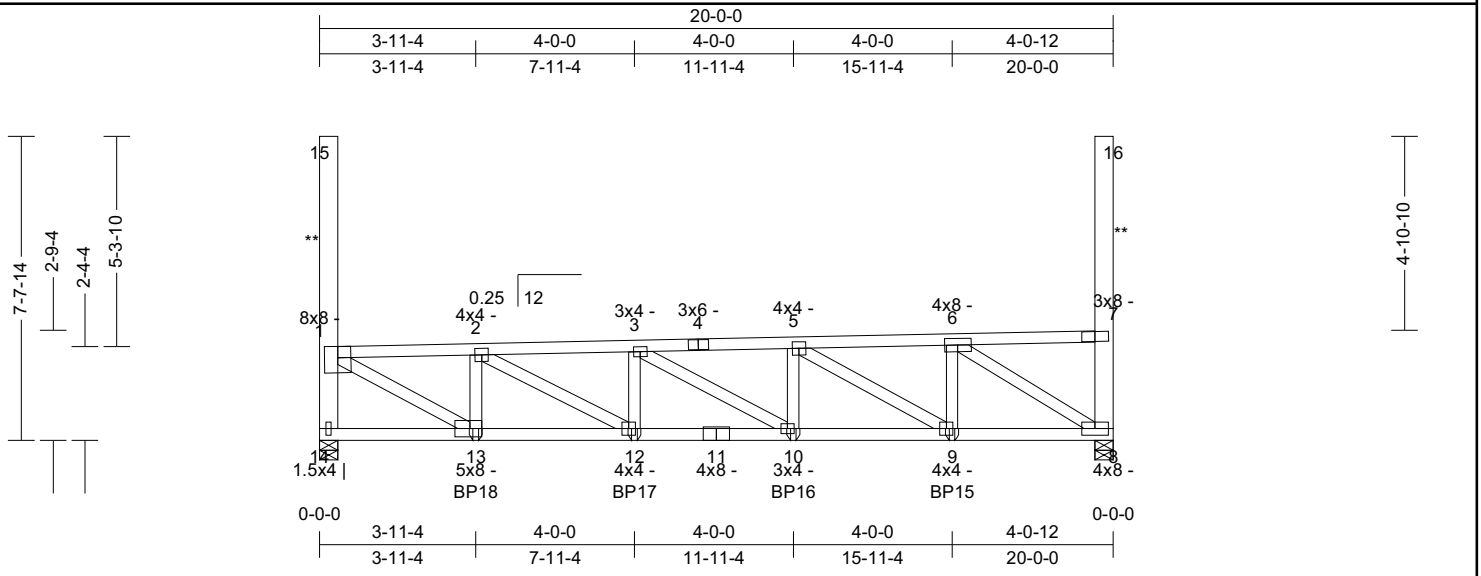
Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 30 % (Cq = 0.70).
3) Provide adequate drainage to prevent ponding.
4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
7) ⌐ Indicates non-structural members.
8) Listed wind uplift reactions based on MWFRS & C&C loading.
9) Parapet TL: 0.43 in, 2L/301 (15-1), Allowable 2L/120.

Vivco Components LLC.
2550 Hwy 33 South
P.O. Box 260
Maysville, Missouri 64469

Truss:GR3
Job: 500 NW Chipman Rd REVISED
Date: 02/16/22 10:56:42

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
20-0-0	0.25 /12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	131 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection		L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code : IBC 2018/	TC : 0.38 (3-5)	Vert TL:	0.26 in	L / 894	11	L / 240
TCLL : 20	TPI 1-2014	BC : 0.69 (10-12)	Vert LL:	0.15 in	L / 999	11	L / 360
TCDL : 15	Rep Mbr : No	Web : 0.93 (15-1)	Horz TL:	0.07 in		8	
BCLL : 0	Lumber D.O.L. : 115 %						
BCDL : 10							

02/16/2022

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
14	1	5.5 in	1.87 in	1,589 lbs	·	-74 lbs	-14 lbs	-74 lbs	487 lbs
8	1	5.5 in	2.02 in	1,708 lbs	·	-27 lbs	·	-27 lbs	·

Material

TC: SYP #1 2 x 4
BC: SYP #1 2 x 4
Web: SYP #2 2 x 4 except:
SYP SSDense 2 x 6: 15-14, 16-8

Bracing

TC: Sheathed or Purlins at 2-11-0, Purlin design by Others.
BC: Sheathed or Purlins at 7-4-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects due to 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has not been designed for the effects of unbalanced snow loads.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
5) Minimum storage attic loading has been applied in accordance with IBC 1607.1
6) ** - Indicates parapet wind loading has been applied to members 15-1 & 16-7

Load Case Lr1: Std Live Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	2-11-4	Down	Proj	20 plf	20 plf	
Top	4-11-4	6-11-4	Down	Proj	20 plf	20 plf	
Top	8-11-4	10-11-4	Down	Proj	20 plf	20 plf	
Top	12-11-4	14-11-4	Down	Proj	20 plf	20 plf	
Top	15-11-4	20-0-0	Down	Proj	20 plf	20 plf	
Top	0-0-0	2-11-4	Down	Proj	20 plf	20 plf	
Top	4-11-4	6-11-4	Down	Proj	20 plf	20 plf	
Top	8-11-4	10-11-4	Down	Proj	20 plf	20 plf	
Top	12-11-4	14-11-4	Down	Proj	20 plf	20 plf	
Top	15-11-4	20-0-0	Down	Proj	20 plf	20 plf	

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
20-0-0	0.25/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	131 lbs

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	14-4-0	16-6-0	Down	Proj	35.64 plf	35.64 plf	
Top	0-0-0	2-11-4	Down	Proj	15 plf	15 plf	
Top	4-11-4	6-11-4	Down	Proj	15 plf	15 plf	
Top	8-11-4	10-11-4	Down	Proj	15 plf	15 plf	
Top	12-11-4	14-11-4	Down	Proj	15 plf	15 plf	
Top	15-11-4	20-0-0	Down	Proj	15 plf	15 plf	
Top	0-0-0	2-11-4	Down	Proj	15 plf	15 plf	
Top	4-11-4	6-11-4	Down	Proj	15 plf	15 plf	
Top	8-11-4	10-11-4	Down	Proj	15 plf	15 plf	
Top	12-11-4	14-11-4	Down	Proj	15 plf	15 plf	
Top	15-11-4	20-0-0	Down	Proj	15 plf	15 plf	
Bot	0-0-0	2-11-4	Down	Proj	10 plf	10 plf	
Bot	4-11-4	6-11-4	Down	Proj	10 plf	10 plf	
Bot	8-11-4	10-11-4	Down	Proj	10 plf	10 plf	
Bot	12-11-4	14-11-4	Down	Proj	10 plf	10 plf	
Bot	15-11-4	20-0-0	Down	Proj	10 plf	10 plf	
Bot	0-0-0	2-11-4	Down	Proj	10 plf	10 plf	
Bot	4-11-4	6-11-4	Down	Proj	10 plf	10 plf	
Bot	8-11-4	10-11-4	Down	Proj	10 plf	10 plf	
Bot	12-11-4	14-11-4	Down	Proj	10 plf	10 plf	
Bot	15-11-4	20-0-0	Down	Proj	10 plf	10 plf	

User-defined Load Case S2: Drift

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	1-0-12	2-11-4	Down	Proj	20 plf	20 plf	
Top	1-0-12	2-11-4	Down	Proj	20 plf	20 plf	
Top	4-11-4	6-11-4	Down	Proj	20 plf	20 plf	
Top	4-11-4	6-11-4	Down	Proj	20 plf	20 plf	
Top	8-11-4	10-11-4	Down	Proj	20 plf	20 plf	
Top	8-11-4	10-11-4	Down	Proj	20 plf	20 plf	
Top	12-11-4	14-11-4	Down	Proj	20 plf	20 plf	
Top	12-11-4	14-11-4	Down	Proj	20 plf	20 plf	
Top	16-11-4	18-11-4	Down	Proj	20 plf	20 plf	
Top	16-11-4	18-11-4	Down	Proj	20 plf	20 plf	
Top	0-0-0	20-0-0	Down	Proj	29 psf	29 psf	24 in
Top	0-0-0	4-6-0	Down	Proj	3 psf	0 psf	24 in

Point Loads

Member	Location	Direction	Load	Trib Width
Bot	3-11-4	Down	38 lbs	
Bot	3-11-4	Down	38 lbs	
Bot	7-11-4	Down	38 lbs	
Bot	7-11-4	Down	38 lbs	
Bot	11-11-4	Down	38 lbs	
Bot	11-11-4	Down	38 lbs	
Bot	15-11-4	Down	38 lbs	
Bot	15-11-4	Down	38 lbs	

Load Combinations

#	Load Combo	Factor
1	D1	1.000
2	D1 + Lr1	1.000
3	D1 + S1	1.000
4	D1 + S2	1.000
5	0.60 D1 + 0.60 W1 [Uplift]	1.000
6	0.60 D1 + 0.60 W2 [Uplift]	1.000
7	0.60 D1 + 0.60 W4 [Uplift]	1.000
8	0.60 D1 + 0.60 W8 [Uplift]	1.000
9	D1 + L10*1	1.000
10	D1 + Lr1 + L10*1	1.000
11	D1 + S1 + L10*1	1.000
12	D1 + S2 + L10*1	1.000
13	D1 + UR1	1.000
14	D1 + UR2	1.000
15	D1 + AS10*1	1.000
16	D1 + Lr1 + AS10*1	1.000
17	D1 + S1 + AS10*1	1.000
18	D1 + S2 + AS10*1	1.000

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
20-0-0	0.25 /12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	131 lbs

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.377	-2,324 lbs		3-5	0.377	-3,400 lbs		6-7	0.314	1,048 lbs	(-492 lbs)
	2-3	0.370	-3,432 lbs		5-6	0.358	-2,337 lbs					
BC	8-9	0.521	2,333 lbs	(-416 lbs)	10-12	0.691	3,427 lbs	(-540 lbs)	13-14	0.118	798 lbs	(-717 lbs)
	9-10	0.660	3,396 lbs	(-462 lbs)	12-13	0.491	2,320 lbs	(-634 lbs)				
Web	1-14	0.647	-1,516 lbs		3-12	0.048	-350 lbs					
	1-13	0.652	2,656 lbs	(-198 lbs)	5-9	0.444	-1,228 lbs					
	2-13	0.140	-1,036 lbs		6-9	0.240	980 lbs	(-30 lbs)				
	2-12	0.308	1,256 lbs	(-189 lbs)	6-8	0.924	-2,750 lbs					

Truss to Truss Connection Summary

Carried Truss	Carrying Chord	Carrying Offset
BP18	BC	3-11-4
BP18	BC	3-11-4
BP17	BC	7-11-4
BP17	BC	7-11-4
BP16	BC	11-11-4
BP16	BC	11-11-4
BP15	BC	15-11-4
BP15	BC	15-11-4

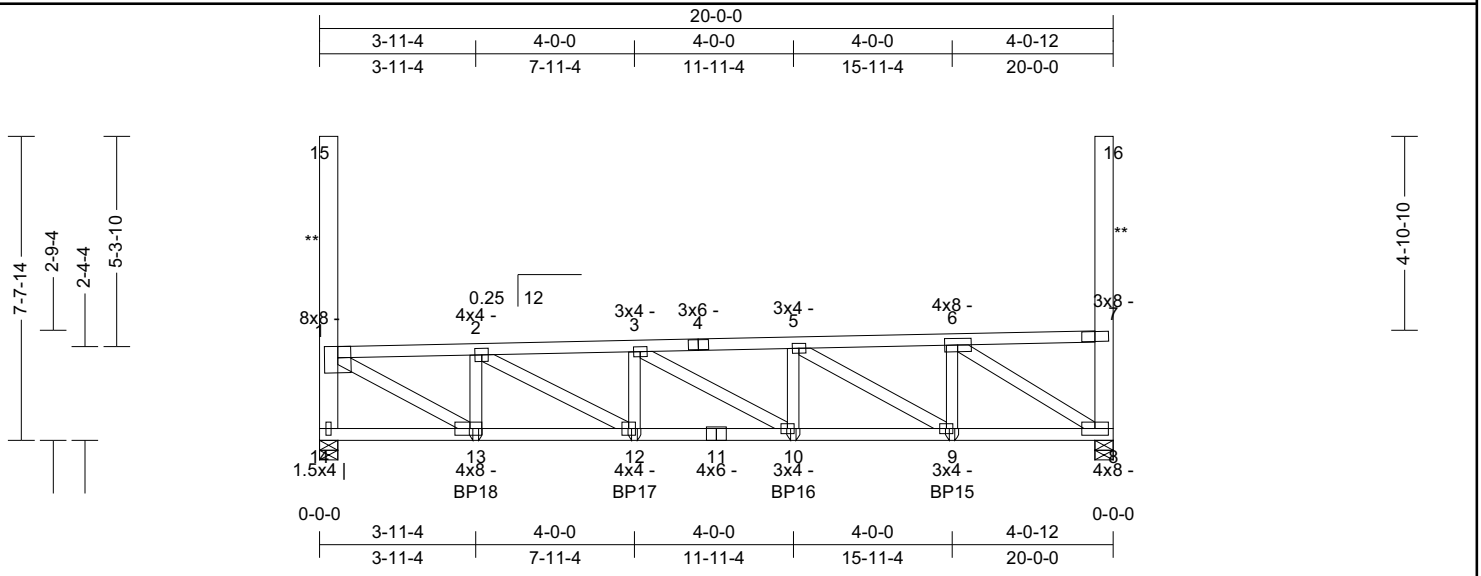
Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 30 % (Cq = 0.70).
3) Provide adequate drainage to prevent ponding.
4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
7) Listed wind uplift reactions based on MWFRS & C&C loading.
8) Parapet TL: 0.84 in, 2L/155 (15-1), Allowable 2L/120.

