

12300 Ford Rd, Suite 110 Dallas, Texas 75234

eaglemetal.com

The truss designs referenced below have been prepared by me or under my direct supervision based on the truss design criteria and requirements ("design criteria") provided by Vivco Components.

These truss designs are intended for the fabrication of individual building components that will perform to the design criteria provided. Any variance from the design criteria will render the affected truss designs inapplicable.

Listed below are the truss designs included in this package and covered by this seal.

Job: 500 NW Chipman Rd REVISED - 1160083

BP15, BP16, BP17, BP18, BP19, BP20, BP21, BP22, BP23, BP24, BP6, BP7, DT10, DT11, DT12, DT13, DT1, DT25, DT2, DT3, DT4, DT5, DT8, DT9, GR10, GR11, GR12, GR1, GR2, GR3, GR4, GR5, GR6, GR7, GR8, GR9, T1, T2, T3, T4, T5, T6, T7

Any location identification is for file reference only. No determination of the appropriateness of design criteria for any specific project has been made in preparing the truss designs.

Please refer to individual truss designs for specific design criteria.



Arturo A. Hernandez (MO, 2006000095)

My license renewal date for the state of MO is 12/31/2022.

IMPORTANT NOTE: The responsibility of the engineer sealing this package, as a Truss Engineer, is solely for design of individual trusses as individual building components based upon design criteria provided by others and set forth in the referenced truss drawings. The truss design criteria for the components have not been verified as appropriate for any particular building, project or use. Adequacy and suitability of design criteria and requirements for the truss designs for any specific project are the responsibility of the building designer, not the Truss Engineer, per ANSI/TPI-1, Chapter 2.

This review is only for conformance with the design concept of the project and general compliance with the information given in the Contract Documents. Corrections or comments made on the shop drawings during this review do not relieve contractor from compliance with the requirements of the plans and specifications. Contractor is responsible for: dimensions to be confirmed and correlated at the job site; information that pertains solely to the fabrication processes or to the means, method, techniques, sequences and procedures of construction; coordination of Work of all trades; and for performing all work in a safe and satisfactory manner.

SHOP DRAWING APPROVAL

NO EXCEPTION TAKEN APPROVED/REVISE AS NOTED

☐ REVISE AND RESUBMIT

KREHER ENGINEERING, INC. Structural Engineers

REVIEWER: NATHAN DREYER DATE: 03-16-2022



DESIGN NOTES

- The Truss Design Drawing(s) provided with these Design Notes have been prepared under and are subject to ANSI / TPI 1 published by the Truss Plate Institute, www.tpinst.org.
 Capitalized terms have the meanings provided in ANSI / TPI 1.
- Copies of each Truss Design Drawing shall be furnished to the installation contractor, Building Designer, Owner and all persons fabricating, handling, installing, bracing, or erecting the trusses.

DESIGN LIMITATIONS

- 3. The Truss Design Drawing is based upon specifications provided by the Building Designer in accordance with ANS1 / TPl 1. Neither the Truss Designer, Eagle, nor an engineer who seals this design (if any) assumes any responsibility for the adequacy or accuracy of specifications provided by the Building Designer.
- 4. The Building Designer is solely responsible for the suitability based upon the Truss Design Drawing and shall be responsible for reviewing and verifying that the information shown is in general conformance with the design of the Building.
- Each Truss Design Drawing is for the individual building component (a truss). A seal on the Truss Design Drawing indicates acceptance of professional engineering responsibility solely for the individual truss.
- **6.** Each Truss Design Drawing assumes trusses will be suitably protected from the environment.

HANDLING, INSTALLING, & BRACING

- Refer to Building Component Safety Information (BCSI) for handling, installing, restraining and bracing trusses. Copies can be obtained from the Structural Building Components Association, www.sbcindustry.com.
- 8. Bracing shown on each Truss Design Drawing is for lateral support of individual truss components only to reduce buckling lengths. All temporary and permanent bracing, including lateral load and diagonal or cross bracing, are the responsibility, respectively, of the erector and Building Designer.
- **9.** Eagle is not responsible for improper truss fabrication, handling, erection or bracing.
- **10.** Compression chords shall be laterally braced by the roof or floor sheathing, directly attached, or have purlins provided at spacing shown, unless noted otherwise.

- Bottom chord required bracing shall be at 10ft spacing or less, if no structural rated ceiling is installed, unless noted otherwise.
- **12.** Strongbacking shall be installed on all parallel chord trusses, including flooring systems, to limit deflection and reduce vibration. Refer to BCSI-B7.
- Never exceed the design loading shown. Never stack building or other materials on inadequately braced truss; refer to BCSI.
- **14.** Concentration of construction loads greater than the design loads shall not be applied to the trusses at any time; refer to BCSI.
- **15.** Trusses shall be handled with care prior to erection to avoid damage. Refer to BCSI for recommended truss handling and erection.

MATERIALS & FABRICATION

- Lumber moisture content shall be 19% or less at the time of fabrication unless noted otherwise.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- **18.** Unless expressly noted, the truss designs are not applicable for use with fire retardant or preservative treated lumber.
- 19. Plates shall be applied on both faces of truss at each joint and embedded fully. Knots and wane at joint locations shall be regulated in accordance with ANSI / TPI 1.
- **20.** For a specified plate gauge and grade, the specified size is a minimum.
- 21. Connections not shown are the responsibility of others.
- **22.** Adequate support shall be provided to resist gravity, lateral and uplift loads.
- 23. For 4X2 truss orientation, locate plates 0 1/16" from outside the edge of the truss.
- 24. Fabrication of truss shall be in accordance with ANSI / TPI 1.

OTHER NOTES

- **25.** Camber is a non-structural consideration and is the responsibility of truss fabricator.
- 26. Do not cut or alter any truss member or plate without prior approval from a professional engineer.
- 27. Lumber design values are in accordance with ANSI / TPI 1; lumber design values are by others.
- 28. Install specified hangers per manufacturer recommendations.

PL 3X

S

3X pe th

-, / slc

inf

20 **3X**

SX 8X

LA WI

co re: BE

Inc (sc

PL Th joi

wi sc.

R

Μŧ

•B Inf Ha Me

Co

•E!

Co

2550 Hwy 33 South P.O. Box 260 Maysville, Missouri 64469 Truss:BP15

Job: 500 NW Chipman Rd REVISED

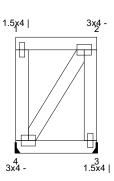
Date: 02/16/22 10:56:03

Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
1-10-8	0/12	2	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	15 lbs

1-10-8 1-10-8 1-10-8







All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflecti	on	L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2018/	TC:	0.07 (1-2)	Vert TL:	0 in	L/999	(3-4)	L/240
TCDL: 15		TPI 1-2014	BC:	0.03 (3-4)	Vert LL:	0 in	L/999	(3-4)	L/360
BCLL: 0	Rep Mbr:	No	Web	: 0.09 (1-4)	Horz TL:	0 in		3	
BCDL: 10	Lumber D.O.I	L.:115 %							02/16

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Upl	ift Max MWFRS UpliftN	Max C&C Uplift	Max Uplift	Max Horiz
4	1	1.5 in		137 lbs	•	-48 lbs	-45 lbs	-48 lbs	-90 lbs
3	1	1.5 in		137 lbs		-48 lbs	-45 lbs	-48 lbs	

Material

TC: SYP#1 2 x 4 BC: SYP#1 2 x 4 Web: SYP#2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- $1) This truss has been designed for the effects due to \\10 psf bottom chord live load plus dead loads.$
- 2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 3) This truss has not been designed for the effects of unbalanced snow loads.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL=1.60
- 5) Minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case D1: Std Dead Load

Distributed Load

Tom	0.00	1 10 9	Dorra	Duci	25 64 -16	25 64 -16	
Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
DISH IDUICU LOR	ius						

N	1ember Forces	Table indicates: M	lember ID, max CSI, max axial force, (max cor	npr. force if different from max axial force). Or	lly forces greater than 300lbs are shown in this tabl
TO					_
BO					
W	eh				

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 30% (Cq = 0.70).
- 3) Hangers are for graphical interpretation only. Install hangers per manufacturer's recommendations.
- 4) Provide adequate drainage to prevent ponding.
- 5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 7) Listed wind uplift reactions based on MWFRS & C&C loading.

Maysville, Missouri 64469

Job: 500 NW Chipman Rd REVISED

ARTURO A

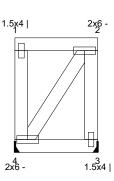
Date: 02/16/22 10:56:04

Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
1-10-8	0/12	2	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	15 lbs

1-10-8 1-10-8 1-10-8







All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI	Deflection	l	L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2018/	TC: 0.05 (1-2)	Vert TL:	0 in	L/999	(3-4)	L/240
TCDL: 15		TPI 1-2014	BC: 0.03 (3-4)	Vert LL:	0 in	L/999	(3-4)	L/360
BCLL: 0	Rep Mbr:	No	Web: 0.09 (1-4)	Horz TL:	0 in		3	
BCDL: 10	Lumber D.O.1	[4:115%	1					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Upl	ift Max MWFRS Uplift!	Max C&C Uplift	Max Uplift	Max Horiz
4	1	1.5 in		103 lbs	•	-64 lbs	-65 lbs	-65 lbs	-87 lbs
3	1	1.5 in		103 lbs		-64 lbs	-65 lbs	-65 lbs	

Material

TC: SYP#1 2 x 4 BC: SYP#1 2 x 4 Web: SYP#2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- 1) This truss has been designed for the effects due to 10 psf bottom chord live load plus dead loads.
- 2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 3) This truss has not been designed for the effects of unbalanced snow loads.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Factored),
- Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL=1.60
- 5) Minimum storage attic loading has been applied in accordance with IBC 1607.1

Men	nber Forces	Table indicates: M	ember ID, max CSI, max axial force, (max con	npr. force if different from max axial force). On	ly forces greater than 300lbs are shown in this table.
TC					
BC					
Web					

- $1) \, Unless \, noted \, otherwise, do \, not \, cut \, or \, alter \, any \, truss \, member \, or \, plate \, without \, prior \, approval \, from \, a \, Professional \, Engineer.$
- 2) The fabrication tolerance for this roof truss is 30% (Cq = 0.70).
- 3) Hangers are for graphical interpretation only. Install hangers per manufacturer's recommendations.
- 4) Provide adequate drainage to prevent ponding.
- 5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 7) Listed wind uplift reactions based on MWFRS & C&C loading.

Maysville, Missouri 64469

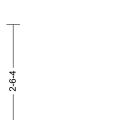
Job: 500 NW Chipman Rd REVISED

Date: 02/16/22 10:56:05

Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
1-10-8	0/12	2	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	14 lbs

1-10-8 1-10-8 1-10-8





0-0-0 0-0-0 1-10-8 1-10-8

All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflecti	on	L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2018/	TC: 0.	.05 (1-2)	Vert TL:	0 in	L/999	(3-4)	L/240
TCDL: 15	*	TPI 1-2014	BC: 0.	.03 (3-4)	Vert LL:	0 in	L/999	(3-4)	L/360
BCLL: 0	Rep Mbr:	No	Web: 0	.09 (1-4)	Horz TL:	0 in		3	
BCDL: 10	Lumber D.O.1	L.:115 %							02/16/

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Upl	lift Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
4	1	1.5 in		103 lbs		-60 lbs	-65 lbs	-65 lbs	-84 lbs 🖃
3	1	1.5 in		103 lbs		-60 lbs	-65 lbs	-65 lbs	. 3

Material

TC: SYP#1 2 x 4 BC: SYP#1 2 x 4 Web: SYP#2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- $1) This truss has been designed for the effects due to \\10 psf bottom chord live load plus dead loads.$
- 2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 3) This truss has not been designed for the effects of unbalanced snow loads.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL=1.60
- 5) Minimum storage attic loading has been applied in accordance with IBC 1607.1

Men	nber Forces	Table indicates: M	ember ID, max CSI, max axial force, (max con	npr. force if different from max axial force). On	ly forces greater than 300lbs are shown in this table.
TC					
BC					
Web					

Notes

- $1) \, Unless \, noted \, otherwise, do \, not \, cut \, or \, alter \, any \, truss \, member \, or \, plate \, without \, prior \, approval \, from \, a \, Professional \, Engineer.$
- 2) The fabrication tolerance for this roof truss is 30 % (Cq = 0.70).
- $3) \, Hangers \, are \, for \, graphical \, intrepretation \, only. \, In stall \, hangers \, per \, manufacturer's \, recommendations.$
- 4) Provide adequate drainage to prevent ponding.
- 5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 7) Listed wind uplift reactions based on MWFRS & C&C loading.

"Himmuni

Maysville, Missouri 64469

Truss:BP18

Job: 500 NW Chipman Rd REVISED

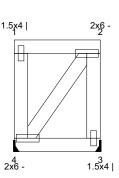
Date: 02/16/22 10:56:07

Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
1-10-8	0/12	2	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	14 lbs

1-10-8 1-10-8 1-10-8







All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI	Deflection		L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2018/	TC: 0.05 (1-2)	Vert TL:	0 in	L/999	(3-4)	L/240
TCDL: 15		TPI 1-2014	BC: 0.03 (3-4)	Vert LL:	0 in	L/999	(3-4)	L/360
BCLL: 0	Rep Mbr:	No	Web: 0.08 (1-4)	Horz TL:	0 in		3	
BCDL: 10	Lumber DO	I. ·115 %						

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplif	ft Max MWFRS Uplift!	Max C&C Uplift	Max Uplift	Max Horiz
4	1	1.5 in		103 lbs		-56 lbs	-65 lbs	-65 lbs	-81 lbs
3	1	1.5 in		103 lbs		-56 lbs	-65 lbs	-65 lbs	

Material

TC: SYP#1 2 x 4 BC: SYP#1 2 x 4 Web: SYP#2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- $1) This truss has been designed for the effects due to 10\,psf bottom chord live load plus dead loads.$
- 2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 3) This truss has not been designed for the effects of unbalanced snow loads.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL=1.60
- 5) Minimum storage attic loading has been applied in accordance with IBC 1607.1

Men	nber Forces	Table indicates: M	ember ID, max CSI, max axial force, (max con	npr. force if different from max axial force). On	ly forces greater than 300lbs are shown in this table.
TC					
BC					
Web					

- $1) \, Unless \, noted \, otherwise, do \, not \, cut \, or \, alter \, any \, truss \, member \, or \, plate \, without \, prior \, approval \, from \, a \, Professional \, Engineer.$
- 2) The fabrication tolerance for this roof truss is 30 % (Cq = 0.70).
- 3) Hangers are for graphical intrepretation only. Install hangers per manufacturer's recommendations.
- 4) Provide adequate drainage to prevent ponding.
- 5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 7) Listed wind uplift reactions based on MWFRS & C&C loading.

Maysville, Missouri 64469

Job: 500 NW Chipman Rd REVISED

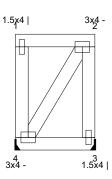
Date: 02/16/22 10:56:08

Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
1-10-0	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	15 lbs

1-10-0 1-10-0 1-10-0







All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		I	Deflection	n	L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2018/	TC:	0.05 (1-2)	1	Vert TL:	0 in	L/999	(3-4)	L/240
TCDL: 15	*	TPI 1-2014	BC:	0.03 (3-4)	7	Vert LL:	0 in	L/999	(3-4)	L/360
BCLL: 0	Rep Mbr:	No	Web:	0.10 (1-4)	H	Horz TL:	0 in		3	
BCDL: 10	Lumber D.O.1	L.:115 %								02/16

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Upl	ift Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
4	1	1.5 in		101 lbs	•	-70 lbs	-63 lbs	-70 lbs	-89 lbs
3	1	1.5 in		101 lbs		-70 lbs	-63 lbs	-70 lbs	

Material

TC: SYP#1 2 x 4 BC: SYP#1 2 x 4 Web: SYP#2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- $1) This truss has been designed for the effects due to \\10 psf bottom chord live load plus dead loads.$
- 2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 3) This truss has not been designed for the effects of unbalanced snow loads.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL=1.60
- 5) Minimum storage attic loading has been applied in accordance with IBC 1607.1

Men	nber Forces	Table indicates: M	ember ID, max CSI, max axial force, (max con	npr. force if different from max axial force). On	ly forces greater than 300lbs are shown in this table.
TC					
BC					
Web					

- $1) \, Unless \, noted \, otherwise, do \, not \, cut \, or \, alter \, any \, truss \, member \, or \, plate \, without \, prior \, approval \, from \, a \, Professional \, Engineer.$
- 2) The fabrication tolerance for this roof truss is 30 % (Cq = 0.70).
- $3) \, Hangers \, are \, for \, graphical \, intrepretation \, only. \, In stall \, hangers \, per \, manufacturer's \, recommendations.$
- 4) Provide adequate drainage to prevent ponding.
- 5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 7) Listed wind uplift reactions based on MWFRS & C&C loading.

Maysville, Missouri 64469

Job: 500 NW Chipman Rd REVISED

Date: 02/16/22 10:56:09

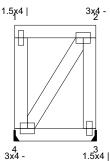
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
1-10-8	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	15 lbs

1-10-8 1-10-8 1-10-8









All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		I	Deflection	n	L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2018/	TC: (0.05 (1-2)	V	/ert TL:	0 in	L/999	(3-4)	L/240
TCDL: 15		TPI 1-2014	BC: (0.03 (3-4)	V	/ert LL:	0 in	L/999	(3-4)	L/360
BCLL: 0	Rep Mbr:	No	Web: (0.10 (1-4)	H	Horz TL:	0 in		3	
BCDL: 10	Lumber D.O.I	L.:115 %								

Reaction

J	T Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Upl	lift Max MWFRS UpliftN	Max C&C Uplift	Max Uplift	Max Horiz
	4 1	1.5 in		103 lbs	•	-68 lbs	-65 lbs	-68 lbs	-89 lbs
3	3 1	1.5 in		103 lbs		-68 lbs	-65 lbs	-68 lbs	

Material

TC: SYP#1 2 x 4 BC: SYP#1 2 x 4 Web: SYP#2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- 1) This truss has been designed for the effects due to 10 psf bottom chord live load plus dead loads.
- 2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 3) This truss has not been designed for the effects of unbalanced snow loads.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL=1.60
- 5) Minimum storage attic loading has been applied in accordance with IBC 1607.1

Men	nber Forces	Table indicates: M	ember ID, max CSI, max axial force, (max con	npr. force if different from max axial force). On	ly forces greater than 300lbs are shown in this table.
TC					
BC					
Web					

- $1) \, Unless \, noted \, otherwise, do \, not \, cut \, or \, alter \, any \, truss \, member \, or \, plate \, without \, prior \, approval \, from \, a \, Professional \, Engineer.$
- 2) The fabrication tolerance for this roof truss is 30% (Cq = 0.70).
- 3) Hangers are for graphical interpretation only. Install hangers per manufacturer's recommendations.
- 4) Provide adequate drainage to prevent ponding.
- 5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.

Maysville, Missouri 64469

Job: 500 NW Chipman Rd REVISED

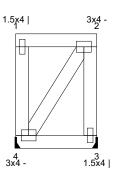
Date: 02/16/22 10:56:10

Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
1-10-0	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	15 lbs

1-10-0 1-10-0 1-10-0





0-0-0 0-0-0 | 1-10-0 |

All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI	Deflection	1	L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2018/	TC: 0.05 (1-2)	Vert TL:	0 in	L/999	(3-4)	L/240
TCDL: 15	*	TPI 1-2014	BC: 0.03 (3-4)	Vert LL:	0 in	L/999	(3-4)	L/360
BCLL: 0	Rep Mbr:	No	Web: 0.09 (1-4)	Horz TL:	0 in		3	
BCDL: 10	Lumber D.O.1	L.:115 %						63/16

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Upli	ift Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
4	1	1.5 in		101 lbs	•	-65 lbs	-63 lbs	-65 lbs	-87 lbs
3	1	1.5 in		101 lbs		-65 lbs	-63 lbs	-65 lbs	

Material

TC: SYP#1 2 x 4 BC: SYP#1 2 x 4 Web: SYP#2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- $1) This truss has been designed for the effects due to \\10 psf bottom chord live load plus dead loads.$
- 2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shallverify snow loads.
- 3) This truss has not been designed for the effects of unbalanced snow loads.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL=1.60
- 5) Minimum storage attic loading has been applied in accordance with IBC 1607.1

Men	nber Forces	Table indicates: M	ember ID, max CSI, max axial force, (max con	npr. force if different from max axial force). On	ly forces greater than 300lbs are shown in this table.
TC					
BC					
Web					

- $1) \, Unless \, noted \, otherwise, do \, not \, cut \, or \, alter \, any \, truss \, member \, or \, plate \, without \, prior \, approval \, from \, a \, Professional \, Engineer.$
- 2) The fabrication tolerance for this roof truss is 30 % (Cq = 0.70).
- 3) Hangers are for graphical intrepretation only. Install hangers per manufacturer's recommendations.
- 4) Provide adequate drainage to prevent ponding.
- 5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 7) Listed wind uplift reactions based on MWFRS & C&C loading.

P.O. Box 260 Maysville, Missouri 64469

Job: 500 NW Chipman Rd REVISED

HERNANDEZ

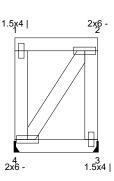
Date: 02/16/22 10:56:11

Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
1-10-8	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	15 lbs

1-10-8 1-10-8 1-10-8







All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI	Deflection	n	L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2018/	TC: 0.05 (1-2)	Vert TL:	0 in	L/999	(3-4)	L/240
TCDL: 15	_	TPI 1-2014	BC: 0.03 (3-4)	Vert LL:	0 in	L/999	(3-4)	L/360
BCLL: 0	Rep Mbr:	No	Web: 0.09 (1-4)	Horz TL:	0 in		3	
BCDL: 10	Lumber D.O.I	:115 %	Ì					62/16

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Upl	ift Max MWFRS Uplift!	Max C&C Uplift	Max Uplift	Max Horiz
4	1	1.5 in		103 lbs		-63 lbs	-65 lbs	-65 lbs	-87 lbs
3	1	1.5 in		103 lbs		-63 lbs	-65 lbs	-65 lbs	

Material

TC: SYP#1 2 x 4 BC: SYP#1 2 x 4 Web: SYP#2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- $1) This truss has been designed for the effects due to 10\,psf bottom chord live load plus dead loads.$
- 2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 3) This truss has not been designed for the effects of unbalanced snow loads.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL=1.60
- 5) Minimum storage attic loading has been applied in accordance with IBC 1607.1

Men	nber Forces	Table indicates: M	ember ID, max CSI, max axial force, (max con	npr. force if different from max axial force). On	ly forces greater than 300lbs are shown in this table.
TC					
BC					
Web					

- $1) \, Unless \, noted \, otherwise, do \, not \, cut \, or \, alter \, any \, truss \, member \, or \, plate \, without \, prior \, approval \, from \, a \, Professional \, Engineer.$
- 2) The fabrication tolerance for this roof truss is 30 % (Cq = 0.70).
- $3) \, Hangers \, are \, for \, graphical \, interpretation \, only. \, In stall \, hangers \, per \, manufacturer's \, recommendations.$
- 4) Provide adequate drainage to prevent ponding.
- 5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 7) Listed wind uplift reactions based on MWFRS & C&C loading.

P.O. Box 260 Maysville, Missouri 64469

Truss:BP23

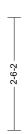
Job: 500 NW Chipman Rd REVISED

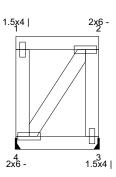
Date: 02/16/22 10:56:12

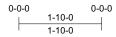
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
1-10-0	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	14 lbs

1-10-0 1-10-0 1-10-0







All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Defle	ction		L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2018/	TC:	0.05 (1-2)	Vert TI	<i>.</i> :	0 in	L/999	(3-4)	L/240
TCDL: 15		TPI 1-2014	BC:	0.03 (3-4)	Vert LI	<i>:</i> :	0 in	L/999	(3-4)	L/360
BCLL: 0	Rep Mbr:	No	Web:	0.08 (1-4)	Horz T	L:	0 in		3	
BCDL: 10	Lumber D.O.I	L.:115 %								27/16

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Upl	ift Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
4	1	1.5 in		101 lbs	•	-61 lbs	-63 lbs	-63 lbs	-84 lbs
3	1	1.5 in		101 lbs		-61 lbs	-63 lbs	-63 lbs	

Material

TC: SYP#1 2 x 4 BC: SYP#1 2 x 4 Web: SYP#2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- $1) This truss has been designed for the effects due to \\10 psf bottom chord live load plus dead loads.$
- 2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 3) This truss has not been designed for the effects of unbalanced snow loads.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL=1.60
- 5) Minimum storage attic loading has been applied in accordance with IBC 1607.1

Men	nber Forces	Table indicates: M	ember ID, max CSI, max axial force, (max con	npr. force if different from max axial force). On	ly forces greater than 300lbs are shown in this table.
TC					
BC					
Web					

- $1) \, Unless \, noted \, otherwise, do \, not \, cut \, or \, alter \, any \, truss \, member \, or \, plate \, without \, prior \, approval \, from \, a \, Professional \, Engineer.$
- 2) The fabrication tolerance for this roof truss is 30 % (Cq = 0.70).
- $3) \, Hangers \, are \, for \, graphical \, intrepretation \, only. \, In stall \, hangers \, per \, manufacturer's \, recommendations.$
- 4) Provide adequate drainage to prevent ponding.
- 5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 7) Listed wind uplift reactions based on MWFRS & C&C loading.

P.O. Box 260 Maysville, Missouri 64469

Truss:BP24

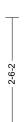
Job: 500 NW Chipman Rd REVISED

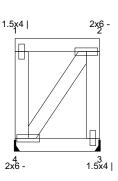
Date: 02/16/22 10:56:14

Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
1-10-8	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	14 lbs

1-10-8 1-10-8 1-10-8







All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI]	Deflection	n	L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2018/	TC:	0.05 (1-2)	1	Vert TL:	0 in	L/999	(3-4)	L/240
TCDL: 15	_	TPI 1-2014	BC:	0.03 (3-4)	1	Vert LL:	0 in	L/999	(3-4)	L/360
BCLL: 0	Rep Mbr:	No	Web:	: 0.09 (1-4)	I	Horz TL:	0 in		3	
BCDL: 10	Lumber D.O.I	L.:115 %								02/1

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Upli	ift Max MWFRS Uplift!	Max C&C Uplift	Max Uplift	Max Horiz
4	1	1.5 in		103 lbs	•	-59 lbs	-65 lbs	-65 lbs	-84 lbs
3	1	1.5 in		103 lbs		-59 lbs	-65 lbs	-65 lbs	

Material

TC: SYP#1 2 x 4 BC: SYP#1 2 x 4 Web: SYP#2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- $1) This truss has been designed for the effects due to \\10 psf bottom chord live load plus dead loads.$
- 2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shallverify snow loads.
- 3) This truss has not been designed for the effects of unbalanced snow loads.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL=1.60
- 5) Minimum storage attic loading has been applied in accordance with IBC 1607.1

Men	nber Forces	Table indicates: M	ember ID, max CSI, max axial force, (max con	npr. force if different from max axial force). On	ly forces greater than 300lbs are shown in this table.
TC					
BC					
Web					

- $1) \, Unless \, noted \, otherwise, do \, not \, cut \, or \, alter \, any \, truss \, member \, or \, plate \, without \, prior \, approval \, from \, a \, Professional \, Engineer.$
- 2) The fabrication tolerance for this roof truss is 30 % (Cq = 0.70).
- 3) Hangers are for graphical intrepretation only. Install hangers per manufacturer's recommendations.
- 4) Provide adequate drainage to prevent ponding.
- 5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 7) Listed wind uplift reactions based on MWFRS & C&C loading.

