GENERAL NOTES - STRUCTURAL

1. General Information

A. The contractor shall verify dimensions and conditions before construction and notify the engineer of any discrepancies, inconsistencies, or difficulties affecting the work before proceeding.

B. The contractor shall coordinate all disciplines, verifying size and location of all openings, whether shown on structural drawings or not, as called for on architectural, mechanical, or electrical drawings. In the case of work in an existing building the contractor shall scan existing structure to locate all rebar in the area of the new core/opening using ground penetrating radar and notify the engineer of record for review prior to coring/cutting. Conflicts, inconsistencies, or other difficulties affecting structural work shall be called to the architect or engineer's attention for direction before proceeding.

C. All design and construction work for this project shall conform to the requirements of the following governing design codes:
1.) International Building Code (IBC **2018**) as amended by the city of Lee's Summit, Missouri
2.) Minimum Design Loads for Buildings and Other Structures (ASCE7-**16**)
3.) Specification for Structural Steel Buildings (AISC 360-**16**)
Member Design Basis is Allowable Stress Design (ASD)
Connection Design Basis is Allowable Stress Design (ASD)
4.) Structural Welding Code (AWS D1.3-98)
5.) Building Code Requirements for Structural Concrete (ACI 318-**14**)
6.) Building Code Requirements for Masonry Structures (TMS 402-**14**)
7.) North American Specification for the Design of Cold-Formed Steel Structural Members (AISI S100-16/S1-1)
8.) National Design Specification (NDS) for Wood Constriction with 2015 Supplements (ANSI/AWC NDS-2015)
9.) Special Design Provisions for Wind and Seismic (AWC SDPWS-2015)

D. These drawings are for this specific project and no other use is authorized.

2. Structural Load Design Criteria

A. Dead Load:

Deck Floors	= 35 psf
Apartment Floors	= 35 psf
Roofs	= 20 psf
Stairs	= 40 psf

B. Live Load:

Public Rooms	= 100 psf
Stairs	= 100 psf
Apartment Floors (Private Rooms)	= 40 psf
Corridors	= 100 psf
Storage Areas	= 125 psf
Decks/Balconies (Private)	= 60 psf
Decks/Balconies (Public)	= 100 psf
Roofs	= 20 psf

C. Snow:
Pg = 20 psf, Ce = 1.0
Pf = 14 psf, Pm = 20 psf
Is = 1.0, Cs = 1.0, Ct = 1.0
Drift & unbalanced snow loads per ASCE/SEI 7-10

D. Lateral Loads:
1.) Wind V(ult) = 109 mph, Exposure B, GCpi = +/- 0.18
Design wind pressures to be used for the design of exterior components and cladding material son the designated zones of walls and roof structures shall be per section 30.7 and table 30.7-2 of ASCE/SEI 7-16. Tabulated pressures shall be multiplied by effective are reduction factors, exposure adjustment factors, and topographic factors where applicable.
2.) Seismic: Ss = 0.099, S1 = 0.068, Ie = 1.0
Sds=0.086; Sd1=0.068; Site Classification C
Seismic Design Category B
Basic Seismic Force-Resisting System:
A.17- Light-Framed Walls with Shear Panels of All Other Materials
R=2, Omega = 2 1/2, Cd = 2, V=0.043W

E. This project is designed to resist the most critical effects resulting from the load combinations of section 1605.3 of the 2018 International Building Code.

3. Concrete

A. All concrete for foundations (walls, grade beams, footings and piers) shall develop minimum ultimate compressive design strength of 3500 psi in 28 days, but not less than 500 pounds of cement shall be used per cubic yard of concrete regardless of strengths obtained, not over 6 gallons of water per 100 pounds of cement and not over 4 inches of slump.

B. All concrete for garage slab on grade shall develop minimum ultimate compressive design strength of 4000 psi in 28 days, but not less than 525 pounds of cement shall be used per cubic yard of concrete regardless of strengths obtained, not over 5.75 gallons of water per 100 pounds of cement and not over 4 inches of slump. Concrete mix shop drawing shall contain testing data proving concrete design mix shrinkage is less than 0.034% at 28 days when tested according to ASTM C157 (air drying method only).

C. All concrete for interior flatwork (except garage slab on grade) shall develop minimum ultimate compressive design strength of 4000 psi in 28 days, but not less than 540 pounds of cement shall be used per cubic yard of concrete regardless of strengths obtained, not over 5.40 gallons of water per 100 pounds of cement and not over 4 inches of slump. Concrete mix shop drawing shall contain testing data proving concrete design mix shrinkage is less than 0.034% at 28 days when tested according to ASTM C157 (air drying method only).

D. All concrete for exterior flatwork shall have a minimum design compressive strength of 4500 psi in 28 days, with not less than 560 pounds of cement per cubic yard of concrete, not over 5 gallons of water per 100 pounds of cement, with 6% +/- 1% air entrainment, and a maximum of 4 inches of slump.

E. The preceding minimum mix requirements may have water-reducing admixtures conforming to ASTM C494 added to the mix at manufacturer's dosage rates for improved workability.

F. The preceding minimum mix requirements may have up to 15% maximum of the cement content replaced with an approved ASTM C618 Class C fly ash, provided the total minimum cementitious content is not reduced.

G. All interior concrete slabs on grade shall be placed over 15 mil, Class A Vapor Barrier per ASTM E1745 with less than 0.01 perms, tested after mandatory conditioning. All joints shall be lapped and sealed per manufacturer's recommendations. All penetrations, as well as damaged vapor barrier material shall also be sealed per manufacturer's recommendation prior to concrete placement. Install barrier per manufacturer recommended details at all discontinuous edges (at interior columns, exterior edge of slab, etc.) to ensure terms of warranty are followed. The vapor barrier shall be placed over free-draining granular material as prescribed by the project soils report.

H. All concrete is reinforced concrete unless specifically called out as unreinforced. Reinforce all concrete not otherwise shown with same steel as in similar sections or areas. Any details not shown shall be detailed per ACI 315 and meet requirements of ACI 318, current editions.

I. Control joints in dirt formed slab to be as shown on plans. Where not shown, limit controlled areas to not more than 144 square feet, or 12 feet on any side. Slab panel side ratio shall not exceed 1 1/2 to 1.

J. Contractor shall verify that all concrete inserts, reinforcing and embedded items are correctly located and rigidly secured prior to concrete placement.

K. Construction joints in beams, slabs, and grade beams shall occur at midspan (middle third) unless noted otherwise. Provide 2 x 4 horizontal keys at construction joints for shear transfer.

L. No aluminum items shall be embedded in any concrete.

4. Reinforcing Steel

A. All reinforcing steel shall conform to the requirements of ASTM A615 or A706 grade 60 steel. Welded plain wire fabric shall be supplied in sheets and conform to the requirements of ASTM A185.

B. Clear minimum coverage of concrete over reinforcing steel shall be as follows:

1.) Concrete placed against earth:	3"
2.) Formed concrete against earth:	2"
3.) Slabs:	1"
4.) Beams or Columns:	1-1/2"
5.) Other	2"

All coverage shall be nominal bar diameter minimum.

C. All dowels shall be the same size and spacing as adjoining main bars (splice lap 48 bar diameters or 24" minimum unless noted otherwise).

D. At corners of all walls, beams, and grade beams supply corner bars (minimum 2'-0" in each direction or 48 bar diameters) in outside face of wall, matching size and spacing of horizontal bars. Where there are no vertical bars in outside face of wall, supply 3 - #4 vertical support bars for corner bars.

E. Bars marked continuous and all vertical steel shall be lapped 48 bar diameters (2'-0" minimum) at splices and embedments, unless shown otherwise. Splice top bars near midspan and splice bottom bars over supports, unless noted otherwise.

F. At all holes in concrete walls and slabs, add 2 - #5 bars (opening dimension plus 96 diameters long) at each of four sides and add 2 - #5 x 5'-0" diagonally at each of four corners of hole. Openings in 8" thick walls are reinforced similar, but with 1 - #5 instead of 2 - #5, respectively.

G. Unless otherwise covered on architectural plans or specifications, vertical control joints in concrete wall shall be spaced at a maximum of 20'-0" on center and coordinated with the architect. Every other horizontal wall reinforcing bar shall be discontinuous at control joints except heavy top and bottom bars unless noted otherwise. Provide base seal waterstop style number 772 (by Greenstreak Inc. or approved equal) on dirt face side of wall at all walls below grade.

H. Accessories shall be as specified in latest edition of the ACI Detailing Handbook and the concrete Reinforcing Steel Institute Design Handbook. Maximum accessory spacing shall be 4'-0" on center, and all accessories on exposed surfaces are to have plastic coated feet.

I. All slabs and stairs not shown otherwise shall be 6" thick with #4 bars at 12" on center each way. All exterior porches and stoops not otherwise detailed may be constructed in any standard manner, solid or hollow, but must be reinforced with #4 bars at 12" on center each way minimum. Porches shall be doweled to adjacent walls or grade beams with #4 bars at 12" on center, hooked or embedded 48 diameters into both members. Slope porches 1/8" per foot for drainage unless noted otherwise.

J. Allow 1 ton of reinforcing bars #4 or larger to be used as directed in the field for special conditions by the engineer of record (labor for placing same to be included).

5. Structural Steel

A. All structural steel beams and columns shall be ASTM A992, grade 50 steel and all miscellaneous steel shall be ASTM A36 grade steel (except at moment connections where plates shall be ASTM A572, grade 50). Hollow Structural Sections (HSS) shall be ASTM A500, grade C. Fabrication and erection shall be in accordance with AISC 303-16 "Code of Standard Practice for Steel Buildings and Bridges" in the 15th Edition of the AISC Steel Construction Manual.

B. All welding shall conform to the recommendations of the AWS.

C. All exterior steel and connections, and brick relief angles shall be hot-dip galvanized.

D. All bolts not otherwise specified shall be 3/4" diameter high strength (ASTM A325-N). All bolts shall be fully pretensioned. All beam connections shall be designed per the AISC Manual of Steel Construction "Framed Beam Connections" for the indicated reactions or at least 0.3 x beam total shear capacity, Vn/Omega, shown in the maximum total uniform load tables, whichever is greater; and, shall account for eccentricity when the bolt line is more than 2" from the center of the support. All connections must be two bolt minimum.

E. All anchor bolts shall be 3/4" diameter, ASTM F1554, Grade 36 unless noted otherwise. Washers of minimum size and thickness for the given anchor diameter in Table 14-2 of the AISC Steel Construction Manual shall be provided at every column anchor bolt. Washers shall have a standard size hole for the anchor bolt.

6. Post Installed Anchors

A. Post-installed anchors shall be used only where specified on the drawings unless approved in writing by the engineer of record. See drawings for anchor diameter, spacing and embedment. Performance values of the anchors shall be obtained for specified products using appropriate design procedures and/or standards as required by the governing building code. Anchors installed in concrete shall have an ICC-ES Evaluation Service Report. Special inspection is required for all post installed anchors. The contractor shall coordinate an on-site meeting with the post installed anchor manufacturer field representative to educate the construction team on the anchor installation guidelines and requirements.

B. Mechanical anchors used in cracked and uncracked concrete shall have been tested and qualified for use in accordance with ACI 355.2 and ICC-ES AC193. All anchors shall be installed per the anchor manufacturer's written instructions.

C. Adhesive anchors used in cracked and uncracked concrete shall have been tested and qualified for use in accordance with ICC-ES AC308. All anchors shall be installed per the anchor manufacturer's written instructions.

D. Mechanical anchors used in solid grouted masonry shall have been tested and qualified for use in accordance with ICC-ES AC01. All anchors shall be installed per the anchor manufacturer's written instructions.

E. Adhesive anchors used in solid grouted masonry shall have been tested and qualified for use in accordance with ICC-ES AC58. All anchors shall be installed per the anchor manufacturer's written instructions.

F. Anchors used in hollow concrete masonry shall have been tested and qualified in accordance with ICC-ES AC106 or ICC-ES AC58 as appropriate. All anchors shall be installed per the anchor manufacturer's written instructions with appropriate screen tubes used for adhesives.

7. Foundations

A. The soil investigation was prepared by CFS Engineers, the report number is 20-5555 and their telephone number is 913-627-9040.

B. Spread footings, grade beams, and retaining walls are designed to bear on insitu clay or engineered clay fill capable of safely sustaining 2,500 psf.

C. Retaining walls are designed for an active lateral load of 55 pcf equivalent fluid pressure.

D. Basement walls are designed for an at rest lateral load of 80 pcf equivalent fluid pressure. See General Note 3H for wall bracing requirements.

E. Contractor shall provide for dewatering at excavations from either surface water or seepage.

F. All foundation excavations shall be inspected by a qualified soil engineer, approved by the architect and/or structural engineer, prior to placement of steel or concrete. This inspection shall be at the owner's expense.

G. All concrete in the structural portion retaining the backfill shall have attained its design strength prior to being backfilled.

H. Moisture content in soils beneath building locations should not be allowed to change after footing excavations and after grading for slabs on grade are completed. If subgrade materials become desiccated or softened by water or other conditions, recompact materials to the density and water content specified for engineered fill. Do not place concrete on frozen ground.

8. Concrete Masonry Units

A. Concrete block used in exterior walls or load bearing walls shall meet the requirements of ASTM C90 and have a minimum net compressive strength of 1900 psi and laid up using type N mortar such that f'm equals 1500 psi. Mortar shall be volume proportion based cement lime mortar. Proportioning shall be completed by box measure. Any block in contact with earth shall be normal weight units, laid using type "S" mortar and grouted solid.

B. The contractor shall provide adequate temporary bracing for all masonry walls during construction.

C. All concrete block shall have 9 gage (or larger) horizontal joint reinforcing (ladder or truss) per architectural drawings and specifications (16" maximum vertical spacing).

D. Cavity wall construction shall be reinforced as designed for specific concrete block used. The horizontal joint reinforcing shall be of the ladder or truss style per specification and continuous between brick and block, as prescribed by the architectural drawings.

E. Concrete block shall be reinforced as follows in 6", 8", 10", and 12" walls:
1.) Vertical reinforcing shall be a minimum of 1 - #5 bar in 6" and 8" walls and 2 - #5 bars in 10" and 12" walls at 24" OC, at each corner, at each door and window jamb, each side of control joints and in the end void of each length of wall. Lap splices for masonry vertical reinforcing shall be 48 bar diameters or 24" minimum.
2.) Horizontal reinforcing:
A.) Horizontal joint reinforcing as noted above.
B.) Continuous horizontal bars shall be included per section or detail in bond beam or optional running bond beam where noted. Where bond beams are continuous at corners of walls, supply corner bars matching size of horizontal bars (minimum 2'-0" or 40 bar diameters in each direction).

F. Grout, where noted above, shall have a minimum design ultimate compressive strength of 2500 psi at 28 day test and 3/8" maximum aggregate size.

G. Non-load bearing concrete block walls shall be isolated from adjacent structural elements with vertical 3/8" control joints and at the top of the wall with 1" air space or compressible material and support per architectural detail.

H. Unless otherwise covered on architectural plans or specifications, vertical control joints in masonry construction shall be 3/8" wide, full height of wall. Joints shall be spaced at a maximum of 24'-0" on center and coordinated with the architect. All horizontal joint reinforcing shall be discontinuous at control joints in masonry. All bond beam horizontal reinforcing shall be continuous through control joints.

I. Lintels over all openings up to 8'-0" wide in new and existing masonry walls not otherwise covered shall be one 6x3 1/2x5/16 angle for each 4" width of masonry. All exterior lintels to be galvanized.

J. Walls shall be anchored top and bottom by dowels matching wall vertical reinforcing(unless noted otherwise) from floor slab bottom and bracing angles at the top, per details on the drawings.

9. Timber and Wood Framing

A. Quality and construction of wood framing members and their fasteners for load supporting purposes not otherwise indicated on the drawings shall be in accordance with the International Building Code.

B. All studs and top and bottom plates shall be Douglas Fir No. 2 grade visually graded lumber, with an allowable fiber stress in bending of 900 psi minimum and an elastic modulus of 1,600,000psi unless noted otherwise. All joist, truss members, and headers to be No. grade 2 (min.) unless noted otherwise. All lumber for exterior decks and balconies shall be treated Southern Yellow Pine No. 2 grade.

C. Blocking of stud bearing walls and shear walls shall be solid, matching sheathing joints.

D. Joist blocking and bridging shall be solid wood or cross bridging of either wood or metal straps. Spacing, in general, shall not exceed 8'-0".

E. Wood members and sheathing shall be fastened with number and size of fasteners not less than that set forth in Table 2304.9.1 of the International Building Code. Floor sheathing shall be APA rated tongue and groove Sturd-I-Floor, exposure 1, glued and nailed with 8d ring shank nails or # 10 screws at 12" on center to all supports. Sheathing of shear walls or roof diaphragms shall be edge nailed with 8d common nails at 6" on center and nailed to intermediate framing and/or blocking members with 8d common nails at 12" on center unless otherwise noted on the drawings. All floor sheathing shall be installed with 1/8 inch gaps between panel edges and end joints.

F. Sill plates shall be bolted to concrete walls or steel beams with 1/2" diameter bolts at 32" on center. Sill plates in direct contact with concrete or masonry shall be treated lumber.

G. Joist hangers shall have Uniform Building Code approval and shall be equal to Simpson Strong Tie "LUS" for wood application and "LB" for steel weld-on application.

H. Service condition - dry with moisture content at or below 19% in service.

I. Laminated veneer lumber (LVL) shall have an allowable flexural stress (Fb) of 2,600 psi (reduced by size factor) and an elastic modulus (E) of 1,900,000 psi.

J. Parallel Strand Lumber (PSL) shall have an allowable flexural stress (Fb) of 2,900 psi (reduced by size factor) and an elastic modulus (E) of 2,000,000 psi.

K. Pre-engineered wood trusses shall be designed in accordance with the Truss Plate Institute's national design standard for metal-plate connected wood truss construction (ANSI/TPI-1 latest edition). Trusses shall be designed and manufactured by an authorized member of the Wood Truss Council of America (WTCA). Truss design shall conform to specified codes, allowable stress increases, deflection limitations and other applicable criteria of the governing code.

L. Shop drawings showing complete erection and fabrication details and calculations (including connections) shall be submitted to the project architect/engineer for review prior to fabrication and/or erection. Such drawings shall bear the seal of a professional engineer, registered in the state of the project location. Shop drawings shall also be submitted to the local government controlling agency when requested by that agency.

M. All trusses shall be securely braced both during erection and permanently, as indicated on the approved truss design drawings and in accordance with TPI's commentary and recommendations for handling, installing and bracing metal-plate connected wood trusses (HIB-91, booklet) and the latest edition of ANSI/TPI-1.

N. The truss manufacturer shall supply all hardware and fasteners for joining truss members together and fastening truss members to their supports. Metal connector plates shall be manufactured by a member of the Wood Truss Council of America (WTCA) and shall be 20 gauge minimum. Connector plates shall meet or exceed ASTM A653, grade 33, with ASTM A924 galvanized coating designation G60.

O. Shipment, handling, and erection of trusses shall be by experienced, qualified persons and shall be performed in a manner so as not to endanger life or property. Apparent truss damage shall be reported to the truss manufacturer for evaluation prior to erection. Cutting or alteration of trusses is not permitted.

P. Contractor shall coordinate truss layout for openings and penetrations required by other trades including for plumbing, HVAC, electrical, roof access hatches, chases, etc.

Q. Pre-engineered floor truss and I-Joist design load and deflection criteria are as follows:

Top Chord Dead Load	= 20psf
Top Chord Live Load	= Per General Note 2A
Bottom Chord Dead Load	= 5psf
Allowable Total Load Deflection	= L/360
Allowable Live Load Deflection	= L/480; ½" maximum

R. Pre-engineered roof truss design load and deflection criteria are as follows:

Top Chord Dead Load	= 15psf
Top Chord Live Load	= 20psf
Bottom Chord Dead Load	= 10psf
Allowable Total Load Deflection	= L/300
Allowable Live Load Deflection	= L/360

Roof trusses shall be designed for wind uplift loads indicated in Building Components & Cladding Wind Loads Diagram.

10. Shop Drawing Review

A. Bob D. Campbell and Company, Inc. will review the General Contractor's (GC) shop drawings and related submittals (as indicated below) with respect to the ability of the detailed work, when complete, to be a properly functioning integral element of the overall structural system designed by Bob D. Campbell and Company, Inc.

B. Prior to submittal of a shop drawing or any related material to Bob D. Campbell and Company, Inc., the GC shall:
1.) Review each submission for conformance with the means, methods, techniques, sequences and operations of construction and safety precautions and programs incidental thereto, all of which are the sole responsibility of the GC.
2.) Review and approve each submission.
3.) Stamp each submission as approved.

C. Bob D. Campbell and Company, Inc. shall assume that no submission comprises a variation unless the GC advises Bob D. Campbell and Company, Inc. with written documentation.

D. Bob D. Campbell and Company, Inc. shall review shop drawings and related materials with comments provided that each submission has met the above requirements. Bob D. Campbell and Company, Inc. shall return without comment material or submissions without GC approval stamp.

E. Shop drawings and related material (if any) required are indicated below. Should Bob D. Campbell and Company, Inc. require more than ten (10) working days to perform the review, Bob D. Campbell and Company, Inc. shall so notify the GC.
1.) Concrete mix designs and material certificates including admixtures and components applied to the concrete after placement.
2.) Reinforcing steel shop drawings including erection drawings and bending details. Bar list will not be reviewed for correct quantities.
3.) Elevations of all reinforced concrete masonry walls at a scale no smaller than 3/8" = 1'-0" showing all required reinforcing.
4.) Grout mix designs (for CMU).
5.) Construction and control joint plans and/or elevations.
6.) Structural steel shop drawings including erection drawings and piece details. Include joist, decking and connector submittals. Include miscellaneous framing specified on the structural drawings, but do not submit framing specified on non-structural drawings for Bob D. Campbell and Company, Inc. review.
7.) Miscellaneous anchors shown on the structural drawings.
8.) Wood truss design calculations and detailed erection and fabrication drawings. Standard stick framing shop drawings need not be submitted.

11. Statement of Structural Special Inspections

A. The structural design for this project is based on completion of special inspections during construction in accordance with section 1704 of the International Building Code. The owner shall employ one or more qualified special inspectors to provide the required special inspections.

B. The special inspector shall furnish inspection reports to the building official, owner, architect and structural engineer, and any other designated person.

C. All discrepancies shall be brought to the immediate attention of the contractor for correction, then, if uncorrected, to the proper design authority, building official and structural engineer.

D. The special inspector shall submit a final signed report stating that the work requiring special inspection was, to the best of the inspector's knowledge, in conformance with the approved plans and specifications and the applicable workmanship provisions of the building code.

E. The following inspections and tests are required with the frequency (continuous or periodic) as defined within the referenced section or standard listed below. The General Contractor shall provide notification to the inspector when items requiring inspection are ready to be inspected and provide access for those inspections.

1. Shop Fabrication – structural steel per Section 1704.2.5 unless AISC certified shop
2. Shop Fabrication – pre-engineered wood trusses per Section 1704.2.5 unless TPI certified shop
3. Steel Construction per Section 1705.2 and the quality assurance requirements of AISC 341 Chapter J (as referenced by AISC 360)
5. Concrete Construction per Section 1705.3 and Table 1705.3
a. Reinforcing Steel Placement
b. Reinforcing Steel Welding
c. Cast in Place Anchors
d. Post Installed Anchors
e. Design Mix Verification
f. Concrete Sampling and Testing
g. Concrete Placement
h. Concrete Curing
6. Masonry Construction per Section 1705.4 and the quality assurance requirements of TMS 402/ACI530/ASCE5 and TMS602/ASO.1/ASCE6 **[Level B]**
7. Verification of Soils per Table 1705.6
8. Wood Lateral System (periodic)
a. Wood shearwalls (include sheathing, rim board and bottom plate attachments)
b. Portal frames
c. Shear wall and portal frame holdowns
d. Shear wall tension rod system
9. Wood Gravity Framing and Placement (adjust frequency of random sampling where indicated as required)
a. Heavy timber/SCL/glulam beams and supports (periodic)
b. Headers and jambs (random sampling)
c. Bearing walls (random sampling)
d. Connector/hardware installation (random sampling)
e. Floor and roof trusses (random sampling)

12. Copyright and Disclaimer

A. All drawings in the structural set (S-series drawings) are the copyrighted work of Bob D. Campbell and company, Inc. These drawings may not be photographed, traced, or copies in any manner without the written permission of Bob D. Campbell and Company, Inc. Exception: Original drawings may be printed for distribution to the owner, architect, and general contractor for coordination, bidding, and construction. Subcontractors may not reproduce these drawings for any purpose or in any manner.

B. I, Clark A. Basinger, P.E., registered engineer and a representative of Bob D. Campbell and Company, Inc., do hereby accept professional responsibility as required by the professional registration laws of this state for the structural design drawings consisting of S-series drawings. I hereby disclaim responsibility for all other drawings in the construction document package, they being the responsibility of other design professionals whose seals and signed statements may appear elsewhere in the construction document package.

ESTIMATED BUILDING MOVEMENT TABLE

FLOOR	ACCUMULATIVE WOOD SHRINKAGE	HEIGHT OF BRICK	ACCUMULATIVE BRICK EXPANSION
ROOF	1"	30'	0.33"
3rd FLOOR	0.7"	20'	0.22"
2nd FLOOR	0.35"	10'	0.11"

Wood Shrinkage Notes:

Bob D. Campbell & Company takes no responsibility for the naturally-occurring shrinkage that will occur in a wood structure or the impact the movement will have on the architectural, mechanical, electrical and plumbing systems that are designed by others. The analysis provided below are estimated values in accordance with IBC Section 2304.3.3 and indicate the systems and/or routing of the systems shall be designed to accommodate the movement. Failure to follow the considerations below can result in a failure of the impacted components within the system.

Estimated values are based on the following moisture content in the framing
a. At install (MC) = 19%
b. At equilibrium (EMC) = 8%

Reference wall sections on this sheet for estimated cumilative values per floor.

The following is a list of recommendations to minimize potential issues related to wood shrinkage and veneer expansion. Veneer expansion is seasonable and variable depending on sun exposure. The majority of wood shrinkage will occur in the first 24 months of occupancy with minor seasonal variations.

1. MEP System Considerations
a. Postpone MEP installation as long as possible to allow as much dead load to be applied--allowing construction gaps to close.
b. Provide oversized and vertically slotted holes at pipe horizontal penetration and notches. Refer to typical notching and cutting of stud wall detail for additional considerations on size limitations.
c. Plumbing pipe and electrical conduit joints and connections shall be flexible and allow for expansion/contraction to prevent a rigid assembly.
d. Hangers and necessary rigid connections shall be adjusted prior to completion of construction or closing of wall/ceiling assembly.
e. Horizontal vent penetrations through exterior veneers shall be provided with double flashing.
f. All sheet metal vertical down spouts shall have intermediate slip joints.
g. Roof drains shall utilize adjustable fittings that are adjusted back to the roof finish sheathing elevation at the completion of construction and then shall be adjusted as required to maintain proper drainage.

2. Architectural System Considerations
a. At stucco, EIFS and thin set veneer systems provide horizontal expansion joints, slip joints with appropriate flashing, this includes transitions between changes in veneer material.
b. At brick and stone veneers provide veneers ties designed to accommodate differential movement.
c. Refer to architectural window and door head and sill; parapet; and horizontal material changes for specific horizontal gap requirements between materials.

3. Construction Tolerance Considerations
a. All studs shall be cut level, square and tight to top and bottom plates to reduce any additional shortening of the building due to nesting.
b. All wood structural panels on the walls shall have a 1/2" relief gap at each floor level to reduce the potential for bulging.
c. All floor sheathing shall have 1/8" gaps around all four sides at time of install to allow for expannsion.
d. All shearwall holdown shall be checked and retighten immediately prior to sheathing of the walls. If a continuous rod system is utilized for holdowns or uniform uplift anchors, the take-up devise pins shall be verified to have been pulled prior to sheathing the walls.

4. Material Storage and Protection
a. All stored material shall remain covered and elevated from the elements to reduce the potential for an increase in moisture content.
b. Do not allow water to pond on the floor sheathing. Provide drain holes in the floor sheathing as required to relieve any water that might temporary pond.

5. Post Occupancy Consideration
a. Recommend a review of roof drains every 3 months for the first 24 months of occupancy and then annually and adjusted as needed.
b. Recommend a review of vertical joints at exterior doors, windows and at changes in materials. Caulked as needed as shrinkage occurs and original joint fails.

NAILING SCHEDULE (REFER TO NOTES #1 and #2)

No.	CONNECTION	ATTACHMENTS	(REF NOTE #3 and #4)
1	JOIST TO SILL OR GIRDER	3- 3" x 0.131" NAILS-TOENAIL	3-8d NAILS-TOENAIL
2	BRIDGING TO JOIST	2- 3" x 0.131" NAILS-TOENAIL EACH END	2-8d NAILS-TOENAIL EACH END
3	SOLE PLATE TO JOIST OR BLOCKING	3" x 0.131" NAILS AT 8"o.c.- TYPICAL FACE NAIL 4-3" x 0.131" NAILS AT 6"o.c. BRACED WALL PANELS	16d BOX NAILSZ AT 16"o.c. MAX. FACE NAILING 3-16d BOX NAILS AT 16"o.c. BRACED WALL PANEL
4	TOP PLATE TO STUD	3- 3" x 0.131" NAILS-END NAIL	2-16d NAILS-END NAIL
5	STUD TO SOLE PLATE	4- 3" x 0.131" NAILS-TOENAIL OR 3- 3" x 0.131" NAILS-END NAIL	4-8d NAILS-TOENAIL OR 2-16d NAILS-END NAIL
6	DOUBLE STUDS	3" x 0.131" NAILS AT 8"o.c.-FACE NAIL	16d BOX NAILS AT 24"o.c. MAX. FACE NAIL
7	DOUBLED TOP PLATES	3" x 0.131" NAILS AT 12"o.c.-FACE NAIL	16d BOX NAILS AT 16"o.c. MAX. FACE NAIL
8	DOUBLE TOP PLATE LAPS AND INTERSECTIONS	12-3" x 0.131" NAILS	8-16d NAILS
9	BLOCKING BETWEEN JOISTS OR RAFTERS TO TOP PLATE	3-3" x 0.131" NAILS -TOENAIL	3-8d NAILS-TOENAIL
10	RIM JOIST TO TOP PLATE	3" x 0.131" NAILS AT 6"o.c.-TOENAIL	8d NAILS AT 6"o.c. MAX.-TOENAIL
11	TOP PLATE LAPS AND INTERSECTIONS	3- 3" x 0.131" NAILS-FACE NAIL	2-16d NAILS-FACE NAIL
12	CONTINUOUS HEADER, TWO PIECES	3" x 0.131" NAILS AT 10"o.c. ALONG EACH EDGE	16d NAILS AT 16"o.c. MAX. ALONG EACH EDGE-TOENAIL
13	CEILING JOISTS TO PLATE	5- 3" x 0.131" NAILS-TOENAIL	3-8d NAILS-TOENAIL
14	CONTINUOUS HEADER TO STUD	4- 3" x 0.131" NAILS-TOENAIL	4-8d NAILS-TOENAIL
15	CEILING JOISTS, LAPS OVER PARTITIONS	4- 3" x 0.131" NAILS-FACE NAIL	3-16d NAILS-FACE NAIL
16	CEILING JOISTS TO PARALLEL RAFTERS	4- 3" x 0.131" NAILS-FACE NAIL	3-16d NAILS-FACE NAIL
17	RAFTER TO PLATE	3- 3" x 0.131" NAILS-TOENAIL	3-8d NAILS-TOENAIL
18	1" BRACE TO EACH STUD AND PLATE	2- 3" x 0.131" NAILS-FACE NAIL	2-8d NAILS-FACE NAIL
19	BUILT-UP CORNER AND MULTIPLE STUDS	3" x 0.131" NAILS AT 16"o.c.	16d NAILS AT 24"o.c. MAX.
20	BUILT-UP GIRDER AND BEAMS	3" x 0.131" NAILS AT 24"o.c. FACE NAILED TOP AND BOTTOM STAGGERED ON OPPOSITE SIDES 3- 3" x 0.131" NAILS AT ENDS AND EACH SPLICE	20d NAILS AT 32"o.c. MAX. TOP AND BOTTOM, STAGGERED ON 0PPSITE SIDES. 2-20d NAILS AT ENDS AND EACH SPLICE
21	BUILT-UP LAMINATED VENEER LUMBER BEAMS	3" x 0.131" NAILS AT 6"o.c. TOP AND BOTTOM ALONG EDGE	16d NAILS AT 12"o.c. TOP AND BOTTOM ALONG EDGE
22	2" PLANKING	4- 3" x 0.131" NAILS AT EACH SUPPORT	16d NAILS AT EACH SUPPORT
23	RIM BOARD TO TRUSS	2- 3" x 0.131" FACE NAILS (IT/IB @ EA. TRUSS)	2- 10d NAILS - FACE NAILS (IT/IB @ EA. TRUSS)
24	BUILD-UP STUD-PACK COLUMNS	REFER TO DETAIL 3/S003	REFER TO DETAIL 3/S003

NOTES:
1.) ALL NAILS SHALL BE AS NOTED UNLESS OTHERWISE SPECIFIED ON STRUCTURAL DRAWINGS OR ALTERNATE PROVIDED BY ENGINEER IN WRITING.
2.) CONDITIONS NOT SPECIFIED SHALL BE IN ACCORDANCE WITH CURRENT INTERNATIONAL BUILDING CODE.
3.) NAILING DESIGNATION:
4 - 3" x 0.131" NAILS
DIAMETER IN INCHES
NAIL LENGTH
QUANITY
4.) ALL NAILS NOTED AS 8d, 10d, 16d, ETC. SHALL BE COMMON NAILS UNLESS NOTED BOX.

STRUCTURAL DECK & SLAB SCHEDULE

MARK	DESCRIPTION
T-1	COMPOSITE DECKING PER ARCHITECTURAL DRAWINGS/SPECIFICATIONS
FD-1	½" GYPCRETE ATOP 23/32" T&G PLYWOOD SHEATHING. SHEATHING SHALL BE GLUED AND NAILED W/ 8d RING SHANK NAILS OR #10 SCREWS @ 6"o.c. @ EDGES & 12"o.c. AT FIELD.
RD-1	19/32" PLYWOOD SHEATHING ATTACHED WITH 8d NAILS @ 6"o.c. AT EDGES & 12"o.c. AT FIELD.
SOG-1	4" CONCRETE SLAB REINFORCED W/ 6x6-W2.9xW2.9 WWF ATOP VAPOR BARRIER PER GENEARL NOTES ATOP 4" COMPACTED GRANULAR FILL ATOP 4" MINIMUM OPEN GRADED STONE ATOP PREVIOUSLY PREPARED PAD IN COMPLIANCE WITH SOILS REPORT

NOTES:
1. CD = COMPOSITE/CONCRETE DECK TYPE
2. FD = FLOOR DECK TYPE
3. NCD = NON-COMPOSITE DECK TYPE
4. RD = ROOF DECK TYPE
5. SOG = SLAB-ON-GRADE TYPE
6. REFER TO NOTE 10.T ON SHEET S0.01 FOR FIRE - RETERDANT TREAD SHEATHING REQUIREMENTS.
7. PROVIDE 1" DEEP TOOLED CONTROL JOINT (TRANSVERSE DIRECTION) @ MID-SPAN OF SINGLE BAY BALCONY OR @ THIRD POINTS OF DOUBLE BAY BALCONY. FILL JOINT w/ SEALANT

LEGEND:

- ..... SPAN DIRECTION OF DECK
- 3.0 ..... FOOTING MARK - SEE SCHEDULE ON SHEET S002.
- 1 — HSS 8"x8"x5/16" .... COLUMN SIZE; BASE PLATE MARK - SEE SCHEDULE ON SHEET S002
- LEVEL BEAM DESIGNATION: W14x22 — STEEL BEAM SIZE; T 117'-6" — TOP OF BEAM ELEVATION
- SLOPING BEAM DESIGNATION: W14x22 — STEEL BEAM SIZE; T 133'-0" T 132'-5" — TOP OF BEAM ELEVATION EACH END
- A#-# ..... BEAM OR HEADER PER SCHEDULE ON S003
- A#-#u ..... UPSET BEAM OR HEADER PER SCHEDULE ON S003
- SJ ..... SAW JOINT PER 1/S200
- CJ ..... CONSTRUCTION JOINT PER 2/S200
- ..... "WIDE CORRIDOR" WITH ADDITIONAL STUDS PER WOOD STUD BEARING WALL SCHEDULE ON SHEET S003
- SWX ..... SHEAR WALL PER SCHEDULE ON SHEET S004

STRUCTURAL ABBREVIATIONS

@	AT
&	AND
ø	ROUND, DIAMETER
ADTL	ADDITIONAL
AFF	ABOVE FINISHED FLOOR
ALT	ALTERNATE
ARCH	ARCHITECTURAL
BLDG	BUILDING
B/	BOTTOM OF
BM	BEAM
BOTT	BOTTOM
BRG	BEARING
C	CAMBER
CD-#	CONCRETE DECK TYPE
CJ	CONSTRUCTION/CONTROL JOIN
CJP	COMPLETE JOINT PENETRATION
CL	CENTERLINE
CMU	CONCRETE MASONRY UNIT
COL	COLUMN
CONC	CONCRETE
CONN	CONNECTION
CONT	CONTINUOUS
COORD	COORDINATE
COV, CVR	COVER
DBL	DOUBLE
DET	DETAIL
DIA	DIAMETER
DIM	DIMENSION
DL	DEAD LOAD
DWG	DRAW

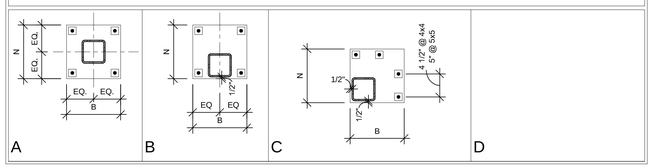
COLUMN BASE PLATE SCHEDULE

TYPE	COLUMN	BASE PLATE (NxBxT)	SHAPE	ANCHOR RODS	EMBEDMENT
1	PER PLAN	10"x10"x3/4"	A	4- 3/4" DIA.	1'-4"
2	PER PLAN	11"x11"x3/4"	A	4- 3/4" DIA.	1'-4"
3	PER PLAN	10"x10"x3/4"	B	4- 3/4" DIA.	1'-4"
4	PER PLAN	11"x11"x3/4"	C	4- 3/4" DIA.	1'-4"
5	PER PLAN	11"x11"x3/4"	B	4- 3/4" DIA.	1'-4"
6	PER PLAN	12"x12"x3/4"	C	4- 3/4" DIA.	1'-4"

NOTES:

1. SEE PLAN FOR ORIENTATION OF COLUMNS.
2. PROVIDE PLATE WASHED & EMBEDDED PLATE PER SCHEDULE @ ALL ANCHOR BOLTS.
3. U.N.O. ALL THREADED ROD A.B's SHALL BE F1554 (36ksi) MATERIAL.