Vivco Components LLC.
2550 Hwy 33 South
P.O. Box 260
Maysville, Missouri 64469

Truss:GR4
Job: 500 NW Chipman Rd REVISED
Date: 02/16/22 10:56:45

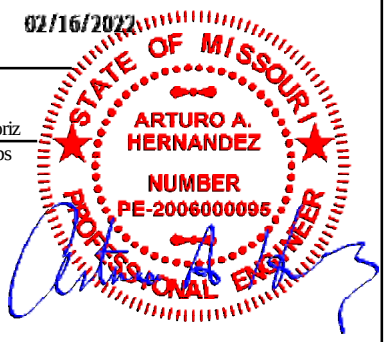
SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
20-0-0	0.25 /12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	131 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code : IBC 2018/	TC : 0.35 (1-2)	Vert TL: 0.23 in	L / 999	11	L / 240
TCLL : 20	TPI 1-2014	BC : 0.61 (10-12)	Vert LL: 0.11 in	L / 999	11	L / 360
TCDL : 15	Rep Mbr : No	Web : 0.93 (15-1)	Horz TL: 0.06 in		8	
BCLL : 0	Lumber D.O.L. : 115 %					
BCDL : 10						

02/16/2022



Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
14	1	5.5 in	1.68 in	1,424 lbs	·	-196 lbs	-124 lbs	-196 lbs	489 lbs
8	1	5.5 in	1.78 in	1,511 lbs	·	·	-87 lbs	-87 lbs	·

Material

TC: SYP #1 2 x 4
BC: SYP #1 2 x 4
Web: SYP #2 2 x 4 except:
SYP SSDense 2 x 6: 15-14, 16-8

Bracing

TC: Sheathed or Purlins at 3-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 6-9-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects due to 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has not been designed for the effects of unbalanced snow loads.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
5) Minimum storage attic loading has been applied in accordance with IBC 1607.1
6) ** - Indicates parapet wind loading has been applied to members 15-1 & 16-7

Load Case Lr1: Std Live Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	20-0-0	Down	Proj	20 plf	20 plf	
Top	0-0-0	2-11-4	Down	Proj	20 plf	20 plf	
Top	4-11-4	6-11-4	Down	Proj	20 plf	20 plf	
Top	8-11-4	10-11-4	Down	Proj	20 plf	20 plf	
Top	12-11-4	14-11-4	Down	Proj	20 plf	20 plf	
Top	15-11-4	20-0-0	Down	Proj	20 plf	20 plf	

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
20-0-0	0.25/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	131 lbs

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	14-4-0	16-6-0	Down	Proj	35.64 plf	35.64 plf	
Top	0-0-0	20-0-0	Down	Proj	15 plf	15 plf	
Top	0-0-0	2-11-4	Down	Proj	15 plf	15 plf	
Top	4-11-4	6-11-4	Down	Proj	15 plf	15 plf	
Top	8-11-4	10-11-4	Down	Proj	15 plf	15 plf	
Top	12-11-4	14-11-4	Down	Proj	15 plf	15 plf	
Top	15-11-4	20-0-0	Down	Proj	15 plf	15 plf	
Bot	0-0-0	20-0-0	Down	Proj	10 plf	10 plf	
Bot	0-0-0	2-11-4	Down	Proj	10 plf	10 plf	
Bot	4-11-4	6-11-4	Down	Proj	10 plf	10 plf	
Bot	8-11-4	10-11-4	Down	Proj	10 plf	10 plf	
Bot	12-11-4	14-11-4	Down	Proj	10 plf	10 plf	
Bot	15-11-4	20-0-0	Down	Proj	10 plf	10 plf	

User-defined Load Case S2: Drift

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	1-0-12	2-11-4	Down	Proj	20 plf	20 plf	
Top	1-0-12	2-11-4	Down	Proj	20 plf	20 plf	
Top	2-11-4	4-11-4	Down	Proj	20 plf	20 plf	
Top	4-11-4	6-11-4	Down	Proj	20 plf	20 plf	
Top	4-11-4	6-11-4	Down	Proj	20 plf	20 plf	
Top	6-11-4	8-11-4	Down	Proj	20 plf	20 plf	
Top	8-11-4	10-11-4	Down	Proj	20 plf	20 plf	
Top	8-11-4	10-11-4	Down	Proj	20 plf	20 plf	
Top	10-11-4	12-11-4	Down	Proj	20 plf	20 plf	
Top	12-11-4	14-11-4	Down	Proj	20 plf	20 plf	
Top	12-11-4	14-11-4	Down	Proj	20 plf	20 plf	
Top	14-11-4	16-11-4	Down	Proj	20 plf	20 plf	
Top	16-11-4	18-11-4	Down	Proj	20 plf	20 plf	
Top	16-11-4	18-11-4	Down	Proj	20 plf	20 plf	
Top	0-0-0	20-0-0	Down	Proj	16 psf	16 psf	24 in
Top	0-0-0	4-6-0	Down	Proj	16 psf	0 psf	24 in
Top	15-6-0	20-0-0	Down	Proj	0 psf	16 psf	24 in

Point Loads

Member	Location	Direction	Load	Trib Width
Bot	3-11-4	Down	38 lbs	
Bot	7-11-4	Down	38 lbs	
Bot	11-11-4	Down	38 lbs	
Bot	15-11-4	Down	38 lbs	

Load Combinations

#	Load Combo	Factor
1	D1	1.000
2	D1 + Lr1	1.000
3	D1 + S1	1.000
4	D1 + S2	1.000
5	0.60 D1 + 0.60 W1 [Uplift]	1.000
6	0.60 D1 + 0.60 W2 [Uplift]	1.000
7	0.60 D1 + 0.60 W4 [Uplift]	1.000
8	0.60 D1 + 0.60 W8 [Uplift]	1.000
9	D1 + L10*1	1.000
10	D1 + Lr1 + L10*1	1.000
11	D1 + S1 + L10*1	1.000
12	D1 + S2 + L10*1	1.000
13	D1 + UR1	1.000
14	D1 + UR2	1.000
15	D1 + AS10*1	1.000
16	D1 + Lr1 + AS10*1	1.000
17	D1 + S1 + AS10*1	1.000
18	D1 + S2 + AS10*1	1.000

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.346	-2,054 lbs		3-5	0.313	-2,966 lbs		6-7	0.297	1,048 lbs	(-492 lbs)
	2-3	0.320	-3,005 lbs		5-6	0.322	-2,029 lbs					
BC	8-9	0.462	2,025 lbs	(-595 lbs)	10-12	0.615	3,000 lbs	(-820 lbs)	13-14	0.116	796 lbs	(-721 lbs)
	9-10	0.587	2,962 lbs	(-736 lbs)	12-13	0.444	2,050 lbs	(-825 lbs)				

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
20-0-0	0.25 /12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	131 lbs

Web	1-14	0.643	-1,351 lbs		3-12	0.043	-313 lbs	
	1-13	0.576	2,347 lbs	(-437 lbs)	5-9	0.392	-1,082 lbs	
	2-13	0.125	-930 lbs		6-9	0.199	810 lbs	
	2-12	0.264	1,078 lbs	(-315 lbs)	6-8	0.802	-2,387 lbs	

Truss to Truss Connection Summary

Carried Truss	Carrying Chord	Carrying Offset
BP18	BC	3-11-4
BP17	BC	7-11-4
BP16	BC	11-11-4
BP15	BC	15-11-4

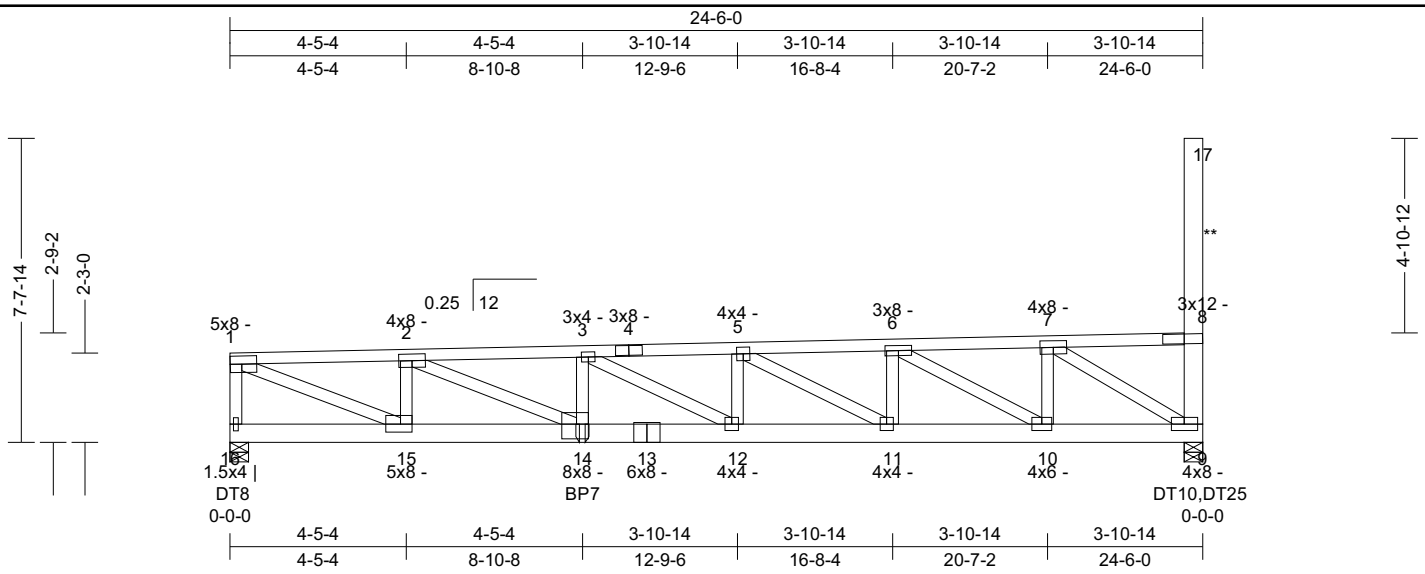
Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 30 % (Cq = 0.70).
3) Provide adequate drainage to prevent ponding.
4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
7) Listed wind uplift reactions based on MWFRS & C&C loading.
8) Parapet TL: 0.85 in, 2L/153 (15-1), Allowable 2L/120.

Vivco Components LLC.
2550 Hwy 33 South
P.O. Box 260
Maysville, Missouri 64469

Truss:GR5
Job: 500 NW Chipman Rd REVISED
Date: 02/16/22 10:56:47

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
24-6-0	0.25/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	159 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code: IBC 2018/	TC: 0.53 (2-3)	Vert TL: 0.42 in	L/672	(12-13)	L/240
TCLL: 20	TPI 1-2014	BC: 0.44 (12-14)	Vert LL: 0.16 in	L/999	(12-13)	L/360
TCDL: 15	Rep Mbr: No	Web: 0.85 (1-15)	Horz TL: 0.06 in		9	
BCLL: 0	Lumber D.O.L.: 115 %					
BCDL: 10						

02/16/2022

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
16	1	5.5 in	1.75 in	1,729 lbs	·	·	-210 lbs	-210 lbs	-479 lbs
9	1	5.5 in	1.65 in	1,634 lbs	·	-112 lbs	-109 lbs	-112 lbs	·

Material

TC: SYP #1 2 x 4
BC: SYP SSDense 2 x 6
Web: SYP #2 2 x 4 except:
SYP SSDense 2 x 6: 17-9

Bracing

TC: Sheathed or Purlins at 2-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects due to 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has not been designed for the effects of unbalanced snow loads.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
5) Minimum storage attic loading has been applied in accordance with IBC 1607.1
6) ** - Indicates parapet wind loading has been applied to member 17-8

Load Case Lr1: Std Live Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	7-10-8	Down	Proj	20 plf	20 plf	
Top	8-10-8	24-6-0	Down	Proj	20 plf	20 plf	
Top	0-0-0	7-10-8	Down	Proj	20 plf	20 plf	
Top	8-10-8	24-6-0	Down	Proj	20 plf	20 plf	

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
24-6-0	0.25 /12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	159 lbs

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	7-10-8	Down	Proj	15 plf	15 plf	
Top	8-10-8	24-6-0	Down	Proj	15 plf	15 plf	
Top	0-0-0	7-10-8	Down	Proj	15 plf	15 plf	
Top	8-10-8	24-6-0	Down	Proj	15 plf	15 plf	
Bot	0-0-0	7-10-8	Down	Proj	10 plf	10 plf	
Bot	8-10-8	24-6-0	Down	Proj	10 plf	10 plf	
Bot	0-0-0	7-10-8	Down	Proj	10 plf	10 plf	
Bot	8-10-8	24-6-0	Down	Proj	10 plf	10 plf	

User-defined Load Case S2: Snow Drift

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	9-0-0	24-6-0	Down	Proj	5 psf	5 psf	24 in
Top	9-0-0	13-0-0	Down	Proj	27 psf	27 psf	24 in
Top	20-6-0	24-6-0	Down	Proj	0 psf	27 psf	24 in

Point Loads

Member	Location	Direction	Load	Trib Width

User-defined Load Case W12: W1 (DOL = 1.60)

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	1-0-12	7-6-0	N Up	Rake	25.39 plf	25.39 plf	
Top	1-0-12	7-6-0	N Up	Rake	25.39 plf	25.39 plf	
Top	7-6-0	7-10-8	N Up	Rake	25.39 plf	25.39 plf	
Top	7-6-0	7-10-8	N Up	Rake	25.39 plf	25.39 plf	
Top	9-10-8	15-0-0	N Up	Rake	25.39 plf	25.39 plf	
Top	9-10-8	15-0-0	N Up	Rake	25.39 plf	25.39 plf	
Top	15-0-0	23-5-4	N Up	Rake	15.97 plf	15.97 plf	
Top	15-0-0	23-5-4	N Up	Rake	15.97 plf	15.97 plf	

Point Loads

Member	Location	Direction	Load	Trib Width
Bot	0-0-12	Down	44 lbs	
Bot	8-10-8	Down	44 lbs	
Bot	8-10-8	Up	139 lbs	
Bot	24-5-4	Down	86 lbs	
Bot	24-5-4	Down	86 lbs	
Top	0-0-0	Right	600 lbs	

User-defined Load Case W13: W2 (DOL = 1.60)

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	1-0-12	7-10-8	N Up	Rake	15.97 plf	15.97 plf	
Top	1-0-12	7-10-8	N Up	Rake	15.97 plf	15.97 plf	
Top	9-10-8	17-0-0	N Up	Rake	25.39 plf	25.39 plf	
Top	9-10-8	17-0-0	N Up	Rake	25.39 plf	25.39 plf	
Top	17-0-0	23-5-4	N Up	Rake	25.39 plf	25.39 plf	
Top	17-0-0	23-5-4	N Up	Rake	25.39 plf	25.39 plf	

Point Loads

Member	Location	Direction	Load	Trib Width
Bot	0-0-12	Up	115 lbs	
Bot	8-10-8	Up	139 lbs	
Bot	8-10-8	Down	44 lbs	
Bot	24-5-4	Up	159 lbs	
Bot	24-5-4	Up	159 lbs	
Top	0-0-0	Left	600 lbs	

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
24-6-0	0.25 /12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	159 lbs

User-defined Load Case W14: W4(DOL = 1.60)

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	1-0-12	3-0-0	N Up	Rake	48.89 plf	48.89 plf	
Top	1-0-12	3-0-0	N Up	Rake	48.89 plf	48.89 plf	
Top	3-0-0	7-10-8	N Up	Rake	43.75 plf	43.75 plf	
Top	3-0-0	7-10-8	N Up	Rake	43.75 plf	43.75 plf	
Top	9-10-8	23-5-4	N Up	Rake	43.75 plf	43.75 plf	
Top	9-10-8	23-5-4	N Up	Rake	43.75 plf	43.75 plf	

Point Loads

Member	Location	Direction	Load	Trib Width
Top	0-0-0	Left	600 lbs	

Load Combinations

#	Load Combo	Factor
1	D1	1.000
2	D1 + Lr1	1.000
3	D1 + S1	1.000
4	D1 + S2	1.000
5	0.60 D1 + 0.60 W1 [Uplift]	1.000
6	0.60 D1 + 0.60 W2 [Uplift]	1.000
7	0.60 D1 + 0.60 W4 [Uplift]	1.000
8	0.60 D1 + 0.60 W8 [Uplift]	1.000
9	0.60 D1 + 0.60 W0 [Uplift]	1.000
10	0.60 D1 + 0.60 W0 [Uplift]	1.000
11	0.60 D1 + 0.60 W0 [Uplift]	1.000
12	D1 + L10*1	1.000
13	D1 + Lr1 + L10*1	1.000
14	D1 + S1 + L10*1	1.000
15	D1 + S2 + L10*1	1.000
16	D1 + UR1	1.000
17	D1 + UR2	1.000
18	D1 + AS10*1	1.000
19	D1 + Lr1 + AS10*1	1.000
20	D1 + S1 + AS10*1	1.000
21	D1 + S2 + AS10*1	1.000

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.391	-3,159 lbs		3-5	0.534	-4,792 lbs		6-7	0.227	-2,156 lbs	
	2-3	0.534	-5,000 lbs		5-6	0.399	-3,791 lbs		7-8	0.197	1,064 lbs	(-501 lbs)
BC	9-10	0.179	2,153 lbs	(-577 lbs)	11-12	0.388	4,788 lbs	(-329 lbs)	14-15	0.312	3,156 lbs	(-359 lbs)
	10-11	0.307	3,788 lbs	(-534 lbs)	12-14	0.436	4,995 lbs	(-135 lbs)	15-16	0.065	516 lbs	(-383 lbs)
Web	1-16	0.199	-1,582 lbs		3-14	0.055	309 lbs	(-161 lbs)	6-11	0.191	780 lbs	
	1-15	0.845	3,444 lbs	(-396 lbs)	3-12	0.136	-417 lbs		6-10	0.635	-1,881 lbs	
	2-15	0.152	-1,193 lbs		5-12	0.086	350 lbs		7-10	0.268	1,091 lbs	(-12 lbs)
	2-14	0.546	2,224 lbs	(-123 lbs)	5-11	0.434	-1,306 lbs		7-9	0.774	-2,541 lbs	