2550 Hwy 33 South P.O. Box 260 Maysville, Missouri 64469 Truss:BP6

Job: 500 NW Chipman Rd REVISED

Date: 02/16/22 10:56:15

Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
1-10-0	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	2	24 in	14 lbs

1-10-0 1-10-0 1-10-0

2x6 -





0-0-0 0-0-0

All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflectio	n	L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2018/	TC: (0.13 (1-2)	Vert TL:	0 in	L/999	(3-4)	L/240
TCDL: 15		TPI 1-2014	BC: (0.01 (3-4)	Vert LL:	0 in	L/999	(3-4)	L/360
BCLL: 0	Rep Mbr:	No	Web: (0.03 (1-4)	Horz TL:	0 in		3	
BCDL: 10	Lumber D.O.I	L.:115 %							02/16/

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	:Max C&C Uplift	Max Uplift	
4	1	1.5 in		412 lbs	•		•	•	_
3	1	1.5 in		412.lbs					

Max Horiz 81 lbs

Material

TC: SYP#1 2 x 4 BC: SYP#1 2 x 4 Web: SYP#2 2 x 4

Bracing

Start Load

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

End Load

Trih Width

Loads

 $1) This truss has been designed for the effects due to \\10 psf bottom chord live load plus dead loads.$

Direction

- 2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 3) This truss has not been designed for the effects of unbalanced snow loads.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL=1.60

Spread

5) Minimum storage attic loading has been applied in accordance with IBC 1607.1

Location 2

Load Case Lr1: Std Live Load

Location 1

Member

Тор	0-0-0	1-10-0	Down	Proj	90 plf	90 plf					
Load Case D1: Std Dead Load											
Distributed Load	ls										
Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width				
Тор	0-0-0	1-10-0	Down	Proj	160 plf	160 plf					
User-defined Load Case S2: Snow Drift											
Distributed Load	ls										
Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width				

Тор	0-0-0	1-10-0	Down	Proj	20 psf	20 psf	24 in
Тор	0-0-0	1-10-0	Down	Proj	180 plf	180 plf	
Point Loads							
Member	Location	Direction	Load	Trib Width			

2550 Hwy 33 South P.O. Box 260 Maysville, Missouri 64469 Truss:BP6

Job: 500 NW Chipman Rd REVISED

Date: 02/16/22 10:56:15

Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
1-10-0	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	2	24 in	14 lbs

Load Co	Load Combinations											
#	Load Combo	Factor										
1	D1	1.000										
2	D1+Lr1	1.000										
3	D1 +S1	1.000										
4	D1 +S2	1.000										
5	0.60 D1 +0.60 W1 [Uplift]	1.000										
6	0.60 D1 +0.60 W2 [Uplift]	1.000										
7	0.60 D1 +0.60 W4 [Uplift]	1.000										
8	0.60 D1 +0.60 W8 [Uplift]	1.000										
9	D1 +L10*1	1.000										
10	D1 +Lr1 +L10*1	1.000										
11	D1+S1+L10*1	1.000										
12	D1 +S2 +L10*1	1.000										
13	D1 + UR1	1.000										
14	D1 +UR2	1.000										
15	D1 +AS10*1	1.000										
16	D1 +Lr1 +AS10*1	1.000										
17	D1 +S1 +AS10*1	1.000										
18	D1 +S2 +AS10*1	1.000										

Men	nber Forces	Table indicates: M	Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.								
TC											
BC											
Web											

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 30% (Cq = 0.70).
- 3) Hangers are for graphical interpretation only. Install hangers per manufacturer's recommendations.
- 4) Provide adequate drainage to prevent ponding.
- 5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 7) The forces shown for this multi-ply truss are per ply and the reactions are for all plies. Two identical trusses shall be built and attached as follows: SDS Simpson 0.25"x3" (2 ply) Screws TC 1 row @ 13.5 in oc, BC 1 row @ 24 in oc, Webs 1 row 10d Nails or Gun Nails [min .12"x2] @ 24 in oc. 8) When applied loads are on one side of girder, do not flip girder during girder connector installation, install connectors on the girder side where supported loads are applied. When applied loads are on both sides of girder, double the spacing and install half of the connectors on one side of girder and then flip the girder to install the other half of the connectors on the opposite side (at double the connector spacing). Connectors on opposite sides of the girder shall be
- 9) Lateral bracing shall be attached to each ply.
- 10) Install screws per manufacturer recommendations.
- 11) Listed wind uplift reactions based on MWFRS & C&C loading.

2550 Hwy 33 South P.O. Box 260 Maysville, Missouri 64469 Truss:BP7

Job: 500 NW Chipman Rd REVISED

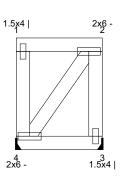
Date: 02/16/22 10:56:17

Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
1-10-8	0/12	3	0-0-0	0-0-0	0-0-0	0-0-0	2	24 in	14 lbs

1-10-8 1-10-8 1-10-8





All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflectio	n	L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2018/	TC:	0.12 (1-2)	Vert TL:	0 in	L/999	(3-4)	L/240
TCDL: 15		TPI 1-2014	BC:	0.01 (3-4)	Vert LL:	0 in	L/999	(3-4)	L/360
BCLL: 0	Rep Mbr:	Yes	Web:	0.03 (1-4)	Horz TL:	0 in		3	na tre t
BCDL: 10	Lumber D.O.I	L.:115 %							02/16/

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	tMax C&C Uplift	Max Uplift	
4	1	1.5 in		422 lbs	•	•	•	•	Τ
3	1	1.5 in		422.lbs					

Max Horiz ARTURO ARTURO

Material

TC: SYP#1 2 x 4 BC: SYP#1 2 x 4 Web: SYP#2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- $1) This truss has been designed for the effects due to \\10 psf bottom chord live load plus dead loads.$
- 2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 3) This truss has not been designed for the effects of unbalanced snow loads.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL=1.60
- 5) Minimum storage attic loading has been applied in accordance with IBC 1607.1

Direction

Load

Load Case Lr1: Std Live Load

Location

Point Loads Member

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	TribWidth							
Top	0-0-0	1-10-8	Down	Proj	90 plf	90 plf								
Load Case D1	Load Case D1: Std Dead Load													
Distributed Loads	S													
Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width							
Тор	0-0-0	1-10-8	Down	Proj	160 plf	160 plf								
User-defined L	.oad Case S2: Sr	now Drift												
Distributed Loads	S													
Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	TribWidth							
Тор	0-0-0	1-10-8	Down	Proj	20 psf	20 psf	24 in							
Top	0-0-0	1-10-8	Down	Proj	180 plf	180 plf								

Trib Width

2550 Hwy 33 South P.O. Box 260 Maysville, Missouri 64469 Truss:BP7

Job: 500 NW Chipman Rd REVISED

Date: 02/16/22 10:56:17

Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
1-10-8	0/12	3	0-0-0	0-0-0	0-0-0	0-0-0	2	24 in	14 lbs

Load Co	ombinations	
#	Load Combo	Factor
1	D1	1.000
2	D1 +Lr1	1.000
3	D1 +S1	1.000
4	D1 +S2	1.000
5	0.60 D1 +0.60 W1 [Uplift]	1.000
6	0.60 D1 +0.60 W2 [Uplift]	1.000
7	0.60 D1 +0.60 W4 [Uplift]	1.000
8	0.60 D1 +0.60 W8 [Uplift]	1.000
9	D1 +L10*1	1.000
10	D1 +Lr1 +L10*1	1.000
11	D1 + S1 + L10*1	1.000
12	D1 + S2 + L10*1	1.000
13	D1 +UR1	1.000
14	D1 + UR2	1.000
15	D1 +AS10*1	1.000
16	D1 +Lr1 +AS10*1	1.000
17	D1 +S1 +AS10*1	1.000
18	D1 +S2 +AS10*1	1.000

Member F	Orces Table indicates: M	ember ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.						
TC				<u> </u>				
BC								
Web								

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 30% (Cq = 0.70).
- 3) Hangers are for graphical interpretation only. Install hangers per manufacturer's recommendations.
- 4) Provide adequate drainage to prevent ponding.
- 5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 7) The forces shown for this multi-ply truss are per ply and the reactions are for all plies. Two identical trusses shall be built and attached as follows: SDS Simpson 0.25"x3" (2 ply) Screws TC-1 row @ 13.5 in oc, BC-1 row @ 24 in oc, Webs 1 row 10d Nails or Gun Nails [min .12"x2] @ 24 in oc. 8) When applied loads are on one side of girder, do not flip girder during girder connector installation, install connectors on the girder side where supported loads are applied. When applied loads are on both sides of girder, double the spacing and install half of the connectors on one side of girder and then flip the girder to install the other half of the connectors on the opposite side (at double the connector spacing). Connectors on opposite sides of the girder shall be
- 9) Lateral bracing shall be attached to each ply.
- 10) Install screws per manufacturer recommendations.
- 11) Listed wind uplift reactions based on MWFRS & C&C loading.

P.O. Box 260 Maysville, Missouri 64469 Truss:DT1

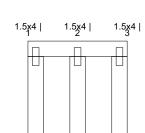
Job: 500 NW Chipman Rd REVISED

Date: 02/16/22 10:56:18

Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
1-10-8	0/12	13	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	13 lbs

1-10-8 1-10-8 1-10-8





All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI	Deflectio	n	L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2018/	TC: 0.06 (1-2)	Vert TL:	0 in UP	L/999	4	L/240
TCDL: 15		TPI 1-2014	BC: 0.00 (4-5)	Vert LL:	0 in	L/999	4	L/360
BCLL: 0	Rep Mbr:	Yes	Web: 0.08 (1-6)	Horz TL:	0 in			
BCDL: 10	Lumber D.O.I	L.:115 %						

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift N	Max MWFRS Up	oliftMax C&C Uplift	Max Uplift	Max Horiz
1		93 lbs	110 plf		-53 lbs	-55 lbs	-55 lbs	46 lbs

Material

TC: SYP#1 2 x 4 BC: SYP#1 2 x 4 Web: SYP#1 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

I nads

- 1) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 2) This truss has not been designed for the effects of unbalanced snow loads.
- 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL=1.60

Men	nber Forces	Table indicates: M	ember ID, max CSI, max axial force, (max con	npr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.
TC				
BC				
Web				<u> </u>

- $1) \, Unless \, noted \, otherwise, do \, not \, cut \, or \, alter \, any \, truss \, member \, or \, plate \, \, without \, prior \, approval \, from \, a \, Professional \, Engineer.$
- 2) Gable requires continuous bottom chord bearing.
- 3) Gable webs placed at 16 "OC, U.N.O.
- 4) Attach gable webs with 1.5x4 20ga plates, U.N.O.
- 5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- 6) The fabrication tolerance for this roof truss is 30% (Cq = 0.70).
- 7) Provide adequate drainage to prevent ponding.
- 8) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 9) Listed wind uplift reactions based on MWFRS & C&C loading.

P.O. Box 260 Maysville, Missouri 64469 Truss:DT10

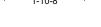
Job: 500 NW Chipman Rd REVISED

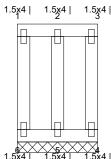
Date: 02/16/22 10:56:19

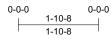
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
1-10-8	0/12	2	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	15 lbs

1-10-8 1-10-8 1-10-8







All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection	n	L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2018/	TC:	0.09 (1-2)	Vert TL:	0 in UP	L/999	4	L/240
TCDL: 15		TPI 1-2014	BC:	0.00 (4-5)	Vert LL:	0 in	L/999	4	L/360
BCLL: 0	Rep Mbr:	No	Web:	0.10 (1-6)	Horz TL:	0 in			
BCDL: 10	Lumber D.O.I	L.:115 %							ne

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Up	olift Max MWFRS Uj	oliftMax C&C Uplift	Max Uplift	Max Horiz
1		93 lbs	124 plf		-73 lbs	-63 lbs	-73 lbs	-53 lbs

Material

TC: SYP#1 2 x 4 BC: SYP#1 2 x 4 Web: SYP#1 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

I nads

- 1) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 2) This truss has not been designed for the effects of unbalanced snow loads.
- 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL=1.60

Men	nber Forces	Table indicates: M	ember ID, max CSI, max axial force, (max con	npr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.
TC				
BC				
Web				

- $1) \, Unless \, noted \, otherwise, do \, not \, cut \, or \, alter \, any \, truss \, member \, or \, plate \, \, without \, prior \, approval \, from \, a \, Professional \, Engineer.$
- 2) Gable requires continuous bottom chord bearing.
- 3) Gable webs placed at 16 "OC, U.N.O.
- 4) Attach gable webs with 1.5x4 20ga plates, U.N.O.
- 5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- 6) The fabrication tolerance for this roof truss is 30 % (Cq = 0.70).
- 7) Provide adequate drainage to prevent ponding.
- 8) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 9) Listed wind uplift reactions based on MWFRS & C&C loading.

Vivco Components LLC. 2550 Hwy 33 South P.O. Box 260 Maysville, Missouri 64469

Truss:DT11

Job: 500 NW Chipman Rd REVISED

Date: 02/16/22 10:56:20

Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
0-10-8	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	9 lbs

0-10-8 0-10-8





0-0-0 0-0 0-0-0 0-10-8

All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2018/	TC: 0.09 (1-2)	Vert TL: 0 i	in L/999	3	L/240
TCDL: 15		TPI 1-2014	BC: 0.00 (3-4)	Vert LL: 0 i	in L/999	3	L/360
BCLL: 0	Rep Mbr:	No	Web: 0.12 (1-4)	Horz TL: 0 i	in		
BCDL: 10	Lumber D.O.I	L.:115 %					

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav U	plift Max MWFRS U	pliftMax C&C Uplift	Max Uplift	Max Horiz
1		195 lbs	445 plf		-195 lbs	-30 lbs	-195 lbs	-52 lbs

Material

TC: SYP#1 2 x 4 BC: SYP#1 2 x 4 Web: SYP#1 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

1) This truss has been designed for the effects of balanced (20 psf) and unbalanced flat roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Risk Category II (I = 1.00), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.

2) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60

Men	nber Forces	Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.							
TC									
BC									
Web									

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Gable requires continuous bottom chord bearing.
- 3) Gable webs placed at 16 "OC, U.N.O.
- 4) Attach gable webs with 1.5x4 20ga plates, U.N.O.
- 5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- 6) The fabrication tolerance for this roof truss is 30% (Cq = 0.70).
- 7) Provide adequate drainage to prevent ponding.
- 8) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 9) Listed wind uplift reactions based on MWFRS & C&C loading.

Maysville, Missouri 64469

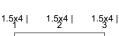
Job: 500 NW Chipman Rd REVISED

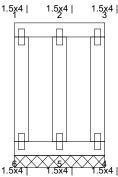
Date: 02/16/22 10:56:21

Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
1-10-8	0/12	11	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	15 lbs

1-10-8 1-10-8 1-10-8





0-0-0 0-0-0 1-10-8 1-10-8

All plates shown to be Eagle 20 unless otherwise noted.

Lumber D.O.L.:115 %

Loading (psf)	General		CSI	Deflection	n	L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2018/	TC: 0.08 (1-2)	Vert TL:	0 in UP	L/999	4	L/240
TCDL: 15		TPI 1-2014	BC: 0.00 (4-5)	Vert LL:	0 in	L/999	4	L/360
RCII · O	Ren Mhr	Vec	Web: 0.10(1-6)	Horz TI ·	() in			

Reaction

BCDL: 10

2-9-2

Brg Combo	Brg Width	Max React	Ave React	Max Grav Up	olift Max MWFRS U	pliftMax C&C Uplift	Max Uplift	Max Horiz
1		93 lbs	124 plf		-73 lbs	-63 lbs	-73 lbs	-53 lbs

Material

TC: SYP#1 2 x 4 BC: SYP#1 2 x 4 Web: SYP#1 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

- 1) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 2) This truss has not been designed for the effects of unbalanced snow loads.
- 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL=1.60

Men	nber Forces	Table indicates: M	ember ID, max CSI, max axial force, (max con	apr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.
TC				
BC				
Web				

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Gable requires continuous bottom chord bearing.
- 3) Gable webs placed at 16 "OC, U.N.O.
- 4) Attach gable webs with 1.5x4 20ga plates, U.N.O.
- 5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- 6) The fabrication tolerance for this roof truss is 30% (Cq = 0.70).
- 7) Provide adequate drainage to prevent ponding.
- 8) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 9) Listed wind uplift reactions based on MWFRS & C&C loading.

Maysville, Missouri 64469

Truss:DT13

Job: 500 NW Chipman Rd REVISED

ARTURO A

"Himmuni

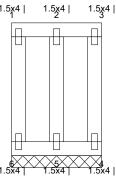
Date: 02/16/22 10:56:23

Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
1-10-8	0/12	5	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	15 lbs

1-10-8 1-10-8 1-10-8





0-0-0 0-0-0 1-10-8 1-10-8

All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI	Deflectio	n	L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2018/	TC: 0.08 (2-3)	Vert TL:	0 in UP	L/999	4	L/240
TCDL: 15		TPI 1-2014	BC: 0.00 (4-5)	Vert LL:	0 in	L/999	4	L/360
BCLL: 0	Rep Mbr:	Yes	Web: 0.10 (1-6)	Horz TL:	0 in			
BCDL: 10	Lumber D.O.L	.:115 %	, ,					{

Reaction

2-9-4

Brg Combo	Brg Width	Max React	Ave React	Max Grav U	plift Max MWFRS U	pliftMax C&C Uplift	Max Uplift	Max Horiz
1		93 lbs	125 plf		-74 lbs	-64 lbs	-74 lbs	-54 lbs

Material

TC: SYP#1 2 x 4 BC: SYP#1 2 x 4 Web: SYP#1 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

1) This truss has been designed for the effects of balanced (20 pst) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.

2) This truss has not been designed for the effects of unbalanced snow loads.

3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL=1.60

Men	nber Forces	Table indicates: M	ember ID, max CSI, max axial force, (max con	apr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.
TC				
BC				
Web				_

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Gable requires continuous bottom chord bearing.

- 7) Provide adequate drainage to prevent ponding.
- 8) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 9) Listed wind uplift reactions based on MWFRS & C&C loading.

P.O. Box 260 Maysville, Missouri 64469 Truss:DT2

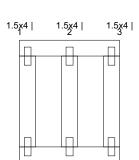
Job: 500 NW Chipman Rd REVISED

Date: 02/16/22 10:56:24

Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
1-10-0	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	13 lbs

1-10-0 1-10-0 1-10-0



0-0-0 0-0-0 1-10-0 1-10-0

All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection	n	L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2018/	TC:	0.06 (2-3)	Vert TL:	0 in UP	L/999	4	L/240
TCDL: 15	*	TPI 1-2014	BC:	0.00 (4-5)	Vert LL:	0 in	L/999	4	L/360
BCLL: 0	Rep Mbr:	No	Web	: 0.07 (1-6)	Horz TL:	0 in			
BCDL: 10	Lumber D.O.1	L.:115 %							nn nn

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav U	plift Max MWFRS U	pliftMax C&C Uplift	Max Uplift	Max Horiz
1		90 lbs	110 plf		-49 lbs	-53 lbs	-53 lbs	-44 lbs

Material

TC: SYP#1 2 x 4 BC: SYP#1 2 x 4 Web: SYP#1 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.

2) This truss has not been designed for the effects of unbalanced snow loads.

3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL=1.60

Men	nber Forces	Table indicates: M	ember ID, max CSI, max axial force, (max con	apr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.
TC				
BC				
Web				_

- $1) \, Unless \, noted \, otherwise, do \, not \, cut \, or \, alter \, any \, truss \, member \, or \, plate \, \, without \, prior \, approval \, from \, a \, Professional \, Engineer.$
- 2) Gable requires continuous bottom chord bearing.
- 3) Gable webs placed at 16 "OC, U.N.O.
- 4) Attach gable webs with 1.5x4 20ga plates, U.N.O.
- 5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- 6) The fabrication tolerance for this roof truss is 30 % (Cq = 0.70).
- 7) Provide adequate drainage to prevent ponding.
- 8) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 9) Listed wind uplift reactions based on MWFRS & C&C loading.

Maysville, Missouri 64469

Truss:DT25

Job: 500 NW Chipman Rd REVISED

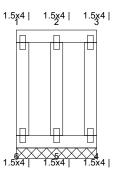
Date: 02/16/22 10:56:25

Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
1-10-8	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	15 lbs

1-10-8 1-10-8 1-10-8







All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI	Deflectio	n	L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2018/	TC: 0.09 (1-2)	Vert TL:	0 in UP	L/999	4	L/240
TCDL: 15		TPI 1-2014	BC: 0.00 (4-5)	Vert LL:	0 in	L/999	4	L/360
BCLL: 0	Rep Mbr:	No	Web: 0.10 (1-6)	Horz TL:	0 in			
BCDL: 10	Lumber D.O.1	L.:115 %						

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	t Max MWFRS Upli	iftMax C&C Uplift	Max Uplift	Max Horiz
1		93 lbs	124 plf	•	-73 lbs	-63 lbs	-73 lbs	-53 lbs

Material

TC: SYP#1 2 x 4 BC: SYP#1 2 x 4 Web: SYP#1 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

I nads

1) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following the user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.

2) This truss has not been designed for the effects of unbalanced snow loads.

3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL=1.60

Men	nber Forces	Table indicates: M	ember ID, max CSI, max axial force, (max con	apr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.
TC				
BC				
Web				

- $1) \, Unless \, noted \, otherwise, do \, not \, cut \, or \, alter \, any \, truss \, member \, or \, plate \, \, without \, prior \, approval \, from \, a \, Professional \, Engineer.$
- 2) Gable requires continuous bottom chord bearing.
- 3) Gable webs placed at 16 "OC, U.N.O.
- 4) Attach gable webs with 1.5x4 20ga plates, U.N.O.
- 5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- 6) The fabrication tolerance for this roof truss is 30% (Cq = 0.70).
- 7) Provide adequate drainage to prevent ponding.
- 8) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 9) Listed wind uplift reactions based on MWFRS & C&C loading.

Maysville, Missouri 64469

Truss:DT3

Job: 500 NW Chipman Rd REVISED

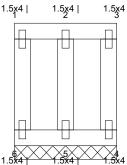
Date: 02/16/22 10:56:26

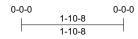
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
1-10-8	0/12	2	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	13 lbs

1-10-8 1-10-8 1-10-8







All plates shown to be Eagle 20 unless otherwise noted.

Lumber D.O.L.:115 %

Loading (psf)	General		CSI	Deflection	1	L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2018/	TC: 0.06 (1-2)	Vert TL:	0 in UP	L/999	4	L/240
TCDL: 15		TPI 1-2014	BC: 0.00 (4-5)	Vert LL:	0 in	L/999	4	L/360
BCLL: 0	Ren Mhr	No	Web: 0.07 (1-6)	Horz TL:	() in			

Reaction

BCDL: 10

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	tMax C&C Uplift	Max Uplift	Max Horiz
1		93 lbs	110 plf	•	-48 lbs	-53 lbs	-53 lbs	-44 lbs

Material

TC: SYP#1 2 x 4 BC: SYP#1 2 x 4 Web: SYP#1 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

1) This truss has been designed for the effects of balanced (20 pst) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.

2) This truss has not been designed for the effects of unbalanced snow loads.

3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL=1.60

Men	nber Forces	Table indicates: M	ember ID, max CSI, max axial force, (max con	npr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.
TC				
BC				
Web				

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Gable requires continuous bottom chord bearing.
- 3) Gable webs placed at 16 "OC, U.N.O.
- 4) Attach gable webs with 1.5x4 20ga plates, U.N.O.
- 5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- 6) The fabrication tolerance for this roof truss is 30% (Cq = 0.70).
- 7) Provide adequate drainage to prevent ponding.
- 8) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 9) Listed wind uplift reactions based on MWFRS & C&C loading.

P.O. Box 260 Maysville, Missouri 64469 Truss:DT4

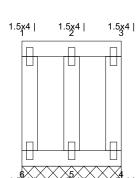
Job: 500 NW Chipman Rd REVISED

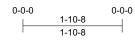
Date: 02/16/22 10:56:27

Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
1-10-8	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	13 lbs

1-10-8 1-10-8 1-10-8





All plates shown to be Eagle 20 unless otherwise noted.

Lumber D.O.L.:115 %

Loading (psf)	General		CSI	Deflection	n	L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2018/	TC: 0.07 (1-2)	Vert TL:	0 in UP	L/999	4	L/240
TCDL: 15		TPI 1-2014	BC: 0.00 (4-5)	Vert LL:	0 in	L/999	4	L/360
BCLL: 0	Ren Mhr	No	Web: 0.08 (1-6)	Horz TI:	() in			

Reaction

BCDL: 10

Brg Combo	Brg Width	Max React	Ave React	Max Grav Upl	lift Max MWFRS U	pliftMax C&C Uplift	Max Uplift	Max Horiz
1		93 lbs	110 plf		-53 lbs	-55 lbs	-55 lbs	46 lbs

Material

TC: SYP#1 2 x 4 BC: SYP#1 2 x 4 Web: SYP#1 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- 1) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shallverify snow loads.
- 2) This truss has not been designed for the effects of unbalanced snow loads.
- 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL=1.60

Men	nber Forces	Table indicates: M	ember ID, max CSI, max axial force, (max con	npr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.
TC				
BC				
Web				<u> </u>

- $1) \, Unless \, noted \, otherwise, do \, not \, cut \, or \, alter \, any \, truss \, member \, or \, plate \, \, without \, prior \, approval \, from \, a \, Professional \, Engineer.$
- 2) Gable requires continuous bottom chord bearing.
- 3) Gable webs placed at 16 "OC, U.N.O.
- 4) Attach gable webs with 1.5x4 20ga plates, U.N.O.
- 5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- 6) The fabrication tolerance for this roof truss is 30% (Cq = 0.70).
- 7) Provide adequate drainage to prevent ponding.
- 8) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 9) Listed wind uplift reactions based on MWFRS & C&C loading.

P.O. Box 260 Maysville, Missouri 64469 Truss:DT5

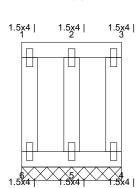
Job: 500 NW Chipman Rd REVISED

Date: 02/16/22 10:56:28

Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
1-10-8	0/12	3	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	13 lbs

1-10-8 1-10-8 1-10-8





All plates shown to be Eagle 20 unless otherwise noted.

Lumber D.O.L.:115 %

Loading (psf)	General		CSI	Deflection	1	L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2018/	TC: 0.06 (2-3)	Vert TL:	0 in UP	L/999	4	L/240
TCDL: 15		TPI 1-2014	BC: 0.00 (4-5)	Vert LL:	0 in	L/999	4	L/360
BCLL: 0	Rep Mbr:	Yes	Web: 0.07 (1-6)	Horz TL:	0 in			

Reaction

BCDL: 10

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift!	Max C&C Uplift	Max Uplift	Max Horiz
1		93 lbs	110 plf	•	-53 lbs	-55 lbs	-55 lbs	-46 lbs

Material

TC: SYP#1 2 x 4 BC: SYP#1 2 x 4 Web: SYP#1 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

I nads

- 1) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 2) This truss has not been designed for the effects of unbalanced snow loads.
- 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL=1.60

Men	nber Forces	Table indicates: M	ember ID, max CSI, max axial force, (max con	apr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.
TC				
BC				
Web				

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Gable requires continuous bottom chord bearing.
- 3) Gable webs placed at 16 "OC, U.N.O.
- 4) Attach gable webs with 1.5x4 20ga plates, U.N.O.
- 5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- 6) The fabrication tolerance for this roof truss is 30% (Cq = 0.70).
- 7) Provide adequate drainage to prevent ponding.
- 8) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 9) Listed wind uplift reactions based on MWFRS & C&C loading.

P.O. Box 260 Maysville, Missouri 64469 Truss:DT8

Job: 500 NW Chipman Rd REVISED

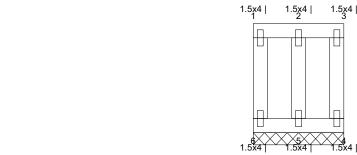
Date: 02/16/22 10:56:29

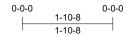
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
1-10-8	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	13 lbs

1-10-8 1-10-8 1-10-8







All plates shown to be Eagle 20 unless otherwise noted.

Lumber D.O.L.:115 %

Loading (psf)	General		CSI	Deflection	n	L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2018/	TC: 0.06 (1-2)	Vert TL:	0 in UP	L/999	4	L/240
TCDL: 15		TPI 1-2014	BC: 0.00 (4-5)	Vert LL:	0 in	L/999	4	L/360
BCLL: 0	Rep Mbr:	No	Web: 0.07 (1-6)	Horz TL:	0 in			no fe

Reaction

BCDL: 10

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplif	t Max MWFRS Up	liftMax C&C Uplift	Max Uplift	Max Horiz
1		93 lbs	110 plf		-48 lbs	-53 lbs	-53 lbs	-44 lbs

Material

TC: SYP#1 2 x 4 BC: SYP#1 2 x 4 Web: SYP#1 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

I nads

- 1) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 2) This truss has not been designed for the effects of unbalanced snow loads.
- 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL=1.60

Men	nber Forces	Table indicates: M	ember ID, max CSI, max axial force, (max con	npr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.
TC				
BC				
Web				<u> </u>

- $1) \, Unless \, noted \, otherwise, do \, not \, cut \, or \, alter \, any \, truss \, member \, or \, plate \, \, without \, prior \, approval \, from \, a \, Professional \, Engineer.$
- 2) Gable requires continuous bottom chord bearing.
- 3) Gable webs placed at 16 "OC, U.N.O.
- 4) Attach gable webs with 1.5x4 20ga plates, U.N.O.
- 5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- 6) The fabrication tolerance for this roof truss is 30% (Cq = 0.70).
- 7) Provide adequate drainage to prevent ponding.
- 8) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 9) Listed wind uplift reactions based on MWFRS & C&C loading.

P.O. Box 260 Maysville, Missouri 64469

Truss:DT9)
-----------	---

Job: 500 NW Chipman Rd REVISED

Date: 02/16/22 10:56:30

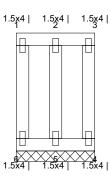
Page: 1 of 1

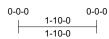
SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
1-10-0	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	15 lbs

1-10-0 1-10-0 1-10-0









All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI	Deflectio	n	L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2018/	TC: 0.09 (1-2)	Vert TL:	0 in UP	L/999	4	L/240
TCDL: 15		TPI 1-2014	BC: 0.00 (4-5)	Vert LL:	0 in	L/999	4	L/360
BCLL: 0	Rep Mbr:	No	Web: 0.10 (1-6)	Horz TL:	0 in			
BCDL: 10	Lumber D.O.1	[4:115%						

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav U	plift Max MWFRS Up	oliftMax C&C Uplift	Max Uplift	Max Horiz
1		90 lbs	128 plf		-75 lbs	-63 lbs	-75 lbs	-53 lbs

Material

TC: SYP#1 2 x 4 BC: SYP#1 2 x 4 Web: SYP#1 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

1) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.

