

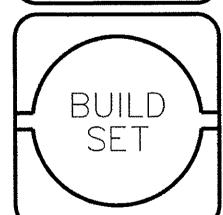
DESCRIPTION:

LEFT AND RIGHT ELEVATIONS

MODEL: Sycamore 2

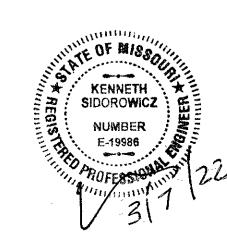
DATE: 10/22/15

ARCHITECT IS NOT RESPONSIBLE FOR THE STRUCTURAL ELEMENTS OF THESE PLANS. A STRUCTURAL ENGINEER MAY NEED TO VERIFY ALL STRUCTURAL ASPECTS OF THESE PRINTS BEFORE CONSTRUCTION BEGINS. FIELD CONDITIONS MAY BE DIFFERENT FROM PLAN. ALL STATE AND LOCAL CODES TAKE PRECIDENCE OVER THESE PLANS. CONTRACTOR WILL BE RESPONSIBLE FOR PLAN INTEGRITY AND CODE COMPLIANCE.



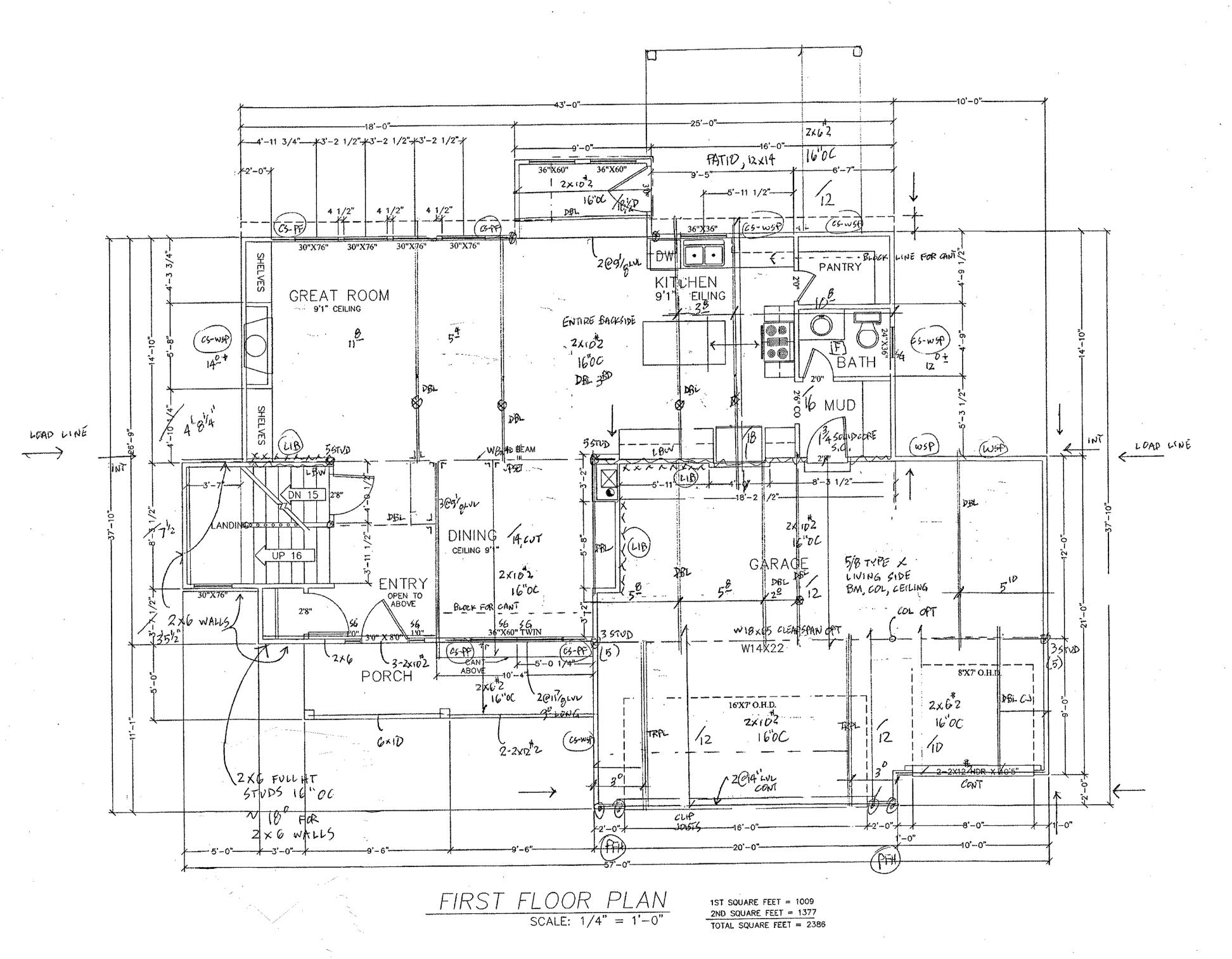
LSMO SVF 95

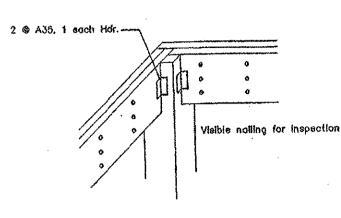
3112 SW SUMMIT VIEW TRAIL



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2 of 6





DF/L M

CS-WSP HOUSE IS SHEATHED W/ 78" OSB APA PANELS, SMART PANEL OR EQUAL, INSTALLED PER MANU. SPECS, SHIP LAPPED PANELS REQUIRE NAILING OF OVER AND UNDER PANELS SEPARATELY.

LIB INT SHALL BE SIMPSON STRAP (CS16)

CS-PF HEADER LENGTHS ARE SHOWN

SIDING LAPS RIM

2x4, 9' PLATE, FULL HT. STUDS

S.C. = SELF CLOSING

D2 GN #25 FOR WINDOWS

CS = CONTINUOUSLY SHEATHED

EC = END CONDITION

SEE D2 FOR INSULATION VALUES

EC#5, 16" LONG CS16 STRAP,
CENTERED ON SUBFLOOR, FILL

ALL NAIL HOLES.

DESCRIPTION: FIRST FLOOR FF

MODEL:

SYCAMORE 2 DATE: 1/1/16

ARCHITECT IS NOT RESPONSIBLE FOR THE STRUCTURAL ELEMENTS OF THESE PLANS. A STRUCTURAL ENGINEER MAY NEED TO VERIFY ALL STRUCTURAL ASPECTS OF THESE PRINTS BEFORE CONSTRUCTION BEGINS. FIELD CONDITIONS MAY BE DIFFERENT FROM PLAN. ALL STATE AND LOCAL CODES TAKE PRECIDENCE OVER THESE PLANS. CONTRACTOR WILL BE RESPONSIBLE FOR PLAN INTEGRITY AND CODE COMPLIANCE

