

DEFERRED CORE SLAB SUBMITTAL

AS NOTED ON THE CONTRACT DOCUMENTS.

SHALL CONTACT APEX ENGINEERS, INC.

3RD/PARTY INSPECTION AGENCY.

- CORE SLAB DESIGN, BEARING THE SEAL OF LICENSED PROFESSIONAL

PRIOR TO FABRICATION AND INSTALLATION OF CORE SLABS.

ENGINEER, SHALL BE SUBMITTED TO APEX ENGINEERS, INC. FOR REVIEW

-CORE SLAB DESIGNER/MANUFACTURER SHALL FOLLOW ASSUMED CORE

SLAB DESIGN DIRECTIONS AS CLOSELY AS POSSIBLE TO CONFORM WITH

HOUSE STRUCTURE AS A WHOLE AND SHALL PROVIDE VERIFICATION THAT CORE SLABS HAVE BEEN DESIGNED FOR TOPPING SLAB THICKNESS(ES)

- IF DEVIATIONS FROM ASSUMED DESIGN ARE REQUIRED, MANUFACTURER

INSPECTED PRIOR TO PLACEMENT OF TOPPING SLAB. INSPECTION SHALL

- CORE SLAB INSTALLATION AND TOPPING SLAB FORMWORK SHALL BE

BE PERFORMED BY APEX ENGINEERS, INC. OR OTHER CERTIFIED

DECK BRACKET DETAIL NOT TO SCALE

APPLIED, FIRE-PROTECTIVE COATING

4000 PSI CONCRETE

LAP SPLICES 24" MIN

NO POINT LOADS ON

SUSPENDED SLAB.

DURING CONSTRUCTION

GRADE 40 REBAR

I-JOISTS IN UNFINISHED AREAS EXCEEDING 80 SQUARE FEET NEED

TO BE PROTECTED BY FACTORY

BY I-JOIST MANUFACTURER

BRACED WALL METHODOLOGY
CONTINUOUS EXTERIOR SHEATHING PER WSP METHOD (BELOW) UNLESS OTHERWISE NOTED ON THE PLAN

(3) PAIR BUNDLED #4 BAR

HDR OPENING

(6 TOTAL), EXTEND 2'-0"

CONCRETE HEADER

DETAIL

MIN PAST EACH SIDE OF

XXXX EXTERIOR BRACED WALLS:

OR

WSP METHOD: WOOD STRUCTURAL PANEL SHEATHING WITH A THICKNESS NOT LESS THAN 3/8" WITH MINIMUM SPAN RATING OF 24/0 FOR 16" OC STUD SPACING WITH 6d COMMON NAILS AT 6" OC EDGES AND 12" OC FIELD OR SHEATHING THICKNESS NOT LESS THAN $\frac{7}{6}$ " WITH MINIMUM SPAN RATING OF $\frac{24}{16}$ FOR 24" OC SPACING WITH 8d COMMON NAILS AT 6" OC EDGES AND 12" OC IN FIELD. (NOTE: FRAMING MEMBERS 16" OC MAX, UNBLOCKED, AND WITH SHEATHING APPLIED DIRECTLY TO FRAMING

//// INTERIOR BRACED WALLS (REF 2-S4.0):

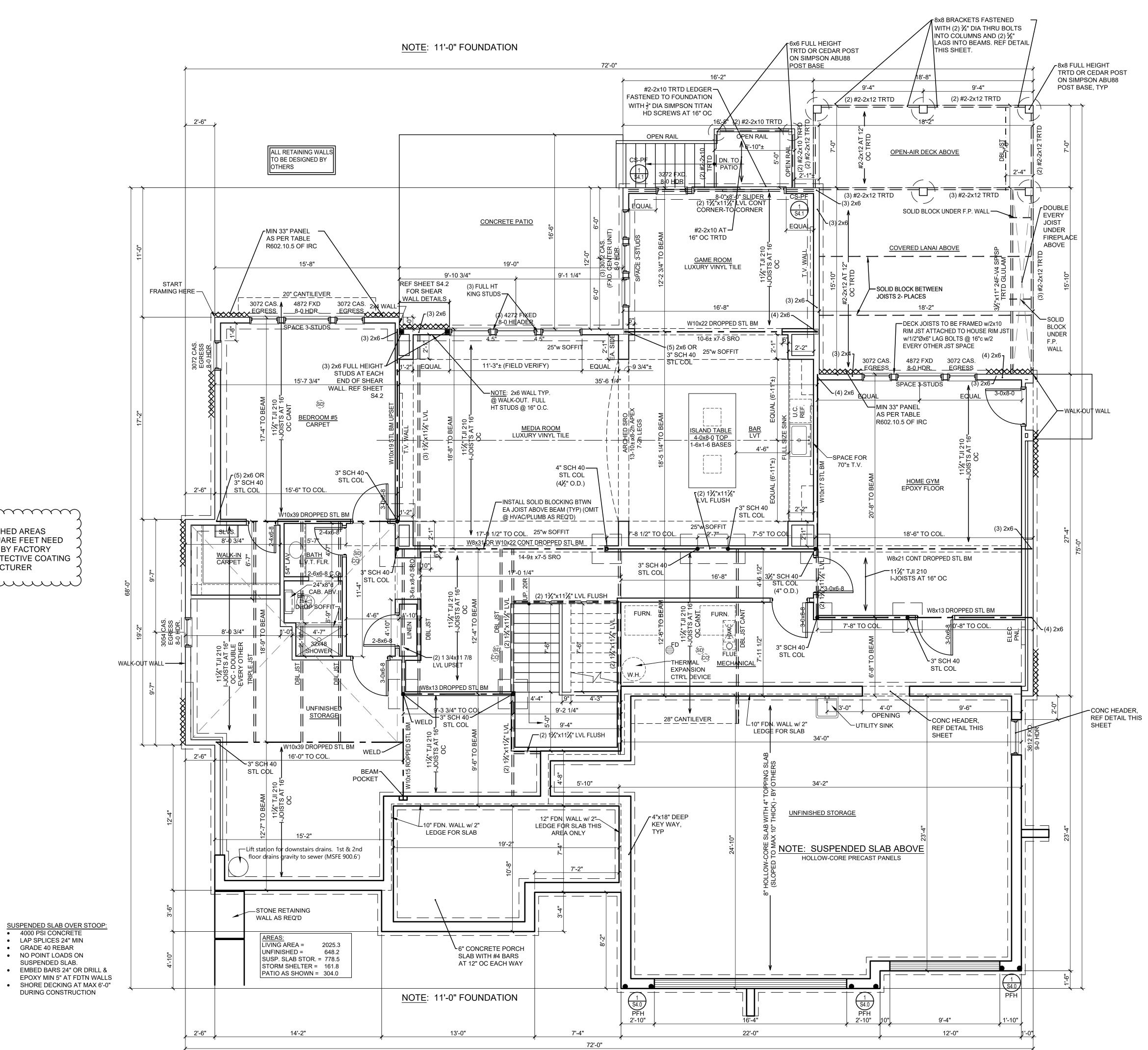
GB METHOD: ½" MIN. GYPSUM BOARD OVER STUDS SPACED 24" MAX. FASTENED WITH No 6 - 11/4" TYPE 'W' OR 'S' DRYWALL SCREWS AT 7" OC EDGES AND FIELD (MIN. 4'-0" SECTION FOR BOTH SIDES.)

LIB METHOD: 1x4 WOOD FASTENED WITH (3) 8d COMMON NAILS OR SIMPSON / USP 16 GA. TYPE WB (OR EQUAL) STL. X-BRACE(S) AT 45° TO 60° ANGLES, MAXIMUM 16" O.C. STUD FASTENED PER MANUFACTURER'S SPECIFICATIONS.

STRUCTURAL NOTES: - ALL UNMARKED HEADERS MIN (2)#2-2x10 - ALL HEADERS AND BEAMS MIN #2 GRADE DF/L (OR EQ.) = BEARING WALL

DEFERRED JOIST SUBMITTAL - JOIST DESIGN SHALL BE SUBMITTED TO APEX

ENGINEERS, INC. FOR REVIEW PRIOR TO CONSTRUCTION AND INSTALLATION OF JOISTS - JOIST DESIGNER/ MANUFACTURER SHALL FOLLOW ASSUMED JOIST DIRECTIONS AS CLOSELY AS POSSIBLE TO CONFORM WITH HOUSE STRUCTURE AS A WHOLE - IF DEVIATIONS FROM ASSUMED JOIST DESIGN ARE REQUIRED, MANUF. SHALL CONTACT APEX





Steenson Residence

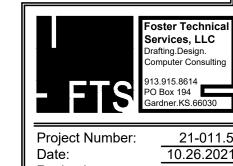
3000 Audubon Lane Winterset Lot #1451 Lee's Summit, MO

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Koehler Building Co. Inc.

12912 State Line Road Leawood, KS 66209 913-491-6565 www.koehlerbuildingco.com

Engineer of Record: Apex Engineers, Inc. 1625 Locust St, Kansas City, MO 64108 816.421.3222



Revised: Drawn By:

Sheet Number: Sheet Title: FOUNDATION PLAN

DRILLED PIER NOTES:

- 1. THIS DRILLED PIER PLAN IS **PRELIMINARY, NOT FOR** CONSTRUCTION AND FOR ESTIMATING PURPOSES ONLY. PRIOR TO CONSTRUCTION, APEX ENGINEERS SHALL BE CONTACTED TO PERFORM A SITE OBSERVATION AFTER EXCAVATION OF PROPOSED SITE AND PRIOR TO PIERS BEING DRILLED AND PLACED. THIS PIER PLAN IS PRELIMINARY AND IS INTENDED TO PROVIDE EQUIVALENT BEARING FOR THE STRUCTURE'S IMPOSED GRAVITY LOADS. NO MEASURES HAVE BEEN TAKEN TO RESIST UPLIFT DUE TO THE EFFECTS OF EXPANSIVE SOIL, LATERAL RESTRAINT DUE TO SITE STABILITY, OR OTHER UNFORESEEN CIRCUMSTANCES. APEX ENGINEERS SHALL PERFORM A SITE OBSERVATION AND RESERVES THE RIGHT TO RECOMMEND CONSULTING A LICENSED GEOTECHNICAL ENGINEER TO EXAMINE THE SITE IF EVIDENCE OF EXPANSIVE SOIL, SITE SLOPE STABILITY OR ANY OTHER ISSUES ARE PREVALENT AT THE SITE. THE FINDINGS FROM THE SITE OBSERVATION PERFORMED BY APEX COULD LEAD TO ADDITIONAL DESIGN CONSIDERATIONS AND/OR MORE STRINGENT DESIGN RECOMMENDATIONS. THIS DRILLED PIER PLAN IS **PRELIMINARY, NOT FOR CONSTRUCTION AND FOR** ESTIMATING PURPOSES ONLY UNTIL SITE OBSERVATION
- APPROVAL REPORT IS ISSUED BY APEX ENGINEERS.

 2. REFERENCE THE DRILLED PIER PLAN FOR THE DIAMETER AND
- LOCATION OF ALL PIERS REQUIRED.

 3. PIERS SHALL BE DRILLED TO END BEARING ON LIMESTONE,
 SANDSTONE OR SHALE BEDROCK WITH A MIN 15KSF ALLOWABLE
- BEARING CAPACITY, PER GEOTECH.

 4. ALL PIER HOLES SHALL BE INSPECTED TO BE CLEAR OF SPOILS,
- DEBRIS AND EXCESS WATER FOR ENTIRE DEPTH.

 5. UNLESS NOTED ON PLAN OR SCHEDULE, ALL PIERS SHALL BE REINFORCED WITH A MINIMUM OF THE FOLLOWING: (2) #4 LONGITUDINAL BARS FOR THE ENTIRE DEPTH. BEND AND DOWEL (4) #4 X 4'-0" BARS FROM TOP OF EACH PIER TO TIE INTO THE FOUNDATION. PROPER LAP SPLICE LENGTHS SHALL BE USED. REFERENCE DEEP FOUNDATION DETAILS.
- 6. ALL PIERS SHALL BE INSPECTED BY THE ENGINEER OF RECORD (APEX ENGINEERS) OR GEOTECHNICAL ENGINEER OF RECORD PRIOR TO PLACEMENT OF CONCRETE. UPON COMPLETION AND APPROVAL OF THE PIERS AND FOOTINGS THE FOUNDATION WALLS MAY BE PLACED PER PERMIT APPROVED DRAWINGS, UNLESS OTHERWISE DICTATED BY SUPPLEMENTAL STRUCTURAL RECOMMENDATIONS.
- ALL SLABS SHALL BE STRUCTURAL. FOR THE BASEMENT THE
 FOLLOWING DESIGN SHALL BE USED.

 a. PLACE 5" THICK CONCRETE SLAB WITH #4 BARS AT 12" OC EACH
 WAY ON 1 1/2" CHAIRS.
- b. ADD (4) 10'-0" LONG #4 BARS EACH WAY OVER THE COLUMN PADS AND SLAB SUPPORT PIERS. PLACE WITH 1" TO 1 1/2" SLAB TOP COVER (3" CHAIRS).
- c. THE PERIMETER OF THE SLAB SHALL BEAR ON THE FOUNDATION AS FOLLOWS: IF A MINIMUM OF 3" OF BEARING IS PROVIDED ON A KEYWAY OR FOOTING, THEN THE SLAB DOES NOT NEED TO BE PINNED TO
- THE WALL. OTHERWISE, DRILL 5" DEEP AND PIN THE SLAB TO THE FOUNDATION WALL WITH #4 BARS AT 12" OC.

 d. DO NOT SAW CUT STRUCTURAL SLABS UNLESS SPECIFICALLY INDICATED TO DO SO ON THE STRUCTURAL SLAB PLAN.
- e. PROVIDE (2) #4 X 4'-0" DIAGONAL BARS AT MID-DEPTH OF SLAB AT ALL RE-ENTRANT CORNERS.
- MIN 3000 PSI CONCRETE FOR PIERS. MIN 4000 PSI CONCRETE FOR STRUCTURAL SLAB.
 #4 AND SMALLER BARS, MIN GRADE 40. #5 AND LARGER BARS, MIN
- GRADE 60. MIN 24" LAP SPLICES.

 10. REFERENCE PIER FOUNDATION DETAILS FOR MORE INFORMATION.

 11. CONTRACTOR TO FIELD VERIFY ALL FOUNDATION ELEVATIONS
- AND STEP LOCATIONS PER SITE CONDITIONS.

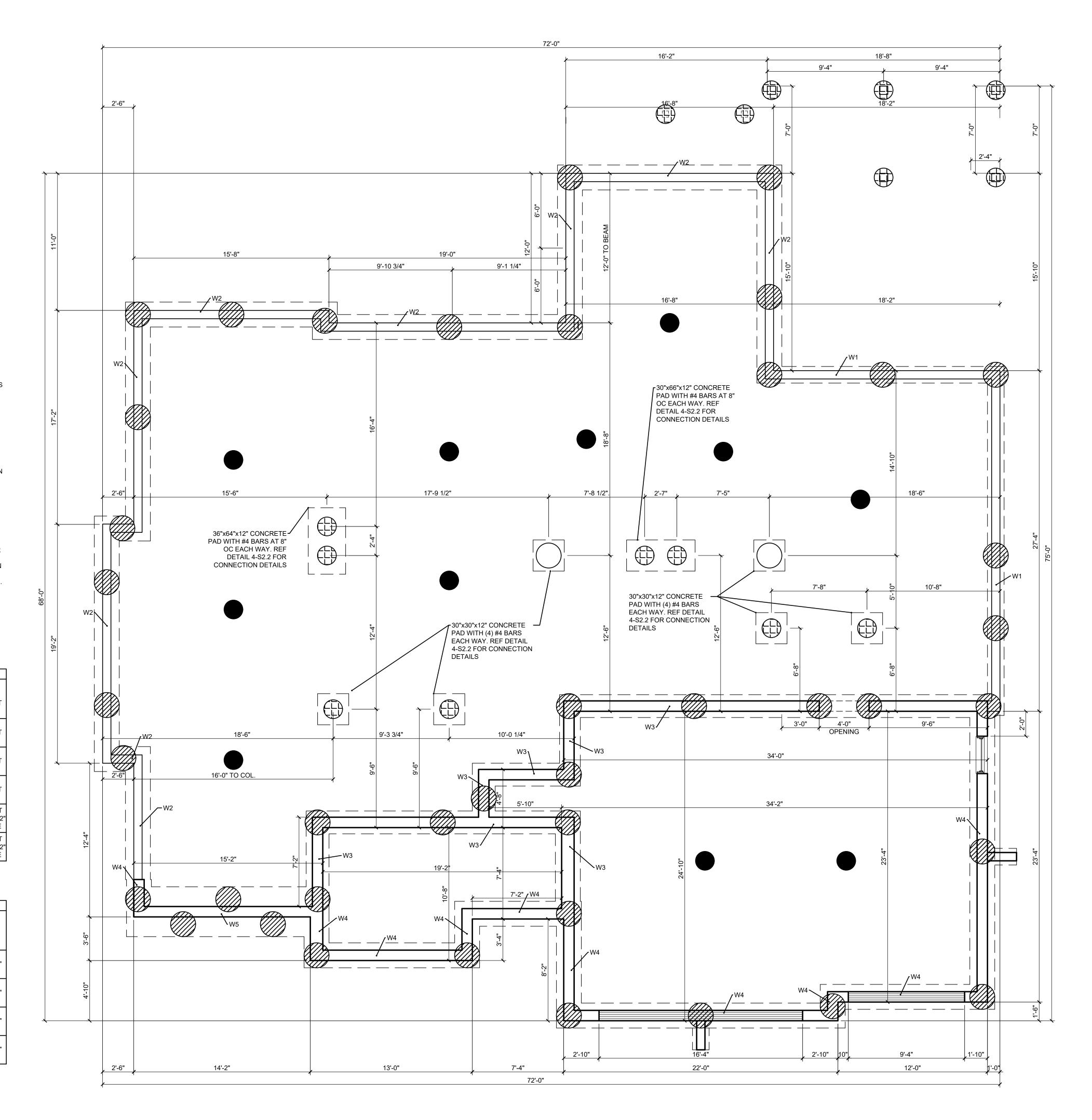
 12. REFER TO GEOTECH REPORT FOR ALL ADDITIONAL INFORMATION AND REQUIREMENTS

			ENGINEERE	D FOUNDAT	TION WALL SCHED	JLE	
MARK	WALL HEIGHT	WALL THICKNESS	FOOTING WIDTH	FOOTING DEPTH	WALL VERTICAL REINFORCEMENT	WALL HORIZONTAL REINFORCEMENT	FOOTING REINFORCEMENT
*W1	9'-0"	8"	16"	8"	#4 BARS AT 12" OC	#4 BARS AT 24" OC	(2) #4 BARS CONT
W2	9'-0"	8"	24"	12"	#4 BARS AT 8" OC	#4 BARS AT 12" OC	(3) #4 BARS CONT
W3	11'-0"	10"	20"	10"	#4 BARS AT 12" OC	#4 BARS AT 24" OC	(3) #4 BARS CONT
W4	11'-0"	10"	24"	12"	#5 BARS AT 12" OC	#5 BARS AT 12" OC	(4) #5 BARS CONT AND #5 BARS AT 12 OC TRANSVERSE
W5	11'-0"	10"	48"	12"	#5 BARS AT 12" OC	#5 BARS AT 12" OC	(8) #5 BARS CONT AND #5 BARS AT 12 OC TRANSVERSE

* WALLS LABELED WITH W1 WALL DESIGNATION SHORTER THAN 9'-0" ARE TO FOLLOW THE STEM THICKNESS, FOOTING SIZE AND REINFORCEMENT REQUIREMENTS OUTLINED ON SHEET S2.0

DRILLED PIER SCHEDULE MIN. SOCKET DIAMETER TIES DOWELS REINFORCING (4) #4 BARS (4) #4 x 4'-0" [FULL HEIGHT] (4) #4 BARS 4) #4 x 4'-0" [FULL HEIGHT] (4) #4 BARS [FULL HEIGHT (15) #5 BARS #3 AT 10" OC [FULL HEIGHT] (4) #4 x 4'-0"

- 1. TIES SHALL BE FULL DEPTH ACCORDING TO SCHEDULE SIZE AND SPACING.
- TIES SHALL BE 3" OC FOR TOP 12" OF PIER.
 2. MIN 3000 PSI CONCRETE FOR PIERS.
- #4 AND SMALLER BARS, MIN GRADE 40. #5 AND LARGER BARS, MIN GRADE 60. MIN 24" LAP SPLICES.