2) This truss has not been designed for the effects of unbalanced snow loads.

3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL=1.60

Men	nber Forces	Table indicates: M	ember ID, max CSI, max axial force, (max con	npr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.
TC				
BC				
Web				

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Gable requires continuous bottom chord bearing.
- 3) Gable webs placed at 16 "OC, U.N.O.
- 4) Attach gable webs with 1.5x4 20ga plates, U.N.O.
- 5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- 6) The fabrication tolerance for this roof truss is 30% (Cq = 0.70).
- 7) Provide adequate drainage to prevent ponding.
- 8) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 9) Listed wind uplift reactions based on MWFRS & C&C loading.

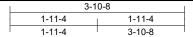
2550 Hwy 33 South P.O. Box 260 Maysville, Missouri 64469 Truss:GR1

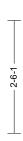
Job: 500 NW Chipman Rd REVISED

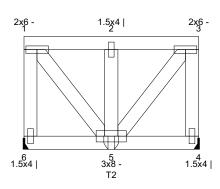
Date: 02/16/22 10:56:32

Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
3-10-8	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	15.75 in	27 lbs









All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflectio	n	L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code:	IBC 2018/	TC:	0.03 (1-2)	Vert TL:	0 in	L/999	(4-5)	L/240
TCLL: 20		TPI 1-2014	BC:	0.03 (5-6)	Vert LL:	0 in UP	L/999	5	L/360
TCDL: 15	Rep Mbr:	No	Web:	: 0.07 (1-5)	Horz TL:	0 in		4	02/16/
BCLL: 0	Lumber D.O.I	:115 %							92/10/

Reaction

BCDL: 10

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Up	olift Max MWFRS Up	oliftMax C&C Uplift	Max Uplift	Max Horiz
6	1	1.5 in		293 lbs	•	-56 lbs	-36 lbs	-56 lbs	-55 lbs \Xi
4	1	1.5 in		293 lbs		-90 lbs	-23 lbs	-90 lbs	. =

Material

TC: SYP#1 2 x 4 BC: SYP#1 2 x 4 Web: SYP#2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- $1) This truss has been designed for the effects due to \\10 psf bottom chord live load plus dead loads.$
- 2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shallverify snow loads.
- 3) This truss has not been designed for the effects of unbalanced snow loads.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL=1.60
- 5) Minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case Lr1: Std Live Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Тор	0-0-0	3-10-8	Down	Proi	26.25 plf	26,25 plf	

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Тор	0-0-0	3-10-8	Down	Proj	19.69 plf	19.69 plf	
Rot	0-0-0	3_10_8	Down	Proi	13 13 plf	13 13 nlf	

Member Forces
Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC		
BC		Ξ
Web		

Truss to Truss Connection Summary

Carried Truss	Carrying Chord	Carrying Offset
T2	BC	1-11-4

"minnun

P.O. Box 260 Maysville, Missouri 64469

OHR

0-0-0

CANTL

0-0-0

CANTR

0-0-0

Truss:GR1

Job: 500 NW Chipman Rd REVISED

Date: 02/16/22 10:56:32

Page: 2 of 2

PLYS SPACING WGT/PLY
1 15.75 in 27 lbs

Notes

SPAN

3-10-8

- $1) \, Unless \, noted \, otherwise, do \, not \, cut \, or \, alter \, any \, truss \, member \, or \, plate \, without \, prior \, approval \, from \, a \, Professional \, Engineer.$
- 2) The fabrication tolerance for this roof truss is 30% (Cq = 0.70).

PITCH

0/12

3) Hangers are for graphical intrepretation only. Install hangers per manufacturer's recommendations.

QTY

- 4) Provide adequate drainage to prevent ponding.
- 5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.

OHL

0-0-0

7) Listed wind uplift reactions based on MWFRS & C&C loading.

2550 Hwy 33 South P.O. Box 260 Maysville, Missouri 64469 Truss:GR10

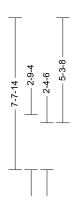
Job: 500 NW Chipman Rd REVISED

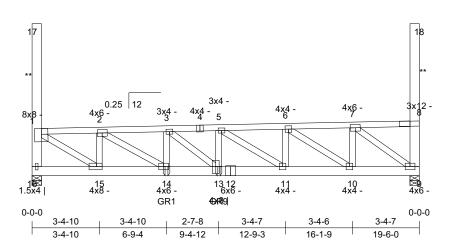
Date: 02/16/22 10:56:34

Page: 1 of 3

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
19-6-0	0.25/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	147 lbs

		1!	9-6-0		
3-4-10	3-4-10	2-7-8	3-4-7	3-4-6	3-4-7
3-4-10	6-9-4	9-4-12	12-9-3	16-1-9	19-6-0





All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf) General CSI Deflection L/ (loc) All	Allowe
	L/240
TCLL: 20 TPI 1-2014 BC: 0.33 (11-13) VertLL: 0.09 in L/999 (13-14) L/	L/360
TCDL: 15 Rep Mbr: No Web: 0.92 (17-1) Horz TL: 0.04 in 9	
BCLL: 0 Lumber D.O.L.:115 %	

BCDL: 10 Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplif	ft Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
16	1	5.5 in	1.65 in	1,637 lbs		•	-105 lbs	-105 lbs	490 lbs
9	1	5.5 in	1.59 in	1.572 lbs		-103 lbs	-94 lbs	-103 lbs	

Material

TC: SYP#1 2 x 4 BC: SYPSSDense 2 x 6 Web: SYP#2 2 x 4 except:

SYPSSDense 2 x 6: 17-16, 18-9

Bracing

TC: Sheathed or Purlins at 2-11-0, Purlin design by Others. BC: Sheathed or Purlins at 9-5-0, Purlin design by Others.

Loads

- 1) This truss has been designed for the effects due to 10 psf bottom chord live load plus dead loads.
- 2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 3) This truss has not been designed for the effects of unbalanced snow loads.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL=1.60
- 5) Minimum storage attic loading has been applied in accordance with IBC 1607.1
- 6) **- Indicates parapet wind loading has been applied to members 17-1 & 18-8

Load Case Lr1: Std Live Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Тор	0-0-0	6-9-4	Down	Proj	20 plf	20 plf	
Тор	6-9-4	9-4-12	Down	Proj	20 plf	20 plf	
Тор	9-4-12	19-6-0	Down	Proj	20 plf	20 plf	
Тор	0-0-0	19-6-0	Down	Proj	20 plf	20 plf	

2550 Hwy 33 South P.O. Box 260 Maysville, Missouri 64469 Truss:GR10

Job: 500 NW Chipman Rd REVISED

Date: 02/16/22 10:56:34

Page: 2 of 3

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
19-6-0	0.25/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	147 lbs

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Тор	13-10-0	16-0-0	Down	Proj	46.15 plf	46.15 plf	
Тор	0-0-0	6-9-4	Down	Proj	15 plf	15 plf	
Тор	6-9-4	9-4-12	Down	Proj	15 plf	15 plf	
Тор	9-4-12	19-6-0	Down	Proj	15 plf	15 plf	
Тор	0-0-0	19-6-0	Down	Proj	15 plf	15 plf	
Bot	0-0-0	6-9-4	Down	Proj	10 plf	10 plf	
Bot	6-9-4	9-4-12	Down	Proj	10 plf	10 plf	
Bot	9-4-12	19-6-0	Down	Proj	10 plf	10 plf	
Bot	0-0-0	19-6-0	Down	Proj	10 plf	10 plf	

User-defined Load Case S2: Drift

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Тор	1-0-12	6-9-4	Down	Proj	20 plf	20 plf	
Top	1-0-12	6-9-4	Down	Proj	20 plf	20 plf	
Top	6-9-4	9-4-12	Down	Proj	20 plf	20 plf	
Top	9-4-12	18-5-4	Down	Proj	20 plf	20 plf	
Тор	9-4-12	18-5-4	Down	Proj	20 plf	20 plf	
Top	0-0-0	4-6-0	Down	Proj	32 psf	0 psf	24 in
Тор	15-0-0	19-6-0	Down	Proj	0 psf	32 psf	24 in
Point Loads							
Member	Location	Direction	Load	Trib Width			

6-9-4 120 lbs Bot Down Bot 9-4-12 Down 153 lbs

Load Combinations

#	Load Combo	Factor
1	D1	1.000
2	Dl +Lrl	1.000
3	D1+S1	1.000
4	D1 +S2	1.000
5	0.60 D1 +0.60 W1 [Uplift]	1.000
6	0.60 D1 +0.60 W2 [Uplift]	1.000
7	0.60 D1 +0.60 W4 [Uplift]	1.000
8	0.60 D1 +0.60 W8 [Uplift]	1.000
9	D1 +L10*1	1.000
10	D1 +Lr1 +L10*1	1.000
11	D1+S1+L10*1	1.000
12	D1 +S2 +L10*1	1.000
13	D1 +UR1	1.000
14	D1 + UR2	1.000
15	D1 +AS10*1	1.000
16	D1 +Lr1 +AS10*1	1.000
17	D1+S1+AS10*1	1.000
18	D1+S2+AS10*1	1.000

Men	nber 1	Forces	Table Table	indicates: M	ember II), max CS	I, max axial fo	rce, (max con	npr. force	if differen	nt from max ax	ial force). Onl	y forces greater than	300lbs are show	wn in this	table
TC	1-2	0.247	-2,187 lbs		3-5	0.401	-3,917 lbs		6-7	0.223	-1,850 lbs					
	2-3	0.375	-3,608 lbs		5-6	0.329	-3,167 lbs		7-8	0.191	1,066 lbs	(-500 lbs)				
BC	9-10	0.154	1,847 lbs	(-598 lbs)	11-13	0.329	3,915 lbs	(-667 lbs)	14-15	0.216	2,184 lbs	(-802 lbs)				
	10-11	0.256	3,164 lbs	(-673 lbs)	13-14	0.312	3,604 lbs	(-711 lbs)	15-16	0.044	804 lbs	(-747 lbs)				
Web	1-16	0.639	-1,557 lbs		3-14	0.058	-442 lbs		6-11	0.155	634 lbs	(-129 lbs)				
	1-15	0.634	2,583 lbs	(-66 lbs)	3-13	0.098	401 lbs	(-143 lbs)	6-10	0.438	-1,586 lbs					
	2-15	0.160	-1,231 lbs		5-13	0.081	329 lbs	(-133 lbs)	7-10	0.248	1,010 lbs	(-103 lbs)				
	2-14	0.408	1,663 lbs		5-11	0.243	-895 lbs		7-9	0.572	-2,298 lbs					

1,663 lbs **Truss to Truss Connection Summary**

Carried Truss	Carrying Chord	Carrying Offset
GR1	BC	6-9-4
GR9	BC	9-4-12

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.

- 2) The fabrication tolerance for this roof truss is 30% (Cq = 0.70).
- 3) Provide adequate drainage to prevent ponding.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to
- the plane of the truss. Lateral braces shall be installed within 6 "of each web panel point.
- 6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.

2550 Hwy 33 South P.O. Box 260 Maysville, Missouri 64469 Truss:GR10

Job: 500 NW Chipman Rd REVISED

Date: 02/16/22 10:56:34

Page: 3 of 3

SPAN PITCH QTY OHL OHR CANTL CANTR PLYS **SPACING** WGT/PLY 0-0-0 19-6-0 0.25/12 0-0-0 0-0-0 0-0-0 1 24 in 147 lbs

7) Listed wind uplift reactions based on MWFRS & C&C loading.

8) Parapet TL: 0.83 in, 2L/158 (17-1), Allowable 2L/120.

2550 Hwy 33 South P.O. Box 260 Maysville, Missouri 64469 Truss:GR11

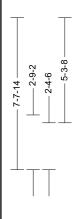
Job: 500 NW Chipman Rd REVISED

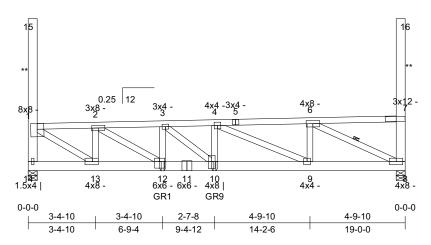
Date: 02/16/22 10:56:36

Page: 1 of 3

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
19-0-0	0.25/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	141 lbs

			-0-0	
3-4-10	3-4-10	2-7-8	4-9-10	4-9-10
3-4-10	6-9-4	9-4-12	14-2-6	19-0-0





All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI	Deflectio	n	L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code:	IBC 2018/	TC: 0.44 (4-6)	Vert TL:	0.21 in	L/999	(9-10)	L/240
TCLL: 20	_	TPI 1-2014	BC: 0.33 (9-10)	Vert LL:	0.09 in	L/999	10	L/360
TCDL: 15	Rep Mbr:	No	Web: 0.92 (15-1)	Horz TL:	0.04 in		8	
BCLL: 0	Lumber DO	L:115 %						

BCDL: 10 Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Up	oliftMax C&C Uplift	Max Uplift
14	1	5.5 in	1.62 in	1,599 lbs	•	•	-106 lbs	-106 lbs
8	1	5.5 in	1.59 in	1,577 lbs		-78 lbs	-87 lbs	-87 lbs

Bracing

TC: Sheathed or Purlins at 3-0-0, Purlin design by Others. BC: Sheathed or Purlins at 9-5-0, Purlin design by Others.

Web: One Midpoint Row: 6-8

Material TO SYD#

TC: SYP#1 2 x 4
BC: SYPSSDense 2 x 6
Web: SYP#2 2 x 4 except:
SYPSSDense 2 x 6: 15-14, 16-8

Loads

1) This truss has been designed for the effects due to 10 psf bottom chord live load plus dead loads.

2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.

3) This truss has not been designed for the effects of unbalanced snow loads.

4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL=1.60

5) Minimum storage attic loading has been applied in accordance with IBC 1607.1

6) ** - Indicates parapet wind loading has been applied to members 15-1 & 16-7

Load Case Lr1: Std Live Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	TribWidth
Тор	0-0-0	19-0-0	Down	Proj	20 plf	20 plf	
Тор	0-0-0	6-9-4	Down	Proj	20 plf	20 plf	
Top	6-9-4	9-4-12	Down	Proj	20 plf	20 plf	
Top	9-4-12	19-0-0	Down	Proj	20 plf	20 plf	

2550 Hwy 33 South P.O. Box 260 Maysville, Missouri 64469 Truss:GR11

Job: 500 NW Chipman Rd REVISED

Date: 02/16/22 10:56:36

Page: 2 of 3

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
19-0-0	0.25/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	141 lbs

Load	Case.	D.	1:St	dΕ	Dead	Load	
------	-------	----	------	----	------	------	--

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	13-10-0	16-0-0	Down	Proj	46.15 plf	46.15 plf	
Тор	0-0-0	19-0-0	Down	Proj	15 plf	15 plf	
Top	0-0-0	6-9-4	Down	Proj	15 plf	15 plf	
Top	6-9-4	9-4-12	Down	Proj	15 plf	15 plf	
Top	9-4-12	19-0-0	Down	Proj	15 plf	15 plf	
Bot	0-0-0	19-0-0	Down	Proj	10 plf	10 plf	
Bot	0-0-0	6-9-4	Down	Proj	10 plf	10 plf	
Bot	6-9-4	9-4-12	Down	Proj	10 plf	10 plf	
Bot	9-4-12	19-0-0	Down	Proj	10 plf	10 plf	

User-defined Load Case S2: Drift

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Тор	1-0-12	6-9-4	Down	Proj	20 plf	20 plf	
Тор	1-0-12	6-9-4	Down	Proj	20 plf	20 plf	
Top	6-9-4	9-4-12	Down	Proj	20 plf	20 plf	
Top	9-4-12	17-11-4	Down	Proj	20 plf	20 plf	
Тор	9-4-12	17-11-4	Down	Proj	20 plf	20 plf	
Top	17-11-4	19-0-0	Down	Proj	20 plf	20 plf	
Top	0-0-0	4-6-0	Down	Proj	32 psf	0 psf	24 in
Тор	14-6-0	19-0-0	Down	Proj	0 psf	32 psf	24 in
Point Loads							
Member	Location	Direction	Load	Trib Width			
Bot	6-9-4	Down	120 lbs				_

Bot 9-4-12 Load Combinations

#	Load Combo	Factor
1	D1	1.000
2	Dl +Lrl	1.000
3	D1+S1	1.000
4	D1+S2	1.000
5	0.60 D1 +0.60 W1 [Uplift]	1.000
6	0.60 D1 +0.60 W2 [Uplift]	1.000
7	0.60 D1 +0.60 W4 [Uplift]	1.000
8	0.60 D1 +0.60 W8 [Uplift]	1.000
9	D1 +L10*1	1.000
10	D1 +Lr1 +L10*1	1.000
11	D1+S1+L10*1	1.000
12	D1+S2+L10*1	1.000
13	D1+UR1	1.000
14	D1+UR2	1.000
15	D1 +AS10*1	1.000
16	D1 +Lr1 +AS10*1	1.000
17	D1+S1+AS10*1	1.000
18	D1+S2+AS10*1	1.000

153 lbs

Men	nber]	Forces	Table	Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.										
TC	1-2	0.243	-2,130 lbs		3-4	0.387	-3,777 lbs		6-7	0.425	1,071 lbs	(-504 lbs)	1	l
	2-3	0.362	-3,491 lbs		4-6	0.440	-2,528 lbs]
BC	8-9	0.221	2,525 lbs	(-641 lbs)	10-12	0.299	3,487 lbs	(-707 lbs)	13-14	0.044	805 lbs	(-745 lbs)		
	9-10	0.334	3,775 lbs	(-670 lbs)	12-13	0.211	2,126 lbs	(-802 lbs)						J
Web	1-14	0.639	-1,519 lbs		3-12	0.054	-411 lbs		6-9	0.190	775 lbs	(-81 lbs)		Ì
	1-13	0.617	2,515 lbs	(-67 lbs)	3-10	0.091	371 lbs	(-166 lbs)	6-8	0.437	-2,822 lbs			
	2-13	0.155	-1,195 lbs		4-10	0.094	382 lbs	(-95 lbs)						
	2-12	0.391	1.592 lbs	(-39 lbs)	4-9	0.642	-1.374 lbs		I				ĺ	

Truss to Truss Connection Summary

Carried Truss	Carrying Chord	Carrying Offset
GR1	BC	6-9-4
GR9	BC	9-4-12

- $1) \, Unless \, noted \, otherwise, do \, not \, cut \, or \, alter \, any \, truss \, member \, or \, plate \, without \, prior \, approval \, from \, a \, Professional \, Engineer.$
- 2) The fabrication tolerance for this roof truss is 30% (Cq = 0.70).
- 3) Provide adequate drainage to prevent ponding.

2550 Hwy 33 South P.O. Box 260 Maysville, Missouri 64469 Truss:GR11

Job: 500 NW Chipman Rd REVISED

Date: 02/16/22 10:56:36

Page: 3 of 3

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
19-0-0	0.25/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	141 lbs

- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 "of each web panel point.
- 6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 7) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 8) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
 9) Listed wind uplift reactions based on MWFRS & C&C loading.
- 10) Parapet TL: 0.82 in, 2L/158 (15-1), Allowable 2L/120.

2550 Hwy 33 South P.O. Box 260 Maysville, Missouri 64469 Truss:GR12

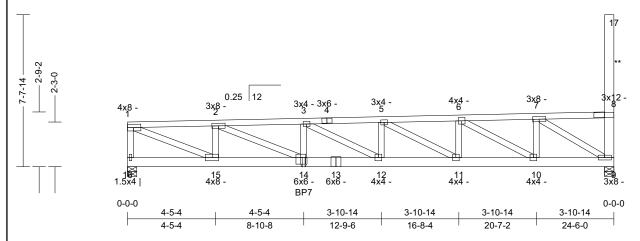
Job: 500 NW Chipman Rd REVISED

Date: 02/16/22 10:56:38

Page: 1 of 3

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
24-6-0	0.25/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	18.25 in	156 lbs
				24.6	Λ				

		24-6-0)			
4-5-4	4-5-4	3-10-14	3-10-14	3-10-14	3-10-14	Τ
4-5-4	8-10-8	12-9-6	16-8-4	20-7-2	24-6-0	



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI	Deflectio	n	L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code:	IBC 2018/	TC: 0.37 (3-5)	Vert TL:	0.29 in	L/970	(12-13)	L/240
TCLL: 20		TPI 1-2014	BC: 0.30 (12-14)	Vert LL:	0.11 in	L/999	(12-13)	L/360
TCDL: 15	Rep Mbr:	No	Web: 0.59 (1-15)	Horz TL:	0.04 in		9	
BCLL: 0	Lumber DO	L :115 %						

BCDL: 10 Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplif	ft Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
16	1	5.5 in	1.50 in	1,184 lbs		•	-211 lbs	-211 lbs	390 lbs
9	1	5.5 in	1.50 in	1.173 lbs		-9 lbs	-70 lbs	-70 lbs	

Material

TC: SYP#1 2 x 4 BC: SYPSSDense 2 x 6 Web: SYP#2 2 x 4 except: SYPSSDense 2 x 6: 17-9

Bracing

TC: Sheathed or Purlins at 2-11-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects due to 10 psf bottom chord live load plus dead loads.

2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used $for the \, roof \, snow \, load, \, DOL = 1.15. \,\, If \, the \, roof \, configuration \, differs \, from \, hip/gable, \, Build \, ing \, Designer \, shall \, verify \, snow \, loads \,.$

3) This truss has not been designed for the effects of unbalanced snow loads.

4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60

5) Minimum storage attic loading has been applied in accordance with IBC 1607.1

6) **- Indicates parapet wind loading has been applied to member 17-8

Load Case Lr1: Std Live Load

Distributed Load	ls						
Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Тор	0-0-0	7-10-8	Down	Proj	20 plf	20 plf	
Top	8-10-8	24-6-0	Down	Proj	20 plf	20 plf	
Тор	0-0-0	5-6-0	Down	Proj	10.42 plf	10.42 plf	
Top	5-6-0	24-6-0	Down	Proj	10 plf	10 plf	