BASE PLATE SHAPE (NOT TO SCALE)



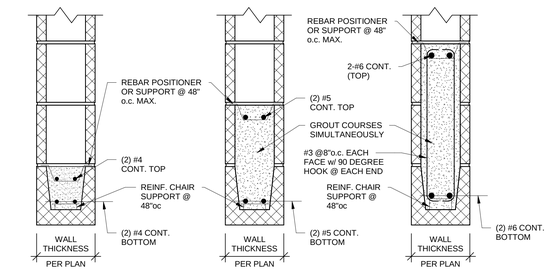
COLUMN BASE PLATE AND ANCHOR-ROD CRITERIA

ANCHOR-ROD DIAMETER.	MAX. BASE PLATE HOLE DIAMETER.	MIN. PLATE WASHER SIZE.	MIN. PLATE WASHER THICKNESS	EMBEDDED ANCHOR PLATE SIZE
3/4"	1 5/16"	2"	1/4"	1/2"x2 1/2"x2 1/2"
7/8"	1 9/16"	2 1/2"	5/16"	1/2"x2 1/2"x2 1/2"
1"	1 7/8"	3"	3/8"	5/8"x3"x3"
1 1/4"	2 1/8"	3 1/2"	1/2"	5/8"x3 1/2"x3 1/2"
1 1/2"	2 3/8"	4"	1/2"	5/8"x3 1/2"x3 1/2"
1 3/4"	2 7/8"	4 1/2"	5/8"	3/4"x3 1/2"x3 1/2"
2"	3 1/4"	5"	3/4"	3/4"x3 1/2"x3 1/2"
2 1/2"	3 3/4"	5 1/2"	7/8"	3/4"x3 1/2"x3 1/2"

NOTES:
1. HOLE SIZES PROVIDED ARE BASE ON ANCHOR ROD SIZE AND CORRELEATE WITH ACI 117 (ACI, 2010)
2. CIRCULAR OR SQUARE WASHERS MEETING THE WASHER SIZE ARE ACCEPTIBLE.
3. HOLE IN PLATE WASHER SHALL BE 1/16" LARGER THAN ANCHOR DIAMETER.=

TYPICAL MASONRY REINFORCING NOTE:

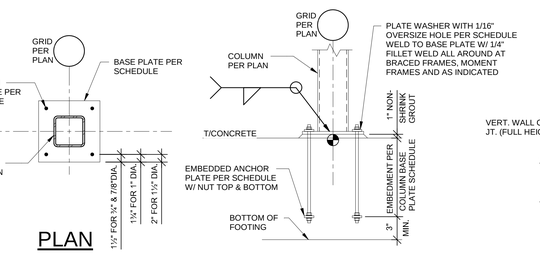
ALL INTERIOR & EXTERIOR MASONRY WALLS SHOWN ON ARCHITECTURAL AND STRUCTURAL DRAWINGS ARE TO BE REINFORCED HORIZONTALLY WITH BOND BEAMS (2 - #5 BOTTOM) AT BOTTOM COURSE, TOP COURSE, JOIST BEARING ELEVATION AND AT 8'-0" MAXIMUM O.C. AND VERTICALLY AS INDICATED ON DRAWINGS. THESE WALLS ARE TO BE ANCHORED TOP AND BOTTOM TO THE FOUNDATION, FLOOR, OR ROOF PER TYPICAL DETAILS. THE VERTICAL REINFORCING IS CONTINUOUS (IN 6'-6" MAXIMUM LENGTHS, LAPPED 2'-6" MINIMUM). FILL BLOCK CELLS AND BOND BEAMS WITH 2,500psi GROUT. RE: DETAILS "A" THROUGH "E" ON THIS SHEET.

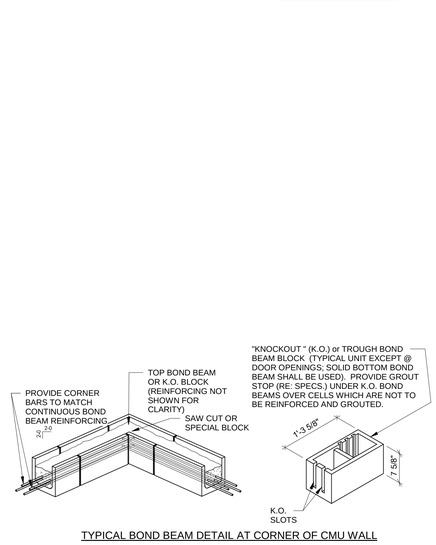


OPENINGS UP TO 4'-0" | OPENINGS 4'-0" TO 7'-4" | OPENINGS 7'-4" TO 12'-0"

TYPICAL LINTELS AT ALL CMU WALLS (U.N.O.)

C SECTION
1 1/2" = 1'-0"





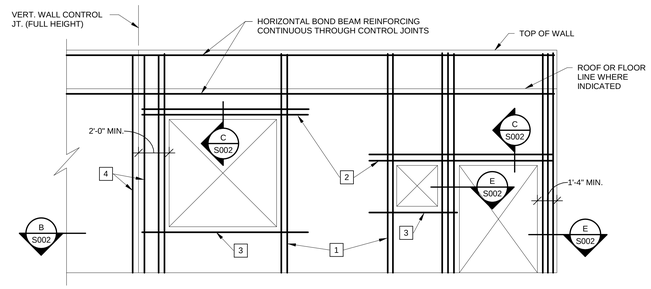
TYPICAL BOND BEAM DETAIL AT CORNER OF CMU WALL

D DETAIL
3/4" = 1'-0"

Structural Foundation Schedule

NOTE:
1) EXTERIOR FOOTINGS OR FOOTING AT GRADE BEAM SHALL MATCH GRADE BEAM DEPTH AND BE PLACED WITH GRADE BEAM. PROVIDE SPECIFIED REBAR TOP AND BOTTOM WITH 4 STANDEES TO SUPPORT MATS.
2.) PROVIDE REINFORCING PER SCHEDULE EACH WAY IN TOP OF FTG. AT ALL MOMENT FRAME AND BRACED BAY COLUMNS.
3.) CENTER FOOTINGS ON COLUMNS AND/OR WALL CENTER LINES PER PLAN UNLESS NOTED OTHERWISE (U.N.O.).

Type Mark	Length	Width	Footing Thickness	Bottom Bars	Quantity (E.W. Bott)	
3.0	3'-0"	3'-0"	1'-4"	Rebar : # 4	4	
3.0A	3'-0"	3'-0"	2'-8"	Rebar : # 4	4	
3.5	3'-6"	3'-6"	1'-4"	Rebar : # 4	5	
3.5A	3'-6"	3'-6"	2'-8"	Rebar : # 4	5	
4.0A	4'-0"	4'-0"	2'-8"	Rebar : # 4	5	
4.5	4'-6"	4'-6"	1'-8"	Rebar : # 4	6	
4.5A	4'-6"	4'-6"	2'-8"	Rebar : # 4	6	
5.0	5'-0"	5'-0"		Rebar : # 5	6	
5.5A	5'-6"	5'-6"	2'-8"	Rebar : # 5	8	
6.0	6'-0"	6'-0"	1'-10"	Rebar : # 5	8	
6.5	6'-6"	6'-6"	1'-8"	Rebar : # 5	9	
8.0	8'-0"	8'-0"	2'-8"	Rebar : # 5	13	



TYPICAL CMU WALL REINFORCING AT OPENINGS

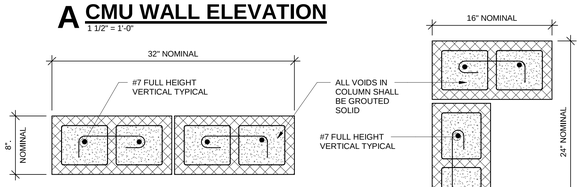
LEGEND:

1. FULL HEIGHT VERTICAL BARS AS JAMB REINFORCING IN FIRST 2 CELLS ADJACENT TO OPENING. REINFORCE EACH CELL WITH SIZE & QUANTITY OF BAR TO MATCH WALL REINFORCING (1 BAR TYPICAL IN 8" WALLS AND 2 BARS TYPICAL IN 12" WALLS).
2. LINTEL REINFORCING PER SECTION C. EXTEND 2'-0" PAST EDGE OF OPENING ON EACH SIDE (TYPICAL).
3. 2-#5 CONTINUOUS HORIZONTAL BARS AS SILL REINFORCING IN 8" COURSE BELOW OPENING (U.N.O.). EXTEND 2'-0" PAST EDGE OF OPENING ON EACH SIDE (TYPICAL).
4. FULL HEIGHT VERTICAL BARS PER MASONRY VERTICAL REINFORCING SCHEDULE LOCATED IN END CELL AT EACH SIDE OF VERTICAL WALL CONTROL JOINTS.

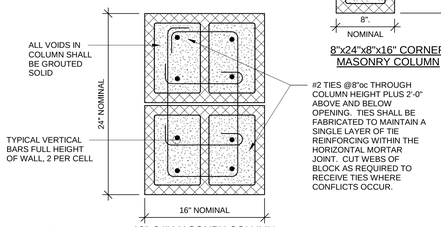
GENERAL CRITERIA: (SECTION A CONTINUED):

1. VERTICAL REINFORCING BARS SHALL BE DOWELED TO FOUNDATION WITH A DOWEL OF MATCHING SIZE AND SPACING.
2. CONTRACTOR SHALL COORDINATE AND VERIFY OPENINGS IN MASONRY WALLS. OPENINGS SHALL BE DETAILED ON REINFORCING STEEL SHOPE DRAWING ELEVATIONS.
3. VERTICAL CONTROL JOINTS IN MASONRY WALLS SHALL BE 3/8" WIDE, FULL HEIGHT OF WALL. JOINTS SHALL BE SPACED AT A MAXIMUM OF 24'-0" ON CENTER AND NOT LESS THAN 2'-0" FROM THE EDGE OF ANY OPENING. ALL HORIZONTAL JOINT REINFORCING SHALL BE DISCONTINUOUS AT CONTROL JOINTS. ALL BOND BEAM HORIZONTAL REINFORCING SHALL BE CONTINUOUS THROUGH CONTROL JOINTS. CONTRACTOR SHALL COORDINATE AND VERIFY ALL CONTROL JOINT LOCATIONS.

A CMU WALL ELEVATION
1 1/2" = 1'-0"



8"x32" MASONRY COLUMN | 8"x24"x8"x16" CORNER MASONRY COLUMN

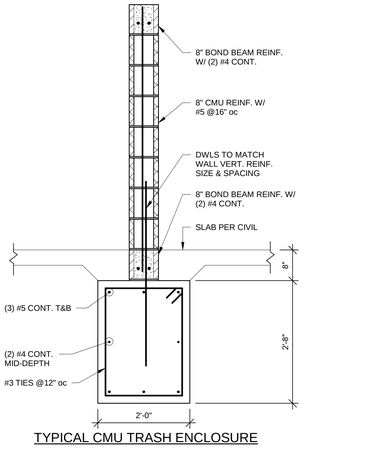


16"x24" MASONRY COLUMN

E SECTION
1 1/2" = 1'-0"

NOTE: REINFORCING SHALL BE PLACED IN POSITIONEERS PRIOR TO GROUTING.

ADJUSTMENTS TO DIMENSIONS TO PROVIDE DEFINED CLEAR GROUT COVER ARE ACCEPTABLE, BUT ALL ADJUSTMENTS SHALL BE APPROVED BY ENGINEER PRIOR TO CONSTRUCTION AND SHALL NOT EXCEED 1/4".



NOTE: VERTICAL REINFORCING PLACEMENT SHALL BE +/-1/4" FROM LOCATIONS NOTED.

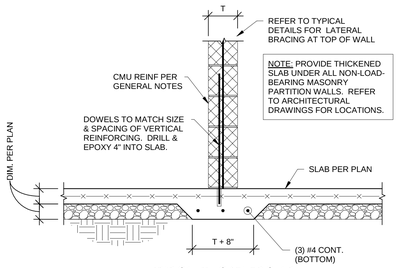
NOTE: ALL MASONRY VOIDS AND BOND BEAMS TO BE GROUTED SHALL BE FREE OF DEBRIS AND MORTAR DROPPINGS PRIOR TO GROUTING. ANY MASONRY w/ DROPPINGS OR DEBRIS OBSERVED IN VOIDS SHALL BE REJECTED.

TYPICAL REBAR POSITIONING DETAIL

B SECTION
1 1/2" = 1'-0"

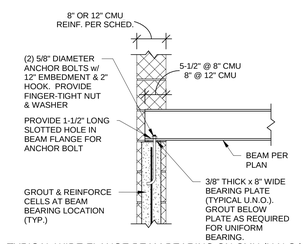
TYPICAL CMU TRASH ENCLOSURE

1 SECTION
3/4" = 1'-0"



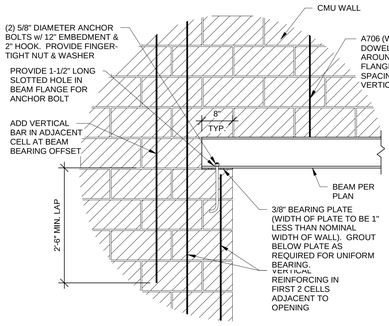
TYPICAL THICKENED SLAB
(UNDER NON-LOAD-BEARING MASONRY)

2 SECTION
3/4" = 1'-0"



TYPICAL WIDE FLANGE BEAM BEARING ON CMU (U.N.O.)

3 SECTION
3/4" = 1'-0"



TYPICAL STEEL LINTEL DETAIL AT CMU WALL

4 SECTION
3/4" = 1'-0"

ARCHITECT
STRUCTURAL ENGINEER — TR¡ ARCHITECTS
CIVIL ENGINEER — BOB D CAMPBELL & COMPANY
GENERAL CONTRACTOR — SM ENGINEERING
MECHANICAL ENGINEER — BRINKMANN CONSTRUCTORS
PLUMBING ENGINEER — LATIMER SOMMERS & ASSOCIATES
ELECTRICAL ENGINEER — LATIMER SOMMERS & ASSOCIATES
LATIMER SOMMERS & ASSOCIATES

STATE OF MISSOURI — CHAD A. WYSE — BUSINESS — NUMBER PE-2010026837 — 12/14/2020 — PROFESSIONAL ENGINEER

Tegethoff DEVELOPMENT

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DATE: 7.15.2021

REVISIONS 1

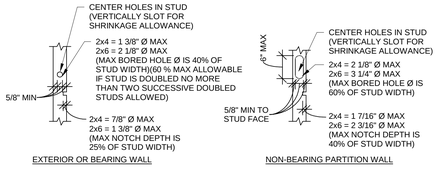
DWG BY CB

TR¡ PROJECT NO. 20-001

SHEET NO.

S002

SCHEDULES & CMU DETAILS

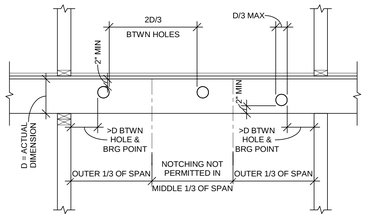


TYPICAL NOTES FOR BEARING WALLS

1. HOLES SHALL NOT BE LOCATED IN THE SAME STUD AS A CUT OR NOTCH
2. CONTACT ARCHITECT PRIOR TO CUTTING OR NOTCHING TO VERIFY SIZE AND LOCATION IF HOLE IS GREATER THAN 20% STUD WIDTH OR NOTCHES GREATER THAN 10% STUD WIDTH ARE REQUIRED IN TWO OR MORE CONSECUTIVE STUDS
3. NOTCHES OR HOLES NOT PERMITTED IN JAMBS, STUD PACKS AND AT ENDS OF SHEARWALLS
4. STUD SHOES ARE NOTE AN ACCEPTABLE REMEDIATION OF OVER-NOTCHED OR OVER-CUT STUDS WITHOUT PRIOR APPROVAL BY EOR
5. SLOT HOLES VERTICALLY FOR SHRINKAGE ALLOWANCE.

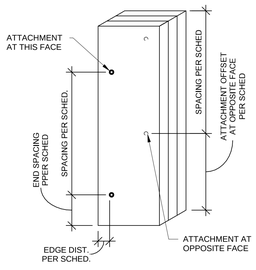
ALLOWABLE HOLES/NOTCHES IN WALL STUDS

1 DETAIL
3/4" = 1'-0"



NOTE:
1. CONTACT ARCHITECT PRIOR TO CUTTING JOISTS TO VERIFY SIZE & LOCATION.
2. DETAIL APPLIES TO 2x FRAMING ONLY. REFER TO ENGINEERED OR COMPOSITE LUMBER MANUFACTURER'S RECOMMENDATIONS AT PSLs, LVLS, LSLs & GLULAM.

2 DETAIL
3/4" = 1'-0"



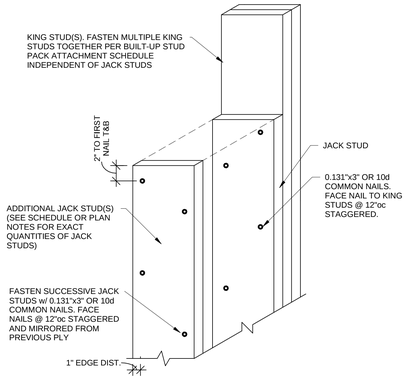
TYPICAL MULTI-PLY STUD CONNECTION

3 DETAIL
1 1/2" = 1'-0"

BUILT-UP STUD PACK COLUMN ATTACHMENT SCHEDULE

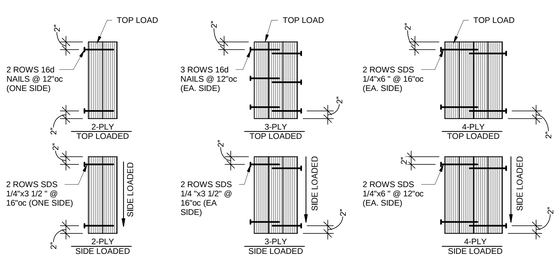
NUMBER OF PLIES	ATTACHMENT AT COLUMN STUD PACKS SUPPORTING BEAMS	ATTACHMENT AT COLUMN STUD PACKS SUPPORTING TRUSSES
2-PLY MEMBERS	8d NAILS AT 12"oc, 1" FROM EDGE, w/ OPPOSITE EDGE NAILED FROM OPPOSITE SIDE OFFSET 6", @ 12"oc w/ FIRST NAIL 2" FROM EA. END	8d NAILS AT 12"oc, 1" FROM EDGE, w/ OPPOSITE EDGE NAILED FROM OPPOSITE SIDE OFFSET 6", @ 12"oc w/ FIRST NAIL 2" FROM EA. END
3-PLY MEMBERS	20d NAILS AT 16"oc, 1 1/2" FROM EDGE w/ OPPOSITE EDGE NAILED FROM OPPOSITE SIDE OFFSET 8", @ 16"oc w/ FIRST NAIL 4" FROM EA. END	8d NAILS AT 12"oc, 1" FROM EDGE, w/ OPPOSITE EDGE NAILED FROM OPPOSITE SIDE OFFSET 6", @ 12"oc w/ FIRST NAIL 2" FROM EA. END
4-PLY MEMBERS	1/4"Øx5" SIMPSON SDS SCREWS AT 16"oc, 1 1/2" FROM EDGE w/ OPPOSITE EDGE SCREWED FROM OPPOSITE SIDE OFFSET 8", @ 16"oc w/ FIRST SCREW 4" FROM EA. END	3 PLIES ATTACHED PER 3-PLY ATTACHMENT w/ 4th PLY ATTACHED w/ 8d NAILS AT 12"oc IN 2 ROWS, 1 1/2" FROM EDGE, OFFSET ROW 6"
5-PLY MEMBERS	1/4"Øx6" SIMPSON SDS SCREWS AT 12"oc, 1 1/2" FROM EDGE w/ OPPOSITE EDGE SCREWED FROM OPPOSITE SIDE OFFSET 6", @ 12"oc w/ FIRST SCREW 4" FROM EA. END	3 PLIES ATTACHED PER 3-PLY ATTACHMENT w/ 4th & 5th PLY ATTACHED w/ 8d NAILS AT 12"oc IN 2 ROWS, 1 1/2" FROM EDGE, OFFSET ROW 6"
6-PLY MEMBERS	1/4"Øx8" SIMPSON SDS SCREWS AT 12"oc, 1 1/2" FROM EDGE w/ OPPOSITE EDGE SCREWED FROM OPPOSITE SIDE OFFSET 6", @ 12"oc w/ FIRST SCREW 4" FROM EA. END	3 PLIES ATTACHED PER 3-PLY ATTACHMENT w/ 4th PLY ATTACHED w/ 8d NAILS AT 12"oc IN 2 ROWS, 1 1/2" FROM EDGE, OFFSET ROW 6" AND 5th AND 6th PLIES ATTACHED w/ 1/4"Øx5" SIMPSON SDS SCREWS @ 12"oc IN 2 ROWS, 1 1/2" FROM EDGE, OFFSET ROSS 6"oc w/ FIRST SCREW 4" FROM EA. END

NOTES:
1. ALL BUILT-UP STUD PACKS MUST ALIGN FLOOR-TO-FLOOR WITH SOLID BLOCKING (SQUASH BLOCKS) AT FLOOR CAVATIES.
2. EXTEND ALL STUD PACKS TO COLUMNS UNLESS NOTED OTHERWISE.
3. ALL NAILS ARE COMMON NAILS UNLESS NOTED OTHERWISE.



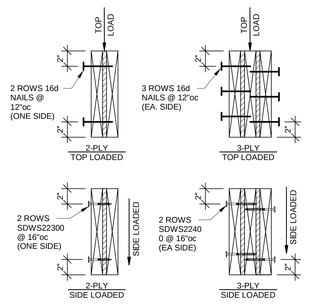
TYPICAL JACK STUD ATTACHMENT

4 DETAIL
1 1/2" = 1'-0"



TYPICAL MULTI-PLY BEAM CONNECTION

5A DETAIL
3/4" = 1'-0"



TYPICAL MULTI-PLY HEADER CONNECTION

5B DETAIL
1 1/2" = 1'-0"

WOOD FLOOR/ROOF FRAMING HEADERS/BEAMS & JAMBS SCHEDULE

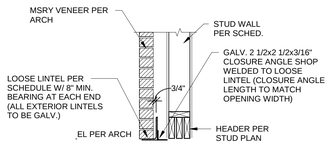
MARK	HEADER	JAMB TYPE # (U.N.O. W/ COLUMN SCHEDULE)			NOTES
		" 3 " (2nd FLOOR FRAMING, TYP U.N.O.)	" 2 " (3rd FLOOR FRAMING, TYP U.N.O.)	" 1 " (ROOF FRAMING, TYP U.N.O.)	
A1-#	(2) 2x8 AT 2x4 WALL (3) 2x8 AT 2x6 WALL	1 JACK / 2 KING	1 JACK / 1 KING	1 JACK / 1 KING	
B1-#	(3) 2x8	1 JACK / 1 KING	1 JACK / 1 KING	1 JACK / 1 KING	
C1-#	(3) 2x10	1 JACK / 2 KING	1 JACK / 1 KING	1 JACK / 1 KING	
D1-#	(3) 2x10	3 JACK/ 1 KING EACH SIDE	3 JACK/ 1 KING EACH SIDE	3 JACK/ 1 KING EACH SIDE	
E1-#	3 1/2x14 PLUS PSL	2 JACK/ 1 KING EACH SIDE	2 JACK/ 1 KING EACH SIDE	2 JACK/ 1 KING EACH SIDE	
F1-#	(3) 1 3/4x18 LVL	2 JACK / 2 KING	2 JACK / 2 KING	1 JACK / 1 KING	
G1-#	(4) 2x10	1 JACK/ 2 KING	1 JACK/ 2 KING	1 JACK/ 1 KING	
H1-#	(3) 1 3/4x11 1/4 LVL	2 JACK/ 1 KING	2 JACK/ 1 KING	1 JACK/ 1 KING	

NOTES:
1. JAMB STUDS SHALL MATCH SIZE & GRADE OF WALL STUDS U.N.O.
2. WHERE BEAM IS NOTED "UPSET", ALL JAMB STUDS NOTED WILL EXTEND TO DOUBLE TOP PLATE.
3. ALL EXTERIOR LUMBER TO BE TREATED AGAINST MOISTURE. REFER TO NOTE 11B ON SHEET S001 FOR FIRE RETARDANT TREATED HEADER AND STUD REQUIREMENTS.
4. PROVIDE SQUASH BLOCKS AT TRUSSES & BLOCKING FRAMING WHERE JAMBS OR STUD PACKS ARE DISCONT AND IN TRUSS CAVITY. QUANTITY TO MATCH JAMB OR STUD PACK ABOVE.
5. PROVIDE ½" PLYWOOD SPACER PLATES AT INTERIOR HEADERS CONSTRUCTED WITH 2x LUMBER.
6. AT CONTRACTOR'S OPTION, PROVIDE GLULAM IN LIEU OF PSL OF EQUAL OR GREATER STRENGTH.
7. REFER TO DETAIL 3/S003 FOR MULTI-PLY MEMBER CONNECTION REQUIREMENTS.
8. ATTACH JAMB & KING STUDS TOGETHER PER CONNECTION TYPE 24 ON NAILING SCHEDULE ON S001.

WOOD STUD BEARING WALL SCHEDULE

TYPE	1st FLOOR WALLS (2nd FLOOR FRAMING)	2nd FLOOR WALLS (3rd FLOOR FRAMING)	3rd FLOOR WALLS (ROOF FRAMING)	NOTES
EXTERIOR	2x6 @ 16"oc	2x6 @ 16"oc	2x6 @ 16"oc	
DEMISING	(2) 2x4 @ 16"oc	2x4 @ 16"oc w/ ADDT'L 2x4 @ 32"oc	2x4 @ 16"oc	PROVIDE 2x BLOCKING AT MID HEIGHT BTWN EA. STUD
UNIT INTERIOR	(2) 2x4 @ 16"oc w/ ADDT'L 2x4 @ 32"oc	(2) 2x4 @ 16"oc	2x4 @ 16"oc	
TYPICAL CORRIDOR	2x6 @ 16"oc	2x6 @ 16"oc	2x6 @ 16"oc	
WIDE CORRIDOR	2x6 @ 16"oc w/ ADDT'L 2x6 @ 32"oc	2x6 @ 16"oc	2x6 @ 16"oc	HATCHED THUS ON PLAN:

NOTES:
1. PROVIDE 2x BLOCKING AT MID HEIGHT (5'-0" MAX) AT ALL LOAD BEARING WALLS NOT SHEATHED ON BOTH SIDES.
2. ALL STUDS TO BE No. 2 GRADE U.N.O.
3. RE: 3/S003 FOR NAILING OF MULTIPLE STUDS.
4. REFER TO ARCH/MEP DRAWING FOR LOCATIONS OF FURRED OUT WALLS TO ACCOMODATE PLUMBING OR MEP ITEMS.
5. REFER TO FRAMING PLANS AND ARCH PLANS FOR LEVEL(S) AT WHICH WALLS OCCUR.
6. STACK/ALIGN WALL STUDS FROM FLOOR TO FLOOR AT ALL EXTERIOR WALLS.



TYPICAL LOOSE LINTEL DETAIL

6 TYPICAL LOOSE LINTEL DETAIL
3/4" = 1'-0"

LOOSE LINTEL SCHEDULE

FOR OPENINGS:	GALV. ANGLE
OPENING < 9'-0"	6"x4"x5/16" (LLV)

JOIST/BEAM/TRUSS HANGER SCHEDULE

JOIST/BEAM/TRUSS SIZE	HANGER SIZE	NOTES
2x10	LUS28 HU210	AT CORRIDOR & BALCONY * AT SKEWED CONDITIONS
2x12	LUS210	AT CORRIDOR
(2) 2x12	LUS210-2	AT CORRIDOR
18" TRUSS FACE MOUNT TO UPSET BEAM	LUS410 *HHUS419	* AT SKEWED CONDITIONS

NOTES:
1. HANGERS APPLY TO ALL LOCATIONS WHERE NOT OTHERWISE SPECIFIED IN DETAIL OR PLAN NOTE

ROOF TRUSS HOLDOWN SCHEDULE

TRUSS TYPE	SIMPSON STRONG TIE CONNECTION
COMMON/HIP/JACK AND 1-PLY TRUSSES	H2.5A
1-PLY TRUSS PER TRUSS SHOP DWGS GIRDER TRUSS AS NOTED PER PLAN	(2) H2.5A (ONE EACH SIDE)
2-PLY TRUSS PER TRUSS SHOP DWGS GIRDER TRUSS AS NOTED PER PLAN	LGT2
3-PLY TRUSS PER TRUSS SHOP DWGS OR GIRDER TRUSS AS NOTED PER PLAN	LGT3

NOTES:
1. GC COORDINATE HOLDOWN CONNECTION TYPE, QUANTITY & LOCATION WITH FINAL ROOF TRUSS SHOP DRAWINGS.

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2021-04-30
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CIVIL ENGINEER
GENERAL CONTRACTOR
MECHANICAL ENGINEER
PLUMBING ENGINEER
ELECTRICAL ENGINEER

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REVISIONS 1

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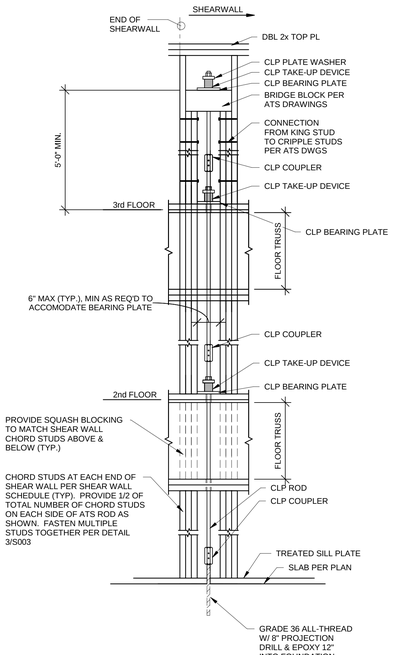
TR¡ PROJECT NO. 20-001

SHEET NO.

S003

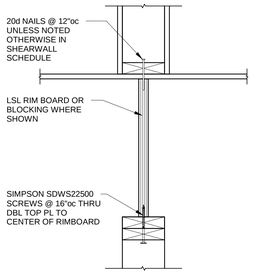
TYPICAL WOOD GRAVITY SCHEDULES & DETAILS



TYPICAL ANCHOR TIEDOWN SYSTEM DETAIL AT EACH END OF SHEAR WALLS

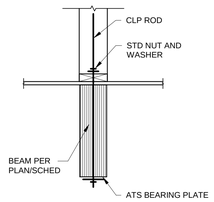
REFER TO PLANS FOR SHEAR WALL LOCATIONS AND TO SHEAR WALL SCHEDULE ON THIS SHEET FOR ADDITIONAL INFORMATION.

1 DETAIL 3/4" = 1'-0"



TYPICAL SHEARWALL SILL PL ATTACHMENT AT 2x SILL

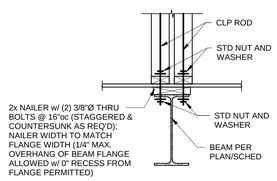
2 SECTION 1 1/2" = 1'-0"



TYPICAL HOLDOWN ATOP WOD BM

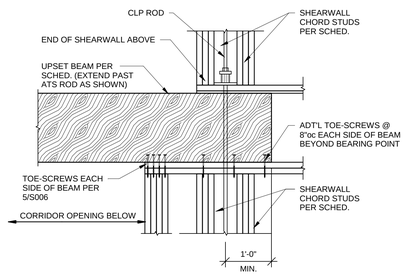
3 SECTION 1" = 1'-0"

NOTE:
IN LIEU OF THRU BOLTING OR WHERE ROD ALIGNS OVER CENTER OF BEAM, A WELDABLE STEEL CAGE OR WELDABLE THREADED ROD MAY BE PROVIDED FOR SHOP OR FIELD WELDING TO BEAM FOR ATS ROD ATTACHMENT



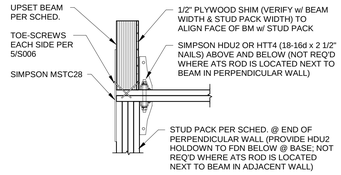
TYPICAL HOLDOWN ATOP STEEL BEAM ABOVE AMENITY SPACE

4 SECTION 1" = 1'-0"



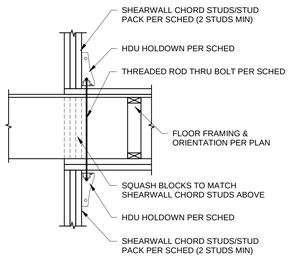
TYPICAL UPSET BEAM NEAR END OF SHEARWALL

5 SECTION 3/4" = 1'-0"



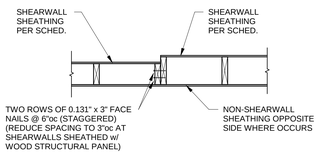
TYPICAL END BEARING AT PERPENDICULAR WALL OF UPSET BEAM SUPPORTING DISCONTINUOUS ATS ROD

6 SECTION 3/4" = 1'-0"



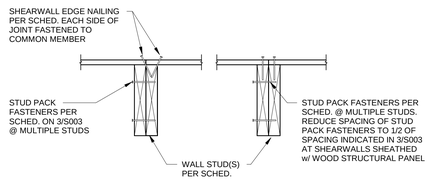
TYPICAL HDU FLOOR TO FLOOR HOLDOWN

7 SECTION 3/4" = 1'-0"



TYPICAL SHEARWALL DETAIL AT WALL STUD SIZE TRANSITION

8 SECTION 3/4" = 1'-0"



TYPICAL SHEARWALL SHEATHING JOINT

9 SECTION 1 1/2" = 1'-0"

SHEARWALL SCHEDULE

SHEARWALL TYPE		FLOOR			PLATE CONNECTION (SILL TO RIM BOARD & RIM BOARD TO TOP PLATE) (RE: NOTES 6 & 7)
		1st FLOOR WALLS	2nd FLOOR WALLS	3rd FLOOR WALLS	
SW1	SHEATHING & FASTENING	5/8" GYPSUM SHEATHING FINISH SIDE, w/ EDGES BLOCKED 6d NAILS @ 7/7 7/16" OSB SHEATHING AIR SIDE, w/ EDGES BLOCKED 6d NAILS @ 6/12	5/8" GYPSUM SHEATHING FINISH SIDE, w/ EDGES BLOCKED 6d NAILS @ 7/7 7/16" OSB SHEATHING AIR SIDE, w/ EDGES BLOCKED 6d NAILS @ 6/12	5/8" GYPSUM SHEATHING FINISH SIDE, w/ EDGES BLOCKED 6d NAILS @ 7/7 7/16" OSB SHEATHING AIR SIDE, w/ EDGES BLOCKED 6d NAILS @ 6/12	
	HOLDOWN	3/8"Ø STANDARD THREADED ROD w/ (2)2x4 OR (2)2x6	3/8"Ø STANDARD THREADED ROD w/ (2)2x4 OR (2)2x6	3/8"Ø STANDARD THREADED ROD w/ (2)2x4 OR (2)2x6	
SW2		5/8" GYPSUM SHEATHING FINISH SIDE, w/ EDGES BLOCKED 6d NAILS @ 7/7 7/16" OSB SHEATHING AIR SIDE, w/ EDGES BLOCKED 6d NAILS @ 6/12	5/8" GYPSUM SHEATHING FINISH SIDE, w/ EDGES BLOCKED 6d NAILS @ 7/7 7/16" OSB SHEATHING AIR SIDE, w/ EDGES BLOCKED 6d NAILS @ 6/12	5/8" GYPSUM SHEATHING FINISH SIDE, w/ EDGES BLOCKED 6d NAILS @ 7/7 7/16" OSB SHEATHING AIR SIDE, w/ EDGES BLOCKED 6d NAILS @ 6/12	2nd FLR - 20d NAILS @ 8"oc
	HOLDOWN	5/8"Ø STANDARD THREADED ROD w/ (6)2x4 OR (4)2x6	3/8"Ø STANDARD THREADED ROD w/ (2)2x4 OR (2)2x6	3/8"Ø STANDARD THREADED ROD w/ (2)2x4 OR (2)2x6	
SW3		5/8" GYPSUM SHEATHING FINISH SIDE, w/ EDGES BLOCKED 6d NAILS @ 7/7 7/16" OSB SHEATHING AIR SIDE, w/ EDGES BLOCKED 6d NAILS @ 6/12	5/8" GYPSUM SHEATHING FINISH SIDE, w/ EDGES BLOCKED 6d NAILS @ 7/7 7/16" OSB SHEATHING AIR SIDE, w/ EDGES BLOCKED 6d NAILS @ 6/12	5/8" GYPSUM SHEATHING FINISH SIDE, w/ EDGES BLOCKED 6d NAILS @ 7/7 7/16" OSB SHEATHING AIR SIDE, w/ EDGES BLOCKED 6d NAILS @ 6/12	3rd FLR - 20d NAILS @ 8"oc 2nd FLR - 20d NAILS @ 6"oc
	HOLDOWN	5/8"Ø STANDARD THREADED ROD w/ (6)2x4 OR (4)2x6	3/8"Ø STANDARD THREADED ROD w/ (2)2x4 OR (2)2x6	3/8"Ø STANDARD THREADED ROD w/ (2)2x4 OR (2)2x6	

NOTES:
1. NAILING SHALL BE TO ALL STUDS, TOP & BOTTOM PLATES, AND BLOCKING WHERE INDICATED. NAILS FOR GYPSUM SHEATHING ARE COOLER NAILS AND NAILS FOR OSB SHEATHING ARE COMMON NAILS. GYPSUM CAN BE ATTACHED WITH DRYWALL SCREWS AT SAME SPACING INDICATED FOR NAILS.
2. HOLDOWNS PER PLAN & SCHEDULE.
3. WHERE THE ENDS OF PERPENDICULAR SHEAR WALLS INTERSECT AND ONLY ON HOLDOWN SHOWN ON PLAN, FASTEN ALL STUDS TOGETHER PER SCHEDULE AND USE LARGER OF THE TWO HOLDOWNS SHOWN IN THE SHEARWALL SCHEDULE.
4. REFER TO HOLDOWN SCHEDULE FOR NUMBER OF STUDS REQ'D AT EA END OF THE SHEARWALL
5. NAIL AND STAPLE SPACING SHOWN AS (#/#) INDICATES FASTENERS SPACING IN INCHES AT THE EDGES/FIELD WHERE FIELD IS THE INTERMEDIATE MEMBERS.
6. TYPICAL SILL PLATE TO WOOD (RIM BOARD) SHALL BE 20d NAILS AT 12"oc UNLESS NOTED OTHERWISE IN SCHEDULE. TYPICAL WOOD (RIM BOARD) TO TOP PLATES SHALL BE SDWS22500 SCREWS @ 16"oc
7. TYPICAL SILL PLATE TO CONCRETE SHALL BE 1/2"Øx6" Lg SIMPSON TITEN HD ANCHOR:
   AT 2x4 WALLS SPACE AT 24"oc MAX WITH 1/4"x2 1/2"x2 1/2" PLATE WASHER OR SIMPSON BPS1/2-3 @ CONTRACTORS OPTION
   AT 2x6 WALLS SPACE AT 24"oc MAX WITH 1/4"x2 1/2"x4 1/2" PLATE WASHER OR SIMPSON BPS1/2-6 @ CONTRACTORS OPTION
   AT 2x8 WALLS STAGGER AT 18"oc MAX WITH 1/4"x2 1/4"x2 1/2" PLATE WASHER OR SIMPSON BPS1/2-3 @ CONTRACTORS OPTION
8. PLATE WASHERS TO MAINTAIN MAX OF 1/2" BETWEEN EDGE OF SILL PLATE AND EDGE OF PLATE WASHER.
9. OSB @ INTERIOR WALL SHALL BE IN ADDITION TO 5/8" GYP SHEATHING.
10. SHEARWALL SHEATHING CALLED OUT AT CORRIDOR WALLS SHALL BE LOCATED AT UNIT SIDE OF WALL.
11. HOLDOWN TYPES ARE BASED UPON CONTINUOUS THREADED RODS UTILIZING CLP SYSTEMS.
12. RODS SHALL HAVE THE FOLLOWING MINIMUM TENSILE CAPACITY:
   A. 3/8"Ø = 2,900 LBS
   B. 1/2"Ø = 4,270 LBS
   C. 5/8"Ø = 6,675 LBS
   D. 3/4"Ø = 9,610 LBS
13. REFER TO SECTION DETAILS ON S004 FOR TYPICAL HOLDOWN DETAILS.
14. ALL RODS TO HAVE HALF OF THE LISTED REQ'D STUDS EA SIDE OF THREADED ROD TO MATCH STUD SIZE & GRADE NOTED IN WALL SCHEDULE. CHORD STUDS ARE IN ADDITION TO BEARING WALL OR HEADER JAMB STUDS - PROVIDE ADDITIONAL STUDS AS REQ'D TO MEET QUANTITY NOTED IN SCHED. OFFSET STUD PACK 3" TYPICAL FROM CENTERLINE OF THREADED ROD. PROVIDE SQUASH BLOCKS WITHING FLOOR PLATE DEPTH (TRUSS DEPTH) ALIGNED WITH STUD PACKS. QUANITY OF SQUASH BLOCK TO MATCH QUANITY OF STUDS BELOW.
15. PROVIDE PLATE WASHER AND NUT CAPABLE OF DEVELOPING CAPACITY OF ROD AT EACH FLOOR.
16. PROVIDE TAKE-UP DEVICE AT EACH FLOOR CAPABLE OF ACCOMODATING THE SHRINKAGE INDICATED IN DETAIL 1/S004.
17. PROVIDE COUPLING TAKE-UP DEVICE AS REQUIRED.
18. PROVIDE SHOP DRAWINGS SHOWING LOCATIONS OF ALL HOLD-DOWNS AND HARDWARE FOR REVIEW BY THE EOR PRIOR TO INSTALLATION.
19. THE HOLE THRU THE TOP AND SILL PLATES SHALL BE EQUAL TO THE ROD DIAMETER PLUS 1/4".

