

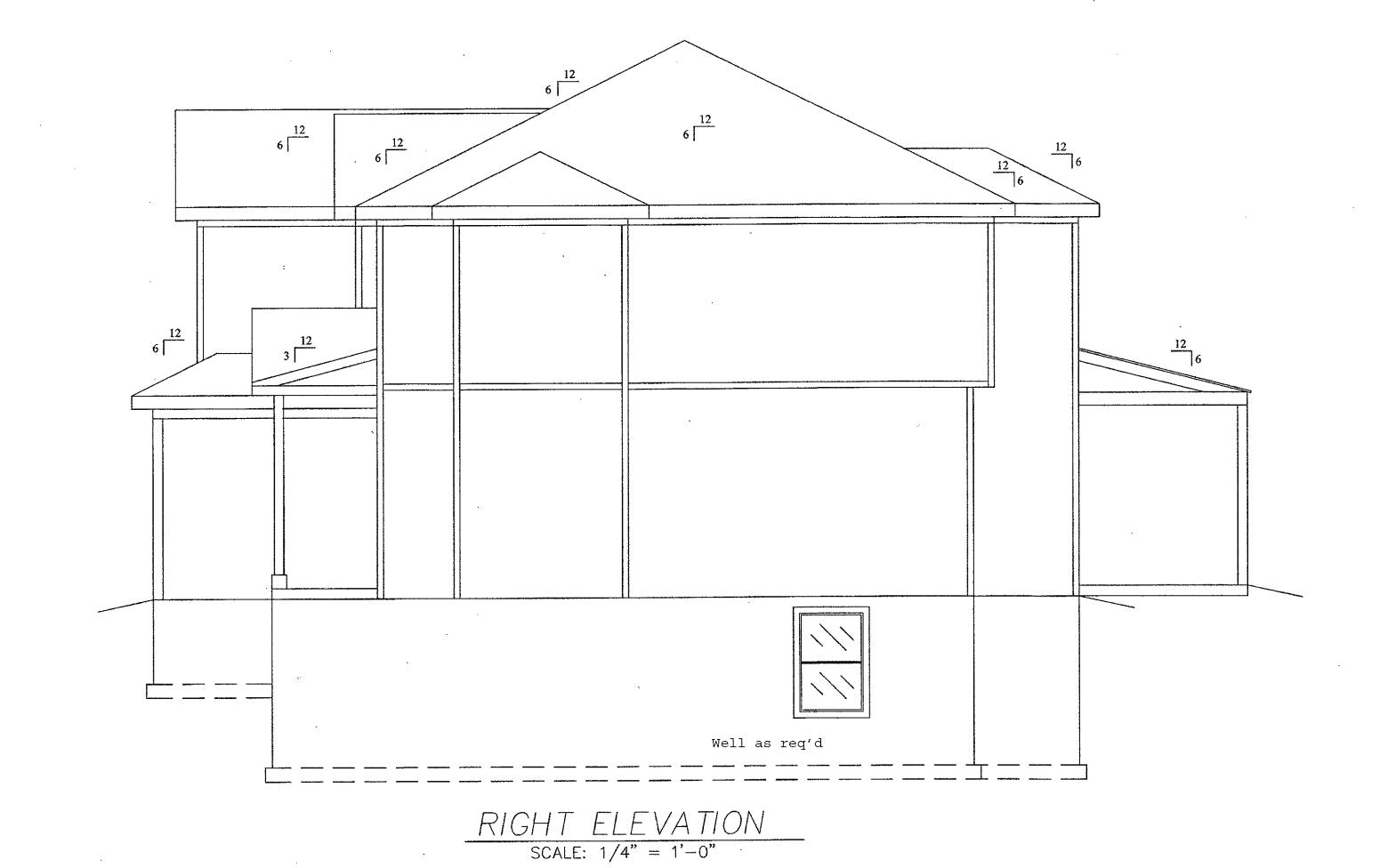
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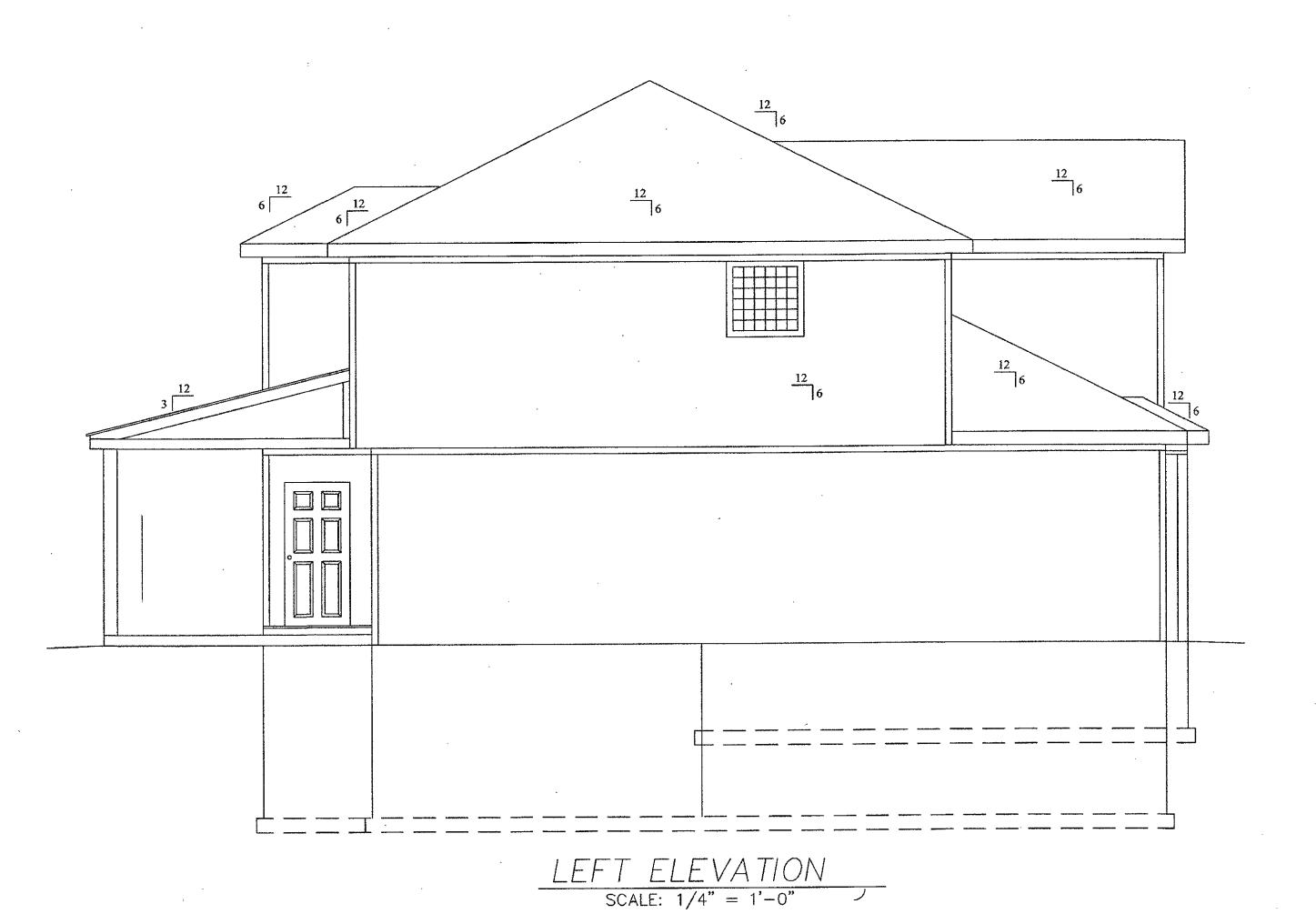
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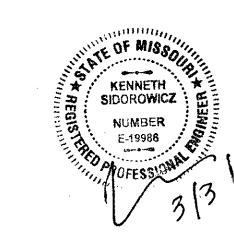
DATE:

ARCHITECT IS NOT RESPONSIBLE FOR THE STRUCTURAL ELEMENTS OF THESE PLANS. A STRUCTURAL ENGINEER MAY NEED TO VERIFY ALL STRUCTURAL ASPECTS OF THESE PRINTS BEFORE CONSTRUCTION BEGINS. FIELD CONDITIONS MAY BE DIFFERENT FROM PLAN. ALL STATE AND LOCAL CODES TAKE PRECIDENCE OVER THESE PLANS, CONTRACTOR WILL BE RESPONSIBLE FOR PLAN INTEGRITY AND CODE COMPLIANCE

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DESCRIPTION: Left/right elevations

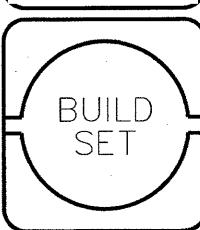
MODEL:

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 $\frac{DATE:}{2/20/2}$

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2310 Serena Pl Summit View Farms Lot 77 Lee's Summit, MO