BUILD

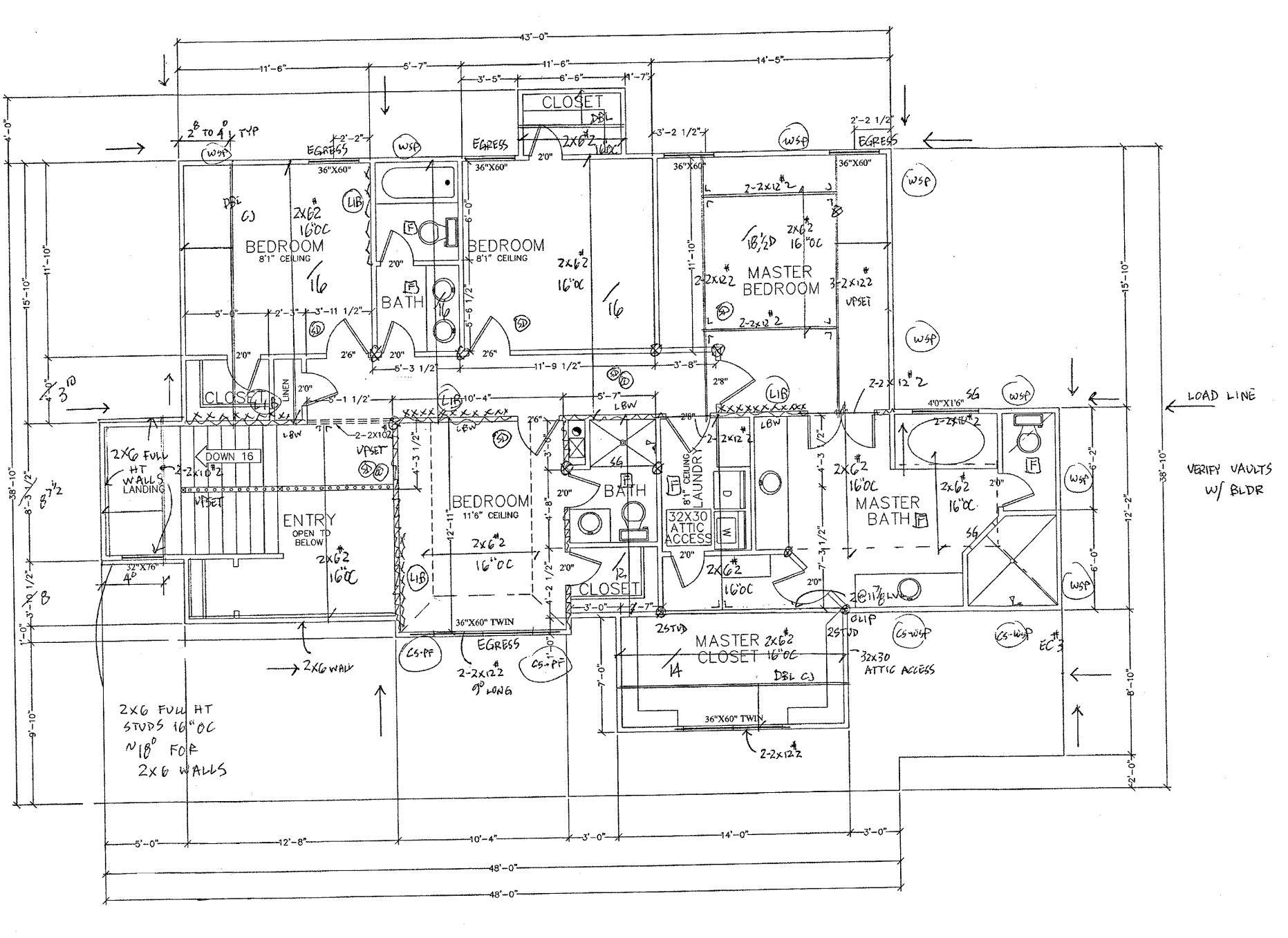
LSMO SVF 95 3112 SW SUMMIT VIEW TRAIL

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 $3_{of}6$ 

SHEET NO



SECOND FLOOR PLAN SCALE: 1/4" = 1'-0" 2ND SQUARE FEET = 1377

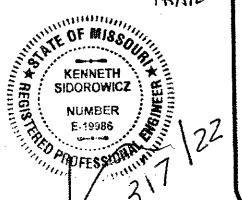
SECOND FLOOR ROOF PLAN DESCRIPTION:

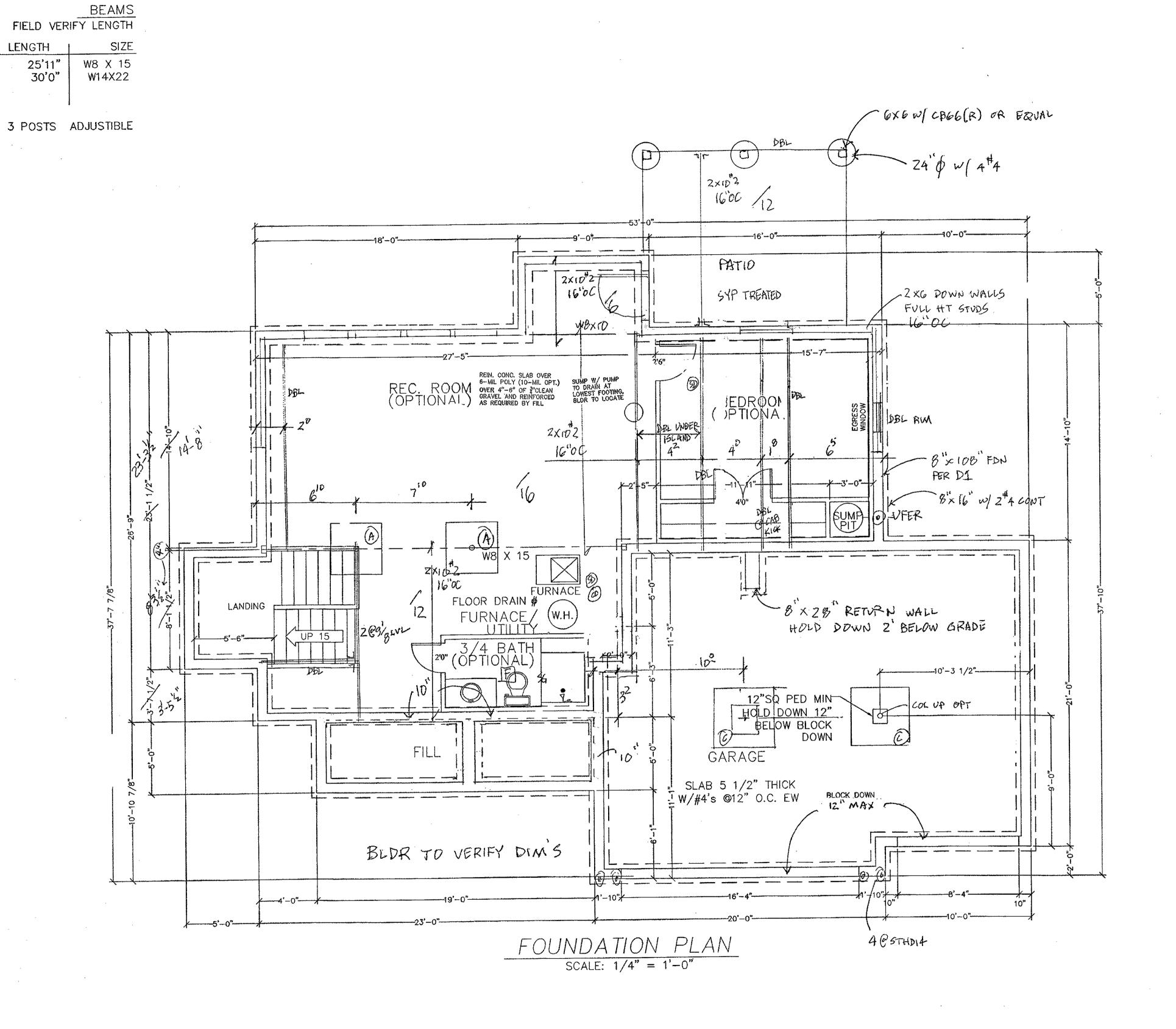
SYCAMORE

ARCHITECT IS NOT RESPONSIBLE FOR THE STRUCTURAL ELEMENTS OF THESE PLANS. A STRUCTURAL ENGINEER MAY NEED TO VERIFY ALL STRUCTURAL ASPECTS LOCAL CODES TAKE PRECIDENCE OVER THESE PLANS. CONTRACTOR WILL BE RESPONSIBLE FOR PLAN INTEGRITY AND CODE COMPLIANCE

BUILD SET

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DESCRIPTION: FOUNDATION

MODEL: Sycamore

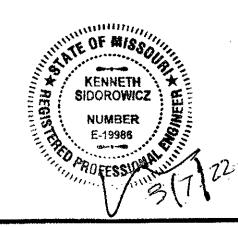
> DATE: 1/4/16

ARCHITECT IS NOT RESPONSIBLE FOR THE STRUCTURAL ELEMENTS OF THESE PLANS. A STRUCTURAL ENGINEER MAY NEED TO VERIFY ALL STRUCTURAL ASPECTS OF THESE PRINTS BEFORE CONSTRUCTION BEGINS. FIELD CONDITIONS MAY BE DIFFERENT FROM PLAN. ALL STATE AND LOCAL CODES TAKE PRECIDENCE OVER THESE PLANS. CONTRACTOR WILL BE RESPONSIBLE FOR PLAN INTEGRITY AND CODE COMPLIANCE

- (A) 36x36x12 PAD W/ (6) #4's E.W. 3" SCH 40 COL UNO ALL PADS
- B 42x42x14 PAD W/ (7) #4's E.W.
- © 48x48x16 PAD W/ (8) #4's E.W.

