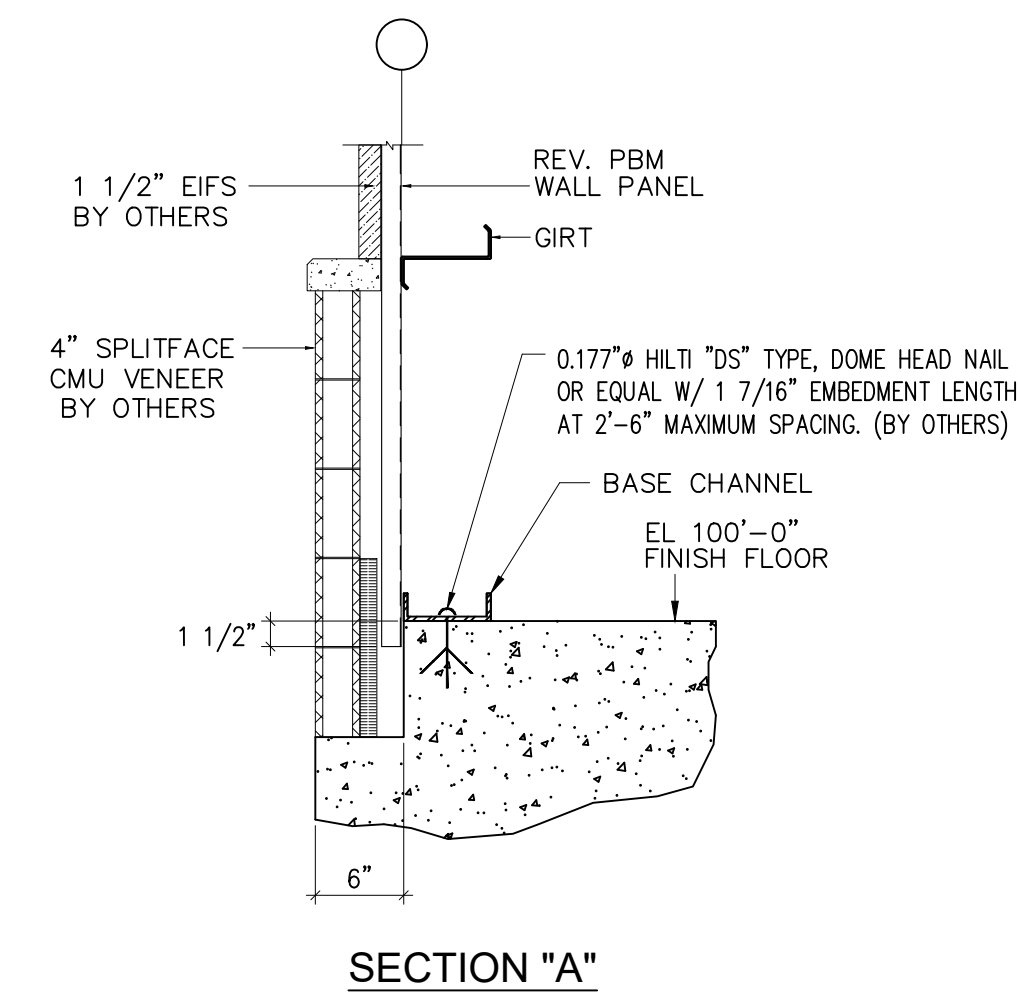
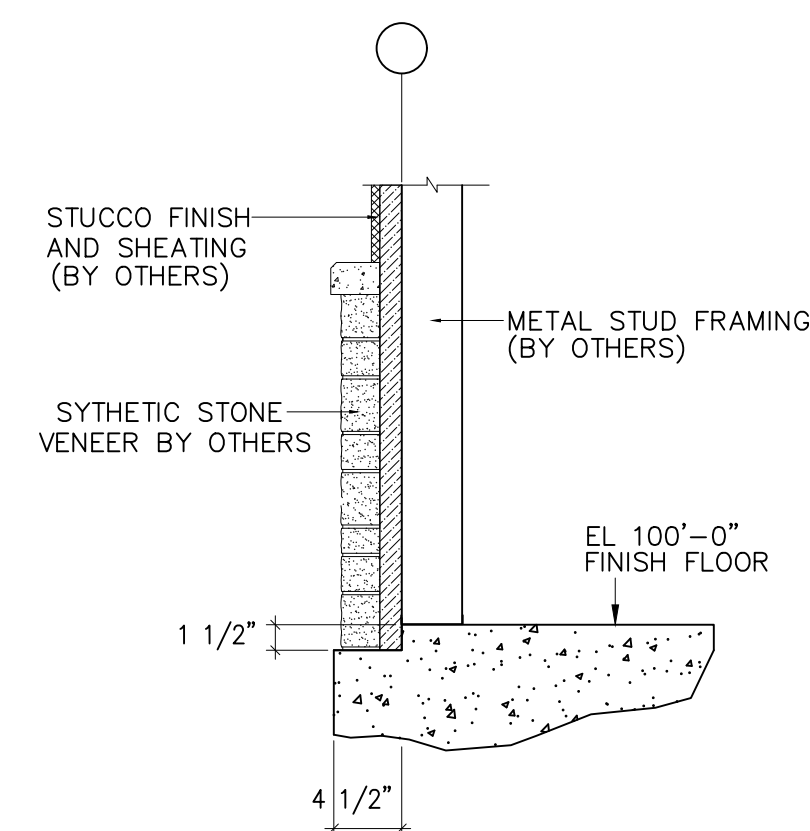


**COLUMN LAYOUT PLAN**

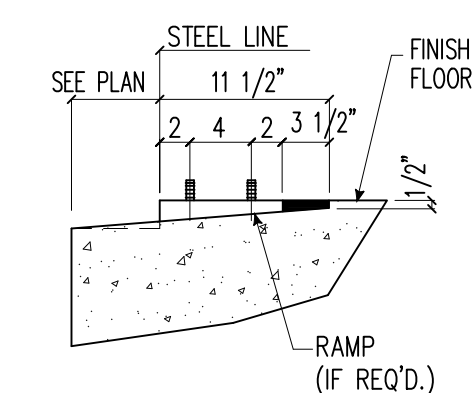
NOTE: All Base Plates @ 100'-0" (F.F.L.) U.N.



**SECTION "A"**

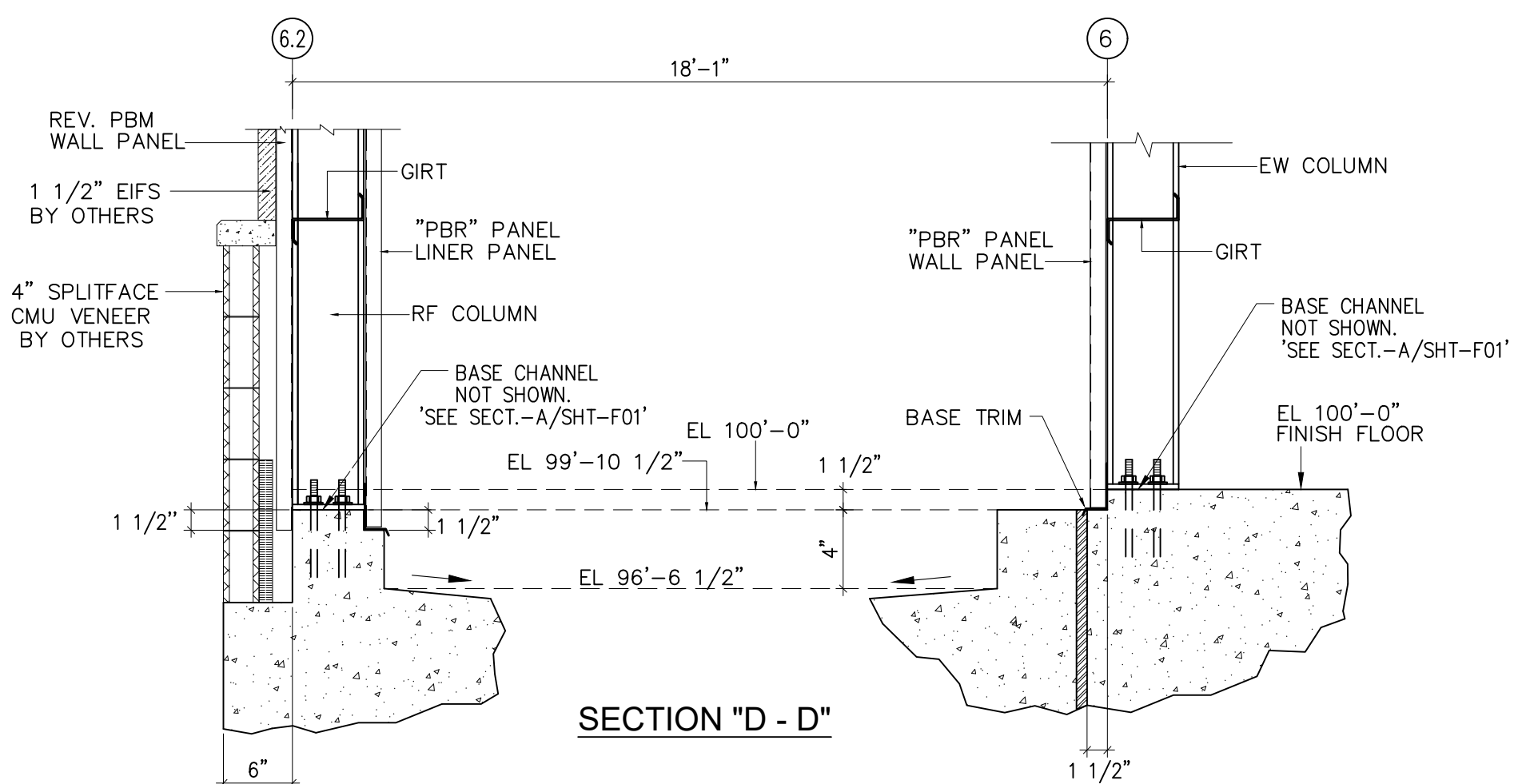


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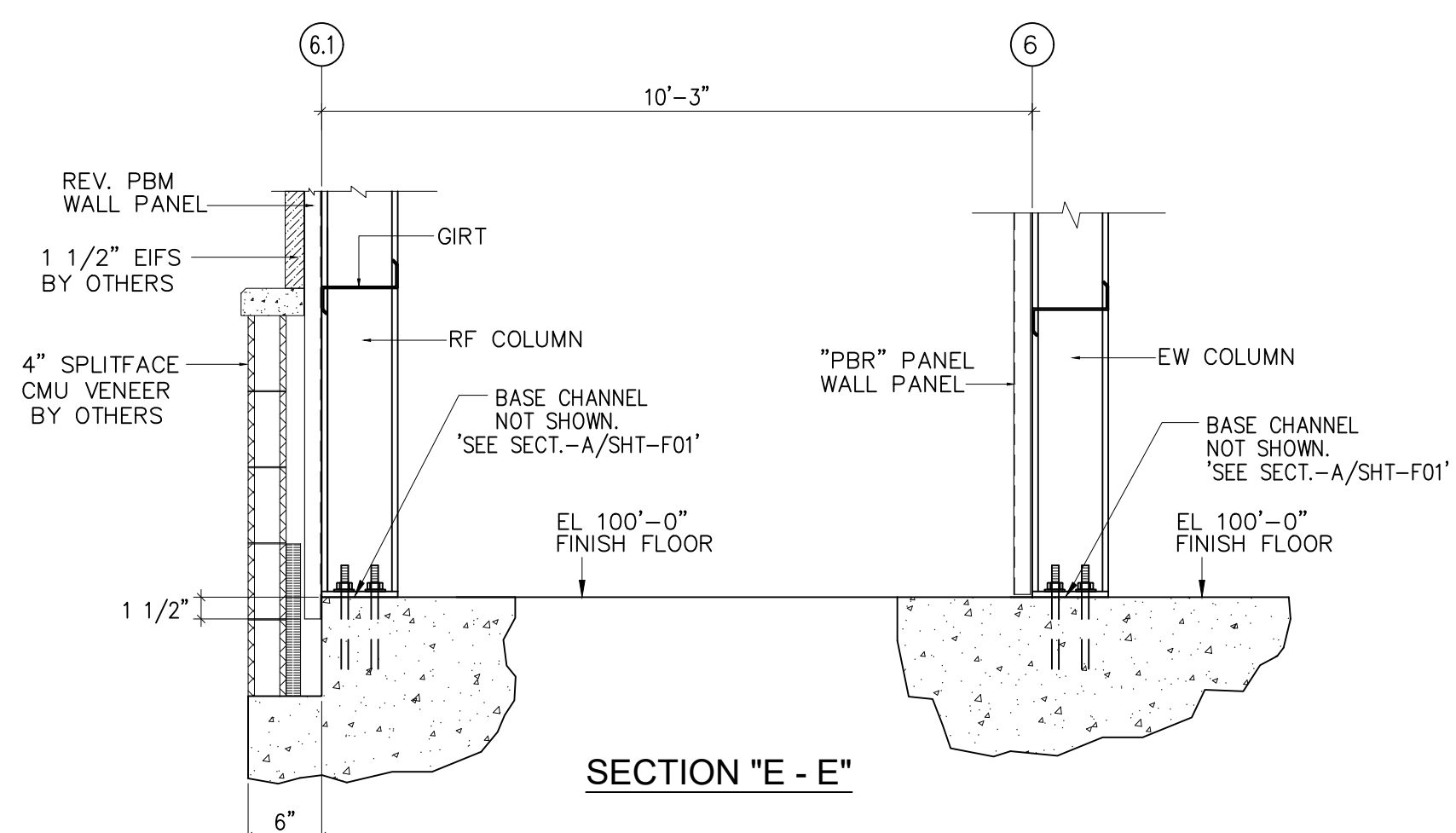