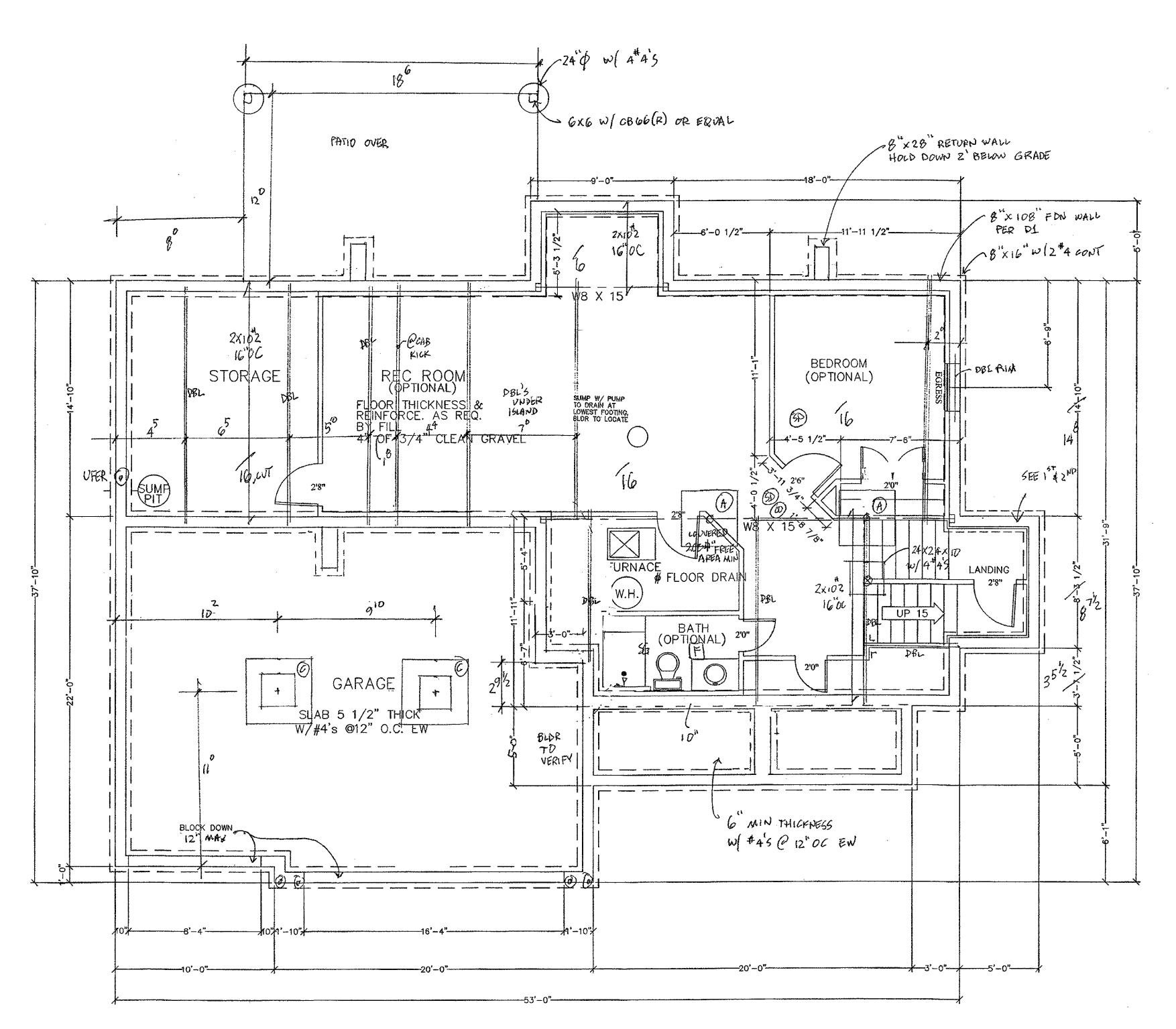


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 $2_{of}6$

SHEET NO

LENGTH 24'11' 30'0" W18X45

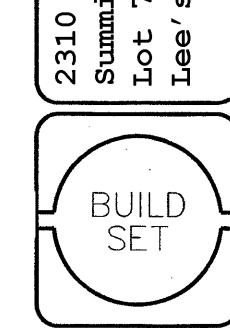


FOUNDATION PLAN
SCALE: 1/4" = 1'-0"

(A) 36x36x12 PAD W/ (6) #4'8 E.W. 3" SCH 40 COL. UNO ALL PADS

B 42x42x14 PAD W/ (7) #4's E.W.

© 48x48x16 PAD W/ (8) #4's E.W.



SCRIPTION:

FOUNDATION

MODEL:

BRANTLY

DATE:

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THESE PLANS. CONTRACTOR WILL BE RESPONSIBLE FOR PLAN INTEGRITY AND CODE COMPLIANCE

Р1 Бр

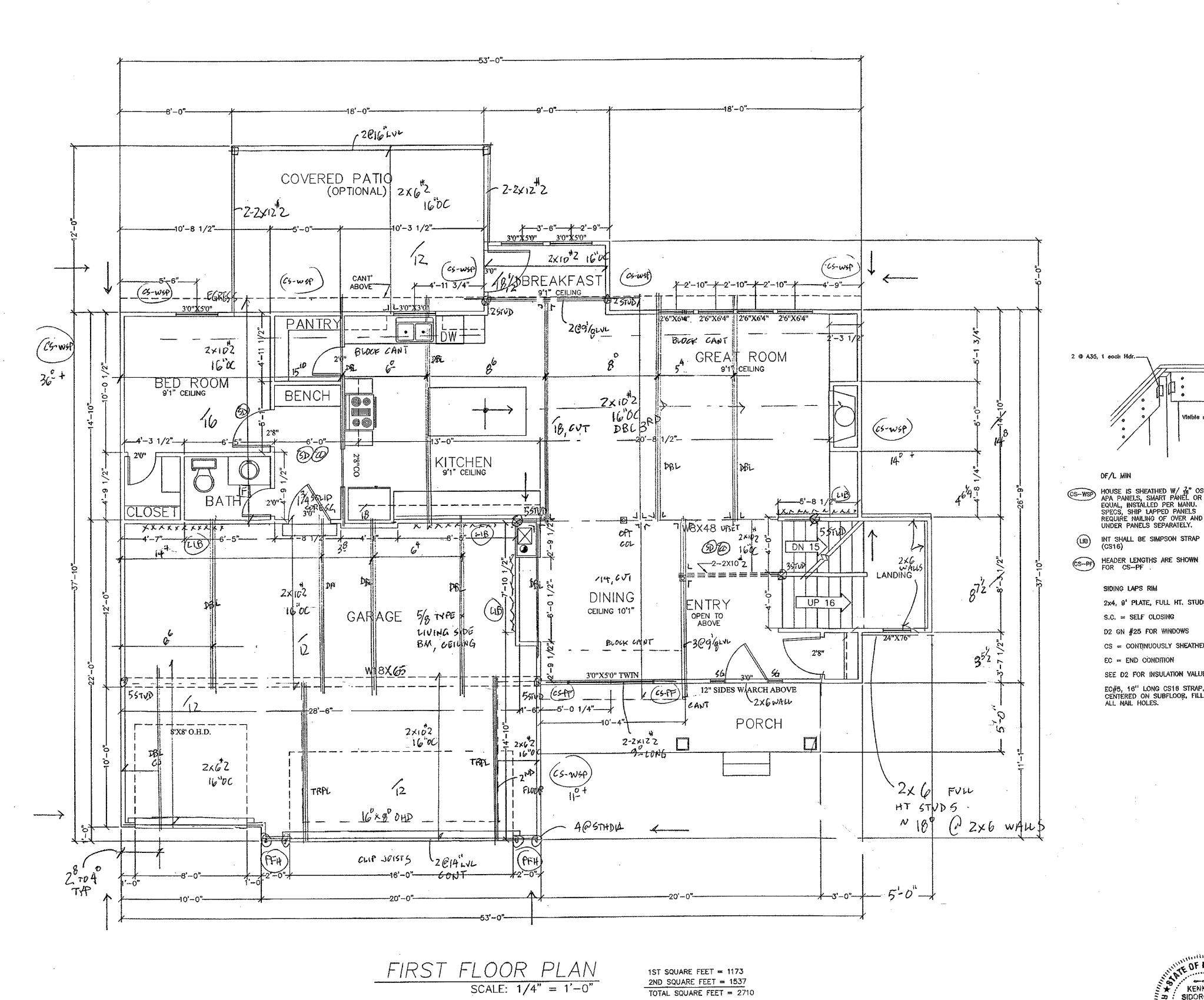
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KENNETH SIDOROWICZ

BEAMS FIELD VERIFY LENGTH SIZE W8 X 15

2 POSTS ADJUSTIBLE



FRAMING FLOORTION: DE

MODEL: BRANTLY

DATE:

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EQUAL, INSTALLED PER MANU. SPECS, SHIP LAPPED PANELS REQUIRE NAILING OF OVER AND UNDER PANELS SEPARATELY.

2x4, 9' PLATE, FULL HT. STUDS

CS = CONTINUOUSLY SHEATHED

SEE D2 FOR INSULATION VALUES

EC#5, 16" LONG CS16 STRAP, CENTERED ON SUBFLOOR, FILL ALL NAIL HOLES.

KENNETH SIDOROWICZ NUMBER E-19986

SIDING LAPS RIM

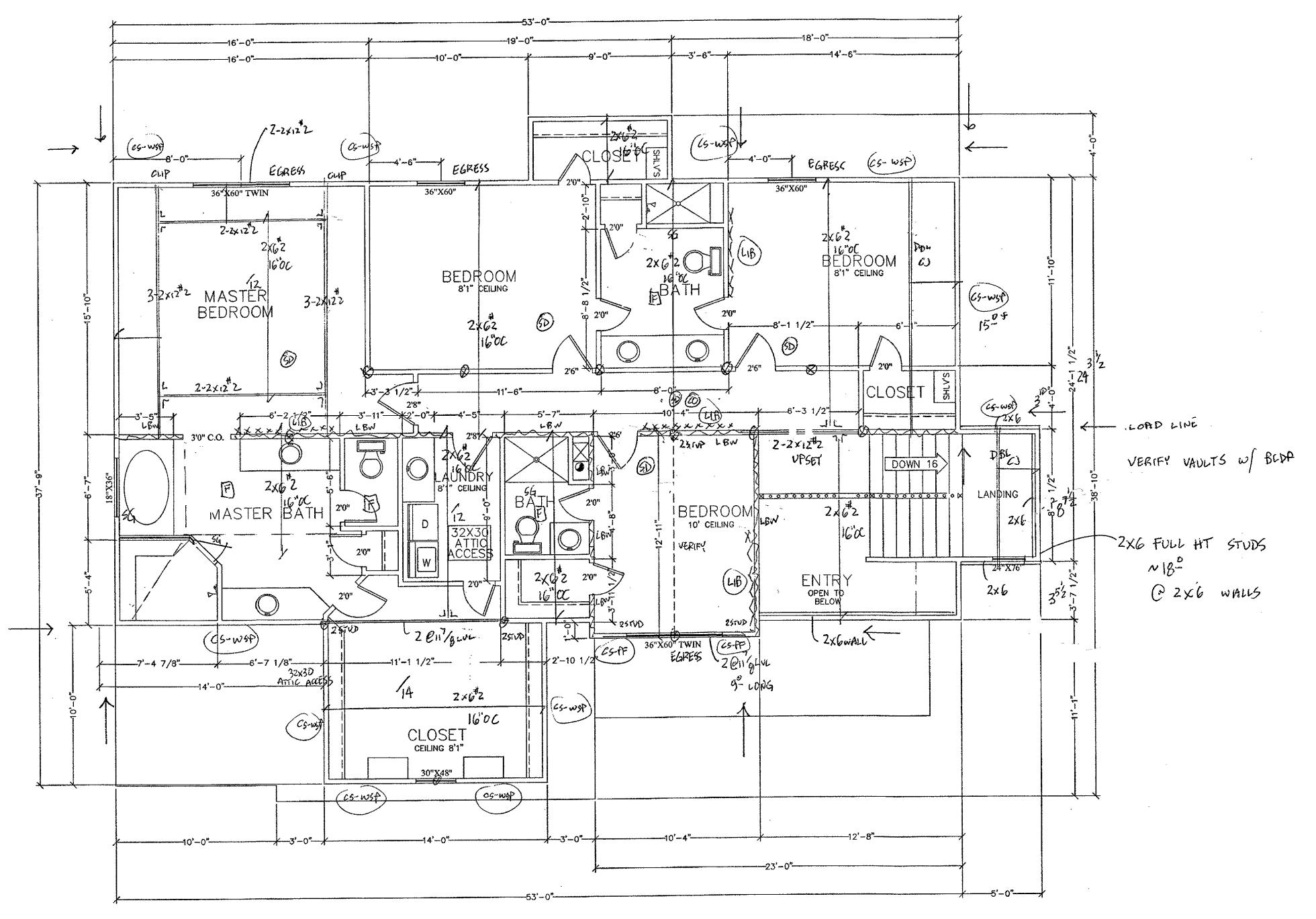
S.C. = SELF CLOSING D2 GN #25 FOR WINDOWS