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Project Number: Date: Revised: Drawn By:

Sheet Number: A4

Sheet Title: FOUNDATION PLAN

BRACED WALL METHODOLOGY CONTINUOUS EXTERIOR SHEATHING PER WSP METHOD (BELOW) UNLESS OTHERWISE NOTED ON THE PLAN

XXXX EXTERIOR BRACED WALLS:

WSP METHOD: WOOD STRUCTURAL PANEL SHEATHING WITH A THICKNESS NOT LESS THAN %" WITH MINIMUM SPAN RATING OF 24/0 FOR 16" OC STUD SPACING WITH 6d COMMON NAILS AT 6" OC EDGES AND 12" OC FIELD OR SHEATHING THICKNESS NOT LESS THAN %" WITH MINIMUM SPAN RATING OF 24/16 FOR 24" OC SPACING WITH 8d COMMON NAILS AT 6" OC EDGES AND 12" OC IN FIELD. (NOTE: FRAMING MEMBERS 16" OC MAX,UNBLOCKED, AND WITH SHEATHING APPLIED DIRECTLY TO FRAMING

//// INTERIOR BRACED WALLS (REF 2-S4.0):

GB METHOD: ½" MIN. GYPSUM BOARD OVER STUDS SPACED 24" MAX. FASTENED WITH No 6 - 1½" TYPE 'W' OR 'S' DRYWALL SCREWS AT 7" OC EDGES AND FIELD (MIN. 4'-0" SECTION FOR BOTH SIDES.)

LIB METHOD: 1x4 WOOD FASTENED WITH (3) 8d COMMON NAILS OR SIMPSON / USP 16 GA.
TYPE WB (OR EQUAL) STL. X-BRACE(S) AT 45° TO 60° ANGLES, MAXIMUM 16"
O.C. STUD FASTENED PER MANUFACTURER'S SPECIFICATIONS.

STRUCTURAL NOTES: - ALL UNMARKED HEADERS MIN

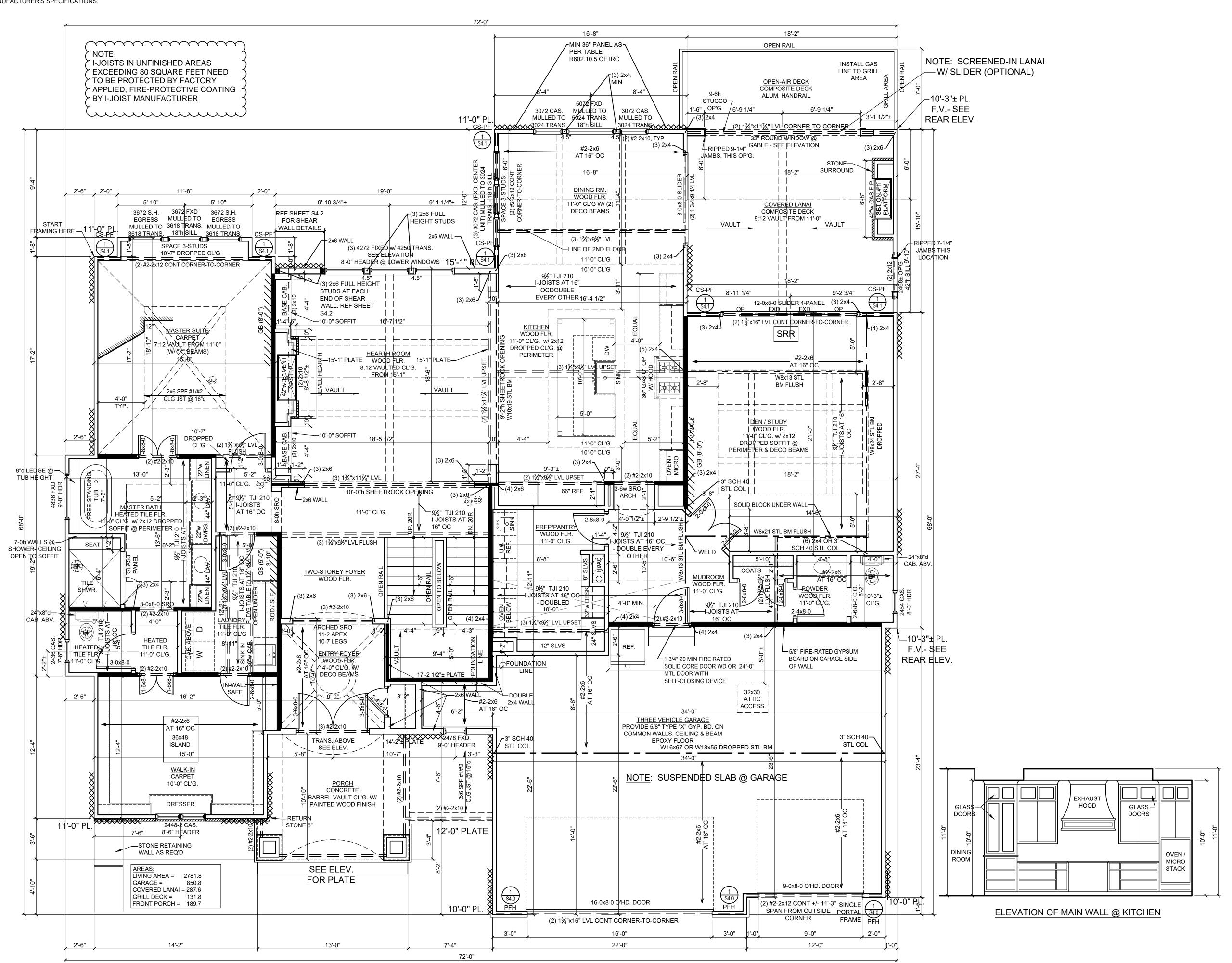
(2)#2-2x10
- ALL HEADERS AND BEAMS MIN #2
GRADE DF/L (OR EQ.)
- = BEARING WALL
- XXXXXX = 4'-0" LONG PANEL,

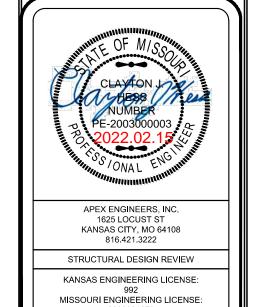
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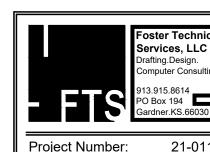
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Project Number:
Date:
Revised:
Drawn By:

Sheet Number: A5
Sheet Title: FIRST FLOOR PLAN

BRACED WALL METHODOLOGY
CONTINUOUS EXTERIOR SHEATHING PER WSP METHOD (BELOW)
UNLESS OTHERWISE NOTED ON THE PLAN

XXXX EXTERIOR BRACED WALLS:

WSP METHOD: WOOD STRUCTURAL PANEL SHEATHING WITH A THICKNESS NOT LESS THAN \(\frac{3}{6}\)" WITH MINIMUM SPAN RATING OF 24/0 FOR 16" OC STUD SPACING WITH 6d COMMON NAILS AT 6" OC EDGES AND 12" OC FIELD OR SHEATHING THICKNESS NOT LESS THAN \(\frac{7}{6}\)" WITH MINIMUM SPAN RATING OF \(\frac{24}{16}\) FOR 24" OC SPACING WITH 8d COMMON NAILS AT 6" OC EDGES AND 12" OC IN FIELD. (NOTE: FRAMING MEMBERS 16" OC MAX, UNBLOCKED, AND WITH SHEATHING APPLIED DIRECTLY TO FRAMING MEMBERS)

//// INTERIOR BRACED WALLS (REF 2-S4.0):

GB METHOD: ½" MIN. GYPSUM BOARD OVER STUDS SPACED 24" MAX. FASTENED WITH No 6 - 1¼" TYPE 'W' OR 'S' DRYWALL SCREWS AT 7" OC EDGES AND FIELD (MIN. 4'-0" SECTION FOR BOTH SIDES.)

OR

LIB METHOD: 1x4 WOOD FASTENED WITH (3) 8d COMMON NAILS OR SIMPSON / USP 16 GA. TYPE WB (OR EQUAL) STL. X-BRACE(S) AT 45° TO 60° ANGLES, MAXIMUM 16" O.C. STUD FASTENED PER MANUFACTURER'S SPECIFICATIONS.

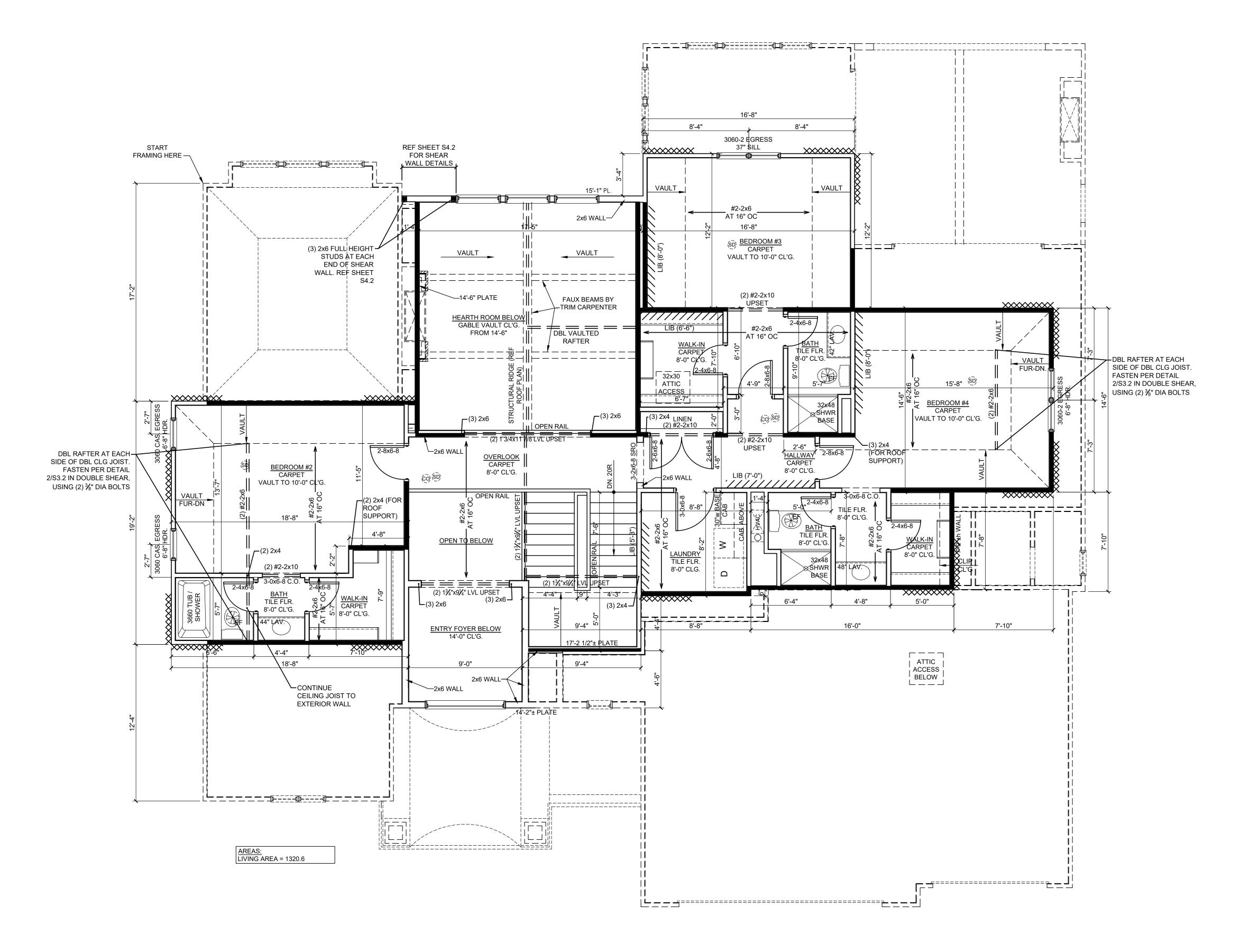
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Project Number
Date:
Revised:
Drawn By:

Sheet Title: SECOND FLOOR PLAN

Sheet Number:

ROOF FRAMING NOTES

ROOF DESIGNED FOR LIGHT ROOF COVERING 30psf TOTAL LOAD [10psf DL, 20psf LL (SL)]

ROOF SYSTEM IS DESIGNED TO MEET REQUIREMENTS OF IRC 802

*RAFTERS (HEM-FIR, DOUG-FIR, OR EQUAL): SEE SPAN CHARTS BELOW

CODE MINIMUM

CODE IVIIIVIIVIOIV		
RAFTERS	SPACING	MAX HORIZONTAL CLEARSPAN
#2-2x6	AT 24" OC	11'-7"
#2-2x6	AT 16" OC	14'-2"
#2-2x8	AT 24" OC	14'-8"
#2-2x8	AT 16" OC	17'-11"
#2-2x10	AT 24" OC	17'-10"
#2-2x10	AT 16" OC	21'-11"

NOTE: CODE MINIMUM ALLOWS FOR A RAFTER DEFLECTION OF L/180 TOTAL LOAD

HIGHER PERFORMANCE

SPACING	MAX HORIZONTAL CLEARSPAN
AT 24" OC	8'-6"
AT 16" OC	9'-9"
AT 24" OC	11'-3"
AT 16" OC	12'-9"
AT 24" OC	14'-3"
AT 16" OC	16'-3"
	AT 24" OC AT 16" OC AT 24" OC AT 16" OC AT 24" OC

APEX ENGINEERS, INC. RECOMMENDED DEFLECTION = L/360 LIVE LOAD, L/240 TOTAL LOAD

*RIDGE BOARDS ARE (UNLESS OTHERWISE NOTED)
#2-2x10 UP TO 9:12 PITCH

#2-2x12 OVER 9:12 PITCH
*ALL HIPS AND VALLEYS ARE (UNLESS OTHERWISE NOTED)
#2-2x10 UP TO 9:12 PITCH

#2-2x12 OVER 9:12 PITCH

*PURLINS ARE 2x6 MIN
- PURLIN STRUTS ARE AT 4'-0" OC

 PURLIN STRUTS ARE AT 4-0 OC
 PURLIN STRUTS SHALL BE INSTALLED AT NOT LESS THAN A 45 DEGREE ANGLE WITH THE HORIZONTAL
 ALL PURLIN STRUTS SHALL HAVE A MAX UNBRACED

LENGTH OF 8'-0"
- PURLIN STRUTS SHALL BE CONSTRUCTED IN A "T"
CONFIGURATION AND PER THE FOLLOWING CHART:

PURLIN STRUT	MAX PURLIN STRUT LENGTH
(2)2x4	8'-0"
(1)2x4 AND (1)2x6	12'-0"
(1)2x6 AND (1)2x8	20'-0"
(2)2x6 AND (1)2x8	30'-0"
CONSULT ARCH ENGR	>30'-0"

*EACH END OF STRUT SHALL BE FASTENED WITH MIN (3)8d OR (2)16d NAILS

*RIDGE BRACERS ARE SAME AS PURLIN BRACES-SPACING, SIZE, CONFIGURATION, AND INSTALLATION (SEE PURLIN

BRACE NOTES ABOVE)
*HIP AND VALLEY BRACES ARE THE SAME AS PURLINS SIZE,
CONFIGURATION, AND INSTALLATION (SEE PURLIN BRACE

= ROOF BRACE/STRUT (PER CHART)
-SLASH IS TOP END OF BRACE
-CIRCLE IS BOTTOM END OF BRACE

= PURLIN STRUTS AT 48" OC (PER CHART) U.N.O.
-SLASH IS TOP END OF BRACE
-ARROW IS BEARING LOCATION

DENOTES BEARING WALL

DENOTES PURLIN

DENOTES BEARING STRUCTURE

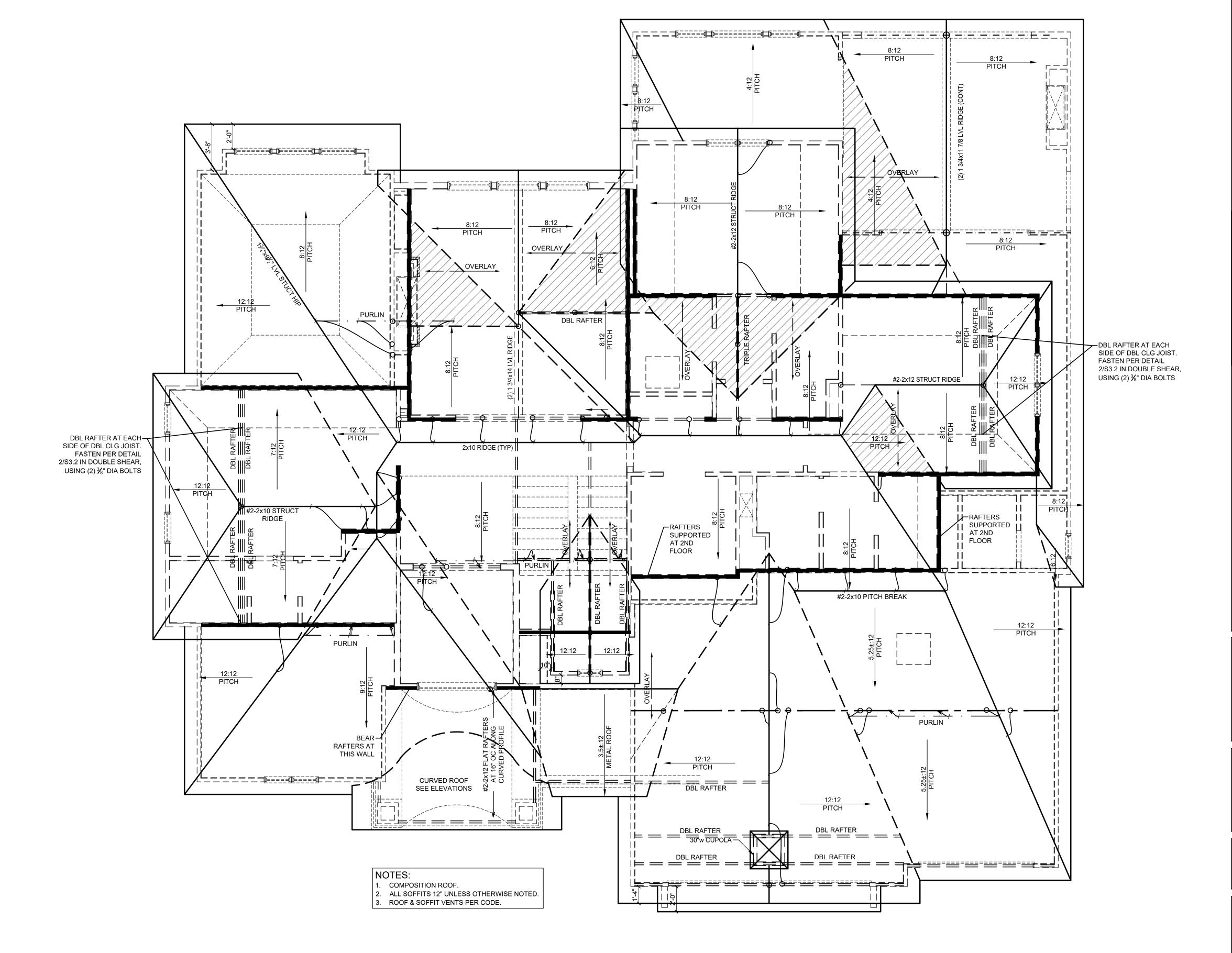
STRUCTURAL NOTES:

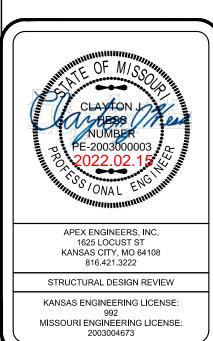
- ALL UNMARKED HEADERS MIN
(2)#2-2x10

- ALL HEADERS AND BEAMS MIN #2
GRADE DF/L (OR EQ.)