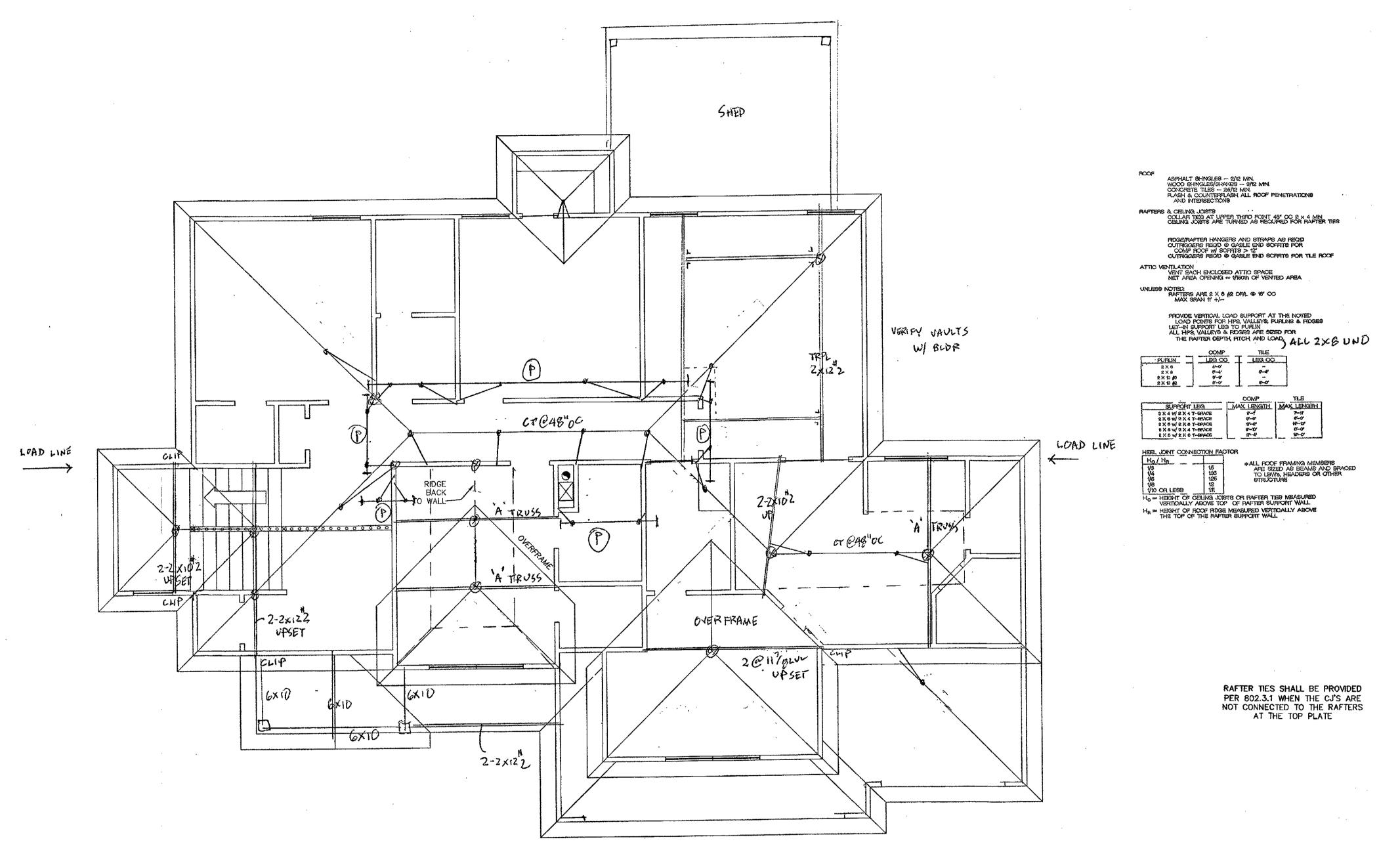
BUILD SET

LSMO SVF 95 3112 SW SUMMIT VIEW TRAIL



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 $\begin{bmatrix} 5 & of & 6 \end{bmatrix}$ SHEET NO:



TION: SCRIP

ROOF

MODEL:

DATE:

ARCHITECT IS NOT RESPONSIBLE FOR THE STRUCTURAL ELEMENTS OF

THESE PLANS. A STRUCTURAL ENGINEER MAY NEED TO

VERIFY ALL STRUCTURAL ASPECTS OF THESE PRINTS BEFORE CONSTRUCTION BEGINS. FIELD

CONDITIONS MAY BE DIFFERENT FROM PLAN. ALL STATE AND LOCAL CODES TAKE PRECIDENCE OVER THESE PLANS. CONTRACTOR WILL BE

RESPONSIBLE FOR PLAN INTEGRITY
AND CODE COMPLIANCE

/22/15

SYCAMORE

ROOF FRAMING

SCALE: 1/4" = 1'-0"

ALL ROOF RAFTERS ARE TO BE #2-2x6 @ 16" OC VNO

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SOFFIT DETAIL

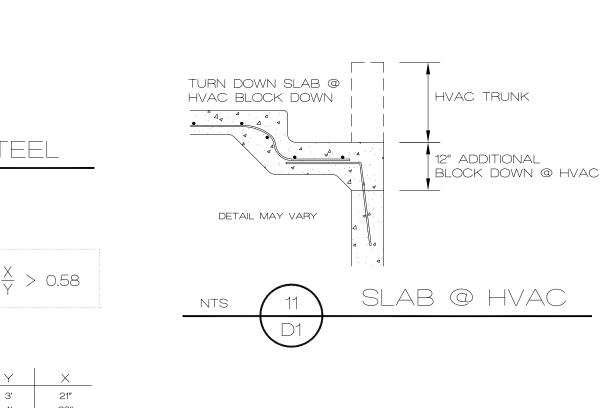
NO STRESS ZONE

HOLD PED. 12" BELOW BLOCK-DOWN

A 3" MIN. COVER

- LOAD PLANE

STRESS ZONE



DETAIL MAY VARY

- TOE OF CUT DETAIL MAY VARY DBL PLATE FOR GYP CRETE FOOTING JOIST HNGR -FOOTING STRESS ZONE PEDESTAL FLOOR DETAIL MAY VARY JOIST 48" X 48" X 16" FTG  $\frac{1}{2}$  1–1/2" COVER MIN. w/ 8 #4's EW DETAIL MAY VARY 1-1/2" LEDGE – 1—1/2" LEDGE MIN. 4 6" MIN. STEM WALL— CAST w/ SLAB 3" MIN COVER - UNDISTURBED PEDESTAL PED @ FTG FLUSH FRAMING @ FDN SLAB @ PEI SLAB ON FILL

<u>DIVISION 3</u> — CONCRETE

TYPE OF CONSTRUCTION

(AIR-ENTRAINED CONCRETE)

CAST AGAINST AND EXPOSED TO EARTH\_

NOT EXPOSED TO EARTH OR WEATHER

EXPOSED TO EARTH OR WEATHER\_

1. ALL CONCRETE WORK SHALL CONFORM TO THE REQUIREMENTS OF ACI 318 "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE" AND ACI 332 "REQUIREMENTS FOR RESIDENTIAL CONCRETE CONSTRUCTION."

COMP. STRENGTH (f'c)

SEE TABLE

SEE TABLE

MASONRY STRENGTH (F'M DESIGN)\_ 1500 BLOCK STRENGTH 1900 MORTAR STRENGTH 1800 GROUT STRENGTH\_\_

SPECIFICALLY NOTED ON PLAN SHALL BE (f'm) 1500 PSI.

1. COMPRESSIVE STRENGTH OF CONCRETE MASONRY CONSTRUCTION

(CMU) SHALL BE AS FOLLOWS (PSI). MASONRY STRENGTH NOT

DIVISION 4 - MASONRY

2. CONCRETE BLOCK SHALL BE HOLLOW LOAD-BEARING CONCRETE MASONRY UNITS CONFORMING TO ASTM C 90, TYPE N-II. ALL BLOCKS SHALL BE PLACED IN RUNNING BOND CONSTRUCTION (UNLESS OTHERWISE NOTED) WITH ALL VERTICAL CELLS IN ALIGNMENT.