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SM ENGINEERING — CIVIL ENGINEER
BRINKMANN CONSTRUCTORS — GENERAL CONTRACTOR
LATIMER SOMMERS & ASSOCIATES — MECHANICAL ENGINEER
LATIMER SOMMERS & ASSOCIATES — PLUMBING ENGINEER
LATIMER SOMMERS & ASSOCIATES — ELECTRICAL ENGINEER

STATE OF MISSOURI — CHAD A. BUSHNELL — NUMBER PE-2010026837 — 12/14/2020 — PROFESSIONAL ENGINEER

Tegethoff DEVELOPMENT

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REVISIONS 1

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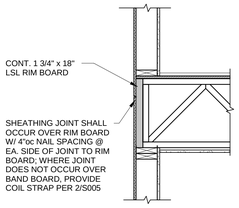
TR¡ PROJECT NO. 20-001

SHEET NO.

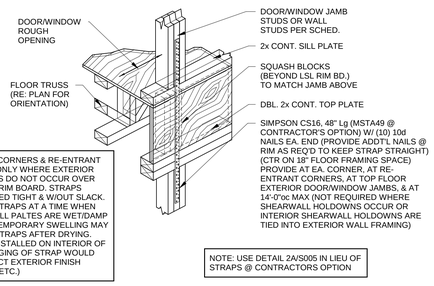
S004

TYPICAL WOOD LATERAL SCHEDULES & DETAILS



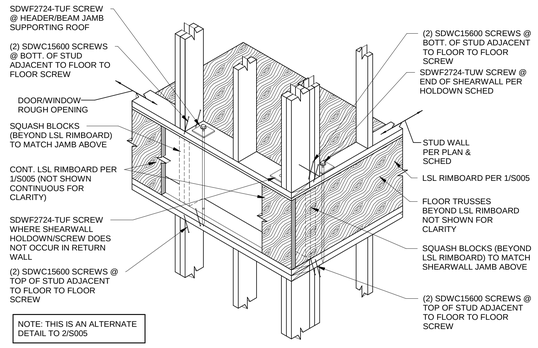
TYPICAL EXTERIOR SHEATHING JOINT

1 SECTION 3/4" = 1'-0"



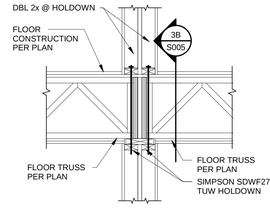
TYPICAL COIL STRAP @ EXTERIOR JAMBS SUPPORTING ROOF FRAMING AT FLOOR DIRECTLY BELOW ROOF AND FLOOR TO FLOOR TIES WHERE DETAIL 1/S005 IS NOT FOLLOWED

2 SECTION 3/4" = 1'-0"



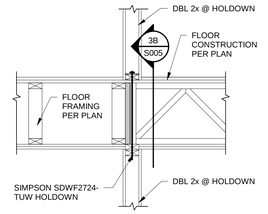
TYPICAL FLOOR-TO-FLOOR SCREW @ EXTERIOR JAMBS SUPPORTING ROOF FRAMING AT FLOOR DIRECTLY BELOW ROOF AND FLOOR TO FLOOR TIES WHERE DETAIL 1/S005 IS NOT FOLLOWED

2A SECTION 3/4" = 1'-0"



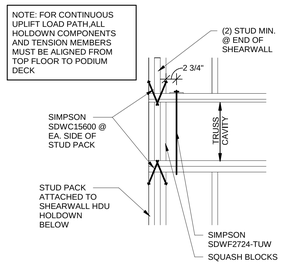
TYPICAL UNIT UPLIFT HOLDOWN DETAIL @ TOP FLOOR ROOF TRUSS BEARING WALLS

3 SECTION 3/4" = 1'-0"



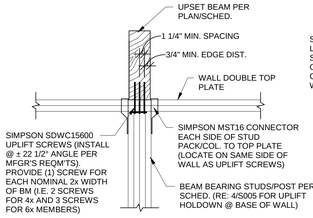
TYPICAL UNIT UPLIFT HOLDOWN DETAIL @ TOP FLOOR ROOF TRUSS BEARING WALLS

3A SECTION 3/4" = 1'-0"

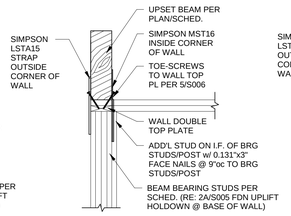


ALTERNATE FLOOR TO FLOOR TIE-DOWN AT END OF WALL

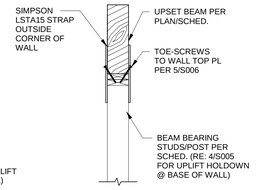
5 SECTION 3/4" = 1'-0"



TYPICAL AT PERPENDICULAR WALL



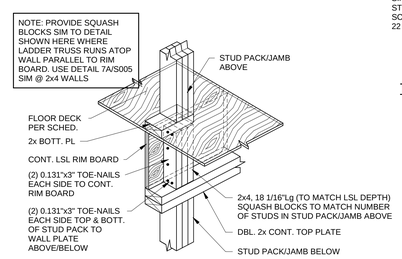
TYPICAL AT WALL CORNERS



TYPICAL AT PARALLEL WALL

TYPICAL UPSET WOOD BEAM UPLIFT HOLDOWN AT BEAMS SUPPORTING ROOF TRUSS BEARING WALLS OR WHERE INDICATED

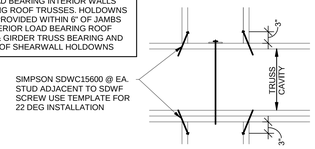
6 SECTION 3/4" = 1'-0"



TYPICAL SQUASH BLOCK DETAIL AT CONTINUOUS RIM BOARD

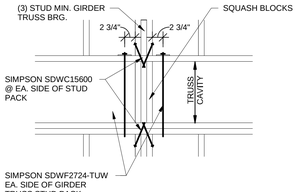
7 SECTION 3/4" = 1'-0"

PROVIDE UNIT UPLIFT HOLDOWNS @ 48"oc MAX @ LOAD BEARING INTERIOR WALLS SUPPORTING ROOF TRUSSES. HOLDOWNS SHALL BE PROVIDED WITHIN 6" OF JAMBS OF ALL INTERIOR LOAD BEARING ROOF HEADERS & GIRDER TRUSS BEARING AND WITHIN 48" OF SHEARWALL HOLDOWNS



TYP SIDE VIEW AT DBL STUD SCREWS AT HOLDOWN

3B DETAIL 3/4" = 1'-0"



TYPICAL GIRDER TRUSS UPLIFT HOLDOWN DETAIL @ TOP FLOOR

4 SECTION 3/4" = 1'-0"

TYPICAL SQUASH BLOCK DETAIL AT LSL BLOCKING

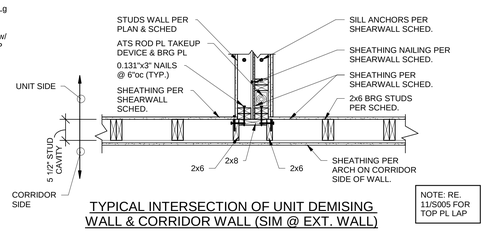
7A SECTION 3/4" = 1'-0"

TYPICAL HDU HOLDOWN DETAIL

8 SECTION 3/4" = 1'-0"

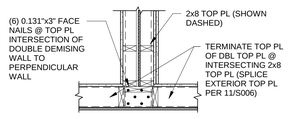
TYPICAL @ UNIT PARTY WALL TRANSITION TO 2x6 EXT. WALL

9 SECTION 3/4" = 1'-0"



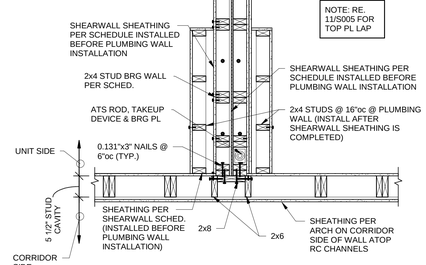
TYPICAL INTERSECTION OF UNIT DEMISING WALL & CORRIDOR WALL (SIM @ EXT. WALL)

10 SECTION 3/4" = 1'-0"



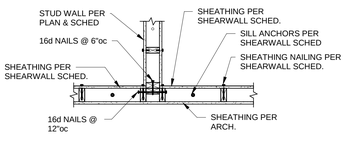
TYPICAL TOP PL LAP @ UNIT DEMISING WALL AT DISCONT. END

11 SECTION 3/4" = 1'-0"



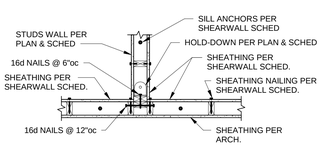
TYPICAL INTERSECTION OF CORRIDOR WALL AND UNIT DEMISING WALL w/ PLUMBING WALL

12 SECTION 3/4" = 1'-0"

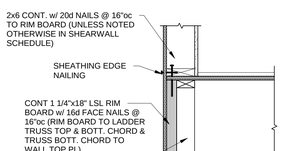


TYPICAL @ DISCONTINUOUS SHEARWALL SHEATHING

13 SECTION 3/4" = 1'-0"

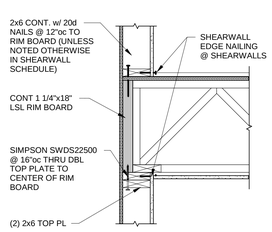


15 SECTION 3/4" = 1'-0"



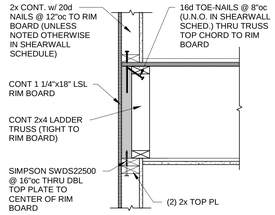
TYPICAL LATERAL NAILING AT EXTERIOR WALL PARALLEL TO TRUSSES

16A SECTION 1" = 1'-0"



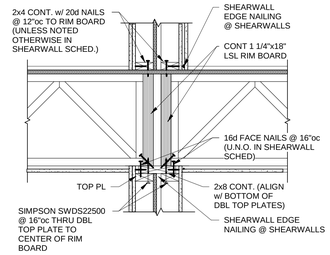
TYPICAL LATERAL NAILING AT ONE SIDED WALL WITH TRUSS BEARING

16B SECTION 1" = 1'-0"



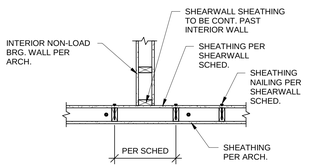
TYPICAL LATERAL NAILING AT UNIT DEMISING WALL OR ONE SIDED WALL PARALLEL TO TRUSSES

16C SECTION 1" = 1'-0"



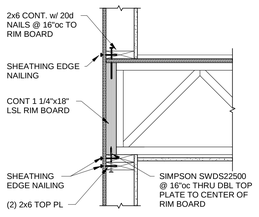
TYPICAL LATERAL NAILING AT DOUBLE UNIT DEMISING WALLS WITH TRUSS BEARING

16D SECTION 1" = 1'-0"



TYPICAL @ SHEARWALL SHEATHING CONTINUOUS PAST NON-LOAD BRG WALL

14 SECTION 3/4" = 1'-0"



TYPICAL LATERAL NAILING AT EXTERIOR WALL WITH TRUSS BEARING

16 SECTION 1" = 1'-0"

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LATIMER SOMMERS & ASSOCIATES

ARCHITECT
STRUCTURAL ENGINEER
CIVIL ENGINEER
GENERAL CONTRACTOR
MECHANICAL ENGINEER
PLUMBING ENGINEER
ELECTRICAL ENGINEER

STATE OF MISSOURI — CHAD A. BOSS — BUSINESS — NUMBER PE-2010026837 — 12/14/2021 — PROFESSIONAL ENGINEER

Together Holdt DEVELOPMENT

The Signature at West Pryor

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REVISIONS 1

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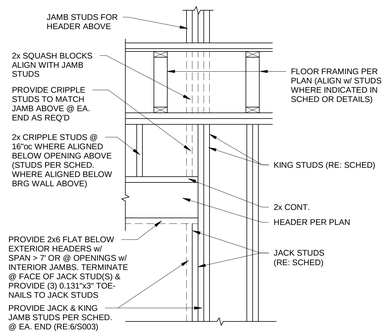
TR¡ PROJECT NO. 20-001

SHEET NO.

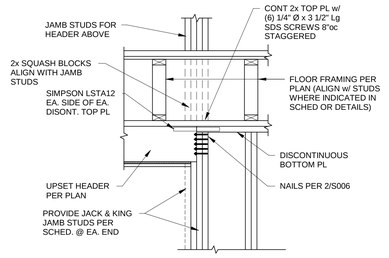
S005

TYPICAL WOOD DETAILS



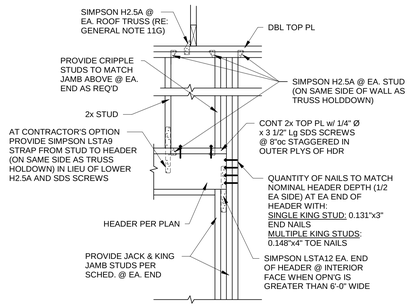
TYPICAL HEADER DETAIL

1 SECTION 3/4" = 1'-0"



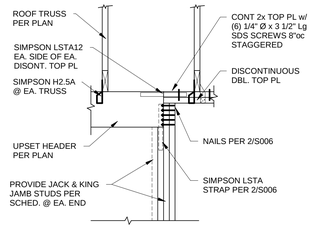
TYPICAL HEADER DETAIL AT DISCONTINUOUS TOP PLATE AT FLOOR

1A SECTION 3/4" = 1'-0"



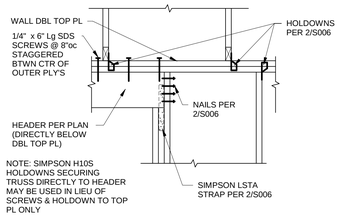
TYPICAL HEADER DETAIL @ ROOF TRUSS BRG LOCATIONS

2 SECTION 3/4" = 1'-0"



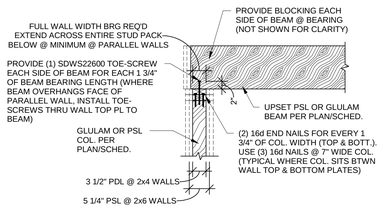
TYPICAL HEADER DETAIL AT DISCONTINUOUS TOP PLATE AT ROOF

3 SECTION 3/4" = 1'-0"



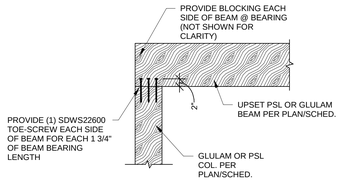
TYPICAL HEADER DETAIL AT ROOF TRUSS BRG LOCATIONS w/HEADER DIRECTLY BELOW DBL TOP PL

4 SECTION 3/4" = 1'-0"



TYPICAL PSL OR GLULAM BEAM AND/OR COLUMN DETAIL

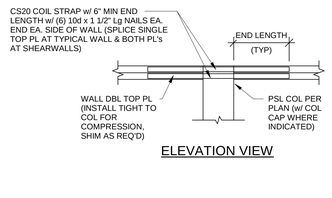
5 SECTION 3/4" = 1'-0"



TYPICAL PSL OR GLULAM BEAM BEARING DIRECTLY ON GLULAM OR PSL COLUMN DETAIL

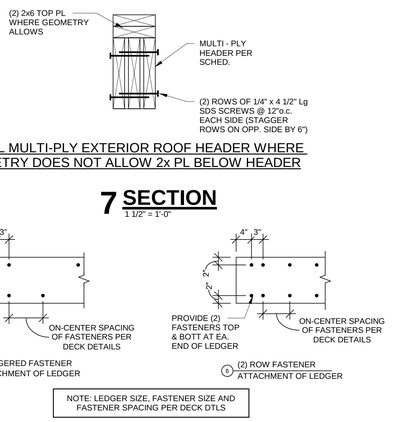
NOTES:
1. REFER TO S/S006 WHERE COL IS SET BTWN WALL TOP & BOTTOM PLATES
2. PROVIDE STRAPS PER 6/S0.06 AT DISCONTINUOUS WALL TOP PLATES.

5A SECTION 3/4" = 1'-0"



TYPICAL STRAPS @ DISCONTINUOUS TOP PL @ COL

6 SECTION 3/4" = 1'-0"

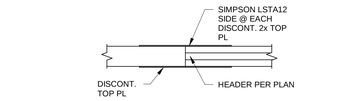


TYPICAL MULTI-PLY EXTERIOR ROOF HEADER WHERE GEOMETRY DOES NOT ALLOW 2x PL BELOW HEADER

7 SECTION 1 1/2" = 1'-0"

TYPICAL LEDGER CONNECTION

8 SECTION 3/4" = 1'-0"

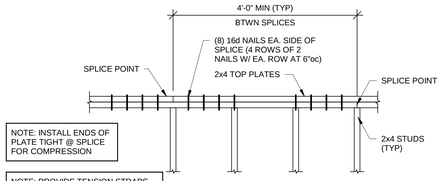


TYPICAL HOLE THRU LEDGER DETAIL

9 SECTION 3/4" = 1'-0"

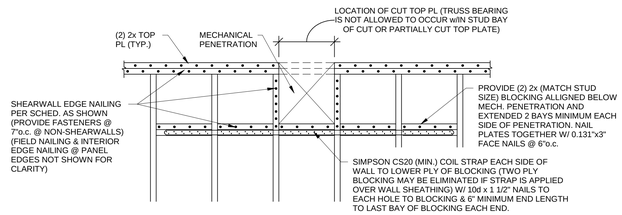
TYPICAL SPLICE AT DISCONTINUOUS TOP PL

10 SECTION 3/4" = 1'-0"



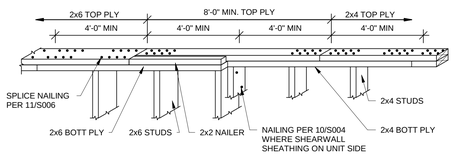
TYPICAL TOP PLATE SPLICE

11 SECTION 3/4" = 1'-0"



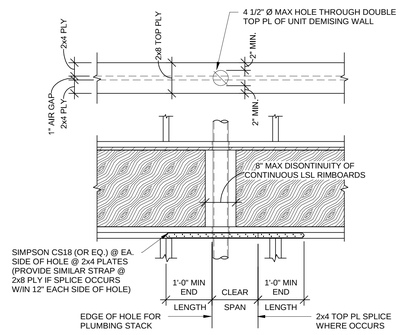
TYPICAL TOP PLATE SPLICE AT SHEARWALL OR BEARING WALL DUE TO MECH. PENETRATION

11A SECTION 3/4" = 1'-0"



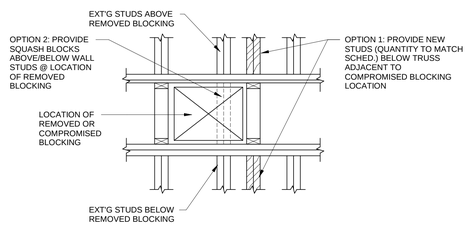
TYPICAL TOP PLATE SPLICE AT WALL SIZE TRANSITION

12 SECTION 3/4" = 1'-0"



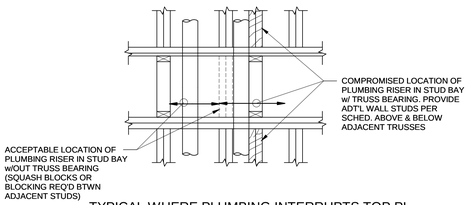
TYPICAL DETAIL AT PLUMBING STACK IN DOUBLE UNIT DEMISING WALLS

13 SECTION 3/4" = 1'-0"



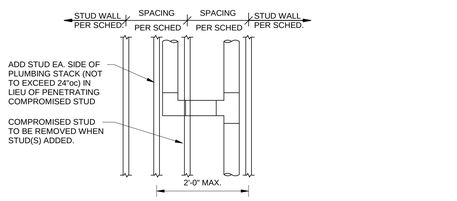
TYPICAL WHERE MECHANICAL DUCT INTERRUPTS LSL

14 SECTION 3/4" = 1'-0"



TYPICAL WHERE PLUMBING INTERRUPTS TOP PL

15 SECTION 3/4" = 1'-0"



TYPICAL WHERE PLUMBING STACK HAS HORIZONTAL JOG

16 SECTION 3/4" = 1'-0"

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ARCHITECT
STRUCTURAL ENGINEER
CIVIL ENGINEER
GENERAL CONTRACTOR
MECHANICAL ENGINEER
PLUMBING ENGINEER
ELECTRICAL ENGINEER

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DATE: 7.15.2021

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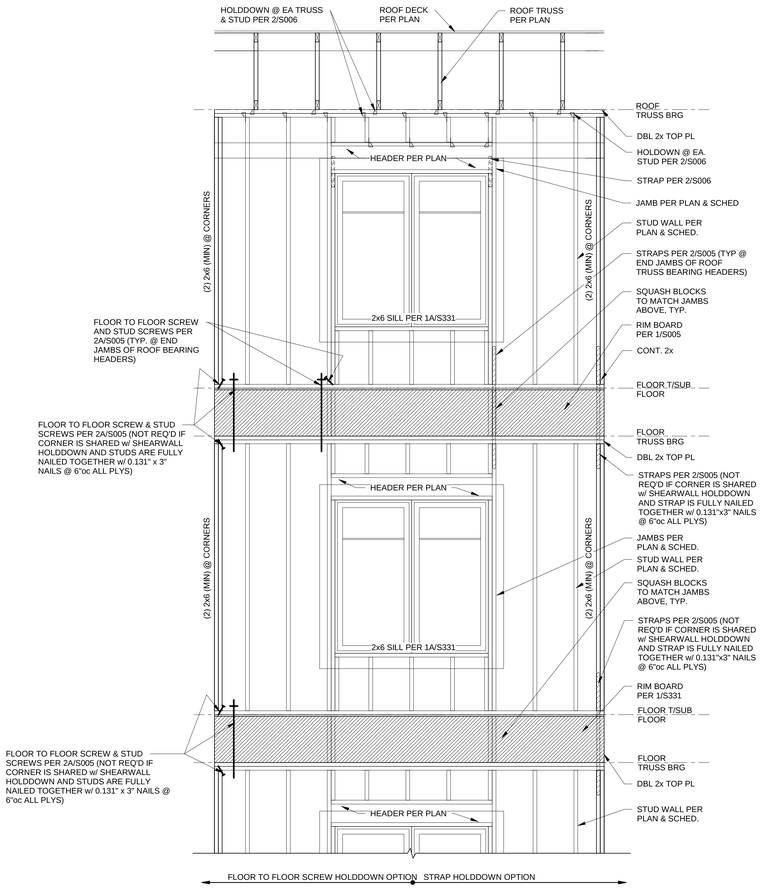
TR¡ PROJECT NO. 20-001

SHEET NO.

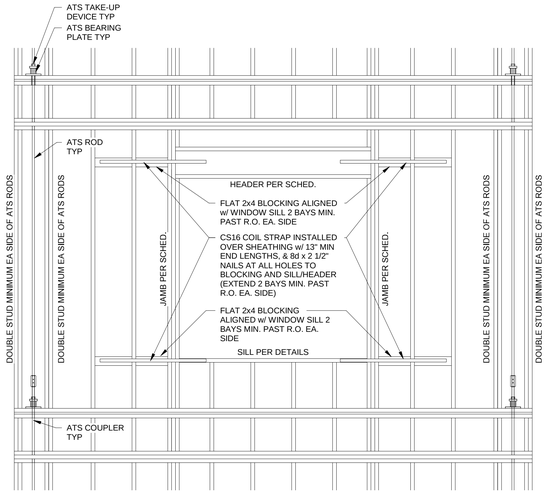
S006

TYPICAL WOOD DETAILS



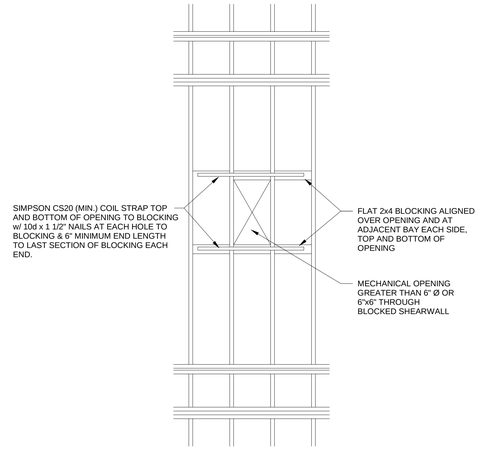
TYPICAL EXTERIOR ELEVATION OF STRUCTURAL BAY @ TYPE V CONSTRUCTION

1 ELEVATION
1/2" = 1'-0"



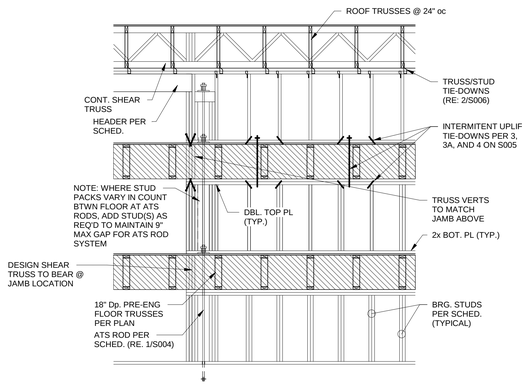
TYPICAL FORCE TRANSFER SHEARWALL ELEVATION @ WINDOW OPENINGS

2 ELEVATION
1/2" = 1'-0"



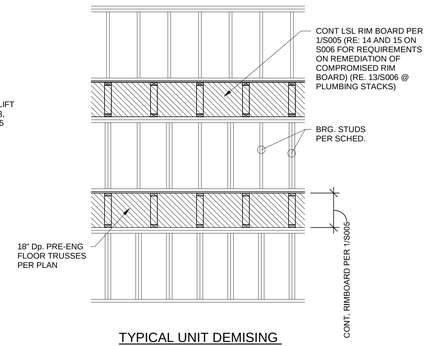
TYPICAL OPENING THROUGH BLOCKED SHEARWALL

3 ELEVATION
1/2" = 1'-0"



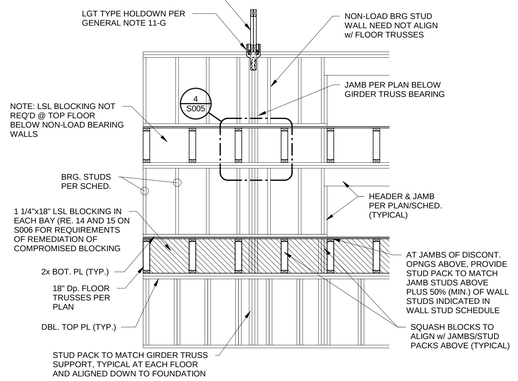
TYPICAL CORRIDOR BEARING WALL ELEVATION

A ELEVATION
3/8" = 1'-0"



TYPICAL UNIT DEMISING WALL ELEVATION

B ELEVATION
3/8" = 1'-0"



TYPICAL INTERIOR BEARING WALL ELEVATION

C ELEVATION
3/8" = 1'-0"

STATE OF MISSOURI
CHAD A. BUSH
BUSINESS
NUMBER
PE-2010026832
12/14/21
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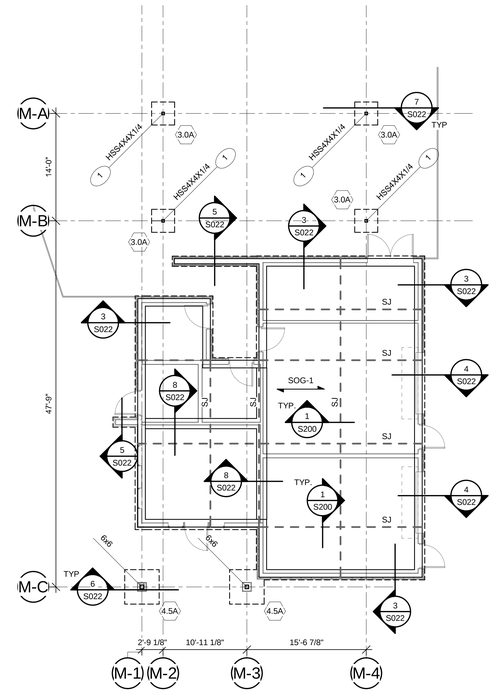
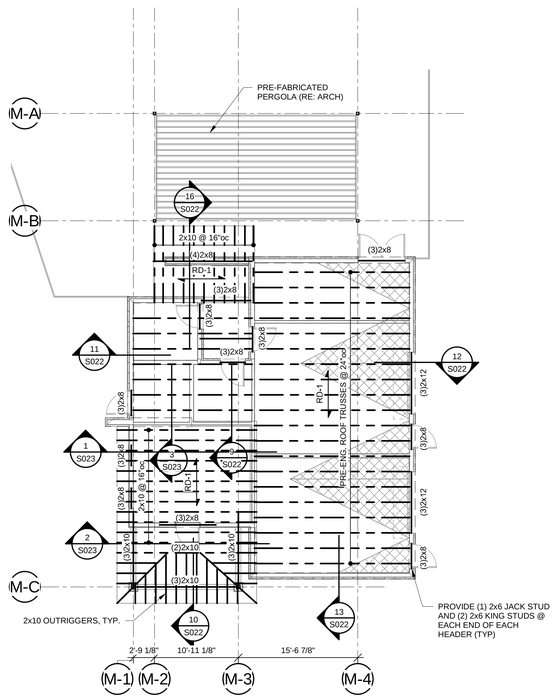
SHEET NO.

S007

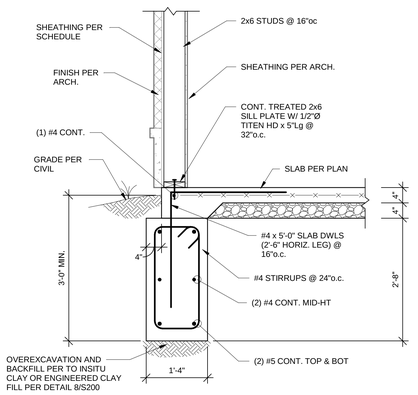
TYPICAL WOOD DETAILS



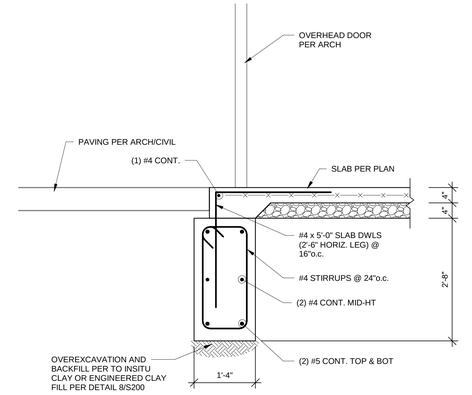
1 FOUNDATION PLAN
1/8" = 1'-0"



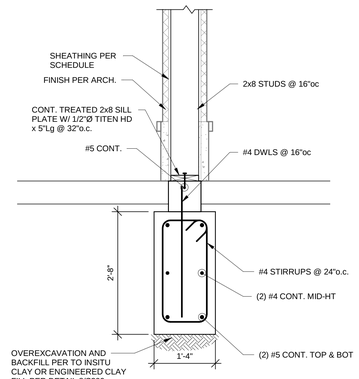
2 ROOF FRAMING PLAN
1/8" = 1'-0"



3 SECTION
3/4" = 1'-0"



4 SECTION
3/4" = 1'-0"



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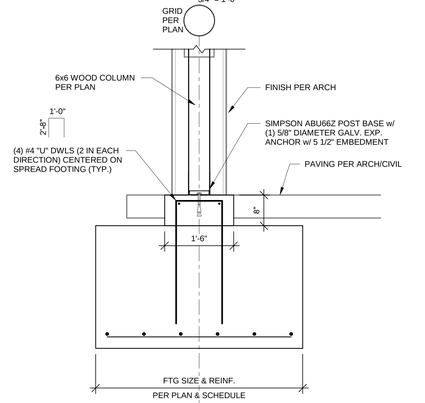
3/4" = 1'-0"



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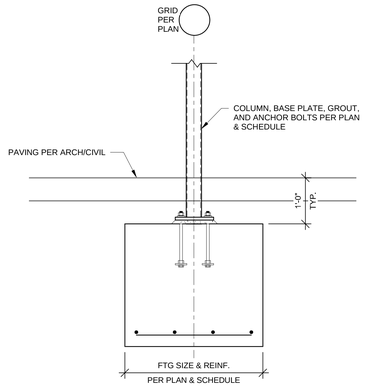
3/4" = 1'-0"



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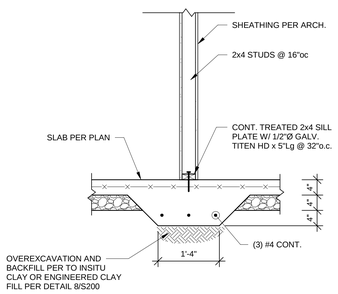
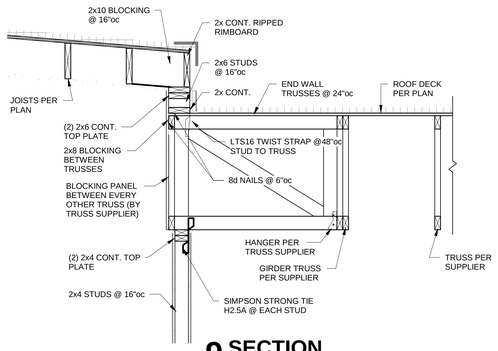
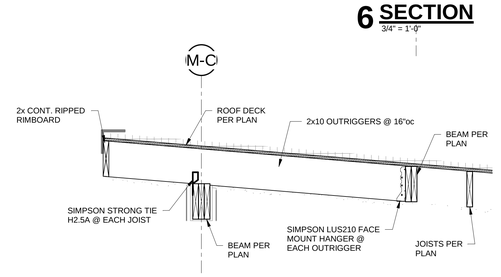
3/4" = 1'-0"



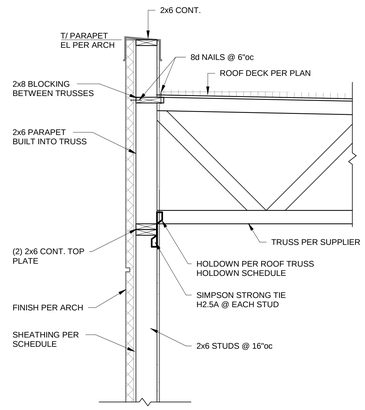
8 SECTION
3/4" = 1'-0"



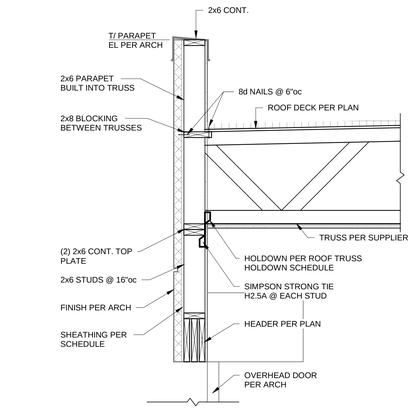
9 SECTION
3/4" = 1'-0"



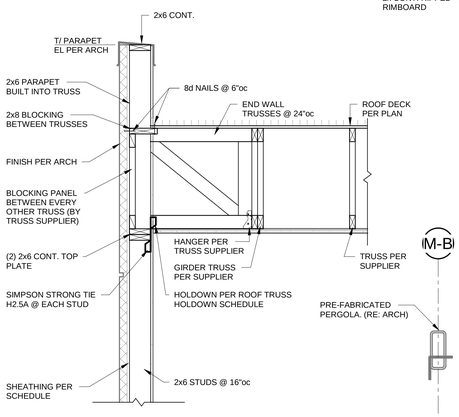
10 SECTION
3/4" = 1'-0"



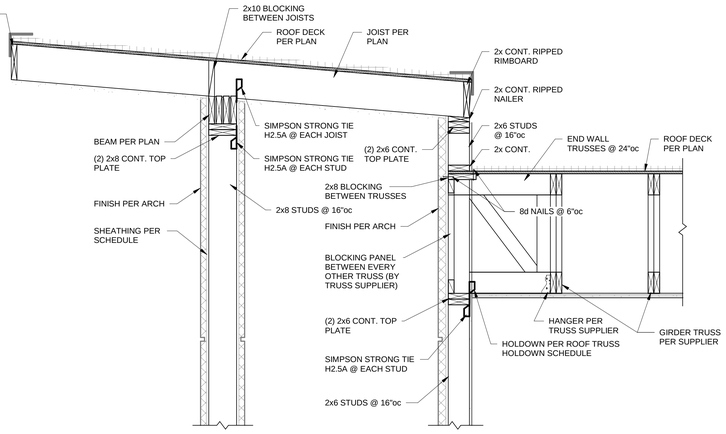
11 SECTION
3/4" = 1'-0"