**SECTION "C"**

**FOR CONSTRUCTION**



**SECTION "D - D"**



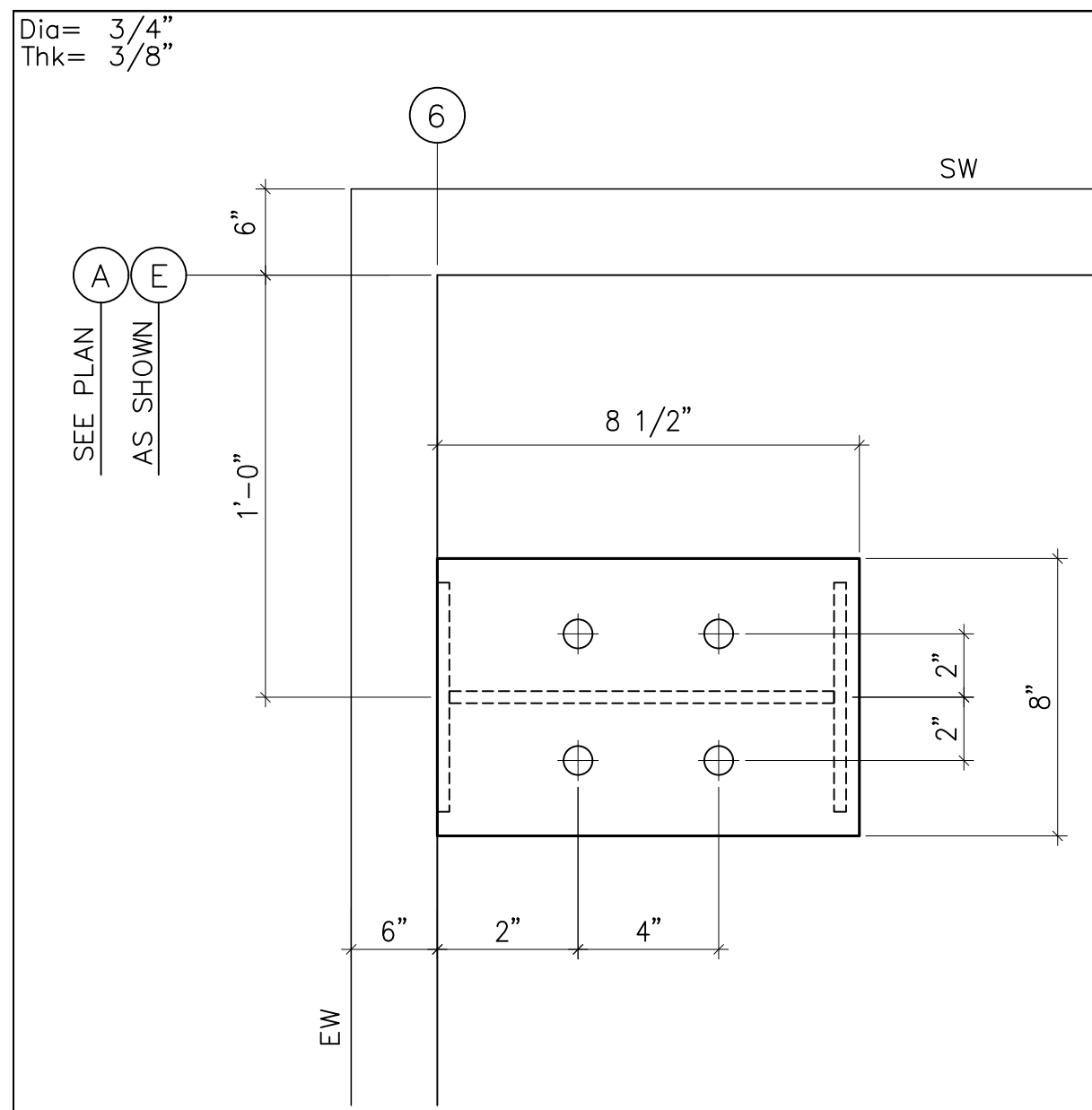
**SECTION "E - E"**

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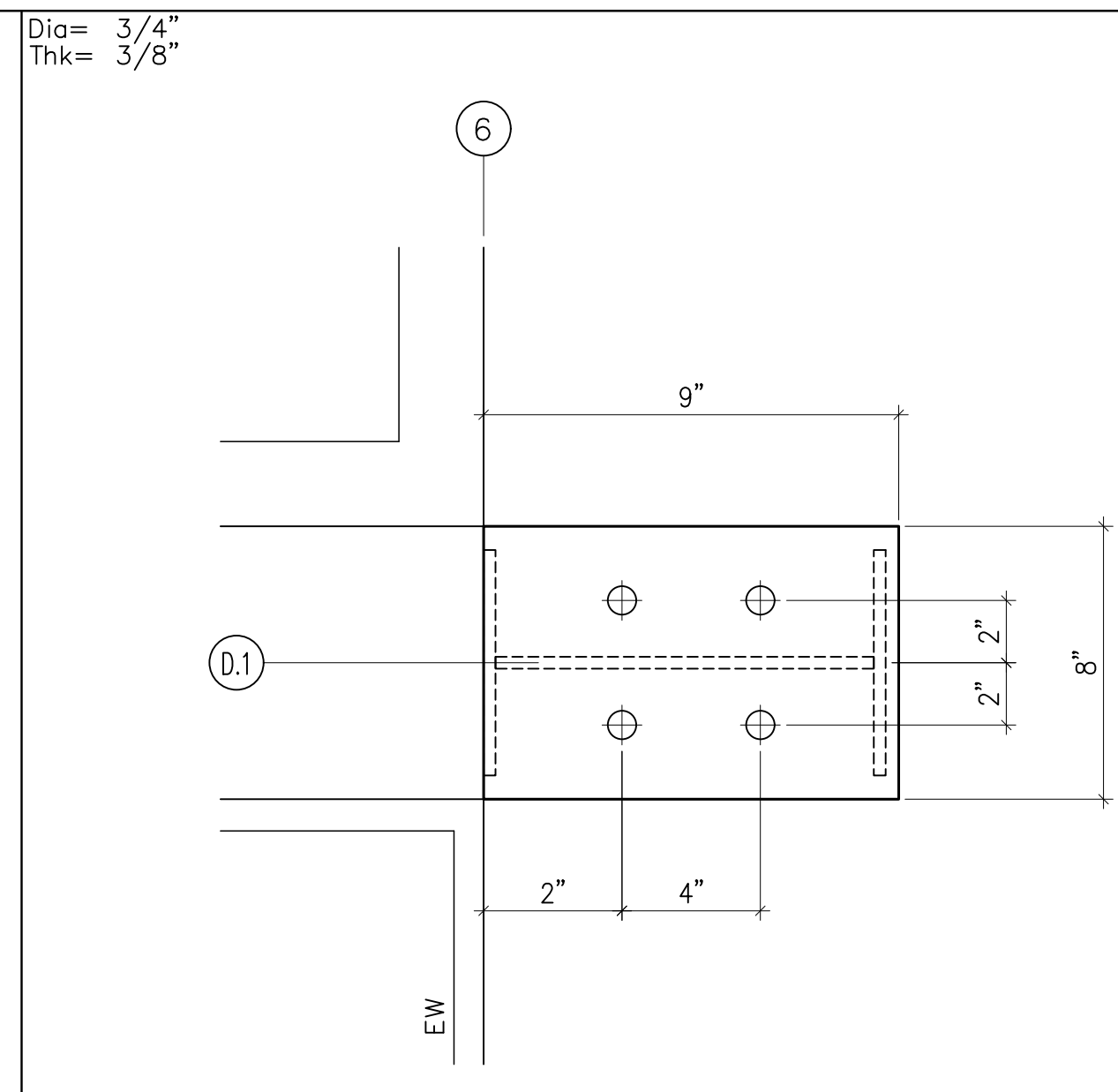
ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
0	CONSTRUCTION/PERMIT	01/13/22	RD	RCR	JPL

**RIGID**  
GLOBAL BUILDINGS  
18933 Aldine Westfield  
Houston, Tx 77073  
Phone : (281) 443-9065  
Fax : (281) 443-9064

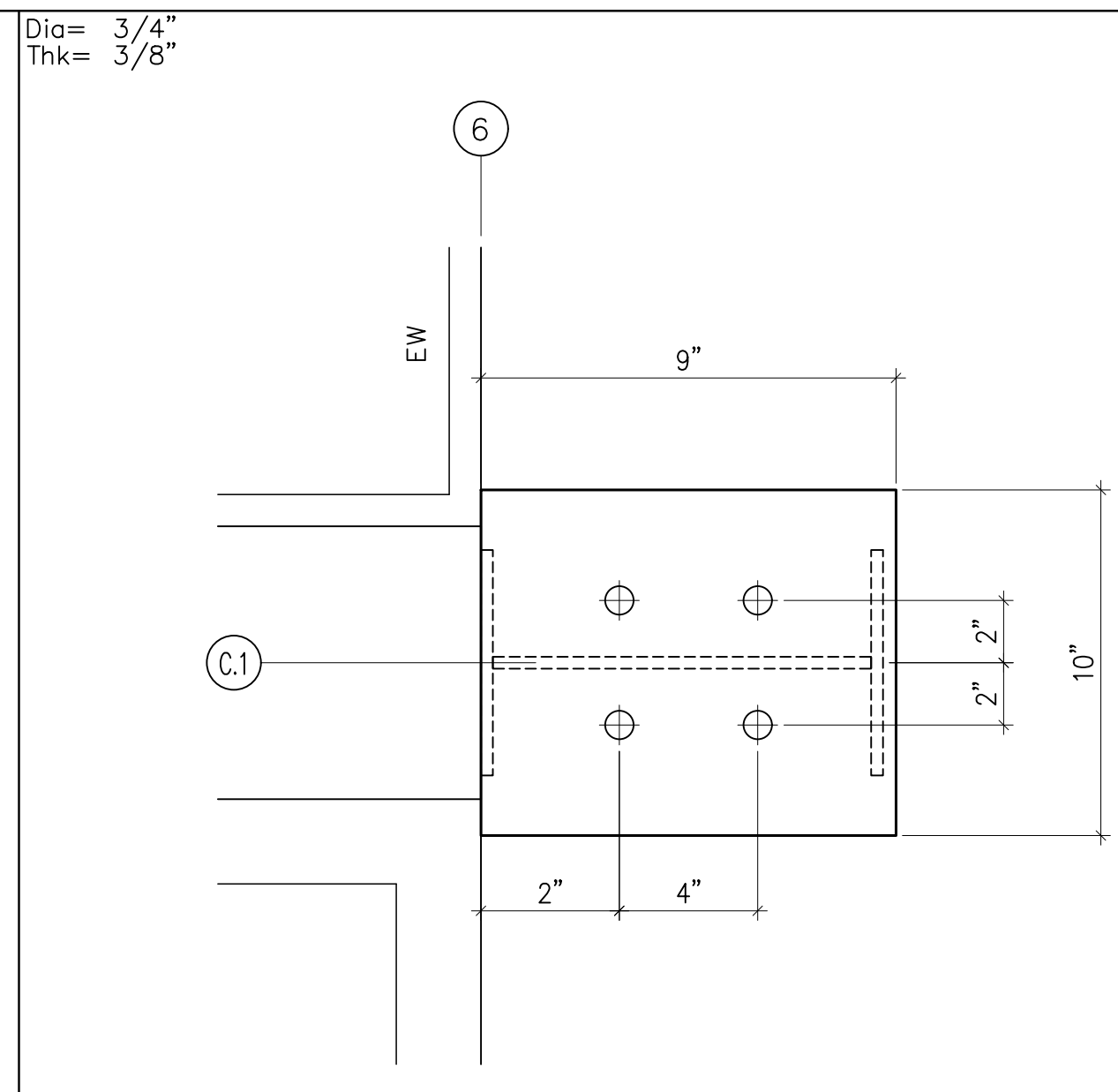
DESCRIPTION	COLUMN LAYOUT PLAN
CUSTOMER	CROSS DEVELOPMENT LLC
END USER	CALIBER COLLISION
END USE	COMMERCIAL
STREET	710 SE BLUE PKWY
CITY ST ZIP	LEES SUMMIT, MO 64002
SALES NO.	71872
JOB NO.	157337-38-39
SCALE	1/8"
DWG. NO.	F01 OF 5
ISSUE	0