- = BEARING WALL

- XXXXXXXX = 4'-0" LONG PANEL,





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Project Number:
Date:
Revised:
Drawn By:
Sheet Number:

Sheet Number: A7
Sheet Title: ROOF PLAN

BUILDING COMPONENT	MATERIAL	FASTENING
	7/16" PLYWOOD	16 GA x 1-3/4" STAPLES AT 3" OC EDGES AND 6" OC IN FIELD
ROOF SHEATHING ¹	1x4 #3 FURRING	1/2" CROWN STAPLES
FLOOR SHEATHING ¹	3/4" T&G YELLOW PINE PLYWOOD APPLIED PERPENDICULAR TO JOISTS AND ENDS STAGGERED	8d COMMON NAILS AT 6" OC EDGES AND 12" OC IN THE FIELD 14 GA x 2" STAPLES AT 4" OC EDGES AND 8" OC IN THE FIELD 12.5 GA x 1-1/2" RING OR SCREW SHANK NAILS AT 6" OC EDGES AND 8" OC IN THE FIELD
CEILING COVERING ¹	1/2" GYPSUM SHEATHING	7" OC NAILED / 12" OC SCREWED WITH 13 GA, 1-3/8" LONG, 19/64" HEAD; 0.098 DIA, 1-1/4" LONG, ANGRINGED; 5d COOLER NAIL, 0.086 DIA, 1-5/8" LONG, 15/64" HEAD; OR GYP BD NAIL, 0.086 DIA, 1-5/8" LONG, 9/32" HEAD
INTERIOR WALL COVERING ¹	1/2" GYPSUM SHEATHING	6d COMMON NAILS; 1-5/8" GALVANIZED STAPLES; 1-1/4" SCREWS, TYPE W OR S- AT 4" OC EDGES AND 8" OC IN THE FIELD
EXTERIOR WALL SHEATHING	MIN 3/8" APA RATED SHEATHING	8d COMMON NAILS AT 6" OC EDGES AND 12" OC IN THE FIELD
CONVENTIONAL WOOD FRAMED WALLS	*SUPPORTING 2 FLOORS, ROOF, AND CEILING OR LESS. *HEIGHT: 10'-0" OR LESS SIZE: NOM 2x4 (NOM 2x6 WHEN SUPPORTING 2 FLOORS, CEILING, AND ROOF) *SPECIES: DOUG-FIR, HEM-FIR, SOUTH PINE, SPRUCE-PINE-FIR *MAXIMUM SPACING 16" OC *STUDS 10' LENGTH OR LESS SHALL BE #3 STANDARD, OR STUD GRADE *STUDS OVER 10' LENGTH SHALL BE MIN #2 GRADE	*TOE NAIL RIM JOIST TO SILL OR TOP PLATE: *TOE NAIL STUD TO TOP AND SOLE PLATE: *END NAIL TOP AND SOLE PLATE TO STUD: *FACE NAIL BUILT-UP CORNER STUDS: *FACE NAIL BUILT-UP CORNER STUDS: *FACE NAIL JOENSTRIMMERS SUPPORTING HEADERS WITH: *FACE NAIL DBL TOP PLATE: *TOE NAIL TOP PLATES WITH MIN 48" OFFSET OF EACH. FACE NAIL LAPPED AREA WITH: *FACE NAIL DBL TOP PLATES AT LAPPED CORNERS AND INTERSECTIONS WITH: *FACE NAIL SOLE PLATE TO FRAMING SYSTEM WITH: *FACE NAIL BRIDGING TO JOIST, EACH END: *FACE NAIL LEDGER STRIPS SUPPORTING JOISTS OR RAFTERS WITH: **GOMMON AT 6" OC; 3"x0.131" AT 16" OC; 3"x0.131" 16d COMMON NAILS AT 16" OC; 3"x0.131" AT 12" OC; 3"x0.128" AT 12" OC (2) 16d COMMON; (12) 3"x0.131"; (12) 3"x0.128" (3) 16d COMMON; (2) 3"x0.131"; (3) 3"x0.128"
CONVENTIONAL WOOD HEADER FRAMING	PER PLAN	*TOE NAIL HEADERS TO WALL STUDS WITH (4) 8d NAILS AT EACH END. *FACE NAIL DOUBLE PIECE HEADERS WITH 16d NAILS AT 16" CENTERS ALONG EACH EDGE.
RAFTER TIES ²	MIN 2x4 MEMBERS AT EACH RAFTER	REF TABLE R802.5.2
COLLAR TIES	MIN 1x4 MEMBERS AT 48" OC	FACENAIL TO RAFTERS IN UPPER 1/3 OF ATTIC SPACE WITH (3) 10d NAILS AT EACH
RAFTER TIES SHALL NOT E	TERIALS TO BE APPLIED PERPENDICUL BE REQUIRED WHEN A STRUCTURAL RII JULTED ROOM). SUCH SHALL BE NOTED	OGE HAS BEEN PROVIDED AND ADEQUATELY
BUILDING COMPONENT	FASTEN TO	FASTEN WITH
	TO RIDGE/VALLEY/HIP RAFTERS	TOENAIL WITH (4) 16d ENDNAIL WITH (3) 16d
RAFTERS	TO PLATE	TOENAIL WITH (2) 16d
CEILING JOISTS	TO TOP PLATE	TOENAIL WITH (3) 8d AT EACH END
SEIE 110 001010		DISTS RUN PARALLEL TO RAFTERS D RAFTERS WITH (3) 10d MIN
FI 6 6 5 1 2 1 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	TO SILL OR GIRDER	TOENAL WITH: (3) 8d COMMON; (3) 3"x0.131"; (4) 3"x
FLOOR JOISTS	TO RIM JOIST	ENDNAIL WITH: (3) 16d COMMON; (4) 3"x0.131"; (4) 3"
RACED WALL PANELS ERP TO FRAMING	TO FRAMING MEMBER	SOLE PL, 16" OC WITH: (3) 16d COMMON; (4) 3"x0.131" TOP PL, 6" OC WITH: 8d COMMON; 3"x0.131"
EMBERS ABOVE/BELOW: ARALLEL TO FRAMING EMBERS ABOVE/BELOW:	TO FRAMING AND BLOCKING AT 16" OC	SOLE PL, 16" OC WITH: (3) 16d COMMON; (4) 3"x0.131" AND AT EACH BLOCK: (3) 16d COMMON; (4) 3"x0.131" TOP PL, 6" OC WITH: 8d COMMON; 3"x0.131" AND AT EACH BLOCK: (3) 8d COMMON; 3"x0.131"

ENERGY REQUIREMENTS

1. LIGHTING FIXTURES PENETRATING THE THERMAL ENVELOPE SHALL BE IC-RATED, LEAKAGE RATED, AND SEALED TO THE GYPSUM WALLBOARD AS REQUIRED PER N1102.4.5. 2. PROGRAMMABLE THERMOSTATS SHALL BE INSTALLED AS REQUIRED PER

3. AIR HANDLERS SHALL BE RATED FOR MAXIMUM 2% AIR LEAKAGE RATE PER

N1103.3.2.1. 4. BUILDING FRAMING CAVITIES SHALL NOT BE USED AS DUCTS OR PLENUMBS PER N1103.3.5

5. HOT WATER PIPES SHALL BE INSULATED AS REQUIRED PER N1103.4. 6. ALL EXHAUST FANS SHALL TERMINATE TO THE BUILDING EXTERIOR AS REQUIRED PER M1501 1

7. MAKEUP AIR SYSTEMS SHALL BE INSTALLED FOR KITCHEN EXHAUST HOODS THAT EXCEED 400 CFM AS REQUIRED PER M1503.6. 8. AN AIR HANDLING SYSTEM SHALL NOT SERVE BOTH THE LIVING SPACE AND THE GARAGE PER M1601.6.

ENERGY CONSERVATION

THE ENERGY EFFICIENCY OF THE DWELLING SHALL COMPLY WITH THE FOLLOWING TABLE(S) (WHERE THERE ARE DISCREPANCIES BETWEEN THIS TABLE AND THE PLANS, THE MOST RESTRICTIVE SHALL APPLY). IF TABLE 1 IS NOT COMPLETED AND ACCOMPANIED BY RESCHECK CALCULATIONS, THEN TABLE 2 SHALL BE APPLIED.

TABLE 1 - ResCheck COMPLIANCE SOFTWARE (FILL IN APPLICABLE VALUES FROM ResCheck CALCS. **BUILDING ELEMENT MIN VALUE** WALLS - FRAMED WALLS - BASEMENT FLOORS - UNCONDITIONED SPACE FLOORS - OVER OUTSIDE AIR FLOORS - CRAWL SPACE **SLAB - PERIMETER CEILING - FLAT** CEILING - CATHEDRA DOORS - GLASS DOORS - SOLID WINDOWS - OPERABL WINDOWS - FIXED WINDOWS - OTHER FURNACE AFUE-AIR CONDITIONER SFFR-

NOTE: FOR USE OF TABLE 1 A ResCheck COMPLIANCE FORM MUST BE SUBMITTED WITH PLANS.
 TABLE 2 -PRESCRIPTIVE ENVELOPE (MIN PRESCRIPTIVE APPROACH

ACCEPTABLE FOR ANY DWELLING.)

BUILDING ELEMENT	MIN VALUE
CEILING - FLAT	R-49
CEILING - CATHEDRAL**	R-30
CEILING - CATHEDRAL	R-38
FLOORS - UNCONDITIONED SPACED	R-19
FLOORS - OVER OUTSIDE AIR	R-30
WALLS - BASEMENT	R-10 (CONT) OR R-13 (CAVITY)
CONCRETE SLAB ON GRADE	R-10 (FOR 2FT)
SKYLIGHTS	U=0.55
WALLS - EXTERIOR (2x4)	R-13 (CAVITY) + R-5 (CONT)
WALLS - EXTERIOR (2x6)	R-20
WALLS - CRAWL SPACE	R-19
GLAZING*	U<=0.32

SHGF<=0.40 TABLE 2 PER IRC TABLE N1102.1.2 *DEFAULT U-FACTOR FOR DOUBLE PANE. ARGON FILLED LOW-E TREATMENT IS U=0.35

**LIMITED TO AREAS LESS THAN 500 SQ-FT OR 20% OF CEILING AREA.

DEFERRED SUBMITTALS 1. THE ARCHITECT OR ENGINEER OF RECORD SHALL LIST THE DEFERRED SUBMITTALS ON THE PLANS FOR REVIEW BY THE BUILDING OFFICIAL DOCUMENTS FOR DEFERRED SUBMITTAL ITEMS SHALL BE SUBMITTED TO THE ARCHITECT OR ENGINEER OF RECORD WHO SHALL REVIEW THEM AND FORWARD THEM TO THE BUILDING OFFICIAL WITH A NOTATION INDICATING THAT THE DEFERRED SUBMITTAL DOCUMENTS HAVE BEEN REVIEWED AND FOUND TO BE IN THE GENERAL CONFORMANCE TO THE DESIGN OF THE BUILDING. THE DEFERRED SUBMITTAL ITEMS SHALL NOT BE INSTALLED UNTIL THE DEFERRED SUBMITTAL DOCUMENTS HAVE BEEN APPROVED BY THE BUILDING OFFICIAL. DEFERRED SUBMITTALS ARE DEFINED AS THOSE PORTIONS OF THE DESIGN THAT ARE NOT SUBMITTED AT THE TIME OF THE APPLICATION AND THAT ARE TO BE SUBMITTED TO THE BUILDING OFFICIAL WITH A SPECIFIED PERIOD. DEFERRAL OF ANY SUBMITTAL ITEMS SHALL HAVE THE PRIOR

APPROVAL OF THE BUILDING OFFICIAL. 2. DEFERRED SUBMITTAL ITEMS (WHEN APPLICABLE):

- A. TRUSSES B. I-JOISTS
- C. GUARDRAILS AND HANDRAILS
- D. STEEL FABRICATED STAIRS
- E. PRE-MANUFACTURED CANOPIES AND AWNINGS F. PRECAST HOLLOW CORE SLABS
- G. GROUND IMPROVEMENT AND/OR STRUCTURAL FOUNDATION SOLUTIONS (SUCH AS DRILLED PIERS)

CONCRETE

CONCRETE SHALL BE AIR ENTRAINED WITH A MINIMUM COMPRESSIVE STRENGTH OF 28 DAYS OF 2,500 PSI FOR BASEMENT AND INTERIOR FLOOR SLABS, 3,000 PSI FOR BASEMENT AND FOUNDATION WALLS, AND 3,500 FOR PORCHES, CARPORTS, AND GARAGE FLOOR SLABS.

GLAZING

GLAZING IN HAZARDOUS LOCATIONS AS IDENTIFIED IN IRC SECTION R308.4 SHALL BE OF APPROVED SAFETY GLAZING MATERIALS: GLASS IN STORM DOORS: INDIVIDUAL FIXED OR OPENABLE PANELS ADJACENT TO A DOOR WHERE THE NEAREST VERTICAL EDGE IS WITHIN A 24" ARCH OF THE DOOR IN A CLOSED POSITION AND WHOSE BOTTOM EDGE IS WITHIN 60" OF THE FLOOR: WALLS ENCLOSING STAIRWAYS AND LANDINGS WHERE THE GLAZING IS WITHIN 60" OF THE TOP OR BOTTOM OF THE STAIR; ENCLOSURES FOR SPAS, TUBS, SHOWERS, AND WHIRLPOOLS; GLAZING IN FIXED OR OPENABLE PANELS EXCEEDING 9 SQUARE FEET AND WHOSE BOTTOM EDGE IS LESS THAN 18" ABOVE THE FLOOR OR WALKING SURFACE WITHIN 36".

EMERGENCY EGRESS AND RESCUE

1. PROVIDE ONE WINDOW FROM EACH BEDROOM THAT HAS A MINIMUM OPENABLE AREA OF 5.7 SQUARE FEET WITH A MINIMUM OPENABLE HEIGHT OF 24 INCHES AND WIDTH OF 20 INCHES.

2. BASEMENT EGRESS TO MEET THE REQUIREMENTS OF IRC SECTION 310. 3. SMOKE ALARMS SHALL BE INSTALLED AS REQUIRED PER IRC 2018 SECTION R314. 4. PROVIDE SMOKE ALARMS IN EACH SLEEPING ROOM, OUTSIDE OF EACH SLEEPING AREA, ON EACH FLOOR INCLUDING BASEMENTS AND HABITABLE ATTICS, AND NOT LESS THAN 3'-0" HORIZONTALLY FROM DOOR OR OPENING OF A BATHROOM THAT CONTAINS A BATHTUB OR SHOWER. ALARMS SHALL BE INTERCONNECTED IN SUCH A MANNER THAT THE ACTUATION OF ONE ALARM WILL ACTIVATE ALL OF THE ALARMS IN THE DWELLING

5. CARBON MONOXIDE ALARMS SHALL BE INSTALLED AS REQUIRED PER IRC 2018 SECTION R315. 6. CARBON MONOXIDE ALARMS SHALL BE INSTALLED OUTSIDE OF EACH

SEPARATE SLEEPING AREA. WHERE A FUEL-BURNING APPLIANCE IS LOCATED WITHIN A BEDROOM OR ITS ATTACHED BATHROOM, A CARBON MONOXIDE ALARM SHALL BE INSTALLED WITHIN THE BEDROOM.

FRAMING GENERAL

1. ALL LUMBER SIZES ARE FOR DOUGLAS FIR-LARCH UNLESS NOTED OTHERWISE. 2. ALL HEADERS TO BE MIN (2) #2-2x10 UNLESS NOTED OTHERWISE. 3. BLOCK CANTILEVERS, DOORJAMBS, AND OVER BEAMS.

4. ALL HEADERS TO BEAR ON A MINIMUM OF (2) 2x4 STUD POSTS UNLESS NOTED OTHERWISE 5. INTERIOR NON-BEARING WALLS, OTHER THAN THOSE RESTING DIRECTLY ON THE FOOTING SHALL BE ISOLATED FROM THE FLOOR FRAMING ABOVE.

6. WHERE JOISTS RUN PARALLEL TO FOUNDATION WALLS, SOLID BLOCKING FOR A MINIMUM OF (2) JOIST SPACES BE PROVIDED TO A MAXIMUM OF 2'-0" CENTERS TO TRANSFER LATERAL LOADS ON THE WALL TO THE FLOOR DIAPHRAGM. THE BLOCKING SHALL BE SECURELY NAILED TO THE JOISTS AND FLOORING. NAIL JOISTS AND BLOCKING TO SILL PLATE WITH (3) 10d NAILS (IRC SECTION R602.3.(1) 7. IF DUCTS ARE INSTALLED IN THE FIRST JOIST SPACE(S), NAIL 2x4s FLAT AT 2'-0" CENTERS WITHIN THE JOIST SPACE(S) AND THEN PROVIDE SOLID BLOCKING, INSTALLED UPRIGHT, IN THE NEXT TWO JOIST SPACES. SECURE THE 2x4s TO THE

SILL PLATE WITH (4) 10d NAILS. 8. ALL SILLS AND SLEEPERS SUPPORTED ON CONCRETE OR MASONRY AND FURRING ATTACHED TO CONCRETE OR MASONRY SHALL BE OF DECAY RESISTANT MATERIALS

9. JOISTS UNDER BEARING PARTITIONS SHALL BE DOUBLED AND COMPLY WITH IRC SECTION R502.4.

10. JOISTS FRAMING FROM OPPOSITE SIDES OVER BEARING SUPPORTS SHALL LAP A MINIMUM 3" AND SHALL BE NAILED TOGETHER WITH A MINIMUM 10d FACE NAILS. 11. JOISTS FRAMING INTO A WOOD GIRDER OR BEAM SHALL BE SUPPORTED BY APPROVED FRAMING ANCHORS OR MINIMUM 2"x2" LEDGER STRIPS. 12. FRAMING OF OPENINGS - HEADERS AND TRIMMERS SHALL BE OF SUFFICIENT CROSS SECTION TO SUPPORT THE FLOOR FRAMING. TRIMMER JOISTS SHALL BE DOUBLED WHEN THE HEADER IS SUPPORTED MORE THAN 3'-0" FROM THE TRIMMER JOIST BEARING. WHEN THE HEADER SPAN EXCEEDS 4'-0", THE

HEADER AND TRIMMER SHALL BE DOUBLED. 13. JOISTS AT SUPPORTS SHALL BE SUPPORTED LATERALLY AT THE ENDS BY FULL-DEPTH SOLID BLOCKING NOT LESS THAN 2" NOMINAL THICKNESS OR BY ATTACHMENT TO A HEADER, BAND OR RIM JOIST OR TO AN ADJOINING STUD OR OTHERWISE PROVIDED WITH LATERAL SUPPORT TO PREVENT ROTATION. 14. WATER-RESISTIVE BARRIER SHALL BE PROVIDED OVER ALL EXTERIOR WALLS. ONE LAYER OF No 15 ASPHALT FELT OR ANY OTHER BARRIER THAT MEETS ASTM D226 TYPE 1 FELT. (R703.2)

15. WHERE CEILING JOISTS ARE NOT INSTALLED CONNECTED TO THE RAFTERS AT THE TOP PLATE AND/OR WHERE CEILING JOISTS ARE NOT INSTALLED PARALLEL TO THE RAFTERS, RAFTER TIES SHALL BE INSTALLED IN THE LOWER 1/3 OF THE ATTIC SPACE AND IN ACCORDANCE WITH TABLE 1-S1.0. 16. COLLAR TIES SHALL BE PROVIDED IN THE UPPER 1/3 OF THE ATTIC SPACE IN ACCORDANCE WITH TABLE 1-S1.0.

GARAGE

1. THE GARAGE FLOOR SHALL SLOPE TOWARDS THE GARAGE DOORWAYS. 2. DOORS BETWEEN THE GARAGE AND THE DWELLING - MINIMUM 1-3/8" SOLID CORE OR HONEY COMBED STEEL DOOR OR 20-MINUTE FIRE RATED. 3. THE GARAGE SHALL BE SEPARATED FROM THE RESIDENCE AND ITS ATTIC AREA BY 5/8", TYPE X GYPSUM BOARD, OR EQUIVALENT MATERIALS APPROVED FOR ONE-HOUR FIRE-RESISTIVE CONSTRUCTION, APPLIED TO GARAGE SIDE. WHERE THE SEPARATION IS A FLOOR-CEILING ASSEMBLY, THE STRUCTURE SUPPORTING THE SEPARATION SHALL ALSO BE PROTECTED BY 5/8", TYPE X GYPSUM BOARD, OR MATERIALS APPROVED FOR ONE-HOUR FIRE-RESISTIVE CONSTRUCTION OR EQUIVALENT, APPLIED TO THE GARAGE SIDE. PULL DOWN STAIRS LOCATED WITHIN GARAGE SHALL BE RATED TO BE ADEQUATELY PROTECTED WITH MATERIALS APPROVED FOR ONE-HOUR FIRE-RESISTIVE CONSTRUCTION. ATTIC ACESS PANELS LOCATED WITHIN GARAGE SHALL BE OF 5/8", TYPE X GYPSUM BOARD, OR MATERIALS FOR ONE-HOUR FIRE-RESISTIVE CONSTRUCTION.
4. GARAGE DOOR AND FRAME- THE H-FRAME FOR THE ATTACHMENT OF THE TRACK AND COUNTER BALANCE SHALL CONSIST OF THE FOLLOWING: 2x6

VERTICAL JAMBS RUNNING FROM THE FLOOR TO CEILING ATTACHED WITH 1-3/4"

x 0.120" NAILS AT 7" OC STAGGERED WITH (7) 3-1/4" x 0.120" NAILS THRU THE JAMB

INTO THE HEADER, MINIMUM 2x8 HEADER FOR ATTACHMENT OF COUNTER

BALANCE SYSTEM.

8. SPACE STRINGERS AT 16" OC MAX.

STAIRWAYS

1. STAIRWAYS SHALL PROVIDE A MAXIMUM 7-3/4" RISE AND MINIMUM 10" RUN. 2. PROVIDE MINIMUM 36" GUARDRAILS ON THE OPEN SIDES OF RAISED FLOORS, PORCHES, AND BALCONIES: MINIMUM 34" GUARDRAILS ON THE OPEN SIDES OF STAIRWAYS LOCATED MORE THAN 30" ABOVE THE FLOOR OR GRADE BELOW. GUARDRAIL ENCLOSURES SHALL HAVE INTERMEDIATE RAILS OR ORNAMENTAL PATTERNS THAT DO NOT ALLOW PASSAGE OF A SPHERE 4" IN DIAMETER. 3. EACH STAIRWAY OF THREE OR MORE RISERS SHALL PROVIDE A CONTINUOUS HANDRAIL ON AT LEAST ONE SIDE BETWEEN 34" AND 38" ABOVE THE NOSING OF

THE TREADS 4. HANDRAILS SHALL HAVE A CIRCULAR CROSS SECTION OF 1-1/4" MINIMUM TO 2" MAXIMUM OR OTHER APPROVED GRASPABLE SHAPER PER IRC SECTION

5. PROVIDE A MINIMUM 6'-8" OF HEADROOM CLEARANCE IN STAIRWAYS. 6. ENCLOSED ACCESSIBLE SPACE UNDER STAIRWAYS SHALL HAVE WALLS AND THE UNDERSIDE OF THE STAIR AND LANDING PROTECTED WITH 1/2" GYPSUM BOARD ON ENCLOSURE SIDE PER IRC SECTION 302.7. 7. SPIRAL STAIRS TO BE CONSTRUCTED PER IRC SECTION 311.7.10.1.