3. MORTAR MIX SHALL CONFORM TO THE REQUIREMENTS OF ASTM C 270, TYPE M OR S. TYPE M MORTAR SHALL BE USED WHERE MASONRY IS IN CONTACT WITH SOIL.

4. GROUT SHALL CONFORM TO THE REQUIREMENTS OF ASTM C 476. USE SUFFICIENT WATER FOR GROUT TO FLOW INTO ALL JOINTS OF THE MASONRY WITHOUT SEGREGATION. ALL CELLS IN CONCRETE BLOCKS CONTAINING REINFORCING SHALL BE FILLED SOLID WITH GROUT. ALL MASONRY BELOW FINISHED FLOOR OR GRADE SHALL BE GROUTED SOLID. HOLD GROUT DOWN 1—3" BELOW TOP OF BLOCK AT GROUT LIFT JOINTS AND AT CONCRETE PLACED OVER

. MINIMUM LINTEL, WHERE NOT ON PLANS, SHALL HAVE A MINIMUM OF 2 — #5's CONTINOUS HORIZONTAL BARS IN BOTTOM OF BOND BEAM OR LINTEL BLOCK AND SHALL BE GROUTED SOLID TO A MIN. DEPTH OF 24." ALL LINTEL REINFORCING AND GROUT SHALL EXTEND 2' MINIMUM PAST JAMBS UNLESS NOTED OTHERWISE ON PLANS OR DETAILS.

6. LAP REINFORCING 48 BAR DIAMETERS. STAGGER LAP SPLICES A MINIMUM OF ONE LAP LENGTH.

7. MASONRY VENEER SHALL BE ATTACHED TO SUPPORT WALL FRAMING WITH 36" DIAMETER WALL TIES OR DOVETAIL-TYPE METAL TIES OF EQUIVALENT STIFFNESS EMBEDDED INTO HORIZONTAL MORTAR JOINTS. MAXIMUM VERTICAL SPACING OF TIES SHALL BE 16," MAXIMUM HORIZONTAL SPACING SHALL BE 24." TIES IN ALTERNATE COURSES SHALL BE STAGGERED. PROVIDE #9 WIRE REINFORCING IN HORIZONTAL MORTAR JOINTS AT 16" OC. ENGAGE #9 WIRE WITH WALL ANCHOR TIES. CONSTRUCTION JOINTS IN MASONRY VENEER WALLS SHALL BE LOCATED PER THE DRAWINGS.

8. WATERPROOFING, DRAINAGE PLANE, AND INSTALLATION PER ADOPTED BUILDING CODE.

<u>DIVISION 5.5</u> — MISC. STRUCTURAL STEEL

1. ALL MISCELLANEOUS STRUCTURAL STEEL WORK SHALL CONFORM TO THE REQUIREMENTS OF AISC "SPECIFICATIONS FOR DESIGN, FABRICATION, AND ERECTION OF STRUCTURAL STEEL FOR

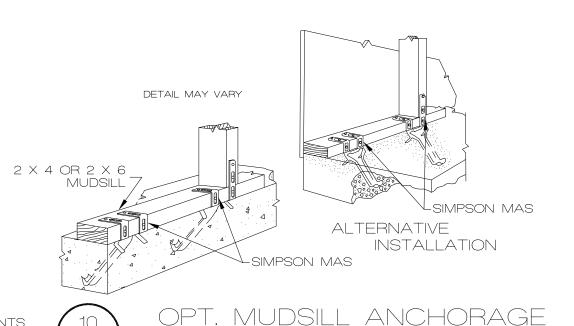
MISCELLANEOUS STRUCTURAL STEEL MATERIAL SHALL COMPLY A) STRUCTURAL STEEL — ASTM A992

B) STEEL PIPE COLUMNS - ASTM A53 GRADE B(Sch 40 TYP) C) ANCHOR BOLTS - ASTM A307 GRADE A, NON-HEADED TYPE UNLESS OTHERWISE NOTED.

3. FLITCH PLATES SHALL HAVE 1" DIA. BOLTS @ 16" OC, STAGGERED TOP AND BOTTOM BETWEEN JOIST LAYOUT.

## RETURN WALLS WALL HT. RETURN SPACING ABOVE FLOOR (HOLD DOWN 24" BELOW GRADE) LESS THAN 4' RETURN WALLS NOT REQ'D 16'-4" ON CENTER (MAX.), AND WITHIN >4' TO 9' 8' OF STEP DOWN OR AS SHOWN

\* RETURN WALLS ALLOW FOR BACKFILL W/O FLOOR DECK IN PLACE FOR 60 PCF EQUIVALENT FLUID WEIGHT SOIL. NO HEAVY EQUIPMENT OR SURCHARE LOADING.



ALTERNATIVE TO J-BOLTS DETAIL MAY VARY MIN.  $\bot$ - 2 X LEDGE 24" LAP, MIN - DRILL & SEAL AS REQ'D

— DOWELS @ 12" OC

SLAB @ WALL NTS SLAB ON FILL CONCRETE OR CMU

1-1/2" COVER -

CONCRETE SLAB

2.5" INSULATION,

ICF WALL

CONC STRENGTH REQ'D STRENGTH FTG 3,000 psi 3,500 psi WALL 3,500 psi SLAB SUS-SLAB 7 SACK MIX

 $\frac{w_{i} * L^{2}}{}$  - 27,206 #-in 40,000 \* 0.2  $0.85 * f_c * b$ 

12'-6" (+/-) MODULE

GARAGE SLAB:

 $\phi M_N = *\phi A * f(d - a/2)$ = 0.9(0.2)(40000)(4-0.22/2)= 28,008 #-in > 27,206 (OKAY).. Use #4 @ 12" OC EW

100 # /中' (LL)

67 # /中' (DL)

 $w_u = 1.2(DL) + 1.6(LL)$ 

 $= 240 \# / \oplus' (TL)$ 

= 0.9(0.2)(40000)(4-0.22/2)= 28,008 #-in > 25,951 (OKAY)

<u>DIVISION 6</u> — ROUGH CARPENTRY

1. ALL ROUGH CARPENTRY WORK SHALL CONFORM TO THE REQUIREMENTS OF NFPA "NATIONAL DESIGN SPECIFICATION OF WOOD CONSTRUCTION", TPI "DESIGN SPECIFICATIONS FOR LIGHT METAL PLATE CONNECTED WOOD TRUSSES", APA "PLYWOOD DESIGN SPECIFICATIONS", DOC PS 1 "PRODUCT STANDARD FOR CONSTRUCTION AND INDUSTRIAL PLYWOOD", DOC PS 56 "STRUCTURAL GLUED" LAMINATED TIMBER", AND APPLICABLE SECTIONS OF THE INTERNATIONAL BUILDING CODE.

2. ROUGH CARPENTRY MATERIALS SHALL COMPLY WITH: A) LUMBER - S4S, S-DRY, KD, OR S-GRN GRADE MARKED, COMPLYING WITH PS 20, GRADED UNDER WWPA OR SPIB RULES: STUD GRADE HEADER: #2 DOUGLAS FIR MIN TYPICAL RAFTER: #2 DOUGLAS FIR PLATES: #2 DOUGLAS FIR BLOCKING: #2 DOUGLAS FIR

> B) METAL FRAMING FASTENERS - ASTM A 153, HOT-DIP GALVANIZED FASTENERS; EQUAL TO SIMPSON STRONG-TIE CONNECTORS COMPLYING WITH APPLICABLE ICC-ES REPORTS C) PLYWOOD - APA RATED SHEATHING, COMPLYING TO PS 1. D) LVL - LAMINATED VENEER LUMBER SHALL BE GRADE 2800 F-2.0E AND SHALL MEET THE REQUIREMENTS OF APPLICABLE

ICC-ES REPORTS. E) GLULAM BEAMS - COMBINATION 24F-V3 IN ACCORDANCE WITH AITC A190.1

3. EXTERIOR WALL AND ROOF SHEATHING SHALL BE  $\frac{7}{16}$ " APA RATED SHEATHING 24/0 EXTERIOR GLUED (MIN) FOR 16" OC STUD SPACING. NAIL SHEATHING TO SUPPORT MEMBERS WITH 8D COMMON NAILS AT 6" ON CENTER ALONG EDGE SUPPORTS AND 12" ON CENTER ALONG FIELD SUPPORTS UNLESS NOTED OTHERWIDE. PROVIDE SOLID BLOCKING AT ALL UNSUPPORTED PANEL EDGES; 4/8 GUN NAILS.