Truss to Truss Connection Summary

Carried Truss	Carrying Chord	Carrying Offset
DT8	BC	0-0-12
BP7	BC	8-10-8
BP7	BC	8-10-8
DT10	BC	24-5-4
DT25	BC	24-5-4

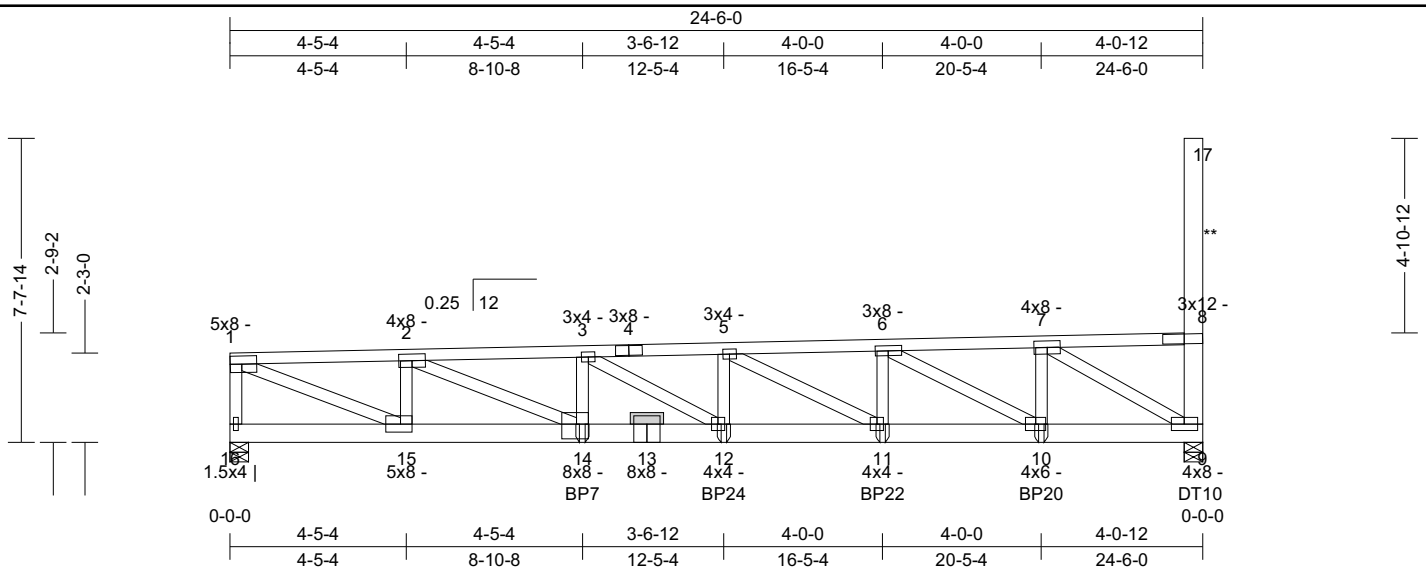
Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 30 % (Cq = 0.70).
3) Provide adequate drainage to prevent ponding.
4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
7) Listed wind uplift reactions based on MWFRS & C&C loading.
8) Parapet TL: 0.67 in, 2L/180 (17-8), Allowable 2L/120.

Vivco Components LLC.
2550 Hwy 33 South
P.O. Box 260
Maysville, Missouri 64469

Truss:GR6
Job: 500 NW Chipman Rd REVISED
Date: 02/16/22 10:56:49

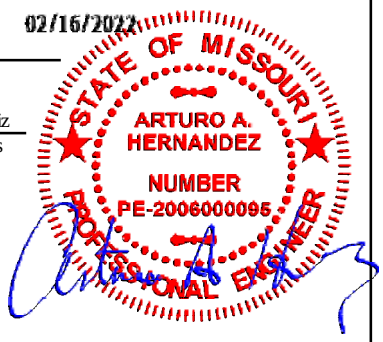
SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
24-6-0	0.25 /12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	160 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection		L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code : IBC 2018/	TC : 0.55 (2-3)	Vert TL:	0.43 in	L / 657	(12-13)	L / 240
TCLL : 20	TPI 1-2014	BC : 0.44 (12-14)	Vert LL:	0.18 in	L / 999	(12-13)	L / 360
TCDL : 15	Rep Mbr : No	Web : 0.88 (7-9)	Horz TL:	0.06 in		9	
BCLL : 0	Lumber D.O.L. : 115 %						
BCDL : 10							

02/16/2022



Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
16	1	5.5 in	1.65 in	1,629 lbs	·	·	-171 lbs	-171 lbs	-482 lbs
9	1	5.5 in	1.70 in	1,684 lbs	·	-241 lbs	·	-241 lbs	·

Material

TC: SYP #1 2 x 4
BC: SYP SSDense 2 x 6
Web: SYP #2 2 x 4 except:
SYP SSDense 2 x 6: 17-9

Bracing

TC: Sheathed or Purlins at 2-4-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects due to 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has not been designed for the effects of unbalanced snow loads.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
5) Minimum storage attic loading has been applied in accordance with IBC 1607.1
6) ** - Indicates parapet wind loading has been applied to member 17-8

Load Case Lr1: Std Live Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	7-10-8	Down	Proj	20 plf	20 plf	
Top	9-10-8	11-5-4	Down	Proj	20 plf	20 plf	
Top	13-5-4	15-5-4	Down	Proj	20 plf	20 plf	
Top	17-5-4	19-5-4	Down	Proj	20 plf	20 plf	
Top	20-5-4	24-6-0	Down	Proj	20 plf	20 plf	
Top	0-0-0	7-10-8	Down	Proj	20 plf	20 plf	
Top	8-10-8	24-6-0	Down	Proj	20 plf	20 plf	

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
24-6-0	0.25 /12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	160 lbs

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	7-10-8	Down	Proj	15 plf	15 plf	
Top	9-10-8	11-5-4	Down	Proj	15 plf	15 plf	
Top	13-5-4	15-5-4	Down	Proj	15 plf	15 plf	
Top	17-5-4	19-5-4	Down	Proj	15 plf	15 plf	
Top	20-5-4	24-6-0	Down	Proj	15 plf	15 plf	
Top	0-0-0	7-10-8	Down	Proj	15 plf	15 plf	
Top	8-10-8	24-6-0	Down	Proj	15 plf	15 plf	
Bot	0-0-0	7-10-8	Down	Proj	10 plf	10 plf	
Bot	9-10-8	11-5-4	Down	Proj	10 plf	10 plf	
Bot	13-5-4	15-5-4	Down	Proj	10 plf	10 plf	
Bot	17-5-4	19-5-4	Down	Proj	10 plf	10 plf	
Bot	20-5-4	24-6-0	Down	Proj	10 plf	10 plf	
Bot	0-0-0	7-10-8	Down	Proj	10 plf	10 plf	
Bot	8-10-8	24-6-0	Down	Proj	10 plf	10 plf	

User-defined Load Case S2: Snow Drift

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	9-0-0	24-6-0	Down	Proj	24 psf	24 psf	24 in
Top	9-0-0	11-0-0	Down	Proj	8 psf	8 psf	24 in
Top	22-6-0	24-6-0	Down	Proj	0 psf	8 psf	24 in

Point Loads

Member	Location	Direction	Load	Trib Width

User-defined Load Case W12: W1(DOL = 1.60)

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	1-0-12	7-6-0	N Up	Rake	25.39 plf	25.39 plf	
Top	1-0-12	7-6-0	N Up	Rake	25.39 plf	25.39 plf	
Top	7-6-0	7-10-8	N Up	Rake	25.39 plf	25.39 plf	
Top	7-6-0	7-10-8	N Up	Rake	25.39 plf	25.39 plf	
Top	9-10-8	11-5-4	N Up	Rake	25.39 plf	25.39 plf	
Top	9-10-8	11-5-4	N Up	Rake	25.39 plf	25.39 plf	
Top	11-5-4	13-5-4	N Up	Rake	25.39 plf	25.39 plf	
Top	13-5-4	15-0-0	N Up	Rake	25.39 plf	25.39 plf	
Top	13-5-4	15-0-0	N Up	Rake	25.39 plf	25.39 plf	
Top	15-0-0	15-5-4	N Up	Rake	15.97 plf	15.97 plf	
Top	15-0-0	15-5-4	N Up	Rake	15.97 plf	15.97 plf	
Top	15-5-4	17-5-4	N Up	Rake	15.97 plf	15.97 plf	
Top	17-5-4	19-5-4	N Up	Rake	15.97 plf	15.97 plf	
Top	17-5-4	19-5-4	N Up	Rake	15.97 plf	15.97 plf	
Top	19-5-4	21-5-4	N Up	Rake	15.97 plf	15.97 plf	
Top	21-5-4	23-5-4	N Up	Rake	15.97 plf	15.97 plf	
Top	21-5-4	23-5-4	N Up	Rake	15.97 plf	15.97 plf	

Point Loads

Member	Location	Direction	Load	Trib Width
Bot	8-10-8	Down	44 lbs	
Bot	8-10-8	Up	139 lbs	
Bot	12-5-4	Down	38 lbs	
Bot	16-5-4	Down	46 lbs	
Bot	20-5-4	Down	53 lbs	
Bot	24-5-4	Down	86 lbs	
Bot	24-5-4	Down	86 lbs	
Top	0-0-0	Right	600 lbs	

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
24-6-0	0.25 /12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	160 lbs

User-defined Load Case W13: W2 (DOL = 1.60)

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	1-0-12	7-10-8	N Up	Rake	15.97 plf	15.97 plf	
Top	1-0-12	7-10-8	N Up	Rake	15.97 plf	15.97 plf	
Top	9-10-8	11-5-4	N Up	Rake	25.39 plf	25.39 plf	
Top	9-10-8	11-5-4	N Up	Rake	25.39 plf	25.39 plf	
Top	11-5-4	13-5-4	N Up	Rake	25.39 plf	25.39 plf	
Top	13-5-4	15-5-4	N Up	Rake	25.39 plf	25.39 plf	
Top	13-5-4	15-5-4	N Up	Rake	25.39 plf	25.39 plf	
Top	15-5-4	17-0-0	N Up	Rake	25.39 plf	25.39 plf	
Top	17-0-0	17-5-4	N Up	Rake	25.39 plf	25.39 plf	
Top	17-5-4	19-5-4	N Up	Rake	25.39 plf	25.39 plf	
Top	17-5-4	19-5-4	N Up	Rake	25.39 plf	25.39 plf	
Top	19-5-4	21-5-4	N Up	Rake	25.39 plf	25.39 plf	
Top	21-5-4	23-5-4	N Up	Rake	25.39 plf	25.39 plf	
Top	21-5-4	23-5-4	N Up	Rake	25.39 plf	25.39 plf	

Point Loads

Member	Location	Direction	Load	Trib Width
Bot	8-10-8	Up	139 lbs	
Bot	8-10-8	Down	44 lbs	
Bot	12-5-4	Up	162 lbs	
Bot	16-5-4	Up	169 lbs	
Bot	20-5-4	Up	176 lbs	
Bot	24-5-4	Up	159 lbs	
Bot	24-5-4	Up	159 lbs	
Top	0-0-0	Left	600 lbs	

User-defined Load Case W14: W4 (DOL = 1.60)

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	1-0-12	3-0-0	N Up	Rake	48.89 plf	48.89 plf	
Top	1-0-12	3-0-0	N Up	Rake	48.89 plf	48.89 plf	
Top	3-0-0	7-10-8	N Up	Rake	43.75 plf	43.75 plf	
Top	3-0-0	7-10-8	N Up	Rake	43.75 plf	43.75 plf	
Top	9-10-8	11-5-4	N Up	Rake	43.75 plf	43.75 plf	
Top	9-10-8	11-5-4	N Up	Rake	43.75 plf	43.75 plf	
Top	11-5-4	13-5-4	N Up	Rake	43.75 plf	43.75 plf	
Top	13-5-4	15-5-4	N Up	Rake	43.75 plf	43.75 plf	
Top	13-5-4	15-5-4	N Up	Rake	43.75 plf	43.75 plf	
Top	15-5-4	17-5-4	N Up	Rake	43.75 plf	43.75 plf	
Top	17-5-4	19-5-4	N Up	Rake	43.75 plf	43.75 plf	
Top	17-5-4	19-5-4	N Up	Rake	43.75 plf	43.75 plf	
Top	19-5-4	21-5-4	N Up	Rake	43.75 plf	43.75 plf	
Top	21-5-4	23-5-4	N Up	Rake	43.75 plf	43.75 plf	
Top	21-5-4	23-5-4	N Up	Rake	43.75 plf	43.75 plf	

Point Loads

Member	Location	Direction	Load	Trib Width
Top	0-0-0	Left	600 lbs	

Load Combinations

#	Load Combo	Factor
1	D1	1.000
2	D1 + Lr1	1.000
3	D1 + S1	1.000
4	D1 + S2	1.000
5	0.60 D1 + 0.60 W1 [Uplift]	1.000
6	0.60 D1 + 0.60 W2 [Uplift]	1.000
7	0.60 D1 + 0.60 W4 [Uplift]	1.000
8	0.60 D1 + 0.60 W8 [Uplift]	1.000
9	0.60 D1 + 0.60 W0 [Uplift]	1.000
10	0.60 D1 + 0.60 W0 [Uplift]	1.000
11	0.60 D1 + 0.60 W0 [Uplift]	1.000
12	D1 + L10*1	1.000
13	D1 + Lr1 + L10*1	1.000
14	D1 + S1 + L10*1	1.000
15	D1 + S2 + L10*1	1.000
16	D1 + UR1	1.000
17	D1 + UR2	1.000
18	D1 + AS10*1	1.000

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
24-6-0	0.25 /12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	160 lbs

19	D1 + Lr1 + AS10*1	1.000
20	D1 + S1 + AS10*1	1.000
21	D1 + S2 + AS10*1	1.000

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.363	-3,047 lbs		3-5	0.521	-5,001 lbs		6-7	0.249	-2,344 lbs	
	2-3	0.547	-5,119 lbs		5-6	0.423	-4,013 lbs		7-8	0.211	1,064 lbs	(-501 lbs)
BC	9-10	0.198	2,340 lbs	(-419 lbs)	11-12	0.406	4,997 lbs	(-79 lbs)	14-15	0.310	3,043 lbs	(-267 lbs)
	10-11	0.324	4,009 lbs	(-258 lbs)	12-14	0.443	5,117 lbs		15-16	0.063	519 lbs	(-379 lbs)
Web	1-16	0.192	-1,529 lbs		3-12	0.089	-327 lbs		7-10	0.274	1,115 lbs	(-54 lbs)
	1-15	0.815	3,322 lbs	(-271 lbs)	5-11	0.386	-1,120 lbs		7-9	0.882	-2,730 lbs	
	2-15	0.146	-1,142 lbs		6-11	0.169	687 lbs					
	2-14	0.563	2,295 lbs		6-10	0.667	-1,908 lbs					

Truss to Truss Connection Summary

Carried Truss	Carrying Chord	Carrying Offset
BP7	BC	8-10-8
BP7	BC	8-10-8
BP24	BC	12-5-4
BP22	BC	16-5-4
BP20	BC	20-5-4
DT10	BC	24-5-4
DT10	BC	24-5-4