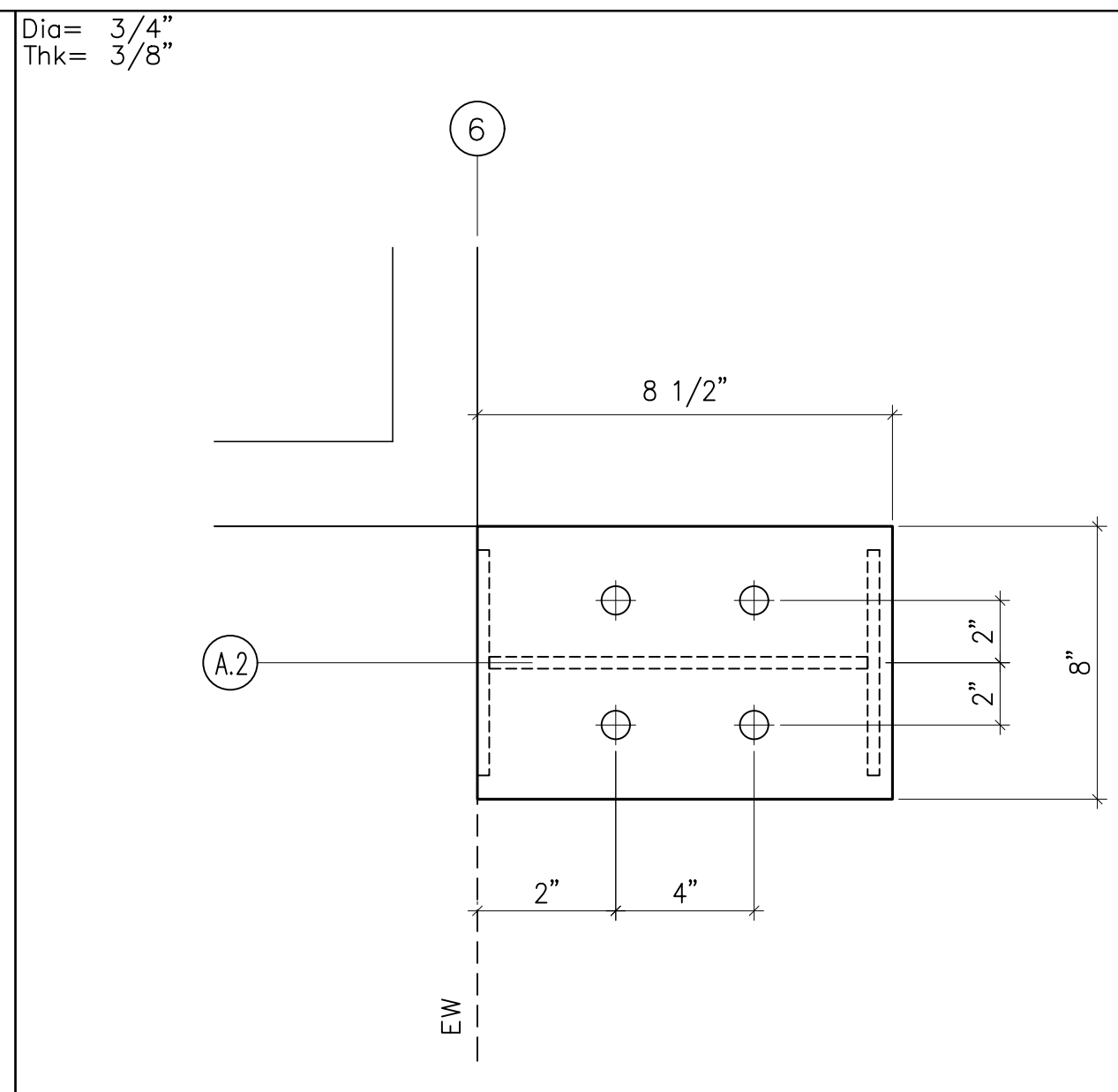
DETAIL A



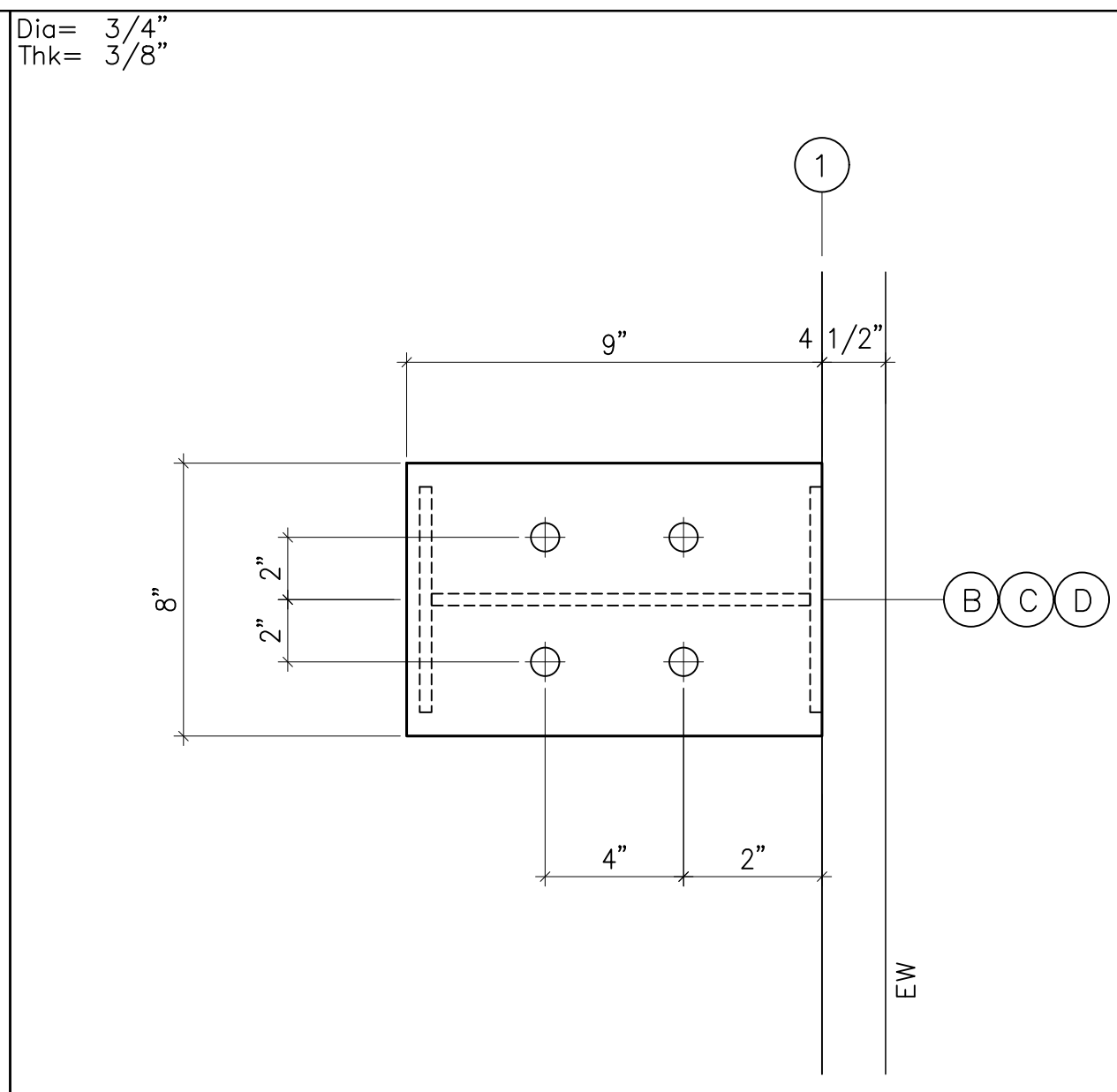
DETAIL B



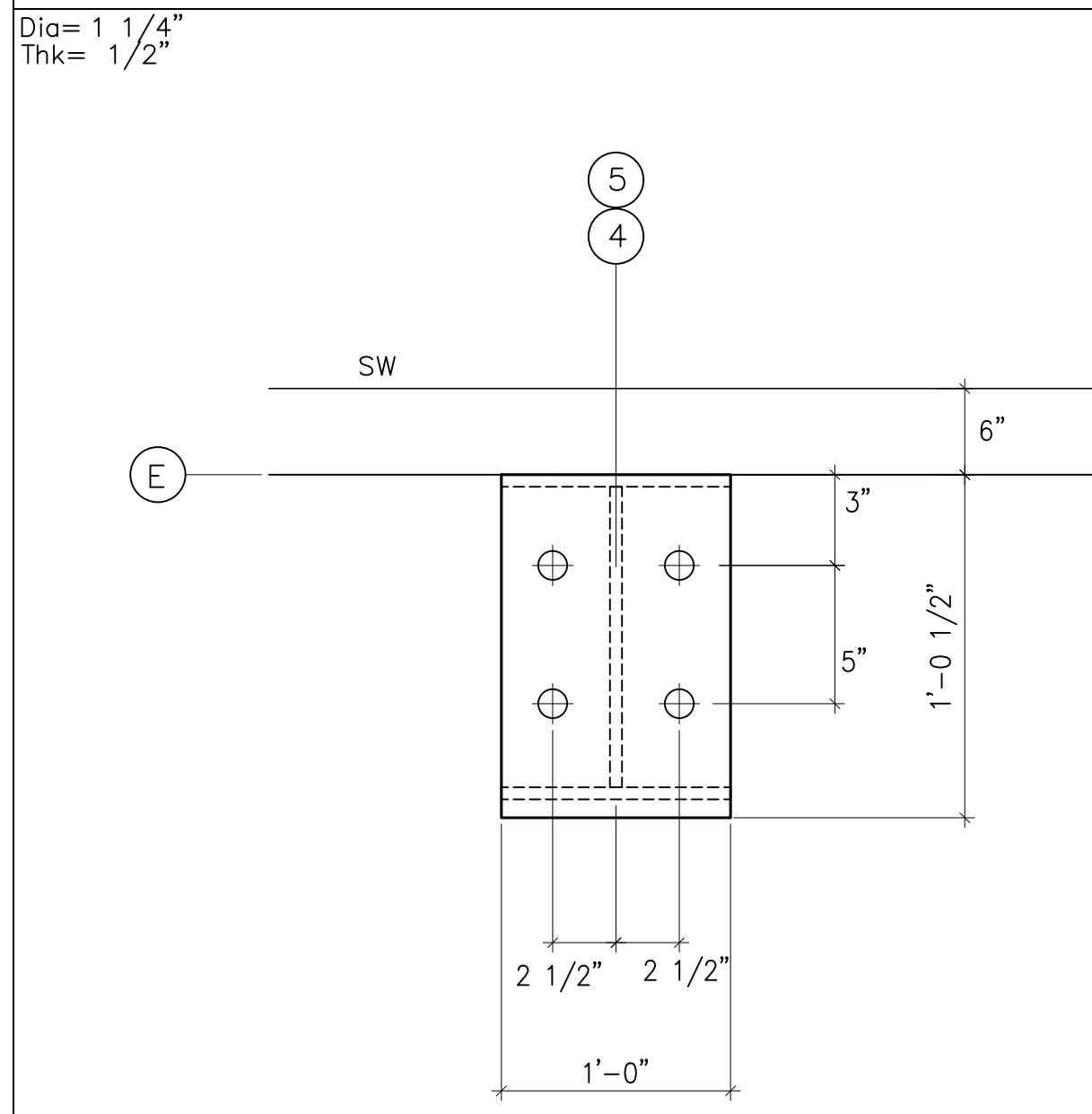
DETAIL B1



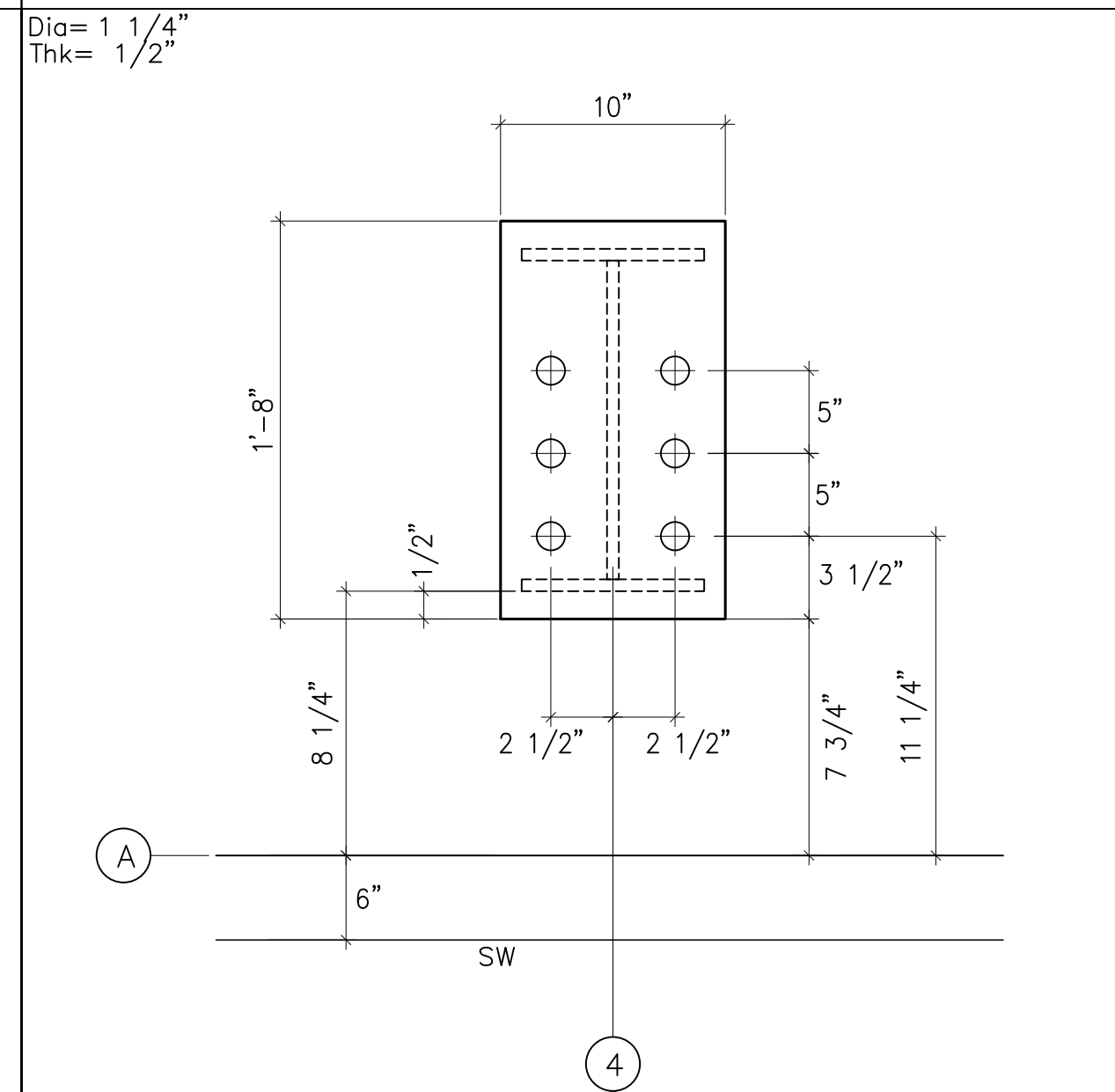
DETAIL B2



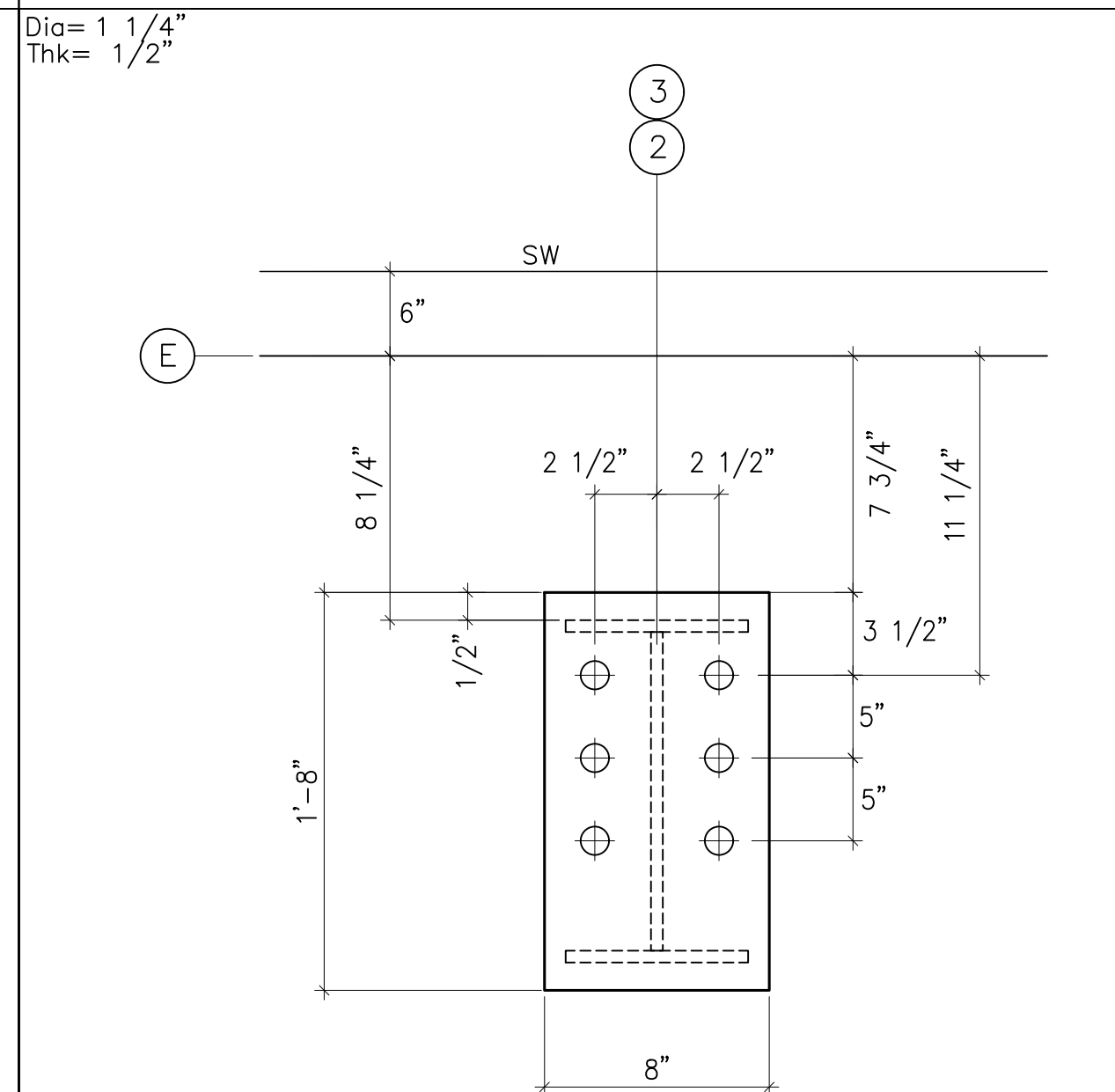
DETAIL C



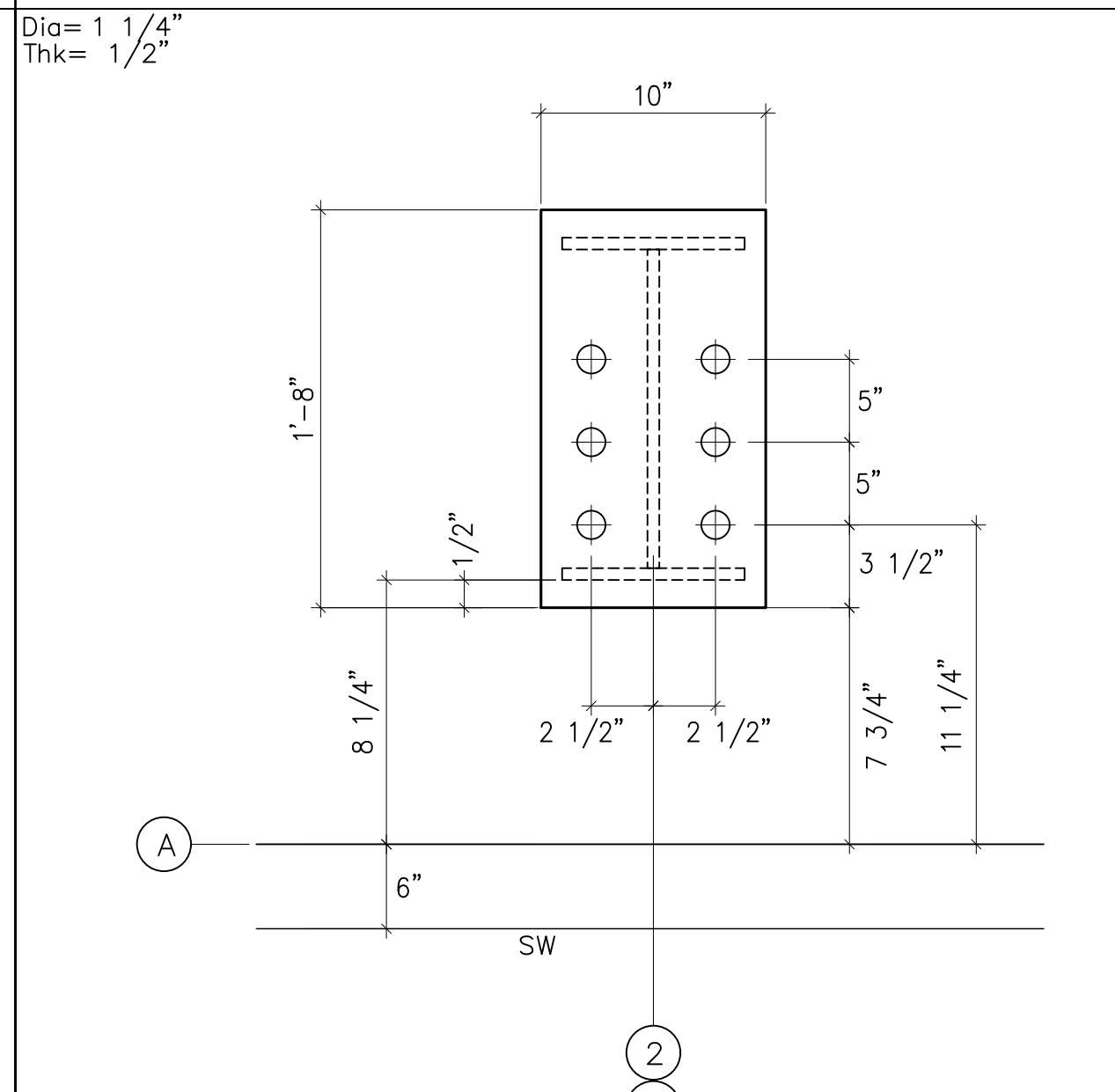
DETAIL D



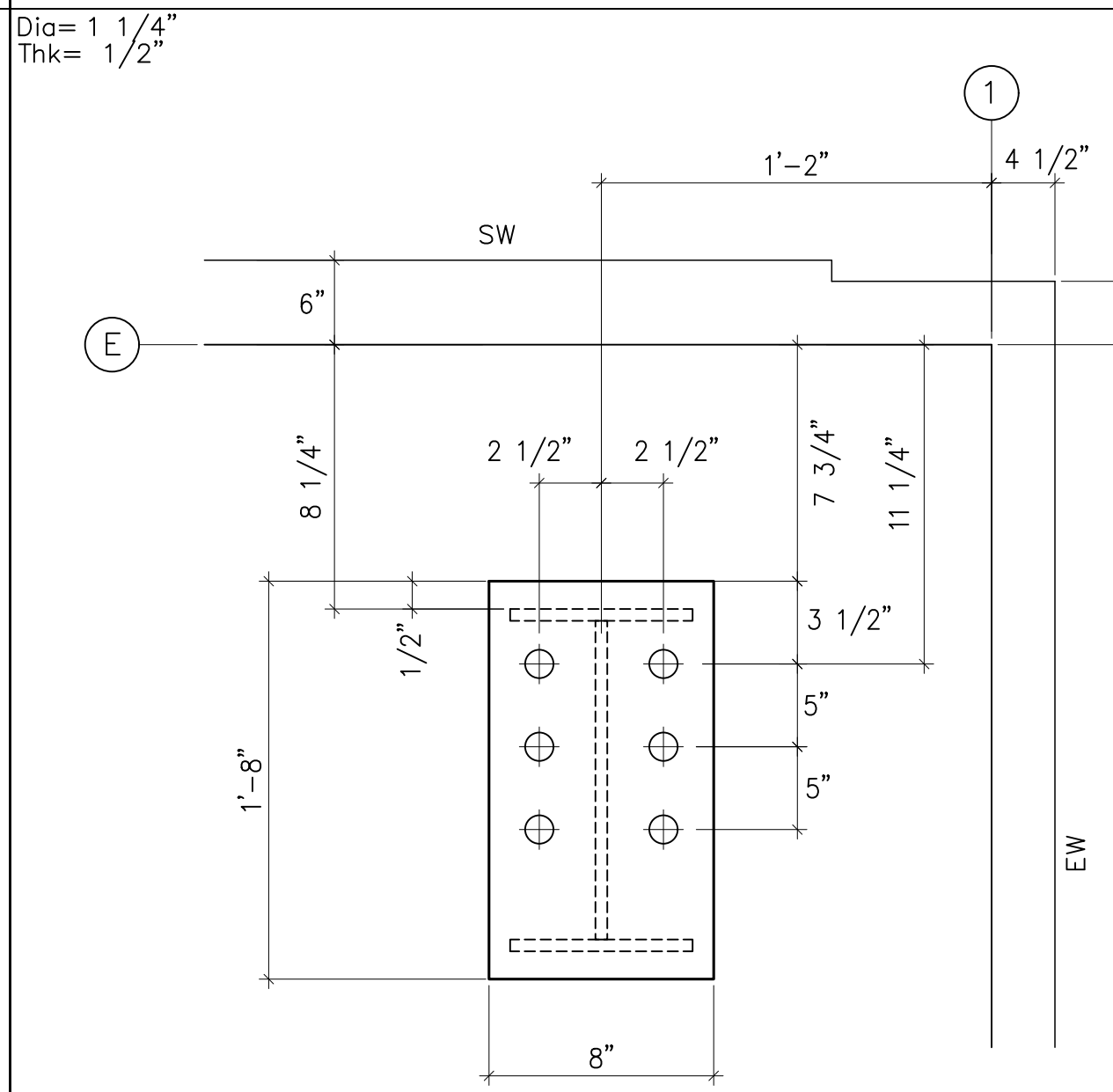
DETAIL D1



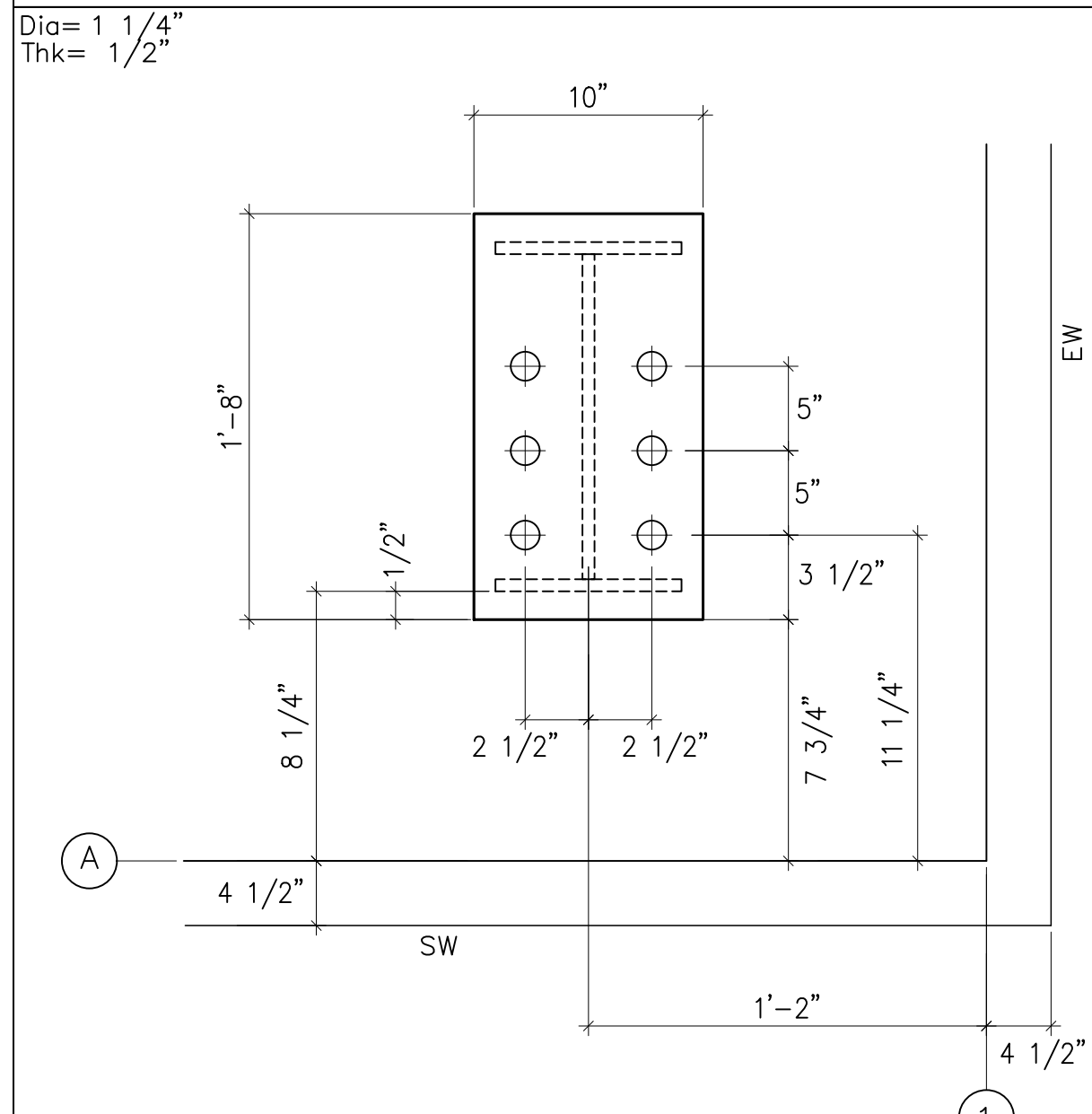
DETAIL E



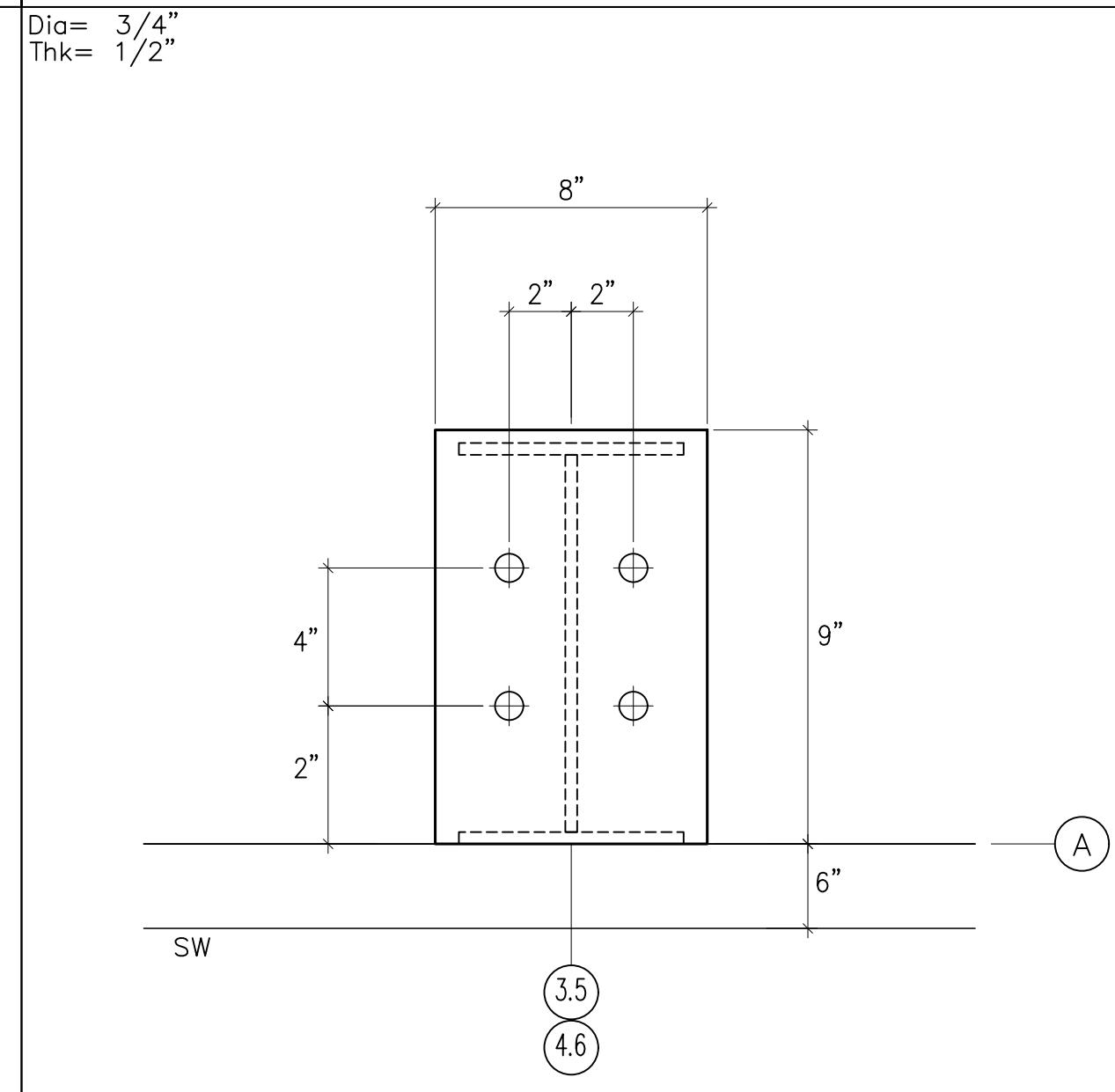
DETAIL E1



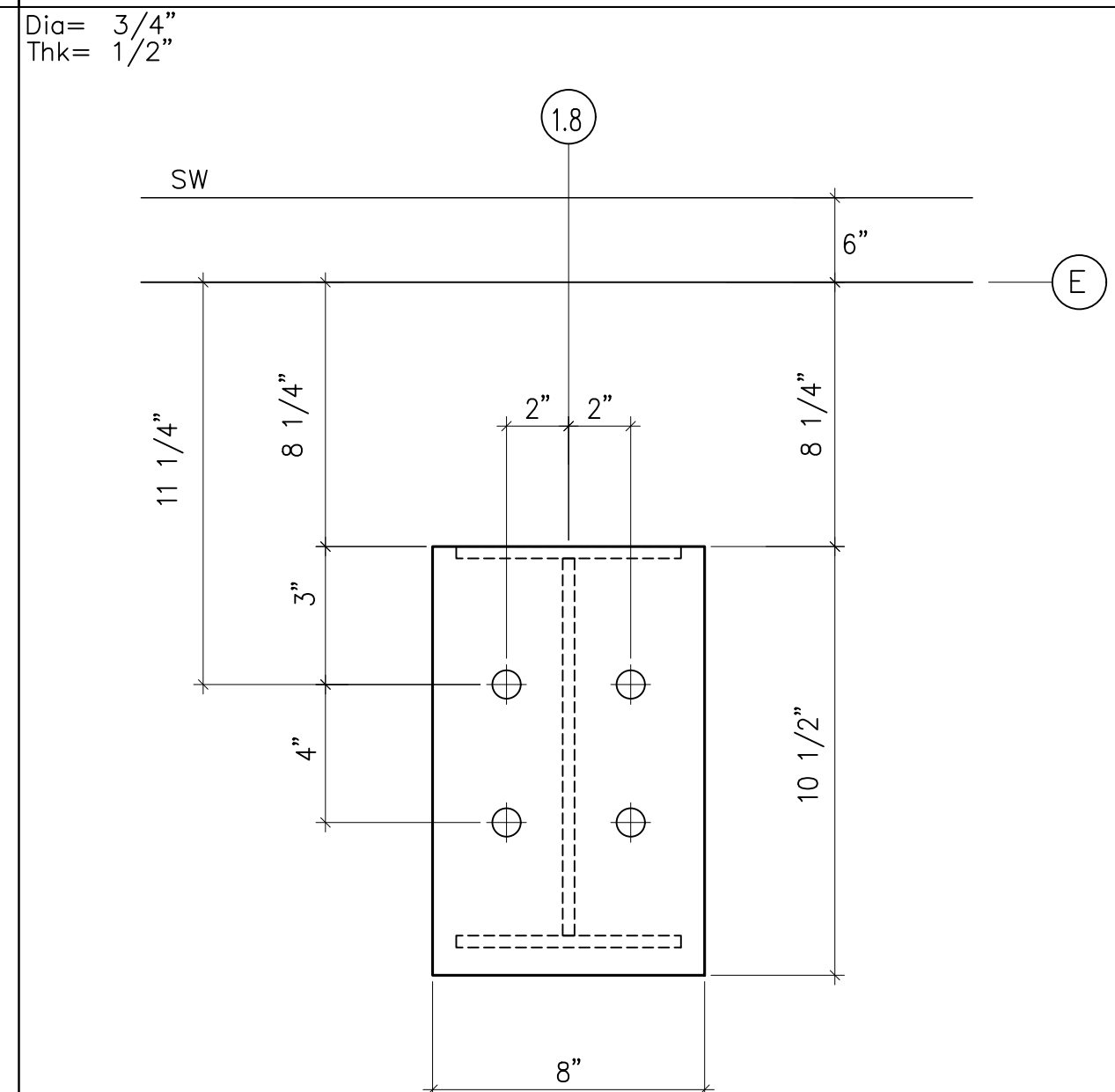
DETAIL F



DETAIL G



DETAIL H



DETAIL I

FOR  
CONSTRUCTION

**GENERAL NOTES:**  
 ① THE ANCHOR BOLT DETAILS SHOWN ON THIS DRAWING LOCATE THE ANCHOR BOLTS IN REFERENCE TO BOTH THE BUILDING STEEL LINE AND THE OUTSIDE OF RIGID'S SUGGESTED PANEL RECESS OF 1-1/2".  
 ② THE ANCHOR BOLT SETTING PLAN LOCATES ANCHOR BOLTS IN REFERENCE TO THE OUTSIDE OF THE PANEL RECESS SHOWN. IF THE ACTUAL PANEL RECESS IS DIFFERENT FROM WHAT IS SHOWN ON THE ANCHOR BOLT SETTING PLAN, THEN ALL REFERENCE DIMENSIONS FROM THE OUTSIDE OF THE PANEL RECESS MUST BE DETERMINED BY THE CUSTOMER.  
 ③ BOTTOM OF ALL BASE PLATES ARE AT THE SAME ELEVATION. (UNLESS NOTED)

**NOTE:**  
 ONLY ANCHOR BOLTS SETTING PLAN ISSUED & STAMPED "FOR CONSTRUCTION" SHALL BE USED IN SETTING ANCHOR BOLTS. "RIGID GLOBAL BUILDINGS" SHALL NOT BE RESPONSIBLE FOR ERROR OR DISCREPANCY IF THE DRAWING USED IS NOT VALID FOR CONSTRUCTION.