NOTE: ROOF SHEATHING SHALL BE §" APA RATED SHEATHING FOR TILE ROOF, OR AS REQUIRED BY MANUFACTURER.

4. INTERIOR SHEAR WALL SHEATHING WHERE NOTED SHALL BE 2" APA RATED SHEATHING 24/0 EXTERIOR GLUED (MIN) FOR 16" OC STUD SPACING. NAIL SHEATHING TO SUPPORT MEMBERS WITH 8D COMMON NAILS AT 4" ON CENTER ALONG EDGE SUPPORTS AND 6" ON CENTER ALONG FIELD SUPPORTS UNLESS NOTED OTHERWISE. PROVIDE SOLID BLOCKING AT ALL UNSUPPORTED PANEL EDGES.

5. ATTACH METAL FRAMING FASTENERS TO FRAMING MEMBERS WITH MINIMUM NUMBER AND SIZE OF NAILS LISTED IN THE APPLICABLE ICC-ES REPORTS.

B) DESIGN LOADS

PLACEMENT DRAWING.

6. WOOD TRUSS SYSTEM; TRUSS JOIST SYSTEM AND GLULAM SYSTEM FOR ROOFS: A) DESIGN, FABRICATE, AND ERECT IN ACCORDANCE WITH BCSI STANDARDS AND NDS SPECIFICATIONS.

> 25 PSF SNOW LIVE LOAD 10 PSF DEAD LOAD TOP CHORD (20 TILE) 10 PSF DEAD LOAD BOTTOM CHORD C) SUBMIT SHOP DRAWINGS, INCLUDING DESIGN CALCULATIONS,

7. DEFAULT HEADER SIZE NOT SPECIFIED SPANNING 8'-0" MAX SHALL BE  $2-2 \times 10$  #2, WITH 2 STUD SUPPORT.

MATERIAL STRESSES, GRADE AND SPECIES OF WOOD, AND

8. ALL HEADERS OVER 4'-0" SHALL HAVE DOUBLE TRIMMER @ EACH SUPPORT, OR AS SPECIFIED, UNO.

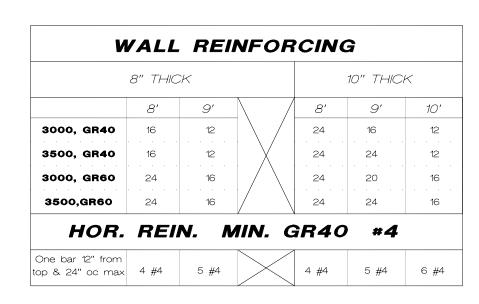
9. SOLID BLOCKING BETWEEN JOISTS @ 36" OC FOR JOISTS PARALLEL TO THE EXTERIOR FOUNDATION WALL, MIN. 48" OR 3 JOIST SPACES.

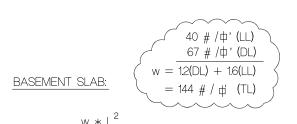
10. ALL FLUSH FRAMING @ HEADERS OR GIRDERS SHALL BE HANGERED. 11. BLOCK BETWEEN JOISTS @ SUPPORTS OR OVER BEAMS.

12. RATED CONSTRUCTION FOR PROJECTIONS INTO SETBACKS AS REQ'D.

13. DOUBLE JOIST BELOW PARALLEL NONBEARING WALLS ON LAYOUT, SINGLE JOIST OFF LAYOUT. STRUCTURE BELOW LOAD-BEARING WALLS AS NOTED ON PLANS.

## FOUNDATION PER JOCOBO RESIDENTIAL FOUNDATION GUIDELINE

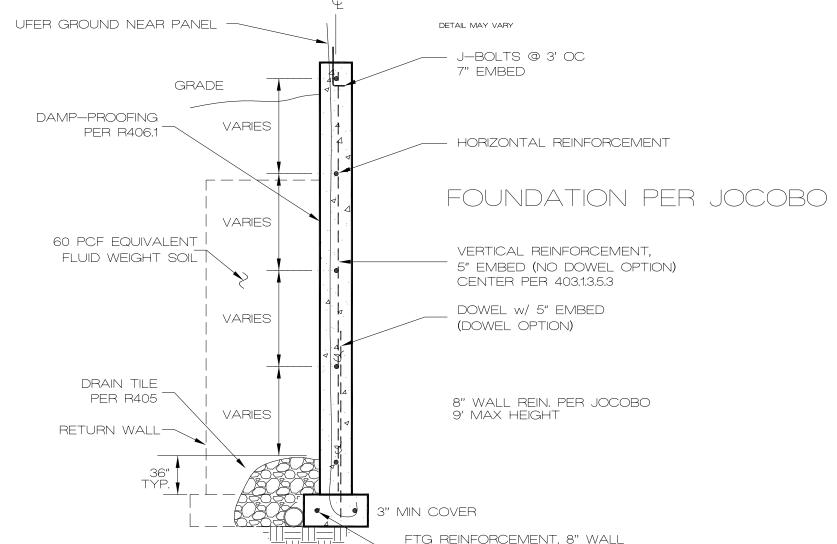


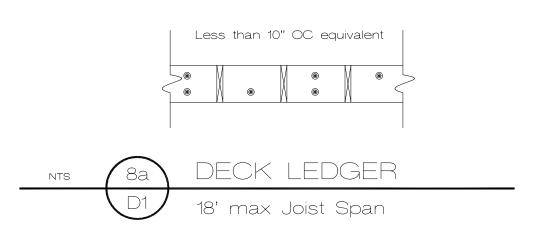


<u>W<sub>i</sub> \* L<sup>-</sup></u> → 25,951 #—in 40,000 \* 0.2  $\frac{0.85 * 3,500 * 12}{0.85 * 3,500 * 12} = 0.22"$ 