12 SECTION
3/4" = 1'-0"



13 SECTION
3/4" = 1'-0"



16 SECTION
3/4" = 1'-0"

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1 PERMIT COMMENTS	8.17.2021
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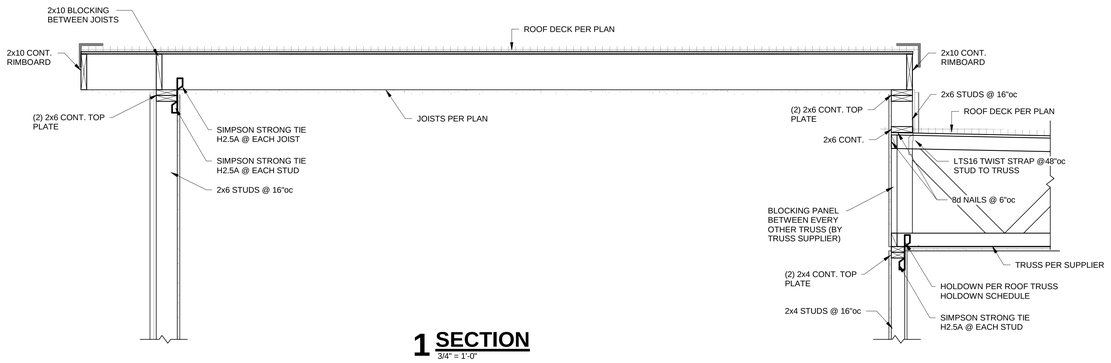
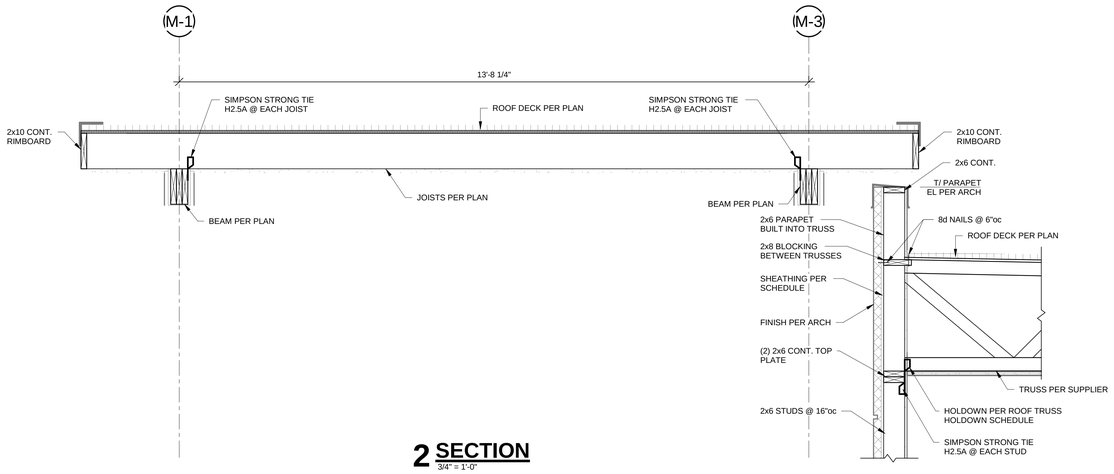
SHEET NO.	S022

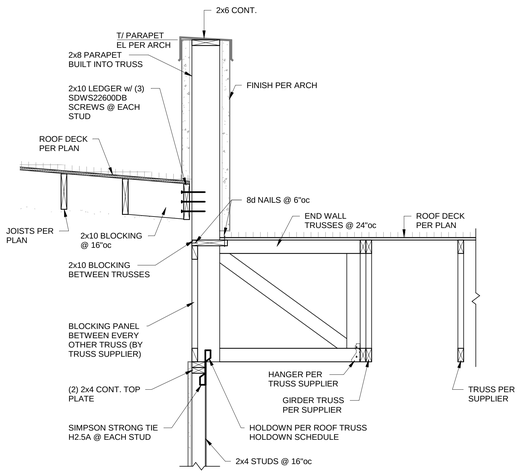
1 MAINTENANCE SHED/POOL HOUSE



1 SECTION
3/4" = 1'-0"



2 SECTION
3/4" = 1'-0"



3 SECTION
3/4" = 1'-0"

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PLUMBING ENGINEER
ELECTRICAL ENGINEER

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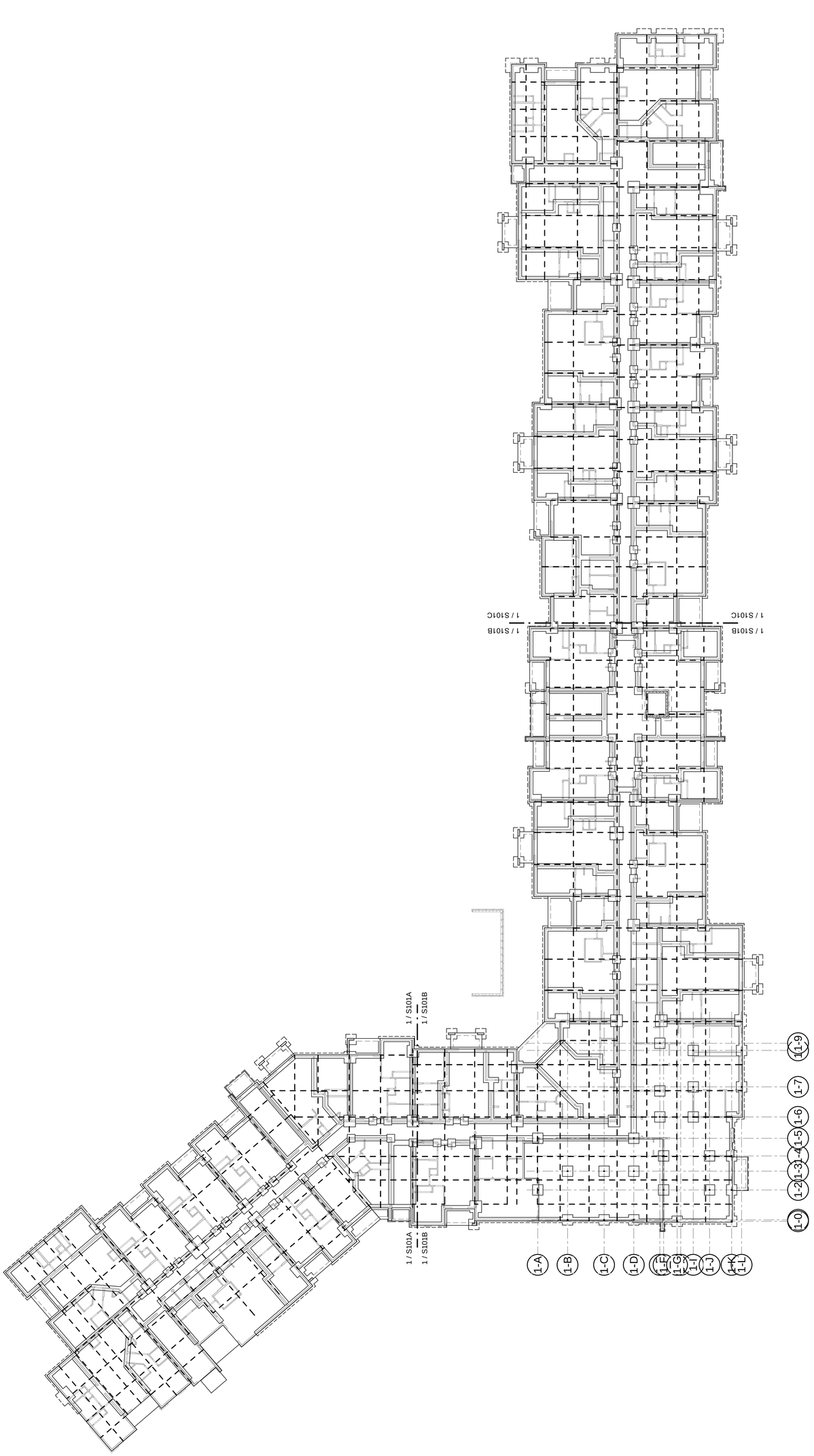
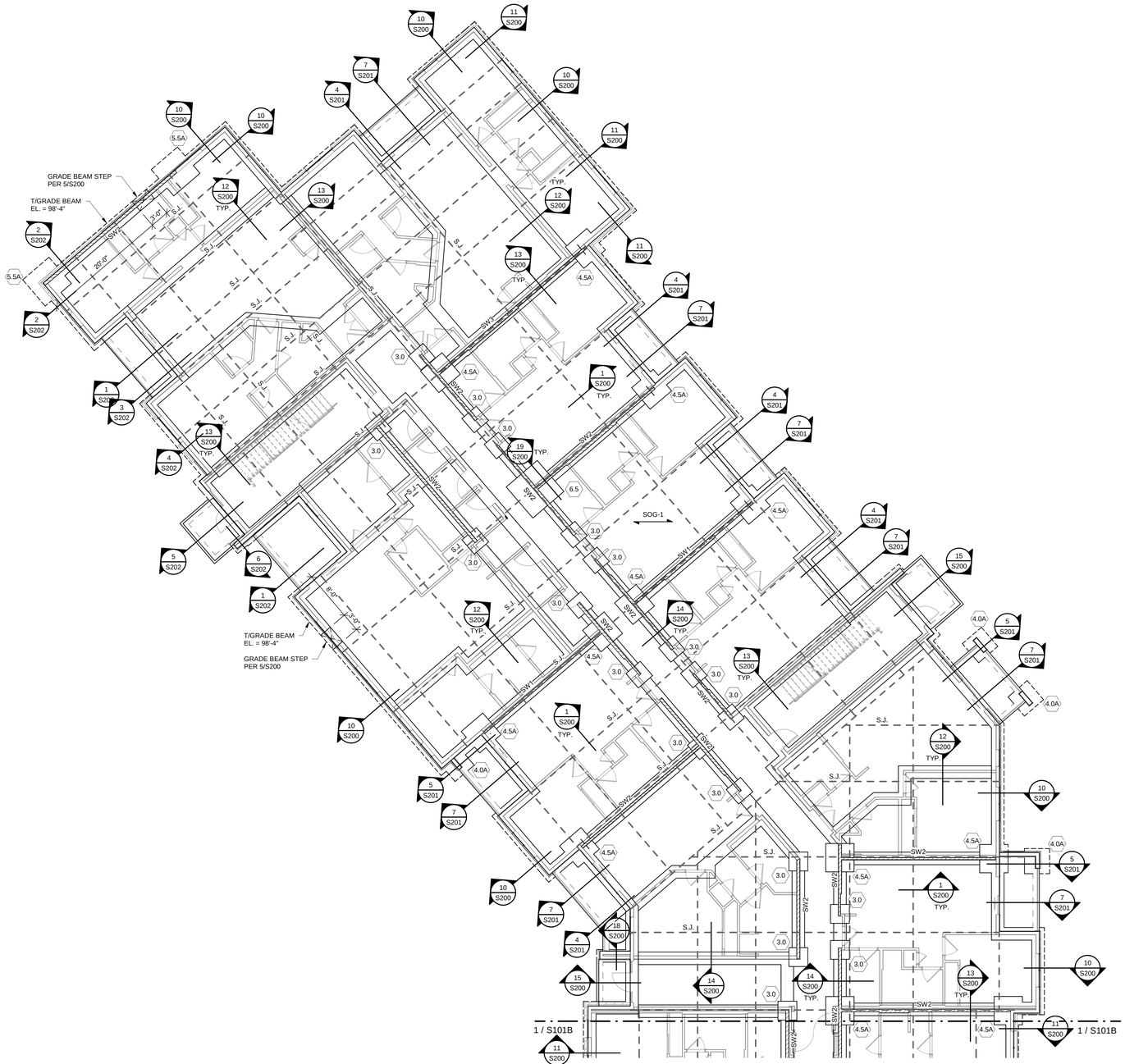
SHEET NO. S101
FOUNDATION PLAN



1 Bldg 1-1-FIRST FLOOR FRAMING PLAN - OVERALL
1/16" = 1'-0"



1 1-FIRST FLOOR FRAMING PLAN - Area A

1/8" = 1'-0"

N

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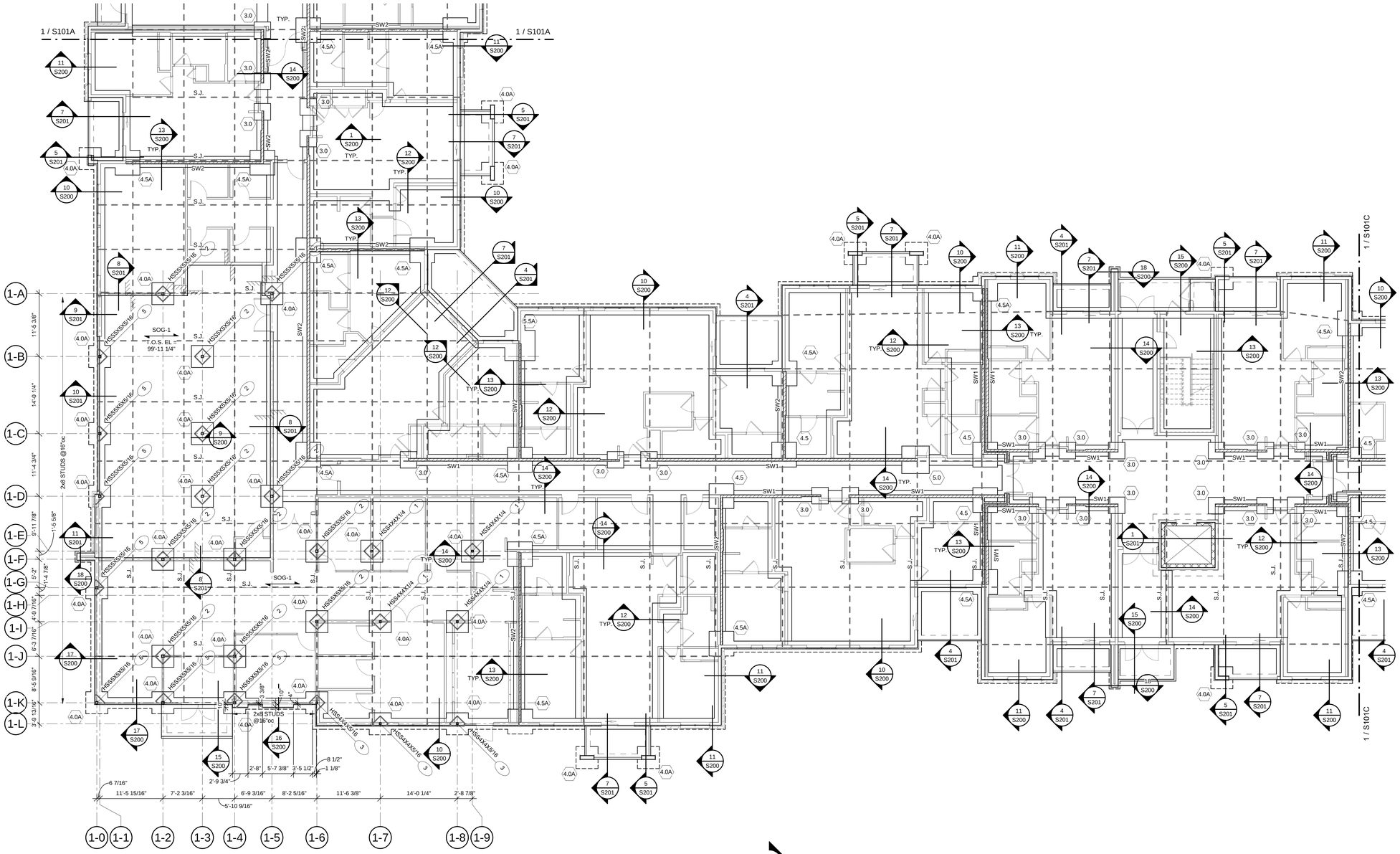
FOUNDATION PLAN - AREA A



1 1-FIRST FLOOR FRAMING PLAN - Area B

1/8" = 1'-0"

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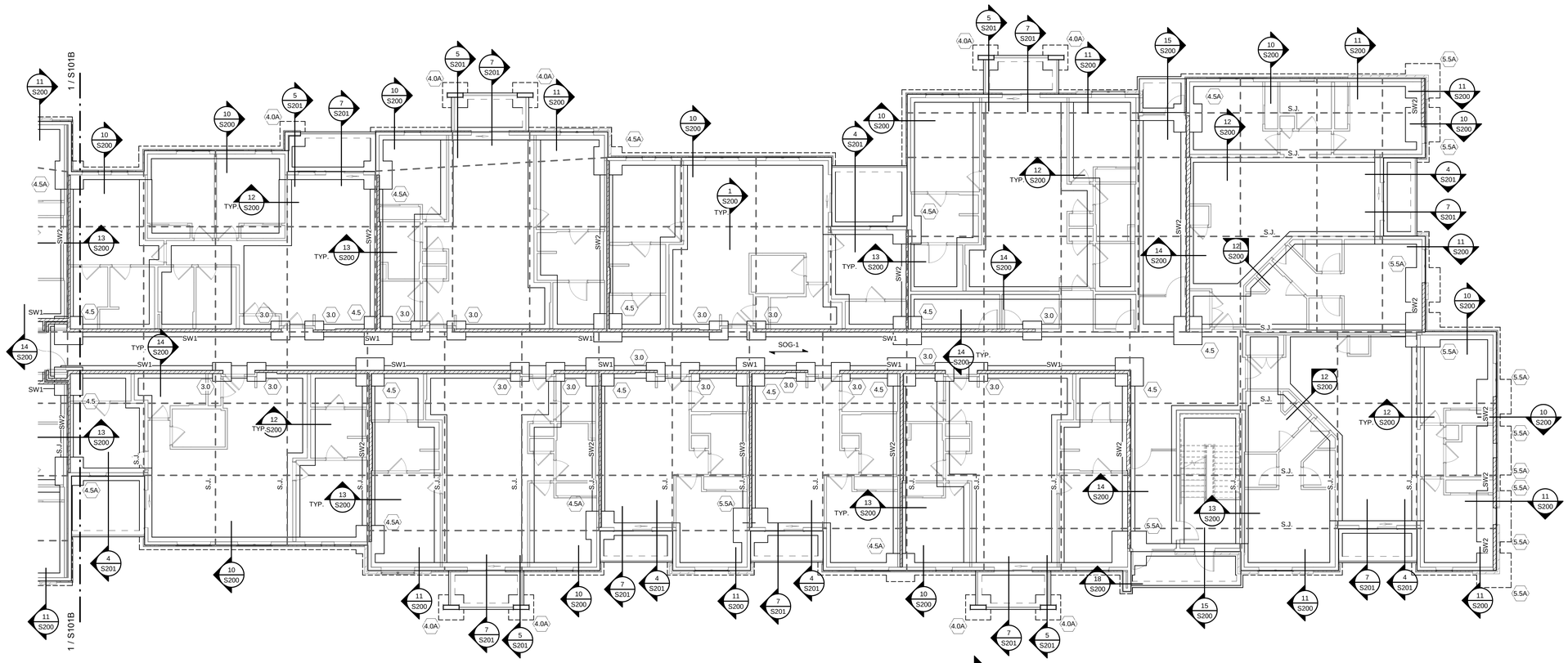
FOUNDATION PLAN - AREA B



1 1-FIRST FLOOR FRAMING PLAN - Area C
1/8" = 1'-0"

N

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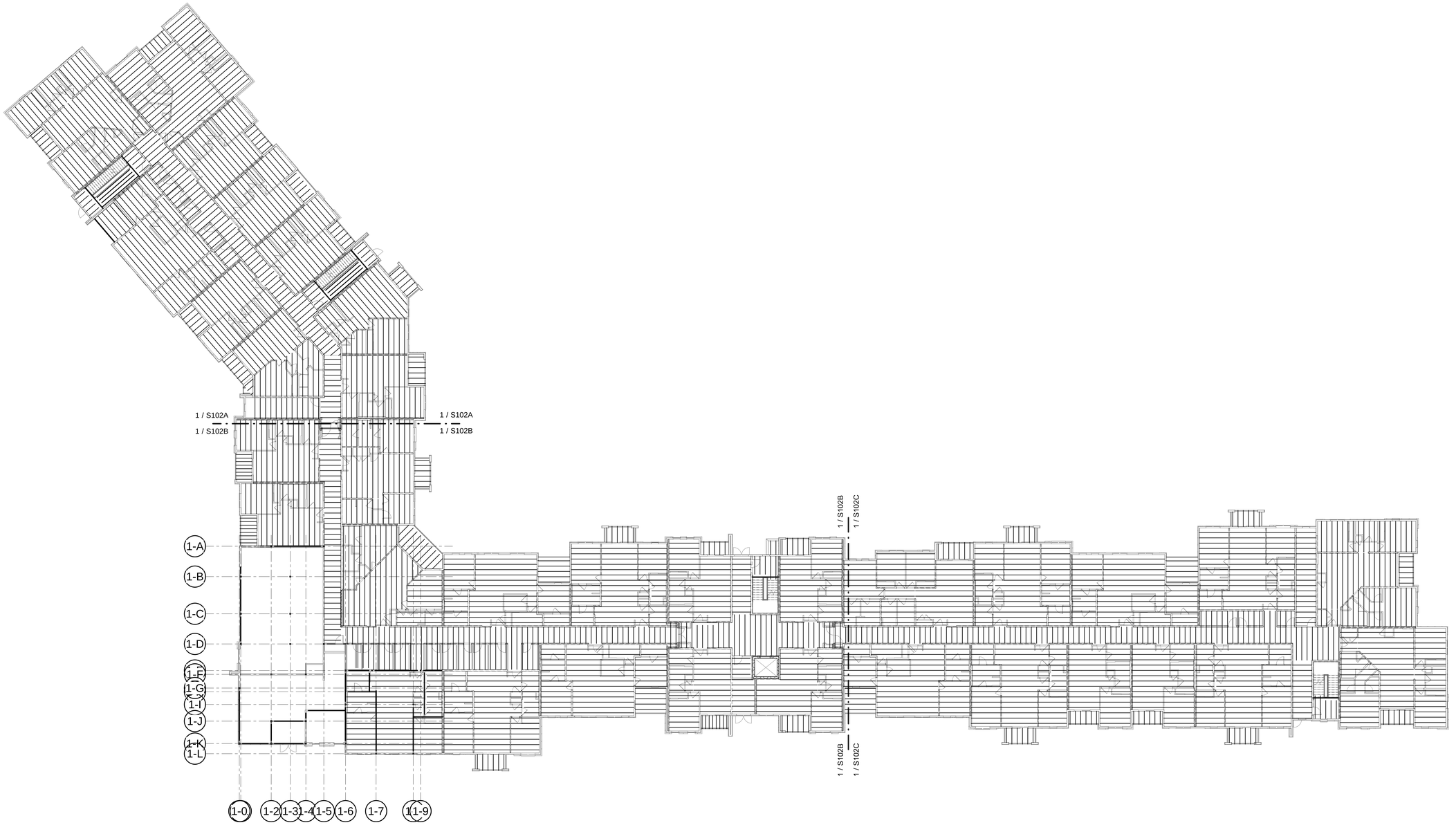
S101C

FOUNDATION PLAN - AREA C



1 SECOND FLOOR FRAMING PLAN

1/16" = 1'-0"

N

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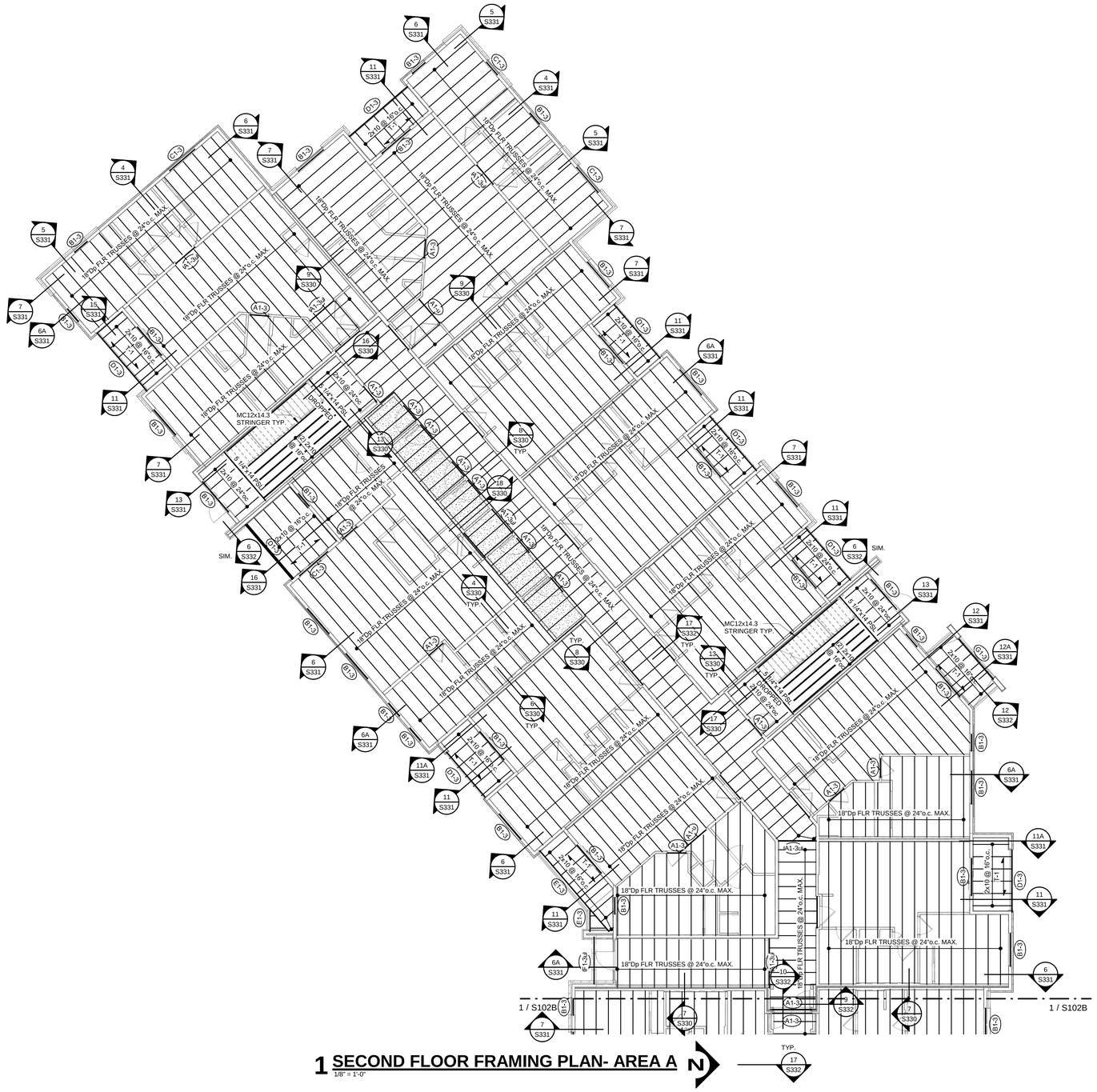
S102

SECOND FLOOR FRAMING PLAN



1 SECOND FLOOR FRAMING PLAN- AREA A

1/8" = 1'-0"

WOOD FLOOR FRAMING

NOTES:

1. REFER TO GENERAL NOTES ON SHEET S001.
2. REFER TO STUD BEARING WALL SCHEDULE ON SHEET S003.
3. REFER TO HEADER/BEAM/JAMB SCHEDULE ON SHEET S003.
4. REFER TO SHEARWALL AND HOLDOWN SCHEDULE ON S004.
5. REFER TO S330 SERIES OF SHEETS FOR ADDITIONAL FLOOR FRAMING DETAILS NOT INDICATED HERE
6. PROVIDE TRUSS SPACE DIRECTLY ABOVE AND CENTERED OVER HVAC CLOSET; REFER TO ARCH AND MEP DRAWINGS FOR LOCATIONS.
7. REFER TO S001 FOR SLAB AND DECK SCHEDULE.
   A. WOOD FLOOR DECK TO BE DECK TYPE "FD-1" U.N.O.
   B. BALCONY FLOOR DECK TO BE DECK TYPE "T-1" U.N.O.
8. PUBLIC AREA: DESIGN FOR LIVE LOAD PER GENERAL NOTE 2.B ON S001
   STORAGE AREA: DESIGN FOR LIVE LOAD PER GENERAL NOTE 2.B ON S001

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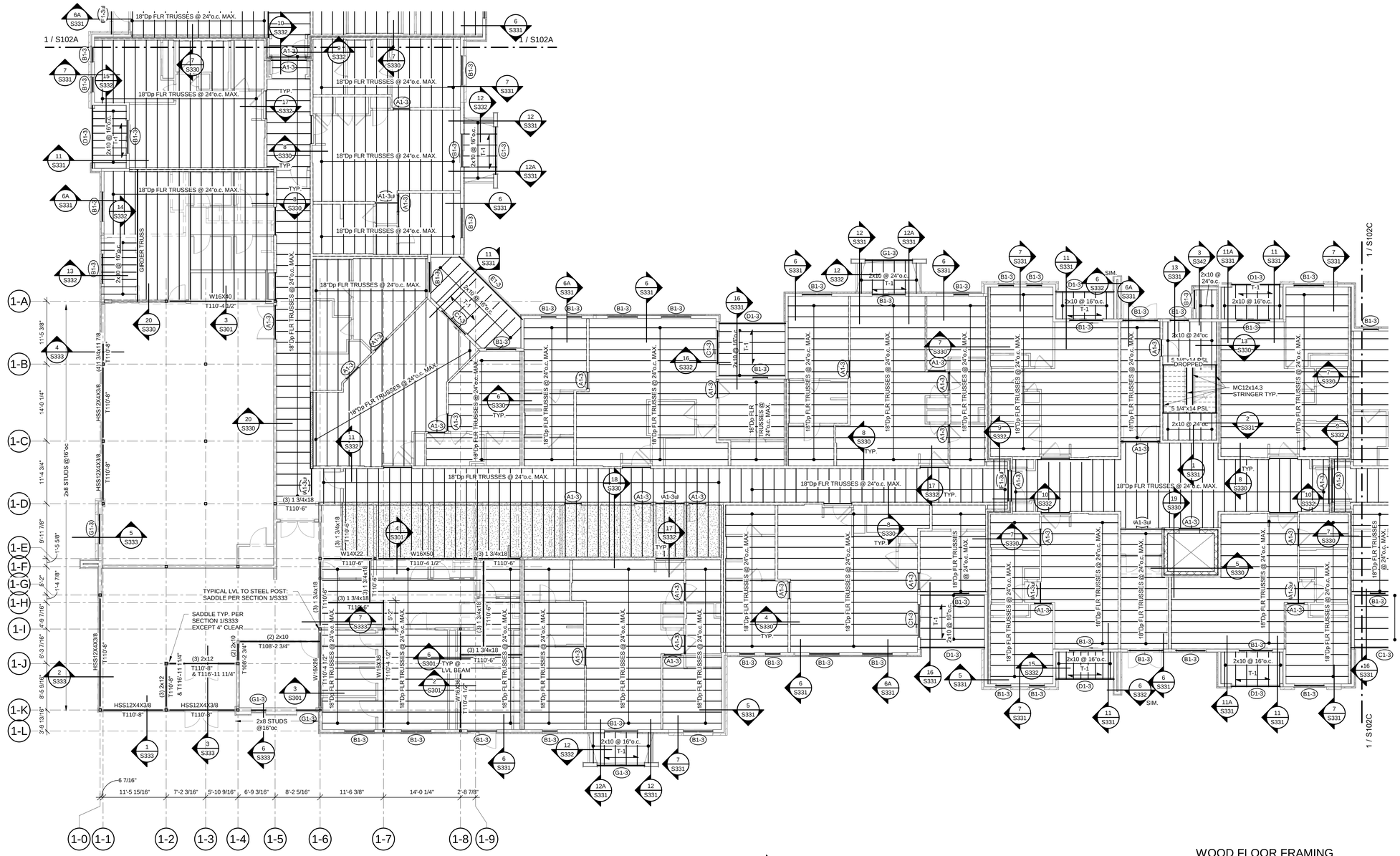
S102A

SECOND FLOOR FRAMING PLAN- AREA A



1 SECOND FLOOR FRAMING PLAN- AREA B

1/8" = 1'-0"

WOOD FLOOR FRAMING

NOTES:

1. REFER TO GENERAL NOTES ON SHEET S001.
2. REFER TO STUD BEARING WALL SCHEDULE ON SHEET S003.
3. REFER TO HEADER/BEAM/JAMB SCHEDULE ON SHEET S003.
4. REFER TO SHEARWALL AND HOLDOWN SCHEDULE ON S004.
5. REFER TO S330 SERIES OF SHEETS FOR ADDITIONAL FLOOR FRAMING DETAILS NOT INDICATED HERE.
6. PROVIDE TRUSS SPACE DIRECTLY ABOVE AND CENTERED OVER HVAC CLOSET; REFER TO ARCH AND MEP DRAWINGS FOR LOCATIONS.
7. REFER TO S001 FOR SLAB AND DECK SCHEDULE.
   A. WOOD FLOOR DECK TO BE DECK TYPE "FD-1" U.N.O.
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   STORAGE AREA: DESIGN FOR LIVE LOAD PER GENERAL NOTE 2.B ON S001

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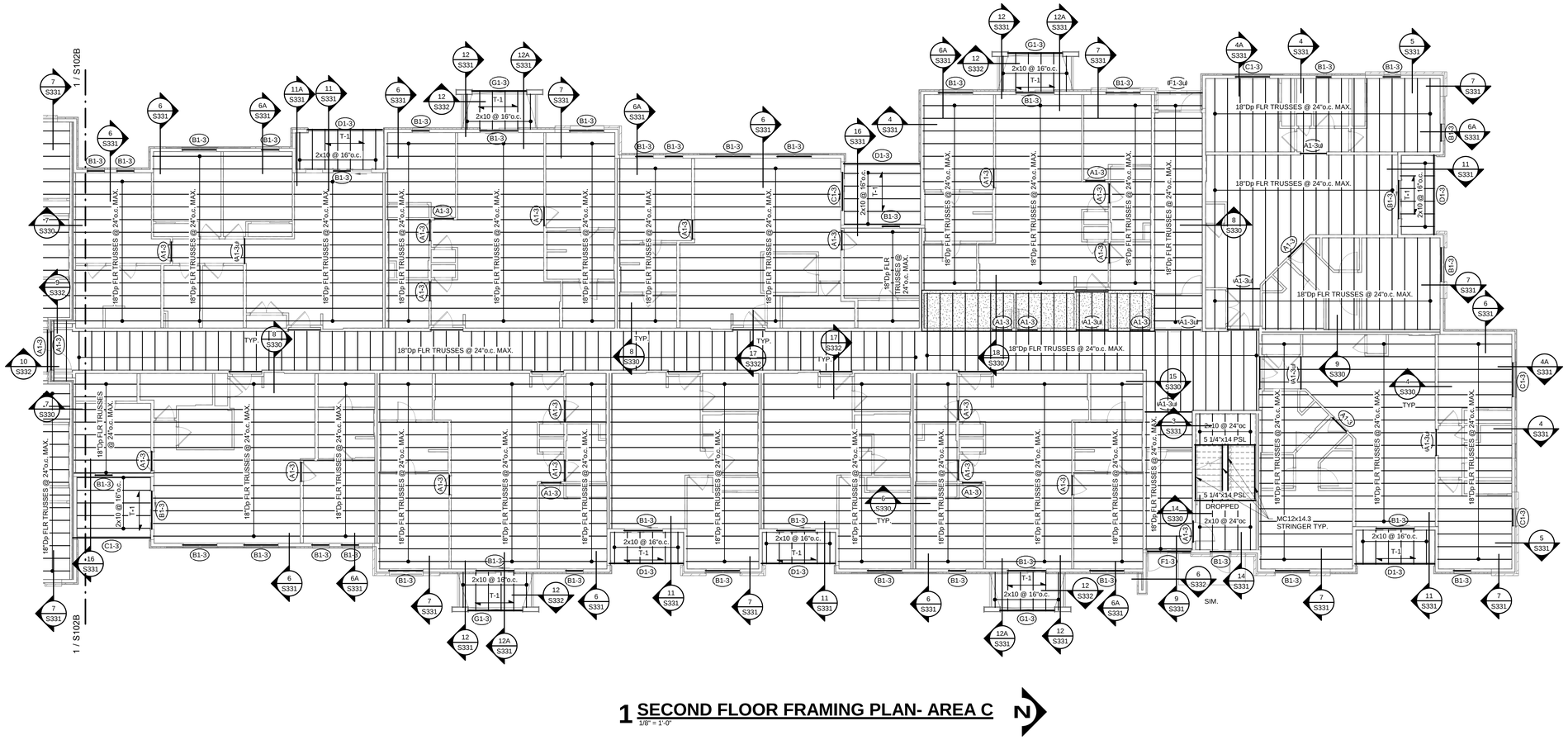
S102B

SECOND FLOOR FRAMING PLAN- AREA B



1 SECOND FLOOR FRAMING PLAN- AREA C

1/8" = 1'-0"

N

WOOD FLOOR FRAMING

NOTES:

1. REFER TO GENERAL NOTES ON SHEET S001.
2. REFER TO STUD BEARING WALL SCHEDULE ON SHEET S003.
3. REFER TO HEADER/BEAM/JAMB SCHEDULE ON SHEET S003.
4. REFER TO SHEARWALL AND HOLDOWN SCHEDULE ON S004.
5. REFER TO S330 SERIES OF SHEETS FOR ADDITIONAL FLOOR FRAMING DETAILS NOT INDICATED HERE
6. PROVIDE TRUSS SPACE DIRECTLY ABOVE AND CENTERED OVER HVAC CLOSET; REFER TO ARCH AND MEP DRAWINGS FOR LOCATIONS.
7. REFER TO S001 FOR SLAB AND DECK SCHEDULE.
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   STORAGE AREA: DESIGN FOR LIVE LOAD PER GENERAL NOTE 2.B ON S001

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SHEET NO.