2550 Hwy 33 South P.O. Box 260

Truss:GR12

Job: 500 NW Chipman Rd REVISED
Date: 02/16/22 10:56:38

				May	P.O. Box 26 sville, Missou				Date: 02/16/22 10:56:30 Page: 2 of 3	8
SPAN 24-6-0	PITCH 0.25/12		TY 1	OHL 0-0-0	OHR 0-0-0	CANTL 0-0-0	CANTR 0-0-0	PLYS 1	S SPACING 18.25 in	WGT/PLY 156 lbs
Load Case D1:	Std Dead Load									
Distributed Loads										
Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width			
Тор	0-0-0	7-10-8	Down	Proj	15 plf	15 plf				
Тор	8-10-8	24-6-0	Down	Proj	15 plf	15 plf				
	0-0-0	5-6-0	Down		7.81 plf	7.81 plf				
Top				Proj						
Тор	5-6-0	24-6-0	Down	Proj	7.5 plf	7.5 plf				
Bot	0-0-0	7-10-8	Down	Proj	10 plf	10 plf				
Bot	8-10-8	24-6-0	Down	Proj	10 plf	10 plf				
Bot	0-0-0	5-6-0	Down	Proj	5.21 plf	5.21 plf				
Bot	5-6-0	24-6-0	Down	Proj	5 plf	5 plf				
User-defined Lo	oad Case S2: Sno	w Drift		v	·	•				
Distributed Loads										
	Location 1	I continu 2	Dimention	Commond	Ctout I and	End Lood	T XX 2.44.			
Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width			
Top	9-0-0	13-6-0	Down	Proj	32 psf	32 psf	24 in			
Top	20-0-0	24-6-0	Down	Proj	32 psf	32 psf	24 in			
Point Loads										
Member	Location	Direction	Load	Trib Width						
	ad Case W12: W			ALLO TERMI						
Distributed Loads	ad Case W 12. W	/1(DOL-1.0	<i>.</i> (0)							
Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width			
Top	0-0-0	1-0-12	NUp	Rake	25.39 plf	25.39 plf	THO THOU			
Top	1-0-12	5-6-0	N Up	Rake	25.39 plf	25.39 plf				
Top	1-0-12	5-6-0	N Up	Rake	25.39 plf	25.39 plf				
Top	5-6-0	7-6-0	N Up	Rake	25.39 plf	25.39 plf				
Top	5-6-0	7-6-0	NUp	Rake	12.7 plf	12.7 plf				
Top	7-6-0	7-10-8	N Up	Rake	25.39 plf	25.39 plf				
Top	7-6-0	7-10-8	N Up	Rake	12.7 plf	12.7 plf				
Top	7-10-8	9-10-8	N Up	Rake	12.7 plf	12.7 plf				
Тор	9-10-8	15-0-0	N Up	Rake	25.39 plf	25.39 plf				
Top	9-10-8	15-0-0	N Up	Rake	12.7 plf	12.7 plf				
Top	15-0-0	23-5-4	NUp	Rake	15.97 plf	15.97 plf				
Top	15-0-0	23-5-4	N Up	Rake	7.98 plf	7.98 plf				
Web 1-16	0-0-0	2-3-0	Right	Rake	12.74 plf	12.74 plf				
Point Loads			Ü		1	1				
Member	Location	Direction	Load	Trib Width						
			44 lbs							
	8-10-8	Down								
Bot Top	0-0-0	Right	600 lbs							
Bot Top		Right	600 lbs							
Bot Top User-defined Lo Distributed Loads	0-0-0 oad Case W13: W	Right //2(DOL = 1.6	600 lbs	Cor. 1	Charle I	E. II	TD.31, XX 14			
Bot Top User-defined Lo Distributed Loads Member	0-0-0 and Case W13: W Location 1	Right 72(DOL = 1.6 Location 2	600 lbs 60) Direction	Spread	Start Load	End Load	Trib Width			
Bot Top User-defined Lo Distributed Loads Member Top	0-0-0 and Case W13: W Location 1 0-0-0	Right $V2(DOL = 1.6)$ Location 2 1-0-12	600 lbs 60) Direction N Up	Rake	15.97 plf	15.97 plf	Trib Width			
Bot Top User-defined Lo Distributed Loads Member Top Top	0-0-0 and Case W13: W Location 1 0-0-0 1-0-12	Right /2 (DOL = 1.6 Location 2 1-0-12 5-6-0	600 lbs 60) Direction N Up N Up	Rake Rake	15.97 plf 15.97 plf	15.97 plf 15.97 plf	Trib Width			
Bot Top User-defined Lo Distributed Loads Member Top Top	0-0-0 and Case W13: W Location 1 0-0-0 1-0-12 1-0-12	Right /2(DOL= 1.6) Location 2 1-0-12 5-6-0 5-6-0	600 lbs 60) Direction N Up N Up N Up N Up	Rake Rake Rake	15.97 plf 15.97 plf 15.97 plf	15.97 plf 15.97 plf 15.97 plf	Trib Width			
Bot Top User-defined Lo Distributed Loads Member Top Top Top Top	0-0-0 and Case W13: W Location 1 0-0-0 1-0-12 1-0-12 5-6-0	Right /2(DOL= 1.6 Location 2 1-0-12 5-6-0 5-6-0 7-10-8	600 lbs 60) Direction N Up N Up N Up N Up N Up N Up	Rake Rake Rake Rake	15.97 plf 15.97 plf 15.97 plf 15.97 plf	15.97 plf 15.97 plf 15.97 plf 15.97 plf	Trib Width			
Bot Top User-defined Lo Distributed Loads Member Top Top Top Top Top	0-0-0 and Case W13: W Location 1 0-0-0 1-0-12 1-0-12 5-6-0 5-6-0	Right /2(DOL=1.6) Location 2 1-0-12 5-6-0 7-10-8 7-10-8	600 lbs 50) Direction NUp NUp NUp NUp NUp NUp NUp NU	Rake Rake Rake Rake Rake	15.97 plf 15.97 plf 15.97 plf 15.97 plf 7.98 plf	15.97 plf 15.97 plf 15.97 plf 15.97 plf 7.98 plf	Trib Width			
Bot Top User-defined Lo Distributed Loads Member Top Top Top Top Top	0-0-0 and Case W13: W Location 1 0-0-0 1-0-12 1-0-12 5-6-0 5-6-0 7-10-8	Right /2(DOL= 1.6 Location 2 1-0-12 5-6-0 5-6-0 7-10-8	600 lbs 60) Direction N Up N Up N Up N Up N Up N Up	Rake Rake Rake Rake	15.97 plf 15.97 plf 15.97 plf 15.97 plf 7.98 plf 7.98 plf	15.97 plf 15.97 plf 15.97 plf 15.97 plf	Trib Width			
Bot Top User-defined Lo Distributed Loads Member Top Top Top Top Top Top Top Top	0-0-0 and Case W13: W Location 1 0-0-0 1-0-12 1-0-12 5-6-0 5-6-0	Right /2(DOL=1.6) Location 2 1-0-12 5-6-0 7-10-8 7-10-8	600 lbs 50) Direction NUp NUp NUp NUp NUp NUp NUp NU	Rake Rake Rake Rake Rake	15.97 plf 15.97 plf 15.97 plf 15.97 plf 7.98 plf 7.98 plf	15.97 plf 15.97 plf 15.97 plf 15.97 plf 7.98 plf	Trib Width			
Bot Top User-defined Lo Distributed Loads Member Top	0-0-0 and Case W13: W Location 1 0-0-0 1-0-12 1-0-12 5-6-0 5-6-0 7-10-8 9-6-0	Right /2(DOL=1.6) Location 2 1-0-12 5-6-0 5-6-0 7-10-8 7-10-8 9-6-0 9-10-8	600 lbs Direction NUp NUp NUp NUp NUp NUp NUp NU	Rake Rake Rake Rake Rake Rake Rake	15.97 plf 15.97 plf 15.97 plf 15.97 plf 15.97 plf 7.98 plf 7.98 plf 12.7 plf	15.97 plf 15.97 plf 15.97 plf 15.97 plf 15.97 plf 7.98 plf 7.98 plf 12.7 plf	Trib Width			
Bot Top User-defined Lo Distributed Loads Member Top	0-0-0 vad Case W13: W Location 1 0-0-0 1-0-12 1-0-12 5-6-0 5-6-0 7-10-8 9-6-0 9-10-8	Right /2(DOL=1.6) Location 2 1-0-12 5-6-0 5-6-0 7-10-8 7-10-8 9-6-0 9-10-8 17-0-0	600 lbs Direction NUp NUp NUp NUp NUp NUp NUp NU	Rake Rake Rake Rake Rake Rake Rake	15.97 plf 15.97 plf 15.97 plf 15.97 plf 15.97 plf 7.98 plf 7.98 plf 12.7 plf 25.39 plf	15.97 plf 15.97 plf 15.97 plf 15.97 plf 15.97 plf 7.98 plf 7.98 plf 12.7 plf 25.39 plf	Trib Width			
Bot Top User-defined Lo Distributed Loads Member Top	0-0-0 and Case W13: W Location 1 0-0-0 1-0-12 1-0-12 5-6-0 5-6-0 7-10-8 9-6-0 9-10-8 9-10-8	Right /2(DOL=1.6) Location 2 1-0-12 5-6-0 5-6-0 7-10-8 7-10-8 9-6-0 9-10-8 17-0-0 17-0-0	600 lbs Direction NUp NUp NUp NUp NUp NUp NUp NU	Rake Rake Rake Rake Rake Rake Rake Rake	15.97 plf 15.97 plf 15.97 plf 15.97 plf 15.97 plf 7.98 plf 7.98 plf 12.7 plf 25.39 plf 12.7 plf	15.97 plf 15.97 plf 15.97 plf 15.97 plf 7.98 plf 7.98 plf 12.7 plf 25.39 plf 12.7 plf	Trib Width			
Bot Top User-defined Lo Distributed Loads Member Top	0-0-0 and Case W13: W Location 1 0-0-0 1-0-12 1-0-12 5-6-0 5-6-0 7-10-8 9-6-0 9-10-8 9-10-8 17-0-0	Right /2(DOL=1.6 Location 2 1-0-12 5-6-0 5-6-0 7-10-8 7-10-8 9-6-0 9-10-8 17-0-0 17-0-0 23-5-4	600 lbs Direction NUp NUp NUp NUp NUp NUp NUp NU	Rake Rake Rake Rake Rake Rake Rake Rake	15.97 plf 15.97 plf 15.97 plf 15.97 plf 15.97 plf 7.98 plf 7.98 plf 12.7 plf 25.39 plf 12.7 plf 25.39 plf	15.97 plf 15.97 plf 15.97 plf 15.97 plf 15.97 plf 7.98 plf 7.98 plf 12.7 plf 25.39 plf 12.7 plf 25.39 plf	Trib Width			
Bot Top User-defined Lo Distributed Loads Member Top	0-0-0 and Case W13: W Location 1 0-0-0 1-0-12 1-0-12 5-6-0 5-6-0 7-10-8 9-6-0 9-10-8 9-10-8 17-0-0 17-0-0	Right /2(DOL = 1.6) Location 2 1-0-12 5-6-0 5-6-0 7-10-8 7-10-8 9-6-0 9-10-8 17-0-0 17-0-0 23-5-4 23-5-4	600 lbs Direction NUp NUp NUp NUp NUp NUp NUp NU	Rake Rake Rake Rake Rake Rake Rake Rake	15.97 plf 15.97 plf 15.97 plf 15.97 plf 15.97 plf 7.98 plf 7.98 plf 12.7 plf 25.39 plf 12.7 plf 25.39 plf 12.7 plf	15.97 plf 15.97 plf 15.97 plf 15.97 plf 15.97 plf 7.98 plf 7.98 plf 12.7 plf 25.39 plf 12.7 plf 25.39 plf 12.7 plf	Trib Width			
Bot Top User-defined Lo Distributed Loads Member Top	0-0-0 and Case W13: W Location 1 0-0-0 1-0-12 1-0-12 5-6-0 5-6-0 7-10-8 9-6-0 9-10-8 9-10-8 17-0-0	Right /2(DOL=1.6 Location 2 1-0-12 5-6-0 5-6-0 7-10-8 7-10-8 9-6-0 9-10-8 17-0-0 17-0-0 23-5-4	600 lbs Direction NUp NUp NUp NUp NUp NUp NUp NU	Rake Rake Rake Rake Rake Rake Rake Rake	15.97 plf 15.97 plf 15.97 plf 15.97 plf 15.97 plf 7.98 plf 7.98 plf 12.7 plf 25.39 plf 12.7 plf 25.39 plf	15.97 plf 15.97 plf 15.97 plf 15.97 plf 15.97 plf 7.98 plf 7.98 plf 12.7 plf 25.39 plf 12.7 plf 25.39 plf	Trib Width			
Bot Top User-defined Lo Distributed Loads Member Top	0-0-0 and Case W13: W Location 1 0-0-0 1-0-12 1-0-12 5-6-0 7-10-8 9-6-0 9-10-8 9-10-8 17-0-0 17-0-0 0-0-0	Right /2(DOL = 1.6 Location 2 1-0-12 5-6-0 5-6-0 7-10-8 7-10-8 9-6-0 9-10-8 17-0-0 17-0-0 23-5-4 23-5-4 2-3-0	600 lbs Direction NUp NUp NUp NUp NUp NUp NUp NU	Rake Rake Rake Rake Rake Rake Rake Rake	15.97 plf 15.97 plf 15.97 plf 15.97 plf 15.97 plf 7.98 plf 7.98 plf 12.7 plf 25.39 plf 12.7 plf 25.39 plf 12.7 plf	15.97 plf 15.97 plf 15.97 plf 15.97 plf 15.97 plf 7.98 plf 7.98 plf 12.7 plf 25.39 plf 12.7 plf 25.39 plf 12.7 plf	Trib Width			
Bot Top User-defined Loads Member Top	0-0-0 and Case W13: W Location 1 0-0-0 1-0-12 1-0-12 5-6-0 5-6-0 7-10-8 9-6-0 9-10-8 9-10-8 17-0-0 17-0-0 0-0-0	Right /2(DOL = 1.6) Location 2 1-0-12 5-6-0 5-6-0 7-10-8 7-10-8 9-6-0 9-10-8 17-0-0 17-0-0 23-5-4 23-5-4 2-3-0 Direction	600 lbs Direction NUp NUp NUp NUp NUp NUp NUp NU	Rake Rake Rake Rake Rake Rake Rake Rake	15.97 plf 15.97 plf 15.97 plf 15.97 plf 15.97 plf 7.98 plf 7.98 plf 12.7 plf 25.39 plf 12.7 plf 25.39 plf 12.7 plf	15.97 plf 15.97 plf 15.97 plf 15.97 plf 15.97 plf 7.98 plf 7.98 plf 12.7 plf 25.39 plf 12.7 plf 25.39 plf 12.7 plf	Trib Width			
Bot Top User-defined Loads Member Top	0-0-0 and Case W13: W Location 1 0-0-0 1-0-12 1-0-12 5-6-0 7-10-8 9-6-0 9-10-8 9-10-8 17-0-0 17-0-0 0-0-0 Location 8-10-8	Right /2(DOL=1.6) Location 2 1-0-12 5-6-0 5-6-0 7-10-8 7-10-8 9-6-0 9-10-8 17-0-0 17-0-0 23-5-4 23-5-4 2-3-0 Direction Up	600 lbs Direction NUp NUp NUp NUp NUp NUp NUp NU	Rake Rake Rake Rake Rake Rake Rake Rake	15.97 plf 15.97 plf 15.97 plf 15.97 plf 15.97 plf 7.98 plf 7.98 plf 12.7 plf 25.39 plf 12.7 plf 25.39 plf 12.7 plf	15.97 plf 15.97 plf 15.97 plf 15.97 plf 15.97 plf 7.98 plf 7.98 plf 12.7 plf 25.39 plf 12.7 plf 25.39 plf 12.7 plf	Trib Width			
Bot Top User-defined Loads Member Top	0-0-0 and Case W13: W Location 1 0-0-0 1-0-12 1-0-12 5-6-0 7-10-8 9-6-0 9-10-8 9-10-8 17-0-0 17-0-0 0-0-0 Location 8-10-8 0-0-0	Right /2(DOL = 1.6 Location 2 1-0-12 5-6-0 5-6-0 7-10-8 7-10-8 9-6-0 9-10-8 17-0-0 17-0-0 23-5-4 23-5-4 2-3-0 Direction Up Left	Direction NUp NUp NUp NUp NUp NUp NUp NU	Rake Rake Rake Rake Rake Rake Rake Rake	15.97 plf 15.97 plf 15.97 plf 15.97 plf 15.97 plf 7.98 plf 7.98 plf 12.7 plf 25.39 plf 12.7 plf 25.39 plf 12.7 plf	15.97 plf 15.97 plf 15.97 plf 15.97 plf 15.97 plf 7.98 plf 7.98 plf 12.7 plf 25.39 plf 12.7 plf 25.39 plf 12.7 plf	Trib Width			
Bot Top User-defined Lo Distributed Loads Member Top	0-0-0 and Case W13: W Location 1 0-0-0 1-0-12 1-0-12 5-6-0 7-10-8 9-6-0 9-10-8 9-10-8 17-0-0 17-0-0 0-0-0 Location 8-10-8	Right /2(DOL = 1.6 Location 2 1-0-12 5-6-0 5-6-0 7-10-8 7-10-8 9-6-0 9-10-8 17-0-0 17-0-0 23-5-4 23-5-4 2-3-0 Direction Up Left	Direction NUp NUp NUp NUp NUp NUp NUp NU	Rake Rake Rake Rake Rake Rake Rake Rake	15.97 plf 15.97 plf 15.97 plf 15.97 plf 15.97 plf 7.98 plf 7.98 plf 12.7 plf 25.39 plf 12.7 plf 25.39 plf 12.7 plf	15.97 plf 15.97 plf 15.97 plf 15.97 plf 15.97 plf 7.98 plf 7.98 plf 12.7 plf 25.39 plf 12.7 plf 25.39 plf 12.7 plf	Trib Width			
Bot Top User-defined Loads Member Top	0-0-0 and Case W13: W Location 1 0-0-0 1-0-12 1-0-12 5-6-0 7-10-8 9-6-0 9-10-8 9-10-8 17-0-0 17-0-0 0-0-0 Location 8-10-8 0-0-0 and Case W14: W	Right /2(DOL = 1.6) Location 2 1-0-12 5-6-0 5-6-0 7-10-8 7-10-8 9-6-0 9-10-8 17-0-0 17-0-0 23-5-4 23-5-4 2-3-0 Direction Up Left /3(DOL = 1.6)	600 lbs Direction NUp NUp NUp NUp NUp NUp NUp NU	Rake Rake Rake Rake Rake Rake Rake Rake	15.97 plf 15.97 plf 15.97 plf 15.97 plf 15.97 plf 7.98 plf 7.98 plf 12.7 plf 25.39 plf 12.7 plf 25.39 plf 12.7 plf	15.97 plf 15.97 plf 15.97 plf 15.97 plf 15.97 plf 7.98 plf 7.98 plf 12.7 plf 25.39 plf 12.7 plf 25.39 plf 12.7 plf	Trib Width			
Bot Top User-defined Lo Distributed Loads Member Top	0-0-0 and Case W13: W Location 1 0-0-0 1-0-12 1-0-12 5-6-0 7-10-8 9-6-0 9-10-8 9-10-8 17-0-0 17-0-0 0-0-0 Location 8-10-8 0-0-0	Right /2(DOL = 1.6 Location 2 1-0-12 5-6-0 5-6-0 7-10-8 7-10-8 9-6-0 9-10-8 17-0-0 17-0-0 23-5-4 23-5-4 2-3-0 Direction Up Left	Direction NUp NUp NUp NUp NUp NUp NUp NU	Rake Rake Rake Rake Rake Rake Rake Rake	15.97 plf 15.97 plf 15.97 plf 15.97 plf 15.97 plf 7.98 plf 7.98 plf 12.7 plf 25.39 plf 12.7 plf 25.39 plf 12.7 plf	15.97 plf 15.97 plf 15.97 plf 15.97 plf 15.97 plf 7.98 plf 7.98 plf 12.7 plf 25.39 plf 12.7 plf 25.39 plf 12.7 plf	Trib Width			
Bot Top User-defined Lo Distributed Loads Member Top	0-0-0 and Case W13: W Location 1 0-0-0 1-0-12 1-0-12 5-6-0 7-10-8 9-6-0 9-10-8 9-10-8 17-0-0 17-0-0 0-0-0 Location 8-10-8 0-0-0 and Case W14: W Location 0-0-0 nations	Right /2(DOL=1.6) Location 2 1-0-12 5-6-0 5-6-0 7-10-8 7-10-8 9-6-0 9-10-8 17-0-0 17-0-0 23-5-4 23-5-4 23-5-4 23-5-4 23-0 Direction Up Left /3(DOL=1.6)	Direction NUp NUp NUp NUp NUp NUp NUp NUp NUp NU	Rake Rake Rake Rake Rake Rake Rake Rake	15.97 plf 15.97 plf 15.97 plf 15.97 plf 15.97 plf 7.98 plf 7.98 plf 12.7 plf 25.39 plf 12.7 plf 25.39 plf 12.7 plf 15.11 plf	15.97 plf 15.97 plf 15.97 plf 15.97 plf 7.98 plf 7.98 plf 12.7 plf 25.39 plf 12.7 plf 25.39 plf 12.7 plf 15.11 plf	Trib Width			
Bot Top User-defined Lo Distributed Loads Member Top	0-0-0 and Case W13: W Location 1 0-0-0 1-0-12 1-0-12 5-6-0 7-10-8 9-6-0 9-10-8 9-10-8 17-0-0 17-0-0 0-0-0 Location 8-10-8 0-0-0 and Case W14: W Location 0-0-0 nations and Combo	Right /2(DOL=1.6) Location 2 1-0-12 5-6-0 5-6-0 7-10-8 7-10-8 9-6-0 9-10-8 17-0-0 17-0-0 23-5-4 23-5-4 23-5-4 23-5-4 23-0 Direction Up Left /3(DOL=1.6)	Direction NUp NUp NUp NUp NUp NUp NUp NUp NUp NU	Rake Rake Rake Rake Rake Rake Rake Rake	15.97 plf 15.97 plf 15.97 plf 15.97 plf 15.97 plf 7.98 plf 7.98 plf 12.7 plf 25.39 plf 12.7 plf 25.39 plf 12.7 plf 15.11 plf	15.97 plf 15.97 plf 15.97 plf 15.97 plf 15.97 plf 7.98 plf 7.98 plf 12.7 plf 25.39 plf 12.7 plf 25.39 plf 12.7 plf 15.11 plf	Trib Width			
Bot Top User-defined Lo Distributed Loads Member Top	0-0-0 and Case W13: W Location 1 0-0-0 1-0-12 1-0-12 5-6-0 7-10-8 9-6-0 9-10-8 9-10-8 17-0-0 17-0-0 0-0-0 Location 8-10-8 0-0-0 and Case W14: W Location 0-0-0 nations and Combo	Right /2(DOL=1.6) Location 2 1-0-12 5-6-0 5-6-0 7-10-8 7-10-8 9-6-0 9-10-8 17-0-0 17-0-0 23-5-4 23-5-4 23-5-4 23-5-4 23-0 Direction Up Left /3(DOL=1.6)	Direction NUp NUp NUp NUp NUp NUp NUp NUp NUp NU	Rake Rake Rake Rake Rake Rake Rake Rake	15.97 plf 15.97 plf 15.97 plf 15.97 plf 15.97 plf 7.98 plf 7.98 plf 12.7 plf 25.39 plf 12.7 plf 25.39 plf 12.7 plf 15.11 plf	15.97 plf 15.97 plf 15.97 plf 15.97 plf 7.98 plf 7.98 plf 12.7 plf 25.39 plf 12.7 plf 25.39 plf 12.7 plf 15.11 plf	Trib Width			

2550 Hwy 33 South P.O. Box 260 Maysville, Missouri 64469 Truss:GR12

Job: 500 NW Chipman Rd REVISED

WGT/PLY 156 lbs

Date: 02/16/22 10:56:38

Page: 3 of 3

SPA 24-6-		QTY 1	OHL 0-0-0	OHR 0-0-0	CANTL 0-0-0	CANTR 0-0-0	PLYS 1	SPACIN 18.25 ir
3	D1+S1				1.000			
4	D1 +S2				1.000			
5	0.60 D1 +0.60 W1 [Uplift]				1.000			
6	0.60 D1 +0.60 W2 [Uplift]				1.000			
7	0.60 D1 +0.60 W4 [Uplift]				1.000			
8	0.60 D1 +0.60 W8 [Uplift]				1.000			
9	0.60 D1 +0.60 W0 [Uplift]				1.000			
10	0.60 D1 +0.60 W0 [Uplift]				1.000			
11	0.60 D1 +0.60 W0 [Uplift]				1.000			
12	D1 +L10*1				1.000			
13	D1 +Lr1 +L10*1				1.000			
14	D1 +S1 +L10*1				1.000			
15	D1 + S2 + L10*1				1.000			
16	D1 +UR1				1.000			
17	D1 + UR2				1.000			
18	D1 +AS10*1				1.000			
19	D1 +Lr1 +AS10*1				1.000			
20	D1 +S1 +AS10*1				1.000			
21	D1 + S2 + AS10*1				1.000			
mber	Forces Table indicates: N	Member ID. max CSI.	max axial force, (max com	pr. force if different fron	n max axial force). Only for	ces greater than 300lbs are	shown in this table.	

TC	1-2	0.248	-2,197 lbs		3-5	0.374	-3,372 lbs		6-7	0.178	-1,541 lbs		
	2-3	0.370	-3,460 lbs		5-6	0.282	-2,683 lbs		7-8	0.232	798 lbs	(-376 lbs)	
BC	9-10	0.131	1,541 lbs	(470 lbs)	11-12	0.273	3,368 lbs	(-372 lbs)	14-15	0.216	2,194 lbs	(-162 lbs)	
	10-11	0.218	2,680 lbs	(480 lbs)	12-14	0.303	3,458 lbs	(-140 lbs)	15-16	0.048	386 lbs	(-380 lbs)	
Web	1-16	0.139	-1,108 lbs		5-11	0.301	-906 lbs		7-9	0.554	-1,818 lbs		
	1-15	0.588	2,395 lbs	(-397 lbs)	6-11	0.137	558 lbs						
	2-15	0.105	-821 lbs		6-10	0.443	-1,313 lbs						
	2-14	0.377	1,535 lbs	(-178 lbs)	7-10	0.189	770 lbs						

Truss to Truss Connection Summary

Carried Truss	Carrying Chord	Carrying Offset
DD7	P.C	8 10 8

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 30% (Cq = 0.70).
- 3) Provide adequate drainage to prevent ponding.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to
- the plane of the truss. Lateral braces shall be installed within 6 "of each web panel point.
- 6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 7) Listed wind uplift reactions based on MWFRS & C&C loading.
- 8) Parapet TL: 0.51 in, 2L/238 (17-8), Allowable 2L/120.

2550 Hwy 33 South P.O. Box 260 Maysville, Missouri 64469 Truss:GR2

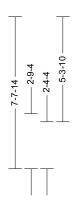
Job: 500 NW Chipman Rd REVISED

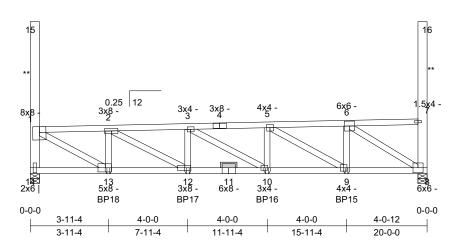
Date: 02/16/22 10:56:40

Page: 1 of 3

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
20-0-0	0.25/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	133 lbs

			20-0-0			
	3-11-4	4-0-0	4-0-0	4-0-0	4-0-12	
ı	3-11-4	7-11-4	11-11-4	15-11-4	20-0-0	Π





All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI	Deflectio	n	L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code:	IBC 2018/	TC: 0.48 (2-3)	Vert TL:	0.29 in	L/788	11	L/240
TCLL: 20	*	TPI 1-2014	BC: 0.79 (10-12)	Vert LL:	0.18 in	L/999	11	L/360
TCDL: 15	Rep Mbr:	No	Web: 0.99 (6-8)	Horz TL:	0.07 in		8	
BCLL: 0	Lumber D.O.1	[.:115%						

BCDL: 10 Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplif	t Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
14	1	5.5 in	2.14 in	1,817 lbs	•	-31 lbs	-144 lbs	-144 lbs	248 lbs
8	1	5.5 in	2.17 in	1.837 lbs		-164 lbs	-122 lbs	-164 lbs	

Material

TC: SYP#1 2 x 4 BC: SYP#1 2 x 4

Web: SYP#2 2 x 4 except: SYP SSDense 2 x 6: 15-14, 16-8

Bracing

TC: Sheathed or Purlins at 2-8-0, Purlin design by Others. BC: Sheathed or Purlins at 8-4-0, Purlin design by Others.

Loads

- 1) This truss has been designed for the effects due to 10 psf bottom chord live load plus dead loads.
- 2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used $for the \, roof \, snow \, load, \, DOL = 1.15. \,\, If \, the \, roof \, configuration \, differs \, from \, hip/gable, \, Build \, ing \, Designer \, shall \, verify \, snow \, loads \,.$
- 3) This truss has not been designed for the effects of unbalanced snow loads.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 5) Minimum storage attic loading has been applied in accordance with IBC 1607.1
- 6) **- Indicates parapet wind loading has been applied to members 15-1 & 16-7

Load Case Lr1: Std Live Load

Distributed Loads	3						
Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	1-0-12	2-11-4	Down	Proj	20 plf	20 plf	
Top	4-11-4	6-11-4	Down	Proj	20 plf	20 plf	
Top	8-11-4	10-11-4	Down	Proj	20 plf	20 plf	
Top	12-11-4	14-11-4	Down	Proj	20 plf	20 plf	
Top	16-11-4	18-11-4	Down	Proj	20 plf	20 plf	
Top	0-0-0	20-0-0	Down	Proj	20 plf	20 plf	



2550 Hwy 33 South P.O. Box 260 Maysville, Missouri 64469 Truss:GR2

Job: 500 NW Chipman Rd REVISED

Date: 02/16/22 10:56:40

Page: 2 of 3

SPAN PITCH QTY OHL OHR CANTL CANTR PLYS **SPACING** WGT/PLY 0-0-0 20-0-0 0.25/12 0-0-0 0-0-00-0-01 24 in 133 lbs

Load Case D1: Std Dead L

Distri	buted 1	Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Тор	1-0-12	2-11-4	Down	Proj	15 plf	15 plf	
Top	4-11-4	6-11-4	Down	Proj	15 plf	15 plf	
Top	8-11-4	10-11-4	Down	Proj	15 plf	15 plf	
Top	12-11-4	14-11-4	Down	Proj	15 plf	15 plf	
Top	16-11-4	18-11-4	Down	Proj	15 plf	15 plf	
Top	0-0-0	20-0-0	Down	Proj	15 plf	15 plf	
Bot	1-0-12	2-11-4	Down	Proj	10 plf	10 plf	
Bot	4-11-4	6-11-4	Down	Proj	10 plf	10 plf	
Bot	8-11-4	10-11-4	Down	Proj	10 plf	10 plf	
Bot	12-11-4	14-11-4	Down	Proj	10 plf	10 plf	
Bot	16-11-4	18-11-4	Down	Proj	10 plf	10 plf	
Bot	0-0-0	20-0-0	Down	Proj	10 plf	10 plf	

User-defined Load Case S2: Drift

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Тор	0-0-0	1-0-12	Down	Proj	20 plf	20 plf	
Top	1-0-12	2-11-4	Down	Proj	20 plf	20 plf	
Top	1-0-12	2-11-4	Down	Proj	20 plf	20 plf	
Top	2-11-4	4-11-4	Down	Proj	20 plf	20 plf	
Top	4-11-4	6-11-4	Down	Proj	20 plf	20 plf	
Top	4-11-4	6-11-4	Down	Proj	20 plf	20 plf	
Top	6-11-4	8-11-4	Down	Proj	20 plf	20 plf	
Top	8-11-4	10-11-4	Down	Proj	20 plf	20 plf	
Top	8-11-4	10-11-4	Down	Proj	20 plf	20 plf	
Top	10-11-4	12-11-4	Down	Proj	20 plf	20 plf	
Top	12-11-4	14-11-4	Down	Proj	20 plf	20 plf	
Top	12-11-4	14-11-4	Down	Proj	20 plf	20 plf	
Top	14-11-4	16-11-4	Down	Proj	20 plf	20 plf	
Top	16-11-4	18-11-4	Down	Proj	20 plf	20 plf	
Top	16-11-4	18-11-4	Down	Proj	20 plf	20 plf	
Тор	18-11-4	20-0-0	Down	Proj	20 plf	20 plf	
Top	0-0-0	20-0-0	Down	Proj	41 psf	41 psf	24 in
Point Loads							

Point Loads Member

Member	Location	Direction	Load	Trib Width
Bot	3-11-4	Down	38 lbs	
Bot	7-11-4	Down	38 lbs	
Bot	11-11-4	Down	38 lbs	
Bot	15-11-4	Down	38 lbs	

Load Combinations

Louid C	omonicuons .	
#	Load Combo	Factor
1	D1	1.000
2	D1+Lr1	1.000
3	D1 +S1	1.000
4	D1 +S2	1.000
5	0.60 D1 +0.60 W1 [Uplift]	1.000
6	0.60 D1 +0.60 W2 [Uplift]	1.000
7	0.60 D1 +0.60 W4 [Uplift]	1.000
8	0.60 D1 +0.60 W8 [Uplift]	1.000
9	D1 +L10*1	1.000
10	D1 +Lr1 +L10*1	1.000
11	D1+S1+L10*1	1.000
12	D1 +S2 +L10*1	1.000
13	D1+UR1	1.000
14	D1 +UR2	1.000
15	D1 +AS10*1	1.000
16	D1 +Lr1 +AS10*1	1.000
17	D1 +S1 +AS10*1	1.000
18	D1 +S2 +AS10*1	1.000

Member Forces
Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC 11-2 0.419 2.689 lbs 13-5 0.454 3.841 lbs 16-7 0.373 523 lbs (247 lbs) 1

10	~ I	1-2	0.419	-2,089 IDS	3-3	0.454	-5,841 IDS		0-7	0.575	323 IDS	(-247 IDS)	i
		2-3	0.484	-3,951 lbs	5-6	0.422	-2,512 lbs						
BO	7	8-9	0.532	2,506 lbs	(-390 lbs) 10-	12 0.789	3,944 lbs	(-550 lbs)	13-14	0.112	394 lbs	(-369 lbs)	
		9-10	0.749	3,835 lbs	(-506 lbs) 12-	13 0.567	2,684 lbs	(-516 lbs)					
													,

2550 Hwy 33 South P.O. Box 260 Maysville, Missouri 64469 Truss:GR2

Job: 500 NW Chipman Rd REVISED

Date: 02/16/22 10:56:40

Page: 3 of 3

	SPAN 20-0-0		PITCI 0.25/1		Q'	TY 1		HL 0-0	OHR 0-0-0	CANTL 0-0-0	CANTR 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 133 lbs	
Web	1-14 1-13 2-13 2-12	0.319 0.754 0.173 0.351	-1,761 lbs 3,073 lbs -1,287 lbs 1,430 lbs	(-168 lbs)	6-9	0.066 0.556 0.249 0.992	-481 lbs -1,535 lbs 1,014 lbs -2,954 lbs	(-120 lbs)							

Truss to Truss Connection Summary

Camed Truss	Carrying Chord	Carrying Offset	
BP18	BC	3-11-4	
BP17	BC	7-11-4	
BP16	BC	11-11-4	
BP15	BC	15-11-4	

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 30% (Cq = 0.70).
- 3) Provide adequate drainage to prevent ponding.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 "of each web panel point.
- 6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
 7) Indicates non-structural members.
- 8) Listed wind uplift reactions based on MWFRS & C&C loading.
- 9) Parapet TL: 0.43 in, 2L/301 (15-1), Allowable 2L/120.