 $\phi M_N = *\phi A * f(d - a/2)$ 

∴ Use #4 @ 12" OC EW 15'-6" (+/—) MODULE

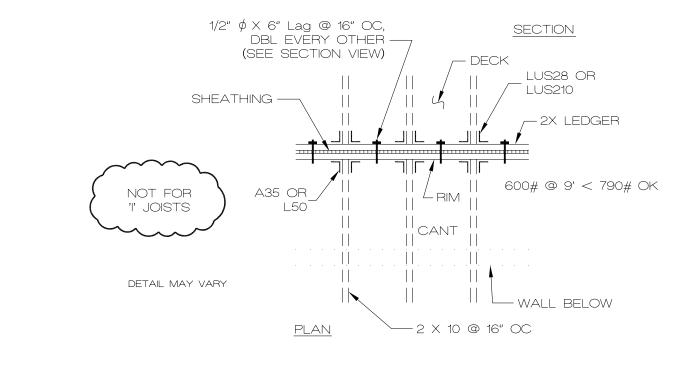


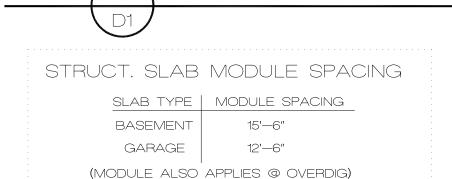


8" × 16" w/

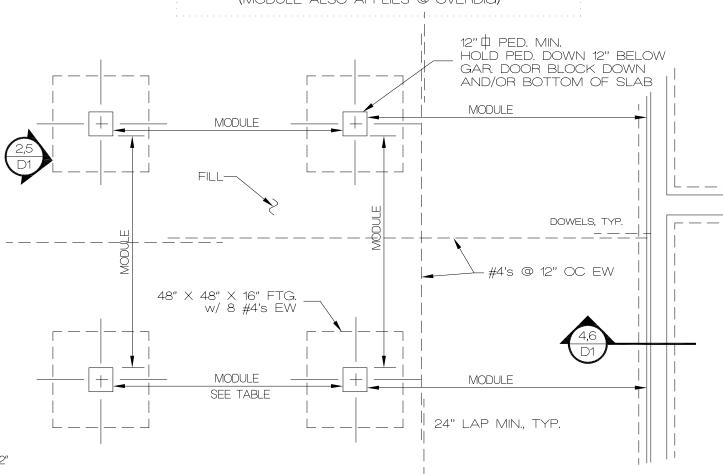
2#4 CONTINUOUS

WALL REINFORCEMENT





DECK @ CANTILEVER



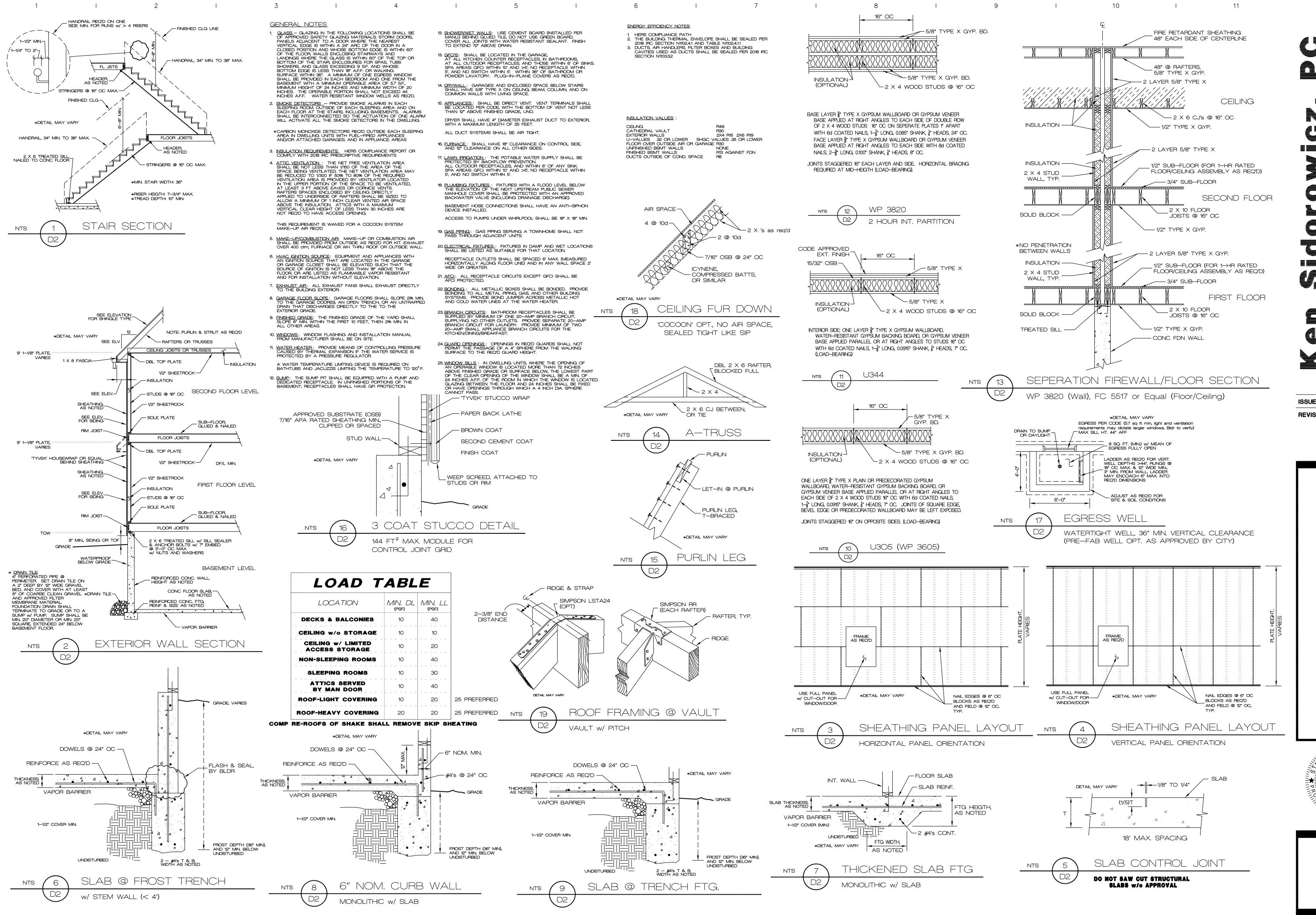
STRUCTURAL SLAB ON FILL DO NOT SAW CUT STRUCTURAL SLABS w/o APPROVAL

VERIFY ALL STRUCTURAL SLAB DETAILS W/ ENGINEER

DO NOT ISOLATE COLUMNS FROM STRUCTURAL SLABS

ISSUE DATE **REVISIONS** 

KENNETH 2 SIDOROWICZ **NUMBER** E-19986



STE DATE
SKOISIAN
SKOOLISOR9, Parkville, Missouri 64152

3 DETAIL SHEET

KENNETH SIDOROWICZ NUMBER E-19986

**D2** 

## STAPLES NOT PERMITTED IN KCMO

	FAST	TENER SCHEDULE FO	OR STRUCTURAL	MEMBERS
tem	Description of building ele	ements	Number & type of fastener (notes: a, b, c)	Spacing of fasteners
of				
1 2 3 4 5	Rafter to plate, toe nail, note trusses	rafter, laps over partitions, face nail 20 ga. ridge strap use STC clips at NLB walls and spec'd holdowns	3-8d (2-1/2" × 0.113") 3-8d (2-1/2" × 0.113") 3-10d 3-10d (3" × 0.128") 3-16d or 3-10d (3-1/2" × 0.135", 0.148")	2 toe nails side 1, 1 toe nail side 2 (note j)
6	Roof rafters to ridge, valley or hip raf	ters:	4-16d (3-1/2" x 0.135")	_
	Face nail:		3-16d (3-1/2" × 0.135")	_
1				
7	Built-up studs-face nail		10d (3" × 0.128")	24" o.c.
8	Abutting studs at intersecting wall co	rners, face nail	16d (3-1/2" × 0.135")	12" o.c.
9	Built-up header, two pieces w/ 1/2" s	spacer	16d (3-1/2" × 0.135")	16" o.c. along each edge 16" o.c. along each edge
10	Continued header, two pieces		16d (3-1/2" x 0.135")	ib oc. along each eage
[11]	Continuous header to stud, toe nail		4-8d (2-1/2" × 0.113")	
.12	Double studs, face nail		10d (3" × 0.128") 10d (3" × 0.128")	24° o.c.
.13	Double top plates, face nail	and taken from the language area.	8—16d (3—1/2" × 0.135")	<del> </del>
.14	Double top plates, min. 48" offset of	end joints, face nail in lapped area	8-16d (3-1/2" x 0.135")	
.15	Sole plate to joist or blocking, face n	all	3-16d (3-1/2" × 0.135")	16° o.c.
.16	Sole plate to joist or blocking at brace	wall panels	3-8d (3-1/2" x 0.113") or	
17	Stud to sole plate, toe nail		2-16d (3-1/2" × 0.135")	
	Top or sole plate to stud, end nail		2-16d (3-1/2" × 0.135")	
.18 19.	Top plates, laps at corners and inter	sections, face nail	2-10d (3" × 0.128")	
.19 .20	1" brace to each stud and plate, face		2-8d (2-1/2" × 0.113")	]
			2 staples 1-3/4"	<b>-</b>
.21	1" x 6" sheathing to each bearing, fa	ce nail	2-8d (2-1/2" × 0.113") 2 staples 1-3/4"	
22	1" x 8" sheathing to each bearing, fa	ce nail	2-8d (2-1/2" × 0.113")	
23	Wider than 1" x 8" sheathing to each	n bearing, face nail	3 staples 1-3/4" 3-8cs( <del>2pl6/2</del> "1×3/0/113")	_
or	The state of the s			
24 25 26 27 28 29 30	Joist to sill or girder, toe nail  Firm joist to top plate, toe nail (roof applications also)  Firm joist or blocking to sill plate, toe nail  1" x 6" subfloor or less to each joist, face nail  2" subfloor to joist of girder, blind and face nail  2" planks (plank & beam — floor and roof)		3-8d (2-1/2" x 0.113") 8d (2-1/2" x 0.113") 8d (2-1/2" x 0.113") 2-8d (2-1/2" x 0.113") 2 staples 1-3/4" 2-16d (3-1/2" x 0.135") 2-16d (3-1/2" x 0.135")	6° o.c. 6° o.c
	Built-up girders and beams, 2" lumb		10d (3" x 0.128")	and bottom and staggered. Two nails at ends and at each splice  © each joist or rafter
31	Ledger strip supporting joists or rafte	ers T	3-16d (3-1/2" x 0.135")	
			Spacing of	f Fasteners
	Description of building materials	Description of fastener	Edges (inches)	Intermediate supports (inches) (notes: c, e)
~		vall sheathing to framing and particleboard wall sheathing		
32 32	3/8" to 1/2"	6d common (2" x 0.113") nail (subfloor, wall) (note j)	6	12 (note: g)
33	19/32" to 1"	8d common (2–1/2" × 0.131") nail (roof) 8d common nail (2–1/2" × 0.131") 10d common (3" × 0.148") nail or	6	12 (note: g)
<b>.</b> ⊶	1-1/8" to 1-1/4"	8d deformed (2-1/2" x 0.131") nail		
er wall s	sheathing (note h)			
35	1/2" structural cellulosic fiberboard	1-1/2" galv. roofing nail, 7/16" crown or	3	6
	sheathing	1" crown staple 16 ga., 1-1/4" long		
36	25/32" structural cellulosic	1-3/4" galv. roofing nail, 7/16" crown or	]	
 37	fiberboard sheathing  1/2" gypsum sheathing (note d)	1" crown staple 16 ga, 1-1/2" long 1-1/2" galvanized roofing nail, staple galv,	7	7
38	5/8" gypsum sheathing (note d)	1-1/2" long; 1-1/4" screws, Type W or S 1-3/4" galvanized roofing nail; staple galv.	7	7
	A color and a section of the section	1-5/8" long; 1-5/8" screws, Type W or S		
	tural panels, combination subfloor unde		6	12
39	3/4" and less	6d deformed (2" x 0.120") nail or		_
	7/8° to 1°	8d common (2-1/2" x 0.131") nail 8d common (2-1/2" x 0.131") nail or	6	12
40	7,5 .5 .	8d deformed (2-1/2" x 0.120") nail		