GENERAL

1. PLANS SHALL COMPLY WITH THE 2018 INTERNATIONAL RESIDENTIAL CODE WITH AMENDMENTS AS ADOPTED BY THE GOVERNING JURISDICTION. IF ANY CHANGES OR DEVIATIONS FROM THE PLANS ARE MADE DURING CONSTRUCTION, CONTRACTOR SHALL NOTIFY THE APPROPRIATE AUTHORITY AND ENGINEER OF RECORD, EITHER (OR BOTH) OF WHOM MAY REQUIRE REVISED DRAWINGS OR CALCULATIONS AT ITS DISCRETION

2. REPRODUCTION, ALTERATION, OR RE-USE BY ANY METHOD OF ALL OR PORTIONS OF THESE STRUCTURAL PLANS OR VARIATIONS THEREOF WITHOUT WRITTEN PERMISSION FROM APEX ENGINEERS, INC IS STRICTLY PROHIBITED. THE DRAWINGS AND DETAILS OF THIS SHEET SET. BEING INSTRUMENTS OF SERVICE. ARE AND SHALL REMAIN THE PROPERTY OF APEX ENGINEERS, INC. AN UNSEALED VERSION, OR A VERSION VOID OF APEX ENGINEERS LOGO AND/OR TITLE BLOCK, SHALL BE CONSIDERED AN UNAUTHORIZED REPRODUCTION.

3. WHERE DISCREPENCIES EXIST BETWEEN THE STANDARD COMMENTS, NOTES FROM THE DESIGN PROFESSIONAL OR THE CODE, THE MOST RESTRICTIVE SHALL

APPLY. THE DWELLING SHALL COMP	LY WITH THE FOLLOWIN	NG LOAD CONDITIONS:
AREA	MIN DEAD LOAD	MIN LIVE LOAD
EXTERIOR BALCONIES	10 PSF	60 PSF
DECKS	10 PSF	40 PSF
CEILING JOISTS/ATTICS NO STORAGE - SCUTTLE ACCESS ONLY ROOF SLOPE 3:12 OR LESS	5 PSF	10 PSF
CEILING JOISTS/ATTICS WITHOUT STORAGE - SCUTTLE ACCESS ONLY ROOF SLOPE OVER 3:12 OR LESS	10 PSF	10 PSF
CEILING JOISTS/ATTICS WITH STORAGE - DOOR/PULL DOWN LADDER ACCESS	10 PSF	20 PSF
ROOMS - NON-SLEEPING	10 PSF	40 PSF
ROOMS - SLEEPING	10 PSF	30 PSF
ROOF - LIGHT ROOF COVERING	10 PSF	20 PSF
ROOF - HEAVY ROOF COVERING CONCRETE/TILE/SLATE	20 PSF	20 PSF
NOTE: HEAVY ROOF COVERING WILL	NOT BE INSTALLED OR	USED IN

NOTE: HEAVY ROOF COVERING WILL NOT BE INSTALLED OR USED IN THE DESIGN CALCULATIONS UNLESS IT IS SPECIFICALLY NOTED ON THE PLANS THAT THE DESIGN IS FOR HEAVY ROOF COVERINGS.

FOUNDATIONS

1. THE FOUNDATION DESIGN SHALL BE BASED ON A MINIMUM SOIL BEARING CAPACITY OF 2000 PSF. UNLESS OTHERWISE INDICATED ON THE PLANS OR IF MODIFIED BY AN ENGINEERING REPORT BASED ON ACTUAL SITE CONDITIONS. 2. CONCRETE SHALL MEET THE FOLLOWING SPECIFIED DESIGN STRENGTH

CRITERIA: - 2500 PSI FOR BASEMENT FLOOR SLABS ON UNDISTURBED SOIL - 3000 PSI FOR FOOTINGS AND FOUNDATION WALLS

- 3500 PSI FOR GARAGE FLOOR SLABS 3. FOOTINGS SHALL EXTEND BELOW THE FROST LINE; MINIMUM DEPTH 36 INCHES BELOW GRADE.

4. UNLESS OTHERWISE NOTED ON THE PLANS OR IF SITE CONDITIONS REQUIRE OTHERWISE, FOOTINGS SHALL BE A MINIMUM OF 16" WIDE AND 8" DEEP WITH (2) #4 BARS CONTINUOUS. 5. COLUMN PÁDS SHALL BE A MINIMUM 30"x30"x12" WITH (4) #4 BARS EACH WAY

UNLESS NOTED OTHERWISE. 6. UNLESS NOTED OTHERWISE ON THE PLANS, FOUNDATION WALLS SHALL BE MINIMUM 8" THICK x 8'-0" (OR 9'-0") TALL AND REINFORCED PER DETAIL 1-S2.0 (AND 2-S2.0 WHERE APPLICABLE). FOUNDATION WALLS GREATER THAN 10'-0" TALL REQUIRE A SEPERATE ENGINEERED DESIGN. PROVIDE A 2'-0" LONG

INTERIOR OR EXTERIOR DEAD-MAN FOR ANY STRAIGHT WALL PANELS EXCEEDING 20'-0" IN LENGTH (REF 3-S2.0) 7. REINFORCEMENT SHALL BE MINIMUM GRADE 40 UNLESS NOTED OTHERWISE. REINFORCEMENT SHALL LAP A MINIMUM OF 24" AT ENDS, SPLICES, AND AROUND

CORNERS 8. FOUNDATION WALLS SHALL BE BACKFILLED WITH A CLEAN LEAN CLAY (OR BETTER) LOW VOLUME CHANGE MATERIAL. ON-SITE MATERIAL MAY BE USED IF DEEMED ACCEPTABLE BY THE GEOTECHNICAL ENGINEER OF RECORD. 9. FOUNDATION WALLS WILL NOT ACHIEVE FULL STRENGTH UNTIL THE BASEMENT SLAB AND THE FIRST FLOOR DECK HAVE BEEN PROPERLY PLACED. IF BACKFILLING THE INTERIOR OF THE FOUNDATION WALL WITH GREATER THAN 8" OF EARTHEN FILL OR 24" OF GRANULAR FILL, A STRUCTURAL BASEMENT SLAB, OR ALTERNATE ENGINEERED SOLUTION (i.e. ENGINEERED FILL) WILL BE

REQUIRED. 10. WHERE JUMPS OR STEPS IN ELEVATION OCCUR FOUNDATION WALLS AND FOOTINGS SHALL BE FORMED CONTINUOUS AND POURED PER DETAIL 4-S2.0. 11. CONCRETE FLOOR SLABS SHALL BE A MINIMUM 4" THICK OVER A MINIMUM 4" BASE OF 1/2" OR 3/4" CLEAN GRADED ROCK, UNLESS NOTED OTHERWISE OR IF

SITE CONDITIONS REQUIRE OTHERWISE. 12. PROVIDE A MIN 6 MIL THICK POLYETHYLENE MOISTURE BARRIER OVER POURUS GRAVEL BASE UNDER BASEMENT FLOOR SLAB PER R405.2.2. LAP JOINTS MINIMUM 6" (NOT REQUIRED FOR GARAGE SLABS OR DETACHED

ACCESSORY BUILDINGS). 13. FOR A STRUCTURAL REINFORCED CONCRETE FLOOR OVER A USABLE AREA, SUCH AS A GARGE FLOOR LOCATED OVER A STORAGE AREA, SUBMIT SEALED ENGINEERED DETAILS AND CALCULATIONS. 14. GARAGE SLABS AND BASEMENT OVERDIGS SUPPORTED BY FILL CONSISTING OF MORE THAN 24" OF GRANULAR FILL OR 8" OF EARTH SHALL BE

REINFORCED PER DETAILS 1-S2.1 AND 6-2.1 RESPECTIVELY. WHERE THE LIMITATIONS OF DETAILS 1-S2.1 AND 6-S2.1 ARE NOT MET, A SEPARATE ENGINEERED DESIGN SHALL BE REQUIRED.

15. BASEMENT FOUNDATION SILL PLATES SHALL BE BOLTED TO THE FOUNDATION WITH A MINIMUM OF 1/2" ANCHOR BOLTS EMBEDDED AT LEAST 7" INTO THE CONCRETE AND SPACED NOT MORE THAN 3'-0" ON CENTER AND WITHIN 12" OF EACH END PIECE. 16. FOUNDATION WALLS SHALL BE DAMP-PROOFED PER IRC SECTION R406.

17. PROVIDE A MINIMUM 4" PERFORATED DRAIN AROUND USABLE SPACE BELOW GRADE OR OTHER EQUIVALENT MATERIALS PER IRC SECTION 405.1. THE PIPE SHALL BE PLACED ON A MINIMUM OF 2" OF WASHED GRAVEL OR CRUSHED ROCK AND COVERED WITH NOT LESS THAN 6". THE DRAIN SHALL DAYLIGHT TO THE EXTERIOR BELOW THE FLOOR LEVEL OR TERMINATE IN A MINIMUM 24" DIAMETER OR 20" SQUARE SUMP PIT EXTENDING A MINIMUM 24" BELOW THE BOTTOM OF BASEMENT FLOOR.

18. INTERIOR BEARING WALLS AND COLUMNS SHALL BE ISOLATED FROM THE BASEMENT FLOOR SLAB.

19. INTERIOR NON-BEARING WALLS, OTHER THAN THOSE RESTING DIRECTLY ON THE FOOTING, SHALL BE ISOLATED FROM THE FLOOR FRAMING ABOVE. 20. ALL EARTH RETAINING STRUCTURES ON THE SITE GREATER THAN 4'-0" TALL (EXCLUDING CONCRETE FOUNDATION WALLS RESTRAINED AT BOTH TOP AND BOTTOM) SHALL REQUIRE A SEPARATE ENGINEERED DESIGN (i.e. RETAINING

WALLS, WING WALLS, ETC.). 21. INSULATION SHALL BE INSTALLED FOR ALL BASEMENT WALLS AS REQUIRED PER N1102.2.9.

22. A CONCRETE ENCASED GROUNDING ELECTRODE CONNECTION SHALL BE PROVIDED TO THE ELECTRICAL SERVICES PER E3608.1. 23. ANY GEOTECHNICAL IMPROVEMENT METHODS AND/OR STRUCTURAL SOLUTIONS (SUCH AS DRILLED PIERS) EMPLOYED TO ADDRESS UNACCEPTABLE SUBGRADE CONDITIONS SHALL BE SUBMITTED TO EOR AS ENGINEERED SHOP

EXPANSIVE SOILS DISCLAIMER:

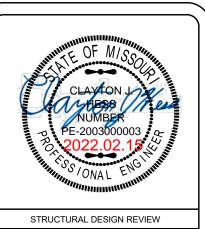
DRAWINGS FOR REVIEW AND APPROVAL

THESE PLANS HAVE BEEN PREPARED BASED ON A PRESUMPTIVE ALLOWABLE BEARING CAPACITY AS ALLOWED BY IRC CODE AND THE LOCAL ENFORCING JURISDICTION.

APEX ENGINEERS, INC. (APEX) RECOMMENDS THAT ALL FOOTING EXCAVATIONS BE EVALUATED BY A LICENSED GEOTECHNICAL ENGINEER PRIOR TO THE PLACEMENT OF ANY FOUNDATION ELEMENTS. GEOTECHNICAL INVESTIGATION AND/OR TESTING IS NOT A SERVICE PROVIDED OR OFFERED BY APEX.

APEX HAS NOT BEEN RETAINED TO DETERMINE THE EXPANSIVE SOIL CHARACTERISTICS OF THE SUBGRADE SOIL AND THEREFORE CANNOT BE HELD RESPONSIBLE FOR THE VOLUMETRIC CHANGES OF THE SOIL (INCLUDING BELOW THE BASEMENT SLAB). BY USE OF THESE PLANS WITHOUT AN ACCOMPANYING GEOTECHNICAL ENGINEERING REPORT, APEX SHALL NOT BE HELD LIABLE FOR ANY FUTURE MOVEMENT AND/OR DIFFERENTIAL MOVEMENT OF THE PROPOSED STRUCTURE AND THE POSSIBLE DAMAGE THAT MAY BE CAUSED AS A RESULT OF SUCH MOVEMENT. DAMAGE FROM EXPANSIVE SOILS AND/OR SETTLEMENT CAN RESULT IN AMONGST OTHER THINGS. THE FOLLOWING: BASEMENT SLAB HEAVE, SHEETROCK CRACKS, WINDOWS AND DOOR BECOMING OUT OF PLUMB AND STICKING AND/OR NOT OPENING, DAMAGE TO TILE, MOULDING, AND OTHER COSMETIC FINISHES.



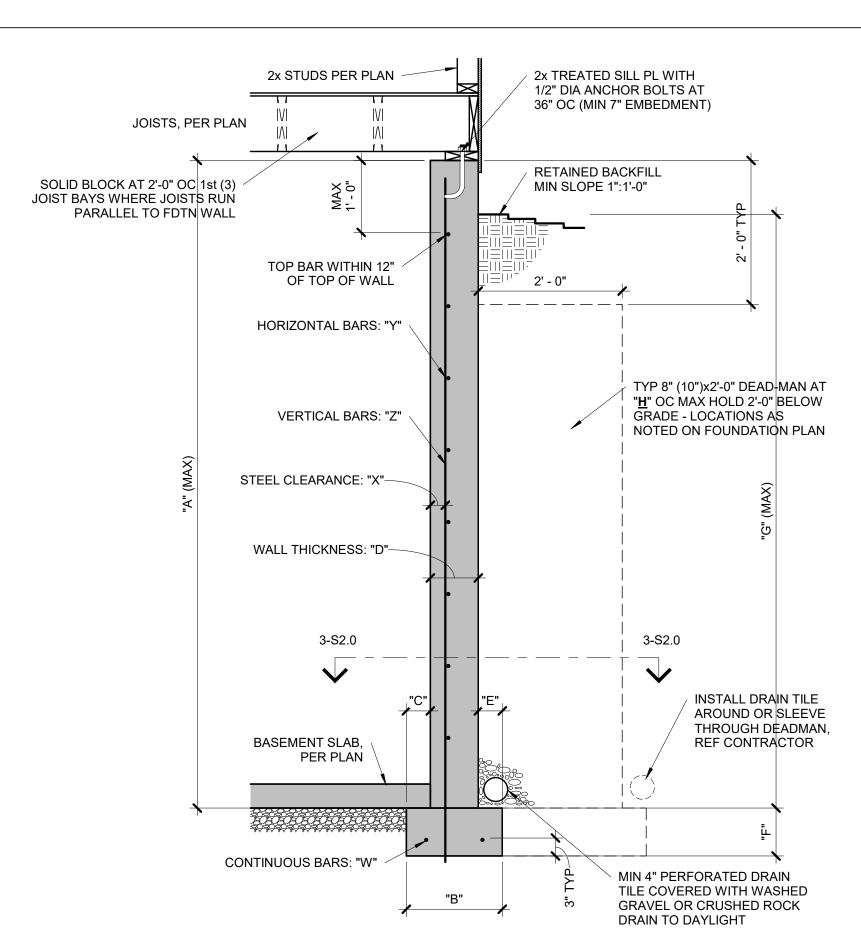


KANSAS ENGINEERING LICENSE MISSOURI ENGINEERING LICENSE: 2003004673

PROJECT #: 41062 DRAWN BY: BCH CHECKED BY BDC SUBMITTAL DATE: 2021.11.17

SHEET:

GENERAL NOTES



	CON	CRE	TE	OIME	ENS	ONS	S	RE	INF	ORCING BAI	RS(GRADE 40 BARS)
"A"	"B"	"C"	"D"	"E"	"F"	"G"	"H" ¹	"W"	"X"	"Y"	"Z"
8'-0"	1'-4"	4"	8"	4"	8"	7'-6"	20'-0"	(2) #4	2 1/2"	#4 BARS AT 24" OC	#4 BARS AT 24" OC
9'-0"	1'-4"	4"	8"	4"	8"	8'-6"	20'-0"	(2) #4	2 1/2"	#4 BARS AT 24" OC	#4 BARS AT 24" OC
10'-0"	1'-8"	5"	10"	5"	10"	9'-6"	20'-0"	(2) #4	2 1/2"	#4 BARS AT 18" OC	#4 BARS AT 18" OC

1. DIMENSION SHOWN IS FOR MAXIMUM UNINTERRUPTED WALL PANEL LENGTH BEFORE A DEAD-MAN SHALL BE INSTALLED. NOTE, A MINIMUM 2'-0" RETURN OR OFFSET IN THE FOUNDATION WALL SHALL SUBSTITUTE AS A DEAD-MAN AND/OR BREAK IN THE WALL PANEL LENGTH. 2. VERTICAL REINFORCING STEEL TO EXTEND TO WITHIN 8" OF TOP WALL. MINIMUM (1) #4 HORIZONTAL BAR WITHIN 12" OF

3. BURIED CONCRETE FOUNDATION WALLS UP TO 9'-0" TALL MAY BE 8" NOMINAL THICKNESS WITH #4 BARS AT 24" OC BOTH WAYS OVER 16"x8" CONCRETE FOOTINGS WITH (2) #4 BARS CONTINUOUS, UNLESS OTHERWISE REQUIRED BY ENGINEERING REPORT BASED ON ACTUAL SITE CONDITIONS. 4. WALL WILL NOT ACHIEVE FULL STRENGTH UNTIL FIRST FLOOR DECK AND BASEMENT SLAB HAVE BEEN PLACED.

TYPICAL FOUNDATION WALL

48"x48"x12"

COLUMN MARK PAD SIZE REINFORCING COL SIZE 30"x30"x12" (4) #4 BARS E-W

36"x36"x12" (4) #4 BARS E-W

42"x42"x12" (5) #4 BARS E-W

54"x54"x16" (8) #4 BARS E-W

60"x60"x16" (10) #4 BARS E-W

1 DETAIL

S2.0 3/4" = 1'-0"

 TYP 8" (10")x2'-0" DEAD-MAN AT "H" OC MAX HOLD 2'-0" BELOW GRADE 1' - 4" (2) #4 VERTICAL BARS EXTEND HORIZONTAL STEEL FROM FOUNDATION WALL INTO DEAD-MAN

1. MIN 3000 PSI FOOTING COMPRESSIVE CONCRETE STRENGTH. 2. MIN 3000 PSI WALL COMPRESSIVE CONCRETE STRENGTH. 3. AIR ENTRAINED BETWEEN 5% & 7% OF CONCRETE VOLUME.

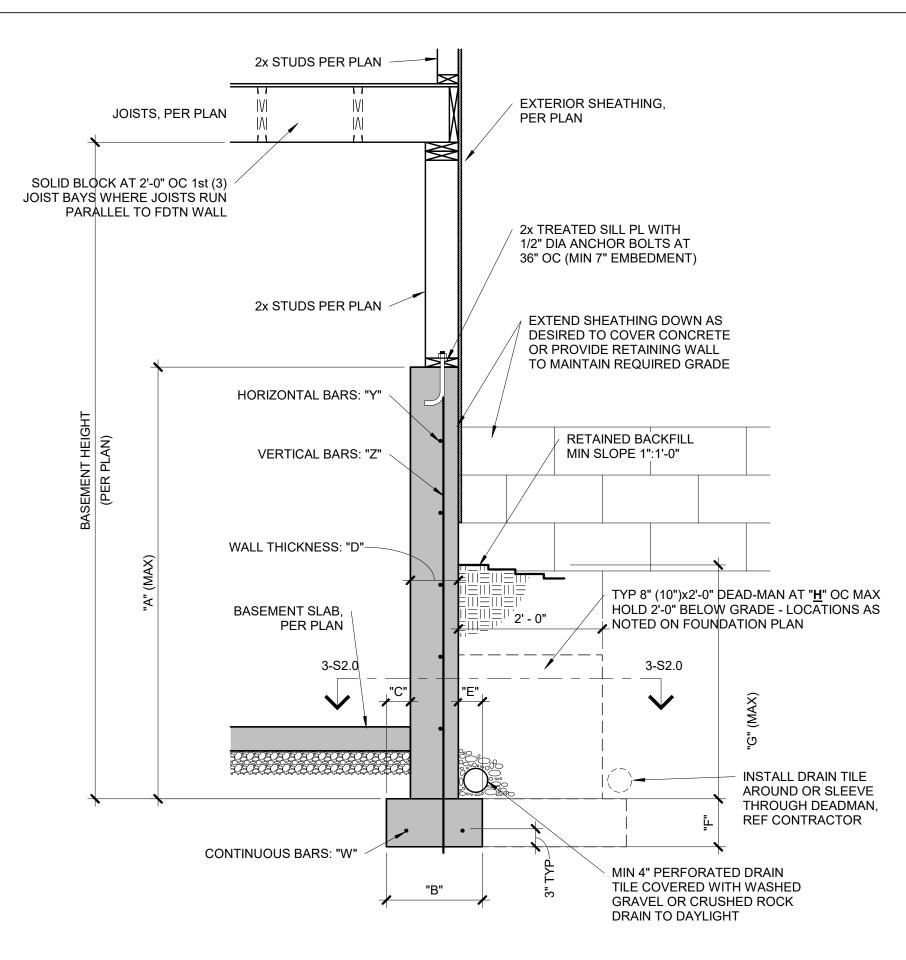
4. GRADE 40 REINFORCING STEEL UNLESS OTHERWISE NOTED. 5. LAP SPLICES 24" MIN.