EC = END CONDITION

ΡΉ Se. 4 L &

Summi Lot Lee's BUILD

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KENNETH SIDOROWICZ NUMBER E.19986

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FLOOR FRAMING FRAMING PLAN

SECOND ROOF

MODEL:

DATE: 2/20/21

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Pl Farms

Serena it View

2310 Summi Lot Lee's

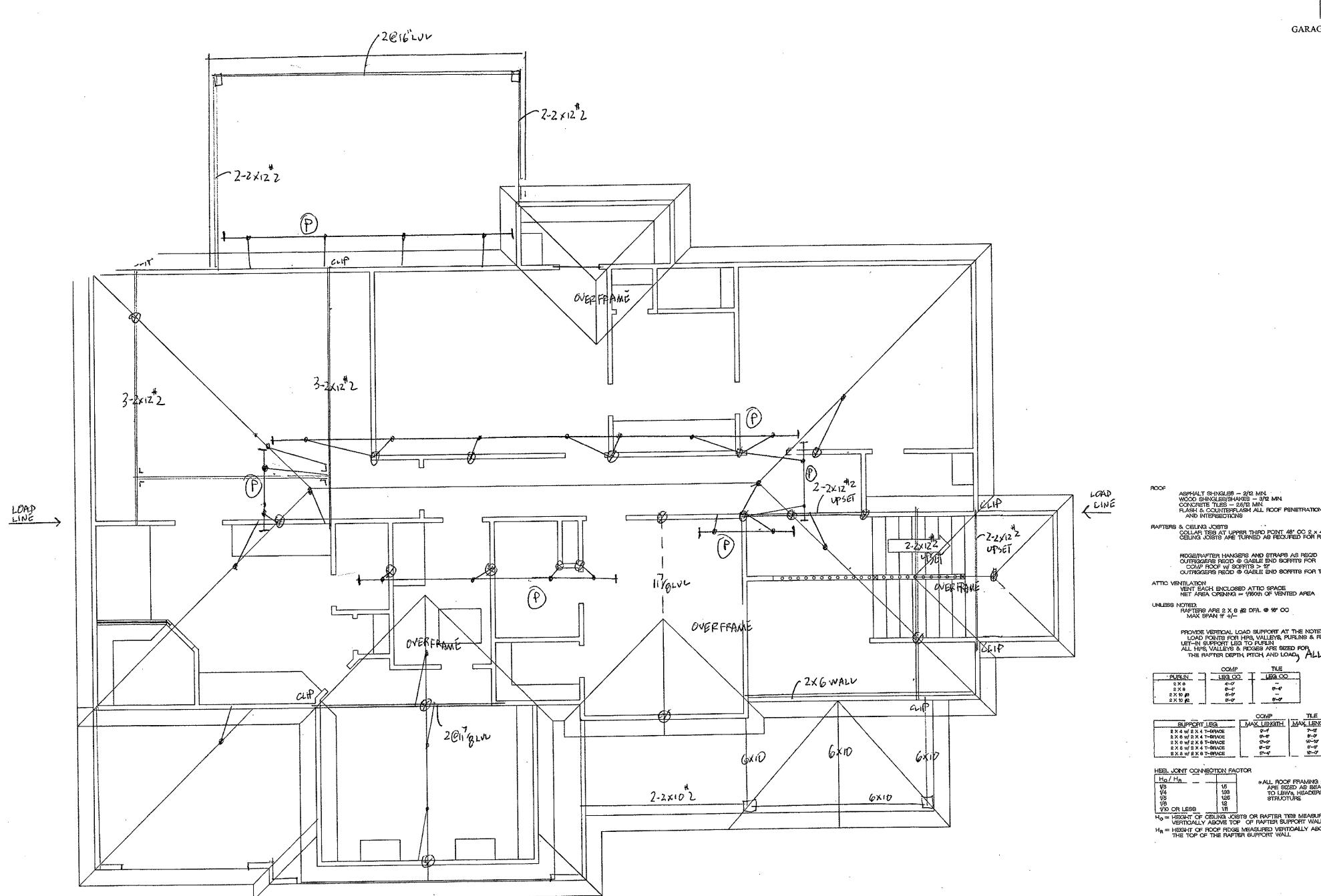
BRANTLY

DESCRIPTION:

2ND SQUARE FEET = 1537

SECOND FLOOR PLAN

SCALE: 1/4" = 1'-0"



ROOF FRAMING PLAN
SCALE: 1/4" = 1'-0"

GARAGE SOFFIT

BRANTLY

FRAMING

FLOOR

FIRST

MODEL:

DESCRIPTION:

ARCHITECT IS NOT RESPONSIBLE FOR THE STRUCTURAL ELEMENTS OF THESE PLANS. A STRUCTURAL ENGINEER MAY NEED TO VERIFY ALL STRUCTURAL ASPECTS OF THESE PRINTS BEFORE CONSTRUCTION BEGINS. FIELD CONDITIONS MAY BE DIFFERENT CONDITIONS MAY BE DIFFERENT FROM PLAN. ALL STATE AND LOCAL CODES TAKE PRECIDENCE OVER THESE PLANS. CONTRACTOR WILL BE RESPONSIBLE FOR PLAN INTEGRITY AND CODE COMPLIANCE

Р1 Fa

Serena t View

UNLESS NOTED; RAFTERS ARE 2 X 6 #2 DF/L @ 16" OC MAX SPAN 11" +/-PROVIDE VERTICAL LOAD SUPPORT AT THE NOTED
LOAD POINTS FOR HPS, VALLEYS, PURLINS & RIDGES
LET-IN SUPPORT LEG TO FURLIN
ALL HPS, VALLEYS & RIDGES ARE SIZED FOR
THE RAFTER DEPTH PITCH AND LOAD, ALL 2X8 UND

RIDGE/RAFTER HANGERS AND STRAPS AS REGID CUTTINGGERS RECID & GABLE END SOFFITS FOR COMP ROOF W/ SOFFITS > 12' CUTTINGGERS RECID & GABLE END SOFFITS FOR TILE ROOF

COMP TLE

SUPPORT LEG MAX LENGTH

2 X 4 W 2 X 4 T-EQUAR

2 X 6 W 2 X 4 T-EQUAR

2 X 6 W 2 X 6 T-EQUAR

2 X 6 W 2 X 4 T-EQUAR

3 T-EQUAR

4 T-EQUAR

6 T-EQUAR

6 T-EQUAR

6 T-EQUAR

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7 T-EQUAR

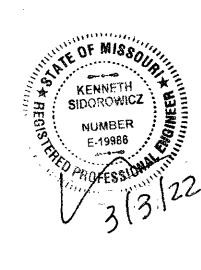
HEEL JOINT CONNECTION FACTOR

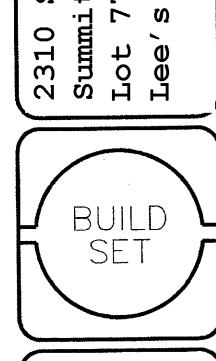
WO OR LESS 111

HO = HEICHT OF CEILING JOSTS OR RAFTER THE MEASURED VERTICALLY ABOVE TOP OF RAFTER SUPPORT WALL

HR = HEICHT OF ROOF ROSE MEASURED VERTICALLY ABOVE
THE TOP OF THE RAFTER SUPPORT WALL

RAFTER TIES SHALL BE PROVIDED PER 802.3.1 WHEN THE CJ'S ARE NOT CONNECTED TO THE RAFTERS AT THE TOP PLATE





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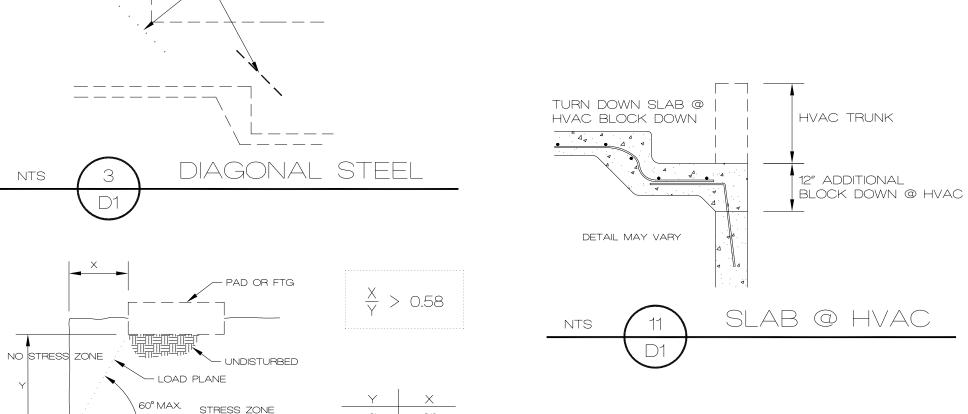
- 2. FURNISH ALL LABOR, MATERIALS, AND EQUIPMENT NECESSARY TO COMPLETE THE WORK AS SHOWN OR INFERRED BY THE DRAWINGS.
- 3. DESIGN FACTORS: A) GROUND SNOW LOAD (INCLUDING DRIFTING SNOW)__20 PSF B) WIND SPEED (EXPOSURE B)_ C) SEISMIC CATEGORY (A), GROUND ACCELERATION = NA
- 4. DESIGN LOADS (PSF, UNLESS NOTED OTHERWISE): A) ROOF (LL/DL) SEE TABLE B) FLOOR (LL/DL)_ _SEE TABLE _SEE TABLE, (0/10 TRUSSES) C) CEILING (LL/DL)_
- 5. DO NOT SCALE DRAWINGS, IF DIMENSIONS ARE IN QUESTION, OBTAIN CLARIFICATION FROM A / E BEFORE CONTINUING CONSTRUCTION.
- 6. THE CONTRACTOR SHALL EXAMINE ACTUAL JOB CONDITIONS AND BE RESPONSIBLE FOR VERIFYING ALL DIMENSIONS AND ELEVATIONS SHOWN ON THE PLANS. IF ERRORS, OMISSIONS, OR DISCREPANCIES ARE FOUND THEY SHALL BE REPORTED TO THE DESIGN PROFESSIONAL BEFORE PROCEEDING WITH THE WORK.
- 7. DIMENSIONS FOR NEW CONSTRUCTION ARE TO FACE OF FINISH OR COLUMNS AND FACE OF CONCRETE, WOOD, OR MASONRY WALLS UNLESS OTHERWISE INDICATED. DIMENSIONS INDICATE NOMINAL DIMENSIONS RATHER THAN ACTUAL DIMENSIONS.
- 8. CONTRACTOR IS RESPONSIBLE FOR COORDINATING ALL TRADES EVEN IF THE TRADE IS UNDER A SEPARATE CONTRACT.
- 9. PROVIDE SUFFICIENT STUDS AND BLOCKING WHERE REQUIRED TO SUPPORT EQUIPMENT AND/OR MISCELLANEOUS ITEMS, I.E., LOAD POINTS, TYPICAL CASEWORK, CABINETS, GRAB BARS ETC.
- 10. PRETREAT FOUNDATION FOR TERMITES AS REQUIRED.
- 11. GARAGE DOORS AND FRAMES SHALL BE DESIGNED AND INSTALLED TO MEET THE 115 MPH WIND LOAD RESISTANCE REQUIREMENTS OF DASMA 108 AND ASTM E 330.
- 12. ALL EXTERIOR DOORS, INCLUDING THE DOOR LEADING FROM THE GARAGE TO THE DWELLING UNIT, SHALL INCORPORATE THE PHYSICAL SECURITY PROVISIONS OF THE JURISDICTION IN WHICH THE CONSTRUCTION TAKES PLACE.