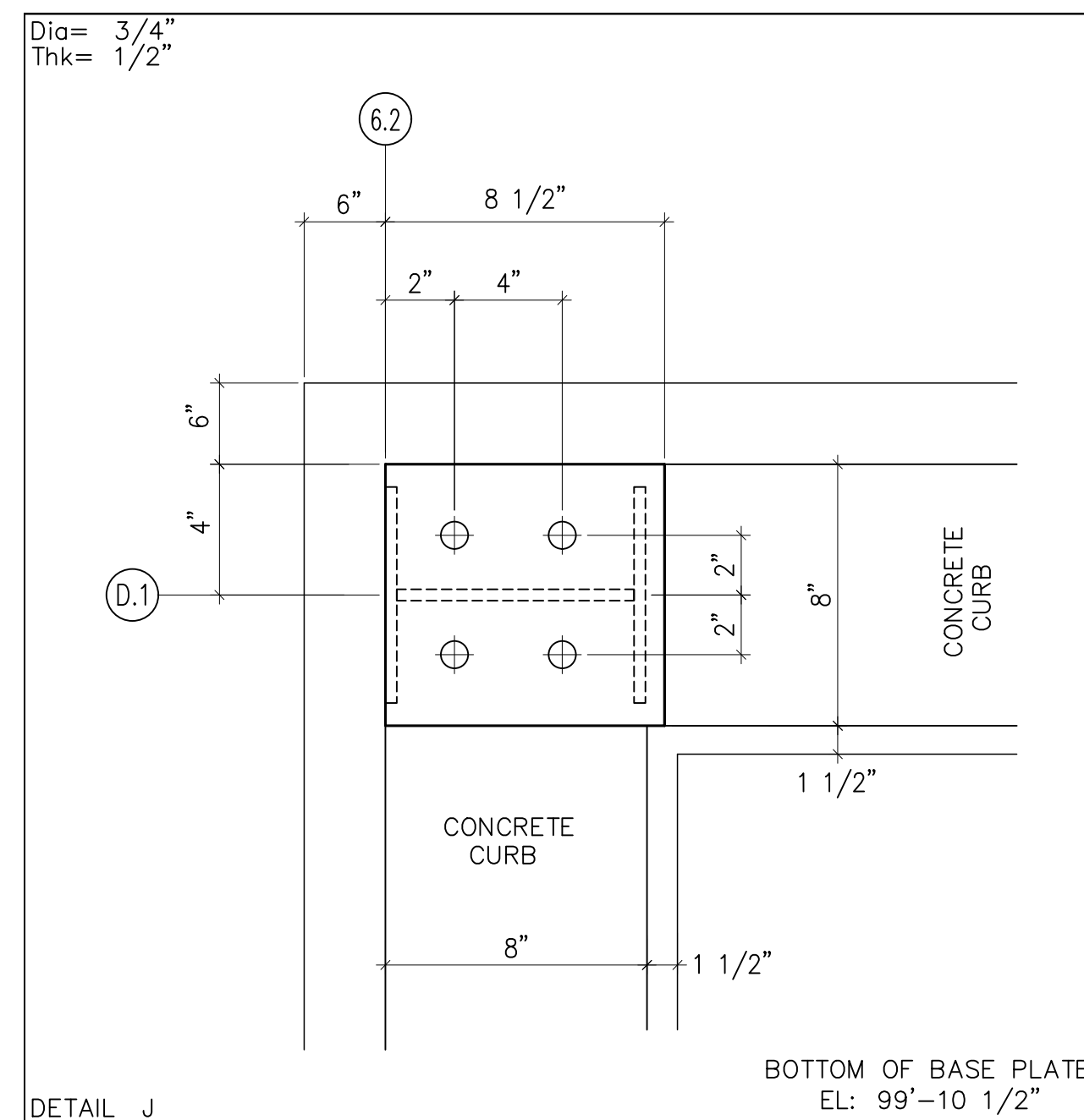
QTY.	SYMBOL	DIA.	PROJ.	ANCHOR BOLT DETAIL
-	+	1/2"	1"	
32	⊕	5/8"	2"	ANCHOR BOLT PROJECTION "PROJ." IS MEASURED FROM BOTTOM OF BASE PLATE
60	⊕	3/4"	2 1/2"	
-	⊕	7/8"	2 3/4"	
-	⊕	1"	3"	LENGTH OF "PROJ." SHOWN IS FOR ONE NUT + ONE WASHER
56	⊕	1 1/4"	3 1/2"	NUTS & WASHERS BY SUPPLIER
-	⊕	1 1/2"	3 1/2"	ANCHOR BOLTS NOT BY RIGID GLOBAL BUILDINGS

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0	CONSTRUCTION/PERMIT	01/13/22	RDA	RCR	JPL

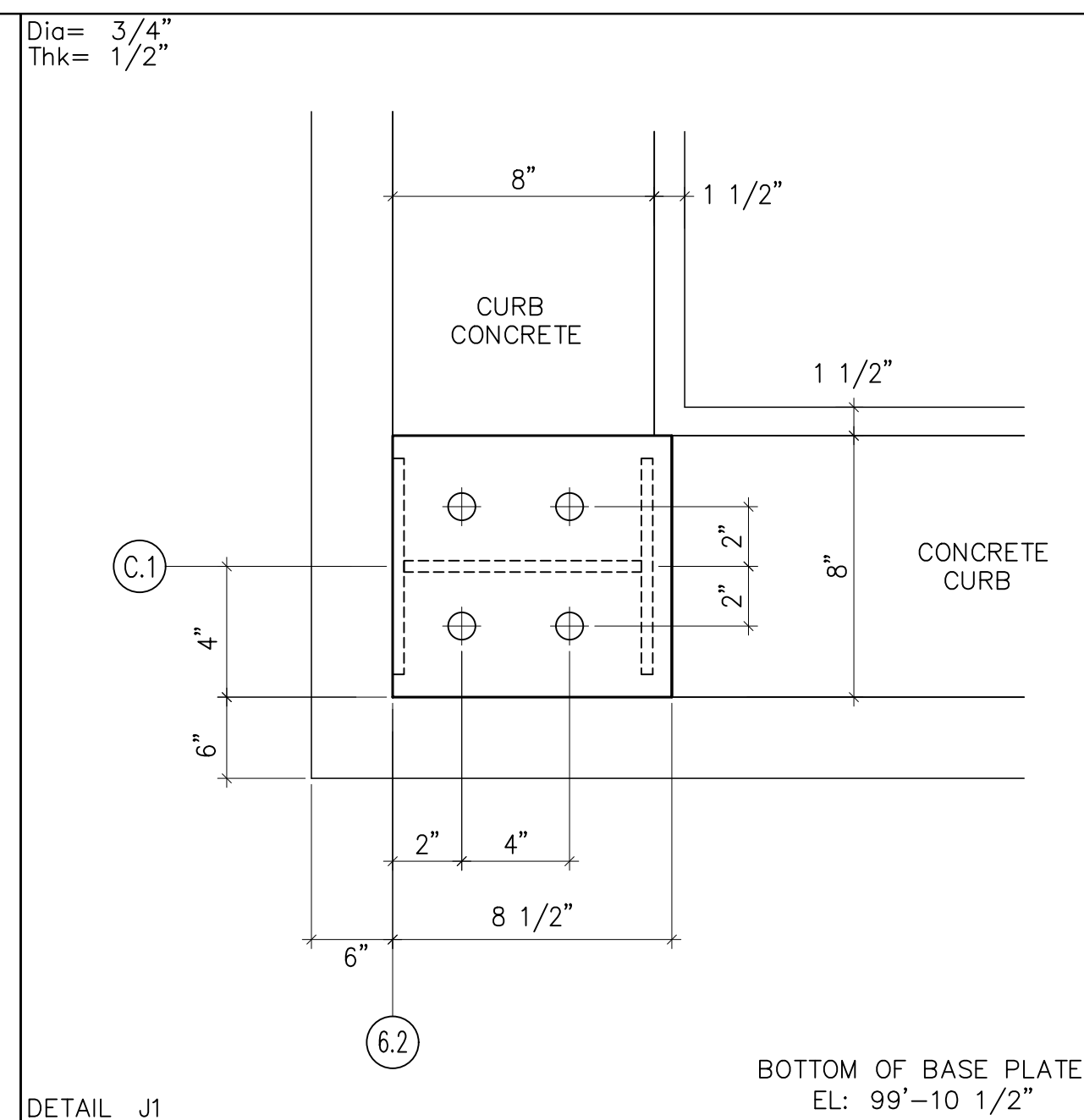


DESCRIPTION	ANCHOR BOLT DETAILS
CUSTOMER	CROSS DEVELOPMENT LLC
END USER	CALIBER COLLISION
END USE	COMMERCIAL BUILDING A
STREET	710 SE BLUE PKWY
CITY ST ZIP	LEES SUMMIT, MO 64002
SALES NO.	71872
JOB NO.	157337-38-39
SCALE	N.T.S.
DWG. NO.	F02 OF 5
ISSUE	0

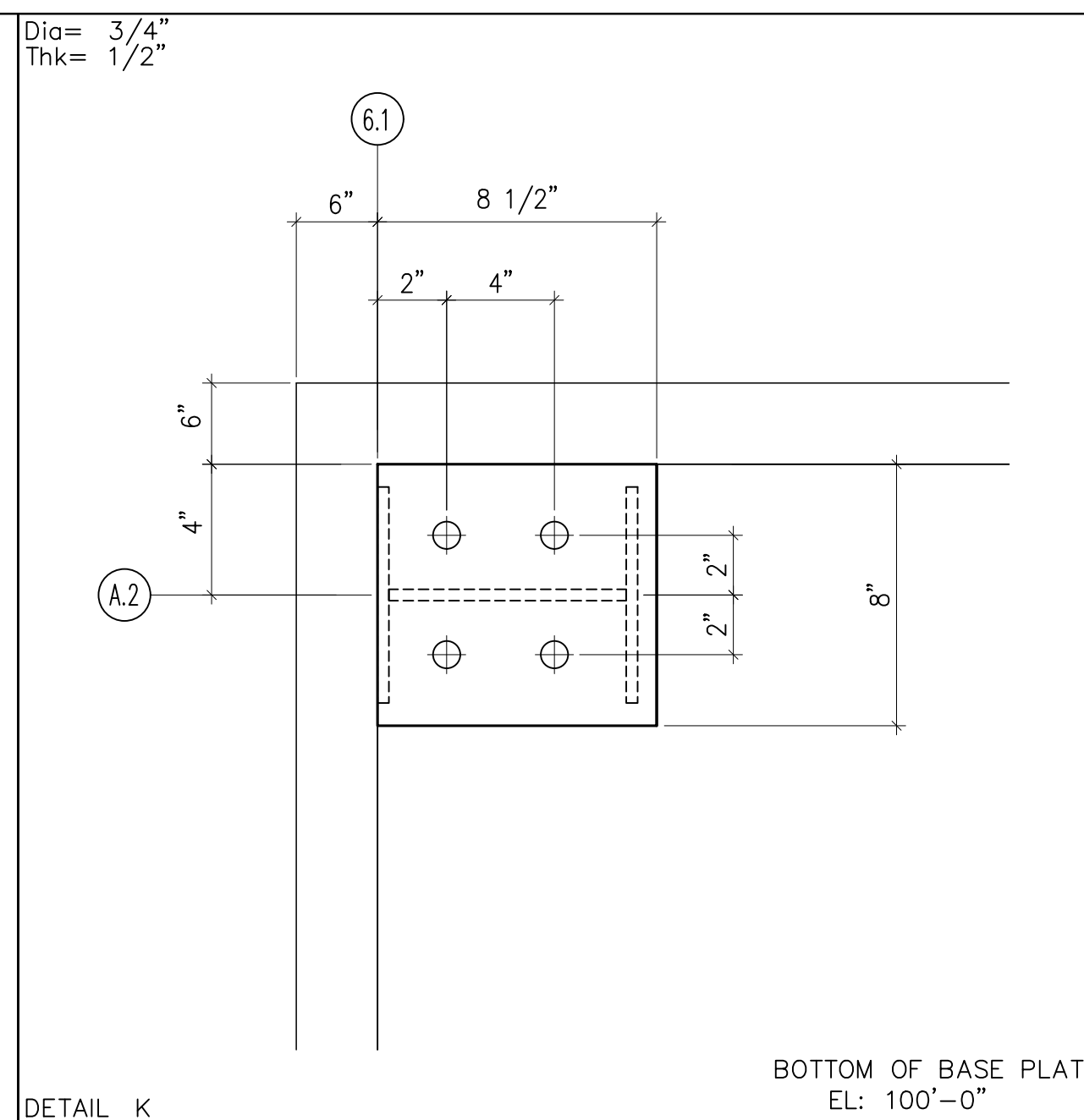
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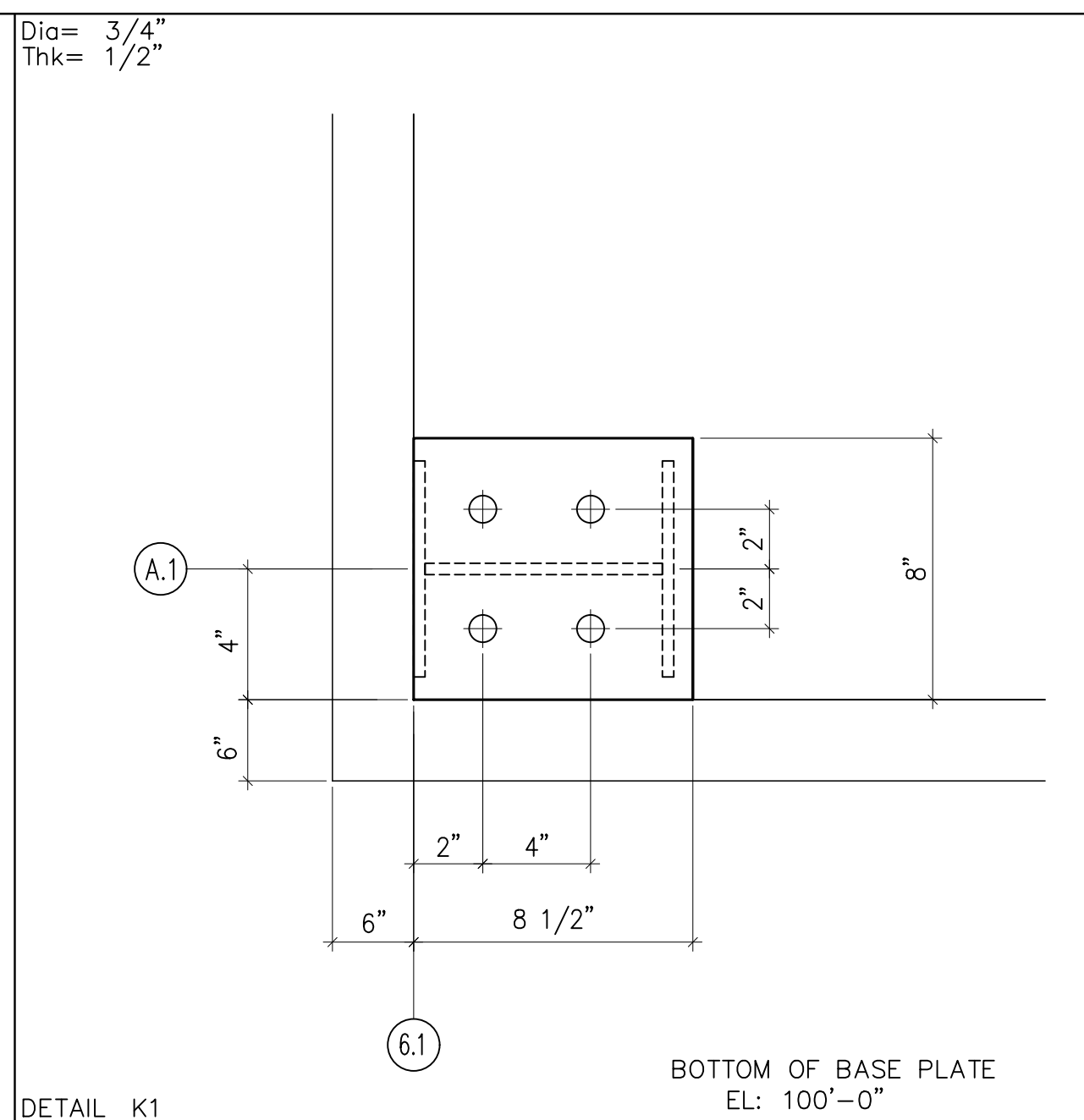
DETAIL J