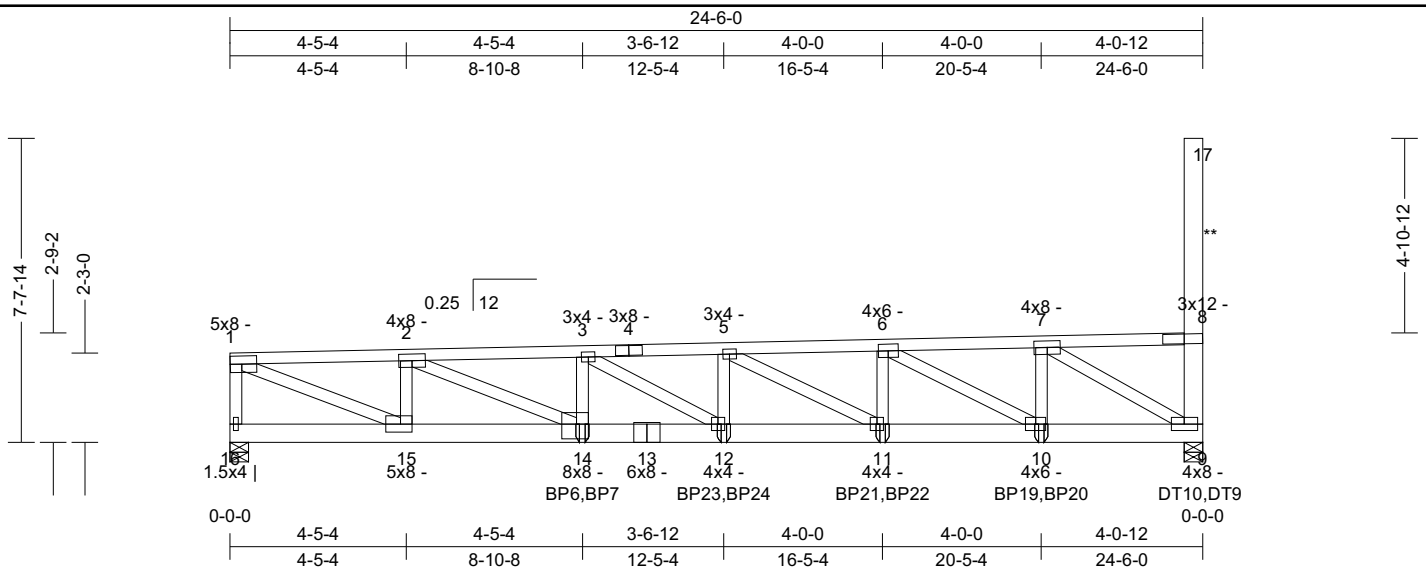
Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 30 % (Cq = 0.70).
3) Provide adequate drainage to prevent ponding.
4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
7) Indicates non-structural members.
8) Listed wind uplift reactions based on MWFRS & C&C loading.
9) Parapet TL: 0.66 in, 2L/183 (17-8), Allowable 2L/120.

Vivco Components LLC.
2550 Hwy 33 South
P.O. Box 260
Maysville, Missouri 64469

Truss:GR7
Job: 500 NW Chipman Rd REVISED
Date: 02/16/22 10:56:52

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
24-6-0	0.25 /12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	23.75 in	159 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection		L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code : IBC 2018/ TPI 1-2014	TC : 0.55 (2-3)	Vert TL:	0.44 in	L / 648	(12-13)	L / 240
TCLL : 20	Rep Mbr : No	BC : 0.44 (12-14)	Vert LL:	0.2 in	L / 999	(12-13)	L / 360
TCDL : 15	Lumber D.O.L. : 115 %	Web : 0.92 (7-9)	Horz TL:	0.06 in		9	
BCLL : 0							
BCDL : 10							

02/16/2022

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
16	1	5.5 in	1.57 in	1,551 lbs	·	·	-115 lbs	-115 lbs	-481 lbs
9	1	5.5 in	1.77 in	1,756 lbs	·	-118 lbs	·	-118 lbs	·

Material

TC: SYP #1 2 x 4
BC: SYP SSDense 2 x 6
Web: SYP #2 2 x 4 except:
SYP SSDense 2 x 6: 17-9

Bracing

TC: Sheathed or Purlins at 2-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects due to 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has not been designed for the effects of unbalanced snow loads.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
5) Minimum storage attic loading has been applied in accordance with IBC 1607.1
6) ** - Indicates parapet wind loading has been applied to member 17-8

Load Case Lr1: Std Live Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	7-10-8	Down	Proj	19.58 plf	19.58 plf	
Top	9-10-8	11-5-4	Down	Proj	19.58 plf	19.58 plf	
Top	13-5-4	15-5-4	Down	Proj	19.58 plf	19.58 plf	
Top	17-5-4	19-5-4	Down	Proj	19.58 plf	19.58 plf	
Top	21-5-4	24-6-0	Down	Proj	19.58 plf	19.58 plf	
Top	0-0-0	7-10-8	Down	Proj	20 plf	20 plf	
Top	9-10-8	11-5-4	Down	Proj	20 plf	20 plf	
Top	13-5-4	15-5-4	Down	Proj	20 plf	20 plf	
Top	17-5-4	19-5-4	Down	Proj	20 plf	20 plf	
Top	20-5-4	24-6-0	Down	Proj	20 plf	20 plf	

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
24-6-0	0.25 /12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	23.75 in	159 lbs

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	7-10-8	Down	Proj	14.69 plf	14.69 plf	
Top	9-10-8	11-5-4	Down	Proj	14.69 plf	14.69 plf	
Top	13-5-4	15-5-4	Down	Proj	14.69 plf	14.69 plf	
Top	17-5-4	19-5-4	Down	Proj	14.69 plf	14.69 plf	
Top	21-5-4	24-6-0	Down	Proj	14.69 plf	14.69 plf	
Top	0-0-0	7-10-8	Down	Proj	15 plf	15 plf	
Top	9-10-8	11-5-4	Down	Proj	15 plf	15 plf	
Top	13-5-4	15-5-4	Down	Proj	15 plf	15 plf	
Top	17-5-4	19-5-4	Down	Proj	15 plf	15 plf	
Top	20-5-4	24-6-0	Down	Proj	15 plf	15 plf	
Bot	0-0-0	7-10-8	Down	Proj	9.79 plf	9.79 plf	
Bot	9-10-8	11-5-4	Down	Proj	9.79 plf	9.79 plf	
Bot	13-5-4	15-5-4	Down	Proj	9.79 plf	9.79 plf	
Bot	17-5-4	19-5-4	Down	Proj	9.79 plf	9.79 plf	
Bot	21-5-4	24-6-0	Down	Proj	9.79 plf	9.79 plf	
Bot	0-0-0	7-10-8	Down	Proj	10 plf	10 plf	
Bot	9-10-8	11-5-4	Down	Proj	10 plf	10 plf	
Bot	13-5-4	15-5-4	Down	Proj	10 plf	10 plf	
Bot	17-5-4	19-5-4	Down	Proj	10 plf	10 plf	
Bot	20-5-4	24-6-0	Down	Proj	10 plf	10 plf	

User-defined Load Case S2: Snow Drift

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	9-0-0	24-6-0	Down	Proj	32 psf	32 psf	24 in

Point Loads

Member	Location	Direction	Load	Trib Width

User-defined Load Case W12: W1(DOL = 1.60)

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	1-0-12	7-6-0	N Up	Rake	24.87 plf	24.87 plf	
Top	1-0-12	7-6-0	N Up	Rake	25.39 plf	25.39 plf	
Top	7-6-0	7-10-8	N Up	Rake	24.87 plf	24.87 plf	
Top	7-6-0	7-10-8	N Up	Rake	25.39 plf	25.39 plf	
Top	9-10-8	11-5-4	N Up	Rake	24.87 plf	24.87 plf	
Top	9-10-8	11-5-4	N Up	Rake	25.39 plf	25.39 plf	
Top	13-5-4	15-0-0	N Up	Rake	24.87 plf	24.87 plf	
Top	13-5-4	15-0-0	N Up	Rake	25.39 plf	25.39 plf	
Top	15-0-0	15-5-4	N Up	Rake	15.63 plf	15.63 plf	
Top	15-0-0	15-5-4	N Up	Rake	15.97 plf	15.97 plf	
Top	17-5-4	19-5-4	N Up	Rake	15.63 plf	15.63 plf	
Top	17-5-4	19-5-4	N Up	Rake	15.97 plf	15.97 plf	
Top	21-5-4	23-5-4	N Up	Rake	15.63 plf	15.63 plf	
Top	21-5-4	23-5-4	N Up	Rake	15.97 plf	15.97 plf	

Point Loads

Member	Location	Direction	Load	Trib Width
Bot	8-10-8	Down	48 lbs	
Bot	8-10-8	Up	139 lbs	
Bot	12-5-4	Down	42 lbs	
Bot	12-5-4	Up	162 lbs	
Bot	16-5-4	Down	50 lbs	
Bot	16-5-4	Up	169 lbs	
Bot	20-5-4	Down	57 lbs	
Bot	20-5-4	Up	176 lbs	
Bot	24-5-4	Down	90 lbs	
Bot	24-5-4	Down	86 lbs	
Top	0-0-0	Right	600 lbs	

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
24-6-0	0.25 /12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	23.75 in	159 lbs

User-defined Load Case W13: W2(DOL = 1.60)

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	1-0-12	7-10-8	N Up	Rake	15.63 plf	15.63 plf	
Top	1-0-12	7-10-8	N Up	Rake	15.97 plf	15.97 plf	
Top	9-10-8	11-5-4	N Up	Rake	24.87 plf	24.87 plf	
Top	9-10-8	11-5-4	N Up	Rake	25.39 plf	25.39 plf	
Top	13-5-4	15-5-4	N Up	Rake	24.87 plf	24.87 plf	
Top	13-5-4	15-5-4	N Up	Rake	25.39 plf	25.39 plf	
Top	17-5-4	19-5-4	N Up	Rake	24.87 plf	24.87 plf	
Top	17-5-4	19-5-4	N Up	Rake	25.39 plf	25.39 plf	
Top	21-5-4	23-5-4	N Up	Rake	24.87 plf	24.87 plf	
Top	21-5-4	23-5-4	N Up	Rake	25.39 plf	25.39 plf	

Point Loads

Member	Location	Direction	Load	Trib Width
Bot	8-10-8	Up	141 lbs	
Bot	8-10-8	Down	44 lbs	
Bot	12-5-4	Up	163 lbs	
Bot	12-5-4	Down	38 lbs	
Bot	16-5-4	Up	170 lbs	
Bot	16-5-4	Down	46 lbs	
Bot	20-5-4	Up	178 lbs	
Bot	20-5-4	Down	53 lbs	
Bot	24-5-4	Up	161 lbs	
Bot	24-5-4	Up	159 lbs	
Top	0-0-0	Left	600 lbs	

User-defined Load Case W14: W4(DOL = 1.60)

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	1-0-12	3-0-0	N Up	Rake	47.87 plf	47.87 plf	
Top	1-0-12	3-0-0	N Up	Rake	48.89 plf	48.89 plf	
Top	3-0-0	7-10-8	N Up	Rake	42.83 plf	42.83 plf	
Top	3-0-0	7-10-8	N Up	Rake	43.75 plf	43.75 plf	
Top	9-10-8	11-5-4	N Up	Rake	42.83 plf	42.83 plf	
Top	9-10-8	11-5-4	N Up	Rake	43.75 plf	43.75 plf	
Top	13-5-4	15-5-4	N Up	Rake	42.83 plf	42.83 plf	
Top	13-5-4	15-5-4	N Up	Rake	43.75 plf	43.75 plf	
Top	17-5-4	19-5-4	N Up	Rake	42.83 plf	42.83 plf	
Top	17-5-4	19-5-4	N Up	Rake	43.75 plf	43.75 plf	
Top	21-5-4	23-5-4	N Up	Rake	42.83 plf	42.83 plf	
Top	21-5-4	23-5-4	N Up	Rake	43.75 plf	43.75 plf	

Point Loads

Member	Location	Direction	Load	Trib Width
Top	0-0-0	Left	600 lbs	

Load Combinations

#	Load Combo	Factor
1	D1	1.000
2	D1 + Lr1	1.000
3	D1 + S1	1.000
4	D1 + S2	1.000
5	0.60 D1 + 0.60 W1 [Uplift]	1.000
6	0.60 D1 + 0.60 W2 [Uplift]	1.000
7	0.60 D1 + 0.60 W4 [Uplift]	1.000
8	0.60 D1 + 0.60 W8 [Uplift]	1.000
9	0.60 D1 + 0.60 W0 [Uplift]	1.000
10	0.60 D1 + 0.60 W0 [Uplift]	1.000
11	0.60 D1 + 0.60 W0 [Uplift]	1.000
12	D1 + L10*1	1.000
13	D1 + Lr1 + L10*1	1.000
14	D1 + S1 + L10*1	1.000
15	D1 + S2 + L10*1	1.000
16	D1 + UR1	1.000
17	D1 + UR2	1.000
18	D1 + AS10*1	1.000
19	D1 + Lr1 + AS10*1	1.000
20	D1 + S1 + AS10*1	1.000
21	D1 + S2 + AS10*1	1.000

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
24-6-0	0.25 /12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	23.75 in	159 lbs

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.339	-3,026 lbs		3-5	0.531	-5,093 lbs		6-7	0.282	-2,442 lbs				
	2-3	0.548	-5,135 lbs		5-6	0.437	-4,144 lbs		7-8	0.242	1,053 lbs	(-495 lbs)			
BC	9-10	0.204	2,438 lbs	(-242 lbs)	11-12	0.414	5,089 lbs		14-15	0.310	3,024 lbs	(-181 lbs)			
	10-11	0.333	4,139 lbs		12-14	0.441	5,133 lbs		15-16	0.062	517 lbs	(-376 lbs)			
Web	1-16	0.183	-1,453 lbs		5-12	0.084	341 lbs		7-10	0.279	1,138 lbs				
	1-15	0.810	3,301 lbs	(-140 lbs)	5-11	0.371	-1,077 lbs		7-9	0.919	-2,843 lbs				
	2-15	0.143	-1,119 lbs		6-11	0.169	688 lbs								
	2-14	0.566	2,306 lbs		6-10	0.680	-1,945 lbs								

Truss to Truss Connection Summary

Carried Truss	Carrying Chord	Carrying Offset
BP6	BC	8-10-8
BP7	BC	8-10-8
BP23	BC	12-5-4
BP24	BC	12-5-4
BP21	BC	16-5-4
BP22	BC	16-5-4
BP19	BC	20-5-4
BP20	BC	20-5-4
DT9	BC	24-5-4
DT10	BC	24-5-4

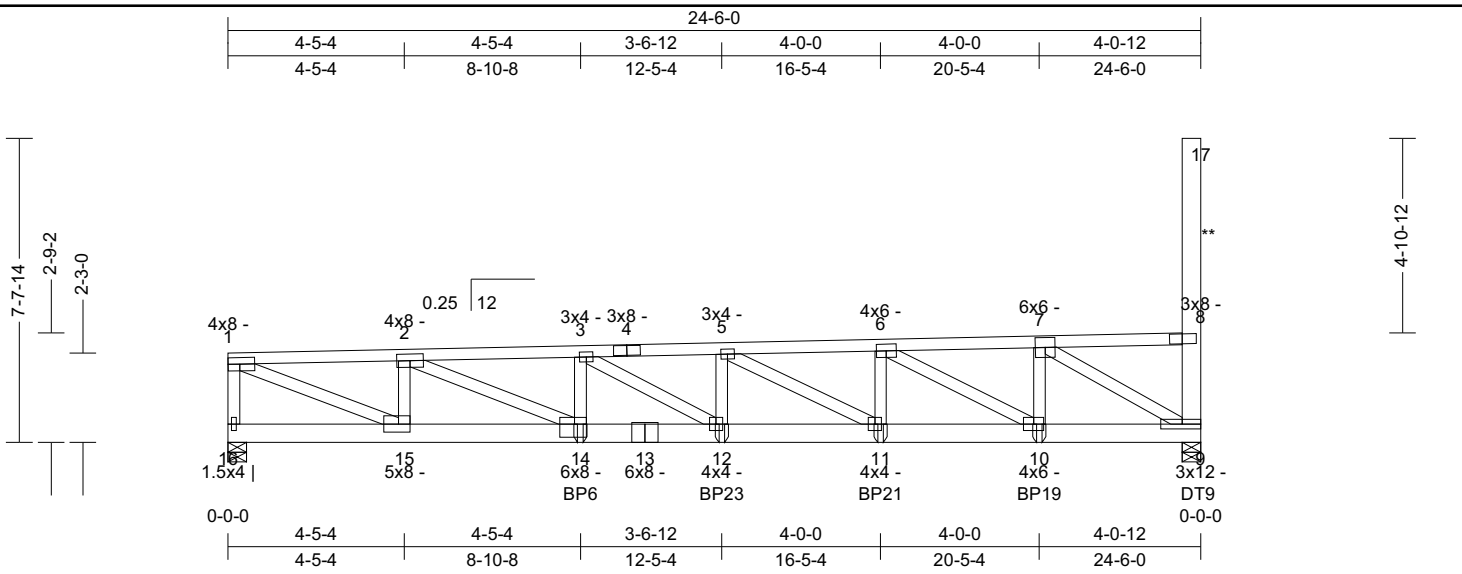
Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 30 % (Cq = 0.70).
3) Provide adequate drainage to prevent ponding.
4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
7) Listed wind uplift reactions based on MWFRS & C&C loading.
8) Parapet TL: 0.64 in, 2L/189 (17-8), Allowable 2L/120.