2550 Hwy 33 South P.O. Box 260 Maysville, Missouri 64469 Truss:GR3

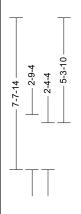
Job: 500 NW Chipman Rd REVISED

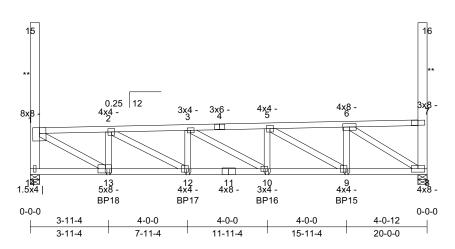
Date: 02/16/22 10:56:42

Page: 1 of 3

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
20-0-0	0.25/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	131 lbs

		20-0-0		
3-11-4	4-0-0	4-0-0	4-0-0	4-0-12
3-11-4	7-11-4	11-11-4	15-11-4	20-0-0





All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflectio	n	L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code:	IBC 2018/	TC:	0.38 (3-5)	Vert TL:	0.26 in	L/894	11	L/240
TCLL: 20		TPI 1-2014	BC:	0.69 (10-12)	Vert LL:	0.15 in	L/999	11	L/360
TCDL: 15	Rep Mbr:	No	Web:	0.93 (15-1)	Horz TL:	0.07 in		8	
BCLL: 0	Lumber D.O.1	L.:115 %							0271

BCDL: 10 Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Upli	ift Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
14	1	5.5 in	1.87 in	1,589 lbs	•	-74 lbs	-14 lbs	-74 lbs	487 lbs
8	1	5.5 in	2.02 in	1.708 lbs		-27 lbs		-27 lbs	

Material

TC: SYP#1 2 x 4 BC: SYP#1 2 x 4 Web: SYP#2 2 x 4 except:

SYPSSDense 2 x 6: 15-14, 16-8

Bracing

TC: Sheathed or Purlins at 2-11-0, Purlin design by Others.

BC: Sheathed or Purlins at 7-4-0, Purlin design by Others.



Loads

1) This truss has been designed for the effects due to 10 psf bottom chord live load plus dead loads.

- 2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 3) This truss has not been designed for the effects of unbalanced snow loads.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL=1.60
- 5) Minimum storage attic loading has been applied in accordance with IBC 1607.1
- 6) **- Indicates parapet wind loading has been applied to members 15-1 & 16-7

Load Case Lr1: Std Live Load

Distributed Loads

Distributed Loads							
Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Тор	0-0-0	2-11-4	Down	Proj	20 plf	20 plf	
Тор	4-11-4	6-11-4	Down	Proj	20 plf	20 plf	
Тор	8-11-4	10-11-4	Down	Proj	20 plf	20 plf	
Тор	12-11-4	14-11-4	Down	Proj	20 plf	20 plf	
Тор	15-11-4	20-0-0	Down	Proj	20 plf	20 plf	
Тор	0-0-0	2-11-4	Down	Proj	20 plf	20 plf	
Тор	4-11-4	6-11-4	Down	Proj	20 plf	20 plf	
Тор	8-11-4	10-11-4	Down	Proj	20 plf	20 plf	
Тор	12-11-4	14-11-4	Down	Proj	20 plf	20 plf	
Top	15-11-4	20-0-0	Down	Proj	20 plf	20 plf	

2550 Hwy 33 South P.O. Box 260 Maysville, Missouri 64469 Truss:GR3

Job: 500 NW Chipman Rd REVISED

Date: 02/16/22 10:56:43

Page: 2 of 3

PLYS SPACING WGT/PLY 1 24 in 131 lbs

SPA 20-0-			TY 1	OHL 0-0-0	OHR 0-0-0	CANTL 0-0-0	CANTR 0-0-0
Load Case	D1: Std Dead Load						
Distributed L Member	oads Location 1	Location 2	Direction	Spread	Start Load	End Load	TribWidth
Top	14-4-0	16-6-0	Down	Proj	35.64 plf	35.64 plf	THE TTEM
Тор	0-0-0	2-11-4	Down	Proj	15 plf	15 plf	
Тор	4-11-4	6-11-4	Down	Proj	15 plf	15 plf	
Тор	8-11-4	10-11-4	Down	Proj	15 plf	15 plf	
Тор	12-11-4	14-11-4	Down	Proj	15 plf	15 plf	
Top	15-11-4	20-0-0	Down	Proj	15 plf	15 plf	
Top	0-0-0	2-11-4	Down	Proj	15 plf	15 plf	
Top	4-11-4	6-11-4	Down	Proj	15 plf	15 plf	
Top	8-11-4	10-11-4	Down	Proj	15 plf	15 plf	
Top	12-11-4	14-11-4	Down	Proj	15 plf	15 plf	
Top	15-11-4	20-0-0	Down	Proj	15 plf	15 plf	
Bot	0-0-0	2-11-4	Down	Proj	10 plf	10 plf	
Bot	4-11-4	6-11-4	Down	Proj	10 plf	10 plf	
Bot	8-11-4	10-11-4	Down	Proj	10 plf	10 plf	
Bot	12-11-4	14-11-4	Down	Proj	10 plf	10 plf	
Bot	15-11-4	20-0-0	Down	Proj	10 plf	10 plf	
Bot	0-0-0	2-11-4	Down	Proj	10 plf	10 plf	
Bot	4-11-4	6-11-4	Down	Proj	10 plf	10 plf	
Bot	8-11-4	10-11-4	Down	Proj	10 plf	10 plf	
Bot	12-11-4	14-11-4	Down	Proj	10 plf	10 plf	
Bot	15-11-4	20-0-0	Down	Proj	10 plf	10 plf	
	ed Load Case S2: Dri	ft					
Distributed L	oads						
Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Тор	1-0-12	2-11-4	Down	Proj	20 plf	20 plf	
Тор	1-0-12	2-11-4	Down	Proj	20 plf	20 plf	
Top	4-11-4	6-11-4	Down	Proj	20 plf	20 plf	
Top	4-11-4	6-11-4	Down	Proj	20 plf	20 plf	
Top	8-11-4	10-11-4	Down	Proj	20 plf	20 plf	
Top	8-11-4	10-11-4	Down	Proj	20 plf	20 plf	
Top	12-11-4	14-11-4	Down	Proj	20 plf	20 plf	
Top	12-11-4	14-11-4	Down	Proj	20 plf	20 plf	
Top	16-11-4	18-11-4	Down	Proj	20 plf	20 plf	
Top	16-11-4	18-11-4	Down	Proj	20 plf	20 plf	24:
Top	0-0-0	20-0-0	Down	Proj	29 psf	29 psf	24 in
Тор	0-0-0	4-6-0	Down	Proj	3 psf	0 psf	24 in
Point Loads	Lagation	Dimention	Load	Tails W.E.14b			
Member	Location	Direction	Load	Trib Width			
Bot	3-11-4	Down	38 lbs				
Bot	3-11-4	Down	38 lbs				
Bot Bot	7-11-4 7-11-4	Down	38 lbs				
Bot	7-11-4	Down	38 lbs 38 lbs				
Bot	11-11-4	Down Down					
Bot	11-11-4 15-11-4	Down	38 lbs 38 lbs				
Bot	15-11-4	Down	38 lbs				
I		Down	30 108				
I	mbinations					En eta ii	
#	Load Combo					Factor	
1	D1					1.000	
2	D1+Lr1					1.000	
3	D1 +S1					1.000	
4	D1+S2	FT 1107				1.000	
5	0.60 D1 +0.60 W1					1.000	
6	0.60 D1 + 0.60 W2	- 1 -				1.000	
7	0.60 D1 +0.60 W4					1.000	
8	0.60 D1 +0.60 W8	[Uplift]				1.000	
9	D1 +L10*1					1.000	
10	D1 +Lr1 +L10*1					1.000	
11	D1 +S1 +L10*1					1.000	
12	D1 +S2 +L10*1					1.000	
13	D1 +UR1					1.000	
14	D1 +UR2					1.000	
15	D1 +AS10*1					1.000	
16	D1 +Lr1 +AS10*1					1.000	
17	D1 + S1 + AS10*1					1.000	

 $ALL\,PERSONS\,FABRICATING,\,HANDLING,\,ERECTING\,OR\,INSTALLINGANY\,TRUSS\,BASED\,UPON\,THIS\,TRUSS\,DESIGN\,DRAWINGARE\,INSTRUCTED\,TO$ REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGNAND AVAILABLE FROM EAGLE UPON REQUEST. DESIGNVALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

1.000

1.000

D1 + S1 + AS10*1

D1+S2+AS10*1

17

18

TrueBuild® Truss Software v5.6.390 Eagle Metal Products

2550 Hwy 33 South P.O. Box 260 Maysville, Missouri 64469 Truss:GR3

Job: 500 NW Chipman Rd REVISED

Date: 02/16/22 10:56:43

Page: 3 of 3

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
20-0-0	0.25/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	131 lbs

Men	nber]	Forces	Table	indicates: M	ember ID	, max CS	I, max axial for	rce, (max con	npr. force	if differen	t from max axi	al force). Onl	ly forces greater than 300lbs are shown in this	table.
TC	1-2	0.377	-2,324 lbs		3-5	0.377	-3,400 lbs		6-7	0.314	1,048 lbs	(492 lbs)		I
	2-3	0.370	-3,432 lbs		5-6	0.358	-2,337 lbs							
BC	8-9	0.521	2,333 lbs	(416 lbs)	10-12	0.691	3,427 lbs	(-540 lbs)	13-14	0.118	798 lbs	(-717 lbs)		1
	9-10	0.660	3,396 lbs	(-462 lbs)	12-13	0.491	2,320 lbs	(-634 lbs)						
Web	1-14	0.647	-1,516 lbs		3-12	0.048	-350 lbs							•
	1-13	0.652	2,656 lbs	(-198 lbs)	5-9	0.444	-1,228 lbs							
	2-13	0.140	-1,036 lbs		6-9	0.240	980 lbs	(-30 lbs)						
	2-12	0.308	1,256 lbs	(-189 lbs)	6-8	0.924	-2,750 lbs						=	

Truss to Truss Connection Summary

Carried Truss	Carrying Chord	Carrying Offset
BP18	BC	3-11-4
BP18	BC	3-11-4
BP17	BC	7-11-4
BP17	BC	7-11-4
BP16	BC	11-11-4
BP16	BC	11-11-4
BP15	BC	15-11-4
BP15	BC	15-11-4

- $1) \, Unless \, noted \, otherwise, do \, not \, cut \, or \, alter \, any \, truss \, member \, or \, plate \, without \, prior \, approval \, from \, a \, Professional \, Engineer.$
- 2) The fabrication tolerance for this roof truss is 30% (Cq = 0.70).
- 3) Provide adequate drainage to prevent ponding.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 "of each web panel point.
- 6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used. 7) Listed wind uplift reactions based on MWFRS & C&C loading.
- 8) Parapet TL: 0.84 in, 2L/155 (15-1), Allowable 2L/120.

2550 Hwy 33 South P.O. Box 260 Maysville, Missouri 64469 Truss:GR4

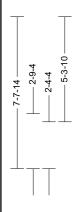
Job: 500 NW Chipman Rd REVISED

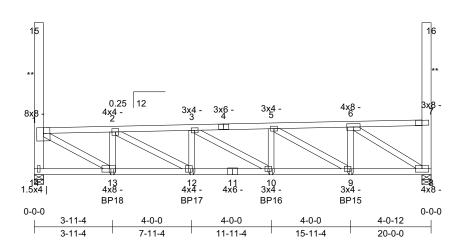
Date: 02/16/22 10:56:45

Page: 1 of 3

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
20-0-0	0.25/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	131 lbs

		20-0-0		
3-11-4	4-0-0	4-0-0	4-0-0	4-0-12
3-11-4	7-11-4	11-11-4	15-11-4	20-0-0





All plates shown to be Eagle 20 unless otherwise noted.

Lumber D.O.L.:115 %

Loading (psf)	General		CSI	Deflectio	n	L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code:	IBC 2018/	TC: 0.35 (1-2)	Vert TL:	0.23 in	L/999	11	L/240
TCLL: 20		TPI 1-2014	BC: 0.61 (10-12)	Vert LL:	0.11 in	L/999	11	L/360
TCDL: 15	Rep Mbr:	No	Web: 0.93 (15-1)	Horz TL:	$0.06 \mathrm{in}$		8	eres da
BCII · O	LumberDOL	.115 %						02/1

BCDL: 10 Reaction

BCLL: 0

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplit	ft Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
14	1	5.5 in	1.68 in	1,424 lbs		-196 lbs	-124 lbs	-196 lbs	489 lbs
8	1	5.5 in	1.78 in	1.511 lbs			-87 lbs	-87 lbs	

Material

TC: SYP#1 2 x 4 BC: SYP#1 2 x 4 Web: SYP#2 2 x 4 except:

SYP SSDense 2 x 6: 15-14, 16-8

Bracing

TC: Sheathed or Purlins at 3-3-0, Purlin design by Others.

BC: Sheathed or Purlins at 6-9-0, Purlin design by Others.



Loads

- 1) This truss has been designed for the effects due to 10 psf bottom chord live load plus dead loads.
- 2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used $for the \, roof \, snow \, load, \, DOL = 1.15. \,\, If \, the \, roof \, configuration \, differs \, from \, hip/gable, \, Build \, ing \, Designer \, shall \, verify \, snow \, loads \,.$
- 3) This truss has not been designed for the effects of unbalanced snow loads.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 5) Minimum storage attic loading has been applied in accordance with IBC 1607.1
- 6) **- Indicates parapet wind loading has been applied to members 15-1 & 16-7

Load Case Lr1: Std Live Load

Dictributed Loads

Distributed Loads							
Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	TribWidth
Тор	0-0-0	20-0-0	Down	Proj	20 plf	20 plf	
Тор	0-0-0	2-11-4	Down	Proj	20 plf	20 plf	
Тор	4-11-4	6-11-4	Down	Proj	20 plf	20 plf	
Тор	8-11-4	10-11-4	Down	Proj	20 plf	20 plf	
Тор	12-11-4	14-11-4	Down	Proj	20 plf	20 plf	
Top	15-11-4	20-0-0	Down	Proj	20 plf	20 plf	

2550 Hwy 33 South P.O. Box 260 Maysville, Missouri 64469 Truss:GR4

Job: 500 NW Chipman Rd REVISED

Date: 02/16/22 10:56:45

Page: 2 of 3

PLYS SPACING WGT/PLY 1 24 in 131 lbs

SPA1 20-0-			TY 1	OHL 0-0-0	OHR 0-0-0	CANTL 0-0-0	CANTR 0-0-0
Load Case	D1: Std Dead Load						
Distributed Lo	oads						
Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Тор	14-4-0	16-6-0	Down	Proj	35.64 plf	35.64 plf	
Top	0-0-0	20-0-0	Down	Proj	15 plf	15 plf	
Top	0-0-0	2-11-4	Down	Proj	15 plf	15 plf	
Top	4-11-4	6-11-4	Down	Proj	15 plf	15 plf	
Top	8-11-4	10-11-4	Down	Proj	15 plf	15 plf	
Тор	12-11-4	14-11-4	Down	Proj	15 plf	15 plf	
Top	15-11-4	20-0-0	Down	Proj	15 plf	15 plf	
Bot	0-0-0	20-0-0	Down	Proj	10 plf	10 plf	
Bot Bot	0-0-0 4-11-4	2-11-4 6-11-4	Down	Proj	10 plf	10 plf	
Bot	8-11-4	10-11-4	Down Down	Proj Proj	10 plf 10 plf	10 plf 10 plf	
Bot	12-11-4	14-11-4	Down	Proj	10 plf	10 plf	
Bot	15-11-4	20-0-0	Down	Proj	10 plf	10 plf	
	xd Load Case S2: Dr		Down	110j	10 рш	тори	
Distributed Lo	oads						
Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	1-0-12	2-11-4	Down	Proj	20 plf	20 plf	
Top	1-0-12	2-11-4	Down	Proj Proj	20 plf	20 plf	
Top Top	2-11-4 4-11-4	4-11-4 6-11-4	Down Down	Proj Proj	20 plf	20 plf 20 plf	
Тор Тор	4-11-4 4-11-4	6-11-4	Down	Proj	20 plf 20 plf	20 plf	
Тор Тор	6-11-4	8-11-4	Down	Proj	20 plf	20 plf	
Тор	8-11-4	10-11-4	Down	Proj	20 plf	20 plf	
Тор	8-11-4	10-11-4	Down	Proj	20 plf	20 plf	
Тор	10-11-4	12-11-4	Down	Proj	20 plf	20 plf	
Тор	12-11-4	14-11-4	Down	Proj	20 plf	20 plf	
Тор	12-11-4	14-11-4	Down	Proj	20 plf	20 plf	
Top	14-11-4	16-11-4	Down	Proj	20 plf	20 plf	
Top	16-11-4	18-11-4	Down	Proj	20 plf	20 plf	
Top	16-11-4	18-11-4	Down	Proj	20 plf	20 plf	
Top	0-0-0	20-0-0	Down	Proj	16 psf	16 psf	24 in
Top	0-0-0	4-6-0	Down	Proj	16 psf	0 psf	24 in
Top	15-6-0	20-0-0	Down	Proj	0 psf	16 psf	24 in
Point Loads							
Member	Location	Direction	Load	Trib Width			
Bot	3-11-4	Down	38 lbs				
Bot	7-11-4	Down	38 lbs				
Bot	11-11-4	Down	38 lbs				
Bot	15-11-4	Down	38 lbs				
Load Cor #	nbinations Load Combo					Factor	
1	D1					1.000	
2	Dl +Lrl					1.000	
3	D1 +S1					1.000	
4	D1+S2					1.000	
	0.60 D1 +0.60 W1	[Uplift]				1.000	
5						1.000	
5 6	$0.60\mathrm{D1} + 0.60\mathrm{W2}$						
5 6 7	0.60 D1 +0.60 W2 0.60 D1 +0.60 W4	- 1 -				1.000	
6	0.60 D1 +0.60 W2 0.60 D1 +0.60 W4 0.60 D1 +0.60 W8	[Uplift]				1.000	
6 7	0.60 D1 +0.60 W4 0.60 D1 +0.60 W8	[Uplift]				1.000	
6 7 8	0.60 D1 + 0.60 W4 0.60 D1 + 0.60 W8 D1 + L10*1	[Uplift]					
6 7 8 9	0.60 D1 +0.60 W4 0.60 D1 +0.60 W8	[Uplift]				1.000 1.000	
6 7 8 9 10	0.60 D1 +0.60 W4 0.60 D1 +0.60 W8 D1 +L10*1 D1 +Lr1 +L10*1 D1 +S1 +L10*1	[Uplift]				1.000 1.000 1.000	
6 7 8 9 10	0.60 D1 +0.60 W4 0.60 D1 +0.60 W8 D1 +L10*1 D1 +Lr1 +L10*1	[Uplift]				1.000 1.000 1.000 1.000 1.000	
6 7 8 9 10 11 12	0.60 D1 + 0.60 W4 0.60 D1 + 0.60 W8 D1 + L10*1 D1 + Lr1 + L10*1 D1 + S1 + L10*1 D1 + S2 + L10*1 D1 + UR1	[Uplift]				1.000 1.000 1.000 1.000	
6 7 8 9 10 11 12	0.60 D1 +0.60 W4 0.60 D1 +0.60 W8 D1 +L10*1 D1 +Lr1 +L10*1 D1 +S1 +L10*1 D1 +S2 +L10*1	[Uplift]				1.000 1.000 1.000 1.000 1.000 1.000	
6 7 8 9 10 11 12 13	0.60 D1 + 0.60 W4 0.60 D1 + 0.60 W8 D1 + L10*1 D1 + Lr1 + L10*1 D1 + S1 + L10*1 D1 + S2 + L10*1 D1 + UR1 D1 + UR2	[Uplift] 3 [Uplift]				1.000 1.000 1.000 1.000 1.000 1.000 1.000	
6 7 8 9 10 11 12 13 14	0.60 D1 + 0.60 W4 0.60 D1 + 0.60 W8 D1 + L10*1 D1 + Lr1 + L10*1 D1 + S1 + L10*1 D1 + S2 + L10*1 D1 + UR1 D1 + UR2 D1 + AS10*1	[Uplift] 3 [Uplift]				1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000	

Ivien	nber .	Forces	Table	indicates: M	ember ID), max CS	I, max axial for	ce, (max con	npr. force	if differen	t from max ax	al force). Onl	ly forces greater than 3	800lbs are shown in this	table
TC	1-2	0.346	-2,054 lbs		3-5	0.313	-2,966 lbs		6-7	0.297	1,048 lbs	(492 lbs)			
	2-3	0.320	-3,005 lbs		5-6	0.322	-2,029 lbs								
BC	8-9	0.462	2,025 lbs	(-595 lbs)	10-12	0.615	3,000 lbs	(-820 lbs)	13-14	0.116	796 lbs	(-721 lbs)			1
	9-10	0.587	2,962 lbs	(-736 lbs)	12-13	0.444	2,050 lbs	(-825 lbs)							

2550 Hwy 33 South P.O. Box 260 Maysville, Missouri 64469 Truss:GR4

Job: 500 NW Chipman Rd REVISED

Date: 02/16/22 10:56:45

Page: 3 of 3

	SPAN 20-0-0		PITC: 0.25/		QTY 1	OHL 0-0-0	OHR 0-0-0	CANTL 0-0-0	CANTR 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 131 lbs
Web	1-14 1-13 2-13 2-12	0.643 0.576 0.125 0.264	-1,351 lbs 2,347 lbs -930 lbs 1,078 lbs	(437 lbs) 5-9 (-315 lbs) 6-8	0.043 0.392 0.199 0.802	-313 lbs -1,082 lbs 810 lbs -2,387 lbs						

Truss to Truss Connection Summary

Camed Tiuss	Carrying Chord	Carrying Offset
BP18	BC	3-11-4
BP17	BC	7-11-4
BP16	BC	11-11-4
BP15	BC	15-11-4

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 30% (Cq = 0.70).
- 3) Provide adequate drainage to prevent ponding.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to
- the plane of the truss. Lateral braces shall be installed within 6 "of each web panel point. 6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
 7) Listed wind uplift reactions based on MWFRS & C&C loading.
- 8) Parapet TL: 0.85 in, 2L/153 (15-1), Allowable 2L/120.

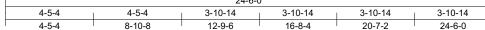
2550 Hwy 33 South P.O. Box 260 Maysville, Missouri 64469 Truss:GR5

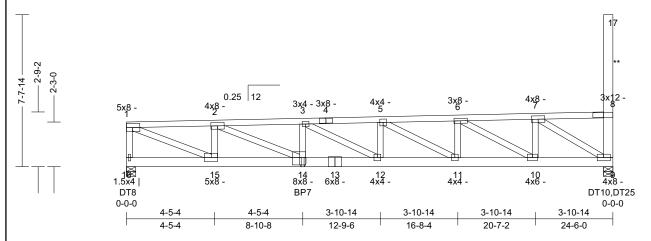
Job: 500 NW Chipman Rd REVISED

Date: 02/16/22 10:56:47

Page: 1 of 3

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
24-6-0	0.25/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	159 lbs
	1			24-6	-0			1	





All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI	Deflectio	n	L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code:	IBC 2018/	TC: 0.53 (2-3)	Vert TL:	0.42 in	L/672	(12-13)	L/240
TCLL: 20	_	TPI 1-2014	BC: 0.44 (12-14)	Vert LL:	0.16 in	L/999	(12-13)	L/360
TCDL: 15	Rep Mbr:	No	Web: 0.85 (1-15)	Horz TL:	$0.06 \mathrm{in}$		9	
BCLL: 0	Lumber D.O.1	[4:115%						

BCDL: 10 Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Upli	ift Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
16	1	5.5 in	1.75 in	1,729 lbs	•	•	-210 lbs	-210 lbs	-479 lbs
9	1	5.5 in	1.65 in	1.634 lbs		-112 lbs	-109 lbs	-112 lbs	

Material

TC: SYP#1 2 x 4 BC: SYPSSDense 2 x 6 Web: SYP#2 2 x 4 except: SYPSSDense 2 x 6: 17-9

Bracing

TC: Sheathed or Purlins at 2-3-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects due to 10 psf bottom chord live load plus dead loads.

2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.

3) This truss has not been designed for the effects of unbalanced snow loads.

4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL=1.60

5) Minimum storage attic loading has been applied in accordance with IBC 1607.1

6) *** - Indicates parapet wind loading has been applied to member 17-8 Load Case Lr1: Std Live Load

Distributed Loads

Distributed Load	ds						
Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Тор	0-0-0	7-10-8	Down	Proj	20 plf	20 plf	
Top	8-10-8	24-6-0	Down	Proj	20 plf	20 plf	
Top	0-0-0	7-10-8	Down	Proj	20 plf	20 plf	
Top	8-10-8	24-6-0	Down	Proi	20 plf	20 plf	



2550 Hwy 33 South P.O. Box 260

Truss:GR5

Job: 500 NW Chipman Rd REVISED
Date: 02/16/22 10:56:47

				May	P.O. Box 26 sville, Missou				Date: 02/16/22 10:56 Page: 2 of 3	:47
SPAN 24-6-0	PITCH 0.25/12		TY 1	OHL 0-0-0	OHR 0-0-0	CANTL 0-0-0	CANTR 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 159 lbs
		•							21111	137 103
Load Case D1: S Distributed Loads	Std Dead Load									
Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width			
Тор	0-0-0	7-10-8	Down	Proj	15 plf	15 plf	THO WIGHT			
Тор Тор	8-10-8	24-6-0	Down	Proj	15 plf	15 plf				
Тор	0-0-0	7-10-8	Down	Proj	15 plf	15 plf				
Тор	8-10-8	24-6-0	Down	Proj	15 plf	15 plf				
Bot	0-0-0	7-10-8	Down	Proj	10 plf	10 plf				
Bot	8-10-8	24-6-0	Down	Proj	10 plf	10 plf				
Bot	0-0-0	7-10-8	Down	Proj	10 plf	10 plf				
Bot	8-10-8	24-6-0	Down	Proj	10 plf	10 plf				
	ad Case S2: Sno		Down	110j	торп	тори				
Oser-defined Lo Distributed Loads	au Case 52. 5110	W DIIII								
Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width			
Тор	9-0-0	24-6-0	Down	Proj	5 psf	5 psf	24 in			
Top	9-0-0	13-0-0	Down	Proj	27 psf	27 psf	24 in			
Top	20-6-0	24-6-0	Down	Proj	0 psf	27 psf	24 in			
Point Loads				·	•	•				
Member	Location	Direction	Load	Trib Width						
User-defined Lo	ad Case W12: W	V1(DOL=1.6	50)							
Distributed Loads										
Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width			
Тор	1-0-12	7-6-0	N Up	Rake	25.39 plf	25.39 plf				
Top	1-0-12	7-6-0	N Up	Rake	25.39 plf	25.39 plf				
Top	7-6-0	7-10-8	N Up	Rake	25.39 plf	25.39 plf				
Top	7-6-0	7-10-8	N Up	Rake	25.39 plf	25.39 plf				
Top	9-10-8	15-0-0	N Up	Rake	25.39 plf	25.39 plf				
Top	9-10-8	15-0-0	N Up	Rake	25.39 plf	25.39 plf				
Top	15-0-0	23-5-4	N Up	Rake	15.97 plf	15.97 plf				
Top	15-0-0	23-5-4	NUp	Rake	15.97 plf	15.97 plf				
Point Loads										
Member	Location	Direction	Load	Trib Width						
Bot	0-0-12	Down	44 lbs							
Bot	8-10-8	Down	44 lbs							
Bot	8-10-8	Up	139 lbs							
Bot	24-5-4	Down	86 lbs							
Bot	24-5-4	Down	86 lbs							
Тор	0-0-0	Right	600 lbs							
	ad Case W13: W	V2(DOL = 1.6	60)							
Distributed Loads Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width			
Тор	1-0-12	7-10-8	NUp	Rake	15.97 plf	15.97 plf				
Тор	1-0-12	7-10-8	NUp	Rake	15.97 plf	15.97 plf				
Тор	9-10-8	17-0-0	NUp	Rake	25.39 plf	25.39 plf				
Тор	9-10-8	17-0-0	NUp	Rake	25.39 plf	25.39 plf				
Тор	17-0-0	23-5-4	NUp	Rake	25.39 plf	25.39 plf				
Top	17-0-0	23-5-4	N Up	Rake	25.39 plf	25.39 plf				
Point Loads					•	•				
Member	Location	Direction	Load	Trib Width						
Bot	0-0-12	Up	115 lbs							
Bot	8-10-8	Up	139 lbs							
Bot	8-10-8	Down	44 lbs							
Bot	24-5-4	Up	159 lbs							
Bot	24-5-4	Up	159 lbs							
	0-0-0	Left	600 lbs							
Top	0-0-0	Len	000 108							

2550 Hwy 33 South P.O. Box 260 Maysville, Missouri 64469 Truss:GR5

Job: 500 NW Chipman Rd REVISED

Date: 02/16/22 10:56:47

Page: 3 of 3

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
24-6-0	0.25/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	159 lbs

User-defined Load Case W14: W4(DOL=1.60)

Die	hri	huted	Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	1-0-12	3-0-0	N Up	Rake	48.89 plf	48.89 plf	
Top	1-0-12	3-0-0	N Up	Rake	48.89 plf	48.89 plf	
Top	3-0-0	7-10-8	N Up	Rake	43.75 plf	43.75 plf	
Top	3-0-0	7-10-8	N Up	Rake	43.75 plf	43.75 plf	
Top	9-10-8	23-5-4	N Up	Rake	43.75 plf	43.75 plf	
Тор	9-10-8	23-5-4	N Up	Rake	43.75 plf	43.75 plf	
Point Loads							
Member	Location	Direction	Load	Trib Width			
Тор	0-0-0	Left	600 lbs	_			

Load Combinations

#	Load Combo	Factor
1	D1	1.000
2	D1 +Lr1	1.000
3	D1 +S1	1.000
4	D1 +S2	1.000
5	0.60 D1 +0.60 W1 [Uplift]	1.000
6	0.60 D1 +0.60 W2 [Uplift]	1.000
7	0.60 D1 +0.60 W4 [Uplift]	1.000
8	0.60 D1 +0.60 W8 [Uplift]	1.000
9	0.60 D1 +0.60 W0 [Uplift]	1.000
10	0.60 D1 +0.60 W0 [Uplift]	1.000
11	0.60 D1 +0.60 W0 [Uplift]	1.000
12	D1 +L10*1	1.000
13	D1 + Lr1 + L10*1	1.000
14	D1 +S1 +L10*1	1.000
15	D1 +S2 +L10*1	1.000
16	D1 +UR1	1.000
17	D1 + UR2	1.000
18	D1 +AS10*1	1.000
19	D1 +Lr1 +AS10*1	1.000
20	D1 +S1 +AS10*1	1.000
21	D1 + S2 + AS10*1	1.000

Member Forces Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table. 0.391 -3,159 lbs 3-5 0.534 -4,792 lbs 0.227 -2,156 lbs (-501 lbs) 7-8 0.534 -5,000 lbs 5-6 0.399 -3,791 lbs 0.197 1.064 lbs 9-10 0.179 2,153 lbs (-577 lbs) 11-12 0.388 4.788 lbs (-329 lbs) 14-15 0.312 3,156 lbs (-359 lbs) 0.436 10-11 (-534 lbs) 12-14 4,995 lbs 0.065 (-383 lbs) 0.307 3.788 lbs (-135 lbs) 15-16 516 lbs 0.199 -1,582 lbs 0.055 309 lbs 0.191 780 lbs 1-16 3-14 (-161 lbs) 6-11 0.845 3,444 lbs (-396 lbs) 3-12 0.136 -417 lbs 0.635 -1,881 lbs 2-15 0.152 -1,193 lbs 5-12 0.086 350 lbs 7-10 0.268 1.091 lbs (-12 lbs) (-123 lbs) 5-11 -1,306 lbs -2,541 lbs

0.774

2,224 lbs **Truss to Truss Connection Summary**

Carried Truss	Carrying Chord	Carrying Offset	
DT8	BC	0-0-12	
BP7	BC	8-10-8	
BP7	BC	8-10-8	
DT10	BC	24-5-4	
DT25	BC	24-5-4	

Notes

2-14

0.546

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.