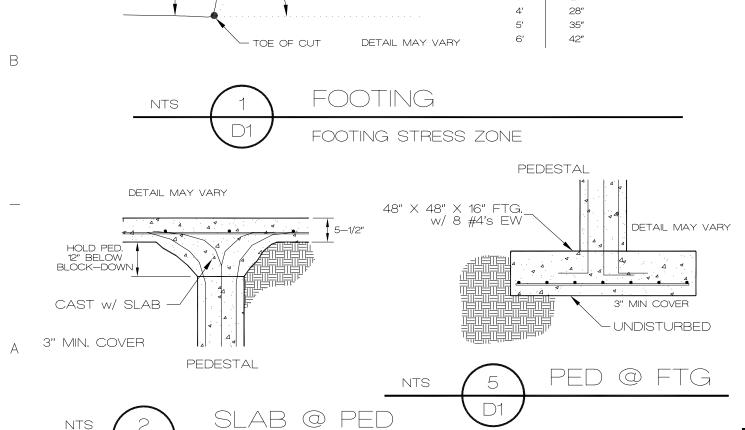
<u>DIVISION 2</u> — EARTHWORK

- 1. ALL PROPERTY MARKERS SHALL BE EXPOSED.
- 2. ALL FOOTINGS ARE DESIGNED TO BEAR ON NATURAL UNDISTURBED SOIL CAPABLE OF ADEQUATELY SUSTAINING A MINIMUM BEARING PRESSURE OF 1,500 PSF. IF SUITABLE UNDISTURBED BEARING CAPACITY IS NOT ENCOUNTERED AT THE ELEVATION INDICATED ON THE DRAWINGS, CONTRACTOR SHALL NOTIFY THE ENGINEER OF RECORD IMMEDIATELY.
- 3. ALL TOPSOIL, ORGANIC MATERIAL, AND EXISTING STRUCTURES SHALL BE REMOVED FROM BUILDING AREA AND FROM AREAS TO BE PAVED. STOCKPILE ALL TOPSOIL FOR REUSE.
- 4. REFERENCE THE SOILS REPORT FOR ALL FILL CONDITIONS.
- 5. OVEREXCAVATE BUILDING AREA BELOW SLAB SUBGRADE ELEVATION AND REPLACE WITH MATERIAL PER SOILS REPORT,
- 6. SITE EROSION CONTROL SHALL COMPLY WITH ALL STATE AND LOCAL ORDINANCES.
- 7. IN-SITU SOIL CONDITIONS, SEE SOILS REPORT OR 1,500 PSF BEARING & 60 PCF EQUIVALENT FLUID WEIGHT.

- 48" LONG MIN.

- 8. SOIL CONDITIONS AT THE DEPTH OF EXCAVATION FOR THE FOOTING SHALL BE UNIFORM AND CONSISTENT. NOTIFY THE ENGINEER OF RECORD OF ANY INCONSISTENCIES.
- 9. CONTRACTOR SHALL BE RESPONSIBLE FOR REMOVING AND DISPOSING OF ANY EXCESS EXCAVATION MATERIALS AND FOR OBTAINING AND SUPPLYING ADDITIONAL FILL MATERIAL AS





SLAB ON FILL

<u>DIVISION 3</u> — CONCRETE

1. ALL CONCRETE WORK SHALL CONFORM TO THE REQUIREMENTS OF ACI 318 "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE" AND ACI 332 "REQUIREMENTS FOR RESIDENTIAL CONCRETE CONSTRUCTION."

MASONRY STRENGTH (F'M DESIGN)_ CONCRETE MATERIALS SHALL COMPLY WITH: BLOCK STRENGTH A) CEMENT - ASTM C 150 TYPE MORTAR STRENGTH B) AGGREGATE — ASTM C 33, MAXIMUM AGGREGATE SIZE 3/4" GROUT STRENGTH__

C) WATER - POTABLE, WATER/CEMENT RATIO .5 (MAX.) D) AIR-ENTRAINING ADMIXTURE - ASTM C 260 E) WATER-REDUCING ADMIXTURE - ASTM C 494, INCLUDING SUPERPLASTICIZERS.

F) FLY ASH — ASTM C 618, CLASS C

3. CONCRETE SHALL DEVELOP THE FOLLOWING MINIMUM 28 DAY DESIGN COMPRESSIVE STRENGTH (f'c): TYPE OF CONSTRUCTION COMP. STRENGTH (f'c)

A) FOOTINGS, WALLS, AND SLABS SEE TABLE B) EXTERIOR SLABS AND CURBS SEE TABLE (AIR-ENTRAINED CONCRETE)

CONCRETE PROPORTIONS SHALL BE ESTABLISHED ON THE BASIS OF FIELD EXPERIENCE AND/OR TRIAL MIXTURES IN ACCORDANCE WITH ACI 318-89 SECTIONS 5.2 AND 5.3. WHEN FLY ASH IS UTILIZED IN THE MIX, MIX SHALL CONTAIN A WATER-REDUCER. FLY ASH SHALL BE ADDED AT THE RATE OF NOT MORE THAN 100 POUNDS PER CUBIC YARD AND CEMENT SHALL BE REDUCED BY NOT MORE THAN 15 PERCENT BY WEIGHT.

- 4. PROPORTION AND DESIGN MIXES TO RESULT IN CONCRETE SLUMP AT A POINT OF PLACEMENT OF NOT MORE THAN 4" TO 5".
- 5. USE AIR-ENTRAINING ADMIXTURES IN EXTERIOR EXPOSED CONCRETE TO RESULT IN CONCRETE AT POINT OF PLACEMENT HAVING AIR CONTENT OF 5 TO 7 PERCENT ENTRAINED AIR.
- 6. ALL PLUMBING AND ELECTRICAL ROUGH—INS MUST BE COMPLETE, INSPECTED AND APPROVED BEFORE REQUESTING THE SLAB
- 7. CONCRETE WORK EXECUTION: A) MINIMUM CONCRETE COVER FOR REINFORCING SHALL BE. UNLESS
- NOTED OTHERWISE ON DRAWINGS: CAST AGAINST AND EXPOSED TO EARTH_ EXPOSED TO EARTH OR WEATHER_ NOT EXPOSED TO EARTH OR WEATHER
- B) IN CORNERS OF GRADE BEAMS PROVIDE CORNER REINFORCEMENT LAP TWO FEET EACH DIRECTION IN OUTSIDE FACE, MATCHING SIZE AND SPACING OF HORIZONTAL REINFORCEMENT.
- C) PROVIDE CONTROL JOINTS IN SLABS-ON-GRADE AT NOT GREATER THAN 20 FEET ON CENTER IN EACH DIRECTION. SAW CUT CONTROL JOINTS MINIMUM 1/4 OF THE SLAB DEPTH, AS SOON AFTER SLAB FINISHING AS POSSIBLE WITHOUT DISLODGING AGGREGATE. (DO NOT SAW CUT STRUCTURAL SLABS w/o APPROVAL).
- 8. BATCH TICKETS SHALL BE SUBMITTED TO A CONTRACTORS REPRESENTATIVE PRIOR TO OFF LOADING. ANY CONCRETE MORE THAN 45 MINUTES OUT PRIOR TO STARTING PLACEMENT SHALL BE
- 9. THE MAXIMUM ADDITION OF WATER SHALL BE LIMITED TO 1 GALLON PER YARD; NOTE THAT THIS ADDITION SHALL BE USED TO CONTROL HEAT ONLY (NOT SLUMP).
- 10. PUMPS SHALL NOT BE PRIMED IN FORMS.
- 11. REINFORCEMENT
- A) ALL REINFORCING BARS SHALL BE A615, GR40 MIN. LAP SPLICES 18" MIN FOR #4 BAR. SEE TABLE B) WELDED WIRE FABRIC SHALL BE ASTM A185, LAP AT LEAST ONE
- FULL MESH AND LACE SPLICES WITH WIRE. C) REBAR SHALL BE CLEAN, AND FREE FROM RUST AND OIL PRIOR TO THE PLACEMENT OF CONCRETE. REBAR SHALL BE TIED AND SECURED AS REQUIRED TO PREVENT DISPLACEMENT IN THE FORMS. D) TIE STEEL TO PREVENT DISPLACEMENT. HOOK AND TIE STEEL AS
- POSSIBLE. TIES, CHAIRS, OR OTHER PRODUCTS SHALL BE PROTECTED WHEN LOCATED NEAR EXPOSED SURFACES E) STEEL SHALL BE STORED ON SITE ABOVE GRADE, AND COVERED AS REQUIRED FOR PROTECTION FROM RAIN AND OTHER POSSIBLE
- 12. ADJUST FOUNDATION FOR SITE AND SOIL CONDITIONS AND VERIFY WITH EOF

DETAIL MAY VARY

JOIST HNGR -

FLOOR

JOIST

6" MIN. STEM WALL—

1-1/2" LEDGE

DIVISION 4 - MASONRY

1. COMPRESSIVE STRENGTH OF CONCRETE MASONRY CONSTRUCTION (CMU) SHALL BE AS FOLLOWS (PSI). MASONRY STRENGTH NOT SPECIFICALLY NOTED ON PLAN SHALL BE (f'm) 1500 PSI.