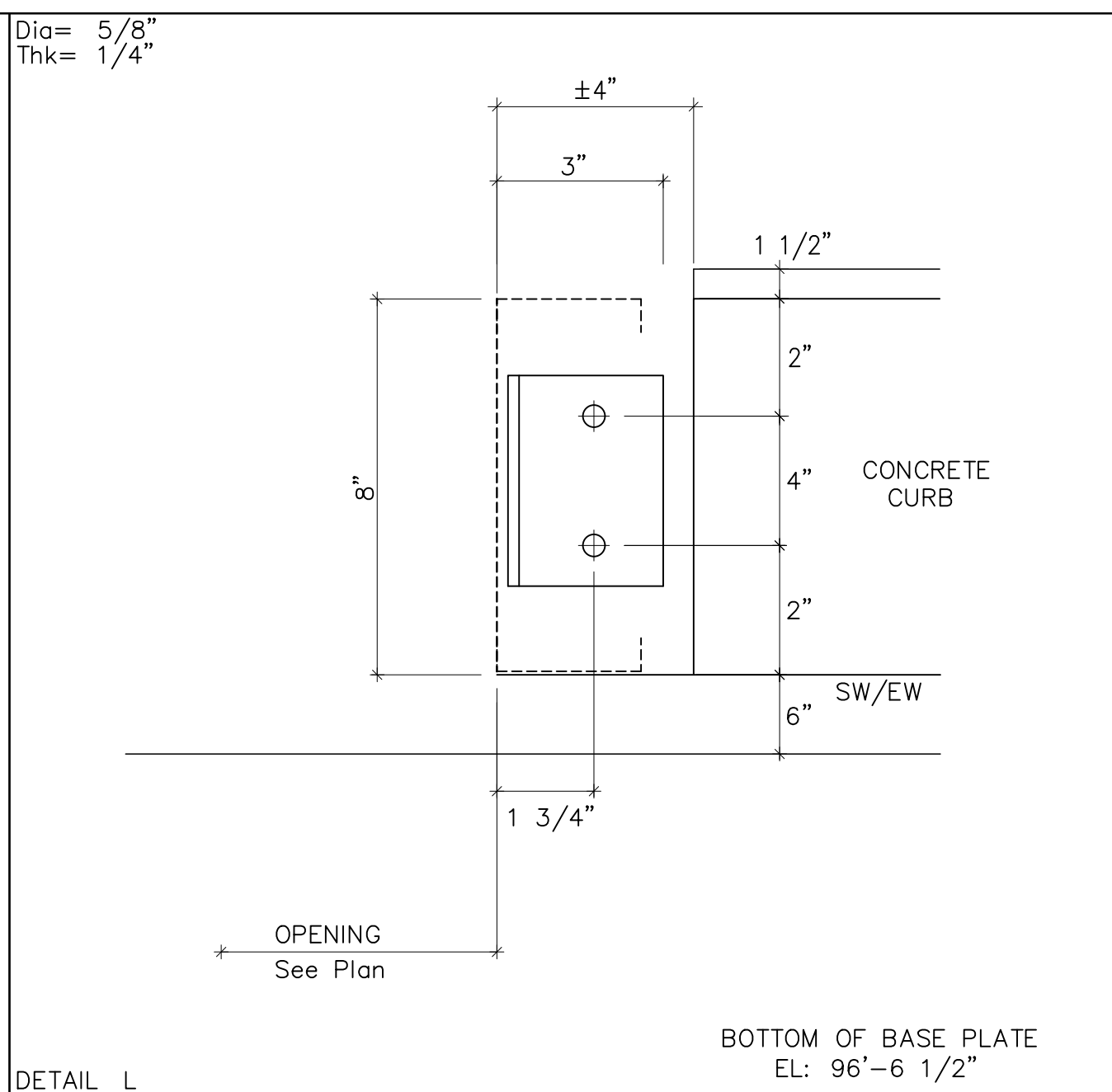
DETAIL J1



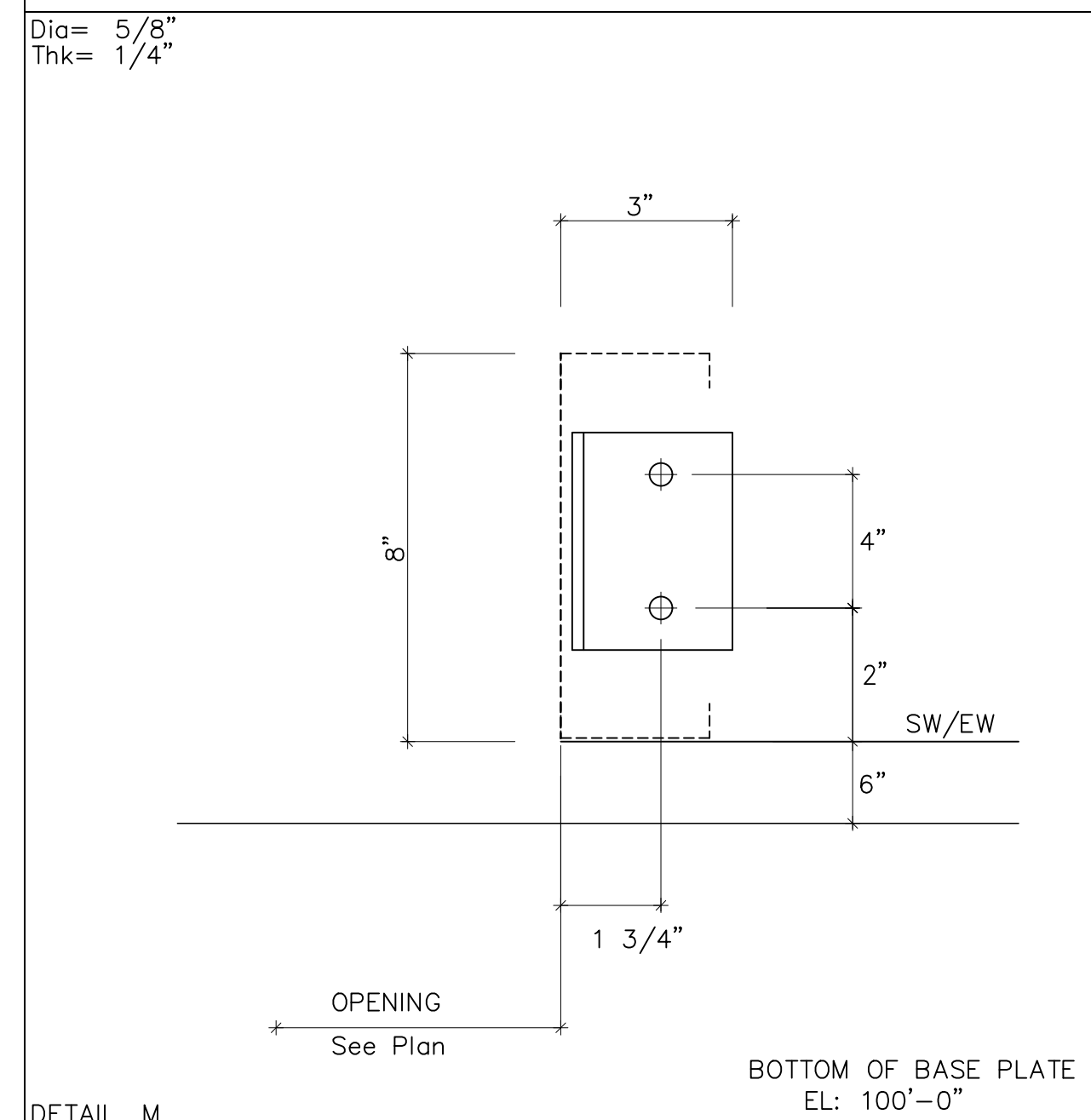
DETAIL K



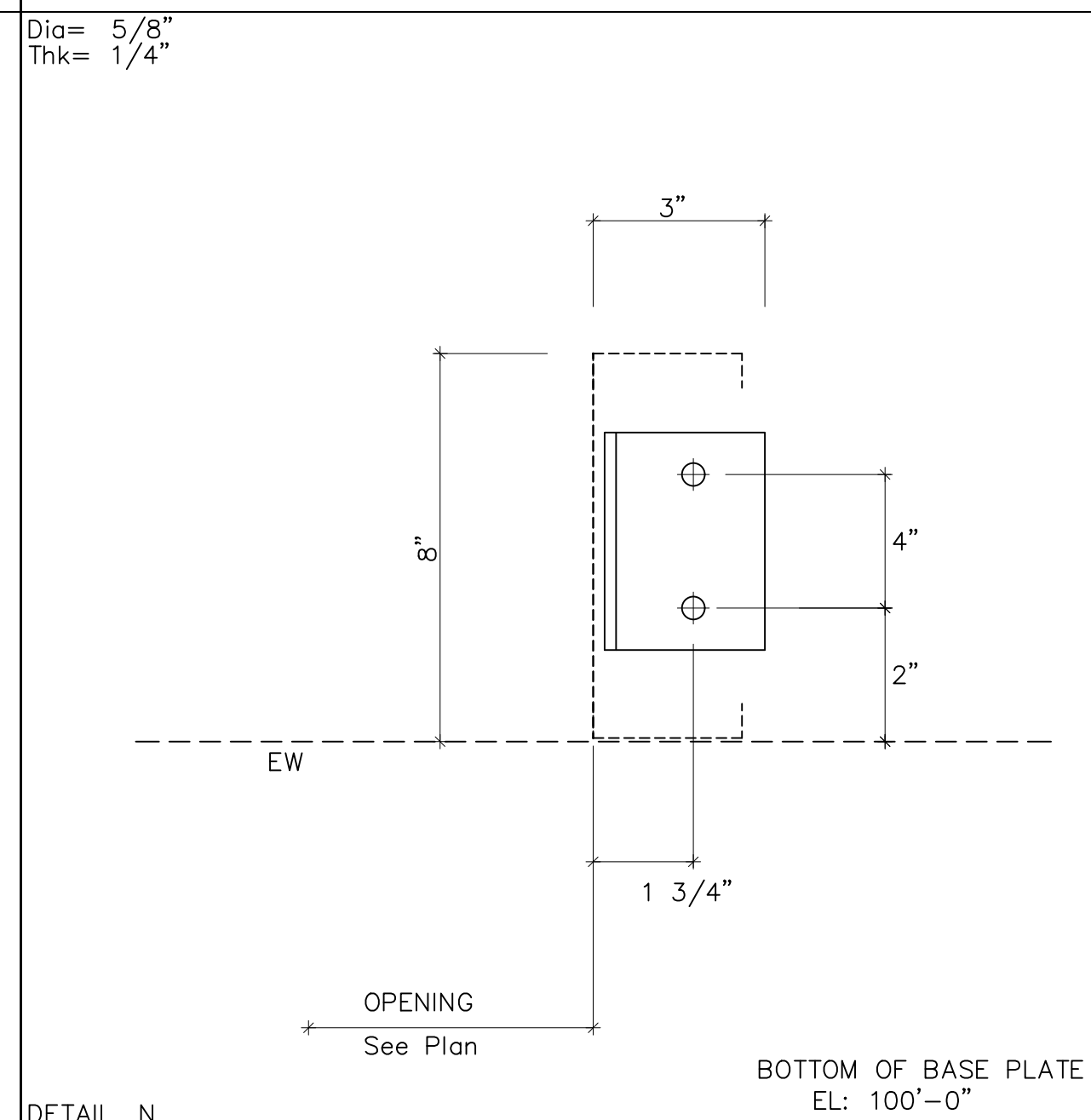
DETAIL K1



DETAIL L



DETAIL M



DETAIL N

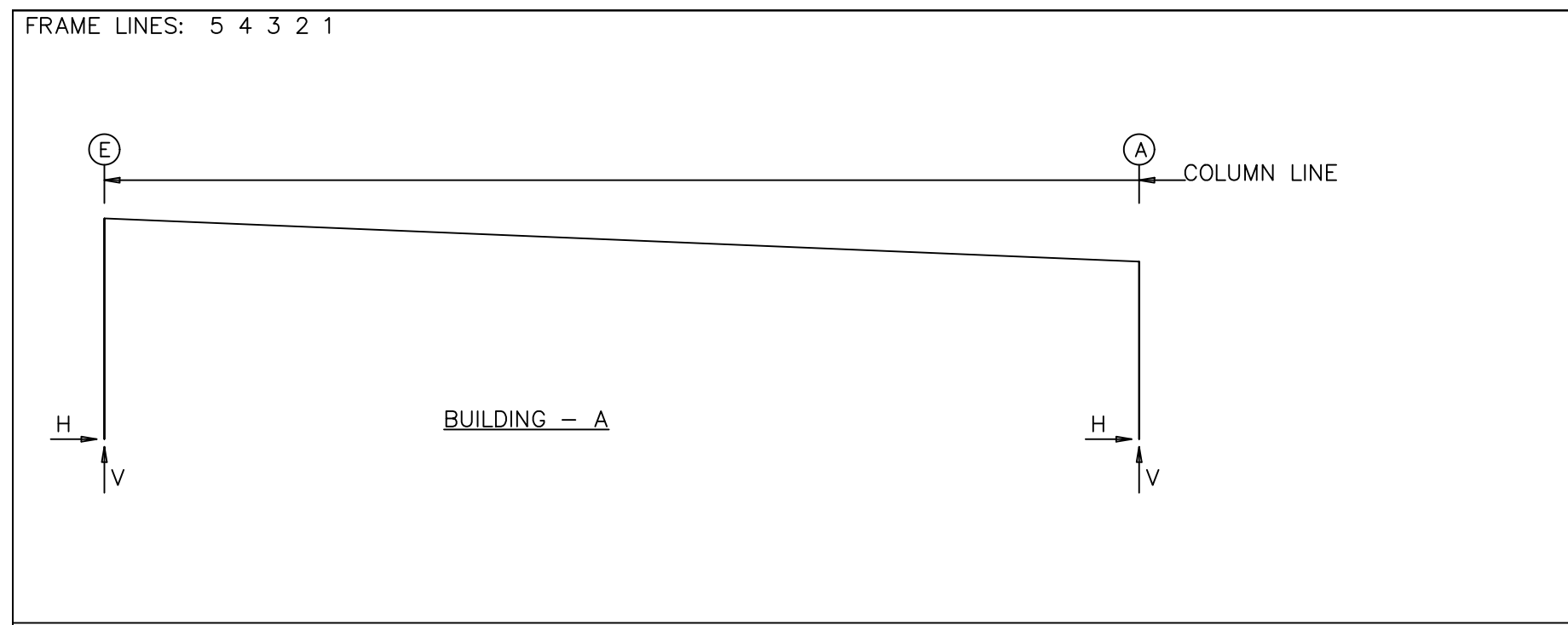
FOR  
CONSTRUCTION

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ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
0	CONSTRUCTION/PERMIT	01/13/22	RDA	RCR	JPL