6. WALL SHALL BE BACK-FILLED WITH CLEAN, LEAN CLAY (OR BETTER) LOW VOLUME CHANGE MATERIAL. ON-SITE MATERIAL MAY BE USED IF DEEMED

ACCEPTABLE BY THE GEOTECHNICAL ENGINEER.
7. ASSUMED 2,000 PSF BEARING (TO BE VERIFIED BY GEOTECHNICAL ENGINEER).

3 TYPICAL DEAD-MAN SECTION

S2.0 3/4" = 1'-0"



	CON	CRE	TEI	DIME	ENS	ONS	S	RE	INF	ORCING BAI	RS(GRADE 40 BARS
"A"	"B"	"C"	"D"	"E"	"F"	"G"	"H" ¹	"W"	"X"	"Y"	"Z"
4'-0"	1'-4"	4"	8"	4"	8"	3'-4"	20'-0"	(2) #4	N/A	#4 BARS AT 24" OC	#4 BARS AT 24" OC
6'-0"	1'-4"	4"	8"	4"	8"	4'-4"	20'-0"	(2) #4	N/A	#4 BARS AT 24" OC	#4 BARS AT 24" OC
9'-0"	1'-8"	5"	8"	4"	8"	4'-4"	20'-0"	(2) #4	N/A	#4 BARS AT 24" OC	#4 BARS AT 24" OC

S2.0 3/4" = 1'-0"

RE	INF	ORCING BA	RS (GRADE 40 BARS
"W"	"X"	"Y"	"Z"
(2) #4	N/A	#4 BARS AT 24" OC	#4 BARS AT 24" OC
(2) #4	N/A	#4 BARS AT 24" OC	#4 BARS AT 24" OC

1. DIMENSION SHOWN IS FOR MAXIMUM UNINTERRUPTED WALL PANEL LENGTH BEFORE A DEAD-MAN SHALL BE INSTALLED. NOTE, A MINIMUM 2'-0" RETURN OR OFFSET IN THE FOUNDATION WALL SHALL SUBSTITUTE AS A DEAD-MAN AND/OR BREAK IN THE WALL PANEL LENGTH.

2. VERTICAL REINFORCING STEEL TO EXTEND TO WITHIN 8" OF TOP WALL. MINIMUM (1) #4 HORIZONTAL BAR WITHIN 12" OF TOP AND BOTTOM OF WALL.

3. THE BASEMENT SLAB IS AN INTEGRAL PART OF THE 'UNRESTRAINED' FOUNDATION WALL DESIGN THEREFORE, IF THE WALL IS BACKFILLED PRIOR TO PLACEMENT OF THE BASEMENT SLAB, THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROPERLY BRACING THE WALL UNTIL THE BASEMENT SLAB HAS BEEN PLACED.

TYPICAL 'UNRESTRAINED' 2 FOUNDATION WALL DETAIL

COLUMN AND PIER PAD SCHEDULE

1. COLUMN AND PIER PAD SIZES SHOWN ARE FOR MAXIMUM COLUMN HEIGHT OF 10'-0", REQUIRES SEPERATE ENGINEERED DESIGN IF GREATER THAN 10'-0" 2. COLUMN AND PIER PAD SIZES SHOWN ARE BASED ON AN ASSUMED MINIMUM ALLOWABLE SOIL BEARING CAPACITY OF 2,000 PSF.

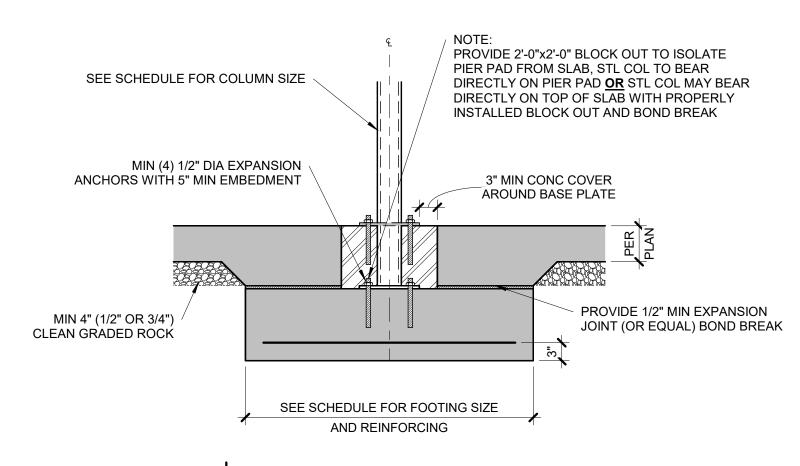
(6) #4 BARS E-W

3" NOMINAL

3" NOMINAL

3" NOMINAL

3 1/2" NOMINAL



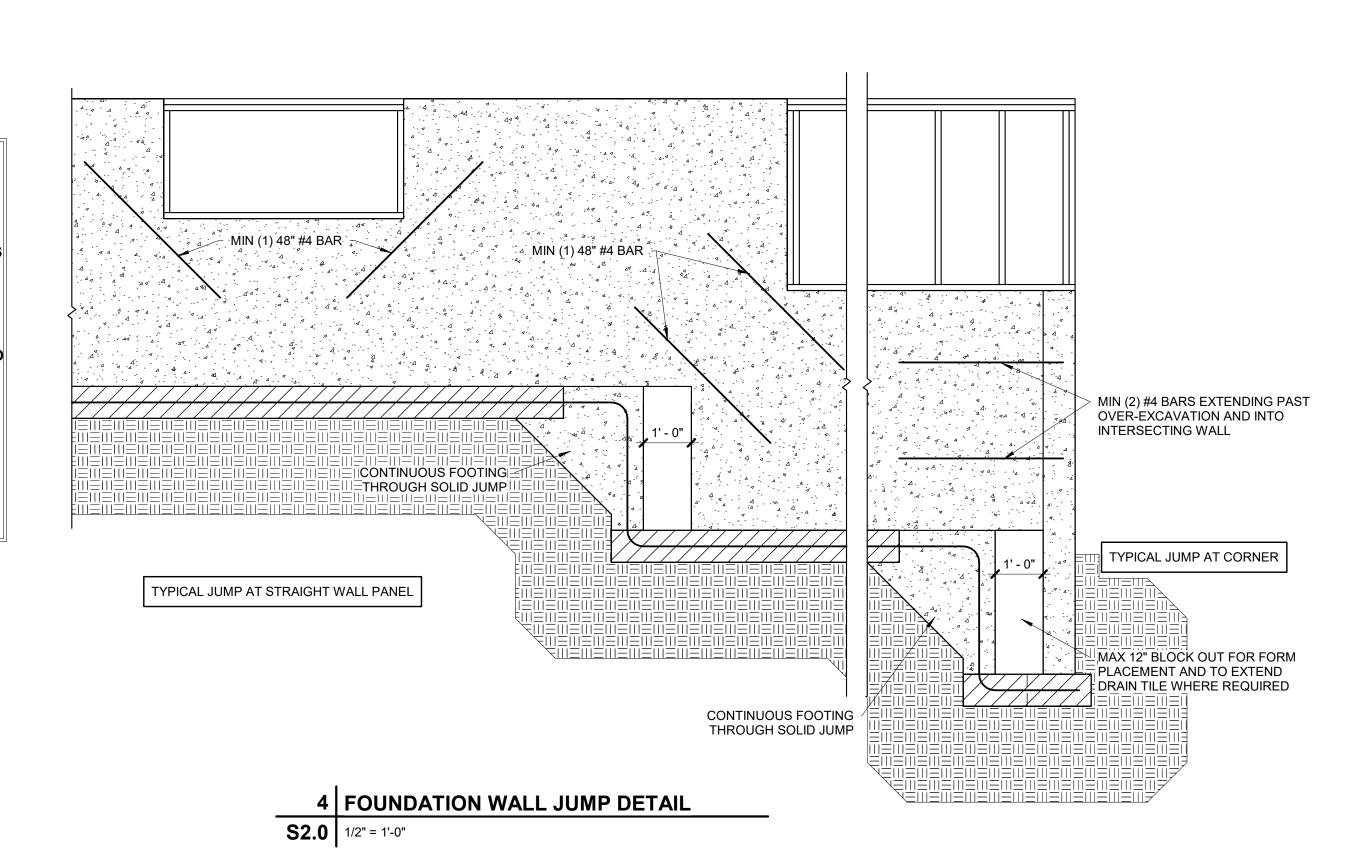
5 COLUMN PAD DETAIL **\$2.0** 3/4" = 1'-0"

EXPANSIVE SOILS DISCLAIMER:

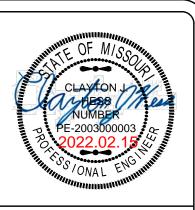
THESE PLANS HAVE BEEN PREPARED BASED ON A PRESUMPTIVE ALLOWABLE BEARING CAPACITY AS ALLOWED BY IRC CODE AND THE LOCAL ENFORCING JURISDICTION.

APEX ENGINEERS, INC. (APEX) RECOMMENDS THAT ALL FOOTING EXCAVATIONS BE EVALUATED BY A LICENSED GEOTECHNICAL ENGINEER PRIOR TO THE PLACEMENT OF ANY FOUNDATION ELEMENTS. GEOTECHNICAL INVESTIGATION AND/OR TESTING IS NOT A SERVICE PROVIDED OR OFFERED BY APEX.

APEX HAS NOT BEEN RETAINED TO DETERMINE THE EXPANSIVE SOIL CHARACTERISTICS OF THE SUBGRADE SOIL AND THEREFORE CANNOT BE HELD RESPONSIBLE FOR THE VOLUMETRIC CHANGES OF THE SOIL (INCLUDING BELOW THE BASEMENT SLAB). BY USE OF THESE PLANS WITHOUT AN ACCOMPANYING GEOTECHNICAL ENGINEERING REPORT, APEX SHALL NOT BE HELD LIABLE FOR ANY FUTURE MOVEMENT AND/OR DIFFERENTIAL MOVEMENT OF THE PROPOSED STRUCTURE AND THE POSSIBLE DAMAGE THAT MAY BE CAUSED AS A RESULT OF SUCH MOVEMENT. DAMAGE FROM EXPANSIVE SOILS AND/OR SETTLEMENT CAN RESULT IN AMONGST OTHER THINGS, THE FOLLOWING: BASEMENT SLAB HEAVE, SHEETROCK CRACKS, WINDOWS AND DOOR BECOMING OUT OF PLUMB AND STICKING AND/OR NOT OPENING, DAMAGE TO TILE, MOULDING, AND OTHER COSMETIC FINISHES.



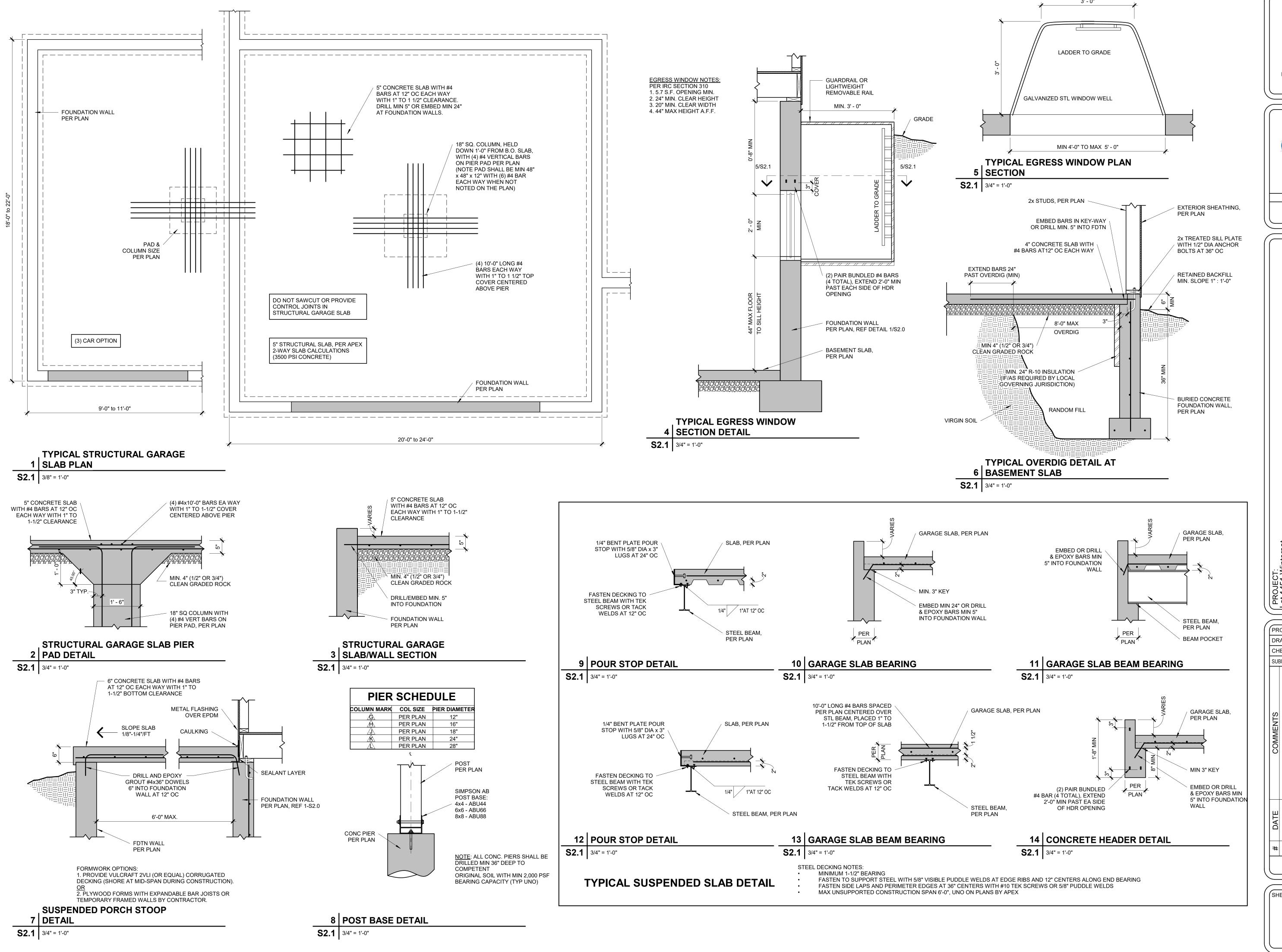
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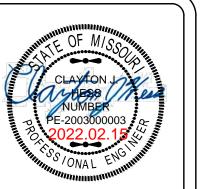
STRUCTURAL DESIGN REVIEW KANSAS ENGINEERING LICENSE: MISSOURI ENGINEERING LICENSE: 2003004673

PROJECT #: 41062 DRAWN BY: BCH **CHECKED BY** BDC SUBMITTAL DATE: 2021.11.17

SHEET: FOUNDATION DETAILS



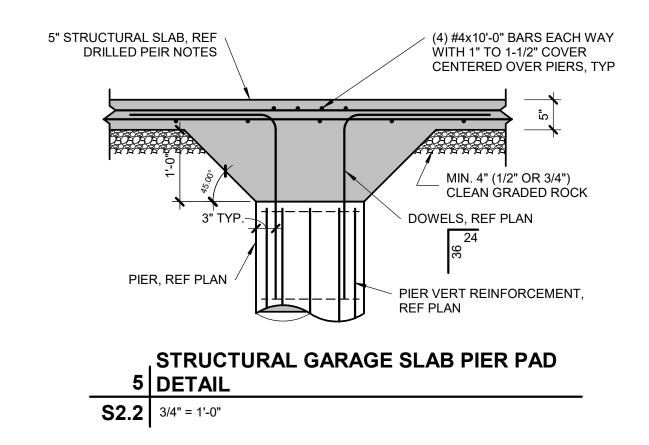
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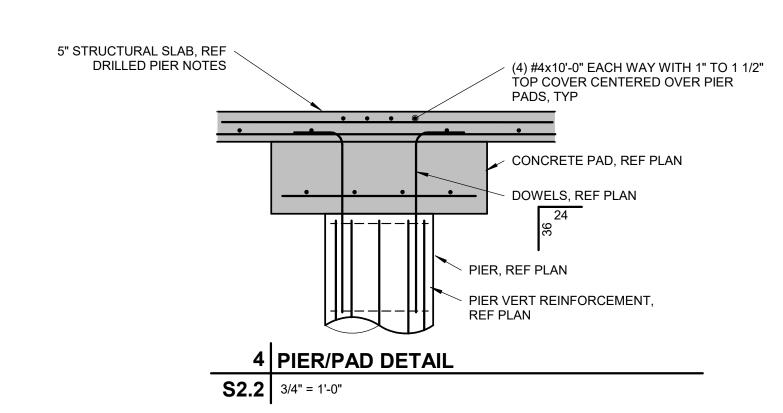


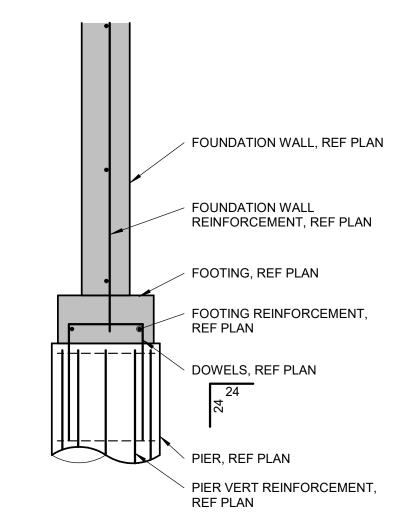
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PROJECT #: 41062 DRAWN BY: BCH **CHECKED BY** BDC 2021.11.17 SUBMITTAL DATE:

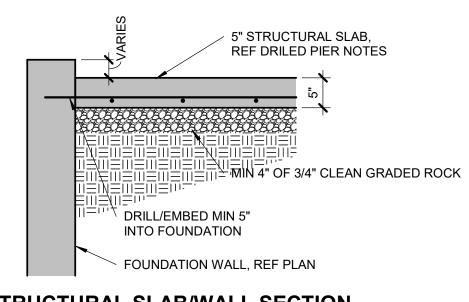
SHEET: FOUNDATION DETAILS



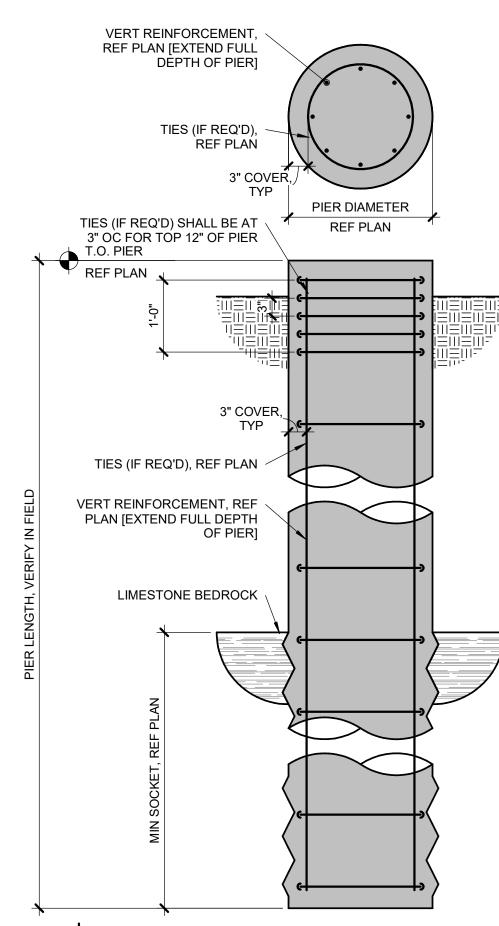




3 PIER/FOOTING DETAIL **S2.2** 3/4" = 1'-0"



2 STRUCTURAL SLAB/WALL SECTION



1 TYPICAL DRILLED PIER

\$2.2 NO SCALE

THIS DETAIL IS TYPICAL TO THE PROJECT AND MAY NOT BE CUT OR CALLED OUT ON PLANS

DRILLED PIER NOTES:

1. THIS DRILLED PIER PLAN IS **PRELIMINARY, NOT FOR CONSTRUCTION** AND FOR ESTIMATING PURPOSES ONLY. PRIOR TO CONSTRUCTION, APEX ENGINEERS SHALL BE CONTACTED TO PERFORM A SITE OBSERVATION AFTER EXCAVATION OF PROPOSED SITE AND PRIOR TO PIERS BEING DRILLED AND PLACED. THIS PIER PLAN IS PRELIMINARY AND IS INTENDED TO PROVIDE EQUIVALENT BEARING FOR THE STRUCTURE'S IMPOSED GRAVITY LOADS. NO MEASURES HAVE BEEN TAKEN TO RESIST UPLIFT DUE TO THE EFFECTS OF EXPANSIVE SOIL, LATERAL RESTRAINT DUE TO SITE STABILITY, OR OTHER UNFORESEEN CIRCUMSTANCES. APEX ENGINEERS SHALL PERFORM A SITE OBSERVATION AND RESERVES THE RIGHT TO RECOMMEND CONSULTING A LICENSED GEOTECHNICAL ENGINEER TO EXAMINE THE SITE IF EVIDENCE OF EXPANSIVE SOIL, SITE SLOPE STABILITY OR ANY OTHER ISSUES ARE PREVALENT AT THE SITE. THE FINDINGS FROM THE SITE OBSERVATION PERFORMED BY APEX COULD LEAD TO ADDITIONAL DESIGN CONSIDERATIONS AND/OR MORE STRINGENT DESIGN RECOMMENDATIONS. THIS DRILLED PIER PLAN IS PRELIMINARY, NOT FOR CONSTRUCTION AND FOR ESTIMATING <u>PURPOSES ONLY</u> UNTIL SITE OBSERVATION APPROVAL REPORT IS

ISSUED BY APEX ENGINEERS. 2. REFERENCE THE DRILLED PIER PLAN FOR THE DIAMETER AND LOCATION OF ALL PIERS REQUIRED. 3. PIERS SHALL BE DRILLED TO END BEARING ON LIMESTONE,

SANDSTONE OR SHALE BEDROCK WITH A MIN 15KSF ALLOWABLE BEARING CAPACITY, PER GEOTECH. 4. ALL PIER HOLES SHALL BE INSPECTED TO BE CLEAR OF SPOILS, DEBRIS AND EXCESS WATER FOR ENTIRE DEPTH. 5. UNLESS NOTED ON PLAN OR SCHEDULE, ALL PIERS SHALL BE REINFORCED WITH A MINIMUM OF THE FOLLOWING: (2) #4

LONGITUDINAL BARS FOR THE ENTIRE DEPTH. BEND AND DOWEL (4) # 4 X 4'-0" BARS FROM TOP OF EACH PIER TO TIE INTO THE FOUNDATION. PROPER LAP SPLICE LENGTHS SHALL BE USED. REFERENCE DEEP FOUNDATION DETAILS. 6. ALL PIERS SHALL BE INSPECTED BY THE ENGINEER OF RECORD (APEX ENGINEERS) OR GEOTECHNICAL ENGINEER OF RECORD PRIOR

TO PLACEMENT OF CONCRETE. UPON COMPLETION AND APPROVAL OF THE PIERS AND FOOTINGS THE FOUNDATION WALLS MAY BE PLACED PER PERMIT APPROVED DRAWINGS, UNLESS OTHERWISE DICTATED BY SUPPLEMENTAL STRUCTURAL RECOMMENDATIONS. 7. ALL SLABS SHALL BE STRUCTURAL. FOR THE BASEMENT THE FOLLOWING DESIGN SHALL BE USED.