1500 1900 1800

- 2. CONCRETE BLOCK SHALL BE HOLLOW LOAD-BEARING CONCRETE MASONRY UNITS CONFORMING TO ASTM C 90, TYPE N-II. ALL BLOCKS SHALL BE PLACED IN RUNNING BOND CONSTRUCTION (UNLESS OTHERWISE NOTED) WITH ALL VERTICAL CELLS IN ALIGNMENT.
- 3. MORTAR MIX SHALL CONFORM TO THE REQUIREMENTS OF ASTM C 270, TYPE M OR S. TYPE M MORTAR SHALL BE USED WHERE MASONRY IS IN CONTACT WITH SOIL.
- 4. GROUT SHALL CONFORM TO THE REQUIREMENTS OF ASTM C 476. USE SUFFICIENT WATER FOR GROUT TO FLOW INTO ALL JOINTS OF THE MASONRY WITHOUT SEGREGATION. ALL CELLS IN CONCRETE BLOCKS CONTAINING REINFORCING SHALL BE FILLED SOLID WITH GROUT. ALL MASONRY BELOW FINISHED FLOOR OR GRADE SHALL BE GROUTED SOLID. HOLD GROUT DOWN 1—3" BELOW TOP OF BLOCK AT GROUT LIFT JOINTS AND AT CONCRETE PLACED OVER
- . MINIMUM LINTEL, WHERE NOT ON PLANS, SHALL HAVE A MINIMUM OF 2 — #5's CONTINOUS HORIZONTAL BARS IN BOTTOM OF BOND BEAM OR LINTEL BLOCK AND SHALL BE GROUTED SOLID TO A MIN. DEPTH OF 24." ALL LINTEL REINFORCING AND GROUT SHALL EXTEND 2' MINIMUM PAST JAMBS UNLESS NOTED OTHERWISE ON PLANS OR DETAILS.
- 6. LAP REINFORCING 48 BAR DIAMETERS. STAGGER LAP SPLICES A MINIMUM OF ONE LAP LENGTH.
- 7. MASONRY VENEER SHALL BE ATTACHED TO SUPPORT WALL FRAMING WITH 36" DIAMETER WALL TIES OR DOVETAIL-TYPE METAL TIES OF EQUIVALENT STIFFNESS EMBEDDED INTO HORIZONTAL MORTAR JOINTS. MAXIMUM VERTICAL SPACING OF TIES SHALL BE 16," MAXIMUM HORIZONTAL SPACING SHALL BE 24." TIES IN ALTERNATE COURSES SHALL BE STAGGERED. PROVIDE #9 WIRE REINFORCING IN HORIZONTAL MORTAR JOINTS AT 16" OC. ENGAGE #9 WIRE WITH WALL ANCHOR TIES. CONSTRUCTION JOINTS IN MASONRY VENEER WALLS SHALL BE LOCATED PER THE DRAWINGS.
- 8. WATERPROOFING, DRAINAGE PLANE, AND INSTALLATION PER ADOPTED BUILDING CODE.

<u>DIVISION 5.5</u> — MISC. STRUCTURAL STEEL

- 1. ALL MISCELLANEOUS STRUCTURAL STEEL WORK SHALL CONFORM TO THE REQUIREMENTS OF AISC "SPECIFICATIONS FOR DESIGN, FABRICATION, AND ERECTION OF STRUCTURAL STEEL FOR
- MISCELLANEOUS STRUCTURAL STEEL MATERIAL SHALL COMPLY A) STRUCTURAL STEEL — ASTM A992
 - B) STEEL PIPE COLUMNS ASTM A53 GRADE B(Sch 40 TYP) C) ANCHOR BOLTS - ASTM A307 GRADE A, NON-HEADED TYPE UNLESS OTHERWISE NOTED.
- 3. FLITCH PLATES SHALL HAVE 1" DIA. BOLTS @ 16" OC, STAGGERED TOP AND BOTTOM BETWEEN JOIST LAYOUT.

RETURN WALLS WALL HT. RETURN SPACING ABOVE FLOOR (HOLD DOWN 24" BELOW GRADE) LESS THAN 4' RETURN WALLS NOT REQ'D 16'-4" ON CENTER (MAX.), AND WITHIN >4' TO 9' 8' OF STEP DOWN OR AS SHOWN

* RETURN WALLS ALLOW FOR BACKFILL W/O FLOOR DECK IN PLACE FOR 60 PCF EQUIVALENT FLUID WEIGHT SOIL. NO HEAVY EQUIPMENT OR SURCHARE LOADING.

OPT. MUDSILL ANCHORAGE

DETAIL MAY VARY

SLAB @ WALL

CONCRETE OR CMU

SLAB ON FILL

REQ'D STRENGTH

3,000 psi

3,500 psi

3,500 psi

7 SACK MIX

CONC STRENGTH

- 2 X LEDGE

- DRILL & SEAL AS REQ'D

— DOWELS @ 12" OC

ALTERNATIVE TO J-BOLTS

MIN. \bot

1-1/2" COVER -

NTS

DBL PLATE FOR GYP CRETE

CONCRETE SLAB

 $\frac{1}{2}$ 1–1/2" COVER MIN.

2.5" INSULATION,

– 1—1/2" LEDGE MIN.

ICF WALL

FLUSH FRAMING @ FDN

24" LAP, MIN

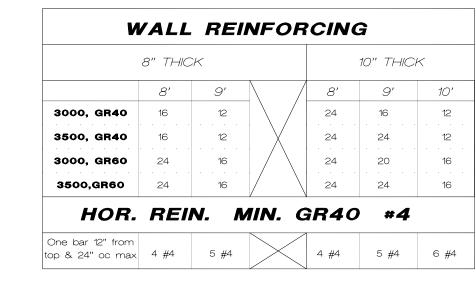
FTG

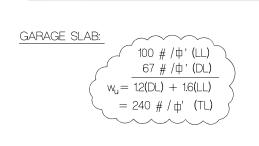
WALL

SLAB

SUS-SLAB

FOUNDATION PER JOCOBO RESIDENTIAL DETAIL MAY VARY FOUNDATION 2 X 4 OR 2 X 6 GUIDELINE LSIMPSON MAS ALTERNATIVE INSTALLATION -SIMPSON MAS





 $\frac{w_{i} * L^{2}}{27,206}$ #-in 40,000 * 0.2 $0.85 * f_c * b$

= 0.9(0.2)(40000)(4-0.22/2)= 28,008 #-in > 27,206 (OKAY).. Use #4 @ 12" OC EW

 $\phi M_N = *\phi A * f(d - a/2)$

12'-6" (+/-) MODULE

 $\phi M_N = *\phi A * f(d - a/2)$ = 0.9(0.2)(40000)(4-0.22/2)= 28,008 #-in > 25,951 (OKAY)

> ∴ Use #4 @ 12" OC EW 15'-6" (+/-) MODULE

<u>DIVISION 6</u> — ROUGH CARPENTRY

1. ALL ROUGH CARPENTRY WORK SHALL CONFORM TO THE REQUIREMENTS OF NFPA "NATIONAL DESIGN SPECIFICATION OF WOOD CONSTRUCTION", TPI "DESIGN SPECIFICATIONS FOR LIGHT METAL PLATE CONNECTED WOOD TRUSSES", APA "PLYWOOD DESIGN SPECIFICATIONS", DOC PS 1 "PRODUCT STANDARD FOR CONSTRUCTION AND INDUSTRIAL PLYWOOD", DOC PS 56 "STRUCTURAL GLUED" LAMINATED TIMBER", AND APPLICABLE SECTIONS OF THE INTERNATIONAL BUILDING CODE.

2. ROUGH CARPENTRY MATERIALS SHALL COMPLY WITH: A) LUMBER - S4S, S-DRY, KD, OR S-GRN GRADE MARKED, COMPLYING WITH PS 20, GRADED UNDER WWPA OR SPIB RULES: STUD GRADE HEADER: #2 DOUGLAS FIR MIN TYPICAL RAFTER: #2 DOUGLAS FIR PLATES: #2 DOUGLAS FIR BLOCKING: #2 DOUGLAS FIR

B) METAL FRAMING FASTENERS - ASTM A 153, HOT-DIP GALVANIZED FASTENERS; EQUAL TO SIMPSON STRONG-TIE CONNECTORS COMPLYING WITH APPLICABLE ICC-ES REPORTS C) PLYWOOD - APA RATED SHEATHING, COMPLYING TO PS 1. D) LVL - LAMINATED VENEER LUMBER SHALL BE GRADE 2800

F-2.0E AND SHALL MEET THE REQUIREMENTS OF APPLICABLE

ICC-ES REPORTS. E) GLULAM BEAMS - COMBINATION 24F-V3 IN ACCORDANCE WITH AITC A190.1

3. EXTERIOR WALL AND ROOF SHEATHING SHALL BE $\frac{7}{16}$ " APA RATED SHEATHING 24/0 EXTERIOR GLUED (MIN) FOR 16" OC STUD SPACING. NAIL SHEATHING TO SUPPORT MEMBERS WITH 8D COMMON NAILS AT 6" ON CENTER ALONG EDGE SUPPORTS AND 12" ON CENTER ALONG FIELD SUPPORTS UNLESS NOTED OTHERWIDE. PROVIDE SOLID BLOCKING AT ALL UNSUPPORTED PANEL EDGES; 4/8 GUN NAILS.

NOTE: ROOF SHEATHING SHALL BE \(\frac{8}{6} \) APA RATED SHEATHING FOR TILE ROOF, OR AS REQUIRED BY MANUFACTURER.