Vivco Components LLC.
2550 Hwy 33 South
P.O. Box 260
Maysville, Missouri 64469

Truss:GR8
Job: 500 NW Chipman Rd REVISED
Date: 02/16/22 10:56:55

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
24-6-0	0.25 /12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	20.75 in	158 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection		L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code : IBC 2018/	TC : 0.49 (3-5)	Vert TL:	0.39 in	L / 719	(12-13)	L / 240
TCLL : 20	TPI 1-2014	BC : 0.39 (11-12)	Vert LL:	0.2 in	L / 999	(12-13)	L / 360
TCDL : 15	Rep Mbr : No	Web : 0.90 (7-9)	Horz TL:	0.05 in		9	
BCLL : 0	Lumber D.O.L. : 115 %						
BCDL : 10							

02/16/2022

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
16	1	5.5 in	1.50 in	1,328 lbs	·	-116 lbs	-202 lbs	-202 lbs	526 lbs
9	1	5.5 in	1.73 in	1,708 lbs	·	·	-108 lbs	-108 lbs	·

Material

TC: SYP #1 2 x 4
BC: SYP SSDense 2 x 6
Web: SYP #2 2 x 4 except:
SYP SSDense 2 x 6: 17-9

Bracing

TC: Sheathed or Purlins at 2-5-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects due to 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has not been designed for the effects of unbalanced snow loads.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
5) Minimum storage attic loading has been applied in accordance with IBC 1607.1
6) ** - Indicates parapet wind loading has been applied to member 17-8

Load Case Lr1: Std Live Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	24-6-0	Down	Proj	15 plf	15 plf	
Top	0-0-0	7-10-8	Down	Proj	19.58 plf	19.58 plf	
Top	9-10-8	11-5-4	Down	Proj	19.58 plf	19.58 plf	
Top	13-5-4	15-5-4	Down	Proj	19.58 plf	19.58 plf	
Top	17-5-4	19-5-4	Down	Proj	19.58 plf	19.58 plf	
Top	21-5-4	24-6-0	Down	Proj	19.58 plf	19.58 plf	

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
24-6-0	0.25 /12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	20.75 in	158 lbs

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	24-6-0	Down	Proj	11.25 plf	11.25 plf	
Bot	0-0-0	24-6-0	Down	Proj	7.5 plf	7.5 plf	
Top	0-0-0	7-10-8	Down	Proj	14.69 plf	14.69 plf	
Top	9-10-8	11-5-4	Down	Proj	14.69 plf	14.69 plf	
Top	13-5-4	15-5-4	Down	Proj	14.69 plf	14.69 plf	
Top	17-5-4	19-5-4	Down	Proj	14.69 plf	14.69 plf	
Top	21-5-4	24-6-0	Down	Proj	14.69 plf	14.69 plf	
Bot	0-0-0	7-10-8	Down	Proj	9.79 plf	9.79 plf	
Bot	9-10-8	11-5-4	Down	Proj	9.79 plf	9.79 plf	
Bot	13-5-4	15-5-4	Down	Proj	9.79 plf	9.79 plf	
Bot	17-5-4	19-5-4	Down	Proj	9.79 plf	9.79 plf	
Bot	21-5-4	24-6-0	Down	Proj	9.79 plf	9.79 plf	

User-defined Load Case S2: Snow Drift

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	9-0-0	24-6-0	Down	Proj	41 psf	41 psf	24 in

Point Loads

Member	Location	Direction	Load	Trib Width

User-defined Load Case W12: W1 (DOL = 1.60)

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	1-0-12	N Up	Rake	25.39 plf	25.39 plf	
Top	1-0-12	7-6-0	N Up	Rake	25.39 plf	25.39 plf	
Top	1-0-12	7-6-0	N Up	Rake	24.87 plf	24.87 plf	
Top	7-6-0	7-10-8	N Up	Rake	25.39 plf	25.39 plf	
Top	7-6-0	7-10-8	N Up	Rake	24.87 plf	24.87 plf	
Top	7-10-8	9-10-8	N Up	Rake	25.39 plf	25.39 plf	
Top	9-10-8	11-5-4	N Up	Rake	25.39 plf	25.39 plf	
Top	9-10-8	11-5-4	N Up	Rake	24.87 plf	24.87 plf	
Top	11-5-4	13-5-4	N Up	Rake	25.39 plf	25.39 plf	
Top	13-5-4	15-0-0	N Up	Rake	25.39 plf	25.39 plf	
Top	13-5-4	15-0-0	N Up	Rake	24.87 plf	24.87 plf	
Top	15-0-0	15-5-4	N Up	Rake	15.97 plf	15.97 plf	
Top	15-0-0	15-5-4	N Up	Rake	15.63 plf	15.63 plf	
Top	15-5-4	17-5-4	N Up	Rake	15.97 plf	15.97 plf	
Top	17-5-4	19-5-4	N Up	Rake	15.97 plf	15.97 plf	
Top	17-5-4	19-5-4	N Up	Rake	15.63 plf	15.63 plf	
Top	19-5-4	21-5-4	N Up	Rake	15.97 plf	15.97 plf	
Top	21-5-4	23-5-4	N Up	Rake	15.97 plf	15.97 plf	
Top	21-5-4	23-5-4	N Up	Rake	15.63 plf	15.63 plf	
Top	23-5-4	24-6-0	N Up	Rake	15.97 plf	15.97 plf	
Web 1-16	0-0-0	2-3-0	Right	Rake	12.74 plf	12.74 plf	
Web 17-8	0-0-0	4-10-12	Right	Rake	25.93 plf	25.93 plf	
Web 8-9	0-0-0	2-9-2	Right	Rake	15.11 plf	15.11 plf	

Point Loads

Member	Location	Direction	Load	Trib Width
Bot	8-10-8	Up	141 lbs	
Bot	12-5-4	Up	163 lbs	
Bot	16-5-4	Up	170 lbs	
Bot	20-5-4	Up	178 lbs	
Bot	24-5-4	Down	90 lbs	
Top	0-0-0	Right	600 lbs	

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
24-6-0	0.25 /12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	20.75 in	158 lbs

User-defined Load Case W13: W2(DOL = 1.60)

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	1-0-12	N Up	Rake	15.97 plf	15.97 plf	
Top	1-0-12	7-10-8	N Up	Rake	15.97 plf	15.97 plf	
Top	1-0-12	7-10-8	N Up	Rake	15.63 plf	15.63 plf	
Top	7-10-8	9-6-0	N Up	Rake	15.97 plf	15.97 plf	
Top	9-6-0	9-10-8	N Up	Rake	25.39 plf	25.39 plf	
Top	9-10-8	11-5-4	N Up	Rake	25.39 plf	25.39 plf	
Top	9-10-8	11-5-4	N Up	Rake	24.87 plf	24.87 plf	
Top	11-5-4	13-5-4	N Up	Rake	25.39 plf	25.39 plf	
Top	13-5-4	15-5-4	N Up	Rake	25.39 plf	25.39 plf	
Top	13-5-4	15-5-4	N Up	Rake	24.87 plf	24.87 plf	
Top	15-5-4	17-0-0	N Up	Rake	25.39 plf	25.39 plf	
Top	17-0-0	17-5-4	N Up	Rake	25.39 plf	25.39 plf	
Top	17-5-4	19-5-4	N Up	Rake	25.39 plf	25.39 plf	
Top	17-5-4	19-5-4	N Up	Rake	24.87 plf	24.87 plf	
Top	19-5-4	21-5-4	N Up	Rake	25.39 plf	25.39 plf	
Top	21-5-4	23-5-4	N Up	Rake	25.39 plf	25.39 plf	
Top	21-5-4	23-5-4	N Up	Rake	24.87 plf	24.87 plf	
Top	23-5-4	24-6-0	N Up	Rake	25.39 plf	25.39 plf	
Web 1-16	0-0-0	2-3-0	Left	Rake	15.11 plf	15.11 plf	
Web 17-8	0-0-0	4-10-12	Left	Rake	38.89 plf	38.89 plf	
Web 8-9	0-0-0	2-9-2	Left	Rake	12.74 plf	12.74 plf	

Point Loads

Member	Location	Direction	Load	Trib Width
Bot	8-10-8	Down	48 lbs	
Bot	12-5-4	Down	42 lbs	
Bot	16-5-4	Down	50 lbs	
Bot	20-5-4	Down	57 lbs	
Bot	24-5-4	Up	161 lbs	
Top	0-0-0	Left	600 lbs	

User-defined Load Case W14: W4(DOL = 1.60)

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	1-0-12	N Up	Rake	48.89 plf	48.89 plf	
Top	1-0-12	3-0-0	N Up	Rake	48.89 plf	48.89 plf	
Top	1-0-12	3-0-0	N Up	Rake	47.87 plf	47.87 plf	
Top	3-0-0	7-10-8	N Up	Rake	43.75 plf	43.75 plf	
Top	3-0-0	7-10-8	N Up	Rake	42.83 plf	42.83 plf	
Top	7-10-8	9-10-8	N Up	Rake	43.75 plf	43.75 plf	
Top	9-10-8	11-5-4	N Up	Rake	43.75 plf	43.75 plf	
Top	9-10-8	11-5-4	N Up	Rake	42.83 plf	42.83 plf	
Top	11-5-4	13-5-4	N Up	Rake	43.75 plf	43.75 plf	
Top	13-5-4	15-5-4	N Up	Rake	43.75 plf	43.75 plf	
Top	13-5-4	15-5-4	N Up	Rake	42.83 plf	42.83 plf	
Top	15-5-4	17-5-4	N Up	Rake	43.75 plf	43.75 plf	
Top	17-5-4	19-5-4	N Up	Rake	43.75 plf	43.75 plf	
Top	17-5-4	19-5-4	N Up	Rake	42.83 plf	42.83 plf	
Top	19-5-4	21-5-4	N Up	Rake	43.75 plf	43.75 plf	
Top	21-5-4	23-5-4	N Up	Rake	43.75 plf	43.75 plf	
Top	21-5-4	23-5-4	N Up	Rake	42.83 plf	42.83 plf	
Top	23-5-4	24-6-0	N Up	Rake	43.75 plf	43.75 plf	
Web 1-16	0-0-0	2-3-0	Left	Rake	27.37 plf	27.37 plf	
Web 17-8	0-0-0	4-10-12	Right	Rake	82.96 plf	82.96 plf	
Web 8-9	0-0-0	2-9-2	Right	Rake	27.37 plf	27.37 plf	

Point Loads

Member	Location	Direction	Load	Trib Width
Top	0-0-0	Left	600 lbs	

Load Combinations

#	Load Combo	Factor
1	D1	1.000
2	D1 + Lr1	1.000
3	D1 + S1	1.000
4	D1 + S2	1.000
5	0.60 D1 + 0.60 W1 [Uplift]	1.000
6	0.60 D1 + 0.60 W2 [Uplift]	1.000
7	0.60 D1 + 0.60 W4 [Uplift]	1.000
8	0.60 D1 + 0.60 W8 [Uplift]	1.000

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
24-6-0	0.25 /12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	20.75 in	158 lbs

9	0.60 D1 + 0.60 W0 [Uplift]	1.000
10	0.60 D1 + 0.60 W0 [Uplift]	1.000
11	0.60 D1 + 0.60 W0 [Uplift]	1.000
12	D1 + L10*1	1.000
13	D1 + Lr1 + L10*1	1.000
14	D1 + S1 + L10*1	1.000
15	D1 + S2 + L10*1	1.000
16	D1 + UR1	1.000
17	D1 + UR2	1.000
18	D1 + AS10*1	1.000
19	D1 + Lr1 + AS10*1	1.000
20	D1 + S1 + AS10*1	1.000
21	D1 + S2 + AS10*1	1.000

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.286	-2,602 lbs		3-5	0.490	-4,695 lbs		6-7	0.324	-2,389 lbs	
	2-3	0.473	-4,424 lbs		5-6	0.420	-3,982 lbs		7-8	0.276	920 lbs	(-433 lbs)
BC	9-10	0.197	2,385 lbs	(-420 lbs)	11-12	0.386	4,691 lbs	(-521 lbs)	14-15	0.262	2,601 lbs	(-419 lbs)
	10-11	0.323	3,977 lbs	(-518 lbs)	12-14	0.378	4,422 lbs	(-449 lbs)	15-16	0.053	-516 lbs	
Web	1-16	0.157	-1,246 lbs		3-14	0.046	-352 lbs		6-10	0.637	-1,821 lbs	
	1-15	0.697	2,839 lbs	(-363 lbs)	3-12	0.076	310 lbs	(-218 lbs)	7-10	0.252	1,028 lbs	(-81 lbs)
	2-15	0.123	-964 lbs		5-11	0.278	-809 lbs		7-9	0.899	-2,781 lbs	
	2-14	0.489	1,993 lbs	(-243 lbs)	6-11	0.128	520 lbs	(-19 lbs)				

Truss to Truss Connection Summary

Carried Truss	Carrying Chord	Carrying Offset
BP6	BC	8-10-8
BP23	BC	12-5-4
BP21	BC	16-5-4
BP19	BC	20-5-4
DT9	BC	24-5-4

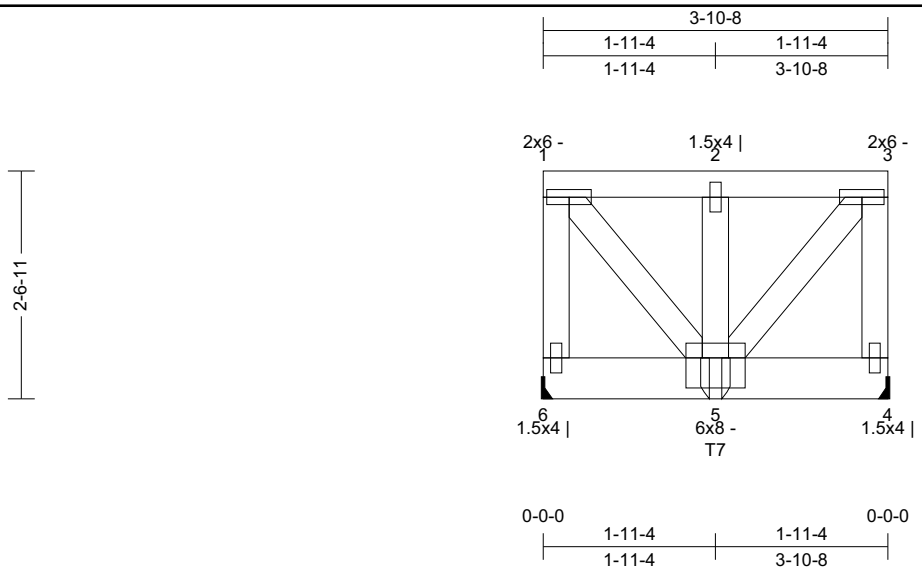
Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 30 % (Cq = 0.70).
3) Provide adequate drainage to prevent ponding.
4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
7) Listed wind uplift reactions based on MWFRS & C&C loading.
8) Parapet TL: 0.58 in, 2L/210 (17-8), Allowable 2L/120.