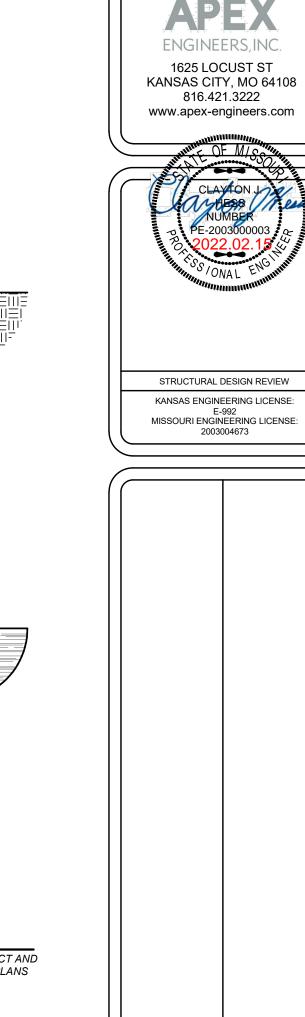
a. PLACE 5" THICK CONCRETE SLAB WITH #4 BARS AT 12" OC EACH WAY ON 1 1/2" CHAIRS.

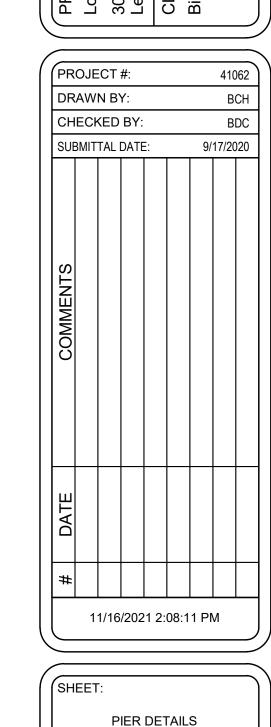
b. ADD (4) 10'-0" LONG #4 BARS EACH WAY OVER THE COLUMN PADS AND SLAB SUPPORT PIERS. PLACE WITH 1" TO 1 1/2" SLAB TOP COVER (3" CHAIRS). c.THE PERIMETER OF THE SLAB SHALL BEAR ON THE FOUNDATION AS FOLLOWS: IF A MINIMUM OF 3" OF BEARING IS PROVIDED ON A KEYWAY OR FOOTING, THEN THE SLAB DOES NOT NEED TO BE PINNED TO THE WALL. OTHERWISE, DRILL 5" DEEP AND PIN THE SLAB TO THE FOUNDATION WALL WITH #4 BARS AT 12" OC.

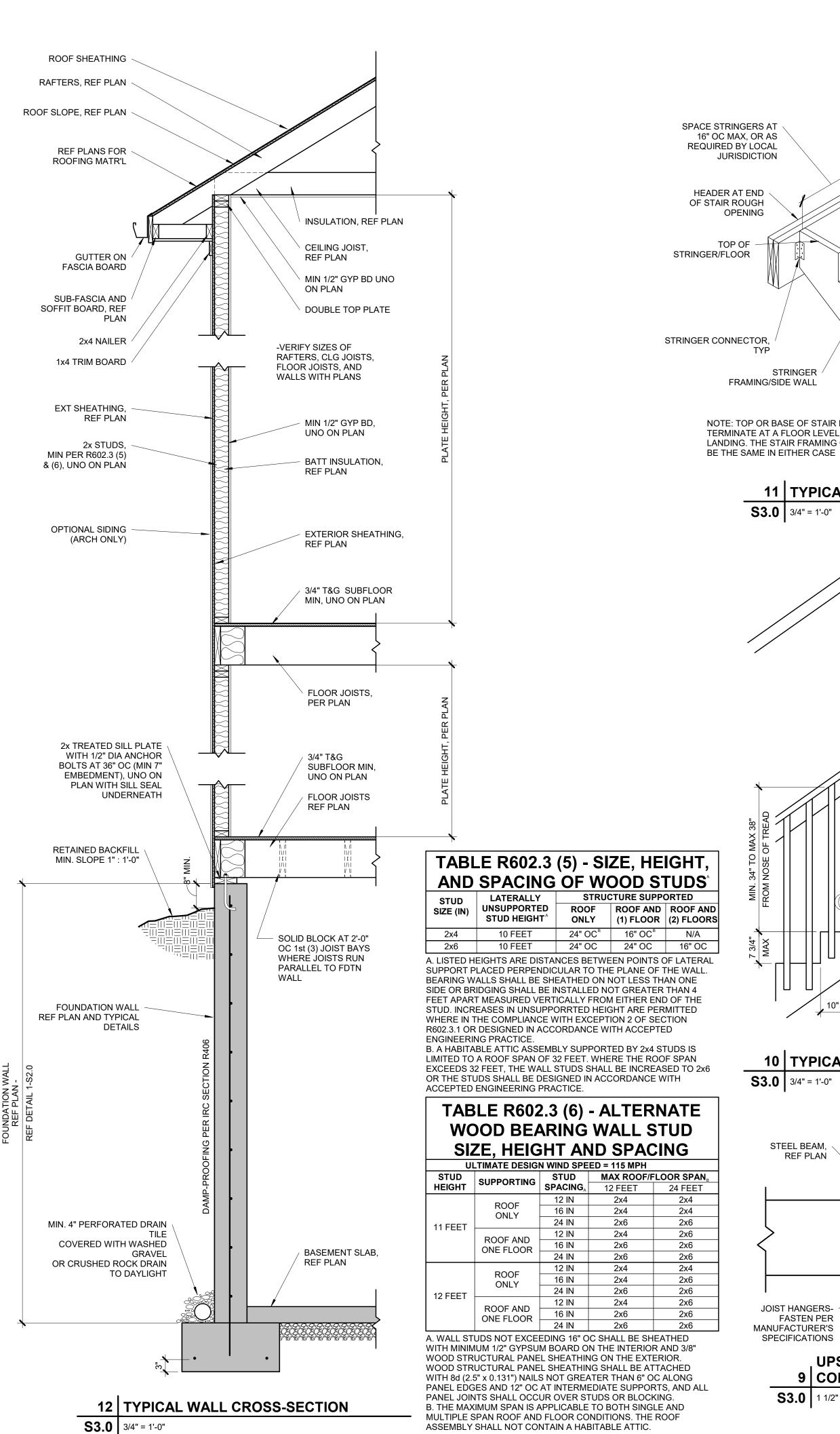
d. DO NOT SAW CUT STRUCTURAL SLABS UNLESS SPECIFICALLY INDICATED TO DO SO ON THE STRUCTURAL SLAB PLAN. e. PROVIDE (2) #4 X 4'-0" DIAGONAL BARS AT MID-DEPTH OF SLAB AT ALL RE-ENTRANT CORNERS.

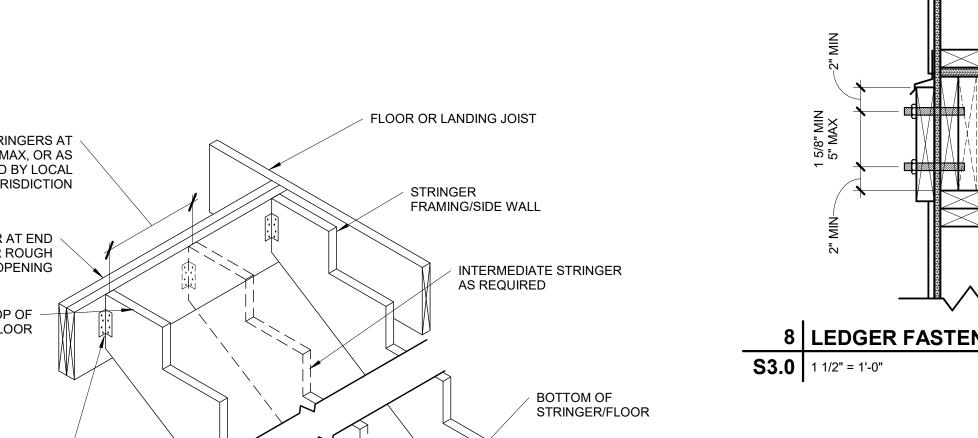
8. MIN 3000 PSI CONCRETE FOR PIERS. MIN 4000 PSI CONCRETE FOR STRUCTURAL SLAB. 9. #4 AND SMALLER BARS, MIN GRADE 40. #5 AND LARGER BARS, MIN

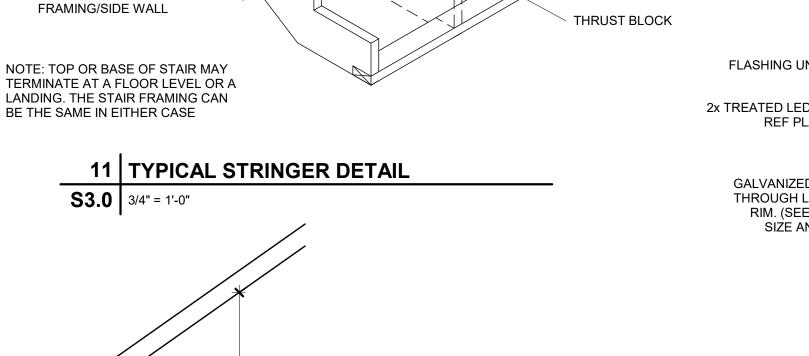
GRADE 60. MIN 24" LAP SPLICES. 10. REFERENCE PIER FOUNDATION DETAILS FOR MORE INFORMATION. 11. CONTRACTOR TO FIELD VERIFY ALL FOUNDATION ELEVATIONS AND STEP LOCATIONS PER SITE CONDITIONS. 12. REFER TO GEOTECH REPORT FOR ALL ADDITIONAL INFORMATION AND REQUIREMENTS.

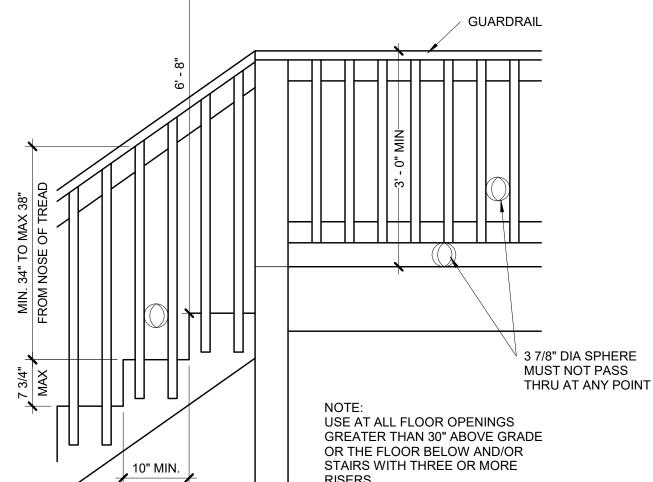




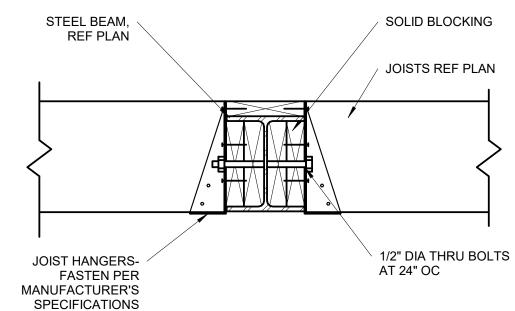




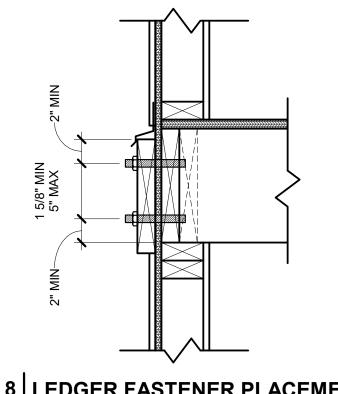




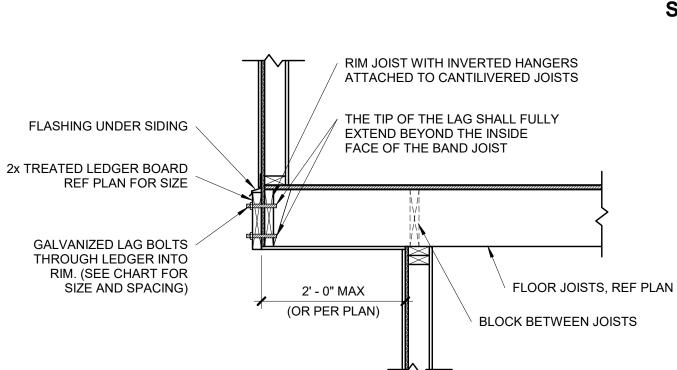
10 TYPICAL STAIR/RAIL DETAIL **S3.0** 3/4" = 1'-0"



UPSET STEEL BEAM/JOIST 9 CONNECTION **S3.0** 1 1/2" = 1'-0"

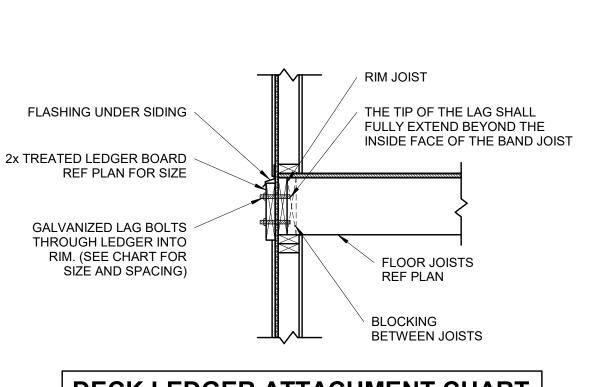


LEDGER FASTENER PLACEMENT



TYPICAL CANTILEVER FRAMING 7 WITH DECK ATTACHMENT

S3.0 3/4" = 1'-0"



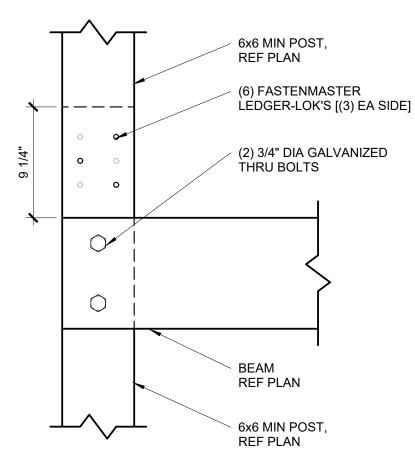
DECK JOIST SPAN	1/2" DIA LAG SPACING	EQUIVALENT SPACING FOR 16" OC JOIST BAYS				
UP TO 10'-0"	16" OC	N/A				
10'-1" TO 12'-0"	15" OC	16" OC DBL EVERY OTHER				
12'-1" TO 14'-0"	13" OC	16" OC DBL EVERY OTHER				
14'-1" TO 16'-0"	11" OC	16" OC DBL EVERY JOIST BAY				
16'-1" TO 18'-0" 10" OC 16" OC DBL EVERY JOIST BAY						

6 TYPICAL LEDGER ATTACHMENT **S3.0** 3/4" = 1'-0"

	BEAM SIZE	"X"	*DISTANCE SHALL BE PERMITTED TO BE
	2x8*	5 1/2" MIN	REDUCED TO 4 1/2" IF LAG SCREWS
	2x10	6 1/2" MIN	ARE USED OR BOLT SPACING IS
	2x12	7 1/2" MIN	REDUCED TO THAT OF LAG SCREWS TO ATTACH 2x8 LEDGERS TO 2x8 BAND
			JOISTS
LAG SCREW OR	o LEDGER, PLAN FOR		STAGGER FASTENERS IN 2 ROWS

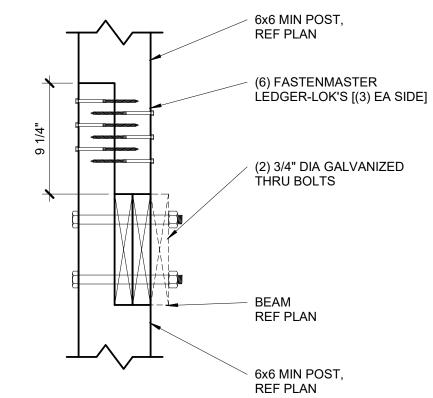
5 TYPICAL LEDGER BOLT SPACING

S3.0 3/4" = 1'-0"



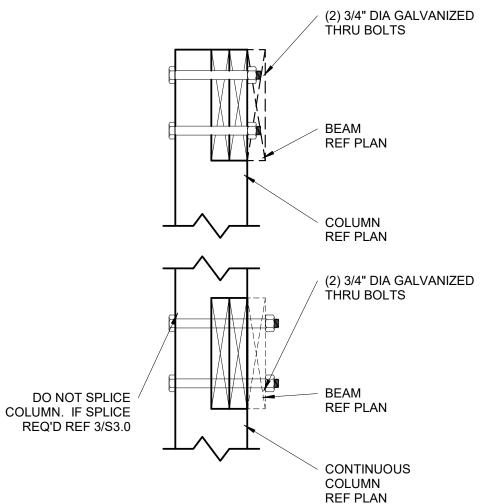
SPLICED DECK COLUMN

4 CONNECTION **S3.0** 1 1/2" = 1'-0"

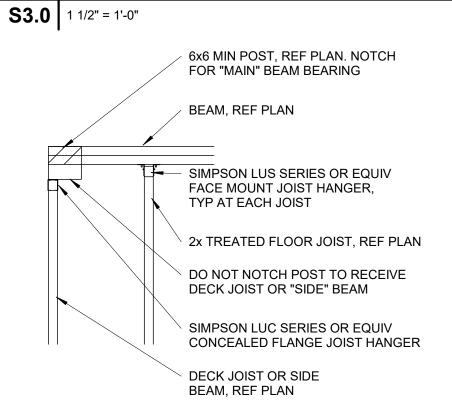


SPLICED DECK COLUMN 3 CONNECTION

S3.0 1 1/2" = 1'-0"



DECK BEAM/COLUMN 2 CONNECTION

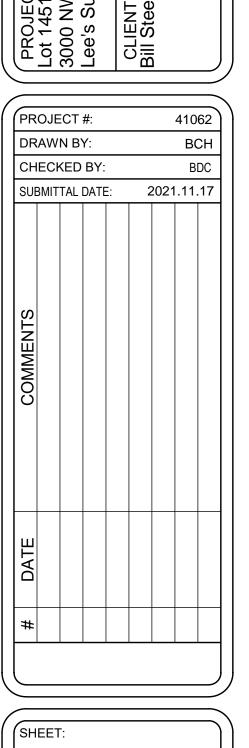


DECK BEAM/COLUMN CORNER 1 CONDITION

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FRAMING DETAILS

RIDGE BEAM,

RAFTERS

PER PLAN

PER PLAN

12 RIDGE BEAM DETAIL

11 TYPICAL WOOD HEADER DETAIL

3 - PLY

(3) ROWS OF 16d x (3) ROWS OF 16d x (2) ROWS OF 1/2' DIA.