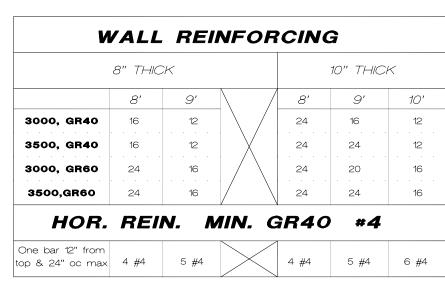
- 4. INTERIOR SHEAR WALL SHEATHING WHERE NOTED SHALL BE 7 APA RATED SHEATHING 24/0 EXTERIOR GLUED (MIN) FOR 16" OC STUD SPACING. NAIL SHEATHING TO SUPPORT MEMBERS WITH 8D COMMON NAILS AT 4" ON CENTER ALONG EDGE SUPPORTS AND 6" ON CENTER ALONG FIELD SUPPORTS UNLESS NOTED OTHERWISE. PROVIDE SOLID BLOCKING AT ALL UNSUPPORTED PANEL EDGES.
- 5. ATTACH METAL FRAMING FASTENERS TO FRAMING MEMBERS WITH MINIMUM NUMBER AND SIZE OF NAILS LISTED IN THE APPLICABLE ICC-ES REPORTS.

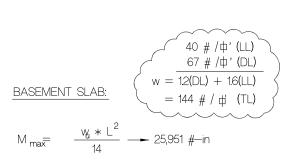
B) DESIGN LOADS

- 6. WOOD TRUSS SYSTEM; TRUSS JOIST SYSTEM AND GLULAM SYSTEM FOR ROOFS: A) DESIGN, FABRICATE, AND ERECT IN ACCORDANCE WITH BCSI STANDARDS AND NDS SPECIFICATIONS.
 - 25 PSF SNOW LIVE LOAD 10 PSF DEAD LOAD TOP CHORD (20 TILE) 10 PSF DEAD LOAD BOTTOM CHORD
 - C) SUBMIT SHOP DRAWINGS, INCLUDING DESIGN CALCULATIONS, MATERIAL STRESSES, GRADE AND SPECIES OF WOOD, AND PLACEMENT DRAWING.
- 7. DEFAULT HEADER SIZE NOT SPECIFIED SPANNING 8'-0" MAX SHALL BE $2-2 \times 10$ #2, WITH 2 STUD SUPPORT.
- 8. ALL HEADERS OVER 4'-0" SHALL HAVE DOUBLE TRIMMER @ EACH SUPPORT, OR AS SPECIFIED, UNO.

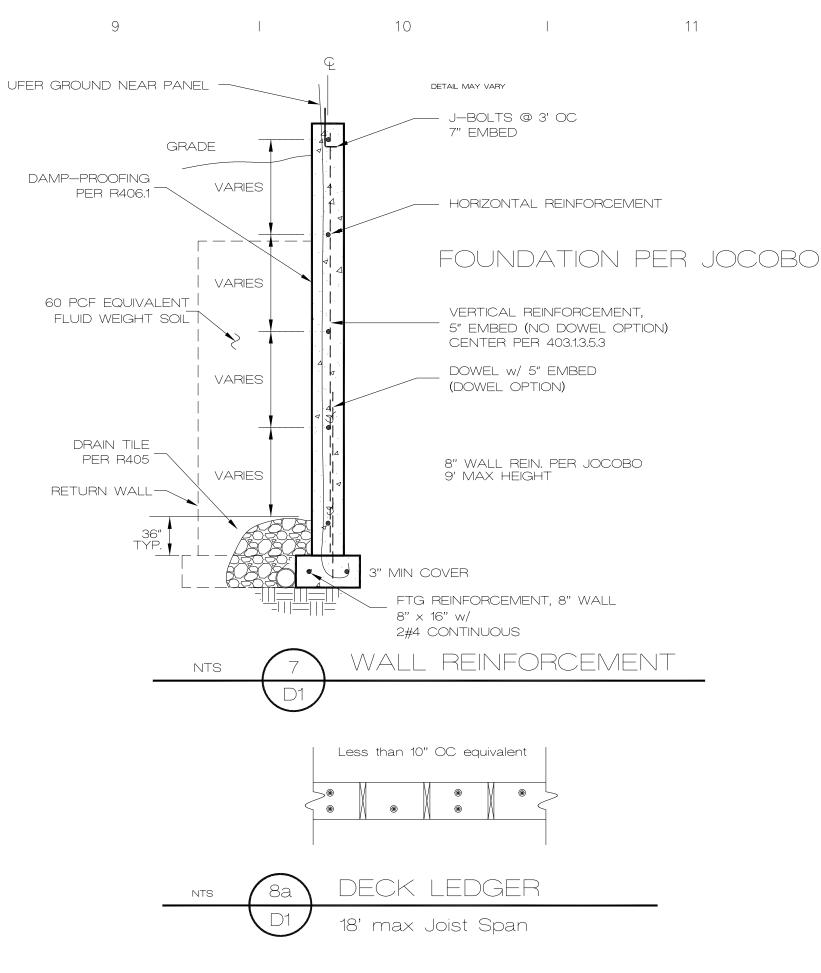
9. SOLID BLOCKING BETWEEN JOISTS @ 36" OC FOR JOISTS PARALLEL

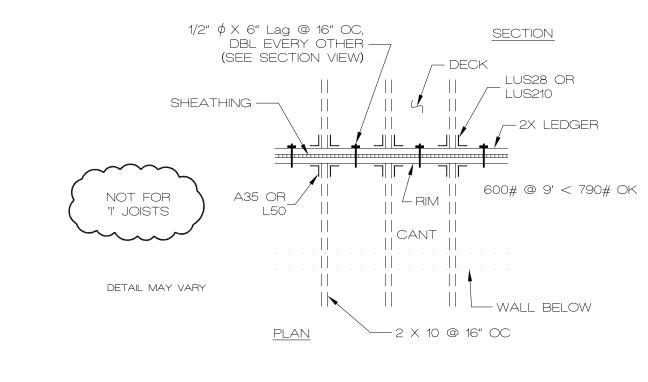
- TO THE EXTERIOR FOUNDATION WALL, MIN. 48" OR 3 JOIST SPACES. 10. ALL FLUSH FRAMING @ HEADERS OR GIRDERS SHALL BE HANGERED.
- 11. BLOCK BETWEEN JOISTS @ SUPPORTS OR OVER BEAMS.
- 12. RATED CONSTRUCTION FOR PROJECTIONS INTO SETBACKS AS REQ'D.
- 13. DOUBLE JOIST BELOW PARALLEL NONBEARING WALLS ON LAYOUT, SINGLE JOIST OFF LAYOUT. STRUCTURE BELOW LOAD-BEARING WALLS AS NOTED ON PLANS.

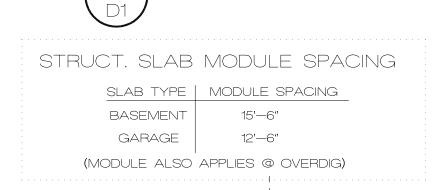




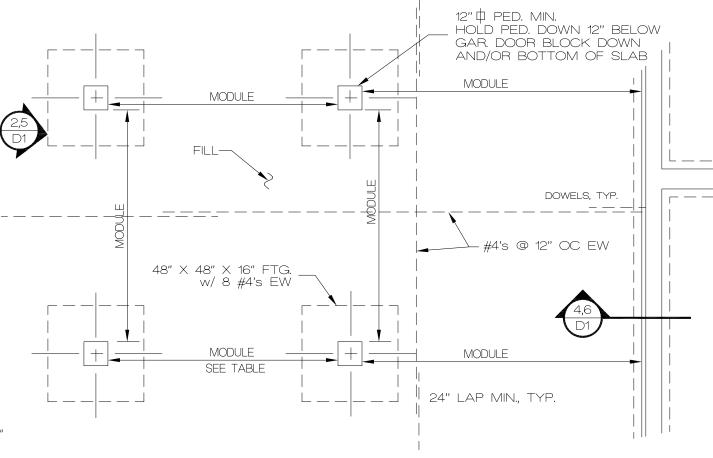
40,000 * 0.2 $\frac{0.85 * 3,500 * 12}{0.85 * 3,500 * 12} = 0.22"$





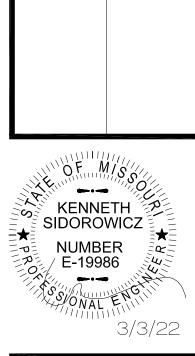


DECK @ CANTILEVER



STRUCTURAL SLAB ON FILL

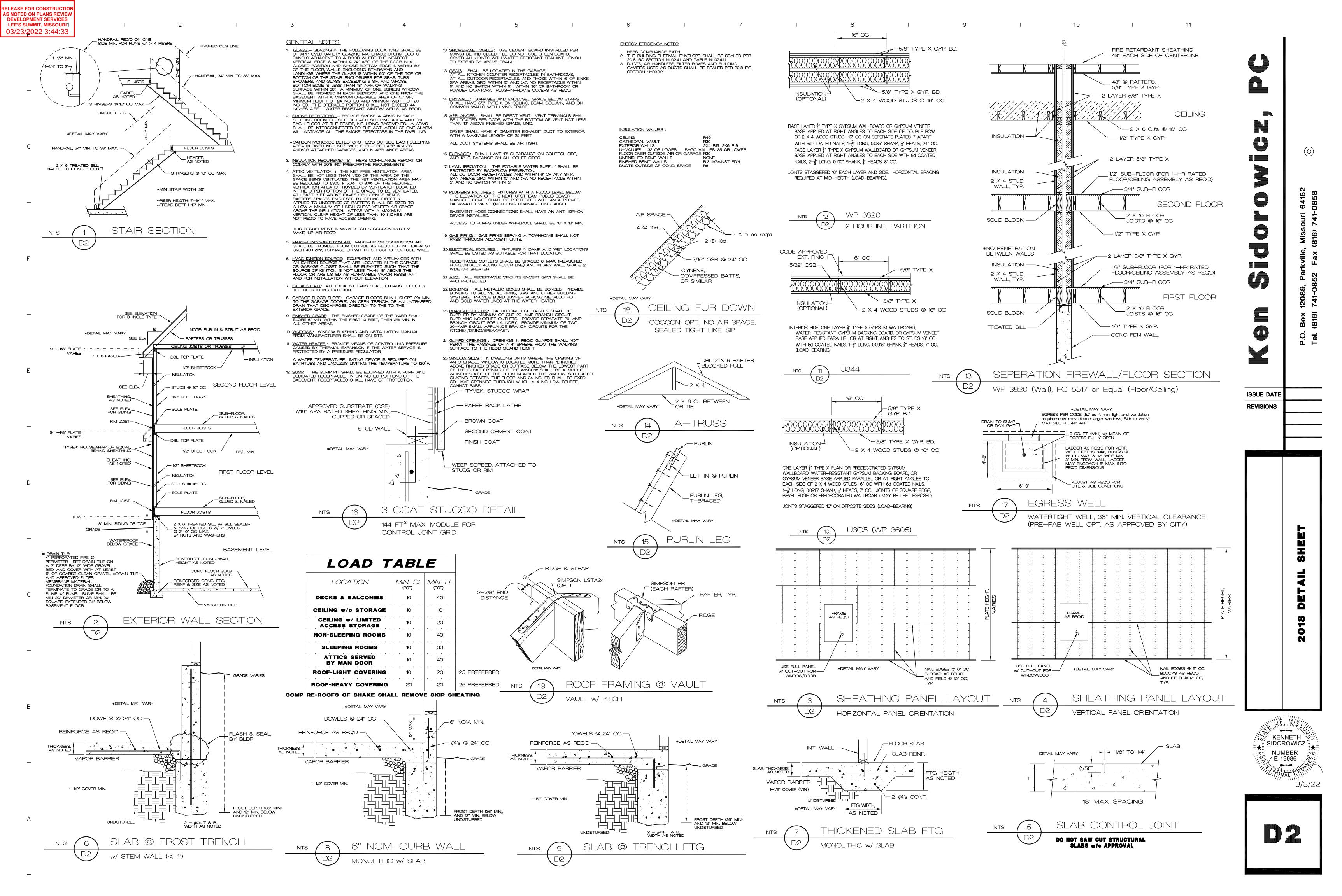
DO NOT SAW CUT STRUCTURAL SLABS W/O APPROVAL VERIFY ALL STRUCTURAL SLAB DETAILS W/ ENGINEER DO NOT ISOLATE COLUMNS FROM STRUCTURAL SLABS