DESCRIPTION	ANCHOR BOLT DETAILS
CUSTOMER	CROSS DEVELOPMENT LLC
END USER	CALIBER COLLISION
END USE	COMMERCIAL BUILDING A
STREET	710 SE BLUE PKWY
CITY ST ZIP	LEES SUMMIT, MO 64002
SALES NO:	71872
JOB NO:	157337-38-39
SCALE:	N.T.S.
DWG. NO.:	F03 OF 5
ISSUE:	0



RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Load Id	Hmax	V	Load Id	Hmin	V	Bolt Qty	Dia	Base_Plate(in)	Width	Length	Thick	BOTT. OF BPL (in)
5*	E	1	30.2	42.1	2	-9.6	-7.0	4	1.250	12.00	12.50	0.500	0.0	
5*	A	3	14.7	-12.3	1	-30.2	42.9	6	1.250	10.00	20.00	0.500	0.0	
5*	Frame lines: 5 4													

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Load Id	Hmax	V	Load Id	Hmin	V	Bolt Qty	Dia	Base_Plate(in)	Width	Length	Thick	BOTT. OF BPL (in)
3	E	1	32.2	40.3	2	-8.8	-7.4	6	1.250	8.000	20.00	0.500	0.0	
3	A	3	16.1	-11.9	1	-32.2	40.6	6	1.250	10.00	20.00	0.500	0.0	

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Load Id	Hmax	V	Load Id	Hmin	V	Bolt Qty	Dia	Base_Plate(in)	Width	Length	Thick	BOTT. OF BPL (in)
2	E	1	34.6	43.2	2	-9.2	-7.4	6	1.250	8.000	20.00	0.500	0.0	
2	A	3	16.0	-12.2	1	-34.5	43.5	6	1.250	10.00	20.00	0.500	0.0	

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Load Id	Hmax	V	Load Id	Hmin	V	Bolt Qty	Dia	Base_Plate(in)	Width	Length	Thick	BOTT. OF BPL (in)
1	E	1	28.9	35.0	2	-5.9	-4.4	6	1.250	8.000	20.00	0.500	0.0	
1	A	3	10.4	-8.3	1	-28.8	35.3	6	1.250	10.00	20.00	0.500	0.0	

RIGID FRAME: BASIC COLUMN REACTIONS (k )

Frame Line	Column Line	Dead	Collateral	Live	Snow	Snow_Drift	Wind_Left1	Wind_Right1	Wind_Left2	Wind_Right2	
5*	E	4.1	6.5	6.9	7.9	17.9	22.4	17.9	22.4	1.3	5.3
5*	A	-4.1	6.0	-6.9	8.6	-17.9	22.9	-17.9	22.9	-1.3	5.4

ENDWALL COLUMN: BASIC COLUMN REACTIONS (k )

Frm Line	Col Line	Dead	Collat	Live	Snow	Snow Drift	Wind_Left1	Wind_Right1	Wind_Left2	Wind_Right2
6	D.1	2.1	2.2	8.2	8.3	5.5	13.3	-10.6	0.0	-2.7
6	C.1	2.9	2.6	9.7	9.8	6.3	0.0	2.9	9.1	-13.9
6	A.2	1.8	1.8	6.3	6.3	5.1	0.0	-5.0	0.0	-2.9
6	A	1.0	0.8	3.0	3.0	3.9	0.0	-0.6	0.0	-4.1

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Load Id	Hmax	V	Load Id	Hmin	V	Bolt Qty	Dia	Base_Plate(in)	Width	Length	Thick	BOTT. OF BPL (in)
1	B	0.2	0.0	0.0	0.0	0.0	1.7	0.0	0.0	0.0	0.0	0.0	0.0	
1	C	8.5	-0.2	1.2	-0.4	2.4	2.0	1.0	0.5	-2.7				
1	D	8.5	-0.2	1.2	-0.4	2.4	2.7	1.0	1.4	-2.7				

SOLDIER COLUMN: BASIC COLUMN REACTIONS (k )

Frm Line	Col Line	Dead	Wind Press	Wind Suct	Wind Long1
4.6	A	0.4	5.5	-4.2	-1.2
3.5	A	0.4	5.5	-4.2	-1.2
1.8	E	0.2	3.5	-3.3	-1.7

ENDWALL COLUMN: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Load Id	Hmax	V	Load Id	Hmin	V	Bolt Qty	Dia	Base_Plate(in)	Width	Length	Thick	BOTT. OF BPL (in)
6	E	7	0.4	-1.3	8	-0.1	-1.1	4	0.750	8.000	8.500	0.375	0.0	
6	D.1	1	0.0	7.4	7	0.4	-1.3	4	0.750	8.000	8.500	0.375	0.0	
6	D.1	8	0.9	-1.7	7	0.3	-6.8	4	0.750	8.000	9.000	0.375	0.0	
6	C.1	1	0.0	18.0	1	0.0	18.0	4	0.750	10.00	9.000	0.375	0.0	
6	C.1	8	2.9	-1.5	9	-0.9	-8.3	4	0.750	10.00	9.000	0.375	0.0	
6	A.2	1	0.0	21.6	1	0.0	21.6	4	0.750	8.000	8.500	0.375	0.0	
6	A.2	8	2.8	-2.4	10	-1.1	-3.5	4	0.750	8.000	8.500	0.375	0.0	
6	A	3	0.0	-1.9	8	-9.9	-5.1	4	0.750	8.000	8.500	0.375	0.0	
1	B	14	1.0	0.1	2	0.2	0.2	4	0.750	8.000	9.000	0.375	0.0	
1	C	14	1.2	5.7	17	-0.5	10.9	4	0.750	8.000	9.000	0.375	0.0	
1	D	15	0.9	3.4	17	-0.4	10.9	4	0.750	8.000	9.000	0.375	0.0	

SOLDIER COLUMN: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Load Id	Hmax	V	Load Id	Hmin	V	Bolt Qty	Dia	Base_Plate(in)	Width	Length	Thick	BOTT. OF BPL (in)
4.6	A	12	3.3	0.2	13	-2.5	0.2	4	0.750	8.000	9.000	0.500	0.0	
3.5	A	6	0.0	0.4	13	-2.5	0.2	4	0.750	8.000	9.000	0.500	0.0	
1.8	E	12	2.1	0.1	13	-2.0	0.1	4	0.750	8.000	10.50	0.500	0.0	

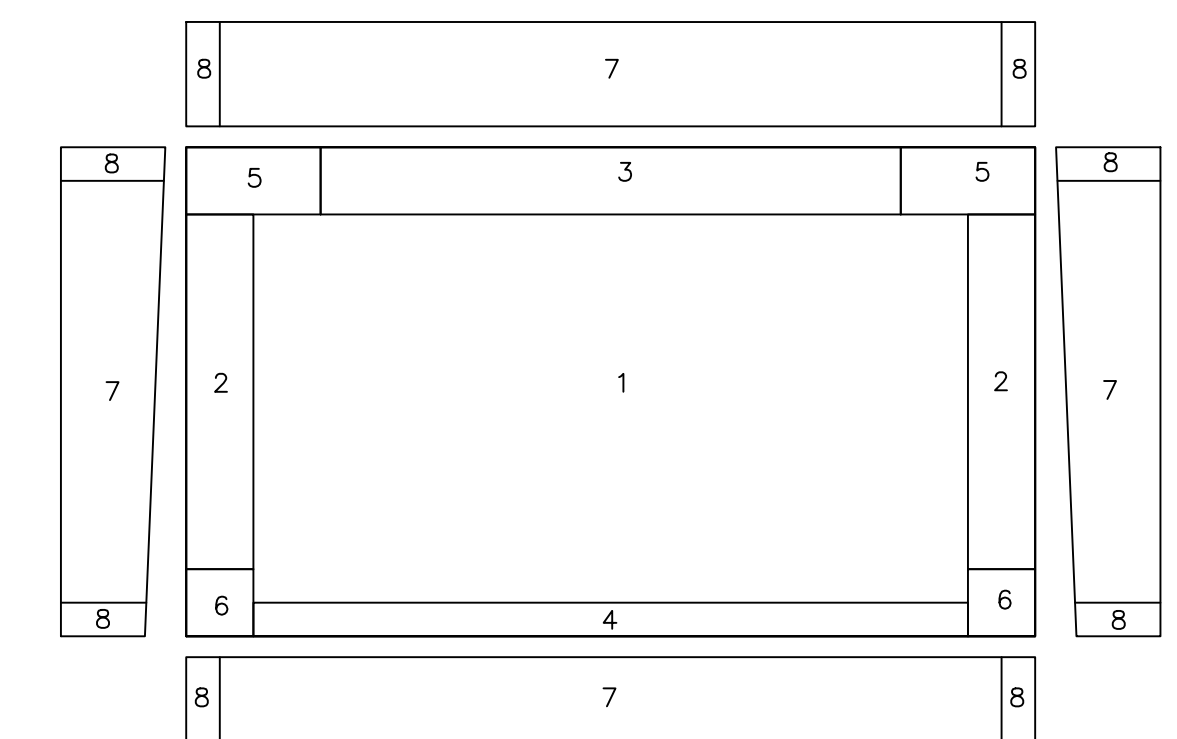
BUILDING BRACING REACTIONS

Loc	Wall Line	Col Line	± Wind	± Seismic	Panel Shear (lb/ft)	Wind	Seis	Note
L_EW	6	0.1,C.1	13.3	7.5	1.2	0.7		
F_SW	A	6.5	18.0	7.7	1.9	0.8		
R_EW	1	3.2	18.0	7.8	1.9	0.8		(h)
B_SW	E	4.5	31.9	18.4	3.8	2.2		

(h) Rigid frame at endwall

NOTES FOR REACTIONS

- All loading conditions are examined and only maximum/minimum H or V and the corresponding H or V are reported.
- Positive reactions are as shown in the sketch. Foundation loads are in opposite directions.
- Bracing reactions are in the plane of the brace with the H pointing away from the braced bay. The vertical reaction is downward.
- Building reactions are based on the following building data.
  - Width (ft) : 81.5
  - Length (ft) : 141.5
  - Eave Height (ft) : 17.4 / 14
  - Roof Slope (rise/12) : 0.5:12
  - Design Code : IBC 18
  - Enclosure : Closed
  - Dead Load (psf) : 3.000
  - Collateral Load (psf) : 6
  - Wind Speed (mph) : 109 mph
  - Wind Importance Factor : 1.00
  - Wind Exposure : C
  - Live Load (psf) : 20.00
  - Frame Live Load (psf) : 20
  - Ground Snow Load (psf) : 20.000
  - Roof Snow Load (psf) : 20
  - Snow Exposure : 1.000
  - Snow Importance Factor : 1.000
  - Thermal Factor : 1.000
  - Seismic Importance Factor : 1.00
  - Spectral Response Accel. : Ss=0.100 : S1=0.068
  - Spectral Response Coeff. : Sds=0.087 : Sd1=0.068
  - Seismic Coeff. (Fa\*Sa) : 0.130 : Fa=1.300
  - Seismic Design Category : B
- Loading conditions are:
  - 1 Dead+Collateral+Snow+Snow\_Drift
  - 2 0.6Dead+0.6Wind\_Left1
  - 3 0.6Dead+0.6Wind\_Right1
  - 4 0.6Dead+0.6Wind\_Long2L
  - 5 0.6Dead+0.6Wind\_Long2R
  - 6 Dead+Collateral+0.75MIN\_SNOW
  - 7 0.6Dead+0.6Wind\_Left1+0.6Wind\_Suction
  - 8 0.6Dead+0.6Wind\_Pressure+0.6Wind\_Long1L
  - 9 0.6Dead+0.6Wind\_Right1+0.6Wind\_Suction
  - 10 0.6Dead+0.6Wind\_Suction+0.6Wind\_Long1L
  - 11 Dead+Collateral+0.75Snow+0.45Wind\_Left2+0.45Wind\_Suction+0.75Snow\_Drift
  - 12 0.6Dead+0.6Wind\_Pressure
  - 13 0.6Dead+0.6Wind\_Suction
  - 14 0.6Dead+0.6Wind\_Pressure+0.6Wind\_Long2L
  - 15 0.6Dead+0.6Wind\_Right2+0.6Wind\_Suction
  - 16 Dead+0.6Wind\_Pressure+0.6Wind\_Long2L
  - 17 Dead+Collateral+Snow+1.0E2PAT\_SL\_2
  - 18 Dead+0.6Wind\_Right2+0.6Wind\_Suction
  - 19 Dead+0.6Wind\_Pressure



Design Calculation Wind

Zone	Width (ft)	Length (ft)	Components & Cladding (Factored)			
			Pressure(psf ) Member	Panel	Suction(psf ) Member	Panel
1			10.08	10.08	-18.73	-23.98
2			10.08	10.08	-24.91	-31.66
3	11.20	11.20	10.08	10.08	-24.91	-31.66
4	5.60		10.08	10.08	-24.91	-31.66
5	11.20	22.40	10.08	10.08	-29.60	-43.17
6	11.20	11.20	10.08	10.08	-29.60	-43.17
7			11.76	13.80	-12.90	-14.94
8	5.60		11.76	13.80	-14.32	-18.38