0.434

- 2) The fabrication tolerance for this roof truss is 30% (Cq = 0.70).
- 3) Provide adequate drainage to prevent ponding.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 "of each web panel point.
- 6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 7) Listed wind uplift reactions based on MWFRS & C&C loading.
- 8) Parapet TL: 0.67 in, 2L/180 (17-8), Allowable 2L/120.

2550 Hwy 33 South P.O. Box 260 Maysville, Missouri 64469

12-5-4

Truss:GR6

Job: 500 NW Chipman Rd REVISED

ARTURO A

"Himmuni

Date: 02/16/22 10:56:49

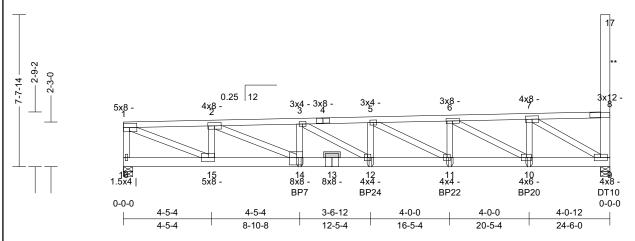
Page: 1 of 4

24-6-0

SPAN 24-6-0	PIT 0.25		QTY 1	OHL 0-0-0	OHR 0-0-0	CANTL 0-0-0	CANTR 0-0-0	PLYS 1	SPACINO 24 in	G WGT/PLY 160 lbs
					24	1-6-0			I	
		4-5-4		4-5-4	3-6-12	4-0-0	4-0-0	4-0-1	2	

16-5-4

20-5-4



All plates shown to be Eagle 20 unless otherwise noted.

4-5-4

8-10-8

Loading (psf)	General		CSI	Deflection	n	L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code:	IBC 2018/	TC: 0.55 (2-3)	Vert TL:	0.43 in	L/657	(12-13)	L/240
TCLL: 20	_	TPI 1-2014	BC: 0.44 (12-14)	Vert LL:	0.18 in	L/999	(12-13)	L/360
TCDL: 15	Rep Mbr:	No	Web: 0.88 (7-9)	Horz TL:	0.06 in		9	
BCLL: 0	Lumber D.O.I	L.:115 %	, ,					Ð

BCDL: 10 Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplif	t Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
16	1	5.5 in	1.65 in	1,629 lbs	•		-171 lbs	-171 lbs	-482 lbs
9	1	5.5 in	1.70 in	1.684 lbs		-241 lbs		-241 lbs	

Material

TC: SYP#1 2 x 4 BC: SYPSSDense 2 x 6 Web: SYP#2 2 x 4 except: SYPSSDense 2 x 6: 17-9

Bracing

TC: Sheathed or Purlins at 2-4-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- 1) This truss has been designed for the effects due to 10 psf bottom chord live load plus dead loads.
- 2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used $for the \, roof \, snow \, load, \, DOL = 1.15. \,\, If \, the \, roof \, configuration \, differs \, from \, hip/gable, \, Build \, ing \, Designer \, shall \, verify \, snow \, loads \,.$
- 3) This truss has not been designed for the effects of unbalanced snow loads.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 5) Minimum storage attic loading has been applied in accordance with IBC 1607.1
- 6) **- Indicates parapet wind loading has been applied to member 17-8

Load Case Lr1: Std Live Load

Distributed Loads

Distributed Dates							
Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Тор	0-0-0	7-10-8	Down	Proj	20 plf	20 plf	
Top	9-10-8	11-5-4	Down	Proj	20 plf	20 plf	
Top	13-5-4	15-5-4	Down	Proj	20 plf	20 plf	
Тор	17-5-4	19-5-4	Down	Proj	20 plf	20 plf	
Тор	20-5-4	24-6-0	Down	Proj	20 plf	20 plf	
Top	0-0-0	7-10-8	Down	Proj	20 plf	20 plf	
Тор	8-10-8	24-6-0	Down	Proj	20 plf	20 plf	

2550 Hwy 33 South P.O. Box 260 Maysville, Missouri 64469 Truss:GR6

Job: 500 NW Chipman Rd REVISED Date: 02/16/22 10:56:50

Page: 2 of 4

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
24-6-0	0.25/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	160 lbs

SPAN 24-6-0	PITCH 0.25/12		TY 1	OHL 0-0-0	OHR 0-0-0	CANTL 0-0-0	CANTR 0-0-0
Load Case D1:	Std Dead Load						
Distributed Loads			D	G 1	GT. 1	P. 17 1	m 1 11 14
Member	Location 1	7-10-8	Direction	Spread	Start Load	End Load	Trib Width
Top Top	0-0-0 9-10-8	7-10-8 11-5-4	Down Down	Proj Proj	15 plf 15 plf	15 plf 15 plf	
Тор	13-5-4	15-5-4	Down	Proj	15 plf	15 plf	
Тор	17-5-4	19-5-4	Down	Proj	15 plf	15 plf	
Тор	20-5-4	24-6-0	Down	Proj	15 plf	15 plf	
Top	0-0-0	7-10-8	Down	Proj	15 plf	15 plf	
Top	8-10-8	24-6-0	Down	Proj	15 plf	15 plf	
Bot	0-0-0	7-10-8	Down	Proj	10 plf	10 plf	
Bot	9-10-8	11-5-4	Down	Proj	10 plf	10 plf	
Bot	13-5-4	15-5-4	Down	Proj	10 plf	10 plf	
Bot	17-5-4	19-5-4	Down	Proj	10 plf	10 plf	
Bot Bot	20-5-4 0-0-0	24-6-0 7-10-8	Down Down	Proj Proj	10 plf 10 plf	10 plf 10 plf	
Bot	8-10-8	24-6-0	Down	Proj	10 plf	10 plf	
User-defined Lo			Down	110j	торш	тори	
Distributed Loads							
Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Тор	9-0-0	24-6-0	Down	Proj	24 psf	24 psf	24 in
Тор	9-0-0	11-0-0	Down	Proj	8 psf	8 psf	24 in
Top	22-6-0	24-6-0	Down	Proj	0 psf	8 psf	24 in
Point Loads				,	1		
Member	Location	Direction	Load	Trib Width			
User-defined Lo	oad Case W12: V	V1(DOL=1.6	(0)				
Distributed Loads							
Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	1-0-12	7-6-0	N Up	Rake	25.39 plf	25.39 plf	
Тор	1-0-12	7-6-0	NUp	Rake	25.39 plf	25.39 plf	
Top	7-6-0	7-10-8	N Up	Rake	25.39 plf	25.39 plf	
Top	7-6-0	7-10-8	N Up	Rake Rake	25.39 plf	25.39 plf	
Top Top	9-10-8 9-10-8	11-5-4 11-5-4	N Up N Up	Rake	25.39 plf 25.39 plf	25.39 plf 25.39 plf	
Тор	11-5-4	13-5-4	N Up	Rake	25.39 plf	25.39 plf	
Тор	13-5-4	15-0-0	NUp	Rake	25.39 plf	25.39 plf	
Тор	13-5-4	15-0-0	NUp	Rake	25.39 plf	25.39 plf	
Top	15-0-0	15-5-4	NUp	Rake	15.97 plf	15.97 plf	
Top	15-0-0	15-5-4	N Up	Rake	15.97 plf	15.97 plf	
Top	15-5-4	17-5-4	N Up	Rake	15.97 plf	15.97 plf	
Top	17-5-4	19-5-4	NUp	Rake	15.97 plf	15.97 plf	
Тор	17-5-4	19-5-4	NUp	Rake	15.97 plf	15.97 plf	
Тор	19-5-4	21-5-4	NUp	Rake	15.97 plf	15.97 plf	
Top	21-5-4	23-5-4	N Up	Rake	15.97 plf	15.97 plf	
Тор	21-5-4	23-5-4	NUp	Rake	15.97 plf	15.97 plf	
Point Loads	T	D: .:	T 1	T 1 337 14			
Member	Location	Direction	Load	Trib Width			
Bot Bot	8-10-8 8-10-8	Down	44 lbs				
Bot Bot	8-10-8 12-5-4	Up Down	139 lbs 38 lbs				
Bot	16-5-4	Down	36 lbs 46 lbs				
Bot	20-5-4	Down	53 lbs				
Bot	24-5-4	Down	86 lbs				
Bot	24-5-4	Down	86 lbs				
Тор	0-0-0	Right	600 lbs				

2550 Hwy 33 South P.O. Box 260 Maysville, Missouri 64469 Truss:GR6

Job: 500 NW Chipman Rd REVISED

Date: 02/16/22 10:56:50

Page: 3 of 4

ı	SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY	
ı	24-6-0	0.25/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	160 lbs	

User-defined l	User-defined Load Case W13: W2(DOL= 1.60)									
Distributed Load	s									
Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width			
Top	1-0-12	7-10-8	N Up	Rake	15.97 plf	15.97 plf				
Top	1-0-12	7-10-8	N Up	Rake	15.97 plf	15.97 plf				
Top	9-10-8	11-5-4	N Up	Rake	25.39 plf	25.39 plf				
Top	9-10-8	11-5-4	N Up	Rake	25.39 plf	25.39 plf				
Top	11-5-4	13-5-4	N Up	Rake	25.39 plf	25.39 plf				
Top	13-5-4	15-5-4	N Up	Rake	25.39 plf	25.39 plf				
Top	13-5-4	15-5-4	N Up	Rake	25.39 plf	25.39 plf				
Top	15-5-4	17-0-0	N Up	Rake	25.39 plf	25.39 plf				
Top	17-0-0	17-5-4	N Up	Rake	25.39 plf	25.39 plf				
Top	17-5-4	19-5-4	N Up	Rake	25.39 plf	25.39 plf				
Top	17-5-4	19-5-4	N Up	Rake	25.39 plf	25.39 plf				
Top	19-5-4	21-5-4	N Up	Rake	25.39 plf	25.39 plf				
Top	21-5-4	23-5-4	N Up	Rake	25.39 plf	25.39 plf				
Тор	21-5-4	23-5-4	N Up	Rake	25.39 plf	25.39 plf				
Point Loads										
Member	Location	Direction	Load	Trib Width						
Bot	8-10-8	Up	139 lbs							
Bot	8-10-8	Down	44 lbs							
Bot	12-5-4	Up	162 lbs							
Bot	16-5-4	Up	169 lbs							
Bot	20-5-4	Up	176 lbs							
Bot	24-5-4	Up	159 lbs							
Bot	24-5-4	Up	159 lbs							
Top	0-0-0	Left	600 lbs							
User-defined l	Load Case W14:	W4(DOL=1.6	50)							

D:		
Distri	buted	Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Тор	1-0-12	3-0-0	N Up	Rake	48.89 plf	48.89 plf	
Тор	1-0-12	3-0-0	N Up	Rake	48.89 plf	48.89 plf	
Тор	3-0-0	7-10-8	N Up	Rake	43.75 plf	43.75 plf	
Тор	3-0-0	7-10-8	N Up	Rake	43.75 plf	43.75 plf	
Тор	9-10-8	11-5-4	N Up	Rake	43.75 plf	43.75 plf	
Тор	9-10-8	11-5-4	N Up	Rake	43.75 plf	43.75 plf	
Тор	11-5-4	13-5-4	N Up	Rake	43.75 plf	43.75 plf	
Top	13-5-4	15-5-4	N Up	Rake	43.75 plf	43.75 plf	
Top	13-5-4	15-5-4	N Up	Rake	43.75 plf	43.75 plf	
Top	15-5-4	17-5-4	N Up	Rake	43.75 plf	43.75 plf	
Top	17-5-4	19-5-4	N Up	Rake	43.75 plf	43.75 plf	
Тор	17-5-4	19-5-4	N Up	Rake	43.75 plf	43.75 plf	
Тор	19-5-4	21-5-4	N Up	Rake	43.75 plf	43.75 plf	
Тор	21-5-4	23-5-4	N Up	Rake	43.75 plf	43.75 plf	
Top	21-5-4	23-5-4	N Up	Rake	43.75 plf	43.75 plf	
Point Loads							
Member	Location	Direction	Load	Trib Width			
Тор	0-0-0	Left	600 lbs				

Load Combinations

#	Load Combo	Factor
1	D1	1.000
2	Dl +Lrl	1.000
3	D1 +S1	1.000
4	D1 +S2	1.000
5	0.60 D1 +0.60 W1 [Uplift]	1.000
6	0.60 D1 +0.60 W2 [Uplift]	1.000
7	0.60 D1 +0.60 W4 [Uplift]	1.000
8	0.60 D1 +0.60 W8 [Uplift]	1.000
9	0.60 D1 +0.60 W0 [Uplift]	1.000
10	0.60 D1 +0.60 W0 [Uplift]	1.000
11	0.60 D1 +0.60 W0 [Uplift]	1.000
12	D1 +L10*1	1.000
13	D1 + Lr1 + L10*1	1.000
14	D1+S1+L10*1	1.000
15	D1 +S2 +L10*1	1.000
16	D1 +UR1	1.000
17	D1 +UR2	1.000
18	D1 +AS10*1	1.000

2550 Hwy 33 South P.O. Box 260 Maysville, Missouri 64469 Truss:GR6

Job: 500 NW Chipman Rd REVISED

Date: 02/16/22 10:56:50

Page: 4 of 4

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
24-6-0	0.25/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	160 lbs

 19
 D1 + Lr1 + AS10*1
 1.000

 20
 D1 + S1 + AS10*1
 1.000

 21
 D1 + S2 + AS10*1
 1.000

Mei	mber .	Forces	Table indicate	s: Member II	O, max CS	I, max axial force	e, (max compr. force	if differer	t from max axi	ial force). On	ly forces greater than 300lbs are shown in this table.
TC	1-2	0.363	-3,047 lbs	3-5	0.521	-5,001 lbs	6-7	0.249	-2,344 lbs		ĺ
	1	0.545	5 110 H	~ -	0.400	4.010.11	7.0	0.011	1.064.11	(CO1 II)	

IC.	1-2	0.505	-5,047 108		5-5	0.521	-5,001 108		0-7	0.249	-2,5 -11 108		
	2-3	0.547	-5,119 lbs		5-6	0.423	-4,013 lbs		7-8	0.211	1,064 lbs	(-501 lbs)	
BC	9-10	0.198	2,340 lbs	(419 lbs)	11-12	0.406	4,997 lbs	(-79 lbs)	14-15	0.310	3,043 lbs	(-267 lbs)	
	10-11	0.324	4,009 lbs	(-258 lbs)	12-14	0.443	5,117 lbs		15-16	0.063	519 lbs	(-379 lbs)	
Web	1-16	0.192	-1,529 lbs		3-12	0.089	-327 lbs		7-10	0.274	1,115 lbs	(-54 lbs)	
	1-15	0.815	3,322 lbs	(-271 lbs)	5-11	0.386	-1,120 lbs		7-9	0.882	-2,730 lbs		
	2-15	0.146	-1,142 lbs		6-11	0.169	687 lbs						
	2-14	0.563	2,295 lbs		6-10	0.667	-1,908 lbs						

Truss to Truss Connection Summary

Carried Truss	Carrying Chord	Carrying Offset
BP7	BC	8-10-8
BP7	BC	8-10-8
BP24	BC	12-5-4
BP22	BC	16-5-4
BP20	BC	20-5-4
DT10	BC	24-5-4
DT10	BC	24-5-4

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 30 % (Cq = 0.70).
- 3) Provide adequate drainage to prevent ponding.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 "of each web panel point.
- 6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 7) Indicates non-structural members.
- 8) Listed wind uplift reactions based on MWFRS & C&C loading.
- 9) Parapet TL: 0.66 in, 2L/183 (17-8), Allowable 2L/120.

2550 Hwy 33 South P.O. Box 260 Maysville, Missouri 64469

3-6-12

12-5-4

Truss:GR7

Job: 500 NW Chipman Rd REVISED

Date: 02/16/22 10:56:52

Page: 1 of 4

4-0-12

24-6-0

4-0-12

24-6-0

SPA		QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
24-6-		1	0-0-0	0-0-0	0-0-0	0-0-0	1	23.75 in	159 lbs
	I			2	24-6-0			ĺ	

4-0-0

16-5-4

4-0-0

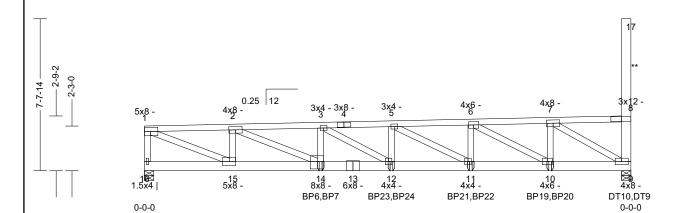
16-5-4

4-0-0

20-5-4

4-0-0

20-5-4



3-6-12

12-5-4

All plates shown to be Eagle 20 unless otherwise noted.

4-5-4

4-5-4

4-5-4

4-5-4

8-10-8

4-5-4

8-10-8

Loading (psf)	General		CSI		Deflection	n	L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code:	IBC 2018/	TC:	0.55 (2-3)	Vert TL:	0.44 in	L/648	(12-13)	L/240
TCLL: 20		TPI 1-2014	BC:	0.44 (12-14)	Vert LL:	0.2 in	L/999	(12-13)	L/360
TCDL: 15	Rep Mbr:	No	Web	: 0.92 (7-9)	Horz TL:	$0.06 \mathrm{in}$		9	
BCLL: 0	Lumber D.O.I	L.:115 %							

BCDL: 10 Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Upl	ift Max MWFRS Uplift!	Max C&C Uplift	Max Uplift	Max Horiz
16	1	5.5 in	1.57 in	1,551 lbs		•	-115 lbs	-115 lbs	-481 lbs
9	1	5.5 in	1.77 in	1.756 lbs		-118 lbs		-118 lbs	

Material

TC: SYP#1 2 x 4
BC: SYP SSDense 2 x 6
Web: SYP#2 2 x 4 except
SYP SSDense 2 x 6: 17-9

Bracing

TC: Sheathed or Purlins at 2-3-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- 1) This truss has been designed for the effects due to 10 psf bottom chord live load plus dead loads.
- 2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 3) This truss has not been designed for the effects of unbalanced snow loads.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL=1.60
- 5) Minimum storage attic loading has been applied in accordance with IBC 1607.1
- 6) **- Indicates parapet wind loading has been applied to member 17-8

Load Case Lr1: Std Live Load

Distributed Loads

Distributed Loads							
Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Тор	0-0-0	7-10-8	Down	Proj	19.58 plf	19.58 plf	
Тор	9-10-8	11-5-4	Down	Proj	19.58 plf	19.58 plf	
Тор	13-5-4	15-5-4	Down	Proj	19.58 plf	19.58 plf	
Тор	17-5-4	19-5-4	Down	Proj	19.58 plf	19.58 plf	
Тор	21-5-4	24-6-0	Down	Proj	19.58 plf	19.58 plf	
Тор	0-0-0	7-10-8	Down	Proj	20 plf	20 plf	
Тор	9-10-8	11-5-4	Down	Proj	20 plf	20 plf	
Тор	13-5-4	15-5-4	Down	Proj	20 plf	20 plf	
Тор	17-5-4	19-5-4	Down	Proj	20 plf	20 plf	
Тор	20-5-4	24-6-0	Down	Proj	20 plf	20 plf	



2550 Hwy 33 South P.O. Box 260 Maysville, Missouri 64469 Truss:GR7

Job: 500 NW Chipman Rd REVISED Date: 02/16/22 10:56:53

Page: 2 of 4

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY	
24-6-0	0.25/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	23.75 in	159 lbs	

				,			
SPAN	PITCH	O	TY	OHL	OHR	CANTL	CANTR
24-6-0	0.25/12		1	0-0-0	0-0-0	0-0-0	0-0-0
Load Case D1:	Std Dead Load						
Distributed Loads							
Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Тор	0-0-0	7-10-8	Down	Proj	14.69 plf	14.69 plf	
Top	9-10-8	11-5-4	Down	Proj	14.69 plf	14.69 plf	
Тор	13-5-4	15-5-4	Down	Proj	14.69 plf	14.69 plf	
Тор	17-5-4	19-5-4	Down	Proj	14.69 plf	14.69 plf	
Top	21-5-4	24-6-0	Down	Proj	14.69 plf	14.69 plf	
Top	0-0-0 9-10-8	7-10-8	Down Down	Proj	15 plf	15 plf	
Top Top	13-5-4	11-5-4 15-5-4	Down	Proj Proj	15 plf 15 plf	15 plf 15 plf	
Тор	17-5-4	19-5-4	Down	Proj	15 plf	15 plf	
Тор	20-5-4	24-6-0	Down	Proj	15 plf	15 plf	
Bot	0-0-0	7-10-8	Down	Proj	9.79 plf	9.79 plf	
Bot	9-10-8	11-5-4	Down	Proj	9.79 plf	9.79 plf	
Bot	13-5-4	15-5-4	Down	Proj	9.79 plf	9.79 plf	
Bot	17-5-4	19-5-4	Down	Proj	9.79 plf	9.79 plf	
Bot	21-5-4	24-6-0	Down	Proj	9.79 plf	9.79 plf	
Bot	0-0-0	7-10-8	Down	Proj	10 plf	10 plf	
Bot	9-10-8	11-5-4	Down	Proj	10 plf	10 plf	
Bot	13-5-4	15-5-4	Down	Proj	10 plf	10 plf	
Bot	17-5-4	19-5-4	Down	Proj	10 plf	10 plf	
Bot	20-5-4	24-6-0	Down	Proj	10 plf	10 plf	
User-defined Lo	oad Case S2: Sno	ow Drift					
Distributed Loads							
Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Тор	9-0-0	24-6-0	Down	Proj	32 psf	32 psf	24 in
Point Loads				5	F	F	
Member	Location	Direction	Load	Trib Width			
				THO WIGHT			
User-defined Lo	oad Case W12: V	V1(DOL = 1.6	50)				
Distributed Loads							
Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Тор	1-0-12	7-6-0	N Up	Rake	24.87 plf	24.87 plf	
Top	1-0-12	7-6-0	N Up	Rake	25.39 plf	25.39 plf	
Тор	7-6-0	7-10-8	N Up	Rake	24.87 plf	24.87 plf	
Тор	7-6-0	7-10-8	N Up	Rake	25.39 plf	25.39 plf	
Top	9-10-8	11-5-4	N Up	Rake	24.87 plf	24.87 plf	
Top	9-10-8	11-5-4	N Up	Rake	25.39 plf	25.39 plf	
Top Top	13-5-4 13-5-4	15-0-0 15-0-0	N Up N Up	Rake Rake	24.87 plf 25.39 plf	24.87 plf 25.39 plf	
Тор	15-0-0	15-5-4	N Up	Rake	15.63 plf	15.63 plf	
Тор	15-0-0	15-5-4	N Up	Rake	15.97 plf	15.97 plf	
Тор	17-5-4	19-5-4	N Up	Rake	15.63 plf	15.63 plf	
Тор	17-5-4	19-5-4	N Up	Rake	15.97 plf	15.97 plf	
Тор	21-5-4	23-5-4	NUp	Rake	15.63 plf	15.63 plf	
Тор	21-5-4	23-5-4	NUp	Rake	15.97 plf	15.97 plf	
Point Loads						1	
Member	Location	Direction	Load	Trib Width			
Bot	8-10-8	Down	48 lbs	THE WIGHT			
Bot	8-10-8	Up	139 lbs				
Bot	12-5-4	Down	42 lbs				
Bot	12-5-4	Up	162 lbs				
Bot	16-5-4	Down	50 lbs				
Bot	16-5-4	Up	169 lbs				
Bot	20-5-4	Down	57 lbs				
Bot	20-5-4	Up	176 lbs				
Bot	24-5-4	Down	90 lbs				
Bot	24-5-4	Down	86 lbs				
Тор	0-0-0	Right	600 lbs				

2550 Hwy 33 South P.O. Box 260 Maysville, Missouri 64469 Truss:GR7

Job: 500 NW Chipman Rd REVISED

Date: 02/16/22 10:56:53

Page: 3 of 4

ı	SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY	
l	24-6-0	0.25/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	23.75 in	159 lbs	

User-defined l	User-defined Load Case W13: W2(DOL=1.60)										
Distributed Load	ls										
Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width				
Тор	1-0-12	7-10-8	N Up	Rake	15.63 plf	15.63 plf					
Top	1-0-12	7-10-8	N Up	Rake	15.97 plf	15.97 plf					
Top	9-10-8	11-5-4	N Up	Rake	24.87 plf	24.87 plf					
Top	9-10-8	11-5-4	N Up	Rake	25.39 plf	25.39 plf					
Top	13-5-4	15-5-4	N Up	Rake	24.87 plf	24.87 plf					
Top	13-5-4	15-5-4	N Up	Rake	25.39 plf	25.39 plf					
Top	17-5-4	19-5-4	N Up	Rake	24.87 plf	24.87 plf					
Top	17-5-4	19-5-4	N Up	Rake	25.39 plf	25.39 plf					
Top	21-5-4	23-5-4	N Up	Rake	24.87 plf	24.87 plf					
Тор	21-5-4	23-5-4	N Up	Rake	25.39 plf	25.39 plf					
Point Loads											
Member	Location	Direction	Load	Trib Width							
Bot	8-10-8	Up	141 lbs								
Bot	8-10-8	Down	44 lbs								
Bot	12-5-4	Up	163 lbs								
Bot	12-5-4	Down	38 lbs								
Bot	16-5-4	Up	170 lbs								
Bot	16-5-4	Down	46 lbs								
Bot	20-5-4	Up	178 lbs								
Bot	20-5-4	Down	53 lbs								
Bot	24-5-4	Up	161 lbs								
Bot	24-5-4	Up	159 lbs								
Top	0-0-0	Left	600 lbs								

User-defined Load Case W14: W4(DOL = 1.60)

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Тор	1-0-12	3-0-0	N Up	Rake	47.87 plf	47.87 plf	_
Top	1-0-12	3-0-0	NUp	Rake	48.89 plf	48.89 plf	
Тор	3-0-0	7-10-8	N Up	Rake	42.83 plf	42.83 plf	
Top	3-0-0	7-10-8	N Up	Rake	43.75 plf	43.75 plf	
Top	9-10-8	11-5-4	N Up	Rake	42.83 plf	42.83 plf	
Top	9-10-8	11-5-4	N Up	Rake	43.75 plf	43.75 plf	
Top	13-5-4	15-5-4	N Up	Rake	42.83 plf	42.83 plf	
Top	13-5-4	15-5-4	N Up	Rake	43.75 plf	43.75 plf	
Top	17-5-4	19-5-4	N Up	Rake	42.83 plf	42.83 plf	
Top	17-5-4	19-5-4	N Up	Rake	43.75 plf	43.75 plf	
Top	21-5-4	23-5-4	N Up	Rake	42.83 plf	42.83 plf	
Top	21-5-4	23-5-4	N Up	Rake	43.75 plf	43.75 plf	
Point Loads							
Member	Location	Direction	Load	Trib Width			
Top	0-0-0	Left	600 lbs		•	•	

Load Combinations

#	Load Combo	Factor
1	D1	1.000
2	Dl +Lrl	1.000
3	D1 +S1	1.000
4	D1 + S2	1.000
5	0.60 D1 +0.60 W1 [Uplift]	1.000
6	0.60 D1 +0.60 W2 [Uplift]	1.000
7	0.60 D1 +0.60 W4 [Uplift]	1.000
8	0.60 D1 +0.60 W8 [Uplift]	1.000
9	0.60 D1 +0.60 W0 [Uplift]	1.000
10	0.60 D1 +0.60 W0 [Uplift]	1.000
11	0.60 D1 +0.60 W0 [Uplift]	1.000
12	D1 +L10*1	1.000
13	D1 +Lr1 +L10*1	1.000
14	D1 +S1 +L10*1	1.000
15	D1 + S2 + L10*1	1.000
16	D1 +UR1	1.000
17	D1 + UR2	1.000
18	D1 +AS10*1	1.000
19	D1 +Lr1 +AS10*1	1.000
20	D1 +S1 +AS10*1	1.000
21	D1 +S2 +AS10*1	1.000

2550 Hwy 33 South P.O. Box 260 Maysville, Missouri 64469 Truss:GR7

Job: 500 NW Chipman Rd REVISED

Date: 02/16/22 10:56:53

Page: 4 of 4

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
24-6-0	0.25/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	23.75 in	159 lbs

Mer	Member Forces Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.													
TC	1-2	0.339	-3,026 lbs		3-5	0.531	-5,093 lbs		6-7	0.282	-2,442 lbs		1	I
	2-3	0.548	-5,135 lbs		5-6	0.437	-4,144 lbs		7-8	0.242	1,053 lbs	(495 lbs)		
BC	9-10	0.204	2,438 lbs	(-242 lbs)	11-12	0.414	5,089 lbs		14-15	0.310	3,024 lbs	(-181 lbs)		Ì
	10-11	0.333	4,139 lbs		12-14	0.441	5,133 lbs		15-16	0.062	517 lbs	(-376 lbs)		J
Web	1-16	0.183	-1,453 lbs		5-12	0.084	341 lbs		7-10	0.279	1,138 lbs			•
	1-15	0.810	3,301 lbs	(-140 lbs)	5-11	0.371	-1,077 lbs		7-9	0.919	-2,843 lbs			
	2-15	0.143	-1,119 lbs		6-11	0.169	688 lbs							
	2-14	0.566	2,306 lbs		6-10	0.680	-1,945 lbs						l	

Truss to Truss Connection Summary

Carried Truss	Carrying Chord	Carrying Offset
BP6	BC	8-10-8
BP7	BC	8-10-8
BP23	BC	12-5-4
BP24	BC	12-5-4
BP21	BC	16-5-4
BP22	BC	16-5-4
BP19	BC	20-5-4
BP20	BC	20-5-4
DT9	BC	24-5-4
DT10	BC	24-5-4

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 30% (Cq = 0.70).
- 3) Provide adequate drainage to prevent ponding.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6" of each web panel point.