S102C

SECOND FLOOR FRAMING PLAN- AREA C

1 THIRD FLOOR FRAMING PLAN

1/16" = 1'-0"

N

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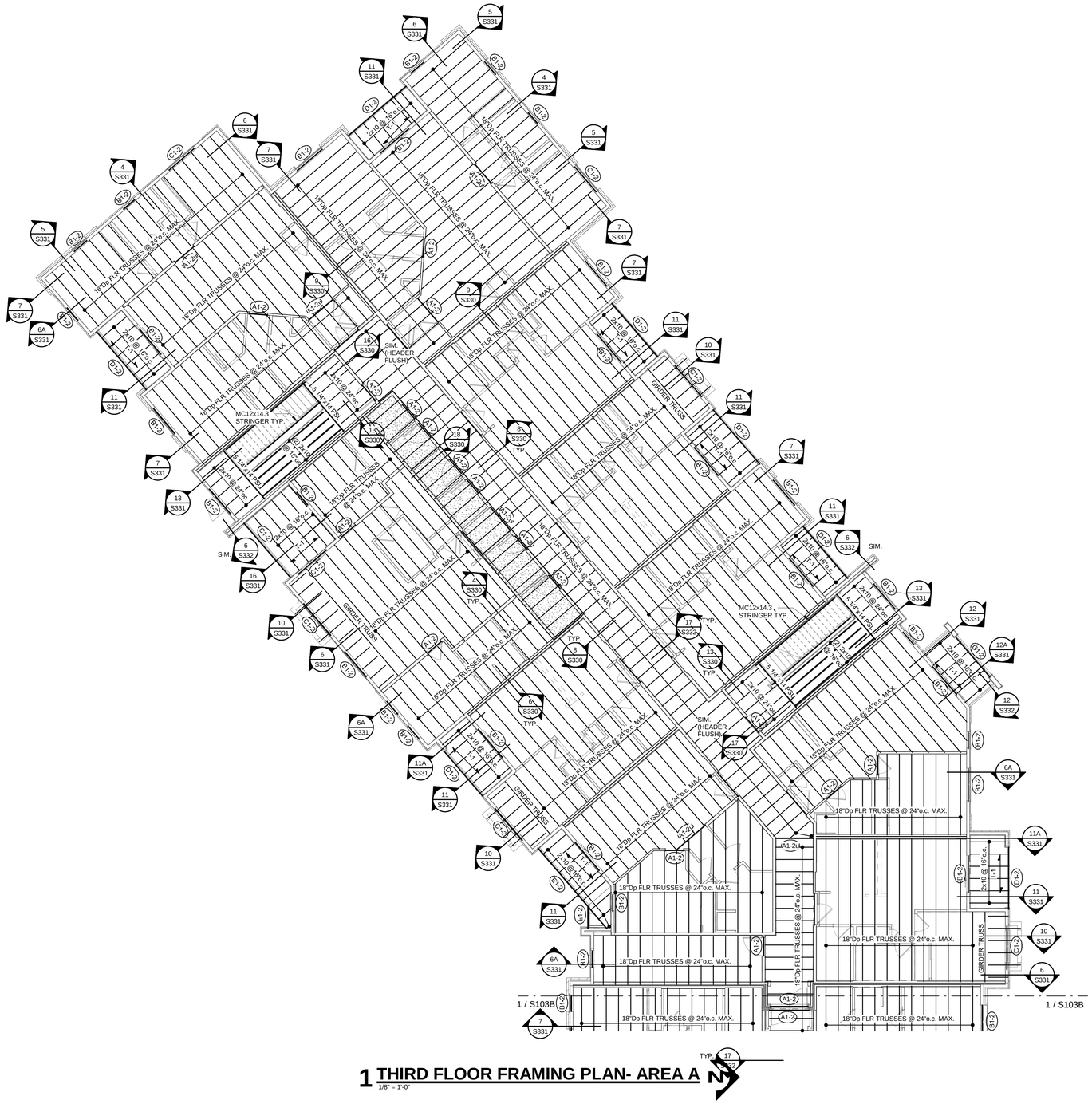
SHEET NO.

S103

THIRD FLOOR FRAMING PLAN



1 THIRD FLOOR FRAMING PLAN- AREA A

1/8" = 1'-0"

WOOD FLOOR FRAMING

NOTES:

1. REFER TO GENERAL NOTES ON SHEET S001.
2. REFER TO STUD BEARING WALL SCHEDULE ON SHEET S003.
3. REFER TO HEADER/BEAM/JAMB SCHEDULE ON SHEET S003.
4. REFER TO SHEARWALL AND HOLDOWN SCHEDULE ON S004.
5. REFER TO S330 SERIES OF SHEETS FOR ADDITIONAL FLOOR FRAMING DETAILS NOT INDICATED HERE
6. PROVIDE TRUSS SPACE DIRECTLY ABOVE AND CENTERED OVER HVAC CLOSET; REFER TO ARCH AND MEP DRAWINGS FOR LOCATIONS.
7. REFER TO S001 FOR SLAB AND DECK SCHEDULE.
   A. WOOD FLOOR DECK TO BE DECK TYPE "FD-1" U.N.O.
   B. BALCONY FLOOR DECK TO BE DECK TYPE "T-1" U.N.O.
8. 40 PSF PUBLIC AREA: DESIGN FOR LIVE LOAD PER GENERAL NOTE 2.B ON S001
   100 PSF STORAGE AREA: DESIGN FOR LIVE LOAD PER GENERAL NOTE 2.B ON S001

STATE OF MISSOURI
CHAD A. BUSH
BUSH
NUMBER
PE-2010026837
PROFESSIONAL ENGINEER
12/14/21

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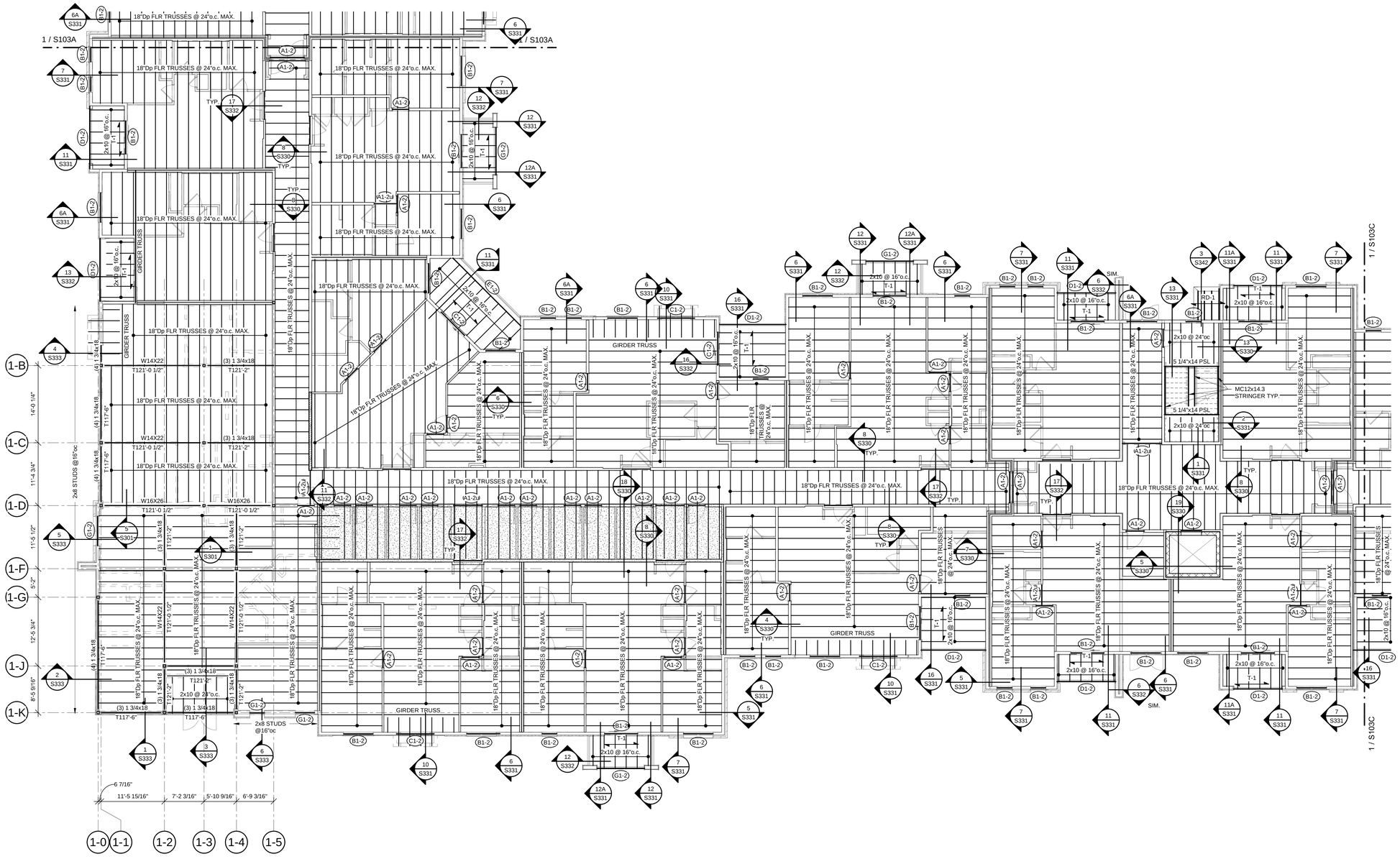
THIRD FLOOR FRAMING PLAN- AREA A



1 THIRD FLOOR FRAMING PLAN- AREA B

1/8" = 1'-0"

N

WOOD FLOOR FRAMING

NOTES:

1. REFER TO GENERAL NOTES ON SHEET S001.
2. REFER TO STUD BEARING WALL SCHEDULE ON SHEET S003.
3. REFER TO HEADER/BEAM/JAMB SCHEDULE ON SHEET S003.
4. REFER TO SHEARWALL AND HOLDOWN SCHEDULE ON S004.
5. REFER TO S330 SERIES OF SHEETS FOR ADDITIONAL FLOOR FRAMING DETAILS NOT INDICATED HERE
6. PROVIDE TRUSS SPACE DIRECTLY ABOVE AND CENTERED OVER HVAC CLOSET; REFER TO ARCH AND MEP DRAWINGS FOR LOCATIONS.
7. REFER TO S001 FOR SLAB AND DECK SCHEDULE.
   A. WOOD FLOOR DECK TO BE DECK TYPE "FD-1" U.N.O.
   B. BALCONY FLOOR DECK TO BE DECK TYPE "T-1" U.N.O.
8. PUBLIC AREA: DESIGN FOR LIVE LOAD PER GENERAL NOTE 2.B ON S001
   STORAGE AREA: DESIGN FOR LIVE LOAD PER GENERAL NOTE 2.B ON S001

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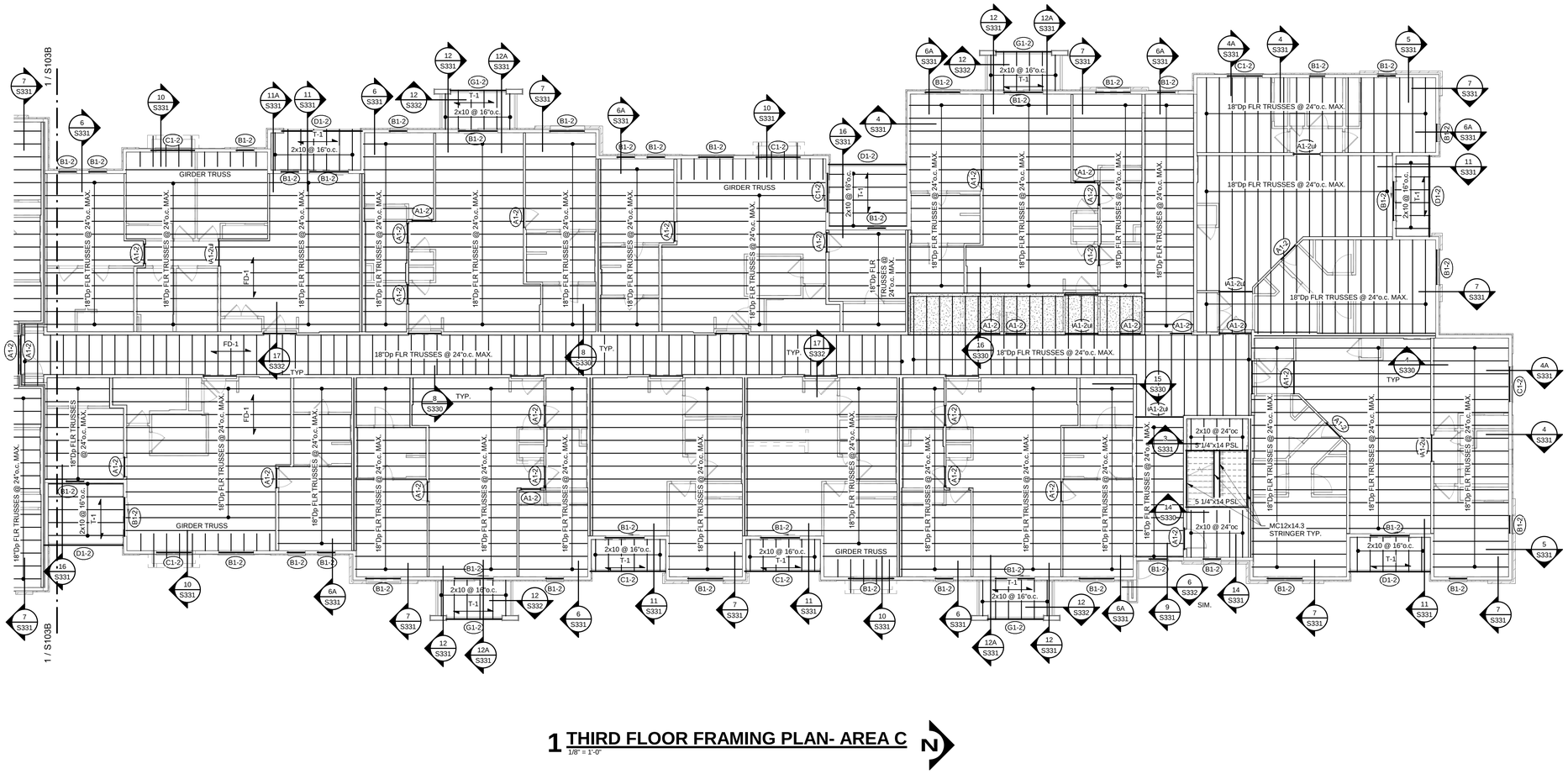
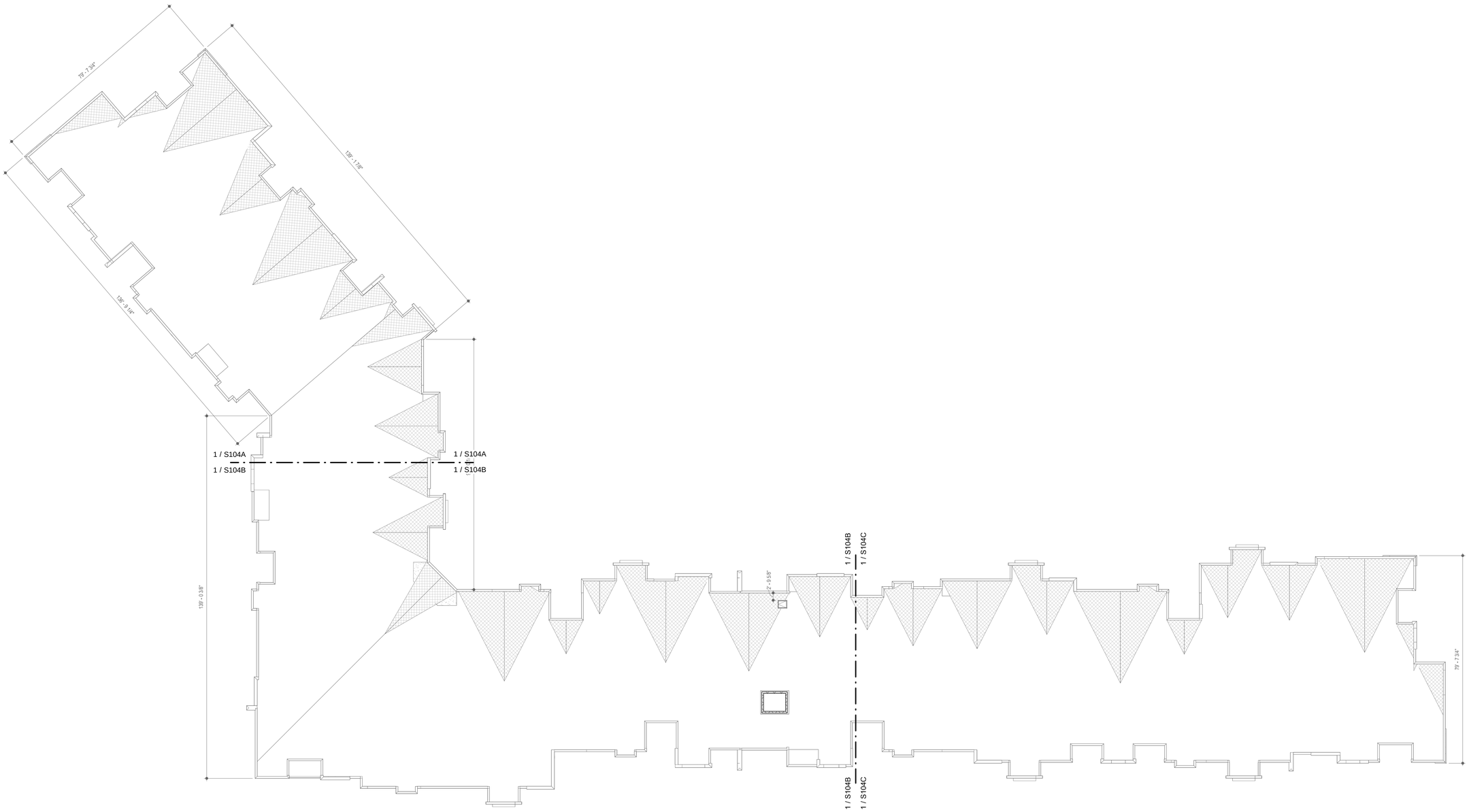
S103B

THIRD FLOOR FRAMING PLAN- AREA B


1 / S103B
1 / S103B
GIRDER TRUSS
GIRDER TRUSS
GIRDER TRUSS
GIRDER TRUSS
18"Dp FLR TRUSSES @ 24"o.c. MAX.
18"Dp FLR TRUSSES @ 24"o.c. MAX.
18"Dp FLR TRUSSES @ 24"o.c. MAX.



1 ROOF FRAMING PLAN
1/16" = 1'-0"

N

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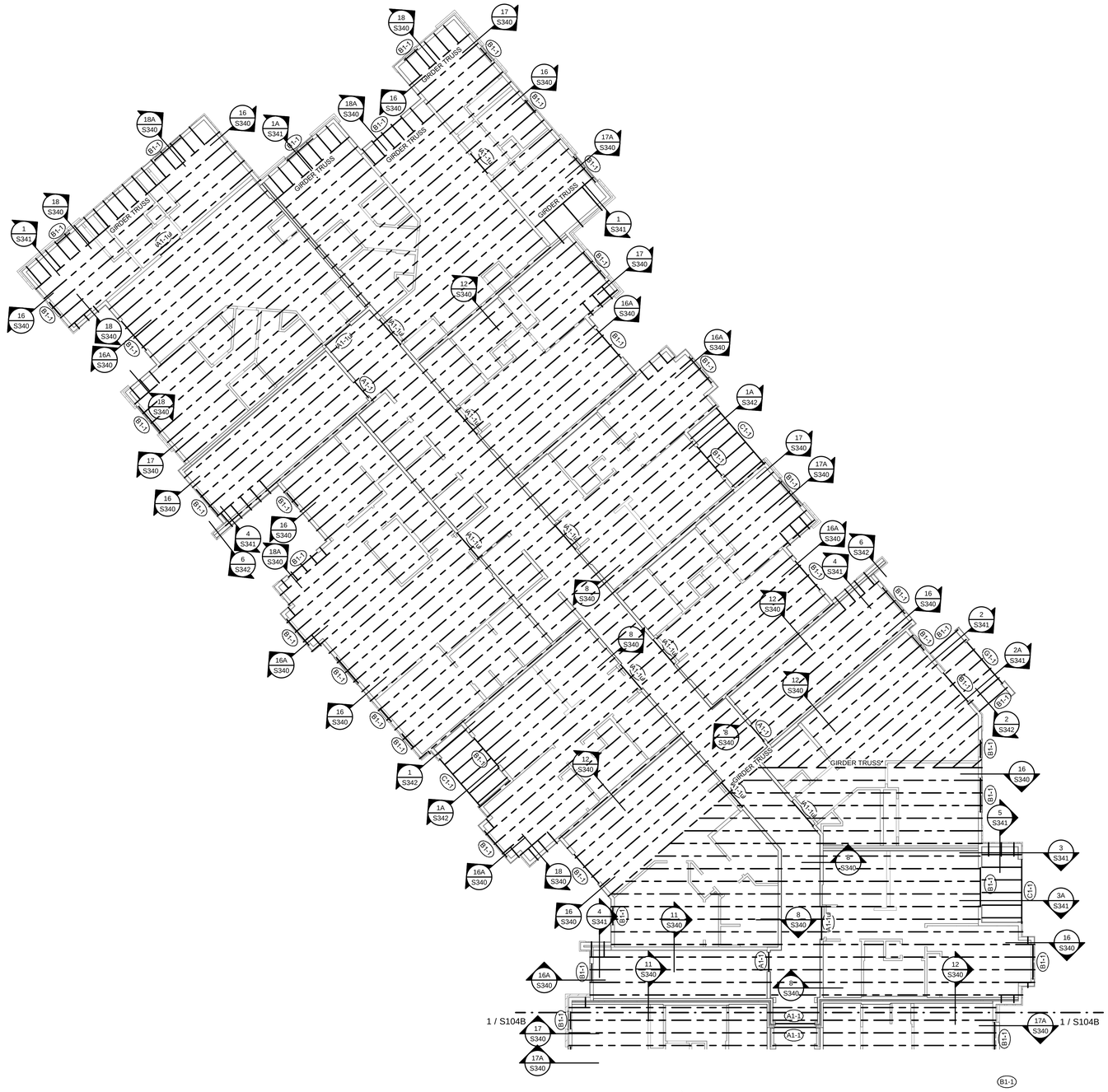
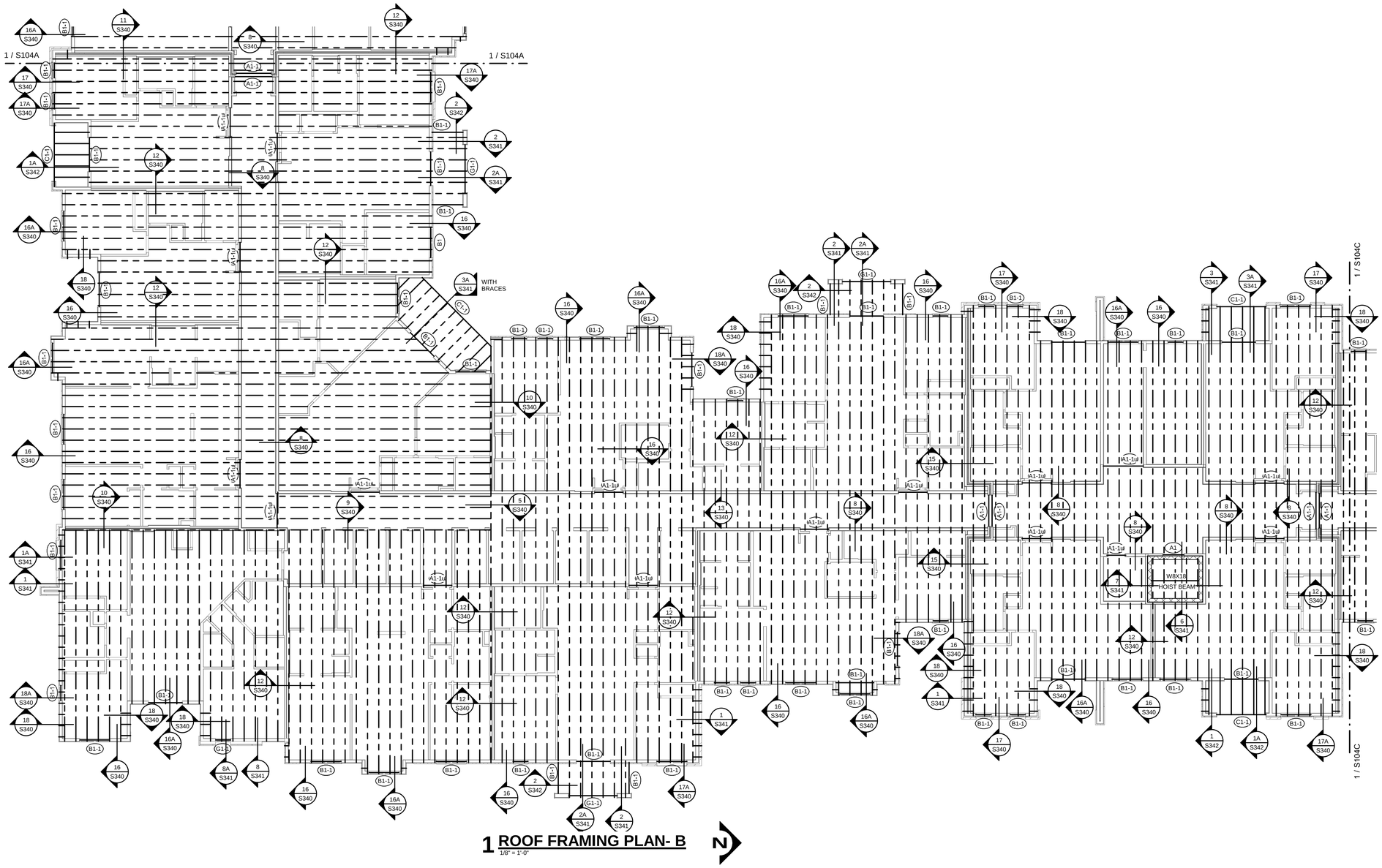
SHEET NO.

S104

ROOF FRAMING PLAN


GIRDER TRUSS
GIRDER TRUSS
GIRDER TRUSS



1 ROOF FRAMING PLAN- B

1/8" = 1'-0"

WOOD ROOF FRAMING

NOTES:

1. REFER TO GENERAL NOTES ON SHEET S001.
2. REFER TO STUD BEARING WALL SCHEDULE ON SHEET S003.
3. REFER TO HEADER/BEAM/JAMB SCHEDULE ON SHEET S003.
4. REFER TO SHEARWALL AND HOLDOWN SCHEDULE ON S004.
5. REFER TO S340 SERIES OF SHEETS FOR ADDITIONAL FLOOR FRAMING DETAILS NOT INDICATED HERE
6. PROVIDE (3) STUDS MINIMUM ALIGNED UNDER EACH END OF EACH GIRDER TRUSS. PROVIDE SQUASH BLOCKS AND JAMBS AT EA. FLOOR BELOW DOWN TO FOUNDATION. (TOTAL STUDS TO MATCH NUMBER OF GIRDER TRUSS PLIES (3 MIN.) REFER TO DETAIL 3/S340.
7. PROVIDE UNIFORM UPLIFT SCREWS @ UPPER FLOOR PER DETAILS 3, 3A, 3B, 4 AND 5 ON S005.
8. PRE-ENG TRUSSES TO HAVE A MINIMUM DEPTH OF 24". SLOPE TOP CHORD PER ARCHITECTUAL DRAWINGS.
9. REFER TO S001 FOR SLAB AND DECK SCHEDULE.
   A. WOOD ROOF DECK TO BE DECK TYPE "RD-1" U.N.O.

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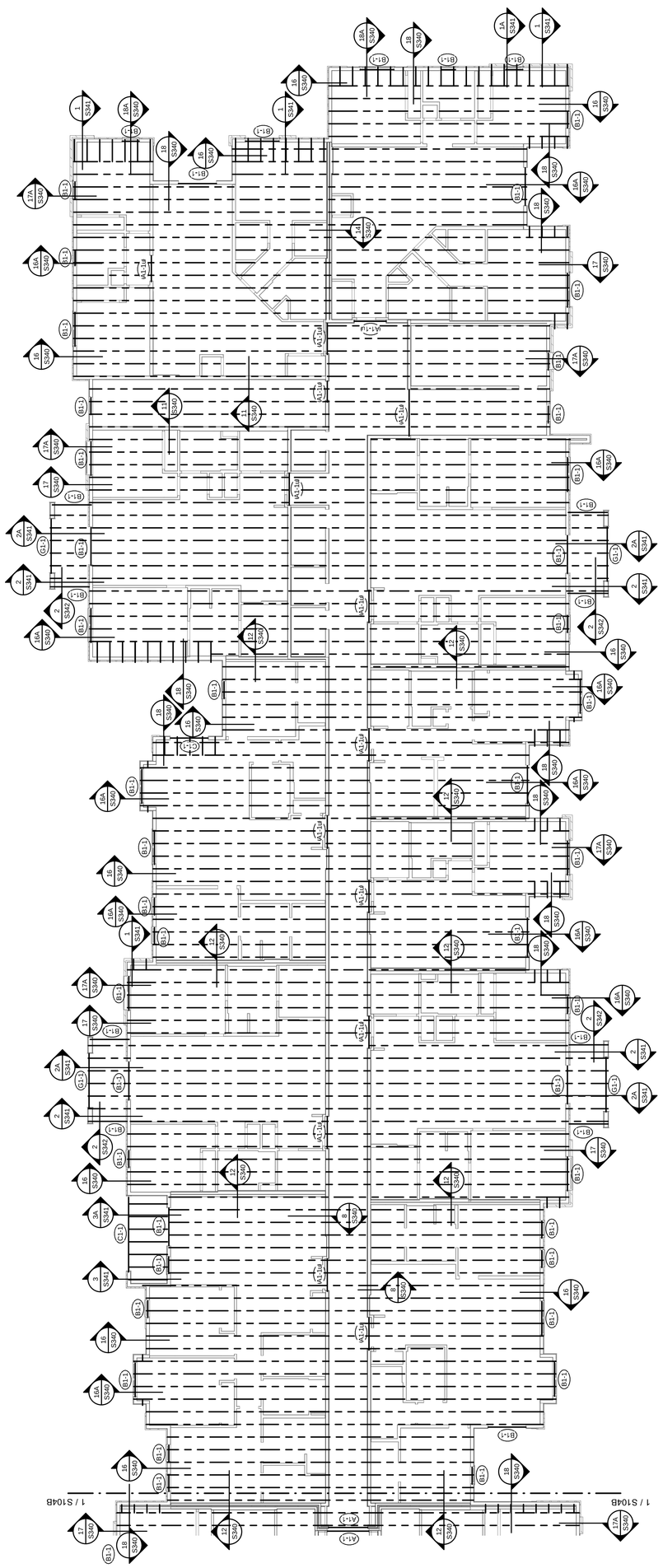
SHEET NO.

S104B

ROOF FRAMING PLAN- AREA B



1 ROOF FRAMING PLAN- AREA C
1/8" = 1'-0"

WOOD ROOF FRAMING

NOTES:

1. REFER TO GENERAL NOTES ON SHEET S001
2. REFER TO STEEL FRAMING SCHEDULE ON SHEET S003.
3. REFER TO HEADER/BEAM/JAMB SCHEDULE ON SHEET S003.
4. REFER TO SHEARWALL AND HOLDOWN SCHEDULE ON S004.
5. REFER TO SSERIES OF SHEETS FOR ADDITIONAL FLOOR FRAMING DETAILS NOT INDICATED HERE
6. PROVIDE (2) STUDS MINIMUM ALIGNED UNDER EACH END OF EACH GIRDER TRUSS FOR BEARING. SEE TYPICAL FRAMING DETAILS 4 & 5 FOR ADDITIONAL INFORMATION TO FOUNDATION. (TOTAL STUDS TO MATCH NUMBER OF GIRDER TRUSS PLIES (3 MIN.) REFER TO DETAIL 3/S340.
7. PROVIDE HURRICANE UPLIFT SCREWS @ UPPER FLOOR PER DETAILS 3, 3A, 3B, 4 AND 5 ON S005.
8. PRE-ENG. TRUSSES TO HAVE A MINIMUM DEPTH OF 24" SLOPE TOP CHORD PER ARCHITECTURAL DRAWINGS.
9. REFER TO S001 FOR SLAB AND DECK SCHEDULE.
   A. WOOD ROOF DECK TO BE DECK TYPE 'RD-1' U.N.O.

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SHEET NO. S104C

ROOF FRAMING PLAN- AREA C

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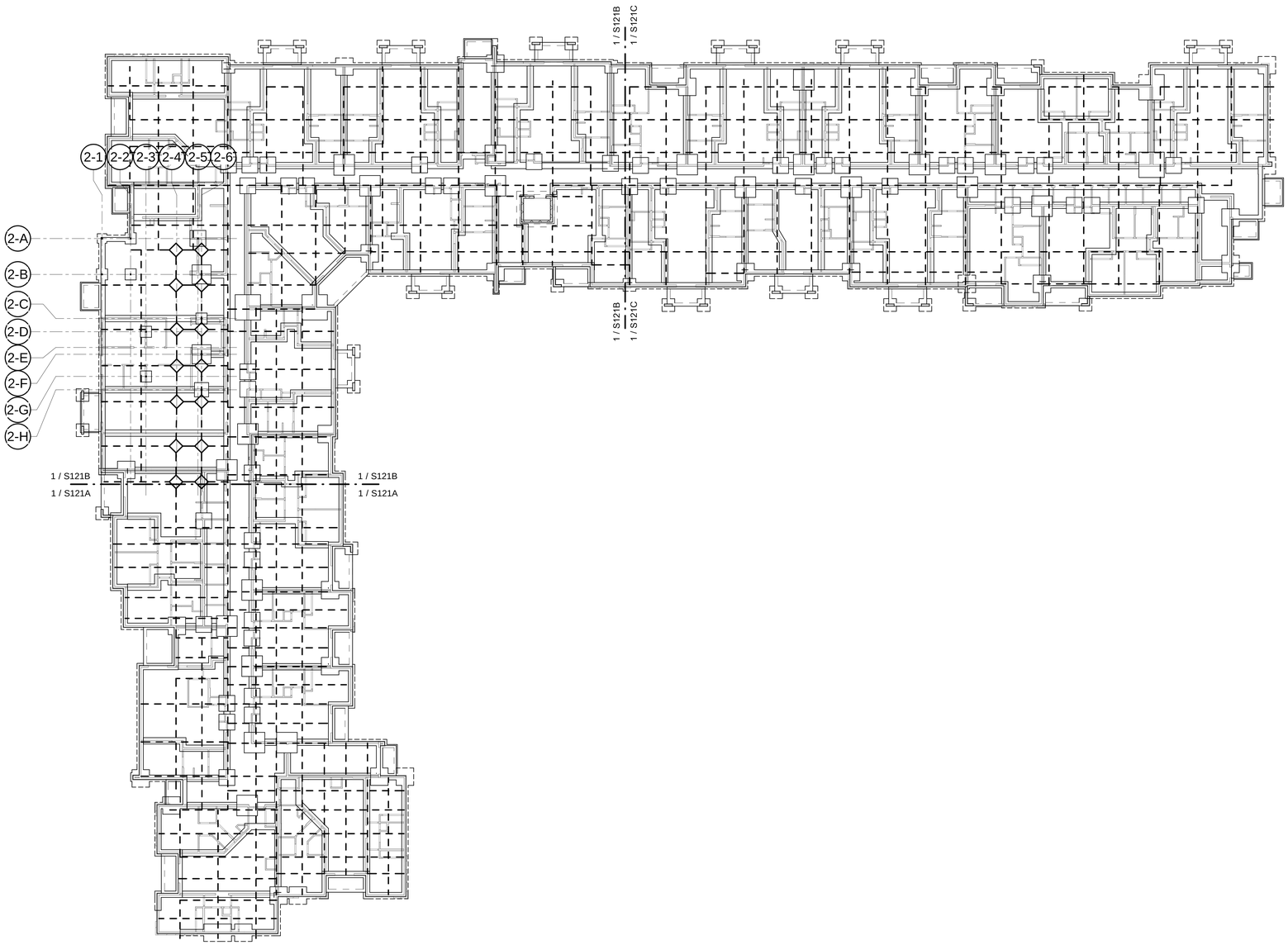
LATTIMER SOMMERS & ASSOCIATES	PLUMBING ENGINEER
LATTIMER SOMMERS & ASSOCIATES	ELECTRICAL ENGINEER



1 Bldg 2- FOUNDATION PLAN

1/16" = 1'-0"

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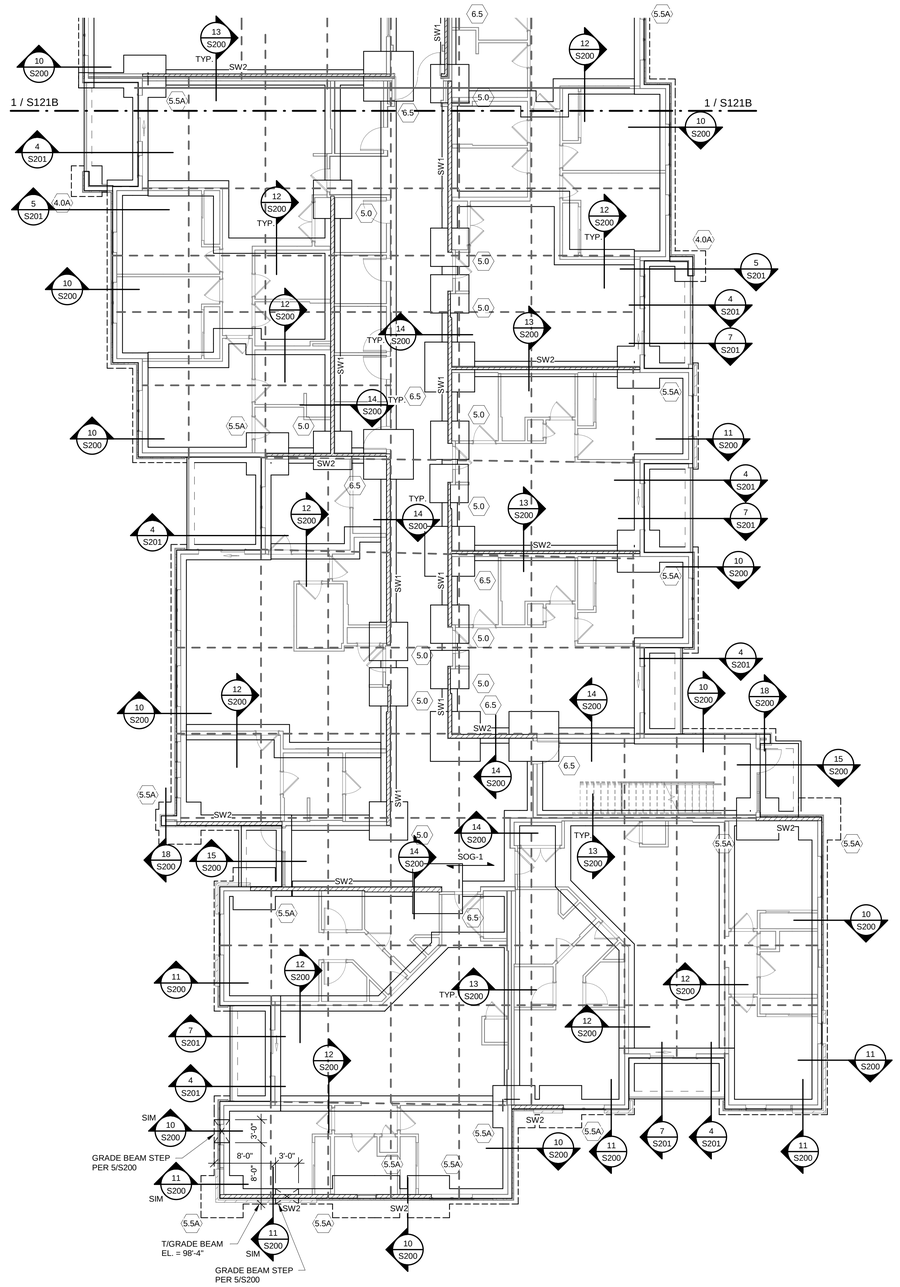
SHEET NO.