3-1/2" NAILS AT 6" 3-1/2" NAILS AT 4" A307 THRU-BOLTS AT

NAILING SHOWN APPLIES UNLESS SPECIFICALLY NOTED

SPACE NAILS EVENLY THROUGHOUT DEPTH OF BEAM.

MULTIPLE PLY BEAM NAILING

S3.1 3/4" = 1'-0"

2x PLATE T&B, UNO

FASTEN PLATE TO EACH

VERT MEMBER WITH 10d

NAILS AT 16" OC UNO

MEMBERS, REF PLAN

2x PACKOUT AT EACH END

THROUGHOUT HEADER SPAN

FASTEN VERT MEMBERS TO

PACKOUT WITH (3) 10d NAILS

HEADER VERT

AND 4'-0" OC MAX

HEADERS WITH GREATER THAN 1"
GAP BETWEEN VERT MEMBERS

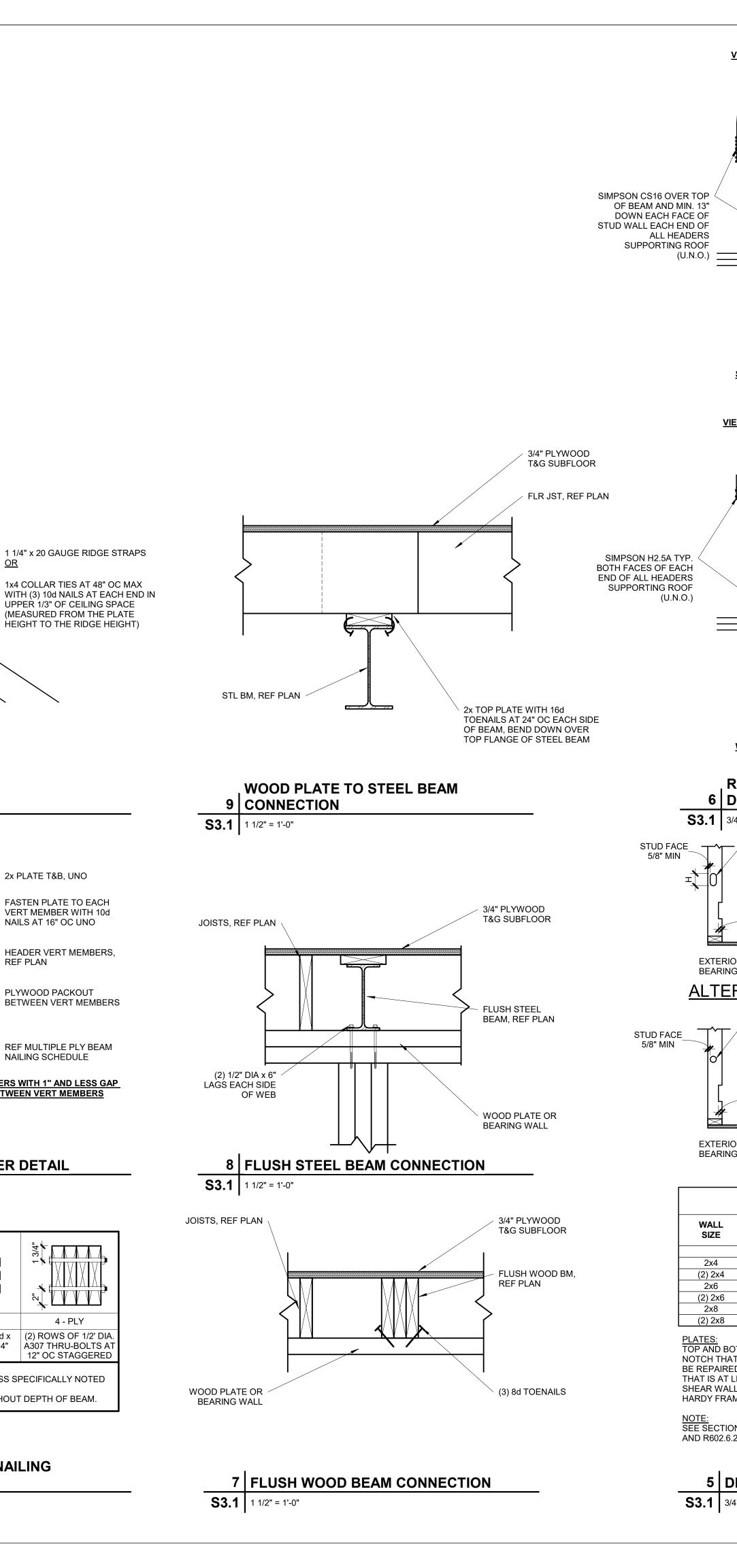
S3.1 NOT TO SCALE

2 - PLY

10 SCHEDULE

S3.1 NOT TO SCALE

IN DETAILS.



1 1/4" x 20 GAUGE RIDGE STRAPS

1x4 COLLAR TIES AT 48" OC MAX

UPPER 1/3" OF CEILING SPACE

(MEASURED FROM THE PLATE

2x PLATE T&B, UNO

FASTEN PLATE TO EACH

VERT MEMBER WITH 10d

HEADER VERT MEMBERS,

BETWEEN VERT MEMBERS

REF MULTIPLE PLY BEAM

4 - PLY

12" OC STAGGERED

NAILING SCHEDULE

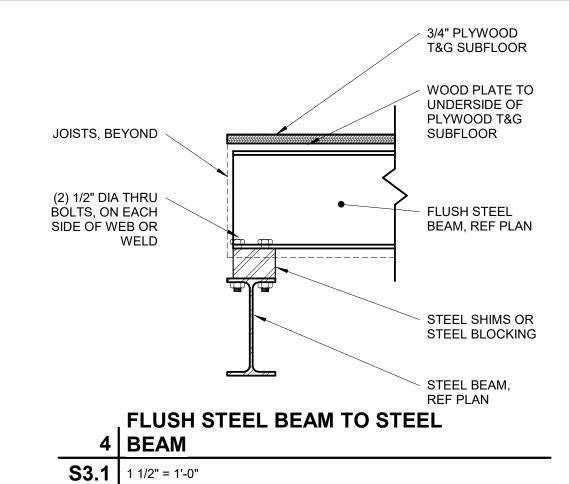
HEADERS WITH 1" AND LESS GAP BETWEEN VERT MEMBERS

NAILS AT 16" OC UNO

PLYWOOD PACKOUT

REF PLAN

HEIGHT TO THE RIDGE HEIGHT)



COPE WHERE APPLICABLE 1/4" DOUBLE ANGLE OR SHEAR TAB NUMBER OF BOLTS "M" INTO GIRDER GIRDER PER PLAN

3 BEAM TO GIRDER CONNECTION

STEEL BEAM, PER PLAN

NUMBER OF BOLTS

"N" INTO BEAM

S3.1 1 1/2" = 1'-0"

BOLTED CONNECTION.

4. ALL BOLTS, 3/4" DIAMETER, A325-N, UNC

VIEW A-A

STEEL BEAM TYP. DROPPED

HEADER

HEADER

NON-BEARING

NON-BEARING PARTITION

1 3/8"

1 3/8"

1 13/16"

1 13/16"

WALL NOTCH

WALL BEARING WALL

1 3/8"

1 3/8"

2 1/4"

2 1/4"

2 7/8"

2 7/8"

VERTICAL

HOLE SIZE (H)

D+1/2" AT Lvls 1&2

D+1" AT Lvl 3

D+1 1/4" AT LvI 4

D+1 1/2" AT Lvl 5

PARTITION

STUD FACE 5/8" MIN

BORED HOLES DIA

STUD FACE

5/8" MIN

BORED HOLES DIA

60% OF STUD WIDTH MAX

NOTCH 40% OF STUD

WIDTH MAX

60% OF STUD WIDTH MAX

NOTCH 40% OF STUD

WIDTH MAX

VIEW A-A

SIMPSON CS16 OVER

TOP OF BEAM AND MIN.

13" DOWN EACH FACE

OF STUD WALL EACH

END OF ALL HEADERS

SUPPORTING ROOF

SIMPSON H2.5A TYP. BOTH FACES OF EACH

END OF ALL HEADERS

SUPPORTING ROOF

(U.N.O.)

(U.N.O.)

 $A \rightarrow$

VIEW B-B

STEEL BEAM TYP. UPSET

HEADER

HEADER

BORED HOLES DIA 40% OF STUD WIDTH, 60% OF STUD WIDTH

BORED HOLES DIA 40% OF STUD WIDTH, 60% OF STUD WIDTH

NOTCH 25% OF STUD

WIDTH MAX

EXTERIOR WALL

2 1/4"

2 7/8"

NOTCH THAT IS 50% MORE OF WIDTH MUST

BE REPAIRED USING 16 GA (MIN) METAL TIE

SHEAR WALL IT MUST BE REPAIRED USING

NOTE: SEE SECTION R602.6 AND FIGURES R602.6.1

HARDY FRAME SADDLE (HFS).

THAT IS AT LEAST 1-1/2" WIDE IF WALL IS A

<u>PLATES:</u> TOP AND BOTTOM PLATE HOLE, CUT OR

IF DOUBLE STUD

NOTCH 25% OF STUD

WIDTH MAX

ALTERNATE FOR OBLONG BORED HOLES

PENETRATIONS THRU STUDS

STUDS LOAD BEARING OR NON LOAD LOAD BEARING NON LOAD

2 1/8"

3 15/16"

3 15/16"

4 3/8"

4 3/8"

WALL | HOLE |

2x8 3 5/8"

SIZE SIZE

BEARING WALL

BORED HOLE SIZE

2 1/8"

3 5/16"

4 3/8"

5 DRILLING & NOTCHING DETAIL

IF DOUBLE STUD

6 DOWN

EXTERIOR OR **BEARING WALL**

EXTERIOR OR

BEARING WALL

SIZE

(2) 2x4

2x6

(2) 2x6

(2) 2x8

AND R602.6.2

S3.1 3/4" = 1'-0"

ROOF SUPPORTING BEAM HOLD

VIEW B-B

ALL HEADERS

(U.N.O.)

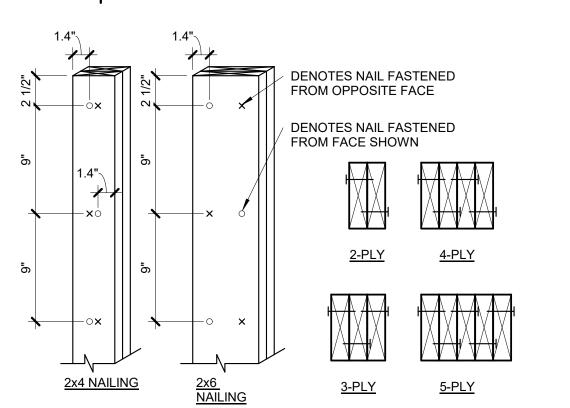
(U.N.O.)

BEAM CONNECTION SCHEDULE					
BEAM SIZE	# OF BOLTS "N"	# OF BOLTS "M"			
W8, W10	2	4			
W12, W14	3	6			
W16, W18	4	8			
2. NUMBER OF BOLTS SMALLER OF TWO BE	NS ARE TYPICAL, UNO. IN UPSET BEAM CONNECT AMS AT CONNECTION. LLET WELD MAY BE SUBST				

3/4" DIA. BOLTS 1/2" STEEL PLATE "N" (SEE CHART) (EACH SIDE) STEEL BEAM, STEEL BEAM, PER PLAN PER PLAN SEE PLAN STEEL COLUMN (PER PLAN)

2 BEAM SPLICE DETAIL

S3.1 1 1/2" = 1'-0"



EACH 2x PLY SHALL BE FASTENED WITH (1) ROW OF 10d NAILS AT 9" OC, ALTERNATING SIDE TO SIDE

1.4" MIN EDGE DISTANCE, AND STARTING 2 1/2" FROM EACH END.

EXTEND FULL AREA OF COLUMN AS SOLID BLOCKING THROUGH JOIST BAYS AND WALLS TO LOAD-BERAING BEAM/WALL BELOW

1 BUILT-UP STUD COLUMN

S3.1 1 1/2" = 1'-0"

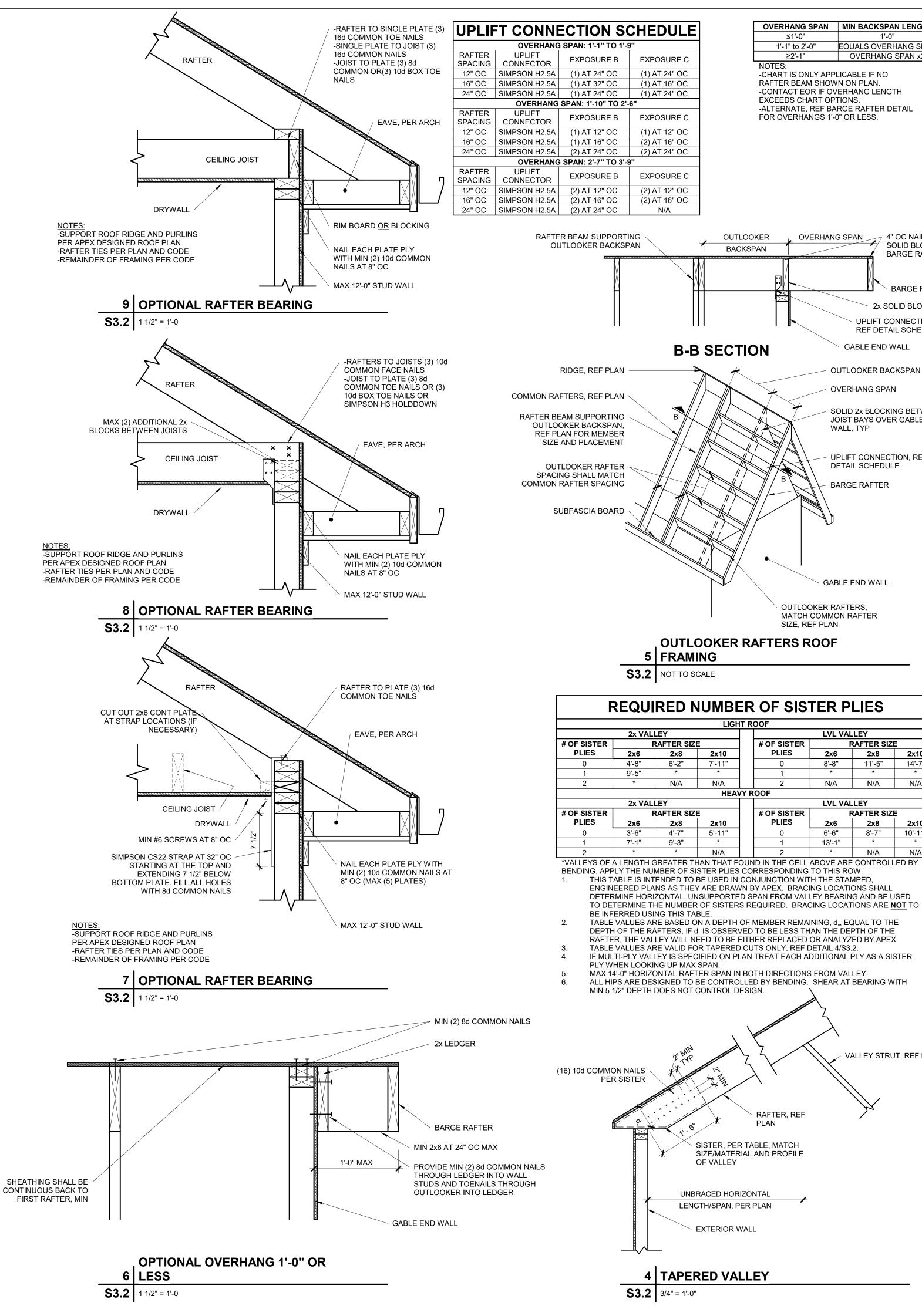
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STRUCTURAL DESIGN REVIEW KANSAS ENGINEERING LICENSE: E-992 MISSOURI ENGINEERING LICENSE: 2003004673

PROJECT: Lot 1451 Winterset 3000 NW Audubon I Lee's Summit, MO 6 PROJECT #: 41062 DRAWN BY: BCH CHECKED BY: BDC 2021.11.17 SUBMITTAL DATE:

SHEET: FRAMING DETAILS



OVERHANG SPAN | MIN BACKSPAN LENGTH 1'-1" to 2'-0" **EQUALS OVERHANG SPAN** ≥2'-1" OVERHANG SPAN x2

RAFTER BEAM SHOWN ON PLAN. -CONTACT EOR IF OVERHANG LENGTH EXCEEDS CHART OPTIONS. FOR OVERHANGS 1'-0" OR LESS.

OVERHANG SPAN

OUTLOOKER

BACKSPAN

-CHART IS ONLY APPLICABLE IF NO -ALTERNATE, REF BARGE RAFTER DETAIL

4" OC NAIL SPACING AT

BARGE RAFTER

BARGE RAFTER

2x SOLID BLOCKING

UPLIFT CONNECTION, REF DETAIL SCHEDULE

GABLE END WALL

OUTLOOKER BACKSPAN

SOLID 2x BLOCKING BETWEEN

JOIST BAYS OVER GABLE END

UPLIFT CONNECTION, REF

DETAIL SCHEDULE

BARGE RAFTER

GABLE END WALL

LVL VALLEY

LVL VALLEY

13'-1"

RAFTER SIZE

2x6 2x8 2x10

8'-8" | 11'-5" | 14'-7"

N/A N/A N/A

RAFTER SIZE

6'-6" 8'-7"

2x6 2x8 2x10

N/A N/A

VALLEY STRUT, REF PLAN

OUTLOOKER RAFTERS,

SIZE, REF PLAN

LIGHT ROOF

HEAVY ROOF

N/A

OF SISTER

PLIES

OF SISTER

RAFTER, REF PLAN

SISTER, PER TABLE, MATCH

SIZE/MATERIAL AND PROFILE

OF VALLEY

EXTERIOR WALL

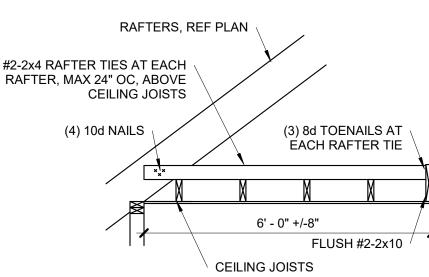
PLIES

MATCH COMMON RAFTER

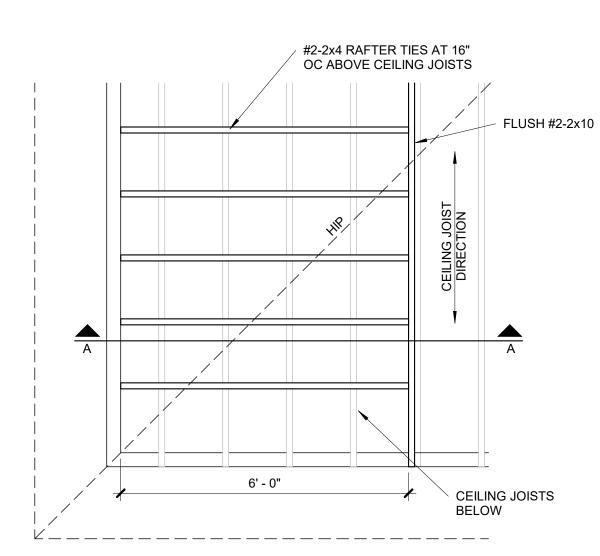
OVERHANG SPAN

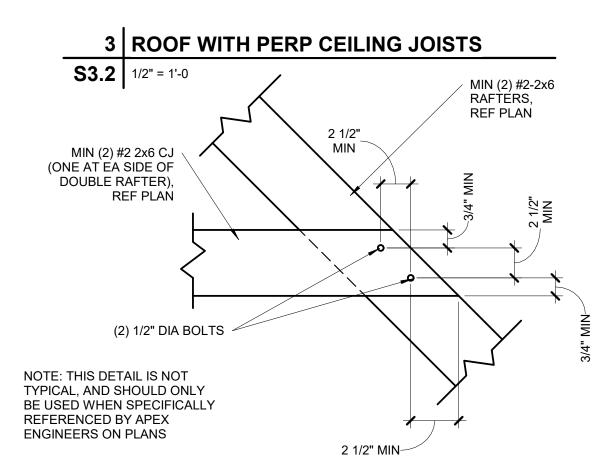
WALL, TYP

SOLID BLOCKING AND



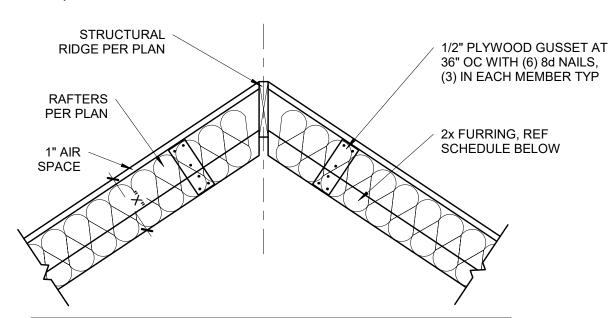
A-A SECTION





BOLTED RAFTER HIP 2 CONNECTION

S3.2 1 1/2" = 1'-0"