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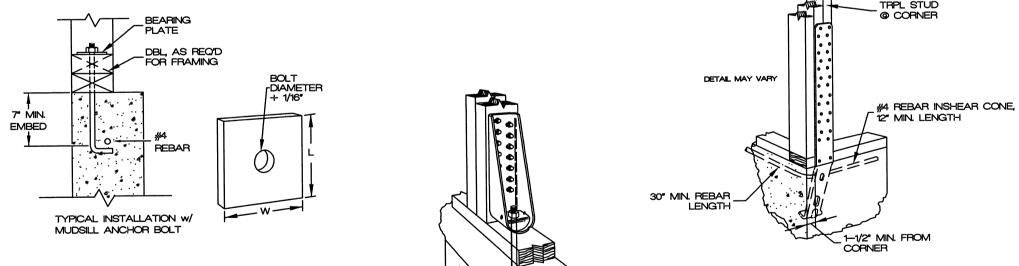


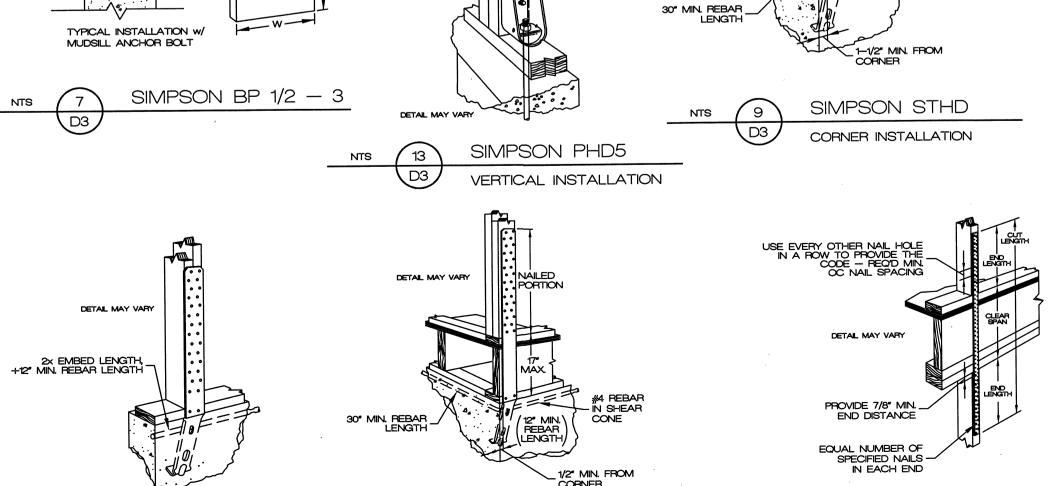
STAPLES NOT PERMITTED IN KCMO

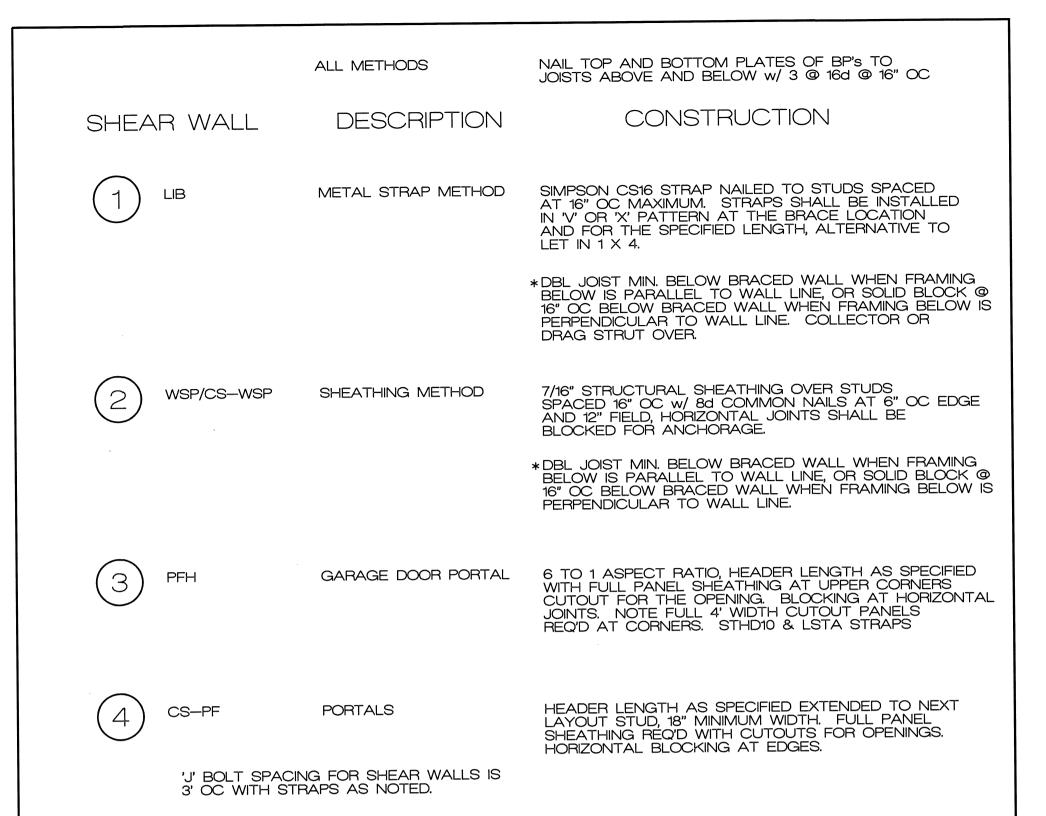
		TENER SCHEDULE FO	ON STRUCTURE	IVILIVIDLI IO
Item	Description of building ele	ements	Number & type of fastener (notes: a, b, c)	Spacing of fasteners
of				
1 2 3 4 5	Blocking between joists or rafters to top plate, toe nail Celling joists to plate, toe nail Celling joists not attached to parallel rafter, laps over partitions, face nail Collar tie rafter, face nail or 1-1/4" x 20 ga. ridge strap Rafter to plate, toe nail, note trusses use STC clips at NLB walls and spec'd holdowns		3-8d (2-1/2" × 0.113") 3-8d (2-1/2" × 0.113") 3-10d 3-10d (3" × 0.128") 3-16d or 3-10d (3-1/2" × 0.135", 0.148")	2 toe nails side 1, 1 toe nail side 2 (note j)
6	Roof rafters to ridge, valley or hip raft	ers:	4-16d (3-1/2" × 0.135")	-
	Face nail:		3-16d (3-1/2" x 0.135")	
JI			(a) (a) (a) (a)	24" 00
7 8 9 10 11 12	Built-up studs-face nail Abutting studs at intersecting wall corners, face nail Built-up header, two pieces w/ 1/2" spacer Continued header, two pieces Continuous header to stud, toe nail Double studs, face nail		10d (3" × 0.128") 16d (3-1/2" × 0.135") 16d (3-1/2" × 0.135") 16d (3-1/2" × 0.135") 4-8d (2-1/2" × 0.113") 10d (3" × 0.128")	24" o.c. 12" o.c. 16" o.c. along each edge 16" o.c. along each edge 24" o.c.
13 14 15 16	Double top plates, face nail Double top plates, min. 48" offset of end joints, face nail in lapped area Sole plate to joist or blocking, face nail Sole plate to joist or blocking at braced wall panels		10d (3" × 0.128") 8-16d (3-1/2" × 0.135") 16d (3-1/2" × 0.135") 3-16d (3-1/2" × 0.135") 3-8d (2-1/2" × 0.113") or	16° o.c.
17 	Stud to sole plate, toe nail Top or sole plate to stud, end nail Top plates, laps at corners and intersections, face nail "brace to each stud and plate, face nail		2-16d (3-1/2" x 0.135") 2-16d (3-1/2" x 0.135") 2-10d (3" x 0.128") 2-8d (2-1/2" x 0.113") 2 staples 1-3/4"	
21 _.	$1^{\circ} \times 6^{\circ}$ sheathing to each bearing, face nail $1^{\circ} \times 8^{\circ}$ sheathing to each bearing, face nail		2-8d (2-1/2" x 0.113") 2 staples 1-3/4" 2-8d (2-1/2" x 0.113")	<u>=</u>
			3 staples 1-3/4" 3-8cs(20)462"1-x3(01113")	_
23	Wider than 1" x 8" sheathing to each	bearing, face nail	3 808 (4 7) (42 17-34 410)	
24 25 26 27 28 29 30	Joist to sill or girder, toe nail Pim joist to top plate, toe nail (roof a Pim joist or blocking to sill plate, toe 1" x 6" subfloor or less to each joist, 2" subfloor to joist of girder, blind and 2" planks (plank & beam — floor and Built—up girders and beams, 2" lumb	nail face nail d face nail	3-8d (2-1/2" x 0.113") 8d (2-1/2" x 0.113") 8d (2-1/2" x 0.113") 2-8d (2-1/2" x 0.113") 2 staples 1-3/4" 2-16d (3-1/2" x 0.135") 2-16d (3-1/2" x 0.135") 10d (3" x 0.128")	6° o.c. 6° o.c. ——————————————————————————————————
31	Ledger strip supporting joists or rafte	ers	3-16d (3-1/2" x 0.135")	@ each joist or rafter
	Description of building materials	Description of fastener	Spacing of Edges (inches)	f Fasteners Intermediate supports (inches) (notes: c, e)
lood structi		vall sheathing to framing and particleboard wall sheath	ing to framing	
32 33 34	3/8" to 1/2" 19/32" to 1" 1-1/8" to 1-1/4"	6d common (2" × 0.113") nail (subfloor, wall) (note j) 8d common (2-1/2" × 0.131") nail (roof) 8d common nail (2-1/2" × 0.131") 10d common (3" × 0.148") nail or 8d deformed (2-1/2" × 0.131") nail	6 	12 (note: g)
Other wall s	sheathing (note h)			
35	1/2" structural cellulosic fiberboard sheathing	1-1/2" galv. roofing nail, 7/16" crown or 1" crown staple 16 ga., 1-1/4" long	3	6
36	25/32" structural cellulosic fiberboard sheathing	1-3/4" galv. roofing nail, 7/16" crown or 1" crown staple 16 ga., 1-1/2" long	3	6
37 	1/2" gypsum sheathing (note d)	1-1/2" galvanized roofing nait, staple galv., 1-1/2" long; 1-1/4" screws, Type W or S	7	7
38	5/8° gypsum sheathing (note d)	1-3/4" galvanized roofing nail; staple galv., 1-5/8" long; 1-5/8" screws, Type W or S	,	
	tural panels, combination subfloor under		6	12
39	3/4" and less	6d deformed (2" x 0.120") nail or 8d common (2-1/2" x 0.131") nail 8d common (2-1/2" x 0.131") nail or		12
	1—1/8° to 1—1/4°	8d deformed (2-1/2" x 0.120") nail 10d common (3" x 0.148") nail or		12