Vivco Components LLC.
2550 Hwy 33 South
P.O. Box 260
Maysville, Missouri 64469

Truss:GR9
Job: 500 NW Chipman Rd REVISED
Date: 02/16/22 10:56:57

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
3-10-8	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	15.75 in	29 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code: IBC 2018/ TPI 1-2014	TC: 0.03 (1-2)	Vert TL: 0 in	L/999	5	L/240
TCLL: 20		BC: 0.03 (5-6)	Vert LL: 0 in	L/999	(4-5)	L/360
TCDL: 15	Rep Mbr: No	Web: 0.09 (1-5)	Horz TL: 0 in		4	
BCLL: 0	Lumber D.O.L.: 115 %					
BCDL: 10						

02/16/2022

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
6	1	1.5 in	---	391 lbs	·	-15 lbs	·	-15 lbs	56 lbs
4	1	1.5 in	---	391 lbs	·	-49 lbs	·	-49 lbs	·

Material

TC: SYP #1 2 x 4
BC: SYP SSDense 2 x 6
Web: SYP #2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects due to 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has not been designed for the effects of unbalanced snow loads.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
5) Minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case Lr1: Std Live Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	3-10-8	Down	Proj	26.25 plf	26.25 plf	

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	3-10-8	Down	Proj	19.69 plf	19.69 plf	
Bot	0-0-0	3-10-8	Down	Proj	13.13 plf	13.13 plf	

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC											
BC											
Web	1-6	0.048	-331 lbs		3-5	0.090	369 lbs	(-58 lbs)			
	1-5	0.090	369 lbs	(-14 lbs)	3-4	0.048	-331 lbs				

Truss to Truss Connection Summary

Carried Truss	Carrying Chord	Carrying Offset
T7	BC	1-11-4

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
3-10-8	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	15.75 in	29 lbs

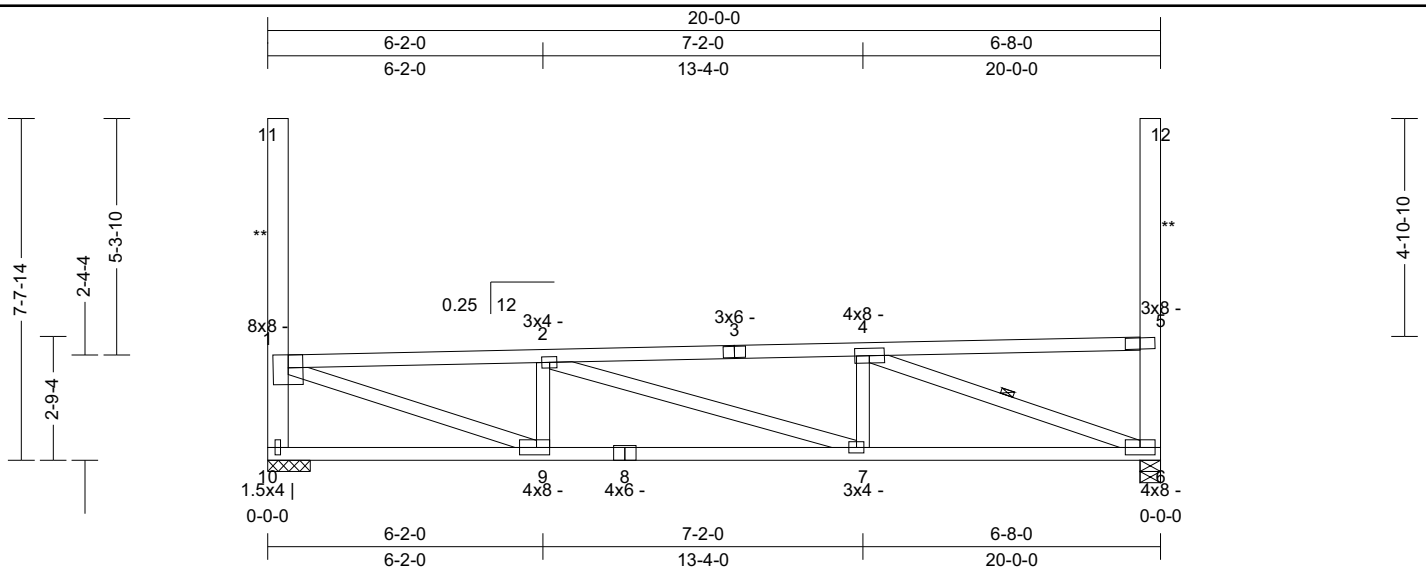
Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 30 % (Cq = 0.70).
3) Hangers are for graphical intrepretation only. Install hangers per manufacturer's recommendations.
4) Provide adequate drainage to prevent ponding.
5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
7) Listed wind uplift reactions based on MWFRS & C&C loading.

Vivco Components LLC.
2550 Hwy 33 South
P.O. Box 260
Maysville, Missouri 64469

Truss:T1
Job: 500 NW Chipman Rd REVISED
Date: 02/16/22 10:56:59

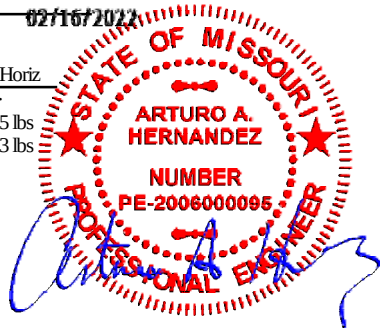
SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
20-0-0	0.25 /12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	122 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 20	Bldg Code : IBC 2018/	TC : 0.91 (4-5)	Vert TL: 0.34 in	L / 657	(7-8)	L / 240
TCDL : 15	TPI 1-2014	BC : 0.82 (6-7)	Vert LL: 0.12 in UP	L / 999	(7-8)	L / 360
BCLL : 0	Rep Mbr : No	Web : 0.93 (11-1)	Horz TL: 0.05 in		6	
BCDL : 10	Lumber D.O.L. : 115 %					

02/16/2022



Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
6	1	5.5 in	1.63 in	1,383 lbs	·	-70 lbs	-219 lbs	-219 lbs	·
10	1	11.5 in	N/A	990 lbs	·	-160 lbs	-236 lbs	-236 lbs	-675 lbs
10	1	11.5 in	N/A	378 lbs	·	·	-30 lbs	-30 lbs	-793 lbs

Material

TC: SYP #1 2 x 4
BC: SYP #1 2 x 4
Web: SYP #2 2 x 4 except:
SYP SSDense 2 x 6: 11-10, 12-6

Bracing

TC: Sheathed or Purlins at 2-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 5-9-0, Purlin design by Others.
Web: One Midpoint Row: 4-6

Loads

1) This truss has been designed for the effects due to 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has not been designed for the effects of unbalanced snow loads.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
5) Minimum storage attic loading has been applied in accordance with IBC 1607.1
6) ** - Indicates parapet wind loading has been applied to members 11-1 & 12-5

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	14-4-0	16-6-0	Down	Proj	35.64 plf	35.64 plf	

User-defined Load Case S2: Drift

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	20-0-0	Down	Proj	20 psf	20 psf	24 in
Top	0-0-0	20-0-0	Down	Proj	5 psf	5 psf	24 in
Top	0-0-0	4-6-0	Down	Proj	32 psf	0 psf	24 in
Top	15-6-0	20-0-0	Down	Proj	0 psf	32 psf	24 in

Point Loads

Member	Location	Direction	Load	Trib Width

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
20-0-0	0.25/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	122 lbs

Load Combinations

#	Load Combo	Factor
1	D1	1.000
2	D1 + Lr1	1.000
3	D1 + S1	1.000
4	D1 + S2	1.000
5	0.60 D1 + 0.60 W1 [Uplift]	1.000
6	0.60 D1 + 0.60 W2 [Uplift]	1.000
7	0.60 D1 + 0.60 W4 [Uplift]	1.000
8	0.60 D1 + 0.60 W8 [Uplift]	1.000
9	D1 + L10*1	1.000
10	D1 + Lr1 + L10*1	1.000
11	D1 + S1 + L10*1	1.000
12	D1 + S2 + L10*1	1.000
13	D1 + UR1	1.000
14	D1 + UR2	1.000
15	D1 + AS10*1	1.000
16	D1 + Lr1 + AS10*1	1.000
17	D1 + S1 + AS10*1	1.000
18	D1 + S2 + AS10*1	1.000

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.895	-2,485 lbs		2-4	0.820	-2,601 lbs		4-5	0.913	1,047 lbs	(-494 lbs)
BC	6-7	0.822	2,595 lbs	(-979 lbs)	7-9	0.803	2,479 lbs	(-1,131 lbs)	9-10	0.400	793 lbs	(-726 lbs)
Web	1-10	0.638	-1,206 lbs		2-7	0.244	362 lbs	(-253 lbs)				
	1-9	0.644	2,624 lbs	(-429 lbs)	4-6	0.630	-2,761 lbs					
	2-9	0.088	-649 lbs		5-6	0.567	-328 lbs					

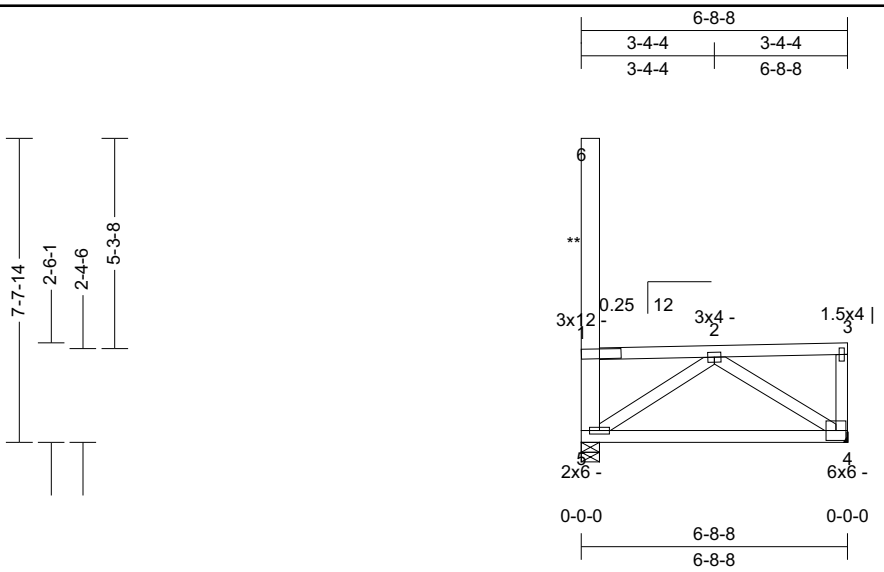
Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 30 % (Cq = 0.70).
3) Provide adequate drainage to prevent ponding.
4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
7) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
8) ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
9) Listed wind uplift reactions based on MWFRS & C&C loading.
10) Parapet TL: 0.86 in, 2L/151 (11-1), Allowable 2L/120.

Vivco Components LLC.
2550 Hwy 33 South
P.O. Box 260
Maysville, Missouri 64469

Truss:T2
Job: 500 NW Chipman Rd REVISED
Date: 02/16/22 10:57:00

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
6-8-8	0.25/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	48 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.27 (1-2)	Vert TL: 0.17 in	L/427	(4-5)	L/240
TCDL: 15	TPI 1-2014	BC: 0.50 (4-5)	Vert LL: 0.09 in	L/855	(4-5)	L/360
BCLL: 0	Rep Mbr: No	Web: 0.92 (6-1)	Horz TL: 0.01 in		4	
BCDL: 10	Lumber D.O.L.: 115 %					

02/16/2022

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
5	1	5.5 in	1.50 in	471 lbs	·	-203 lbs	·	-203 lbs	534 lbs
4	1	1.5 in	—	403 lbs	·	-145 lbs	-557 lbs	-557 lbs	·

Material

TC: SYP #1 2 x 4
BC: SYP #1 2 x 4
Web: SYP #2 2 x 4 except:
SYP SSDense 2 x 6: 6-5

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 7-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects due to 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has not been designed for the effects of unbalanced snow loads.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
5) Minimum storage attic loading has been applied in accordance with IBC 1607.1
6) ** - Indicates parapet wind loading has been applied to member 6-1

User-defined Load Case S2: Drift

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	6-8-8	Down	Proj	20 psf	20 psf	24 in
Top	0-0-0	4-6-0	Down	Proj	32 psf	0 psf	24 in

Point Loads

Member	Location	Direction	Load	Trib Width

Load Combinations

#	Load Combo	Factor
1	D1	1.000
2	D1 + Lr1	1.000
3	D1 + S1	1.000
4	D1 + S2	1.000
5	0.60 D1 + 0.60 W1 [Uplift]	1.000
6	0.60 D1 + 0.60 W2 [Uplift]	1.000

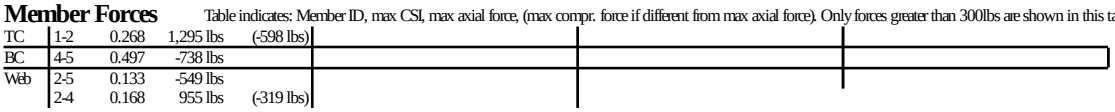
SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
6-8-8	0.25 /12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	48 lbs

7	0.60 D1 + 0.60 W4 [Uplift]	1.000
8	0.60 D1 + 0.60 W8 [Uplift]	1.000
9	D1 + L10*1	1.000
10	D1 + Lr1 + L10*1	1.000
11	D1 + S1 + L10*1	1.000
12	D1 + S2 + L10*1	1.000
13	D1 + UR1	1.000
14	D1 + UR2	1.000
15	D1 + AS10*1	1.000
16	D1 + Lr1 + AS10*1	1.000
17	D1 + S1 + AS10*1	1.000
18	D1 + S2 + AS10*1	1.000

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.268	1,295 lbs	(-598 lbs)
BC	4-5	0.497	-738 lbs	
Web	2-5	0.133	-549 lbs	
	2-4	0.168	955 lbs	(-319 lbs)



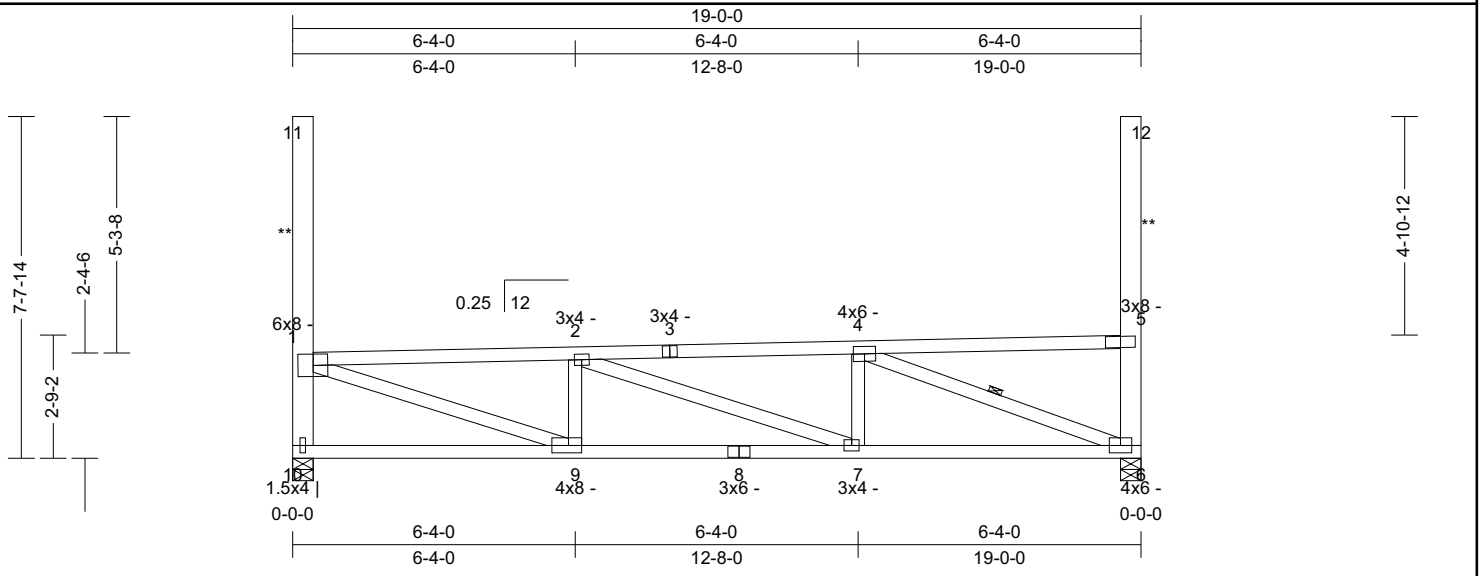
Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 30 % (Cq = 0.70).
3) Hanger is for graphical intrepretation only. Install hanger per manufacturer's recommendation.
4) Provide adequate drainage to prevent ponding.
5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
6) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
7) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
8) Listed wind uplift reactions based on MWFRS & C&C loading.
9) Parapet TL: 0.82 in, 2L/160 (6-1), Allowable 2L/120.