 6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 7) Listed wind uplift reactions based on MWFRS & C&C loading.
- 8) Parapet TL: 0.64 in, 2L/189 (17-8), Allowable 2L/120.

2550 Hwy 33 South P.O. Box 260 Maysville, Missouri 64469

3-6-12

12-5-4

Truss:GR8

Job: 500 NW Chipman Rd REVISED

Date: 02/16/22 10:56:55

Page: 1 of 4

4-0-12

24-6-0

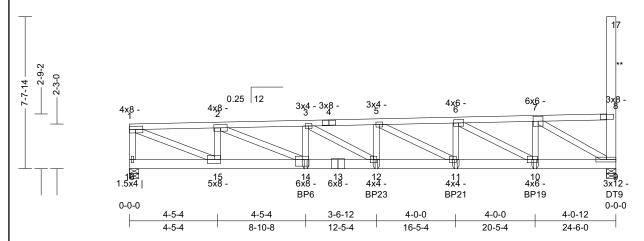
SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
24-6-0	0.25/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	20.75 in	158 lbs
	1			24-6	6-0			I	

4-0-0

16-5-4

4-0-0

20-5-4



All plates shown to be Eagle 20 unless otherwise noted.

4-5-4

4-5-4

4-5-4

8-10-8

Loading (psf)	General		CSI		Deflectio	n	L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code:	IBC 2018/	TC:	0.49 (3-5)	Vert TL:	0.39 in	L/719	(12-13)	L/240
TCLL: 20		TPI 1-2014	BC:	0.39 (11-12)	Vert LL:	0.2 in	L/999	(12-13)	L/360
TCDL: 15	Rep Mbr:	No	Web	: 0.90 (7-9)	Horz TL:	0.05 in		9	
BCLL: 0	Lumber D.O.I	L.:115 %							43/16/3

BCDL: 10 Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Upl	lift Max MWFRS Uplif	Max C&C Uplift	Max Uplift	Max Horiz
16	1	5.5 in	1.50 in	1,328 lbs	•	-116 lbs	-202 lbs	-202 lbs	526 lbs .
9	1	5.5 in	1.73 in	1,708 lbs			-108 lbs	-108 lbs	

Material

TC: SYP#1 2 x 4 BC: SYPSSDense 2 x 6 Web: SYP#2 2 x 4 except: SYPSSDense 2 x 6: 17-9

Bracing

TC: Sheathed or Purlins at 2-5-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- 1) This truss has been designed for the effects due to 10 psf bottom chord live load plus dead loads.
- 2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 3) This truss has not been designed for the effects of unbalanced snow loads.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL=1.60
- 5) Minimum storage attic loading has been applied in accordance with IBC 1607.1
- 6) **- Indicates parapet wind loading has been applied to member 17-8

Load Case Lr1: Std Live Load

Distributed Loads

Distributed Loads							
Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Тор	0-0-0	24-6-0	Down	Proj	15 plf	15 plf	
Тор	0-0-0	7-10-8	Down	Proj	19.58 plf	19.58 plf	
Тор	9-10-8	11-5-4	Down	Proj	19.58 plf	19.58 plf	
Тор	13-5-4	15-5-4	Down	Proj	19.58 plf	19.58 plf	
Тор	17-5-4	19-5-4	Down	Proj	19.58 plf	19.58 plf	
Top	21-5-4	24-6-0	Down	Proj	19.58 plf	19.58 plf	

2550 Hwy 33 South P.O. Box 260 Maysville, Missouri 64469 Truss:GR8

Job: 500 NW Chipman Rd REVISED

Date: 02/16/22 10:56:55

Page: 2 of 4

PLYS **SPACING** WGT/PLY 1 $20.75 \, \text{in}$ 158 lbs

SPAN	PITCI	1 O	TY	OHL	OHR	CANTL	CANTR
24-6-0	0.25/1	-	1	0-0-0	0-0-0	0-0-0	0-0-0
Load Case D1:	: Std Dead Load						
Distributed Loads							
Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	24-6-0	Down	Proj	11.25 plf	11.25 plf	
Bot	0-0-0	24-6-0	Down	Proj	7.5 plf	7.5 plf	
Top	0-0-0	7-10-8	Down	Proj	14.69 plf	14.69 plf	
Top	9-10-8	11-5-4	Down	Proj	14.69 plf	14.69 plf	
Top	13-5-4	15-5-4	Down	Proj	14.69 plf	14.69 plf	
Top	17-5-4	19-5-4	Down	Proj	14.69 plf	14.69 plf	
Top	21-5-4	24-6-0	Down	Proj	14.69 plf	14.69 plf	
Bot	0-0-0	7-10-8	Down	Proj	9.79 plf	9.79 plf	
Bot	9-10-8	11-5-4	Down	Proj	9.79 plf	9.79 plf	
Bot	13-5-4	15-5-4	Down	Proj	9.79 plf	9.79 plf	
Bot	17-5-4	19-5-4	Down	Proj	9.79 plf	9.79 plf	
Bot	21-5-4	24-6-0	Down	Proj	9.79 plf	9.79 plf	
User-defined L	oad Case S2: Sr	ow Drift					
Distributed Loads							
Member Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	TribWidth
Top	9-0-0	24-6-0	Down	Proj	41 psf	41 psf	24 in
•	2 0 0	0 0		,	Po.	Po-	_ · ···
Point Loads Member	Location	Direction	Load	Trib Width			
Member	Location	Direction	Loau	THO WIGHT			
User-defined L	oad Case W12:	W1(DOL = 1.6	60)				
Distributed Loads							
Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	1-0-12	N Up	Rake	25.39 plf	25.39 plf	
Тор	1-0-12	7-6-0	NUp	Rake	25.39 plf	25.39 plf	
Top	1-0-12	7-6-0	NUp	Rake	24.87 plf	24.87 plf	
Top	7-6-0	7-10-8	N Úp	Rake	25.39 plf	25.39 plf	
Top	7-6-0	7-10-8	N Úp	Rake	24.87 plf	24.87 plf	
Top	7-10-8	9-10-8	N Up	Rake	25.39 plf	25.39 plf	
Тор	9-10-8	11-5-4	N Up	Rake	25.39 plf	25.39 plf	
Тор	9-10-8	11-5-4	N Up	Rake	24.87 plf	24.87 plf	
Тор	11-5-4	13-5-4	N Up	Rake	25.39 plf	25.39 plf	
Тор	13-5-4	15-0-0	N Up	Rake	25.39 plf	25.39 plf	
Top	13-5-4	15-0-0	NUp	Rake	24.87 plf	24.87 plf	
Тор	15-0-0	15-5-4	N Up	Rake	15.97 plf	15.97 plf	
Тор	15-0-0	15-5-4	N Up	Rake	15.63 plf	15.63 plf	
Top	15-5-4	17-5-4	N Up	Rake	15.97 plf	15.97 plf	
Top	17-5-4	19-5-4	N Up	Rake	15.97 plf	15.97 plf	
Top	17-5-4 10-5-4	19-5-4 21-5-4	N Up	Rake Poke	15.63 plf	15.63 plf	
Top Top	19-5-4 21-5-4	21-5-4 23-5-4	N Up N Up	Rake Rake	15.97 plf	15.97 plf	
	21-5-4	23-5-4	N Up N Up	Rake Rake	15.97 plf 15.63 plf	15.97 plf 15.63 plf	
Top Top	23-5-4	23-3-4 24-6-0	N Up N Up	Rake	15.65 pii 15.97 plf	15.65 pii 15.97 plf	
Web 1-16	0-0-0	2-3-0	Right	Rake	12.74 plf	13.57 plf 12.74 plf	
Web 17-8	0-0-0	4-10-12	Right	Rake	25.93 plf	25.93 plf	
Web 8-9	0-0-0	2-9-2	Right	Rake	15.11 plf	15.11 plf	
	000	-/-		1 4410	10.11 pm	10.71 рп	
Point Loads Member	Location	Direction	Load	Trib Width			
Bot	8-10-8	Up	141 lbs	THO WIGHT			
Bot	8-10-8 12-5-4	Up	141 lbs 163 lbs				
Bot	16-5-4	Uр	170 lbs				
Bot	20-5-4	Uр	170 lbs				
Bot	24-5-4	Down	90 lbs				
Тор	0-0-0	Right	600 lbs				
-~P	0.00	- 45-11	500 103				

2550 Hwy 33 South P.O. Box 260 Maysville, Missouri 64469 Truss:GR8

Job: 500 NW Chipman Rd REVISED

Date: 02/16/22 10:56:56

Page: 3 of 4

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGI/PLY	
24-6-0	0.25/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	20.75 in	158 lbs	

Distributed Load	Load Case W13:		-/				
Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Тор	0-0-0	1-0-12	N Up	Rake	15.97 plf	15.97 plf	
Top	1-0-12	7-10-8	N Úp	Rake	15.97 plf	15.97 plf	
Top	1-0-12	7-10-8	N Úp	Rake	15.63 plf	15.63 plf	
Top	7-10-8	9-6-0	N Up	Rake	15.97 plf	15.97 plf	
Top	9-6-0	9-10-8	N Up	Rake	25.39 plf	25.39 plf	
Top	9-10-8	11-5-4	N Up	Rake	25.39 plf	25.39 plf	
Тор	9-10-8	11-5-4	N Up	Rake	24.87 plf	24.87 plf	
Top	11-5-4	13-5-4	N Up	Rake	25.39 plf	25.39 plf	
Top	13-5-4	15-5-4	N Up	Rake	25.39 plf	25.39 plf	
Top	13-5-4	15-5-4	N Up	Rake	24.87 plf	24.87 plf	
Top	15-5-4	17-0-0	N Up	Rake	25.39 plf	25.39 plf	
Top	17-0-0	17-5-4	N Up	Rake	25.39 plf	25.39 plf	
Top	17-5-4	19-5-4	N Up	Rake	25.39 plf	25.39 plf	
Top	17-5-4	19-5-4	N Up	Rake	24.87 plf	24.87 plf	
Top	19-5-4	21-5-4	N Up	Rake	25.39 plf	25.39 plf	
Top	21-5-4	23-5-4	N Up	Rake	25.39 plf	25.39 plf	
Top	21-5-4	23-5-4	N Up	Rake	24.87 plf	24.87 plf	
Top	23-5-4	24-6-0	N Up	Rake	25.39 plf	25.39 plf	
Web 1-16	0-0-0	2-3-0	Left	Rake	15.11 plf	15.11 plf	
Web 17-8	0-0-0	4-10-12	Left	Rake	38.89 plf	38.89 plf	
Web 8-9	0-0-0	2-9-2	Left	Rake	12.74 plf	12.74 plf	
Point Loads							
Member	Location	Direction	Load	Trib Width			
Bot	8-10-8	Down	48 lbs				
Bot	12-5-4	Down	42 lbs				
Bot	16-5-4	Down	50 lbs				
Bot	20-5-4	Down	57 lbs				
Bot	24-5-4	Up	161 lbs				
Тор	0-0-0	Left	600 lbs				

User-defined Load Case W14: W4(DOL= 1.60)

Distributed 1	l oads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Тор	0-0-0	1-0-12	N Up	Rake	48.89 plf	48.89 plf	
Тор	1-0-12	3-0-0	N Up	Rake	48.89 plf	48.89 plf	
Top	1-0-12	3-0-0	N Up	Rake	47.87 plf	47.87 plf	
Top	3-0-0	7-10-8	N Úp	Rake	43.75 plf	43.75 plf	
Top	3-0-0	7-10-8	N Up	Rake	42.83 plf	42.83 plf	
Тор	7-10-8	9-10-8	N Úp	Rake	43.75 plf	43.75 plf	
Top	9-10-8	11-5-4	N Up	Rake	43.75 plf	43.75 plf	
Top	9-10-8	11-5-4	N Up	Rake	42.83 plf	42.83 plf	
Top	11-5-4	13-5-4	N Up	Rake	43.75 plf	43.75 plf	
Top	13-5-4	15-5-4	N Up	Rake	43.75 plf	43.75 plf	
Тор	13-5-4	15-5-4	N Up	Rake	42.83 plf	42.83 plf	
Top	15-5-4	17-5-4	N Up	Rake	43.75 plf	43.75 plf	
Top	17-5-4	19-5-4	N Up	Rake	43.75 plf	43.75 plf	
Top	17-5-4	19-5-4	N Up	Rake	42.83 plf	42.83 plf	
Top	19-5-4	21-5-4	N Up	Rake	43.75 plf	43.75 plf	
Top	21-5-4	23-5-4	N Up	Rake	43.75 plf	43.75 plf	
Top	21-5-4	23-5-4	N Up	Rake	42.83 plf	42.83 plf	
Top	23-5-4	24-6-0	N Up	Rake	43.75 plf	43.75 plf	
Web 1-16	0-0-0	2-3-0	Left	Rake	27.37 plf	27.37 plf	
Web 17-8	0-0-0	4-10-12	Right	Rake	82.96 plf	82.96 plf	
Web 8-9	0-0-0	2-9-2	Right	Rake	27.37 plf	27.37 plf	
Point Loads							
Member	Location	Direction	Load	Trib Width			
Top	0-0-0	Left	600 lbs		•	•	

Load Combinations

#	Load Combo	Factor
1	D1	1.000
2	D1+Lr1	1.000
3	D1+S1	1.000
4	D1 +S2	1.000
5	0.60 D1 +0.60 W1 [Uplift]	1.000
6	0.60 D1 +0.60 W2 [Uplift]	1.000
7	0.60 D1 +0.60 W4 [Uplift]	1.000
8	0.60 D1 +0.60 W8 [Uplift]	1.000

ALL PERSONS FABRICATING; HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGNAND AVAILABLE FROM EAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

2550 Hwy 33 South P.O. Box 260 Maysville, Missouri 64469

OHR

CANTL

CANTR

Truss:GR8

Job: 500 NW Chipman Rd REVISED

WGT/PLY

158 lbs

Date: 02/16/22 10:56:56

SPACING

20.75 in

Page: 4 of 4

PLYS

1

24-6-0	0.25/12	1	0-0-0	0-0-0	0-0-0	0-0-0	
9	0.60 D1 +0.60 W0 [Uplift]				1.000		
10	0.60 D1 +0.60 W0 [Uplift]				1.000		
11	0.60 D1 +0.60 W0 [Uplift]				1.000		
12	D1 +L10*1				1.000		
13	D1 +Lr1 +L10*1				1.000		
14	D1 +S1 +L10*1				1.000		
15	D1 +S2 +L10*1				1.000		
16	D1 +UR1				1.000		
17	D1 +UR2				1.000		
18	D1 +AS10*1				1.000		
19	D1 +Lr1 +AS10*1				1.000		
20	D1 +S1 +AS10*1				1.000		
21	D1 + S2 + AS10*1				1.000		

OHL

Member Forces Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table. 0.286 -2,602 lbs 3-5 0.490 -4,695 lbs 0.324 -2,389 lbs -4,424 lbs 0.420 920 lbs (433 lbs 0.276 0.197 2,385 lbs (420 lbs) 11-12 0.386 4,691 lbs 0.262 2,601 lbs 10-11 0.323 3,977 lbs (-518 lbs) 12-14 0.378 4,422 lbs (449 lbs) 15-16 0.053 -516 lbs 1-16 0.157 -1,246 lbs 0.046 -352 lbs 0.637 -1,821 lbs (-363 lbs) 2,839 lbs 310 lbs (-218 lbs) 7-10 (-81 lbs) 1-15 0.697 3-12 0.076 0.252 1.028 lbs 0.123 -964 lbs 0.278 -809 lbs 7-9 0.899 -2,781 lbs 2-15 (-243 lbs) 6-11 (-19 lbs 1,993 lbs 520 lbs

Truss to Truss Connection Summary

Carried Truss	Carrying Chord	Carrying Offset
BP6	BC	8-10-8
BP23	BC	12-5-4
BP21	BC	16-5-4
BP19	BC	20-5-4
DTO	P.C	24.5.4

Notes

SPAN

PITCH

QTY

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 30% (Cq = 0.70).
- 3) Provide adequate drainage to prevent ponding.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 "of each web panel point.
- 6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 7) Listed wind uplift reactions based on MWFRS & C&C loading.
- 8) Parapet TL: 0.58 in, 2L/210 (17-8), Allowable 2L/120.

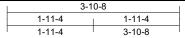
2550 Hwy 33 South P.O. Box 260 Maysville, Missouri 64469 Truss:GR9

Job: 500 NW Chipman Rd REVISED

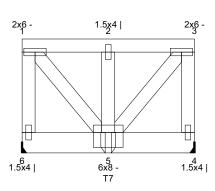
Date: 02/16/22 10:56:57

Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
3-10-8	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	15.75 in	29 lbs









All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI	Deflection	1	L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code: I	IBC 2018/	TC: 0.03 (1-2)	Vert TL:	0 in	L/999	5	L/240
TCLL: 20	7	TPI 1-2014	BC: 0.03 (5-6)	Vert LL:	0 in	L/999	(4-5)	L/360
TCDL: 15	Rep Mbr: 1	No	Web: 0.09 (1-5)	Horz TL:	0 in		4	
BCLL: 0	Lumber D.O.L.: 1	115 %						02/1

Reaction

BCDL: 10

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uj	plift Max MWFRS UpliftM	ax C&C Uplift	Max Uplift	Max Horiz
6	1	1.5 in		391 lbs		-15 lbs		-15 lbs	56 lbs
4	1	1.5 in		391 lbs		-49 lbs		-49 lbs	

Material

TC: SYP#1 2 x 4 BC: SYPSSDense 2 x 6 Web: SYP#2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- $1) This truss has been designed for the effects due to \\10 psf bottom chord live load plus dead loads.$
- 2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 3) This truss has not been designed for the effects of unbalanced snow loads.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL=1.60
- 5) Minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case Lr1: Std Live Load

Distributed Load

Ton	0.00	2 10 9	Dorrm	D.c.:	26.25 -16	26.25 -16	
Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
DISH IDUICU LOXUS							

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	TribWidth
Тор	0-0-0	3-10-8	Down	Proj	19.69 plf	19.69 plf	
Bot	0-0-0	3-10-8	Down	Proi	13 13 nlf	13 13 plf	

Member Forces Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1			I					
10									
BC									
DC									
Web	1.6	0.048	-331 lbs	ī	2.5	0.090	369 lbs	(-58 lbs)	
WED	1-0	0.040	-551 108		3-3	0.090	309 IUS	(-20 108)	
	1.5	0.090	369 lbs	(-14 lbs)	2.4	0.048	-331 lbs		· ·
	1-5	0.090	309 IUS	(-14 IUS)	J -4	0.040	-551 IUS		I I

Truss to Truss Connection Summary

Carried Truss	Carrying Chord	Carrying Offse
T7	BC	1-11-4

"mmmuni

Vivco Components LLC. 2550 Hwy 33 South

P.O. Box 260 Maysville, Missouri 64469

OHR

0-0-0

CANTL

0-0-0

CANTR

0-0-0

Truss:GR9

Job: 500 NW Chipman Rd REVISED

Date: 02/16/22 10:56:57

Page: 2 of 2

PLYS SPACING WGT/PLY
1 15.75 in 29 lbs

Notes

SPAN

3-10-8

- $1) \, Unless \, noted \, otherwise, do \, not \, cut \, or \, alter \, any \, truss \, member \, or \, plate \, without \, prior \, approval \, from \, a \, Professional \, Engineer.$
- 2) The fabrication tolerance for this roof truss is 30% (Cq = 0.70).

PITCH

0/12

3) Hangers are for graphical intrepretation only. Install hangers per manufacturer's recommendations.

QTY

- 4) Provide adequate drainage to prevent ponding.
- 5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.

OHL

0-0-0

7) Listed wind uplift reactions based on MWFRS & C&C loading.

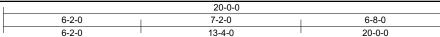
2550 Hwy 33 South P.O. Box 260 Maysville, Missouri 64469 Truss:T1

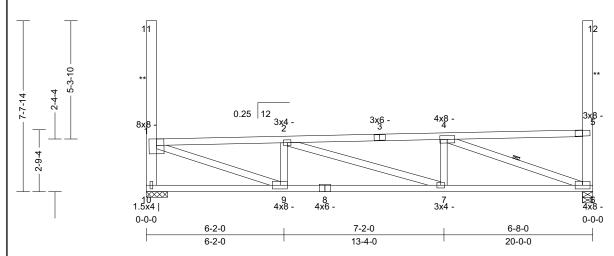
Job: 500 NW Chipman Rd REVISED

Date: 02/16/22 10:56:59

Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
20-0-0	0.25/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	122 lbs





All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI	Deflection	n	L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2018/	TC: 0.91 (4-5)	Vert TL:	0.34 in	L/657	(7-8)	L/240
TCDL: 15		TPI 1-2014	BC: 0.82 (6-7)	Vert LL:	0.12 in UP	L/999	(7-8)	L/360
BCLL: 0	Rep Mbr:	No	Web: 0.93 (11-1)	Horz TL:	0.05 in		6	
BCDL: 10	Lumber D.O.I	L.:115 %						

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplit	ft Max MWFRS Uplift!	Max C&C Uplift	Max Uplift	Max Horiz
6	1	5.5 in	1.63 in	1,383 lbs		-70 lbs	-219 lbs	-219 lbs	
10	1	11.5 in	N/A	990 lbs		-160 lbs	-236 lbs	-236 lbs	-675 lbs 🚊
10	1	11.5 in	N/A	378 lbs	•		-30 lbs	-30 lbs	-793 lbs 🚆

Material

TC: SYP#1 2 x 4 BC: SYP#1 2 x 4 Web: SYP#2 2 x 4 except:

SYPSSDense 2 x 6: 11-10, 12-6

Bracing

TC: Sheathed or Purlins at 2-3-0, Purlin design by Others. BC: Sheathed or Purlins at 5-9-0, Purlin design by Others.

Web: One Midpoint Row: 4-6

Loads

- 1) This truss has been designed for the effects due to 10 psf bottom chord live load plus dead loads.
- 2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 3) This truss has not been designed for the effects of unbalanced snow loads.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL=1.60
- 5) Minimum storage attic loading has been applied in accordance with IBC 1607.1
- 6) **- Indicates parapet wind loading has been applied to members 11-1 & 12-5

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Тор	14-4-0	16-6-0	Down	Proj	35.64 plf	35.64 plf	
TT 1.C 1	I 10 00 D						

User-defined Load Case S2: Drift

Distributed Loads Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Тор	0-0-0	20-0-0	Down	Proi	20 psf	20 psf	24 in
Тор	0-0-0	20-0-0	Down	Proj	5 psf	5 psf	24 in
Тор	0-0-0	4-6-0	Down	Proj	32 psf	0 psf	24 in
Тор	15-6-0	20-0-0	Down	Proj	0 psf	32 psf	24 in
Point Loads				•	•	Î	
Member	Location	Direction	Load	Trib Width			

2550 Hwy 33 South P.O. Box 260 Maysville, Missouri 64469 Truss:T1

Job: 500 NW Chipman Rd REVISED

Date: 02/16/22 10:56:59

Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
20-0-0	0.25/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	122 lbs

Load Co	mbinations	
#	Load Combo	Factor
1	D1	1.000
2	D1 +Lr1	1.000
3	D1 +S1	1.000
4	D1 +S2	1.000
5	0.60 D1 + 0.60 W1 [Uplift]	1.000
6	0.60 D1 +0.60 W2 [Uplift]	1.000
7	0.60 D1 + 0.60 W4 [Uplift]	1.000
8	0.60 D1 +0.60 W8 [Uplift]	1.000
9	D1 +L10*1	1.000
10	D1 +Lr1 +L10*1	1.000
11	D1 +S1 +L10*1	1.000
12	D1 + S2 + L10*1	1.000
13	D1 +UR1	1.000
14	D1 + UR2	1.000
15	D1 +AS10*1	1.000
16	D1 +Lr1 +AS10*1	1.000
17	D1 +S1 +AS10*1	1.000
18	D1 +S2 +AS10*1	1.000

Mer	nber 1	Forces	Table	indicates: M	ember]	D, max CS	I, max axial fo	orce, (max con	npr. forc	e if different	from max ax	ial force). Onl	ly forces greater than 300lbs are shown in this tab	ole.
TC	1-2	0.895	-2,485 lbs		2-4	0.820	-2,601 lbs		4-5	0.913	1,047 lbs	(494 lbs)		
BC	6-7	0.822	2,595 lbs	(-979 lbs)	7-9	0.803	2,479 lbs	(-1,131 lbs)	9-10	0.400	793 lbs	(-726 lbs)		
Web	1-10	0.638	-1,206 lbs		2-7	0.244	362 lbs	(-253 lbs)						
	1-9	0.644	2,624 lbs	(429 lbs)	4-6	0.630	-2,761 lbs							
	2.0	0.088	-6/10 lbc		5-6	0.567	-328 lbc							

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 30% (Cq = 0.70).
- 3) Provide adequate drainage to prevent ponding.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to
- the plane of the truss. Lateral braces shall be installed within 6 "of each web panel point.
- 6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 7) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 8) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
- 9) Listed wind uplift reactions based on MWFRS & C&C loading.
- 10) Parapet TL: 0.86 in, 2L/151 (11-1), Allowable 2L/120.

2550 Hwy 33 South P.O. Box 260

Maysville, Missouri 64469

Truss:T2

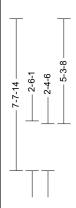
Job: 500 NW Chipman Rd REVISED

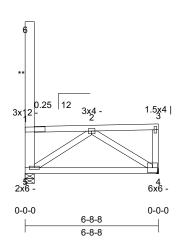
Date: 02/16/22 10:57:00

Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
6-8-8	0.25/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	48 lbs

1 6	-8-8
3-4-4	3-4-4
3-4-4	6-8-8





All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI	Deflection	n	L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2018/	TC: 0.27 (1-2)	Vert TL:	0.17 in	L/427	(4-5)	L/240
TCDL: 15		TPI 1-2014	BC: 0.50 (4-5)	Vert LL:	0.09 in	L/855	(4-5)	L/360
BCLL: 0	Rep Mbr:	No	Web: 0.92 (6-1)	Horz TL:	0.01 in		4	
BCDL: 10	Lumber D.O.l	L.:115 %						

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Up	olift Max MWFRS Uplift!	Max C&C Uplift	Max Uplift	Max Horiz
5	1	5.5 in	1.50 in	471 lbs	•	-203 lbs	•	-203 lbs	534 lbs
4	1	1.5 in		403 lbs		-145 lbs	-557 lbs	-557 lbs	

Material

TC: SYP#1 2 x 4 BC: SYP#1 2 x 4

Web: SYP#2 2 x 4 except: SYPSSDense 2 x 6: 6-5 **Bracing**

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.