For St 1 inch = 25.4 mm, 1 foot = 304.8 mm, 1 mile per hour = 0.447 m/s; 1 ksi = 6.895 MPa.

- a. All nails are smooth-common, box or deformed shanks except where otherwise stated. Nails used for framing and sheathing connections shall have minimum average bending yield strengths as shown: 80 ksi (551 MPa) for shank diameter of 0.192 inch (20d common nail), 90 ksi (620 MPa) for shank diameters larger than 0.142 inch but not larger than 0.177 inch, and 100 ksi (689 MPa) for shank diameters of 0.142 inch or less.
- Staples are 16 gage wire and have a minimum 7/16-inch on diameter crown width. c. Nails shall be spaced at not more than 6 inches on center at all supports where spans are 48 inches or greater.
- Four-foot-by-8-foot or 4-foot-by-9-foot panels shall be applied vertically. e. Spacing of fasteners not included in this table shall be verified w/ EOR.
- f. For regions having basic wind speed of 110 mph or greater, 8d deformed nails shall be used for attaching plywood and wood structural panel roof sheathing to framing within minimum 48-inch distance from gable end walls, if mean roof height is more than 25 feet, up to 35 feet maximum.
- g. For regions having basic wind speed of 100 mph or less, nails for attaching wood structural panel roof sheathing to gable endwall framing shall be spaced 6 inches on center. When basic wind speed is greater than 100 mph, nails for attaching panel roof sheathing to intermediate supports shall be spaced 6 inches on center for minimum 48-inch distance from ridges, eaves and gable end walls; and 4 inches on center to gable end wall framing.
- Gypsum sheathing shall conform to ASTMC 1396 and shall be installed in accordance with GA 253. Fiberboard sheathing shall conform to ASTM C 208. Spacing of fasteners on floor sheathing panel edges applies to panel edges supported by framing members and at all floor perimeters only. Spacing of fasteners on roof sheathing panel edges applies to panel edges supported by framing members and at all roof plane perimeters. Blocking of roof or floor sheathing panel edges perpendicular to the framing members shall not be required except at intersection of adjacent roof planes. Floor and roof perimeter shall be supported by
- Where a rafter is fastened to an adjacent parallel ceiling joist in accordance with this schedule, provide two toe nails on one side of the rafter and toe nails from the ceiling joist to top plate in accordance with this schedule. The toe nail on the opposite side of the rafter shall not be required.







SHEAR WALL SCHEDULE

CHAPTER 6 WALL CONSTRUCTION

TYPICAL PORTAL FRAME CONSTRUCTION -

SECTION

EXTENT OF HEADER WITH SINGLE PORTAL FRAME
(ONE BRACED WALL PANEL)

MIN. 3"x11%" NET HEADER STEEL HEADER PROHIBITED IF %" SPACER IS USED, PLACE ON BACK-SIDE OF HEADEL

FASTEN SHEATHING TO HEADER WITH 8D COMMON OR GALVANIZED BOX NAILS IN 3" GRED PATTERN AS SHOWN

HEADER TO JACK-STUD STRAP PER TABLE —— R602, 10,64 ON BOTH SIDES OF OPENING OPPOSITE SIDE OF SHEATHING

MIN. DOUBLE 2X4 FRAMING COVERED WITH MIN.
NOT THICK WOOD STRUCTURAL PANEL SHEATHING
WITH 8D COMMON OR GALVANIZED BOX NAILS AT
3" O.C. IN ALL FRAMING (STUDS, BLOCKING, AND

IIN. LENGTH OF PANEL PER TABLE RE02.10.5

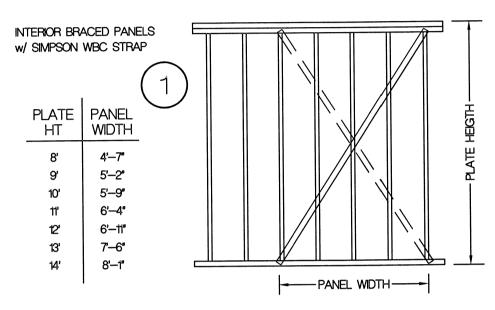
TOP AND BOTTOM OF FOOTING, LAP BARS 15"

Alcon has hopen has has has has has A service of the serv — MIN. FOOTING SIZE UNDER OPENING IS 12"x12". A TURNED DOWN SLAB SHALL BE PERMITTED AT DOOR OPENINGS.

FRONT ELEVATION

--- MIN. (1) % DIAMETER ANCHOR BOLT INSTALLED PER SECTION R403.1.5 - WITH 2"X 2" X*16" PLATE WASHER

LOAD TABLE LOCATION MIN. DL | MIN. LL (PSF) (PSF) 60 10 **EXTERIOR BALCONIES** 10 40 **DECKS** 10 CEILING w/o STORAGE 10 20 CEILING W/ STORAGE 40 10 NON-SLEEPING ROOMS SLEEPING ROOMS 30 10 10 25 ROOF-LIGHT COVERING 20 25 ROOF-HEAVY COVERING



INT. BRACED WALL PANEL LIB, METAL STRAP ALT. TO LET IN 1 X 4

> 2018 International Residential Code Third Printing: Sep 2019

> > SECTION

11

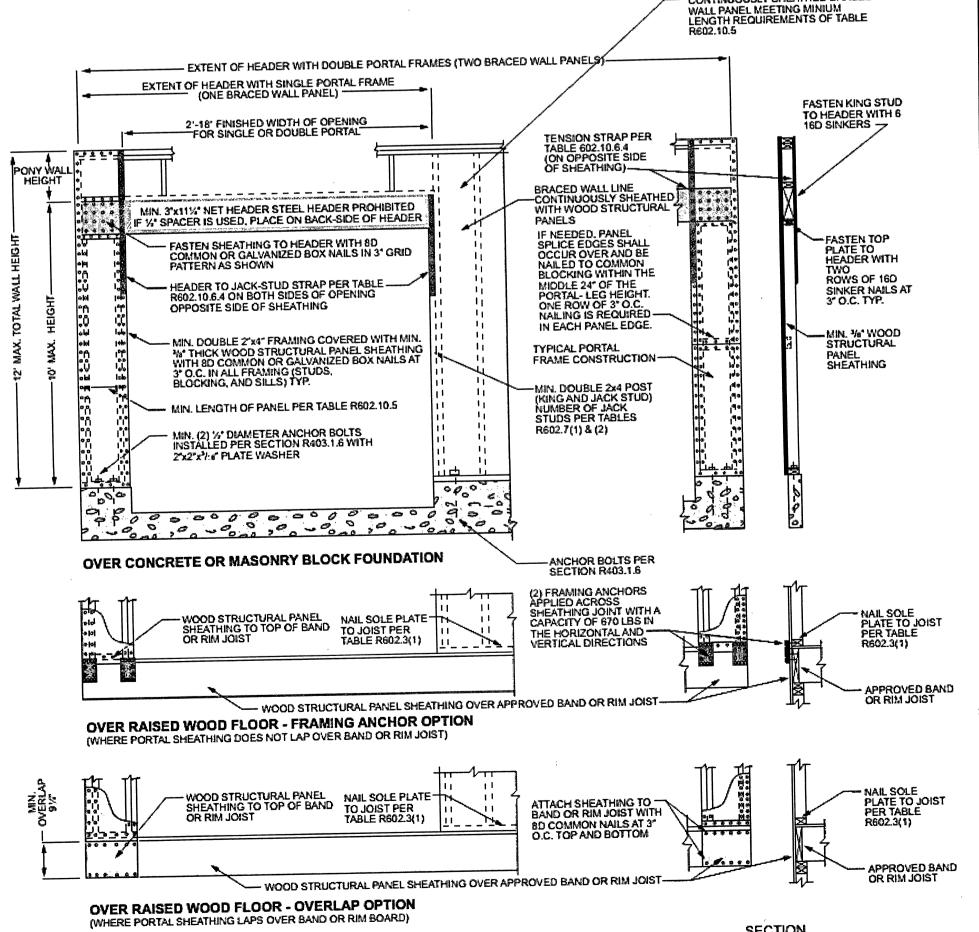
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CHAPTER 6 WALL CONSTRUCTION



For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm.

FRONT ELEVATION

DIGITAL CODES

FIGURE R602.10.6.4 METHOD CS-PF—CONTINUOUSLY SHEATHED PORTAL FRAME PANEL CONSTRUCTION

CONTINUOUSLY SHEATHED BRACED TE OF MISS ---KENNETH SIDOROWICZ NUMBER

EDGE INSTALLATION

SIMPSON STHDRJ RIM JOIST INSTALLATION

SIMPSON CMSTC

For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm.

METHOD PFH—PORTAL FRAME WITH HOLD-DOWNS

FIGURE R602.10.6.2