FURR OUT SCHEDULE					
RAFTER SIZE	R-30C INSULATION (X= 9 1/4")	R-38C INSULATION (X=11 1/4")			
2x6	2x6	2x8			
2x8	2x4	2x6			
2x10	NOT REQUIRED	2x4			
2x12	NOT REQUIRED	REQUIRED			
SPAN CHART, 2. ALL VAULTS REQUIRED DE 3. R-30C INSUL 4. R-38C INSUL 5. INSULATION	ED RAFTERS SHALL BE #2-2x6 D UNLESS NOTED OTHERWISE. IS SHALL BE FURRED DOWN WIT PTH OF INSULATION, PLUS 1" A LATION = 8 1/4" THICK LATION = 10 1/4" THICK I REQUIREMENTS MAY BE RED IS ASSEMBLY DOES NOT ALLOW	TH 2x FRAMING TO THE NIR SPACE. UCED TO R30 IF			

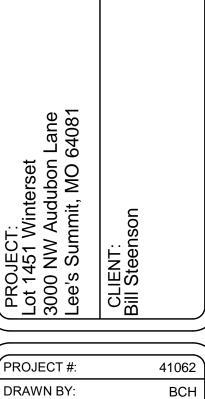
LIMITED TO VAULTED CEILING AREAS THAT ARE LESS THAN 500 SQUARE FEET OR 20 PERCENT OF THE TOTAL INSULATED CEILING AREA. WHICHEVER IS LESS. (PER N1102.2.2)

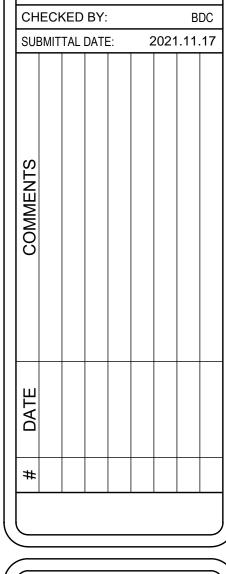
VAULTED RAFTER INSULATION 1 FURR OUT **S3.2** 3/4" = 1'-0"

ENGINEERS, INC 1625 LOCUST ST KANSAS CITY, MO 64108 816.421.3222 www.apex-engineers.com

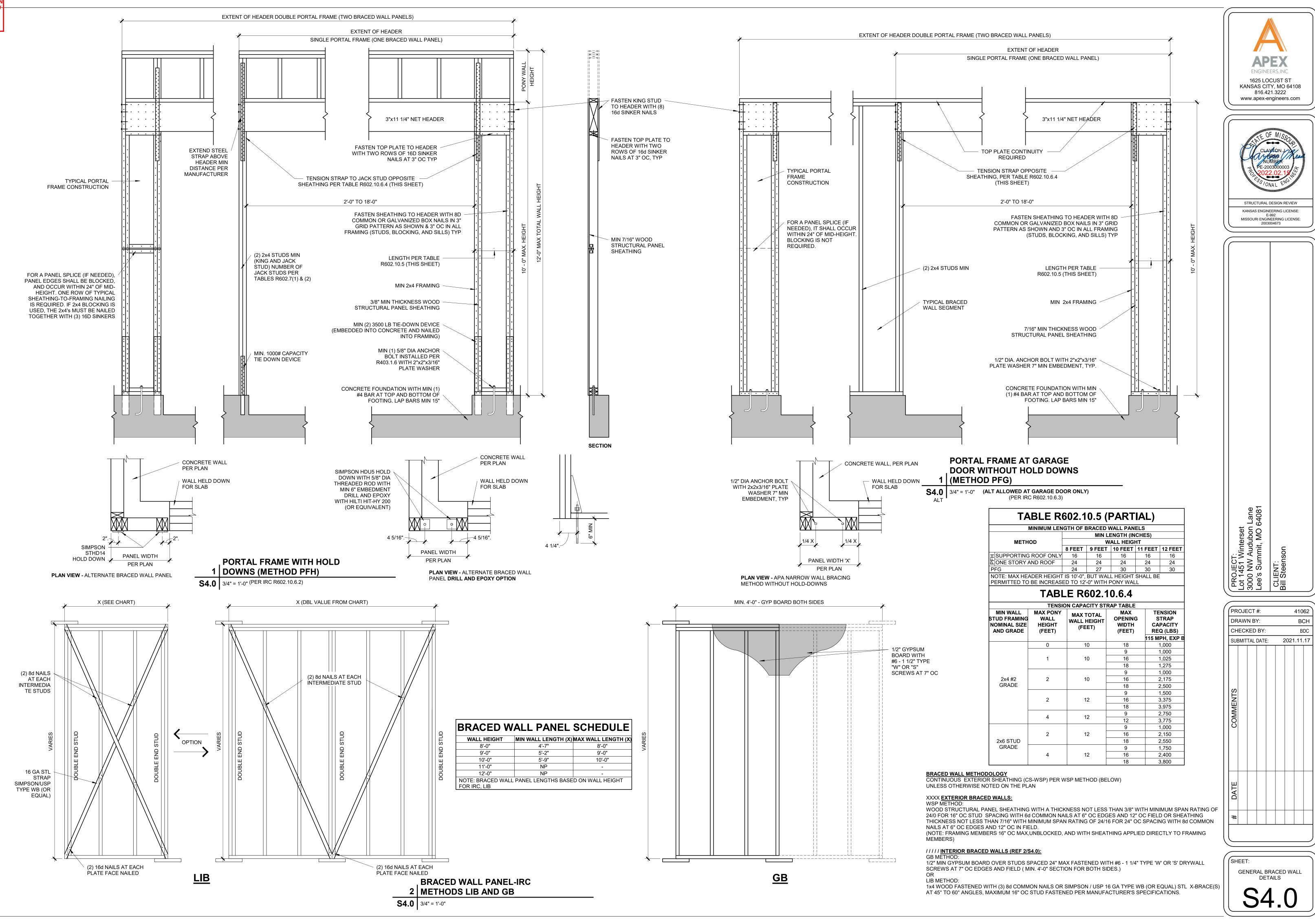


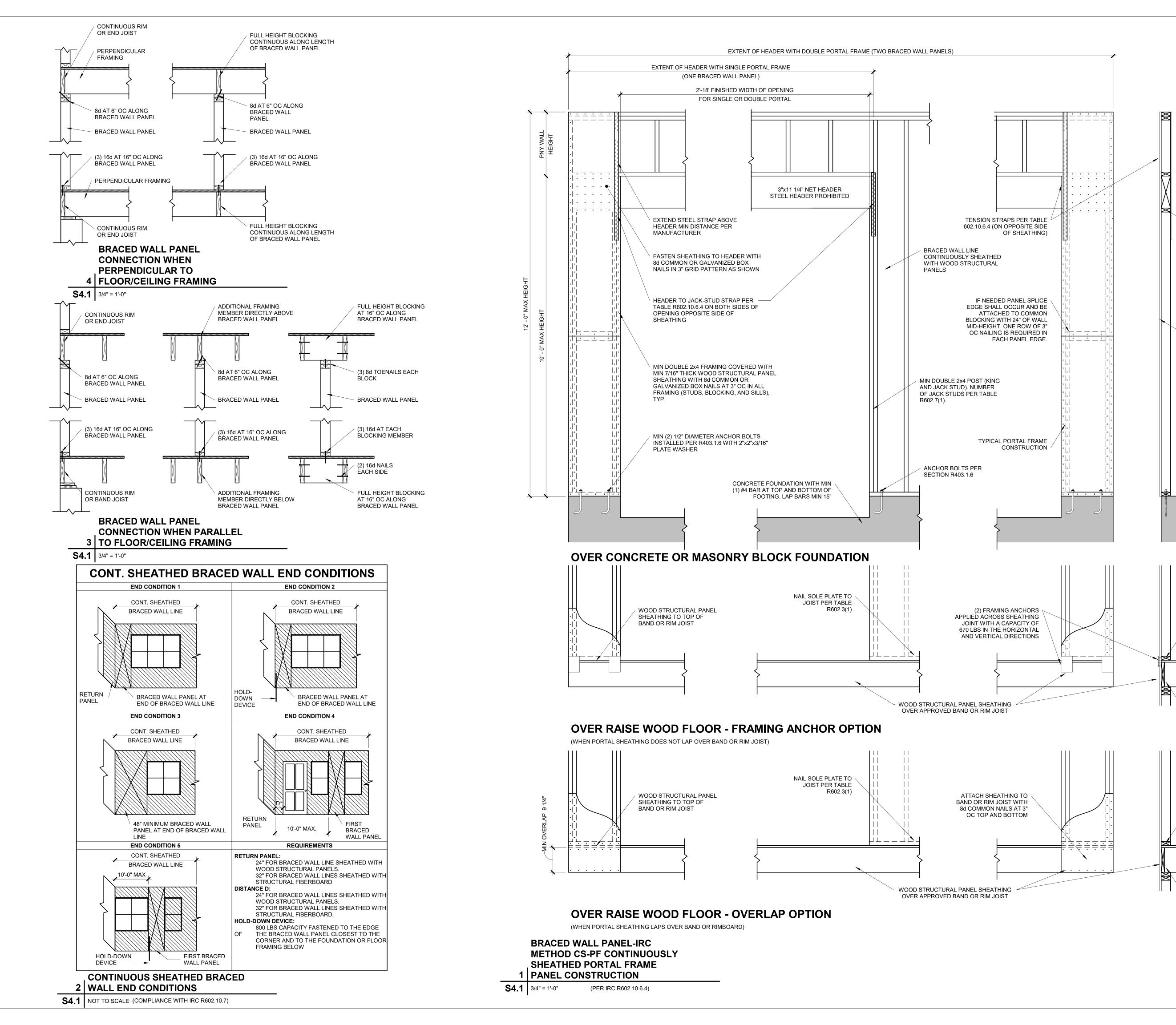
STRUCTURAL DESIGN REVIEW KANSAS ENGINEERING LICENSE: MISSOURI ENGINEERING LICENSE: 2003004673



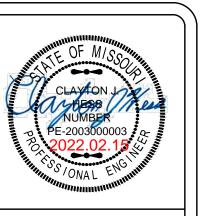


SHEET: FRAMING DETAILS





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STRUCTURAL DESIGN REVIEW

KANSAS ENGINEERING LICENSE:
E-992
MISSOURI ENGINEERING LICENSE:
2003004673

FASTEN TOP PLATE TO

HEADER WITH TWO ROWS OF

MIN 7/16" WOOD STRUCTURAL

PANEL SHEATHING

NAIL SOLE PLATE TO JOIST

APPROVED BAND OR RIM

NAIL SOLE PLATE TO JOIST

APPROVED BAND OR RIM

PER TABLE R602.3(1)

JOIST

PER TABLE R602.3(1)

16d SINKER NAILS AT 3" OC,

PROJECT:
Lot 1451 Winterset
3000 NW Audubon Lane
Lee's Summit, MO 64081
CLIENT:

DRAWN BY: BCH
CHECKED BY: BDC
SUBMITTAL DATE: 2021.11.17

STATE OF THE PROJECT #: 41062

DRAWN BY: BCH
CHECKED BY: BDC
SUBMITTAL DATE: 2021.11.17

##

SHEET:

GENERAL BRACED WALL

DETAILS

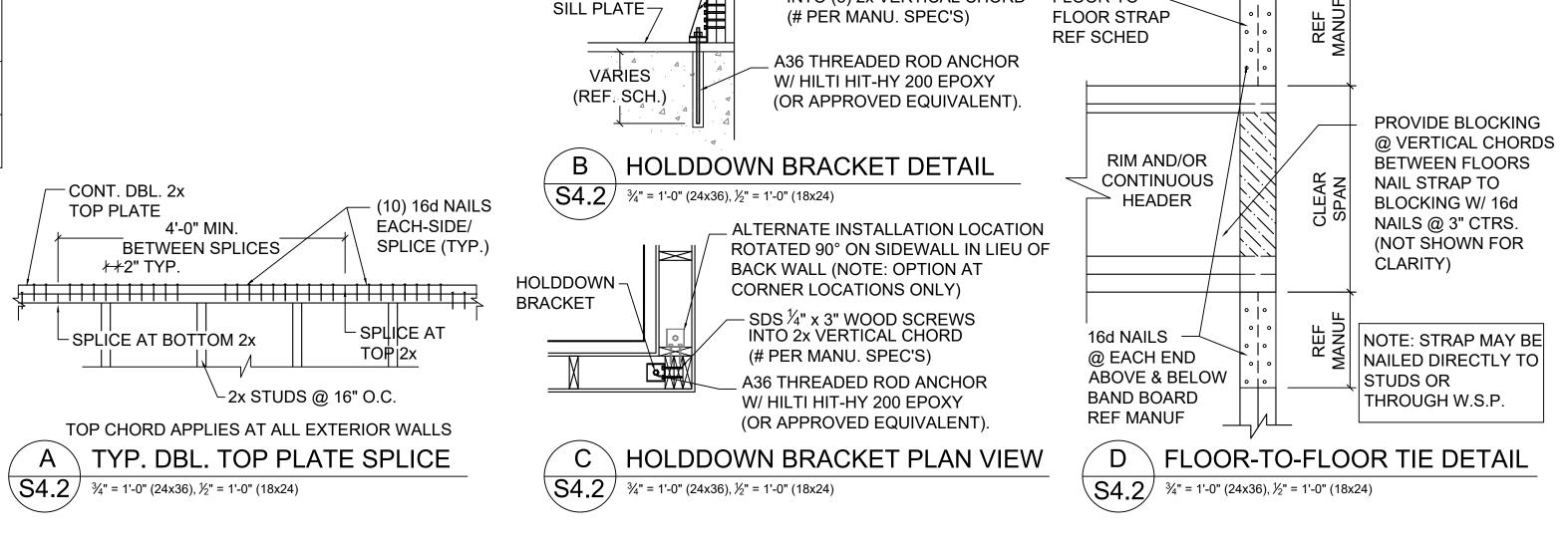
S4.

ENGINEERED BRACED WALL PANEL SCHEDULE BLOCKING **EDGE** INTERMEDIATE SOLE PLATE TIES BETWEEN HOLD DOWN ALL-THREAD DIA & SHEATHING PANEL MARK **FLOOR** REQUIRED NAILING NAILING NAILING/FASTENING **FLOORS** EMBEDMENT DEPTH DEVICE 7/8" DIA WITH 8" SIMPSON HDU8 BASEMENT ⁷∕₁₆" WSP ½" DIA AT 18" OC 8d AT 3" 8d AT 12" N/A **EMBEDMENT** S4.2/ 7/₁₆" WSP (2) 16d / LINEAL FT SIMPSON CMSTC16 N/A N/A 1st FLOOR YES 8d AT 3" 8d AT 12"

NOTE:

NOTES:

- 1. HOLDDOWN ANCHORS ARE TO BE INSTALLED USING HILTI HIT-HY 200 EPOXY (OR APPROVED EQUAL).
- 2. THREADED ROD SHALL BE A36 (OR APPROVED EQUAL) WITH EMBEDMENT DEPTH PER SCHEDULE. 3. USE ALL WOOD SCREWS AND/OR LAG SCREWS IN HOLDDOWN DEVICES AS SPECIFIED BY MANUFACTURER.
- 4. SILL ANCHORS MAY BE CAST-IN-PLACE A307 BOLTS OR EXPANSION BOLTS.
- 5. W.S.P. = WOOD STRUCTURAL PANEL PLYWOOD OR OSB. SEE GENERAL NOTES.

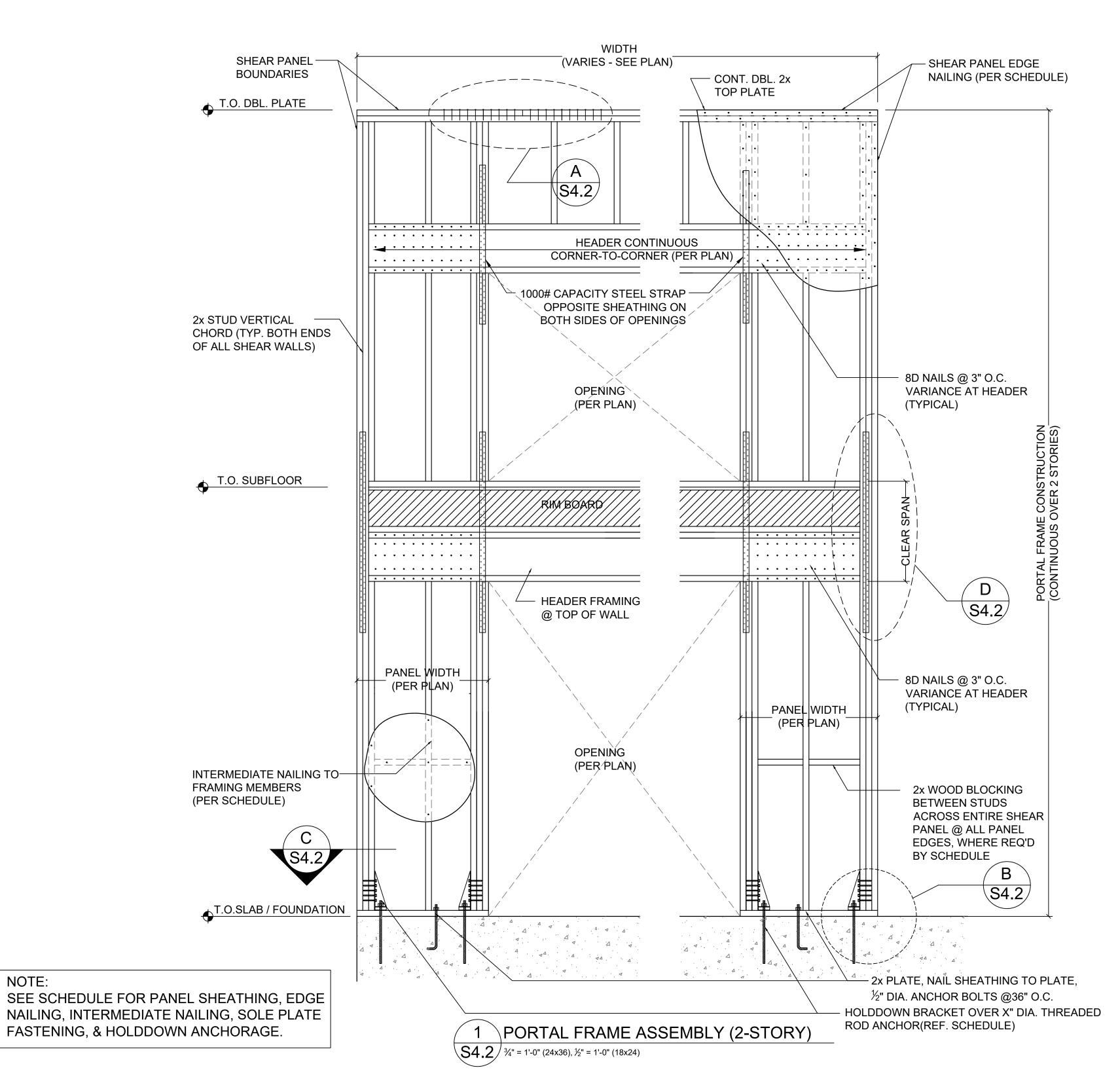


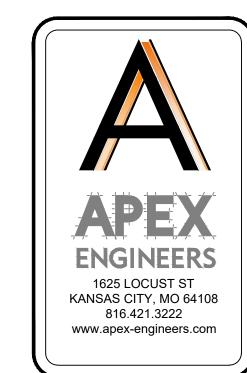
HOLDDOWN (REF. SCHEDULE)

 \sim SDS $\frac{1}{4}$ " x 3" WOOD SCREWS

INTO (3) 2x VERTICAL CHORD

FLOOR-TO-

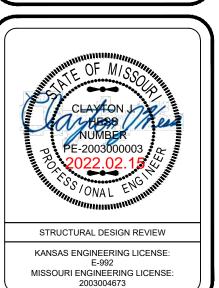


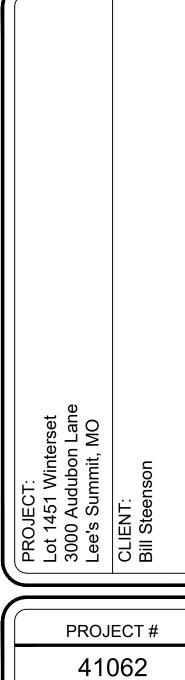


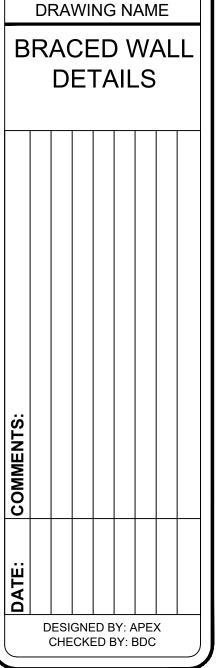
- 2x BUILT-UP

CHORD (TYP.)

VERTICAL







SHEET# **S4.2**