BC: Sheathed or Purlins at 7-0-0, Purlin design by Others.

Loads

- 1) This truss has been designed for the effects due to 10 psf bottom chord live load plus dead loads.
- 2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used $for the \ roof \ snow \ load, DOL = 1.15. \ If the \ roof \ configuration \ differs \ from \ hip/gable, Building \ Designer \ shall \ verify \ snow \ loads \ .$
- 3) This truss has not been designed for the effects of unbalanced snow loads.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 5) Minimum storage attic loading has been applied in accordance with IBC 1607.1
- 6) **- Indicates parapet wind loading has been applied to member 6-1

User-defined Load Case S2: Drift

Dictributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	6-8-8	Down	Proj	20 psf	20 psf	24 in
Top	0-0-0	4-6-0	Down	Proj	32 psf	0 psf	24 in
Point Loads							
Member	Location	Direction	Load	Trib Width			

Load Combinations

#	Load Combo	Factor
1	D1	1.000
2	Dl +Lrl	1.000
3	D1+S1	1.000
4	D1+S2	1.000
5	0.60 D1 +0.60 W1 [Uplift]	1.000
6	0.60 D1 +0.60 W2 [Uplift]	1.000

2550 Hwy 33 South P.O. Box 260 Maysville, Missouri 64469 Truss:T2

Job: 500 NW Chipman Rd REVISED

Date: 02/16/22 10:57:01

Page: 2 of 2

SPA1 6-8-8		QTY 1	OHL 0-0-0	OHR 0-0-0	CANTL 0-0-0	CANTR 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 48 lbs
7	0.60 D1 +0.60 W4 [Uplift]				1.000				
8	0.60 D1 + 0.60 W8 [Uplift]				1.000				
9	D1+L10*1				1.000				
10	D1 +Lr1 +L10*1				1.000				
11	D1+S1+L10*1				1.000				
12	D1 +S2 +L10*1				1.000				
13	D1+UR1				1.000				
14	D1 + UR2				1.000				
15	D1 +AS10*1				1.000				
16	D1 +Lr1 +AS10*1				1.000				
17	D1+S1+AS10*1				1.000				
18	D1 +S2 +AS10*1				1.000				

Member Forces Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.268	1,295 lbs	(-598 lbs)		
BC	4-5	0.497	-738 lbs			
Web	2-5	0.133	-549 lbs			
	2-4	0.168	955 lbs	(-319 lbs)		

- $1) \, Unless \, noted \, otherwise, do \, not \, cut \, or \, alter \, any \, truss \, member \, or \, plate \, without \, prior \, approval \, from \, a \, Professional \, Engineer.$
- 2) The fabrication tolerance for this roof truss is 30% (Cq = 0.70).
- 3) Hanger is for graphical interpretation only. Install hanger per manufacturer's recommendation.
- 4) Provide adequate drainage to prevent ponding.
- 5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 6) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6" of each web panel point.
- 7) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 8) Listed wind uplift reactions based on MWFRS & C&C loading.
- 9) Parapet TL: 0.82 in, 2L/160 (6-1), Allowable 2L/120.

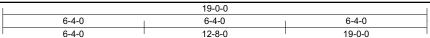
2550 Hwy 33 South P.O. Box 260 Maysville, Missouri 64469 Truss:T3

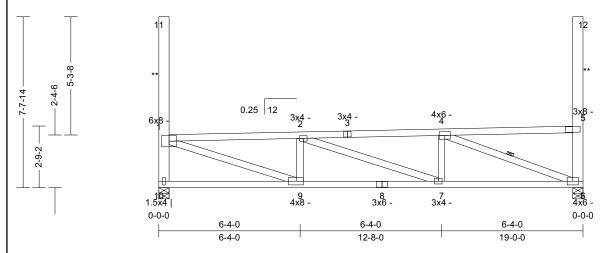
Job: 500 NW Chipman Rd REVISED

Date: 02/16/22 10:57:02

Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
19-0-0	0.25/12	4	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	117 lbs
	1			19-0	-0		1		





All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI	Deflectio	n	L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2018/	TC: 0.78 (1-2)	Vert TL:	0.22 in	L/999	(8-9)	L/240
TCDL: 15		TPI 1-2014	BC: 0.59 (7-9)	Vert LL:	0.1 in UP	L/999	(8-9)	L/360
BCLL: 0	Rep Mbr:	Yes	Web: 0.92 (11-1)	Horz TL:	0.04 in		6	
BCDL: 10	Lumber DO1	[.:115%						

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Upli	ft Max MWFRS UpliftN	Max C&C Uplift	Max Uplift	Max Horiz
10	1	5.5 in	1.50 in	1,182 lbs	•	-110 lbs	-271 lbs	-271 lbs	491 lbs
6	1	5.5 in	1.50 in	1.182.lbs		-112.lbs	-250 lbs	-250 lbs	

Material

TC: SYP#1 2 x 4 BC: SYP#1 2 x 4 Web: SYP#2 2 x 4 except:

SYPSSDense 2 x 6: 11-10, 12-6

Bracing

TC: Sheathed or Purlins at 2-10-0, Purlin design by Others. BC: Sheathed or Purlins at 6-1-0, Purlin design by Others.

Web: One Midpoint Row: 4-6

Loads

- 1) This truss has been designed for the effects due to 10 psf bottom chord live load plus dead loads.
- 2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 3) This truss has not been designed for the effects of unbalanced snow loads.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL=1.60
- 5) Minimum storage attic loading has been applied in accordance with IBC 1607.1
- 6) **- Indicates parapet wind loading has been applied to members 11-1 & 12-5

User-defined Load Case S2: Drift

Distributed Loads

Distributed Data							
Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	TribWidth
Тор	0-0-0	19-0-0	Down	Proj	20 psf	20 psf	24 in
Тор	0-0-0	4-6-0	Down	Proj	32 psf	0 psf	24 in
Top	14-6-0	19-0-0	Down	Proj	0 psf	32 psf	24 in
Point Loads Member	Location	Direction	Load	Trib Width			

Load Combinations

#	Load Combo	Factor
1	D1	1.000
2	D1 +Lr1	1.000
3	D1 +S1	1.000
4	D1+S2	1.000

2550 Hwy 33 South P.O. Box 260 Maysville, Missouri 64469 Truss:T3

Job: 500 NW Chipman Rd REVISED

Date: 02/16/22 10:57:02

Page: 2 of 2

SPAN 19-0-0		QTY 4	OHL 0-0-0	OHR 0-0-0	CANTL 0-0-0	CANTR 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 117 lbs
	0.60 D1 +0.60 W1 [Uplift]				1.000				
6	0.60 D1 +0.60 W2 [Uplift]				1.000				
7	0.60 D1 +0.60 W4 [Uplift]				1.000				
8	0.60 D1 +0.60 W8 [Uplift]				1.000				
9	D1 +L10*1				1.000				
10	D1 +Lr1 +L10*1				1.000				
11	D1+S1+L10*1				1.000				
12	D1 +S2 +L10*1				1.000				
13	D1 +UR1				1.000				
14	D1 + UR2				1.000				
15	D1 +AS10*1				1.000				
16	D1 +Lr1 +AS10*1				1.000				
17	D1 +S1 +AS10*1				1.000				
18	D1 +S2 +AS10*1				1.000				

791 lbs

(-716 lbs)

 Member Forces
 Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

 TC
 1-2
 0.777
 -2,200 lbs
 24
 0.469
 -2,081 lbs
 45
 0.606
 1,052 lbs
 (496 lbs)
 (496 lbs)

0.268

BC	6-7	0.568	2,075 lbs	(-1,009 lbs)	7-9	0.586	2,194 lbs	(-1,133 lbs)	9-10
Web	1-10	0.636	-1,071 lbs		2-7	0.241	-312 lbs		
	1-9	0.569	2,317 lbs	(441 lbs)	4-7	0.077	312 lbs	(-17 lbs)	i
	2-9	0.067	-487 lbs		4-6	0.471	-2 222 lbs		i

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 30% (Cq = 0.70).
- 3) Provide adequate drainage to prevent ponding.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6" of each web panel point.
- 6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 7) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 8) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
- 9) Listed wind uplift reactions based on MWFRS & C&C loading.
- 10) Parapet TL: 0.86 in, 2L/152 (11-1), Allowable 2L/120.

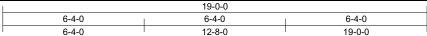
2550 Hwy 33 South P.O. Box 260 Maysville, Missouri 64469 Truss:T4

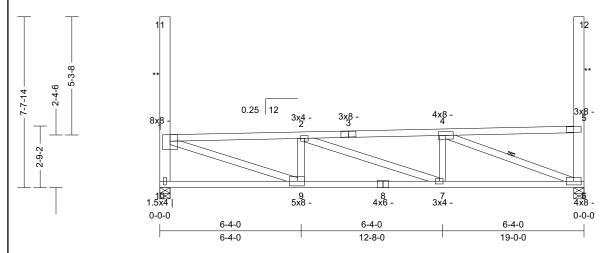
Job: 500 NW Chipman Rd REVISED

Date: 02/16/22 10:57:03

Page: 1 of 2

10.0.0										_
19-0	0.25/12	2	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	119 lbs	
SPA		QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WG1/PLY	





All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI	Deflectio	n	L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2018/	TC: 0.79 (1-2)	Vert TL:	0.26 in	L/831	(8-9)	L/240
TCDL: 15		TPI 1-2014	BC: 0.74 (7-9)	Vert LL:	0.1 in UP	L/999	(8-9)	L/360
BCLL: 0	Rep Mbr:	No	Web: 0.92 (11-1)	Horz TL:	0.05 in		6	
BCDL: 10	Lumber D.O.I	[4:115%						

\mathbf{r}		٠.	
K	eac	TI(or

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplif	t Max MWFRS Upli	iftMax C&C Uplift	Max Uplift	Max Horiz
10	1	5.5 in	1.62 in	1,373 lbs		•	-156 lbs	-156 lbs	491 lbs
6	1	5.5 in	1.55 in	1.315 lbs		-32 lbs	-170 lbs	-170 lbs	

Material

TC: SYP#1 2 x 4 BC: SYP#1 2 x 4 Web: SYP#2 2 x 4 except:

SYPSSDense 2 x 6: 11-10, 12-6

Bracing

TC: Sheathed or Purlins at 2-7-0, Purlin design by Others. BC: Sheathed or Purlins at 6-11-0, Purlin design by Others.

Web: One Midpoint Row: 4-6

Loads

1) This truss has been designed for the effects due to 10 psf bottom chord live load plus dead loads.

2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 - 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used $for the \, roof \, snow \, load, \, DOL = 1.15. \,\, If \, the \, roof \, configuration \, differs \, from \, hip/gable, \, Build \, ing \, Designer \, shall \, verify \, snow \, loads \,.$

3) This truss has not been designed for the effects of unbalanced snow loads.

4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60

5) Minimum storage attic loading has been applied in accordance with IBC 1607.1

6) **- Indicates parapet wind loading has been applied to members 11-1 & 12-5

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	4-10-0	10-10-0	Down	Proj	54.17 plf	54.17 plf	

User-defined Load Case S2: Drift

Dictributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Тор	0-0-0	19-0-0	Down	Proj	20 psf	20 psf	24 in
Top	0-0-0	4-6-0	Down	Proj	32 psf	0 psf	24 in
Top	14-6-0	19-0-0	Down	Proj	0 psf	32 psf	24 in
Point Loads							
Member	Location	Direction	Load	Trib Width			

2550 Hwy 33 South P.O. Box 260 Maysville, Missouri 64469 Truss:T4

Job: 500 NW Chipman Rd REVISED

Date: 02/16/22 10:57:04

Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
19-0-0	0.25/12	2	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	119 lbs

Load Co	Load Combinations									
#	Load Combo	Factor								
1	D1	1.000								
2	Dl +Lrl	1.000								
3	D1+S1	1.000								
4	D1 +S2	1.000								
5	0.60 D1 +0.60 W1 [Uplift]	1.000								
6	0.60 D1 +0.60 W2 [Uplift]	1.000								
7	0.60 D1 +0.60 W4 [Uplift]	1.000								
8	0.60 D1 +0.60 W8 [Uplift]	1.000								
9	D1 +L10*1	1.000								
10	D1 + Lr1 + L10*1	1.000								
11	D1 +S1 +L10*1	1.000								
12	D1 +S2 +L10*1	1.000								
13	D1 + UR1	1.000								
14	D1 +UR2	1.000								
15	D1 +AS10*1	1.000								
16	D1 +Lr1 +AS10*1	1.000								
17	D1 +S1 +AS10*1	1.000								
18	D1 +S2 +AS10*1	1.000								

Member Forces Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table											y forces greater than 300lbs are shown in this table.		
TC	1-2	0.786	-2,749 lbs		2-4	0.739	-2,462 lbs		4-5	0.669	1,052 lbs	(496 lbs)	
BC	6-7	0.696	2,453 lbs	(-782 lbs)	7-9	0.743	2,743 lbs	(-804 lbs)	9-10	0.302	791 lbs	(-716 lbs)	
Web	1-10	0.639	-1,261 lbs		2-7	0.381	-494 lbs						_
	1-9	0.711	2,897 lbs	(-93 lbs)	4-7	0.091	372 lbs						
	2-9	0.093	-677 lbs		4-6	0.557	-2,627 lbs						

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 30% (Cq = 0.70).
- 3) Provide adequate drainage to prevent ponding.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to
- the plane of the truss. Lateral braces shall be installed within 6 "of each web panel point.
- 6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 7) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 8) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
- 9) Listed wind uplift reactions based on MWFRS & C&C loading.
- 10) Parapet TL: 0.84 in, 2L/154 (11-1), Allowable 2L/120.

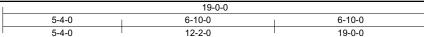
2550 Hwy 33 South P.O. Box 260 Maysville, Missouri 64469 Truss:T5

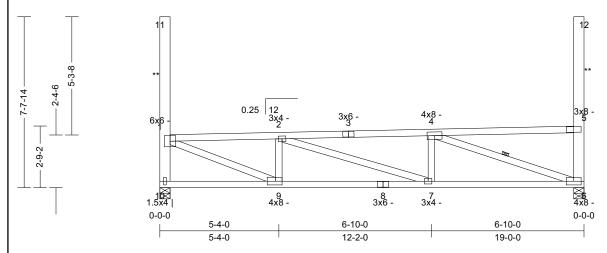
Job: 500 NW Chipman Rd REVISED

Date: 02/16/22 10:57:05

Page: 1 of 2

	40.00											
19-0-0	0.25/12	2	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	118 lbs			
40.00	0.25 /12	`_						- 4.1	440.7			
SPAN	PITCH	OTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WG1/PLY			





All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection	1	L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2018/	TC:	0.80 (1-2)	Vert TL:	0.24 in	L/899	(8-9)	L/240
TCDL: 15		TPI 1-2014	BC:	0.74 (6-7)	Vert LL:	0.1 in UP	L/999	(8-9)	L/360
BCLL: 0	Rep Mbr:	No	Web:	0.92 (11-1)	Horz TL:	0.04 in		6	
BCDL: 10	Lumber D.O.L	:115 %							

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	
10	1	5.5 in	1.50 in	1,274 lbs	•	-55 lbs	-216 lbs	-216 lbs	_
6	1	5.5 in	1.50 in	1.195 lbs		-104 lbs	-242 lbs	-242 lbs	

Material

TC: SYP#1 2 x 4 BC: SYP#1 2 x 4

Web: SYP#2 2 x 4 except: SYPSSDense 2 x 6: 11-10, 12-6

Bracing

TC: Sheathed or Purlins at 2-10-0, Purlin design by Others. BC: Sheathed or Purlins at 6-0-0, Purlin design by Others.

Max Horiz 491 lbs

Web: One Midpoint Row: 4-6

Loads 1) This truss has been designed for the effects due to 10 psf bottom chord live load plus dead loads.

- 2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used $for the \, roof \, snow \, load, \, DOL = 1.15. \,\, If \, the \, roof \, configuration \, differs \, from \, hip/gable, \, Build \, ing \, Designer \, shall \, verify \, snow \, loads \,.$
- 3) This truss has not been designed for the effects of unbalanced snow loads.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 5) Minimum storage attic loading has been applied in accordance with IBC 1607.1
- 6) **- Indicates parapet wind loading has been applied to members 11-1 & 12-5

Load Case D1: Std Dead Load

Distributed Loads

Distributed Data	Distributed Exacts												
Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width						
Ton	1-5-0	3-7-0	Down	Proi	48 46 nlf	48 46 plf							

User-defined Load Case S2: Drift

Dictributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Тор	0-0-0	19-0-0	Down	Proj	20 psf	20 psf	24 in
Тор	0-0-0	4-6-0	Down	Proj	32 psf	0 psf	24 in
Top	14-6-0	19-0-0	Down	Proj	0 psf	32 psf	24 in
Point Loads Member	Location	Direction	Load	Trib Width			

2550 Hwy 33 South P.O. Box 260 Maysville, Missouri 64469 Truss:T5

Job: 500 NW Chipman Rd REVISED

Date: 02/16/22 10:57:05

Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
19-0-0	0.25/12	2	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	118 lbs

Load Co	ombinations	
#	Load Combo	Factor
1	D1	1.000
2	D1 +Lr1	1.000
3	D1 +S1	1.000
4	D1 +S2	1.000
5	0.60 D1 +0.60 W1 [Uplift]	1.000
6	0.60 D1 +0.60 W2 [Uplift]	1.000
7	0.60 D1 +0.60 W4 [Uplift]	1.000
8	0.60 D1 +0.60 W8 [Uplift]	1.000
9	D1 +L10*1	1.000
10	D1 +Lr1 +L10*1	1.000
11	D1 + S1 + L10*1	1.000
12	D1 + S2 + L10*1	1.000
13	D1 +UR1	1.000
14	D1 +UR2	1.000
15	D1 +AS10*1	1.000
16	D1 +Lr1 +AS10*1	1.000
17	D1 +S1 +AS10*1	1.000
18	D1 +S2 +AS10*1	1.000

Mer	nber 1	Forces	Table	e indicates: M	ember]	ID, max CS	I, max axial fo	orce, (max con	npr. forc	e if differen	t from max axi	al force). Onl	y forces greater than 300lbs are shown in this table.
TC	1-2	0.802	-2,107 lbs		2-4	0.644	-2,234 lbs		4-5	0.793	1,052 lbs	(496 lbs)	
BC	6-7	0.736	2,229 lbs	(-1,024 lbs)	7-9	0.715	2,101 lbs	(-1,047 lbs)	9-10	0.271	791 lbs	(-716 lbs)	
Web	1-10	0.638	-1,184 lbs		2-7	0.326	-371 lbs						•
	1-9	0.557	2,268 lbs	(-357 lbs)	4-6	0.559	-2,363 lbs						
	120	0.000	COO 11		ı				ı			•	

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 30% (Cq = 0.70).
- 3) Provide adequate drainage to prevent ponding.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to
- the plane of the truss. Lateral braces shall be installed within 6 "of each web panel point.
- 6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 7) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 8) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
- 9) Listed wind uplift reactions based on MWFRS & C&C loading.
- 10) Parapet TL: 0.86 in, 2L/152 (11-1), Allowable 2L/120.

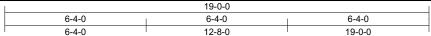
2550 Hwy 33 South P.O. Box 260 Maysville, Missouri 64469 Truss:T6

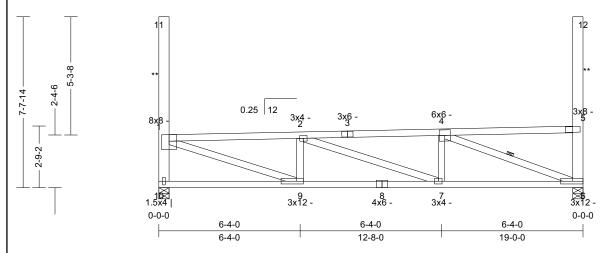
Job: 500 NW Chipman Rd REVISED

Date: 02/16/22 10:57:07

Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY	
19-0-0	0.25/12	3	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	119 lbs	
	1	1								





All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI	Deflection	n	L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2018/	TC: 0.99 (1-2)	Vert TL:	0.27 in	L/808	(8-9)	L/240
TCDL: 15		TPI 1-2014	BC: 0.66 (6-7)	Vert LL:	0.1 in UP	L/999	(8-9)	L/360
BCLL: 0	Rep Mbr:	Yes	Web: 0.92 (11-1)	Horz TL:	0.05 in		6	
BCDL: 10	Lumber D.O.I	L.:115 %						

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	: Max MWFRS Uplift!	Max C&C Uplift	Max Uplift	Max Horiz
10	1	5.5 in	1.68 in	1,428 lbs		-36 lbs	-197 lbs	-197 lbs	491 lbs
6	1	5.5 in	1.66 in	1 409 lbs			-129 lbs	-129 lbs	

Material

TC: SYP#1 2 x 4 BC: SYP#1 2 x 4 Web: SYP#2 2 x 4 except:

SYPSSDense 2 x 6: 11-10, 12-6

Bracing

TC: Sheathed

BC: Sheathed or Purlins at 6-9-0, Purlin design by Others.

Web: One Midpoint Row: 4-6

Loads

- 1) This truss has been designed for the effects due to 10 psf bottom chord live load plus dead loads.
- 2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 3) This truss has not been designed for the effects of unbalanced snow loads.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL=1.60
- 5) Minimum storage attic loading has been applied in accordance with IBC 1607.1
- 6) **- Indicates parapet wind loading has been applied to members 11-1 & 12-5

Load Case D1: Std Dead Load

Distributed Loads

Distributed Dates												
Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width					
Top	8-9-0	14-9-0	Down	Proi	54.17 plf	54.17 plf						

User-defined Load Case S2: Drift

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Тор	0-0-0	19-0-0	Down	Proj	20 psf	20 psf	24 in
Тор	0-0-0	7-2-0	Down	Proj	41 psf	0 psf	24 in
Top	14-6-0	19-0-0	Down	Proj	0 psf	32 psf	24 in
Point Loads							
Member	Location	Direction	Load	Trib Width			



2550 Hwy 33 South P.O. Box 260 Maysville, Missouri 64469 Truss:T6

Job: 500 NW Chipman Rd REVISED

Date: 02/16/22 10:57:07

Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
19-0-0	0.25/12	3	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	119 lbs

Load Co	Load Combinations										
#	Load Combo	Factor									
1	D1	1.000									
2	D1 +Lr1	1.000									
3	D1+S1	1.000									
4	D1 +S2	1.000									
5	0.60 D1 +0.60 W1 [Uplift]	1.000									
6	0.60 D1 +0.60 W2 [Uplift]	1.000									
7	0.60 D1 +0.60 W4 [Uplift]	1.000									
8	0.60 D1 +0.60 W8 [Uplift]	1.000									
9	D1 +L10*1	1.000									
10	D1 + Lr1 + L10*1	1.000									
11	D1 +S1 +L10*1	1.000									
12	D1 +S2 +L10*1	1.000									
13	D1 +UR1	1.000									
14	D1 + UR2	1.000									
15	D1 +AS10*1	1.000									
16	D1 +Lr1 +AS10*1	1.000									
17	D1+S1+AS10*1	1.000									
18	D1 +S2 +AS10*1	1.000									

Men	nber 1	Forces	Table	indicates: M	ember	ID, max CS	I, max axial fo	rce, (max con	npr. forc	e if differen	t from max ax	ial force). Onl	y forces greater than 300lbs are shown in this table.
TC	1-2	0.990	-2,729 lbs		2-4	0.649	-2,665 lbs		4-5	0.641	1,052 lbs	(497 lbs)	
BC	6-7	0.657	2,659 lbs	(-694 lbs)	7-9	0.650	2,722 lbs	(-916 lbs)	9-10	0.263	791 lbs	(-716 lbs)	
Web	1-10	0.638	-1,315 lbs		2-7	0.155	401 lbs	(-202 lbs)					-
	1-9	0.706	2,875 lbs	(-211 lbs)	4-6	0.604	-2,847 lbs						
	2-9	0.091	-668 lbs										

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 30% (Cq = 0.70).
- 3) Provide adequate drainage to prevent ponding.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to
- the plane of the truss. Lateral braces shall be installed within 6 "of each web panel point.
- 6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 7) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 8) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
- 9) Listed wind uplift reactions based on MWFRS & C&C loading.
- 10) Parapet TL: 0.85 in, 2L/154 (11-1), Allowable 2L/120.

2550 Hwy 33 South P.O. Box 260 Maysville, Missouri 64469 Truss:T7

Job: 500 NW Chipman Rd REVISED

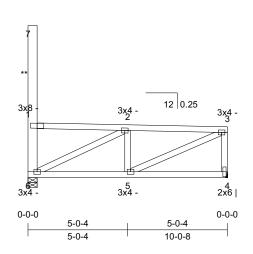
Date: 02/16/22 10:57:09

Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
10-0-8	-0.25/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	63 lbs

10-	-0-8
5-0-4	5-0-4
5-0-4	10-0-8





All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI	Deflection	n	L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2018/	TC: 0.46 (1-2)	Vert TL:	0.05 in	L/999	(4-5)	L/240
TCDL: 15		TPI 1-2014	BC: 0.35 (5-6)	Vert LL:	0.02 in	L/999	(4-5)	L/360
BCLL: 0	Rep Mbr:	No	Web: 0.75 (7-1)	Horz TL:	0.01 in		4	
BCDL: 10	Lumber DO	L :115 %						

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Up	olift Max MWFRS Uplift!	Max C&C Uplift	Max Uplift	Max Horiz
6	1	5.5 in	1.50 in	720 lbs	•	-101 lbs	•	-101 lbs	480 lbs
4	1	1.5 in		621 lbs		-63 lbs	-449 lbs	-449 lbs	

Material

TC: SYP#1 2 x 4 BC: SYP#1 2 x 4 Web: SYP#2 2 x 4 except: SYP SSDense 2 x 6: 7-6

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others. BC: Sheathed or Purlins at 7-8-0, Purlin design by Others.

Loads

- 1) This truss has been designed for the effects due to 10 psf bottom chord live load plus dead loads.
- 2) This truss has been designed for the effects of balanced (20 psf) flat roof snow loads in accordance with ASCE7 16 except as noted, with the following user defined input: 20 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 3) This truss has not been designed for the effects of unbalanced snow loads.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL=1.60
- 5) Minimum storage attic loading has been applied in accordance with IBC 1607.1
- 6) **- Indicates parapet wind loading has been applied to member 7-1

Load Case D1: Std Dead Load

Distributed Loads

Manahan		Location 2	Dimention	Commond	Ctout I and	End Lood	Tails XX 5.44s
Member	Location I	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Ton	3-6-0	5-8-0	Down	Proi	46.15 nlf	46.15 plf	

User-defined Load Case S2: Drift

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Тор	0-0-0	10-0-8	Down	Proj	20 psf	20 psf	24 in
Top	0-0-0	4-6-0	Down	Proj	32 psf	0 psf	24 in
Point Loads							
Member	Location	Direction	Load	Trib Width			

Load Combinations

#	Load Combo	Factor
1	D1	1.000
2	Dl +Lrl	1.000

2550 Hwy 33 South P.O. Box 260 Maysville, Missouri 64469 Truss:T7

Job: 500 NW Chipman Rd REVISED

WGT/PLY

63 lbs

Date: 02/16/22 10:57:09

SPACING

24 in

Page: 2 of 2

SPA 10-0		TCH 25/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANTL 0-0-0	CANTR 0-0-0	PLYS 1
3	D1 +S1					1.000		
4	D1 + S2					1.000		
5	$0.60\mathrm{D1} + 0.60\mathrm{O}$	W1 [Uplift]			1.000		
6	0.60 D1 + 0.60	W2 [Uplift]			1.000		
7	$0.60\mathrm{D1} + 0.60\mathrm{O}$	W4 [Uplift]			1.000		
8	0.60 D1 + 0.60	W8 [Uplift]			1.000		
9	D1 +L10*1					1.000		
10	D1 +Lr1 +L1	0*1				1.000		
11	D1 + S1 + L10)*1				1.000		
12	D1 + S2 + L10)*1				1.000		
13	D1 + UR1					1.000		
14	D1 + UR2					1.000		
15	D1 +AS10*1					1.000		
16	D1 +Lr1 +AS	10*1				1.000		
17	D1 +S1 +AS1	0*1				1.000		
18	D1 +S2 +AS1	0*1				1.000		

Member Forces Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.455	1,051 lbs	(491 lbs) 2-3	0.441	-826 lbs		
BC	5-6	0.354	823 lbs	(-641 lbs)				
Web	2-6	0.433	-918 lbs	3-5	0.369	908 lbs	(-765 lbs)	
	2-5	0.070	399 lbs	(-214 lbs) 3-4	0.149	-537 lbs		

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 30% (Cq = 0.70).
- 3) Hanger is for graphical interpretation only. Install hanger per manufacturer's recommendation.
- 4) Provide adequate drainage to prevent ponding.
- 5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 6) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6" of each web panel point.
- 7) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 8) Listed wind uplift reactions based on MWFRS & C&C loading.
- 9) Parapet TL: 0.66 in, 2L/184 (7-1), Allowable 2L/120.