Vivco Components LLC.
2550 Hwy 33 South
P.O. Box 260
Maysville, Missouri 64469

Truss:T3
Job: 500 NW Chipman Rd REVISED
Date: 02/16/22 10:57:02

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
19-0-0	0.25 /12	4	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	117 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 20	Bldg Code : IBC 2018/	TC : 0.78 (1-2)	Vert TL: 0.22 in	L / 999	(8-9)	L / 240
TCDL : 15	TPI 1-2014	BC : 0.59 (7-9)	Vert LL: 0.1 in UP	L / 999	(8-9)	L / 360
BCLL : 0	Rep Mbr : Yes	Web : 0.92 (11-1)	Horz TL: 0.04 in		6	
BCDL : 10	Lumber D.O.L. : 115 %					

02/16/2022

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
10	1	5.5 in	1.50 in	1,182 lbs	·	-110 lbs	-271 lbs	-271 lbs	491 lbs
6	1	5.5 in	1.50 in	1,182 lbs	·	-112 lbs	-250 lbs	-250 lbs	·

Material

TC: SYP #1 2 x 4
BC: SYP #1 2 x 4
Web: SYP #2 2 x 4 except:
SYP SSDense 2 x 6: 11-10, 12-6

Bracing

TC: Sheathed or Purlins at 2-10-0, Purlin design by Others.
BC: Sheathed or Purlins at 6-1-0, Purlin design by Others.
Web: One Midpoint Row: 4-6

Loads

1) This truss has been designed for the effects due to 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has not been designed for the effects of unbalanced snow loads.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
5) Minimum storage attic loading has been applied in accordance with IBC 1607.1
6) ** - Indicates parapet wind loading has been applied to members 11-1 & 12-5

User-defined Load Case S2: Drift

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	19-0-0	Down	Proj	20 psf	20 psf	24 in
Top	0-0-0	4-6-0	Down	Proj	32 psf	0 psf	24 in
Top	14-6-0	19-0-0	Down	Proj	0 psf	32 psf	24 in

Point Loads

Member	Location	Direction	Load	Trib Width

Load Combinations

#	Load Combo	Factor
1	D1	1.000
2	D1 + Lr1	1.000
3	D1 + S1	1.000
4	D1 + S2	1.000

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
19-0-0	0.25/12	4	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	117 lbs

5	0.60 D1 + 0.60 W1 [Uplift]	1.000
6	0.60 D1 + 0.60 W2 [Uplift]	1.000
7	0.60 D1 + 0.60 W4 [Uplift]	1.000
8	0.60 D1 + 0.60 W8 [Uplift]	1.000
9	D1 + L10*1	1.000
10	D1 + Lr1 + L10*1	1.000
11	D1 + S1 + L10*1	1.000
12	D1 + S2 + L10*1	1.000
13	D1 + UR1	1.000
14	D1 + UR2	1.000
15	D1 + AS10*1	1.000
16	D1 + Lr1 + AS10*1	1.000
17	D1 + S1 + AS10*1	1.000
18	D1 + S2 + AS10*1	1.000

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.777	-2,200 lbs		2-4	0.469	-2,081 lbs		4-5	0.606	1,052 lbs	(-496 lbs)
BC	6-7	0.568	2,075 lbs	(-1,009 lbs)	7-9	0.586	2,194 lbs	(-1,133 lbs)	9-10	0.268	791 lbs	(-716 lbs)
Web	1-10	0.636	-1,071 lbs		2-7	0.241	-312 lbs					
	1-9	0.569	2,317 lbs	(-441 lbs)	4-7	0.077	312 lbs	(-17 lbs)				
	2-9	0.067	-487 lbs		4-6	0.471	-2,222 lbs					

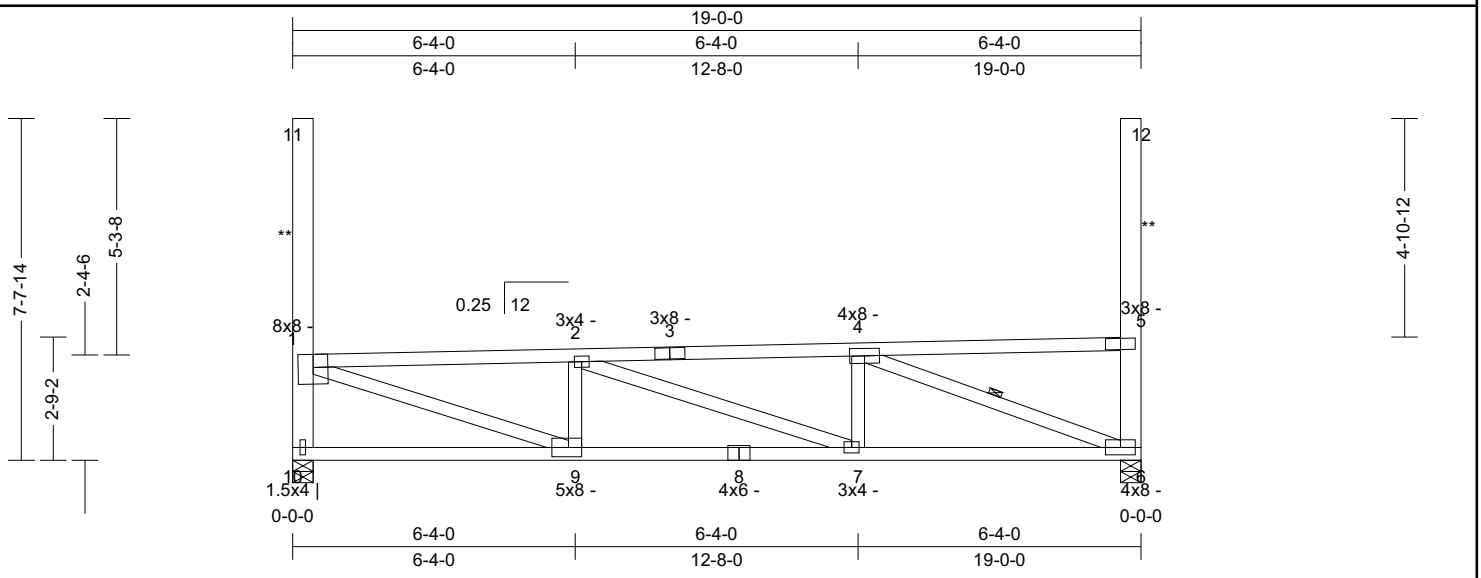
Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 30 % (Cq = 0.70).
3) Provide adequate drainage to prevent ponding.
4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
7) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
8) ⊠ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
9) Listed wind uplift reactions based on MWFRS & C&C loading.
10) Parapet TL: 0.86 in, 2L/152 (11-1), Allowable 2L/120.

Vivco Components LLC.
2550 Hwy 33 South
P.O. Box 260
Maysville, Missouri 64469

Truss:T4
Job: 500 NW Chipman Rd REVISED
Date: 02/16/22 10:57:03
Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
19-0-0	0.25 /12	2	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	119 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 20	Bldg Code : IBC 2018/	TC : 0.79 (1-2)	Vert TL: 0.26 in	L / 831	(8-9)	L / 240
TCDL : 15	TPI 1-2014	BC : 0.74 (7-9)	Vert LL: 0.1 in UP	L / 999	(8-9)	L / 360
BCLL : 0	Rep Mbr : No	Web : 0.92 (11-1)	Horz TL: 0.05 in		6	
BCDL : 10	Lumber D.O.L. : 115 %					

02/16/2022

STATE OF MISSOURI
ARTURO A. HERNANDEZ
NUMBER PE-2006000095
PROFESSIONAL ENGINEER

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
10	1	5.5 in	1.62 in	1,373 lbs	·	·	-156 lbs	-156 lbs	491 lbs
6	1	5.5 in	1.55 in	1,315 lbs	·	-32 lbs	-170 lbs	-170 lbs	·

Material

TC: SYP #1 2 x 4
BC: SYP #1 2 x 4
Web: SYP #2 2 x 4 except:
SYP SSDense 2 x 6: 11-10, 12-6

Bracing

TC: Sheathed or Purlins at 2-7-0, Purlin design by Others.
BC: Sheathed or Purlins at 6-11-0, Purlin design by Others.
Web: One Midpoint Row: 4-6

Loads

1) This truss has been designed for the effects due to 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has not been designed for the effects of unbalanced snow loads.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
5) Minimum storage attic loading has been applied in accordance with IBC 1607.1
6) ** - Indicates parapet wind loading has been applied to members 11-1 & 12-5

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	4-10-0	10-10-0	Down	Proj	54.17 plf	54.17 plf	

User-defined Load Case S2: Drift

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	19-0-0	Down	Proj	20 psf	20 psf	24 in
Top	0-0-0	4-6-0	Down	Proj	32 psf	0 psf	24 in
Top	14-6-0	19-0-0	Down	Proj	0 psf	32 psf	24 in

Point Loads

Member	Location	Direction	Load	Trib Width

TrueBuild® Truss Software v5.6.376
Eagle Metal Products

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
19-0-0	0.25 /12	2	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	119 lbs

Load Combinations

#	Load Combo	Factor
1	D1	1.000
2	D1 + Lr1	1.000
3	D1 + S1	1.000
4	D1 + S2	1.000
5	0.60 D1 + 0.60 W1 [Uplift]	1.000
6	0.60 D1 + 0.60 W2 [Uplift]	1.000
7	0.60 D1 + 0.60 W4 [Uplift]	1.000
8	0.60 D1 + 0.60 W8 [Uplift]	1.000
9	D1 + L10*1	1.000
10	D1 + Lr1 + L10*1	1.000
11	D1 + S1 + L10*1	1.000
12	D1 + S2 + L10*1	1.000
13	D1 + UR1	1.000
14	D1 + UR2	1.000
15	D1 + AS10*1	1.000
16	D1 + Lr1 + AS10*1	1.000
17	D1 + S1 + AS10*1	1.000
18	D1 + S2 + AS10*1	1.000

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.786	-2,749 lbs		2-4	0.739	-2,462 lbs		4-5	0.669	1,052 lbs	(-496 lbs)
BC	6-7	0.696	2,453 lbs	(-782 lbs)	7-9	0.743	2,743 lbs	(-804 lbs)	9-10	0.302	791 lbs	(-716 lbs)
Web	1-10	0.639	-1,261 lbs		2-7	0.381	-494 lbs					
	1-9	0.711	2,897 lbs	(-93 lbs)	4-7	0.091	372 lbs					
	2-9	0.093	-677 lbs		4-6	0.557	-2,627 lbs					

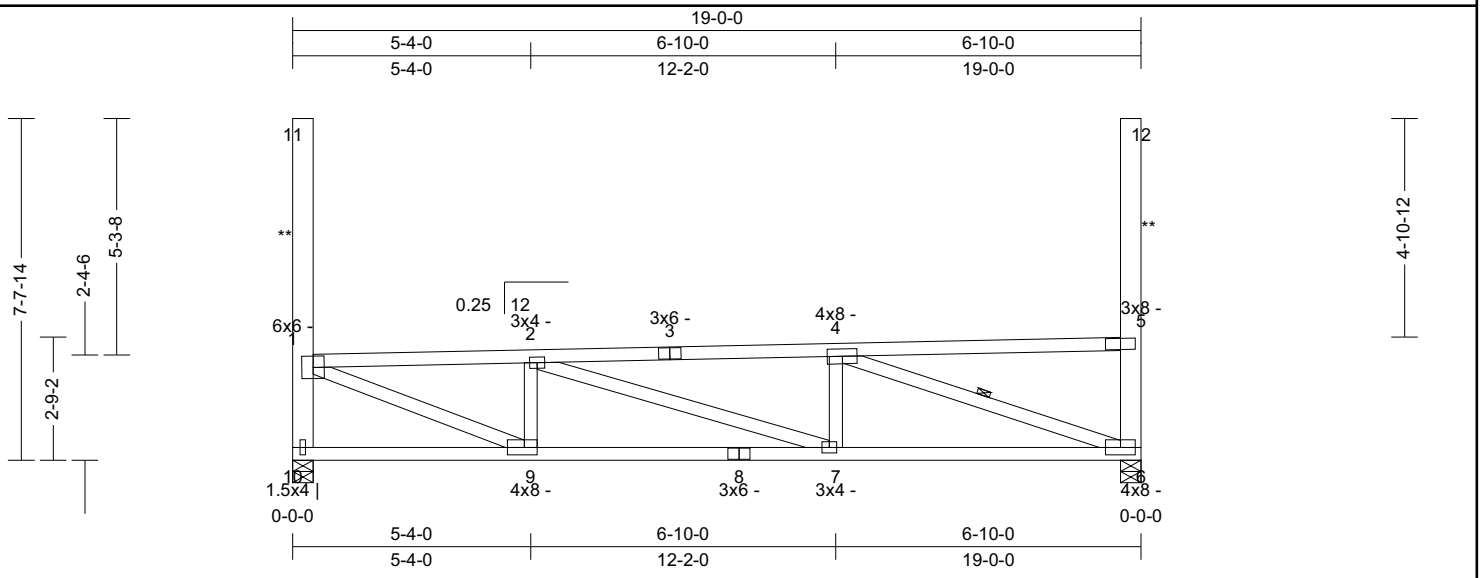
Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 30 % (Cq = 0.70).
3) Provide adequate drainage to prevent ponding.
4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
7) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
8) ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
9) Listed wind uplift reactions based on MWFRS & C&C loading.
10) Parapet TL: 0.84 in, 2L/154 (11-1), Allowable 2L/120.