S121

BLDG 2 - FOUNDATION PLAN



1 BLDG 2- FOUNDATION PLAN - AREA A

1/8" = 1'-0"

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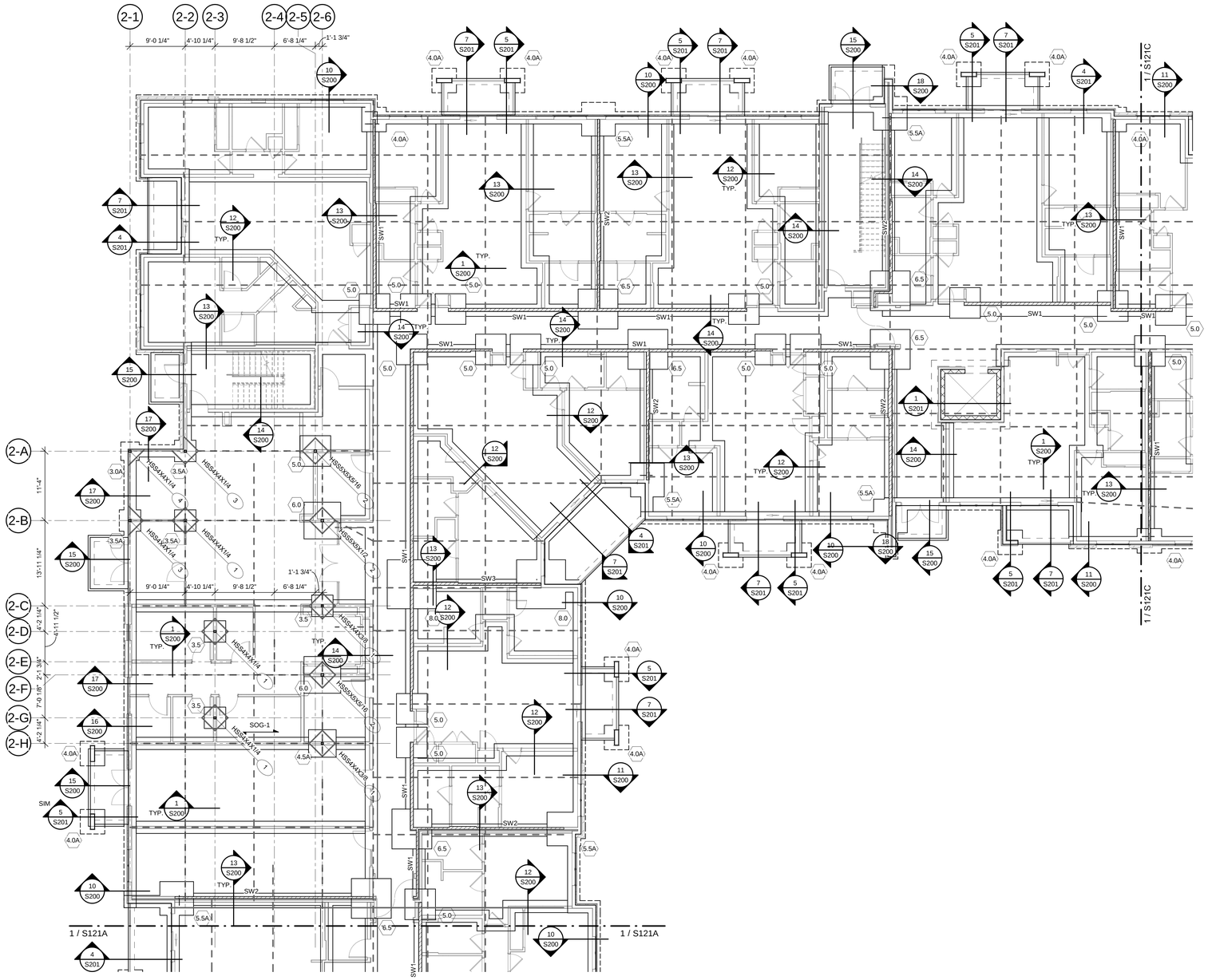
SHEET NO.

S121A

BLDG 2- FOUNDATION PLAN - AREA A



1 BLDG 2- FOUNDATION PLAN - AREA B

1/8" = 1'-0"

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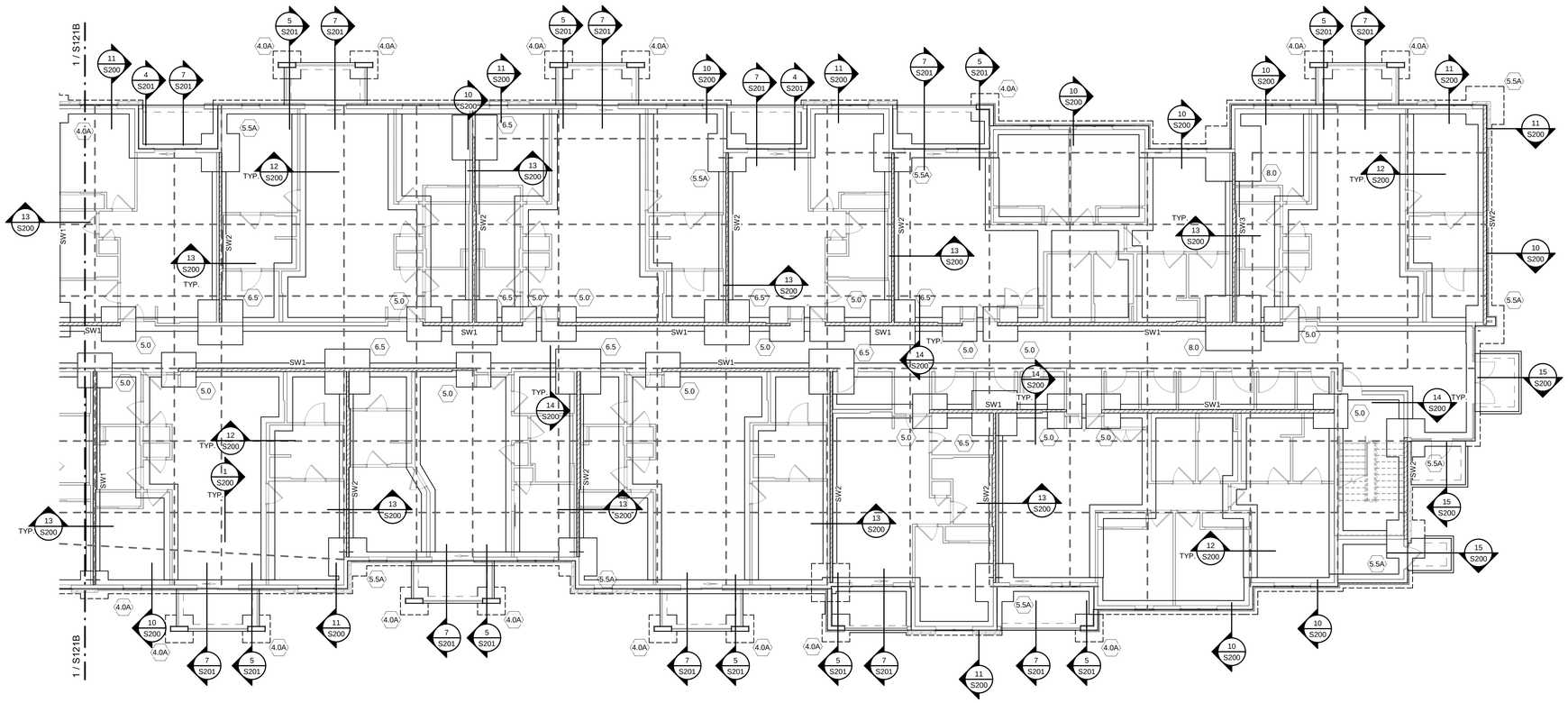
BLDG 2- FOUNDATION PLAN - AREA B



1 **BLDG 2- FOUNDATION PLAN - AREA C**
1/8" = 1'-0"

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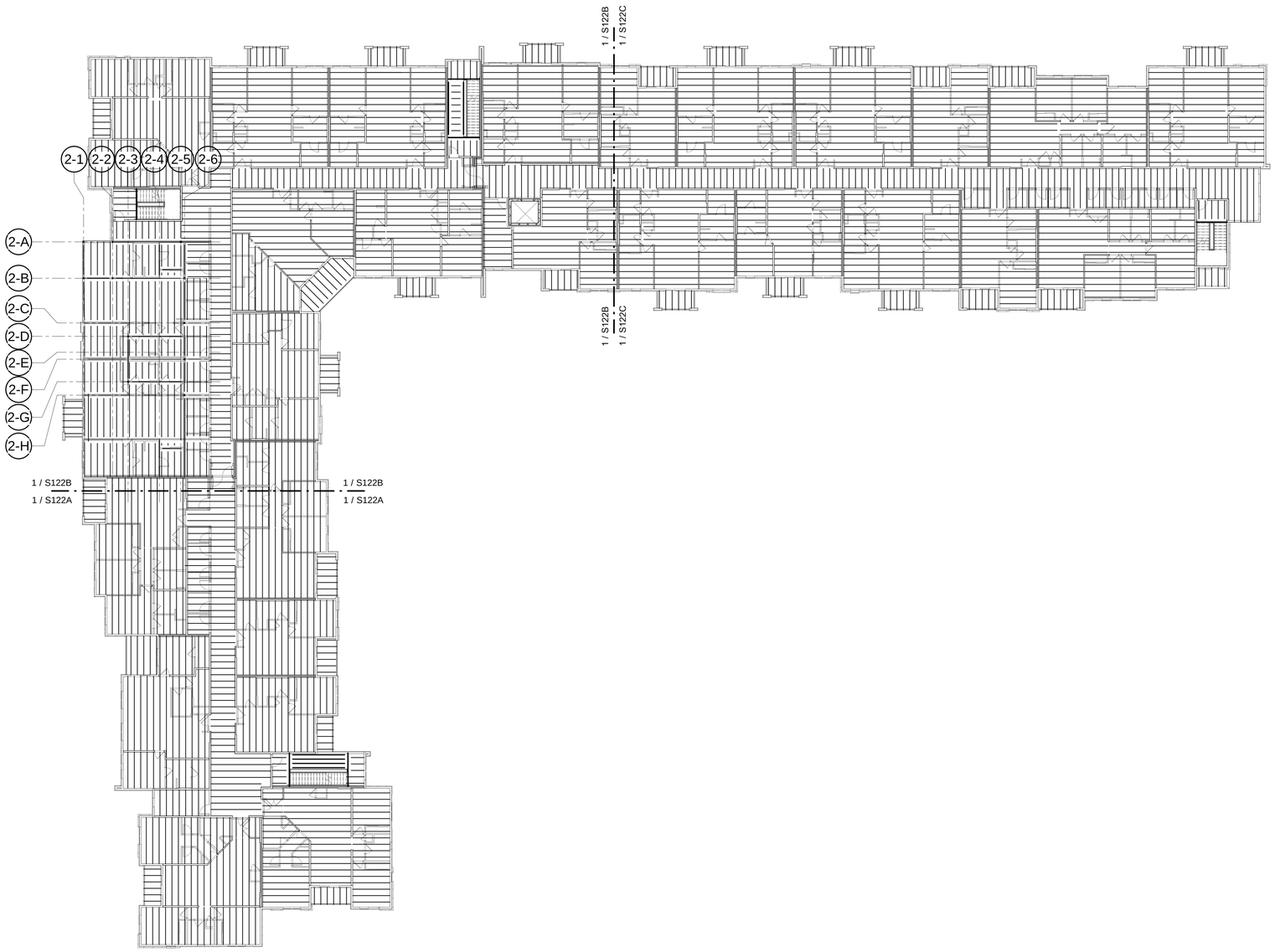
BLDG 2- FOUNDATION PLAN - AREA C



1 BLDG 2- SECOND FLOOR FRAMING PLAN

1/16" = 1'-0"

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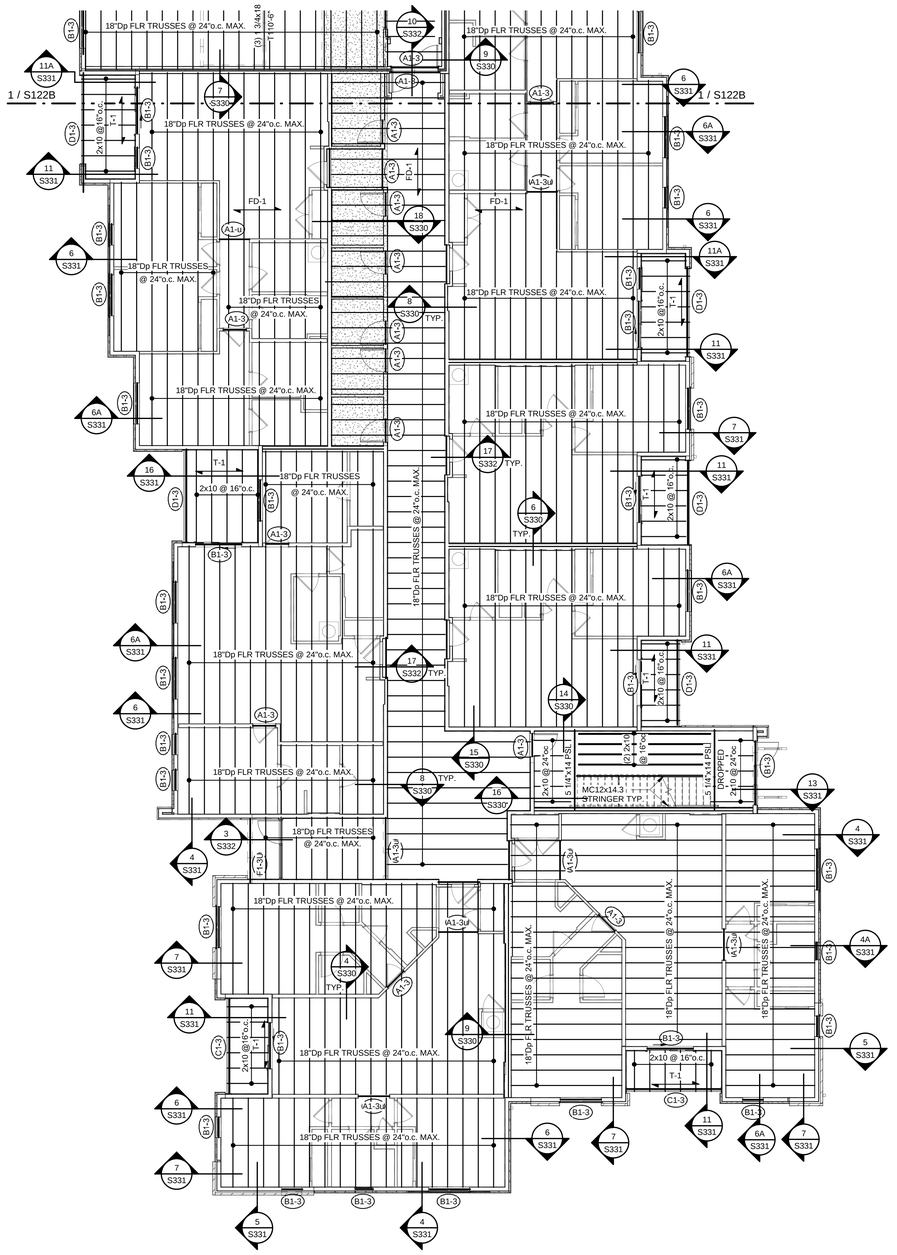
BLDG 2 - SECOND FLOOR FRAMING PLAN



1 BLDG 2- SECOND FLOOR FRAMING - AREA A

1/8" = 1'-0"

WOOD FLOOR FRAMING

NOTES:

1. REFER TO GENERAL NOTES ON SHEET S001.
2. REFER TO STUD BEARING WALL SCHEDULE ON SHEET S003.
3. REFER TO HEADER/BEAM/JAMB SCHEDULE ON SHEET S003.
4. REFER TO SHEARWALL AND HOLDOWN SCHEDULE ON S004.
5. REFER TO S330 SERIES OF SHEETS FOR ADDITIONAL FLOOR FRAMING DETAILS NOT INDICATED HERE.
6. PROVIDE TRUSS SPACE DIRECTLY ABOVE AND CENTERED OVER HVAC CLOSET; REFER TO ARCH AND MEP DRAWINGS FOR LOCATIONS.
7. REFER TO S001 FOR SLAB AND DECK SCHEDULE.
   A. WOOD FLOOR DECK TO BE DECK TYPE "FD-1" U.N.O.
   B. BALCONY FLOOR DECK TO BE DECK TYPE "T-1" U.N.O.
8. PUBLIC AREA: DESIGN FOR LIVE LOAD PER GENERAL NOTE 2.B ON S001
   STORAGE AREA: DESIGN FOR LIVE LOAD PER GENERAL NOTE 2.B ON S001

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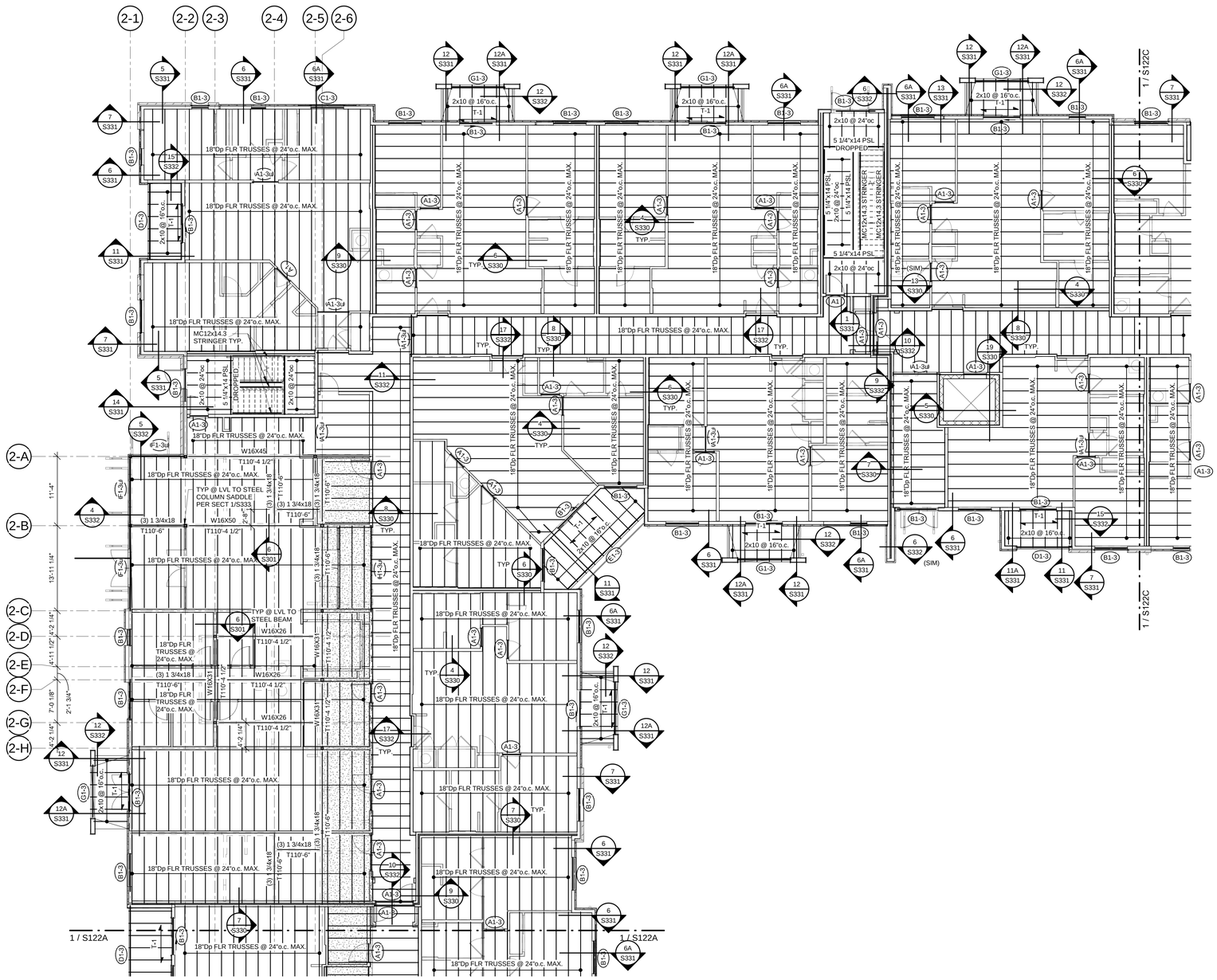
BLDG 2 - SECOND FLOOR FRAMING PLAN - AREA A



1 BLDG 2- SECOND FLOOR FRAMING PLAN - AREA B

1/8" = 1'-0"

WOOD FLOOR FRAMING

NOTES:

1. REFER TO GENERAL NOTES ON SHEET S001.
2. REFER TO STUD BEARING WALL SCHEDULE ON SHEET S003.
3. REFER TO HEADER/BEAM/JAMB SCHEDULE ON SHEET S003.
4. REFER TO SHEARWALL AND HOLDOWN SCHEDULE ON S004.
5. REFER TO S330 SERIES OF SHEETS FOR ADDITIONAL FLOOR FRAMING DETAILS NOT INDICATED HERE.
6. PROVIDE TRUSS SPACE DIRECTLY ABOVE AND CENTERED OVER HVAC CLOSET; REFER TO ARCH AND MEP DRAWINGS FOR LOCATIONS.
7. REFER TO S001 FOR SLAB AND DECK SCHEDULE.
   A. WOOD FLOOR DECK TO BE DECK TYPE "FD-1" U.N.O.
   B. BALCONY FLOOR DECK TO BE DECK TYPE "T-1" U.N.O.
8. PUBLIC AREA: DESIGN FOR LIVE LOAD PER GENERAL NOTE 2.B ON S001.
   STORAGE AREA: DESIGN FOR LIVE LOAD PER GENERAL NOTE 2.B ON S001

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LATIMER SOMMERS & ASSOCIATES — MECHANICAL ENGINEER
LATIMER SOMMERS & ASSOCIATES — PLUMBING ENGINEER
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STATE OF MISSOURI — CHAD A. BUSH — NUMBER PE-2010026837 — PROFESSIONAL ENGINEER — 12/14/21

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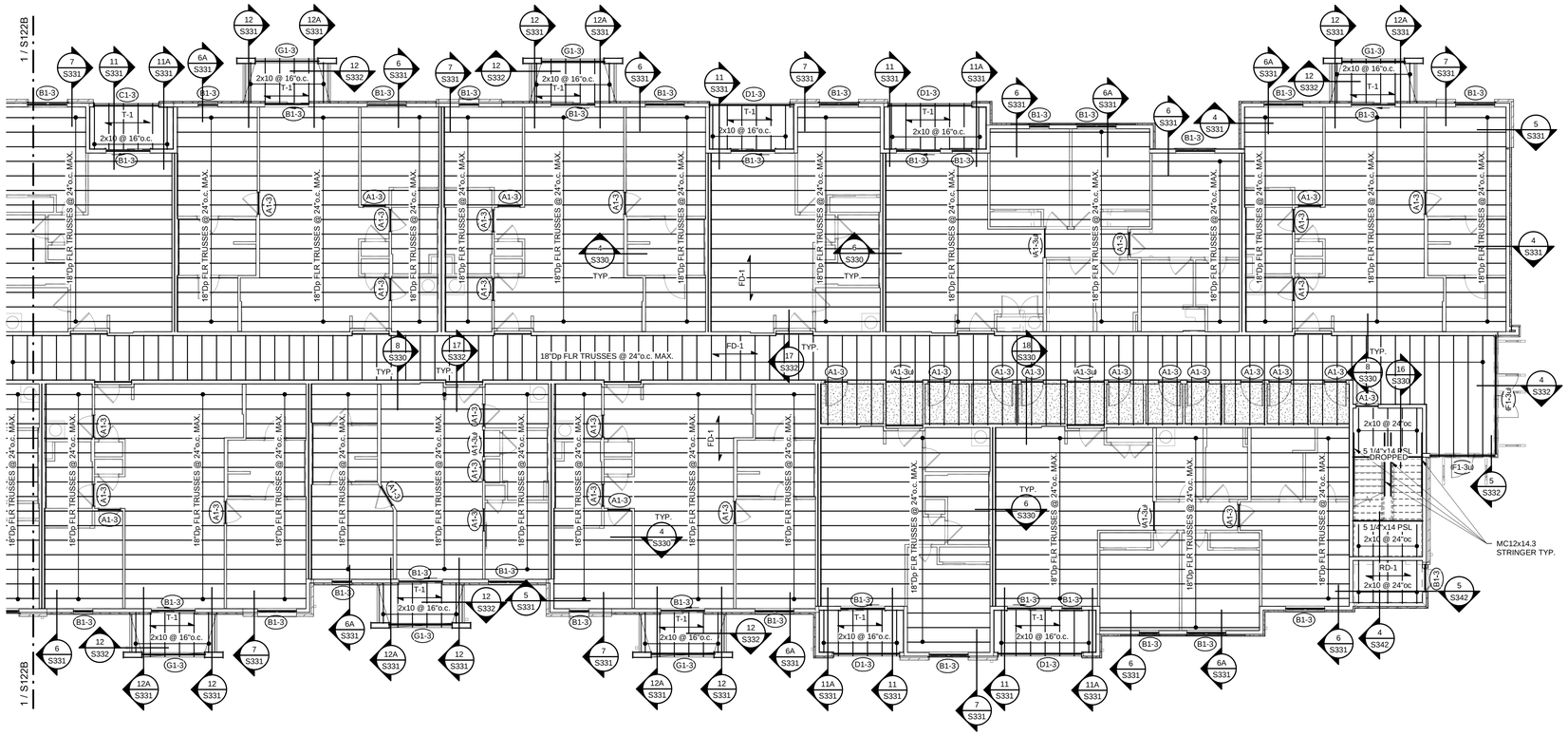
SHEET NO.

S122B

BLDG 2- SECOND FLOOR FRAMING PLAN - AREA B



1 BLDG 2- SECOND FLOOR FRAMING PLAN - AREA C

1/8" = 1'-0"

NOTES:
1.) TOP OF STEEL ELEVATION=103'-3 3/4" UNLESS NOTED OTHERWISE.
2.) REFER TO A401 FOR COURTYARD CONCRETE. SHEM WALL LOCATIONS AND WIDTHS REINFORCE W/ #4 @ 12"oc EACH WAY, DRILL & EPOXY VERTICAL REINF. 3" INTO SLAB BELOW

WOOD FLOOR FRAMING

NOTES:
1. REFER TO GENERAL NOTES ON SHEET S001.
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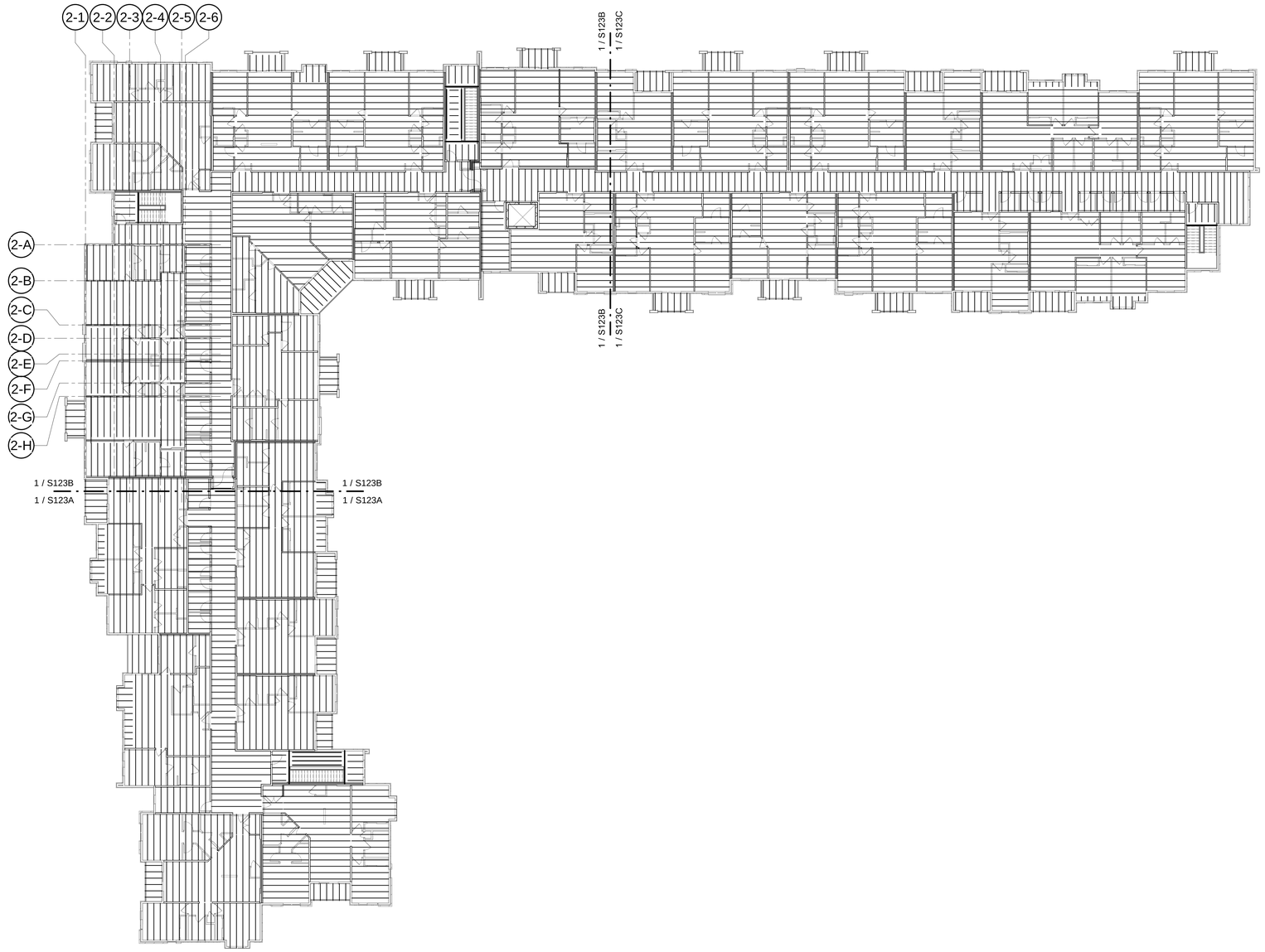
BLDG 2- SECOND FLOOR FRAMING PLAN - AREA C



1 BLDG 2 - THIRD FLOOR FRAMING PLAN

1/16" = 1'-0"

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STATE OF MISSOURI
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BUSINESS
Member
PE-2010026837
PROFESSIONAL ENGINEER
7/14/21

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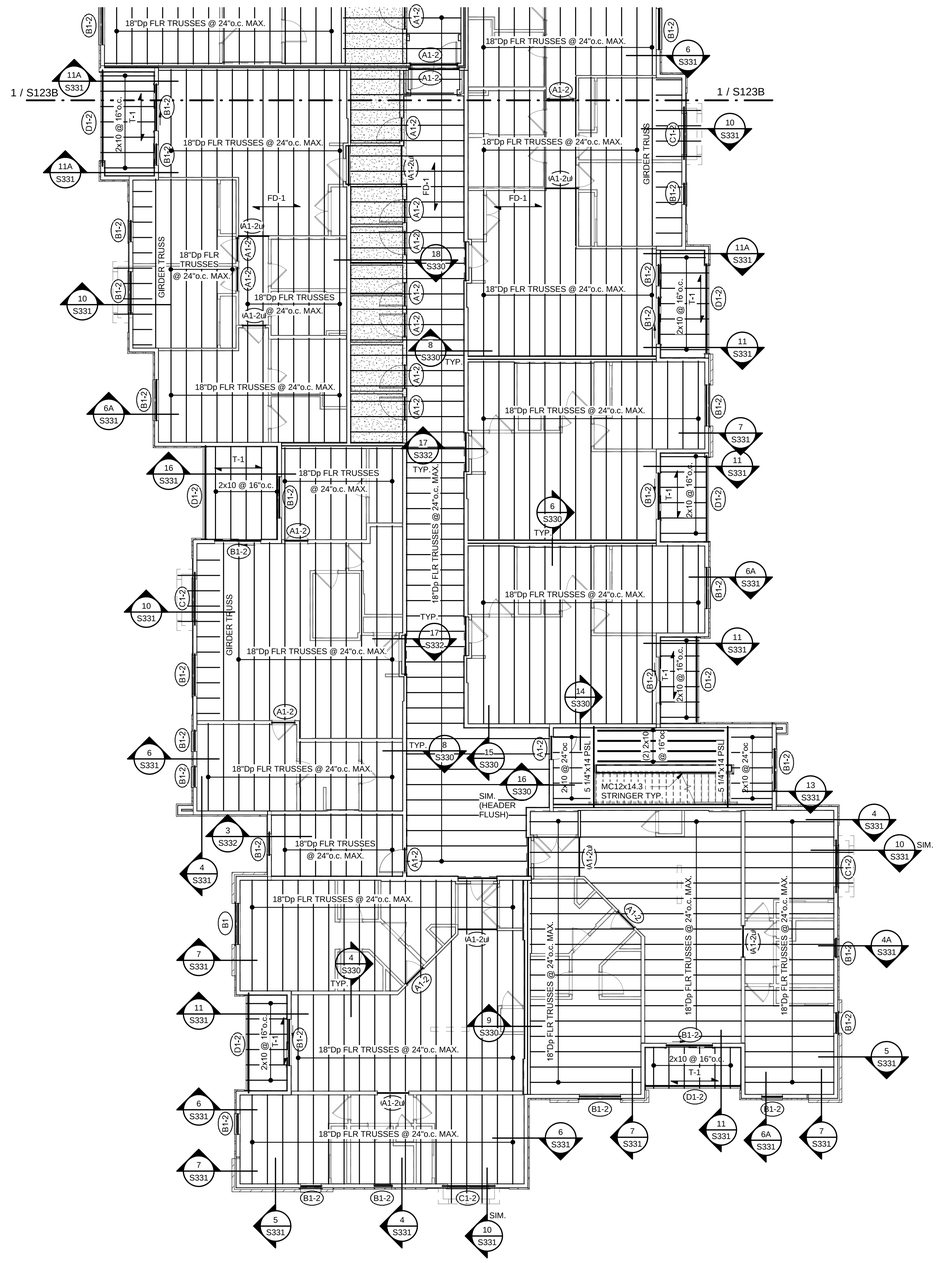
S123

BLDG 2 - THIRD FLOOR FRAMING PLAN



1 BLDG 2- THIRD FLOOR FRAMING PLAN - AREA A

1/8" = 1'-0"

WOOD FLOOR FRAMING

NOTES:

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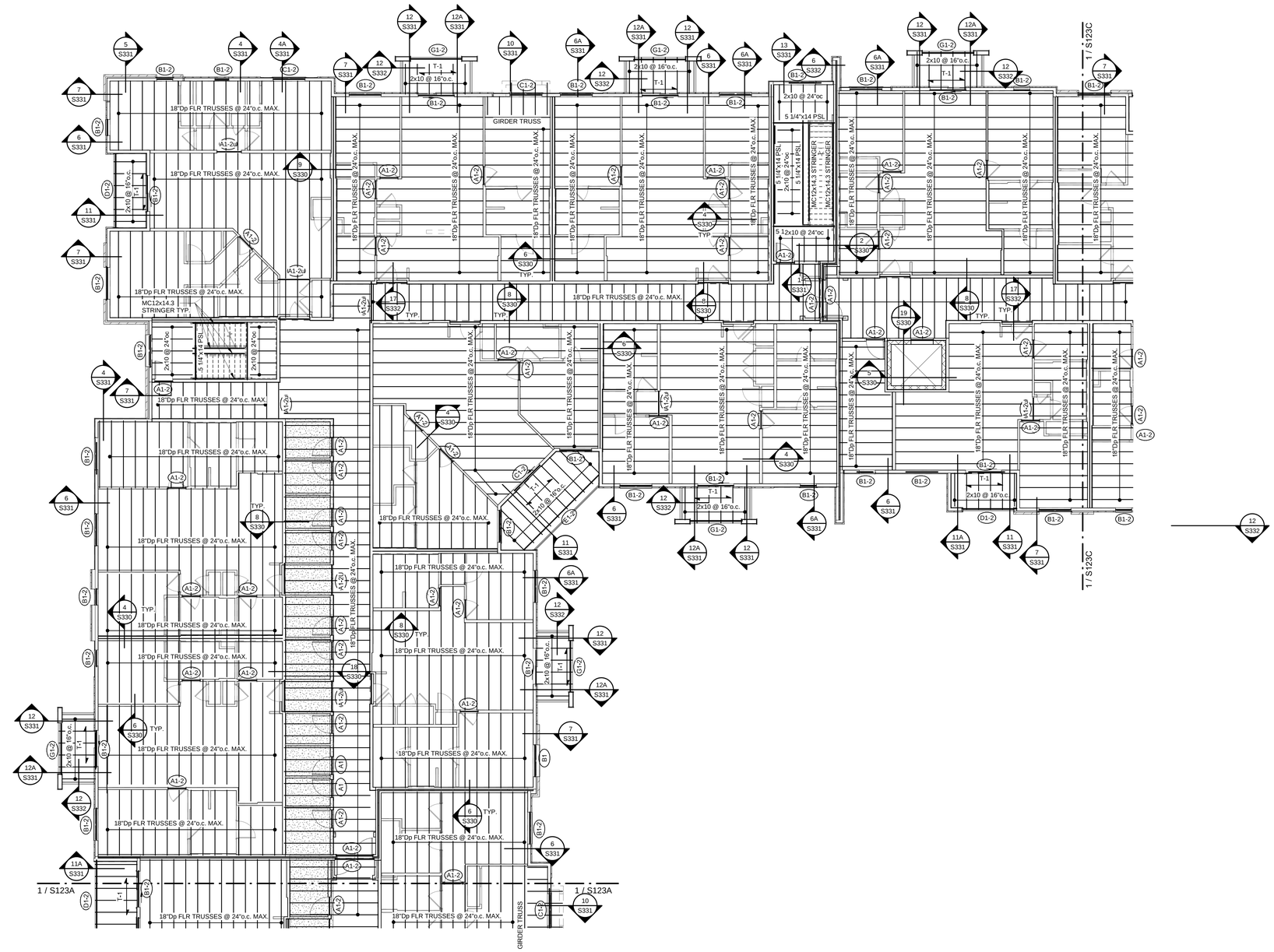
BLDG 2- THIRD FLOOR FRAMING PLAN - AREA A



1 BLDG 2- THIRD FLOOR FRAMING PLAN - AREA B

1/8" = 1'-0"

WOOD FLOOR FRAMING

NOTES:

1. REFER TO GENERAL NOTES ON SHEET S001.
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BRINKMANN CONSTRUCTORS	GENERAL CONTRACTOR
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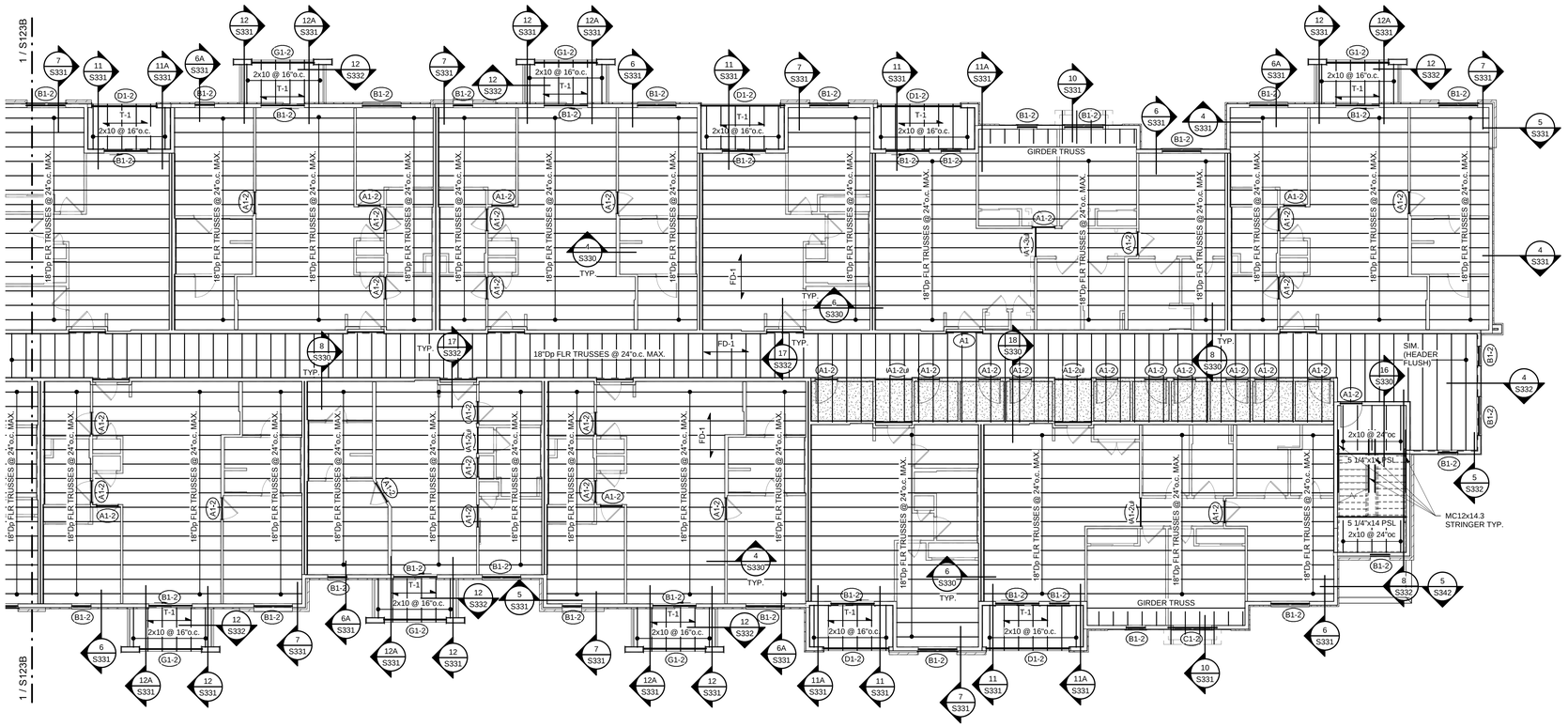
BLDG 2- THIRD FLOOR FRAMING PLAN - AREA B



1 BLDG 2- THIRD FLOOR FRAMING PLAN - AREA C

1/8" = 1'-0"

WOOD FLOOR FRAMING

NOTES:

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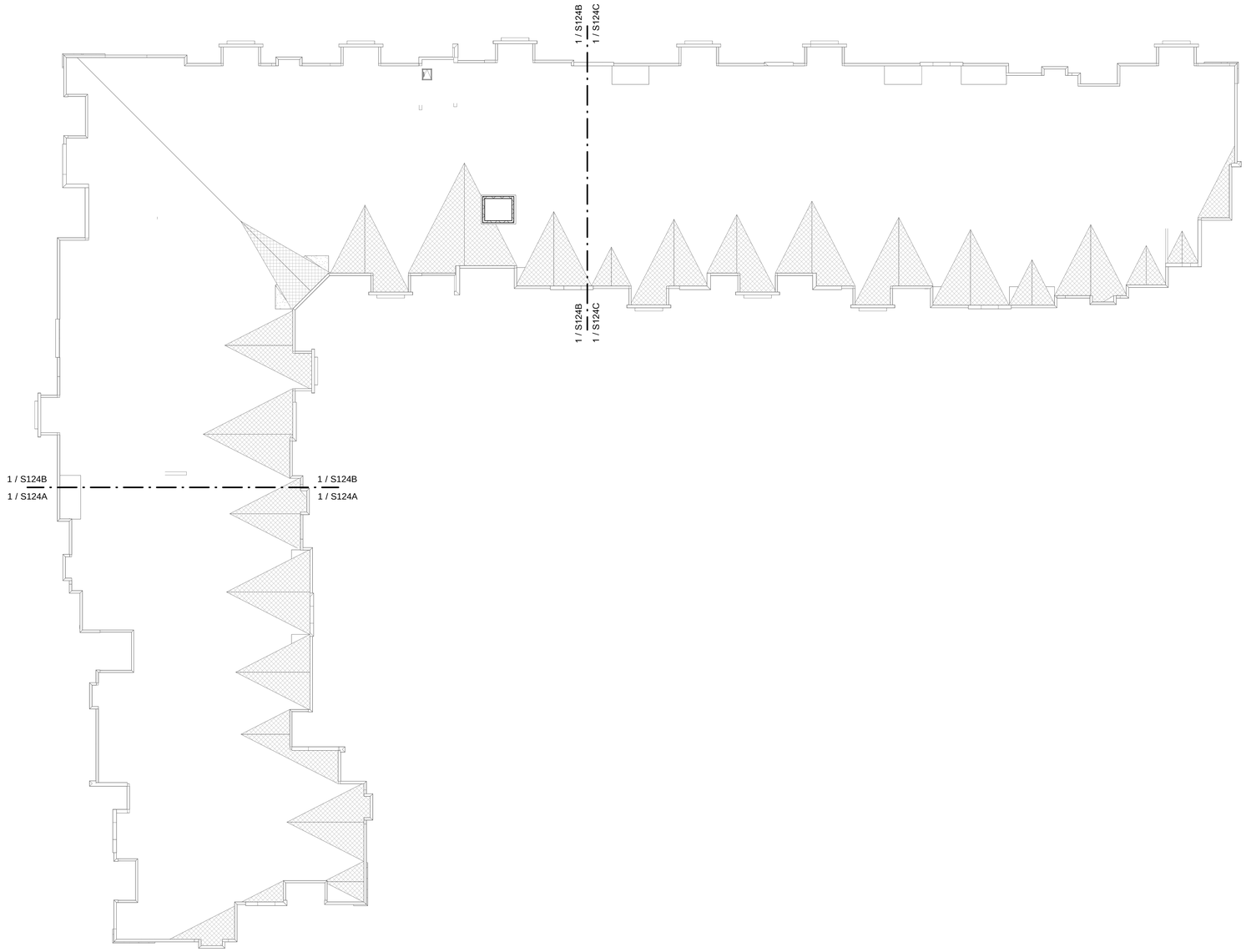
S123C

BLDG 2- THIRD FLOOR FRAMING PLAN - AREA C



1 BLDG 2 - ROOF FRAMING PLAN

1/16" = 1'-0"

WOOD FLOOR FRAMING

NOTES:

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BUSINESS
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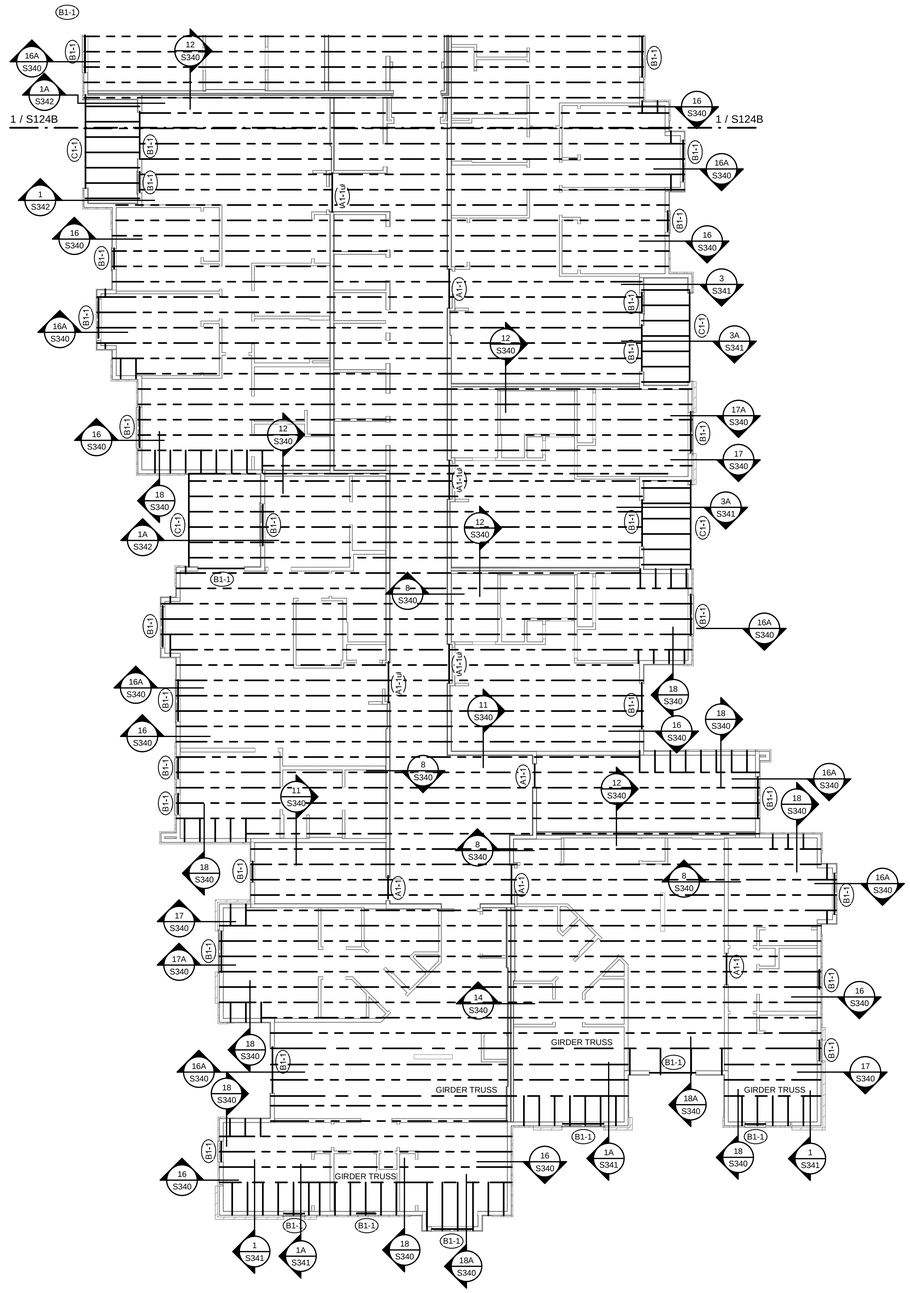
S124

BLDG 2 - ROOF FRAMING PLAN



1 BLDG 2- ROOF FRAMING PLAN - AREA A

1/8" = 1'-0"

WOOD FLOOR FRAMING

NOTES:

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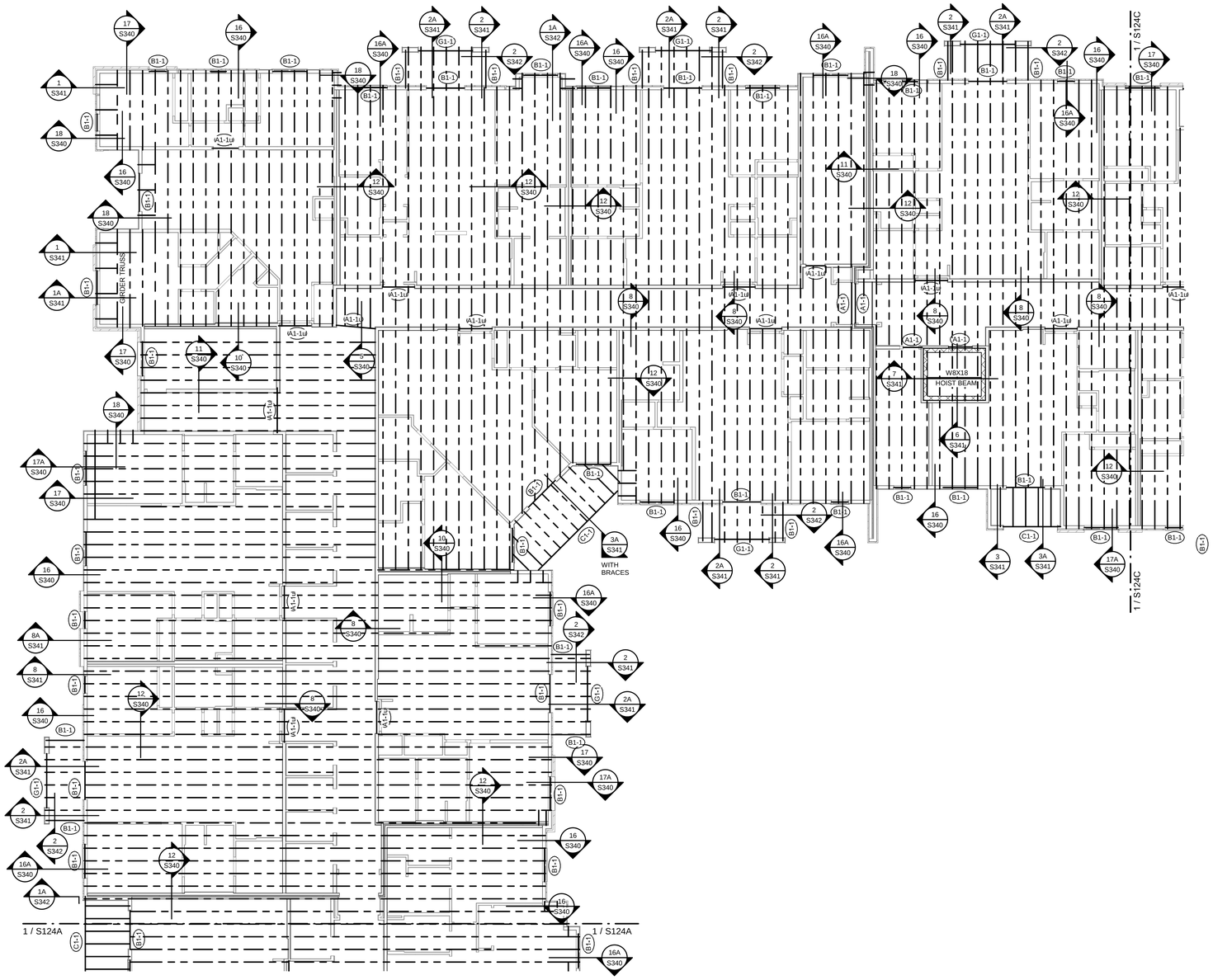
BLDG 2- ROOF FRAMING PLAN - AREA A



1 BLDG 2- ROOF FRAMING PLAN - AREA B

1/8" = 1'-0"

WOOD FLOOR FRAMING

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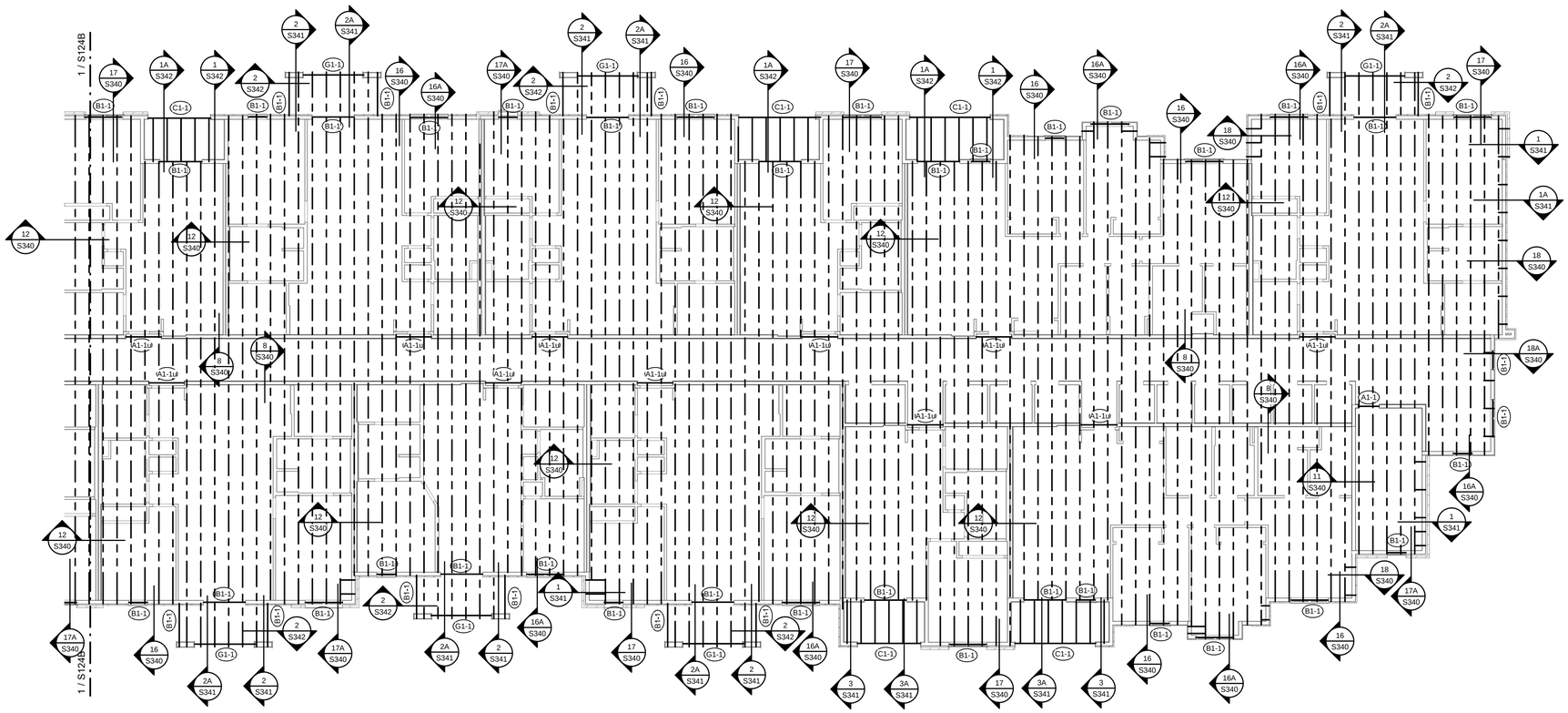

1 BLDG 2- ROOF FRAMING PLAN - AREA C

1/8" = 1'-0"

WOOD FLOOR FRAMING

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CLAY ALVIN BARNES
Number PE-2010026832
12/14/21
PROFESSIONAL ENGINEER

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SHEET NO.

S124C

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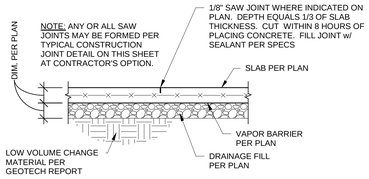
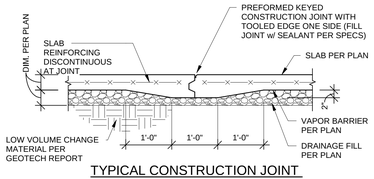


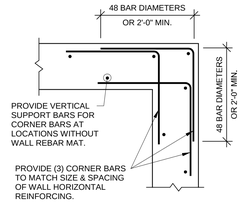
TYPICAL SAW JOINT
NOTED "SJ" ON PLAN

1 SECTION
3/4" = 1'-0"

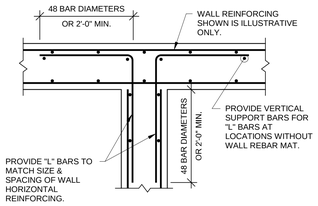


TYPICAL CONSTRUCTION JOINT
NOTED "CJ" ON PLAN

2 SECTION
3/4" = 1'-0"

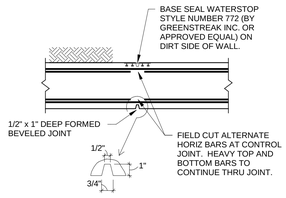


TYPICAL CORNER BARS AT CONCRETE WALLS & FOUNDATIONS

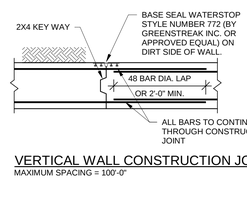


TYPICAL T-INTERSECTION REINFORCING AT CONCRETE WALLS & FOUNDATIONS

3 TYPICAL INTERSECTING CONCRETE WALL REINFORCING
3/4" = 1'-0"

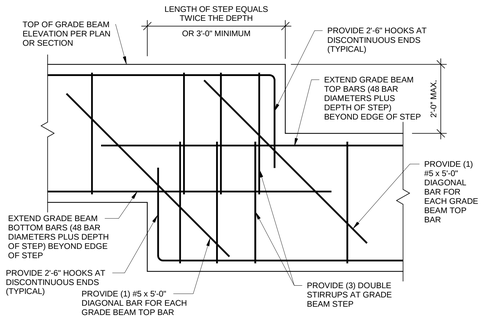


VERTICAL WALL CONTROL JOINT
MAXIMUM SPACING = 25'-0"
(COORDINATE LOCATIONS WITH ARCHITECT/ALIGN WITH MASONRY CONTROL/EXPANSION JOINTS ABOVE)

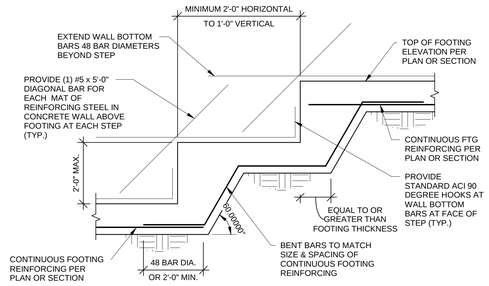


VERTICAL WALL CONSTRUCTION JOINT
MAXIMUM SPACING = 100'-0"

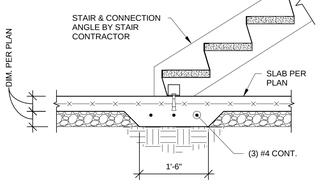
4 VERTICAL WALL CONTROL JOINT
3/4" = 1'-0"



5 TYPICAL GRADE BEAM STEP
3/4" = 1'-0"

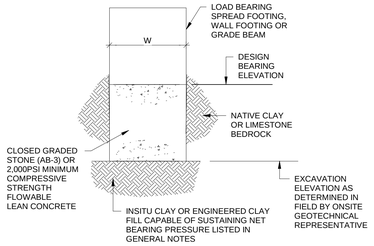


6 TYPICAL FOOTING STEP
1/2" = 1'-0"



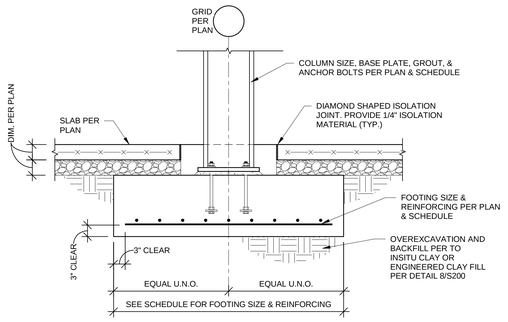
TYPICAL THICKENED SLAB AT BASE OF STAIR

7 SECTION
3/4" = 1'-0"



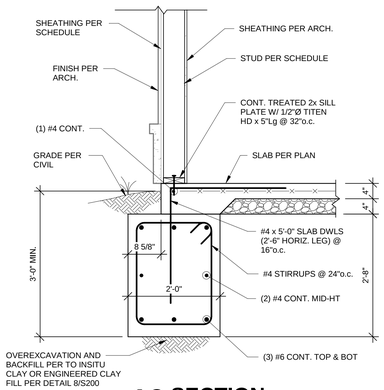
LEAN CONCRETE BACKFILL

8 OVEREXCAVATION DETAIL
3/4" = 1'-0"

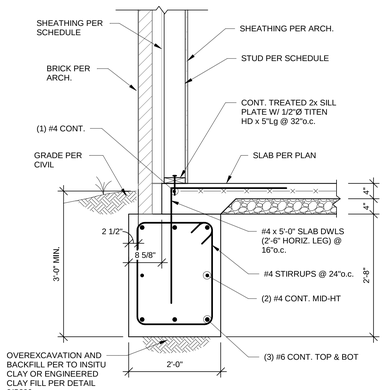


TYPICAL INTERIOR COLUMN FOOTING

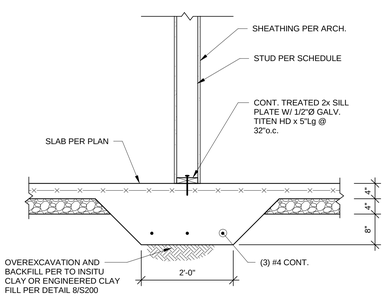
9 SECTION
3/4" = 1'-0"



10 SECTION
3/4" = 1'-0"



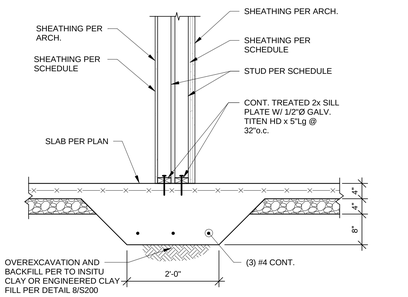
11 SECTION
3/4" = 1'-0"



TYPICAL INTERIOR BEARING WALL FOOTING

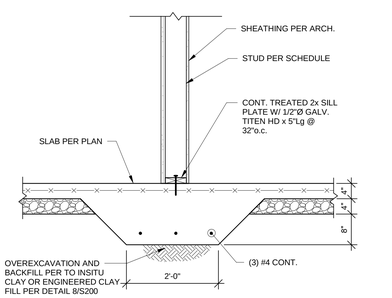
NOTES:
1. REFER TO SECTION 19/S201 FOR TYPICAL DETAIL AT INTERIOR SPREAD FOOTING IN LINE WITH WALL FOOTING

12 SECTION
3/4" = 1'-0"



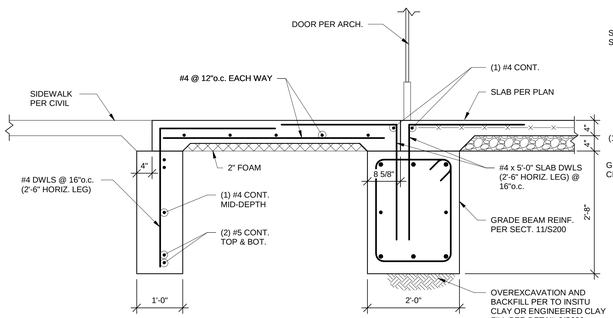
TYPICAL DEMISING WALL FOOTING

13 SECTION
3/4" = 1'-0"

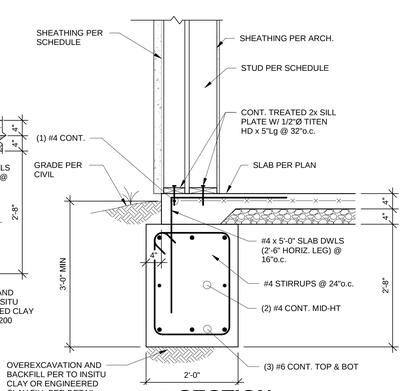


TYPICAL CORRIDOR WALL FOOTING

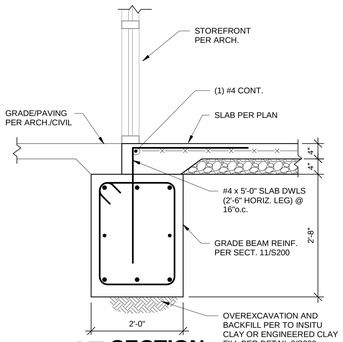
14 SECTION
3/4" = 1'-0"



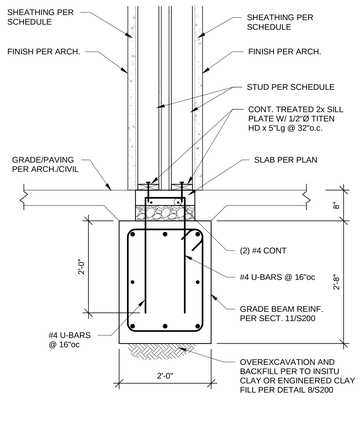
15 SECTION
3/4" = 1'-0"



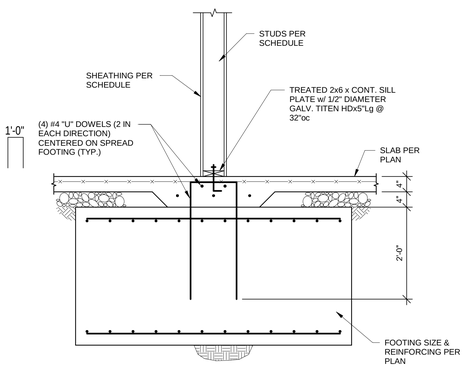
16 SECTION
3/4" = 1'-0"



17 SECTION
3/4" = 1'-0"



18 SECTION
3/4" = 1'-0"



TYPICAL INTERIOR SPREAD FOOTING AT END OF SHEAR WALL

19 SECTION
3/4" = 1'-0"

TR¡ ARCHITECTS
BOB D CAMPBELL & COMPANY
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LATIMER SOMMERS & ASSOCIATES
LATIMER SOMMERS & ASSOCIATES
LATIMER SOMMERS & ASSOCIATES

ARCHITECT
STRUCTURAL ENGINEER
CIVIL ENGINEER
GENERAL CONTRACTOR
MECHANICAL ENGINEER
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The Signature at West Pryor

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REVISIONS 4

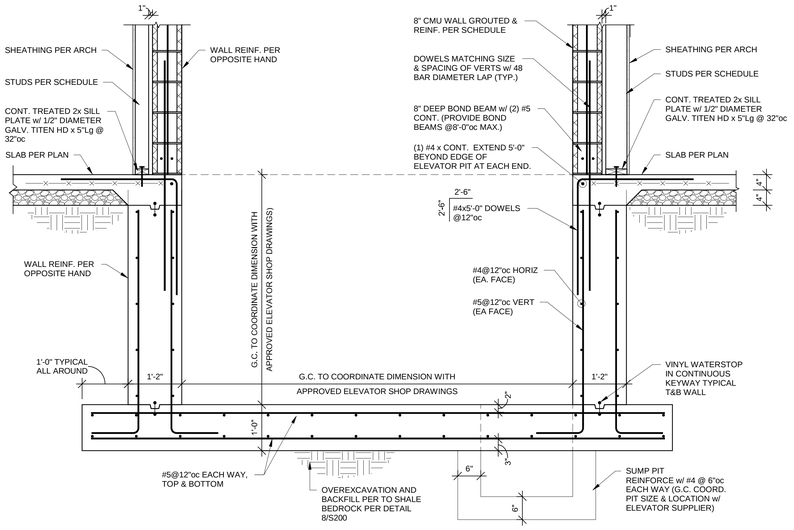
DWG BY CB

TR¡ PROJECT NO. 20-001

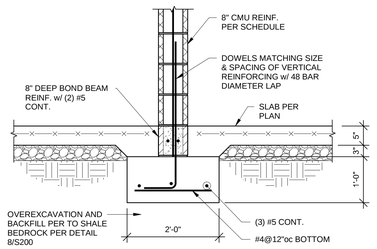
SHEET NO.

S200

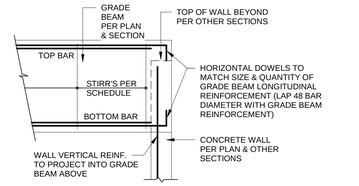
FOUNDATION SECTIONS



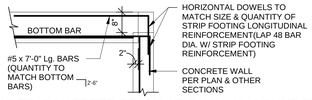
1 SECTION
3/4" = 1'-0"



2 SECTION
3/4" = 1'-0"



GRADE BEAM TO CONCRETE FOUNDATION WALL CONNECTION



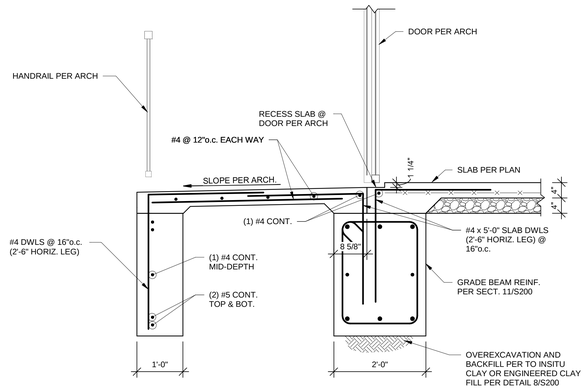
STRIP FOOTING TO CONCRETE FOUNDATION WALL CONNECTION

3 SECTION
1/2" = 1'-0"

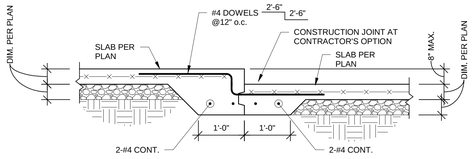
4 SECTION
3/4" = 1'-0"

5 SECTION
3/4" = 1'-0"

6 SECTION
3/4" = 1'-0"



7 SECTION
3/4" = 1'-0"



8 SECTION
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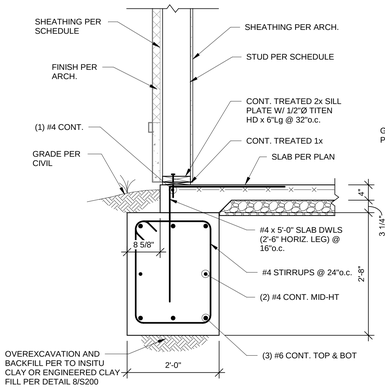
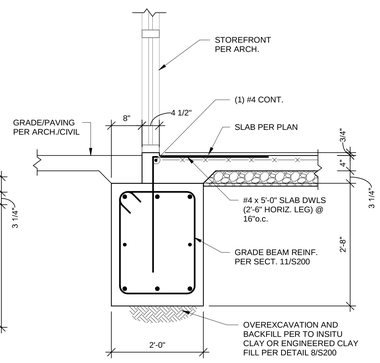
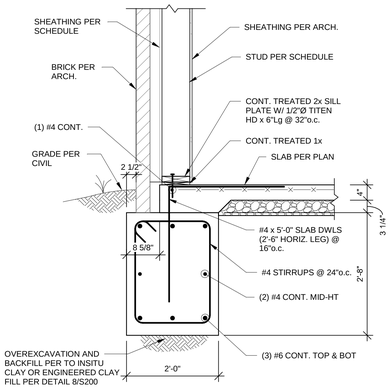


9 SECTION
3/4" = 1'-0"



10 SECTION
3/4" = 1'-0"



11 SECTION
3/4" = 1'-0"

STATE OF MISSOURI
CHAD A. BUSH
BUSH
NUMBER
PE-2010026832
PROFESSIONAL ENGINEER
2/14/2021

TR¡ ARCHITECTS	ARCHITECT
BOB D CAMPBELL & COMPANY	STRUCTURAL ENGINEER
SM ENGINEERING	CIVIL ENGINEER
BRINKMANN CONSTRUCTORS	GENERAL CONTRACTOR
LATIMER SOMMERS & ASSOCIATES	MECHANICAL ENGINEER
LATIMER SOMMERS & ASSOCIATES	PLUMBING ENGINEER
LATIMER SOMMERS & ASSOCIATES	ELECTRICAL ENGINEER

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REVISIONS 1

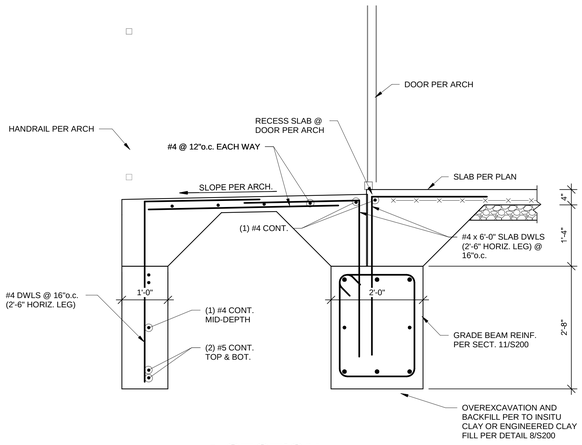
DWG BY CB

TR¡ PROJECT NO. 20-001

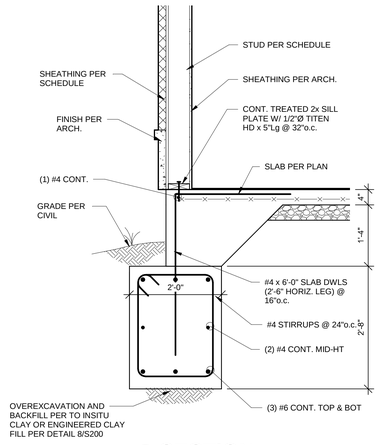
SHEET NO.

S201

FOUNDATION SECTIONS



1 SECTION
3/4" = 1'-0"



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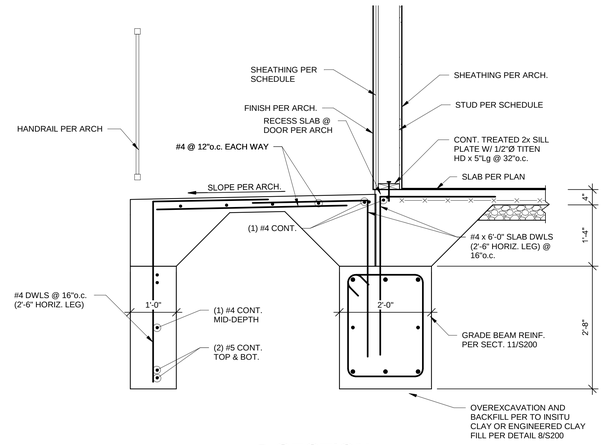
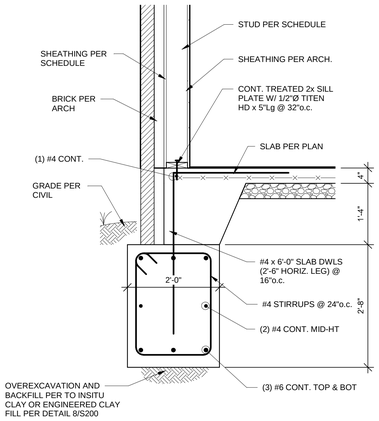
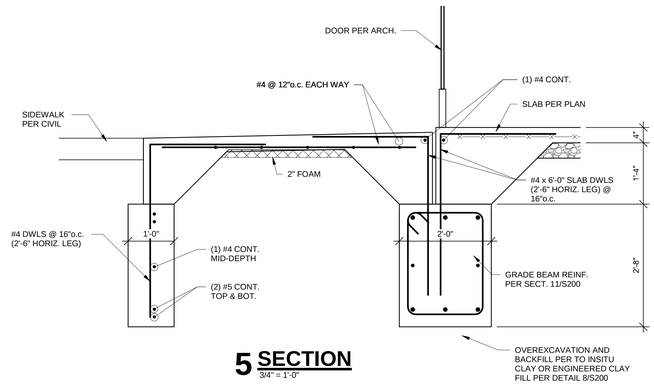


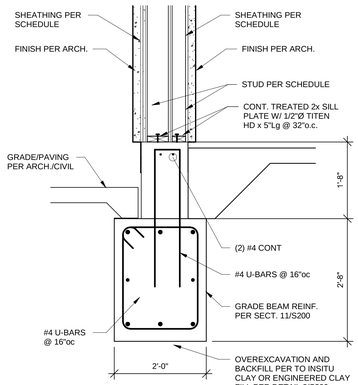
3 SECTION
3/4" = 1'-0"



4 SECTION
3/4" = 1'-0"



5 SECTION
3/4" = 1'-0"



6 SECTION
3/4" = 1'-0"

ARCHITECT
STRUCTURAL ENGINEER
CIVIL ENGINEER
GENERAL CONTRACTOR
MECHANICAL ENGINEER
PLUMBING ENGINEER
ELECTRICAL ENGINEER

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BOB D CAMPBELL & COMPANY
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REVISIONS 1

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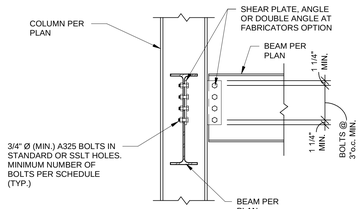
TR¡ PROJECT NO. 20-001

SHEET NO.