FOR CONSTRUCTION

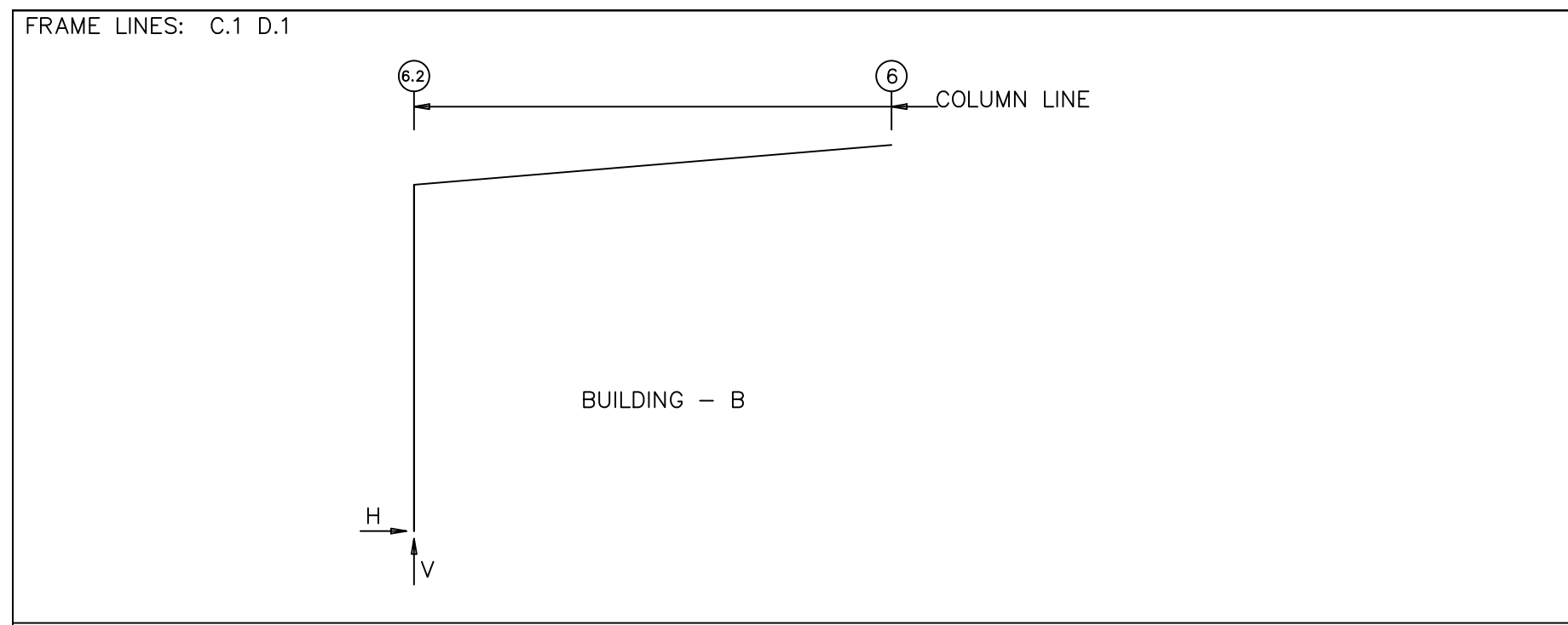
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ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
0	CONSTRUCTION/PERMIT	01/13/22	RDA	RCR	JPL

DESCRIPTION	REACTIONS
CUSTOMER	CROSS DEVELOPMENT LLC
END USER	CALIBER COLLISION
END USE	COMMERCIAL
STREET	710 SE BLUE PKWY
CITY ST ZIP	LEES SUMMIT, MO 64002
SALES NO.	71872
DWG. NO.	157337-38-39
SCALE	N.T.S.
DWG. NO.	F04 OF 5
ISSUE	0



18933 Aldine Westfield  
Houston, Tx 77075  
Phone : (281) 443-9065  
Fax : (281) 443-9064



RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column Reactions(k)						Bolt Qty	Dia	Base Plate(in)			BOTT. OF BPL (in)
		Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin			Width	Length	Thick	
C.1*	6.2	4	0.6	-1.4	3	-0.7	-0.7	4	0.750	8.000	8.500	0.500	-1.50
		1	0.1	3.5	4	0.6	-1.4						

C.1\* Frame lines: C.1 D.1

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	Dead		Collateral		Live		Snow		Snow Drift		Wind Left	
		Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
C.1*	6.2	0.0	0.6	0.0	0.4	0.1	2.5	0.1	2.5	0.0	0.0	-0.6	-2.7

Frame Line	Column Line	Wind Right		Wind Left		Wind Right		Wind Long		Wind Long		MIN SNOW	
		Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
C.1*	6.2	0.9	-1.8	-1.2	-1.7	0.3	-0.7	1.0	-2.9	1.0	-2.9	0.1	2.5

C.1\* Frame lines: C.1 D.1

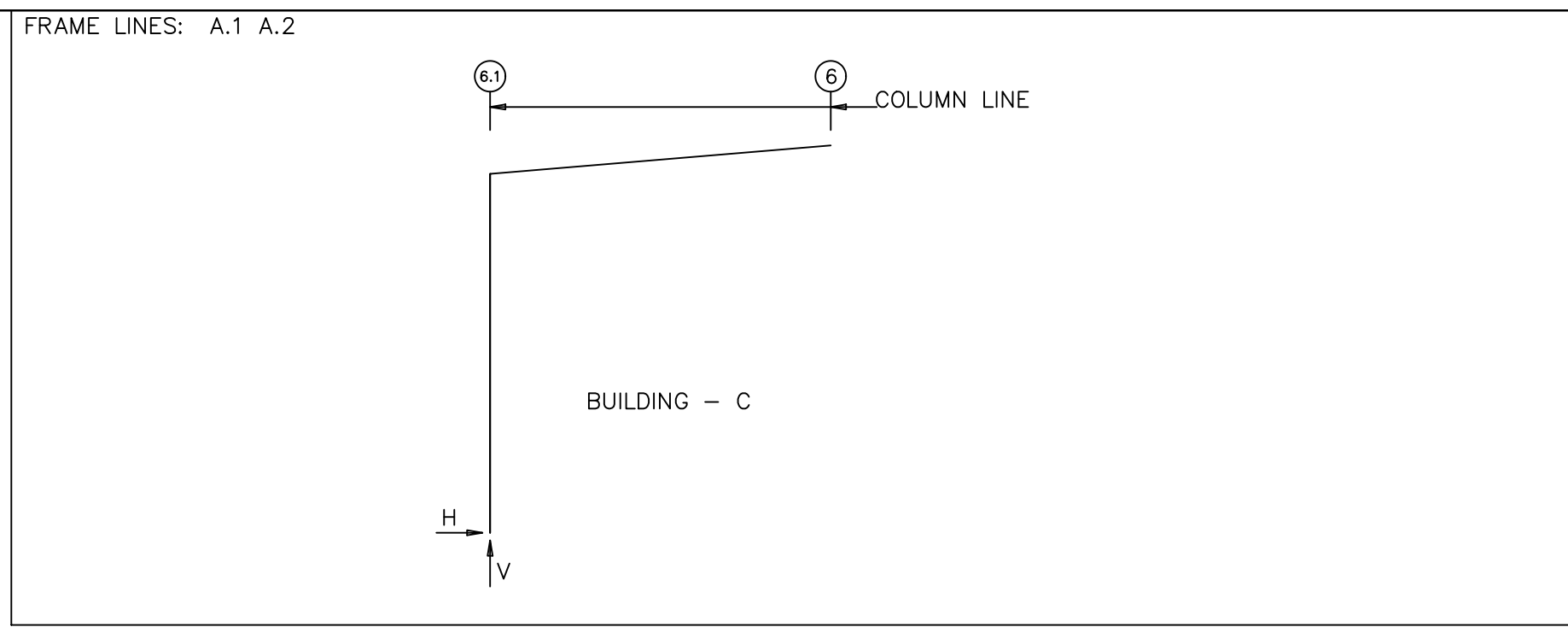
BUILDING BRACING REACTIONS

Wall Loc	Col Line	Reactions(k)				Panel Shear (lb/ft)		Note
		Wind Horz	Wind Vert	Seismic Horz	Seismic Vert	Wind	Seis	
L_EW	C.1							(h)
F_SW	6							(f)
R_EW	D.1							(h)
B_SW	6.2	Torsional Bracing Used						

(f) Bracing loads are applied to adjacent building  
(h) Rigid frame at endwall

NOTES FOR REACTIONS

- All loading conditions are examined and only maximum/minimum H or V and the corresponding H or V are reported.
- Positive reactions are as shown in the sketch. Foundation loads are in opposite directions.
- Bracing reactions are in the plane of the brace with the H pointing away from the braced bay. The vertical reaction is downward.
- Building reactions are based on the following building data.
  - Width (ft) : 18.08
  - Length (ft) : 27.33
  - Eave Height (ft) : 13.13 / 14.63
  - Roof Slope (rise/12) : 1.0/12
  - Design Code : IBC 18
  - Enclosure : Closed
  - Dead Load (psf) : 3.000
  - Collateral Load (psf) : 3
  - Wind Speed (mph) : 109 mph
  - Wind Importance Factor : 1.00
  - Wind Exposure : C
  - Live Load (psf) : 20.00
  - Frame Live Load (psf) : 20
  - Ground Snow Load (psf) : 20.000
  - Roof Snow Load (psf) : 20
  - Snow Exposure : 1.000
  - Snow Importance Factor : 1.000
  - Thermal Factor : 1.000
  - Seismic Importance Factor : 1.00
  - Spectral Response Accel. : Ss=0.100 : S1=0.068
  - Spectral Response Coeff. : Sds=0.087 : Sd1=0.068
  - Seismic Coeff. (Fa\*Ss) : 0.130 : Fa=1.300
  - Seismic Design Category : B
- Loading conditions are:
  - 1 Dead+Collateral+Snow+Snow Drift
  - 2 0.6Dead+0.6Wind\_Left1
  - 3 0.6Dead+0.6Wind\_Left2
  - 4 0.6Dead+0.6Wind\_Long2R
  - 5 0.6Dead+0.6Wind\_Right2+0.6Wind\_Suction
  - 6 0.6Dead+0.6Wind\_Pressure+0.6Wind\_Long2L



RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column Reactions(k)						Bolt Qty	Dia	Base Plate(in)			BOTT. OF BPL (in)
		Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin			Width	Length	Thick	
A.1*	6.1	4	0.3	-0.4	3	-0.3	-0.2	4	0.750	8.000	8.500	0.500	0.00
		1	0.0	1.2	4	0.3	-0.4						

A.1\* Frame lines: A.1 A.2

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	Dead		Collateral		Live		Snow		Snow Drift		Wind Left	
		Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
A.1*	6.1	0.0	0.3	0.0	0.1	0.0	0.8	0.0	0.8	0.0	0.0	-0.3	-0.9

Frame Line	Column Line	Wind Right		Wind Left		Wind Right		Wind Long		Wind Long		MIN SNOW	
		Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
A.1*	6.1	0.4	-0.6	-0.6	-0.6	0.1	-0.3	0.5	-1.0	0.5	-1.0	0.0	0.8

A.1\* Frame lines: A.1 A.2

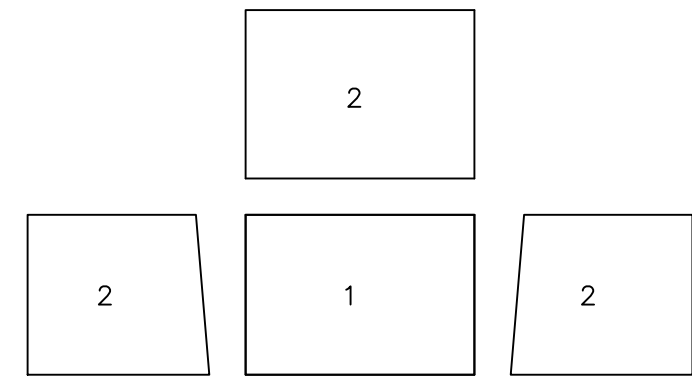
BUILDING BRACING REACTIONS

Wall Loc	Col Line	Reactions(k)				Panel Shear (lb/ft)		Note
		Wind Horz	Wind Vert	Seismic Horz	Seismic Vert	Wind	Seis	
L_EW	A.1							(h)
F_SW	6							(f)
R_EW	A.2							(h)
B_SW	6.1	Torsional Bracing Used						

(f) Bracing loads are applied to adjacent building  
(h) Rigid frame at endwall

NOTES FOR REACTIONS

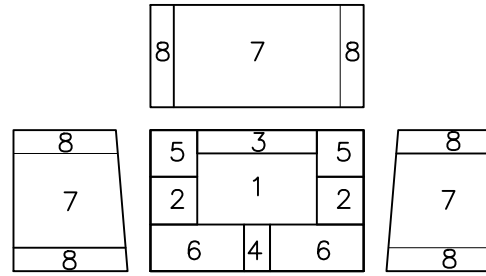
- All loading conditions are examined and only maximum/minimum H or V and the corresponding H or V are reported.
- Positive reactions are as shown in the sketch. Foundation loads are in opposite directions.
- Bracing reactions are in the plane of the brace with the H pointing away from the braced bay. The vertical reaction is downward.
- Building reactions are based on the following building data.
  - Width (ft) : 10.25
  - Length (ft) : 14.67
  - Eave Height (ft) : 10.79 / 11.65
  - Roof Slope (rise/12) : 1.0/12
  - Design Code : IBC 18
  - Enclosure : Closed
  - Dead Load (psf) : 3.000
  - Collateral Load (psf) : 3
  - Wind Speed (mph) : 109 mph
  - Wind Importance Factor : 1.00
  - Wind Exposure : C
  - Live Load (psf) : 20.00
  - Frame Live Load (psf) : 20
  - Ground Snow Load (psf) : 20.000
  - Roof Snow Load (psf) : 20
  - Snow Exposure : 1.000
  - Snow Importance Factor : 1.000
  - Thermal Factor : 1.000
  - Seismic Importance Factor : 1.00
  - Spectral Response Accel. : Ss=0.100 : S1=0.068
  - Spectral Response Coeff. : Sds=0.087 : Sd1=0.068
  - Seismic Coeff. (Fa\*Ss) : 0.130 : Fa=1.300
  - Seismic Design Category : B
- Loading conditions are:
  - 1 Dead+Collateral+Snow+Snow Drift
  - 2 0.6Dead+0.6Wind\_Left1
  - 3 0.6Dead+0.6Wind\_Left2
  - 4 0.6Dead+0.6Wind\_Long2R
  - 5 0.6Dead+0.6Wind\_Right2+0.6Wind\_Suction
  - 6 0.6Dead+0.6Wind\_Pressure+0.6Wind\_Long2L



Design Calculation Wind

Components & Cladding (Factored)

Zone	Width (ft)	Length (ft)	Pressure (psf)		Suction (psf)	
			Member	Panel	Member	Panel
1			10.08	10.08	-22.70	-35.44
2			11.76	13.80	-14.34	-18.42



Design Calculation Wind

Components & Cladding (Factored)

Zone	Width (ft)	Length (ft)	Pressure (psf)		Suction (psf)	
			Member	Panel	Member	Panel
1			10.08	10.08	-16.33	-16.33
2			10.08	10.08	-21.39	-22.70
3	3.00	6.00	10.08	10.08	-17.64	-18.95
4	6.00		10.08	10.08	-21.39	-22.70
5	6.00	6.00	10.08	10.08	-17.64	-25.31
6	6.00	12.00	10.08	10.08	-22.70	-35.44
7			11.76	13.80	-12.90	-14.94
8	3.00		11.76	13.80	-14.32	-18.38

FOR CONSTRUCTION

SEALING OF THIS DRAWING DOES NOT IMPLY OR CONSTITUTE THAT RIGID GLOBAL ENGINEER IS THE ENGINEER OF RECORD OR THE DESIGN PROFESSIONAL FOR THIS PROJECT. ONLY THE DESIGN OF THE METAL BUILDING SYSTEM AS FURNISHED BY RIGID IS INCLUDED. FOUNDATION ANALYSIS, ELECTRICAL, AND MECHANICAL SYSTEMS, AND/OR OTHER PARTS SUPPLIED BY ANYONE OTHER THAN RIGID ARE SPECIFICALLY EXCLUDED. NO INSPECTION OR SUPERVISION IS IMPLIED.

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
0	CONSTRUCTION/PERMIT	01/13/22	DA	RCR	JPL

DESCRIPTION	REACTIONS
CUSTOMER	CROSS DEVELOPMENT LLC
END USER	CALIBER COLLISION
END USE	COMMERCIAL BUILDING A
STREET	710 SE BLUE PKWY
CITY ST ZIP	LEES SUMMIT, MO 64002
SALES NO.	71872
JOB NO.	157337-38-39
SCALE	N.T.S.
DATE	F05 OF 5
ISSUE	0



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