Vivco Components LLC.
2550 Hwy 33 South
P.O. Box 260
Maysville, Missouri 64469

Truss:T5
Job: 500 NW Chipman Rd REVISED
Date: 02/16/22 10:57:05
Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
19-0-0	0.25 /12	2	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	118 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 20	Bldg Code : IBC 2018/	TC : 0.80 (1-2)	Vert TL: 0.24 in	L / 899	(8-9)	L / 240
TCDL : 15	TPI 1-2014	BC : 0.74 (6-7)	Vert LL: 0.1 in UP	L / 999	(8-9)	L / 360
BCLL : 0	Rep Mbr : No	Web : 0.92 (11-1)	Horz TL: 0.04 in		6	
BCDL : 10	Lumber D.O.L. : 115 %					

02/16/2022

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
10	1	5.5 in	1.50 in	1,274 lbs	·	-55 lbs	-216 lbs	-216 lbs	491 lbs
6	1	5.5 in	1.50 in	1,195 lbs	·	-104 lbs	-242 lbs	-242 lbs	·

Material

TC: SYP #1 2 x 4
BC: SYP #1 2 x 4
Web: SYP #2 2 x 4 except:
SYP SSDense 2 x 6: 11-10, 12-6

Bracing

TC: Sheathed or Purlins at 2-10-0, Purlin design by Others.
BC: Sheathed or Purlins at 6-0-0, Purlin design by Others.
Web: One Midpoint Row: 4-6

Loads

1) This truss has been designed for the effects due to 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has not been designed for the effects of unbalanced snow loads.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
5) Minimum storage attic loading has been applied in accordance with IBC 1607.1
6) ** - Indicates parapet wind loading has been applied to members 11-1 & 12-5

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	1-5-0	3-7-0	Down	Proj	48.46 plf	48.46 plf	

User-defined Load Case S2: Drift

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	19-0-0	Down	Proj	20 psf	20 psf	24 in
Top	0-0-0	4-6-0	Down	Proj	32 psf	0 psf	24 in
Top	14-6-0	19-0-0	Down	Proj	0 psf	32 psf	24 in

Point Loads

Member	Location	Direction	Load	Trib Width

TrueBuild® Truss Software v5.6.376
Eagle Metal Products

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
19-0-0	0.25 /12	2	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	118 lbs

Load Combinations

#	Load Combo	Factor
1	D1	1.000
2	D1 + Lr1	1.000
3	D1 + S1	1.000
4	D1 + S2	1.000
5	0.60 D1 + 0.60 W1 [Uplift]	1.000
6	0.60 D1 + 0.60 W2 [Uplift]	1.000
7	0.60 D1 + 0.60 W4 [Uplift]	1.000
8	0.60 D1 + 0.60 W8 [Uplift]	1.000
9	D1 + L10*1	1.000
10	D1 + Lr1 + L10*1	1.000
11	D1 + S1 + L10*1	1.000
12	D1 + S2 + L10*1	1.000
13	D1 + UR1	1.000
14	D1 + UR2	1.000
15	D1 + AS10*1	1.000
16	D1 + Lr1 + AS10*1	1.000
17	D1 + S1 + AS10*1	1.000
18	D1 + S2 + AS10*1	1.000

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.802	-2,107 lbs		2-4	0.644	-2,234 lbs		4-5	0.793	1,052 lbs	(-496 lbs)							
BC	6-7	0.736	2,229 lbs	(-1,024 lbs)	7-9	0.715	2,101 lbs	(-1,047 lbs)	9-10	0.271	791 lbs	(-716 lbs)							
Web	1-10	0.638	-1,184 lbs		2-7	0.326	-371 lbs												
	1-9	0.557	2,268 lbs	(-357 lbs)	4-6	0.559	-2,363 lbs												
	2-9	0.082	-602 lbs																

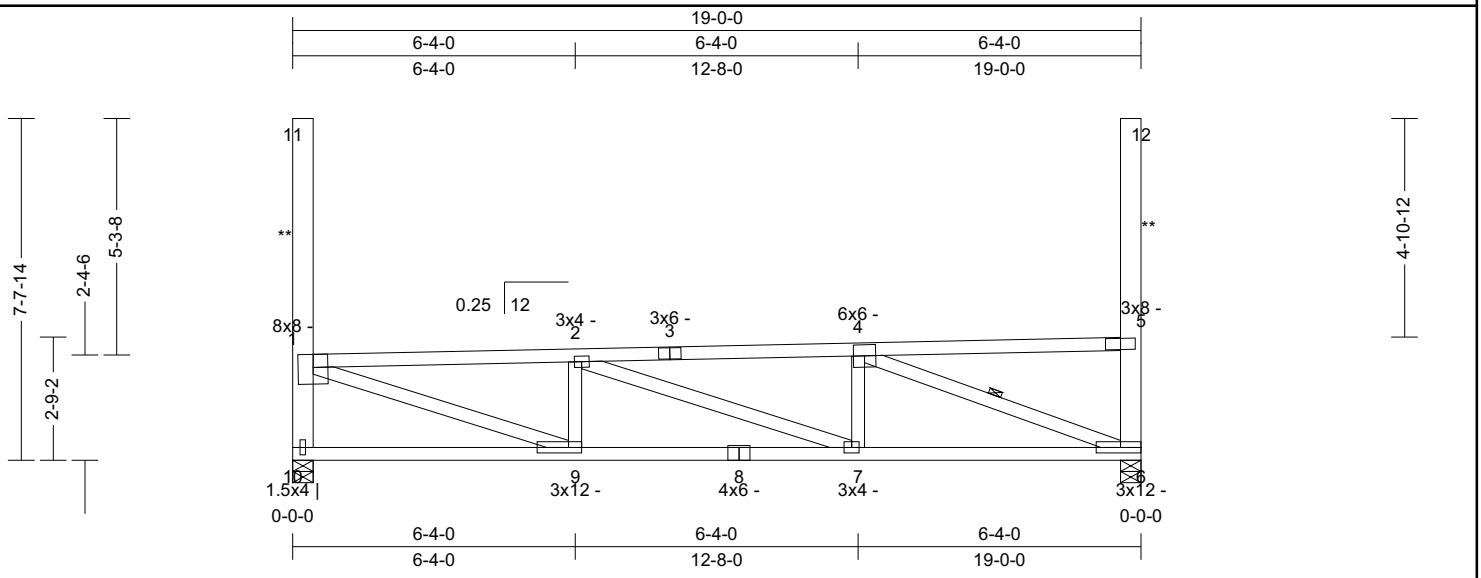
Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 30 % (Cq = 0.70).
3) Provide adequate drainage to prevent ponding.
4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
7) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
8) ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
9) Listed wind uplift reactions based on MWFRS & C&C loading.
10) Parapet TL: 0.86 in, 2L/152 (11-1), Allowable 2L/120.

Vivco Components LLC.
2550 Hwy 33 South
P.O. Box 260
Maysville, Missouri 64469

Truss:T6
Job: 500 NW Chipman Rd REVISED
Date: 02/16/22 10:57:07

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
19-0-0	0.25 /12	3	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	119 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 20	Bldg Code : IBC 2018/	TC : 0.99 (1-2)	Vert TL: 0.27 in	L / 808	(8-9)	L / 240
TCDL : 15	TPI 1-2014	BC : 0.66 (6-7)	Vert LL: 0.1 in UP	L / 999	(8-9)	L / 360
BCLL : 0	Rep Mbr : Yes	Web : 0.92 (11-1)	Horz TL: 0.05 in		6	
BCDL : 10	Lumber D.O.L. : 115 %					

02/16/2022

STATE OF MISSOURI
ARTURO A. HERNANDEZ
NUMBER PE-2006000095
PROFESSIONAL ENGINEER

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
10	1	5.5 in	1.68 in	1,428 lbs	·	-36 lbs	-197 lbs	-197 lbs	491 lbs
6	1	5.5 in	1.66 in	1,409 lbs	·	·	-129 lbs	-129 lbs	·

Material

TC: SYP #1 2 x 4
BC: SYP #1 2 x 4
Web: SYP #2 2 x 4 except:
SYP SSDense 2 x 6: 11-10, 12-6

Bracing

TC: Sheathed
BC: Sheathed or Purlins at 6-9-0, Purlin design by Others.
Web: One Midpoint Row: 4-6

Loads

1) This truss has been designed for the effects due to 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has not been designed for the effects of unbalanced snow loads.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
5) Minimum storage attic loading has been applied in accordance with IBC 1607.1
6) ** - Indicates parapet wind loading has been applied to members 11-1 & 12-5

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	8-9-0	14-9-0	Down	Proj	54.17 plf	54.17 plf	

User-defined Load Case S2: Drift

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	19-0-0	Down	Proj	20 psf	20 psf	24 in
Top	0-0-0	7-2-0	Down	Proj	41 psf	0 psf	24 in
Top	14-6-0	19-0-0	Down	Proj	0 psf	32 psf	24 in

Point Loads

Member	Location	Direction	Load	Trib Width

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
19-0-0	0.25 /12	3	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	119 lbs

Load Combinations

#	Load Combo	Factor
1	D1	1.000
2	D1 + Lr1	1.000
3	D1 + S1	1.000
4	D1 + S2	1.000
5	0.60 D1 + 0.60 W1 [Uplift]	1.000
6	0.60 D1 + 0.60 W2 [Uplift]	1.000
7	0.60 D1 + 0.60 W4 [Uplift]	1.000
8	0.60 D1 + 0.60 W8 [Uplift]	1.000
9	D1 + L10*1	1.000
10	D1 + Lr1 + L10*1	1.000
11	D1 + S1 + L10*1	1.000
12	D1 + S2 + L10*1	1.000
13	D1 + UR1	1.000
14	D1 + UR2	1.000
15	D1 + AS10*1	1.000
16	D1 + Lr1 + AS10*1	1.000
17	D1 + S1 + AS10*1	1.000
18	D1 + S2 + AS10*1	1.000

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.990	-2,729 lbs		2-4	0.649	-2,665 lbs		4-5	0.641	1,052 lbs	(-497 lbs)
BC	6-7	0.657	2,659 lbs	(-694 lbs)	7-9	0.650	2,722 lbs	(-916 lbs)	9-10	0.263	791 lbs	(-716 lbs)
Web	1-10	0.638	-1,315 lbs		2-7	0.155	401 lbs	(-202 lbs)				
	1-9	0.706	2,875 lbs	(-211 lbs)	4-6	0.604	-2,847 lbs					
	2-9	0.091	-668 lbs									

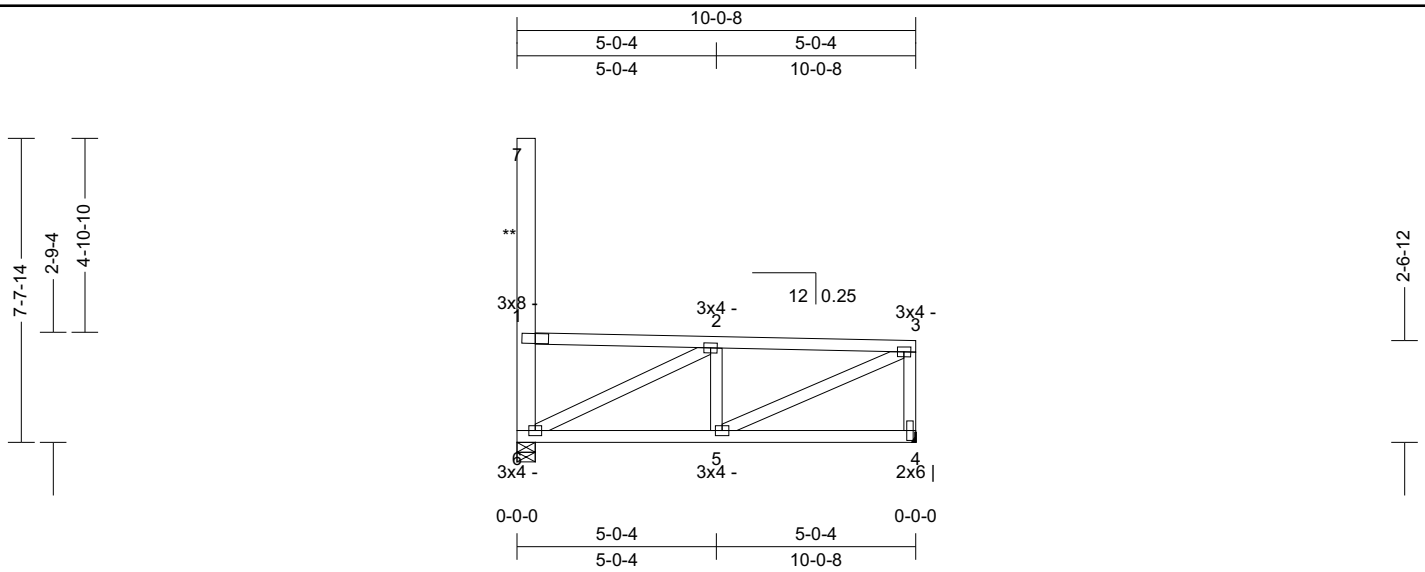
Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 30 % (Cq = 0.70).
3) Provide adequate drainage to prevent ponding.
4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
7) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
8) ☒ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
9) Listed wind uplift reactions based on MWFRS & C&C loading.
10) Parapet TL: 0.85 in, 2L/154 (11-1), Allowable 2L/120.

Vivco Components LLC.
2550 Hwy 33 South
P.O. Box 260
Maysville, Missouri 64469

Truss:T7
Job: 500 NW Chipman Rd REVISED
Date: 02/16/22 10:57:09

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
10-0-8	-0.25/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	63 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection		L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/ TPI 1-2014	TC: 0.46 (1-2)	Vert TL:	0.05 in	L/999	(4-5)	L/240
TCDL: 15	Rep Mbr: No	BC: 0.35 (5-6)	Vert LL:	0.02 in	L/999	(4-5)	L/360
BCLL: 0	Lumber D.O.L.: 115 %	Web: 0.75 (7-1)	Horz TL:	0.01 in		4	
BCDL: 10							

02/16/2022

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
6	1	5.5 in	1.50 in	720 lbs	·	-101 lbs	·	-101 lbs	480 lbs
4	1	1.5 in	—	621 lbs	·	-63 lbs	-449 lbs	-449 lbs	·

Material

TC: SYP #1 2 x 4
BC: SYP #1 2 x 4
Web: SYP #2 2 x 4 except:
SYP SSDense 2 x 6: 7-6

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 7-8-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects due to 10 psf bottom chord live load plus dead loads.
2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
3) This truss has not been designed for the effects of unbalanced snow loads.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
5) Minimum storage attic loading has been applied in accordance with IBC 1607.1
6) ** - Indicates parapet wind loading has been applied to member 7-1

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	3-6-0	5-8-0	Down	Proj	46.15 plf	46.15 plf	

User-defined Load Case S2: Drift

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	10-0-8	Down	Proj	20 psf	20 psf	24 in
Top	0-0-0	4-6-0	Down	Proj	32 psf	0 psf	24 in

Point Loads

Member	Location	Direction	Load	Trib Width

Load Combinations

#	Load Combo	Factor
1	D1	1.000
2	D1 + Lr1	1.000

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
10-0-8	-0.25/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	63 lbs

3	D1 + S1	1.000
4	D1 + S2	1.000
5	0.60 D1 + 0.60 W1 [Uplift]	1.000
6	0.60 D1 + 0.60 W2 [Uplift]	1.000
7	0.60 D1 + 0.60 W4 [Uplift]	1.000
8	0.60 D1 + 0.60 W8 [Uplift]	1.000
9	D1 + L10*1	1.000
10	D1 + Lr1 + L10*1	1.000
11	D1 + S1 + L10*1	1.000
12	D1 + S2 + L10*1	1.000
13	D1 + UR1	1.000
14	D1 + UR2	1.000
15	D1 + AS10*1	1.000
16	D1 + Lr1 + AS10*1	1.000
17	D1 + S1 + AS10*1	1.000
18	D1 + S2 + AS10*1	1.000

Member Forces

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.455	1,051 lbs	(-491 lbs)	2-3	0.441	-826 lbs				
BC	5-6	0.354	823 lbs	(-641 lbs)							
Web	2-6	0.433	-918 lbs		3-5	0.369	908 lbs	(-765 lbs)			
	2-5	0.070	399 lbs	(-214 lbs)	3-4	0.149	-537 lbs				

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 30 % (Cq = 0.70).
3) Hanger is for graphical intrepretation only. Install hanger per manufacturer's recommendation.
4) Provide adequate drainage to prevent ponding.
5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
6) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
7) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
8) Listed wind uplift reactions based on MWFRS & C&C loading.
9) Parapet TL: 0.66 in, 2L/184 (7-1), Allowable 2L/120.