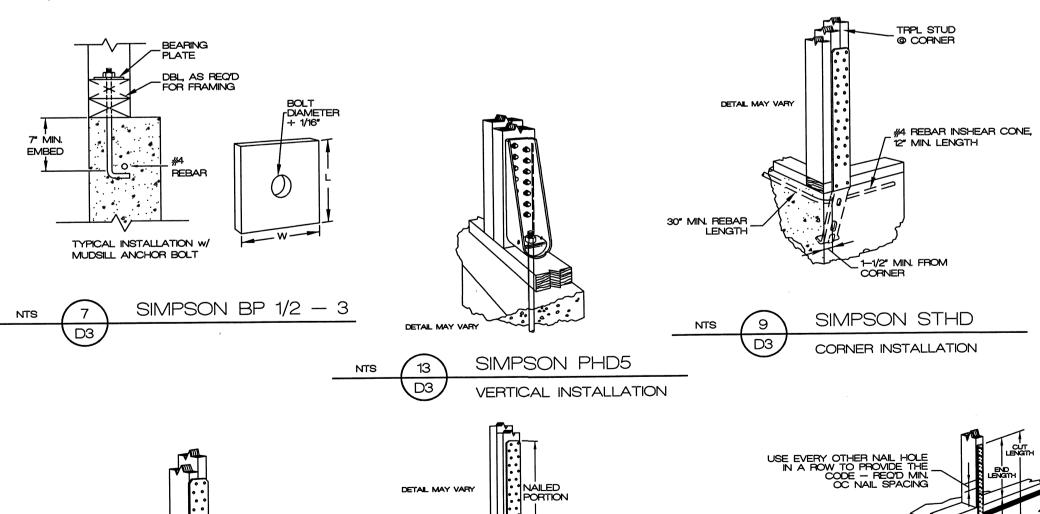
For St 1 inch = 25.4 mm, 1 foot = 304.8 mm, 1 mile per hour = 0.447 m/s; 1 ksi = 6.895 MPa

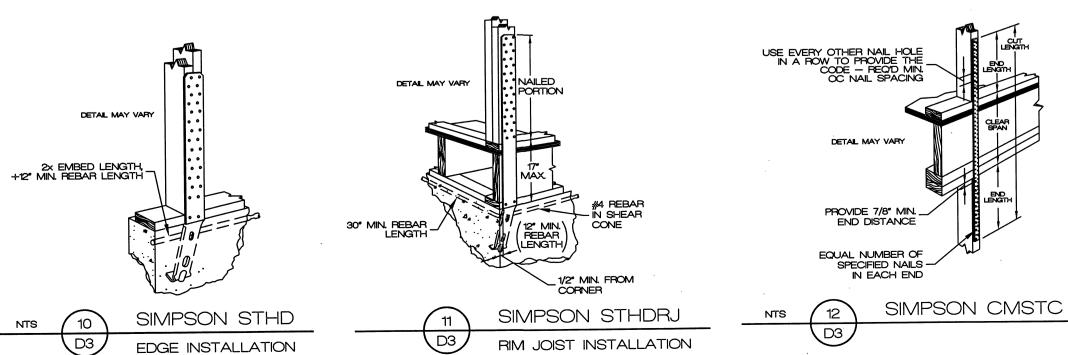
- a. All nails are smooth-common, box or deformed shanks except where otherwise stated. Nails used for framing and sheathing connections shall have minimum average bending yield strengths as shown: 80 ksi (551 MPa) for shank diameter of 0.192 inch (20d common nail), 90 ksi (620 MPa) for shank diameters larger than 0.142 inch but not larger than 0.177 inch, and 100 ksi (689 MPa) for shank diameters of 0.142 inch or less.
- b. Staples are 16 gage wire and have a minimum 7/16—inch on diameter crown width.
  c. Nails shall be spaced at not more than 6 inches on center at all supports where spans are 48 inches or greater.
- four-foot-by-8-foot or 4-foot-by-9-foot panels shall be applied vertically.
   Spacing of fasteners not included in this table shall be verified w/ EOR.
- f. For regions having basic wind speed of 110 mph or greater, 8d deformed nails shall be used for attaching plywood and wood structural panel roof sheathing to framing within minimum 48—inch distance from gable end walls, if mean roof height is more than 25 feet, up to 35 feet maximum.
- g. For regions having basic wind speed of 100 mph or less, nails for attaching wood structural panel roof sheathing to gable endwall framing shall be spaced 6 inches on center to minimum 48—inch distance from ridges, eaves and gable end walls; and 4 inches on center to gable end wall framing.

  ASTIMO 3006 and shall be installed in accordance with GA 253 Fiberboard shealthing shall conform to ASTIMO 3006 and shall be installed in accordance with GA 253 Fiberboard shealthing shall conform to ASTIMO 3006.
- h. Gypsum sheathing shall conform to ASTMC 1396 and shall be installed in accordance with GA 253. Fiberboard sheathing shall conform to ASTM C 208.

  i. Spacing of fasteners on floor sheathing panel edges applies to panel edges supported by framing members and at all floor perimeters only. Spacing of fasteners on roof sheathing panel edges applies to panel edges supported by framing members and at all floor perimeters only. Spacing of fasteners on roof sheathing panel edges applies to panel edges supported by framing members and at all floor perimeters only. Spacing of fasteners on roof sheathing panel edges applies to panel edges supported by framing members and at all floor perimeters only.
- spacing or lasteners on more snearing parties edges applied to partie edges supported by italiang members and at all roof parties. Blocking of roof or floor sheathing panel edges perpendicular to the framing members shall not be required except at intersection of adjacent roof planes. Floor and roof perimeter shall be supported by
- framing members or solid blocking.

  j. Where a rafter is fastened to an adjacent parallel ceiting joist in accordance with this schedule, provide two toe nails on one side of the rafter and toe nails from the ceiting joist to top plate in accordance with this schedule. The toe nail on the opposite side of the rafter shall not be required.





NAIL TOP AND BOTTOM PLATES OF BP's TO JOISTS ABOVE AND BELOW W/ 3 @ 16d @ 16" OC ALL METHODS CONSTRUCTION DESCRIPTION SHEAR WALL SIMPSON CS16 STRAP NAILED TO STUDS SPACED METAL STRAP METHOD AT 16" OC MAXIMUM. STRAPS SHALL BE INSTALLED IN 'V' OR 'X' PATTERN AT THE BRACE LOCATION AND FOR THE SPECIFIED LENGTH, ALTERNATIVE TO \*DBL JOIST MIN. BELOW BRACED WALL WHEN FRAMING BELOW IS PARALLEL TO WALL LINE, OR SOLID BLOCK @ 16" OC BELOW BRACED WALL WHEN FRAMING BELOW IS PERPENDICULAR TO WALL LINE. COLLECTOR OR DRAG STRUT OVER. 7/16" STRUCTURAL SHEATHING OVER STUDS SPACED 16" OC W/ 8d COMMON NAILS AT 6" OC EDGE AND 12" FIELD, HORIZONTAL JOINTS SHALL BE SHEATHING METHOD BLOCKED FOR ANCHORAGE. \*DBL JOIST MIN. BELOW BRACED WALL WHEN FRAMING BELOW IS PARALLEL TO WALL LINE, OR SOLID BLOCK @ 16" OC BELOW BRACED WALL WHEN FRAMING BELOW IS PERPENDICULAR TO WALL LINE. 6 TO 1 ASPECT RATIO, HEADER LENGTH AS SPECIFIED WITH FULL PANEL SHEATHING AT UPPER CORNERS CUTOUT FOR THE OPENING. BLOCKING AT HORIZONTAL JOINTS. NOTE FULL 4' WIDTH CUTOUT PANELS REQ'D AT CORNERS. STHD10 & LSTA STRAPS GARAGE DOOR PORTAL PFH HEADER LENGTH AS SPECIFIED EXTENDED TO NEXT LAYOUT STUD, 18" MINIMUM WIDTH. FULL PANEL SHEATHING REQ'D WITH CUTOUTS FOR OPENINGS. PORTALS CS-PF HORIZONTAL BLOCKING AT EDGES. 'J' BOLT SPACING FOR SHEAR WALLS IS 3' OC WITH STRAPS AS NOTED.

NTS 8 SHEAR WALL SCHEDULE

CHAPTER 6 WALL CONSTRUCTION

FIGURE R602.10.6.2

METHOD PFH—PORTAL FRAME WITH HOLD-DOWNS

TYPICAL PORTAL FRAME CONSTRUCTION -

SECTION

EXTENT OF HEADER WITH SINGLE PORTAL FRAME
(ONE BRACED WALL PANEL)

MIN. 3"x11%" NET HEADER STEEL HEADER PROHIBITED IF %" SPACER IS USED, PLACE ON BACK-SIDE OF HEADEL

> FASTEN SHEATHING TO HEADER WITH 8D COMMON OR GALVANIZED BOX NAILS IN 3" GRED PATTERN AS SHOWN

HEADER TO JACK-STUD STRAP PER TABLE —— R602, 10,64 ON BOTH SIDES OF OPENING OPPOSITE SIDE OF SHEATHING

MIN. DOUBLE 2X4 FRAMING COVERED WITH MIN.
NOT THICK WOOD STRUCTURAL PANEL SHEATHING
WITH 8D COMMON OR GALVANIZED BOX NAILS AT
3" O.C. IN ALL FRAMING (STUDS, BLOCKING, AND

IIN. LENGTH OF PANEL PER TABLE RE02.10.5

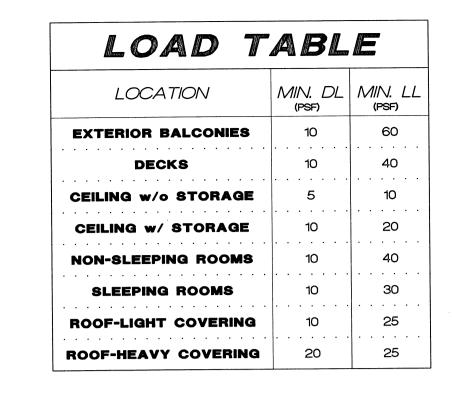
TOP AND BOTTOM OF FOOTING, LAP BARS 15"

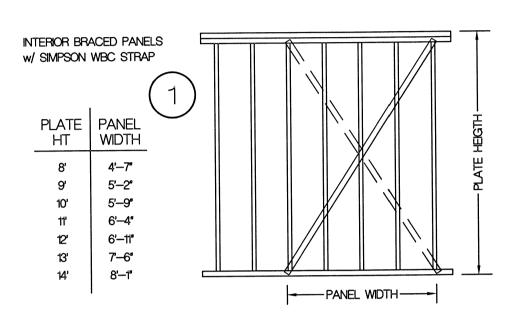
MIN. FOOTING SIZE UNDER OPENING IS 12'X12". A TURNED DOWN SLAB SHALL BE PERMITTED AT DOOR OPENINGS.

FRONT ELEVATION

For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm.

--- MIN. (1) % DIAMETER ANCHOR BOLT INSTALLED PER SECTION R403.1.5 - WITH 2"X 2" X\*16" PLATE WASHER





INT. BRACED WALL PANEL

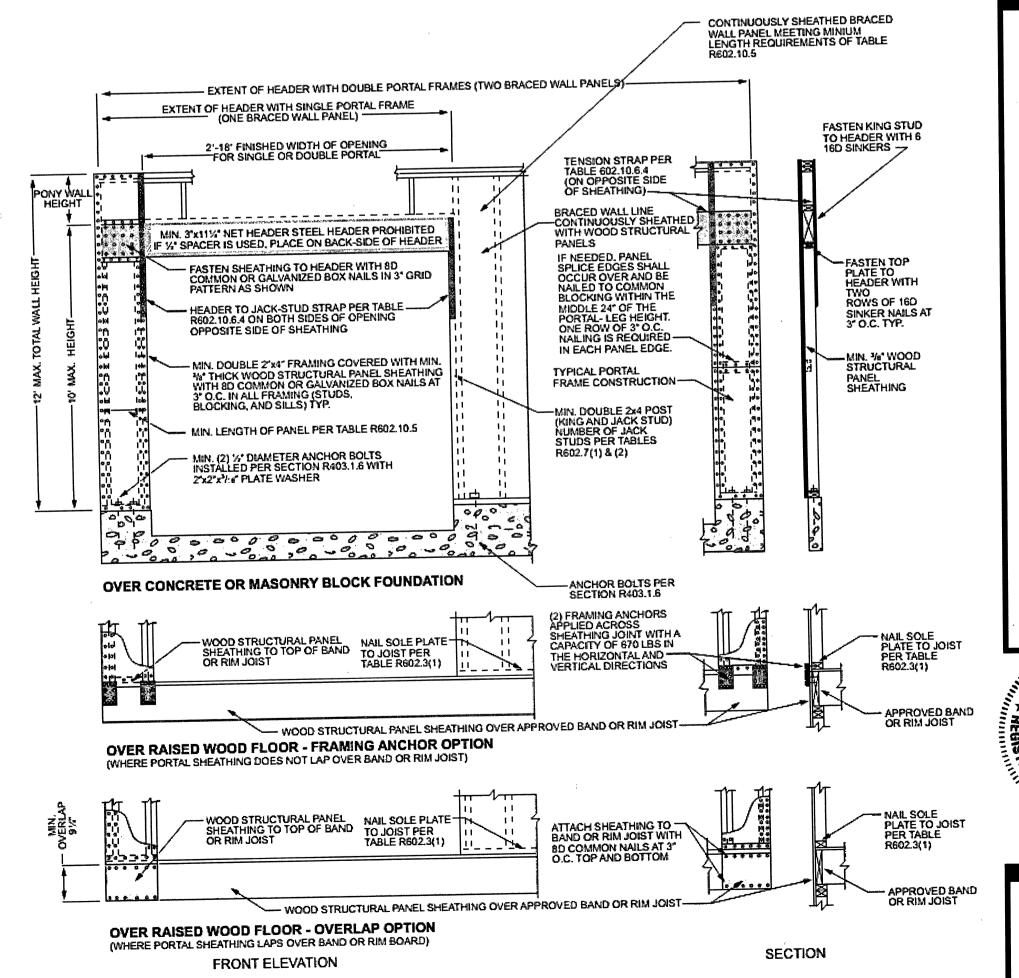
LIB, METAL STRAP ALT. TO

LET IN 1 X 4

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CHAPTER 6 WALL CONSTRUCTION



For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm.

DIGITAL CODES

FIGURE R602.10.6.4
METHOD CS-PF—CONTINUOUSLY SHEATHED PORTAL FRAME PANEL CONSTRUCTION

SSUE DATE

**REVISIONS** 

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KENNETH SIDOROWICZ NUMBER E-19986

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