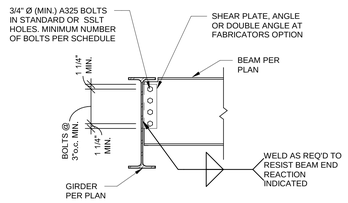
S202

FOUNDATION SECTIONS



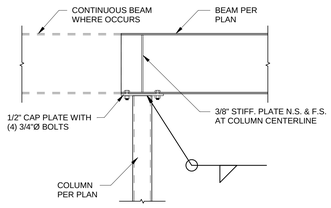
TYPICAL BEAM TO COLUMN SHEAR CONNECTION

1 DETAIL
3/4" = 1'-0"



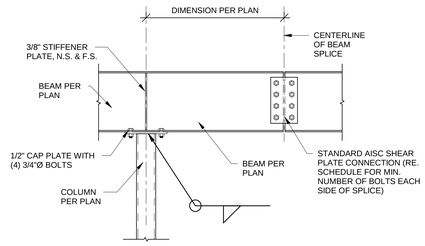
TYPICAL BEAM TO GIRDER CONNECTION

2 DETAIL
3/4" = 1'-0"



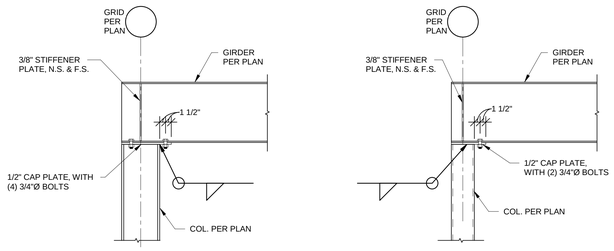
TYPICAL BEAM TO COLUMN CONNECTION

3 DETAIL
3/4" = 1'-0"



TYPICAL BEAM SPLICE

4 DETAIL
3/4" = 1'-0"



@ WIDE FLANGE COLUMN @ HSS COLUMN

TYPICAL ROOF BEAM TO COLUMN CONNECTION AT EXTERIOR WALL

5 SECTION
3/4" = 1'-0"

SHEAR CONNECTION MINIMUM ROWS OF BOLTS SCHEDULE

BEAM SIZE	MINIMUM ROWS OF BOLTS
W8,C8	2
W10,C10	2
W12,C12	2
W14	3
W16, C15	3
W18	4
W21	5
W24	5
W27	6
W30	7
W33	8
W36	8

SCHEDULE NOTES:
1. ALL BOLTS SHALL BE 3/4" Ø ASTM A325 MINIMUM.
2. ALL BOLTS SHALL BE SPACED AT 3"o.c. MINIMUM.
3. ALL BOLTS SHALL HAVE HEAVY HEX NUTS.
4. ALL BOLTS SHALL BE FULLY PRE-TENSIONED.
5. CLIP ANGLES MAY BE SHOP WELDED TO BEAM WEB PER AISC.
6. FOR BEAMS WITH AXIAL LOADS PER DRAWINGS, BOLTS AND CONNECTIONS SHALL BE SLIP-CRITICAL PER AISC GUIDELINES. INCREASE NUMBER OF BOLTS AND/OR PROVIDE EXTENDED SHEAR PLATE CONNECTION w/ AN ADDITIONAL COLUMN OF BOLTS TO ACCOMODATE COMBINED FORCES.
7. PROVIDE ASTM A490 BOLTS IF REQUIRED TO MEET END REACTION LOAD REQUIREMENTS

STEEL CONNECTION NOTES:

1. CONNECTIONS SHOWN IN THESE DETAILS ARE MINIMUM REQUIREMENTS.
2. FABRICATOR SHALL BE RESPONSIBLE FOR THE DESIGNING AND DETAILING OF EACH CONNECTION FOR LOADS SHOWN ON THE DRAWINGS IN ACCORDANCE WITH THE SPECIFICATIONS AND THE STRUCTURAL GENERAL NOTES.
3. SUGGESTED CONNECTION DETAILS ARE SHOWN.
4. FABRICATOR MAY OPT TO USE OTHER AISC APPROVED CONNECTIONS IN LIEU OF THESE SHOWN HEREIN.
5. CONNECTION DETAILING SHALL COMPLY WITH THE STANDARD DETAILS SHOWN IN THE LATEST EDITION OF THE AISC MANUAL OF STEEL CONSTRUCTION.
6. BOLT SPACING AND EDGE DISTANCES SHALL BE ADJUSTED PER AISC MANUAL FOR BOLTS LARGER THAN 3/4" DIAMETER.

STATE OF MISSOURI
CHAD A. BOSE
BUSINESS
Member
PE-2010026832
7/14/21
PROFESSIONAL ENGINEER

TR¡ ARCHITECTS	ARCHITECT
BOB D CAMPBELL & COMPANY	STRUCTURAL ENGINEER
SM ENGINEERING	CIVIL ENGINEER
BRINKMANN CONSTRUCTORS	GENERAL CONTRACTOR
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LATIMER SOMMERS & ASSOCIATES	ELECTRICAL ENGINEER

Tegethoff DEVELOPMENT

The Signature at West Pryor

2100 AND 2150 NW LOWENSTEIN DR.
LEE'S SUMMIT, MISSOURI 64081

TR¡ ARCHITECTS
TR¡ Architects
1790 S. Brentwood Blvd.
St. Louis, Missouri 63144
T: 314-395-9750
www.triarchitects.com

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DATE: 7.15.2021

REVISIONS 4

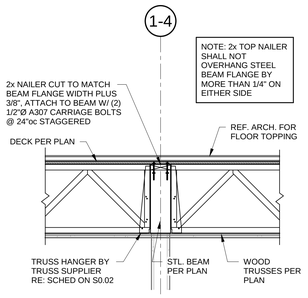
DWG BY CB

TR¡ PROJECT NO. 20-001

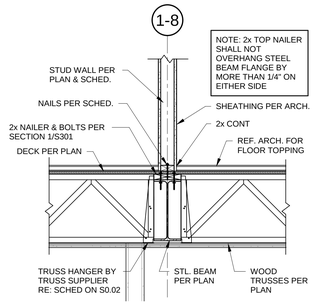
SHEET NO.

S300

STEEL FRAMING SECTIONS



1 SECTION
3/4" = 1'-0"



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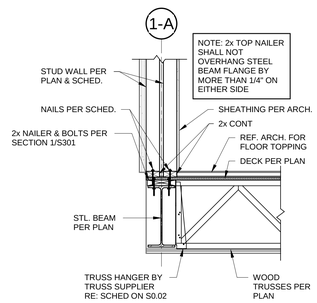

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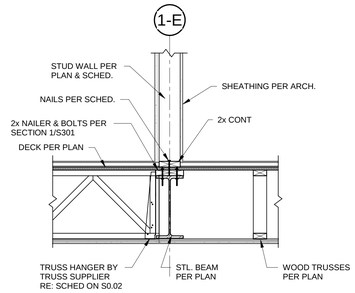
3/4" = 1'-0"



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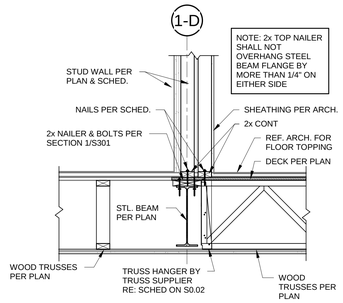

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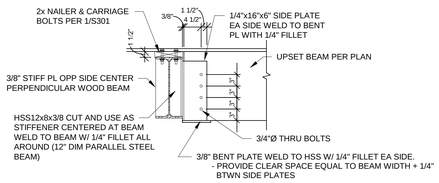

6 SECTION
3/4" = 1'-0"

ARCHITECT — TR¡ ARCHITECTS
STRUCTURAL ENGINEER — BOB D CAMPBELL & COMPANY
CIVIL ENGINEER — SM ENGINEERING
GENERAL CONTRACTOR — BRINKMANN CONSTRUCTORS
MECHANICAL ENGINEER — LATIMER SOMMERS & ASSOCIATES
PLUMBING ENGINEER — LATIMER SOMMERS & ASSOCIATES
ELECTRICAL ENGINEER — LATIMER SOMMERS & ASSOCIATES

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REVISIONS 4

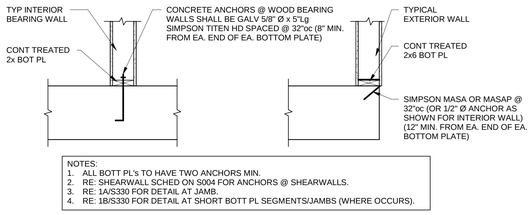
DWG BY CB

TR¡ PROJECT NO. 20-001

SHEET NO.

S301

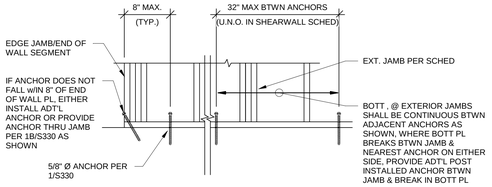
STEEL FRAMING SECTIONS



NOTES:
1. ALL BOTT PL'S TO HAVE TWO ANCHORS MIN.
2. RE: SHEARWALL SCHED ON S004 FOR ANCHORS @ SHEARWALLS.
3. RE: 1A/S330 FOR DETAIL AT JAMB.
4. RE: 1B/S330 FOR DETAIL AT SHORT BOTT PL SEGMENTS/JAMBS (WHERE OCCURS).

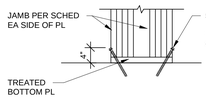
TYPICAL BOTT. PL CONNECTION TO CONCRETE

1 SECTION
3/4" = 1'-0"



TYPICAL BOTT. PL CONNECTION TO CONCRETE AT EXTERIOR JAMBS & ENDS OF WALL

1A SECTION
3/4" = 1'-0"



TYPICAL BOTT. PL CONNECTION TO CONCRETE AT SHORT PL SEGMENTS & JAMBS

1B SECTION
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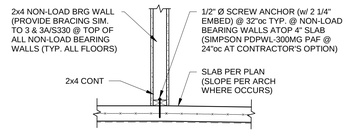

TYPICAL BOTT PL CONNECTION NON-LOAD BEARING WALLS

2 SECTION
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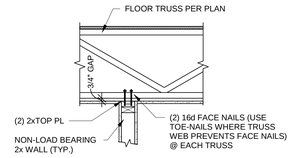

TYPICAL NON-LOAD BEARING WALL @ FLOOR TRUSS

3 SECTION
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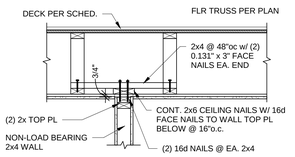
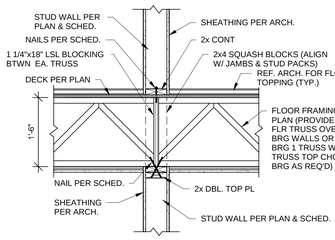


TYPICAL NON-LOAD BEARING WALL @ FLOOR TRUSS

3A SECTION
3/4" = 1'-0"



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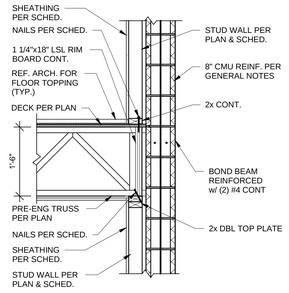
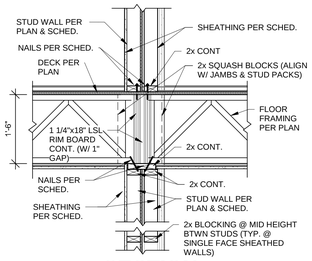
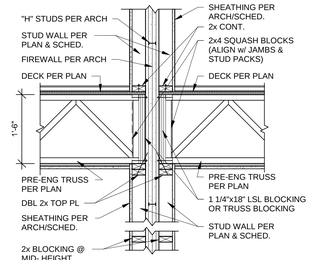
3/4" = 1'-0"



5 SECTION
3/4" = 1'-0"



6 SECTION
3/4" = 1'-0"



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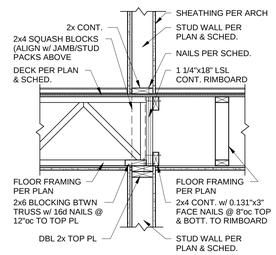
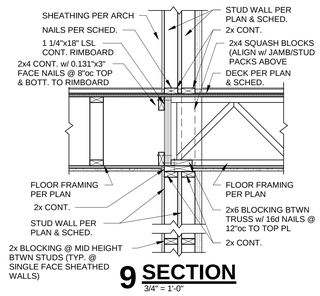
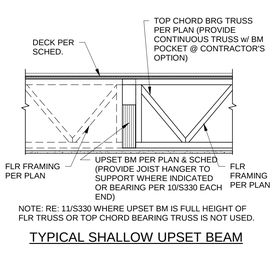


8 SECTION
3/4" = 1'-0"



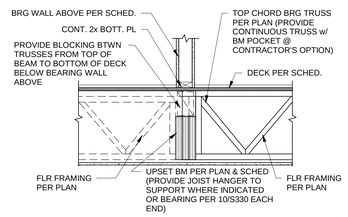
9 SECTION
3/4" = 1'-0"



NOTE: RE: 11/S330 WHERE UPSET BM IS FULL HEIGHT OF FLR TRUSS OR TOP CHORD BEARING TRUSS IS NOT USED.

TYPICAL SHALLOW UPSET BEAM

10 SECTION
3/4" = 1'-0"



TYPICAL SHALLOW UPSET BEAM SUPPORTING BEARING WALL

10A SECTION
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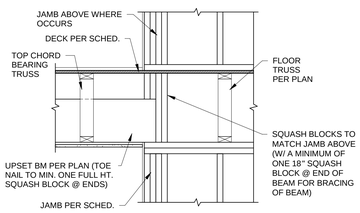

TYPICAL UPSET BEAM BEARING

11 SECTION
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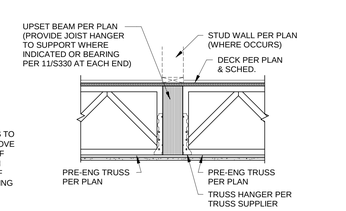

TYPICAL FULL-HEIGHT UPSET BEAM FRAMING

12 SECTION
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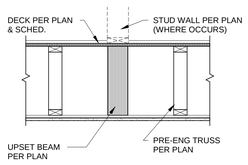

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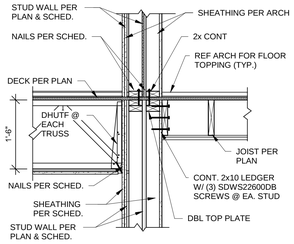

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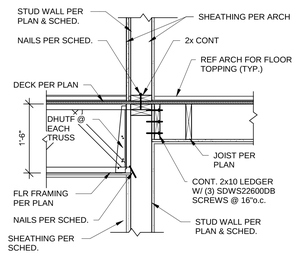

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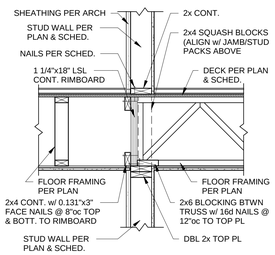

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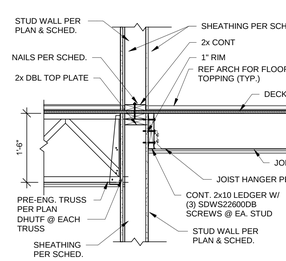

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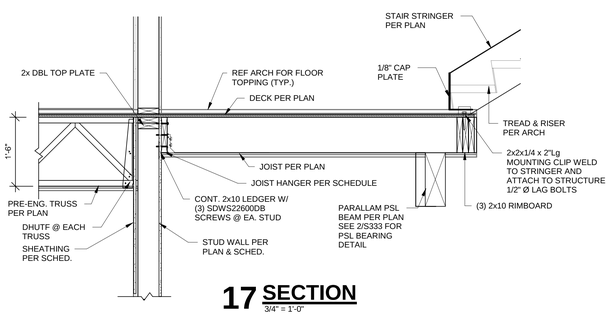

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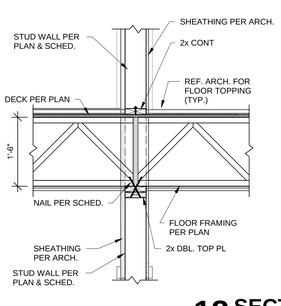

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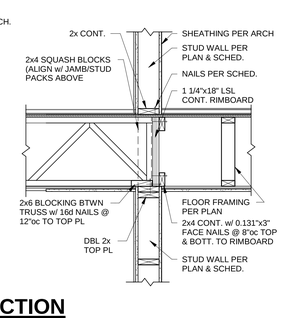

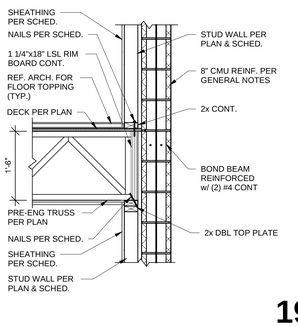

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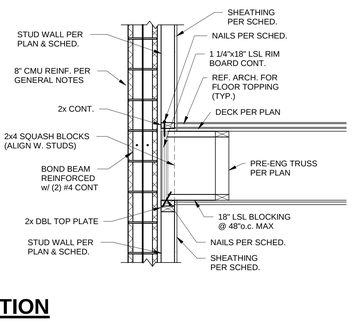

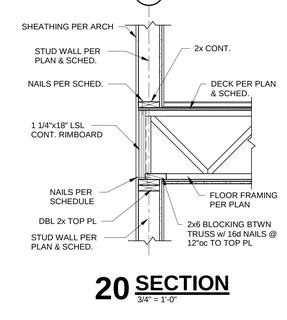

20 SECTION
3/4" = 1'-0"

TR¡ ARCHITECTS
BOB D CAMPBELL & COMPANY — ARCHITECT
SM ENGINEERING — STRUCTURAL ENGINEER
BRINKMANN CONSTRUCTORS — CIVIL ENGINEER
LATIMER SOMMERS & ASSOCIATES — GENERAL CONTRACTOR
LATIMER SOMMERS & ASSOCIATES — MECHANICAL ENGINEER
LATIMER SOMMERS & ASSOCIATES — PLUMBING ENGINEER
ELECTRICAL ENGINEER

STATE OF MISSOURI — CHAD A. SPURR — BARNES — NUMBER PE-2010026832 — 12/14/2020 — PROFESSIONAL ENGINEER

Tegethoff DEVELOPMENT

The Signature at West Pryor

2100 AND 2150 NW LOWENSTEIN DR.
LEE'S SUMMIT, MISSOURI 64081

TRi ARCHITECTS

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DATE: 7.15.2021

REVISIONS 1

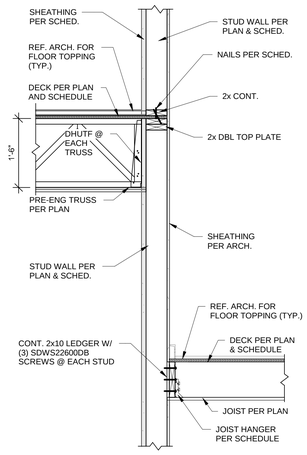
DWG BY CB

TR¡ PROJECT NO. 20-001

SHEET NO.

S330

WOOD FLOOR FRAMING SECTIONS



1 SECTION
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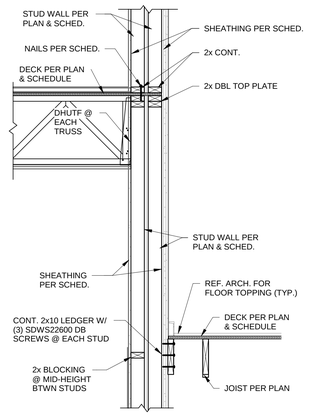

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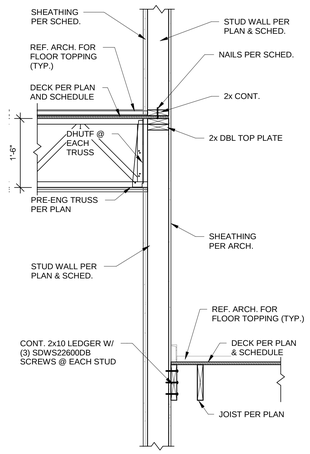

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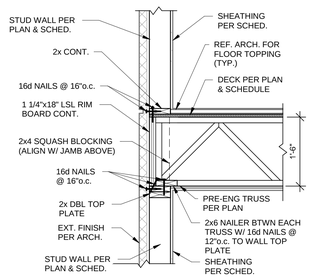

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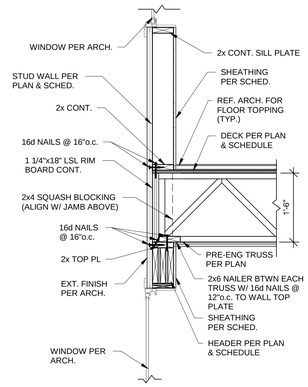
3/4" = 1'-0"



4A SECTION
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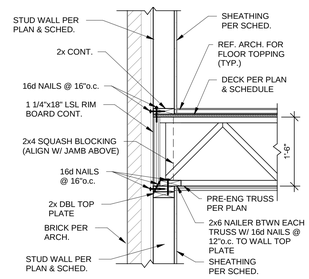

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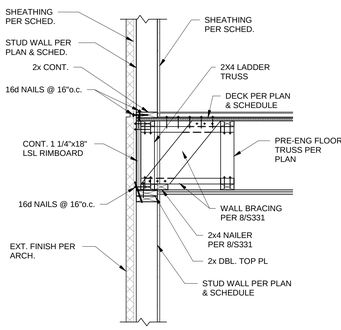
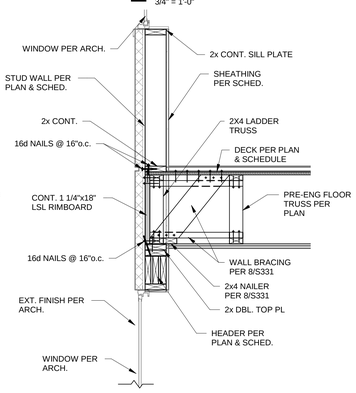


6 SECTION
3/4" = 1'-0"



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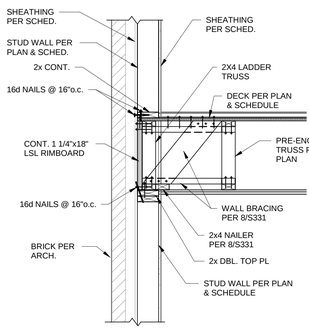
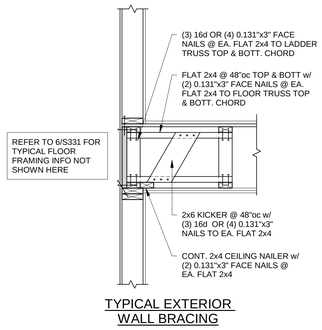
3/4" = 1'-0"

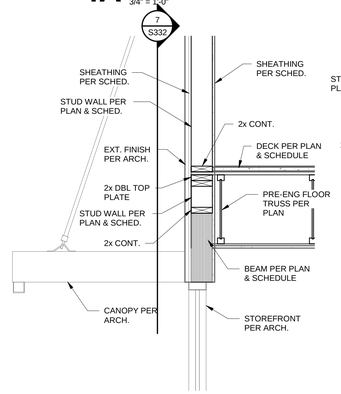


7 SECTION
3/4" = 1'-0"

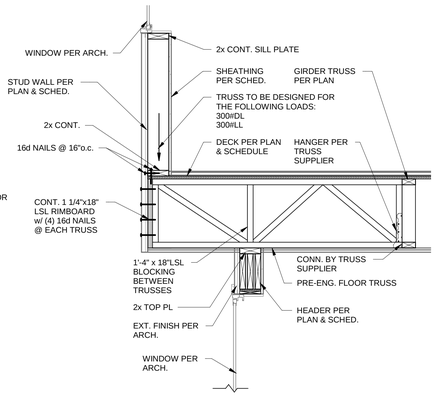


TYPICAL EXTERIOR WALL BRACING

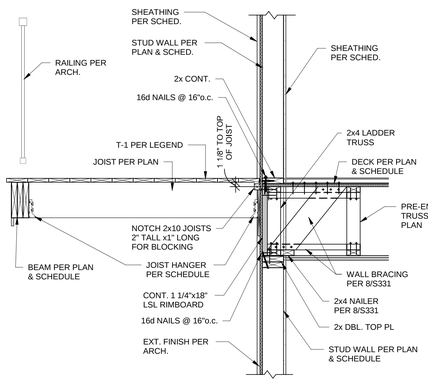
8 SECTION
3/4" = 1'-0"



9 SECTION
3/4" = 1'-0"



10 SECTION
3/4" = 1'-0"



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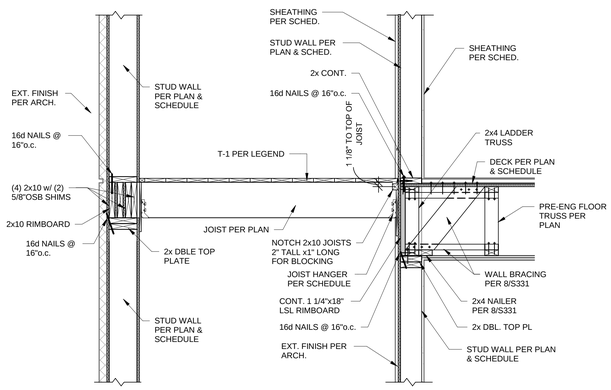

11A SECTION
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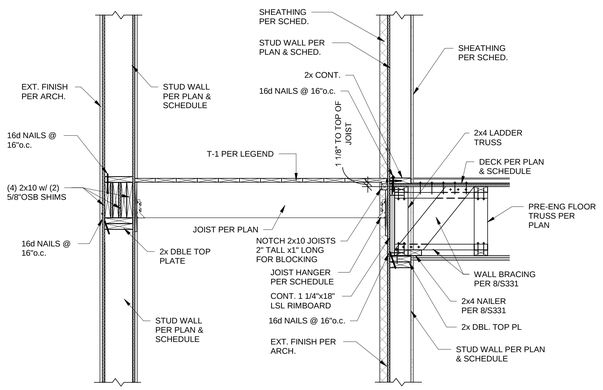

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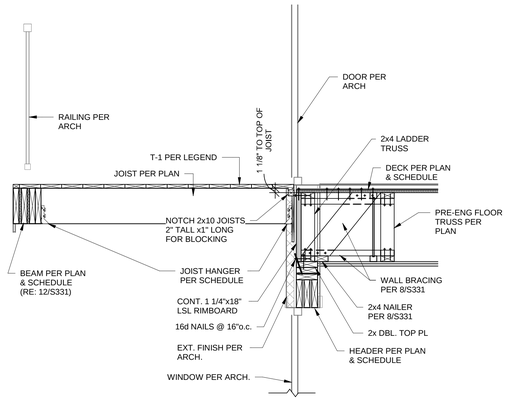

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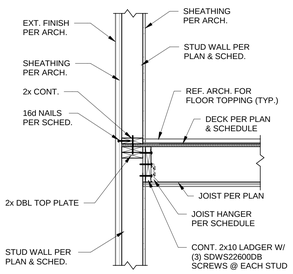

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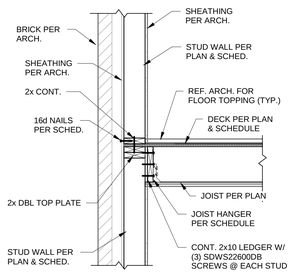

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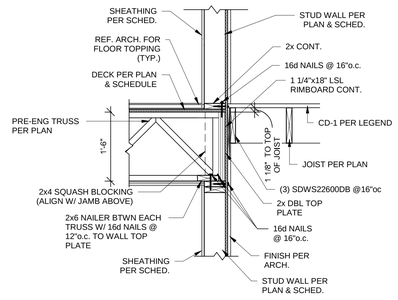

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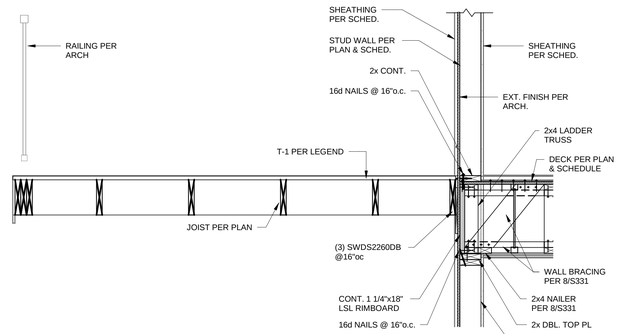

16 SECTION
3/4" = 1'-0"

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LATIMER SOMMERS & ASSOCIATES — PLUMBING ENGINEER
LATIMER SOMMERS & ASSOCIATES — ELECTRICAL ENGINEER

STATE OF MISSOURI
CHAD A. BUSH
BUSINESS
NUMBER
PE-2010026832
12/14/2020
PROFESSIONAL ENGINEER

Tegethoff DEVELOPMENT

The Signature at West Pryor

2100 AND 2150 NW LOWENSTEIN DR.
LEE'S SUMMIT, MISSOURI 64081

TR¡ Architects
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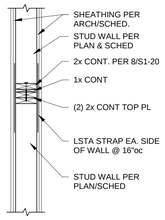
TR¡ PROJECT NO. 20-001

SHEET NO.

S331

WOOD FLOOR FRAMING SECTIONS



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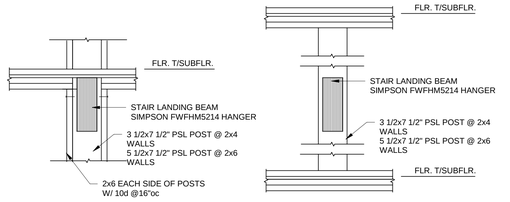
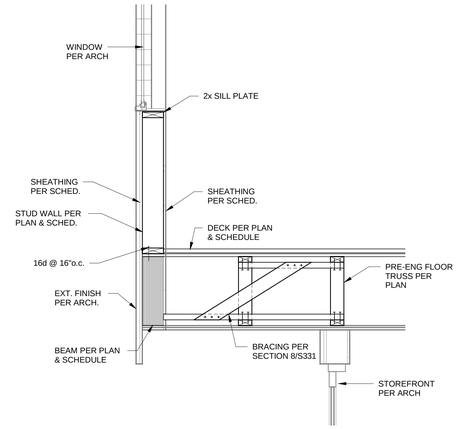


TYPICAL STAIR BEAM JAMBS

2 SECTION
3/4" = 1'-0"



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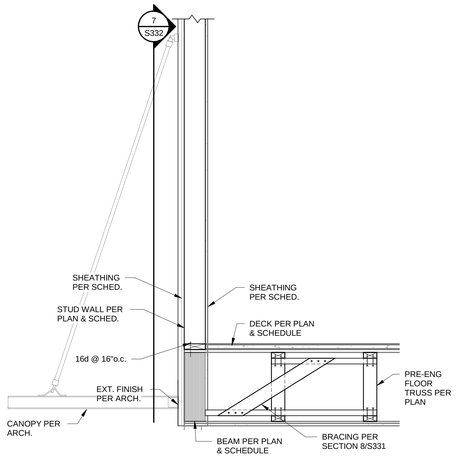

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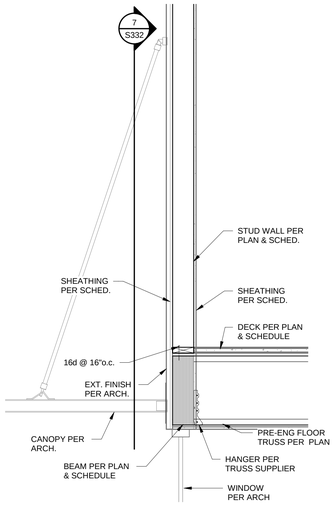

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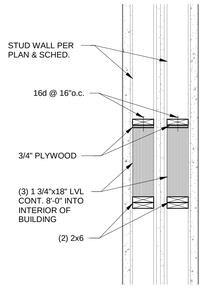

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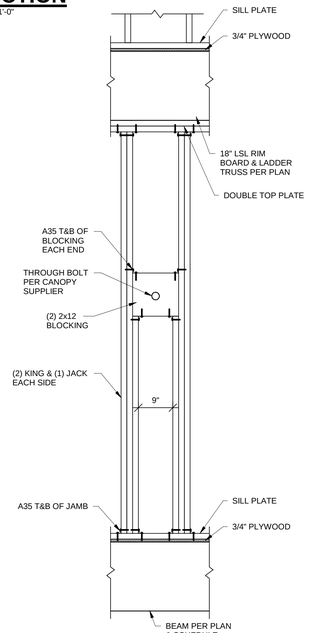

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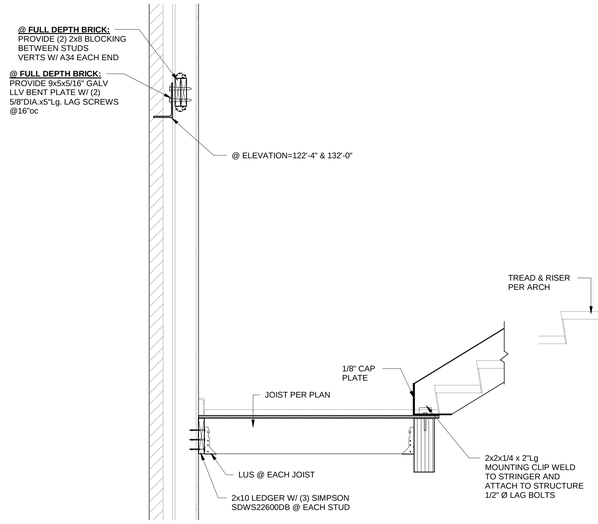

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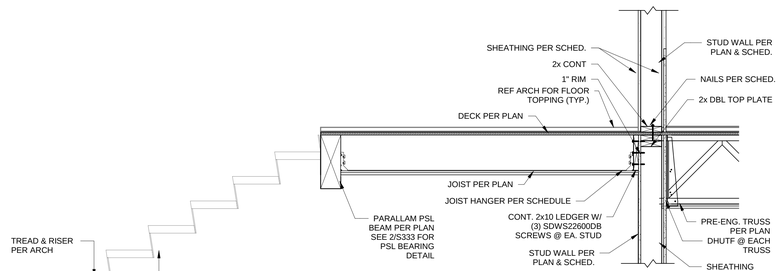
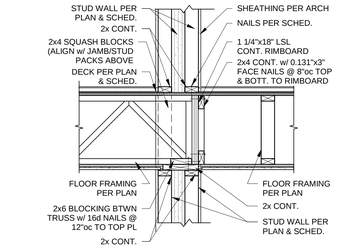
3/4" = 1'-0"





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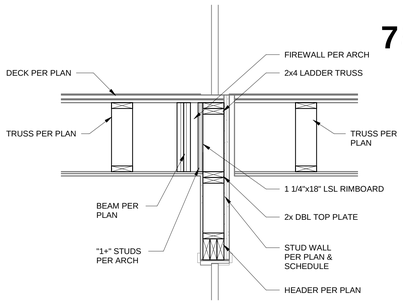
3/4" = 1'-0"



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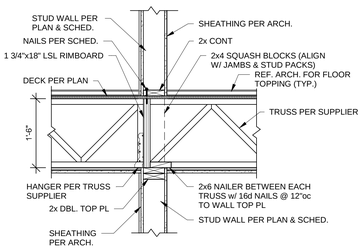
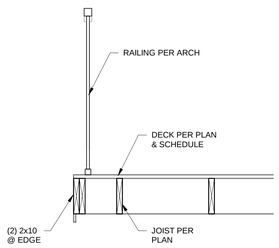


11 SECTION
3/4" = 1'-0"



12 SECTION
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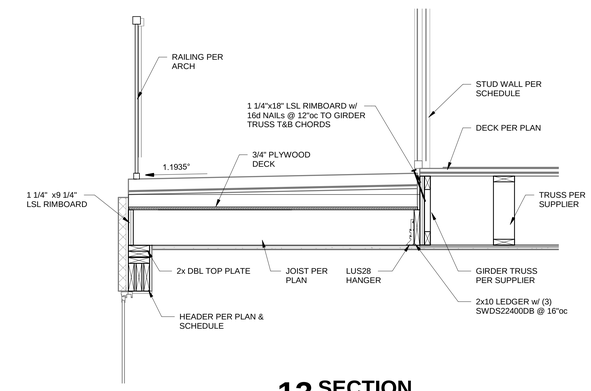

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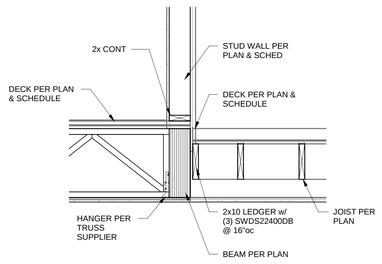

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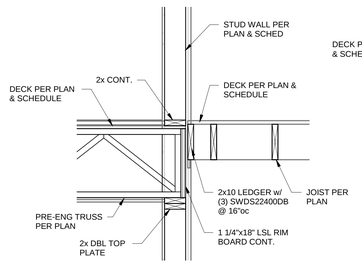

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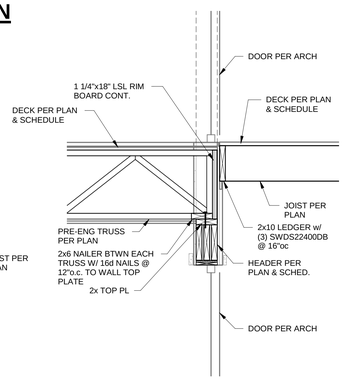
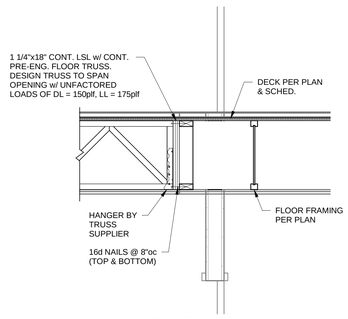


16 SECTION
3/4" = 1'-0"



17 SECTION
3/4" = 1'-0"

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BOB D CAMPBELL & COMPANY — STRUCTURAL ENGINEER
SM ENGINEERING — CIVIL ENGINEER
BRINKMANN CONSTRUCTORS — GENERAL CONTRACTOR
LATIMER SOMMERS & ASSOCIATES — MECHANICAL ENGINEER
LATIMER SOMMERS & ASSOCIATES — PLUMBING ENGINEER
LATIMER SOMMERS & ASSOCIATES — ELECTRICAL ENGINEER

STATE OF MISSOURI — CHAD A. BUSH — NUMBER PE-2010026832 — 12/14/2021 — PROFESSIONAL ENGINEER

Tegethoff DEVELOPMENT

The Signature at West Pryor

2100 AND 2150 NW LOWENSTEIN DR.
LEE'S SUMMIT, MISSOURI 64081

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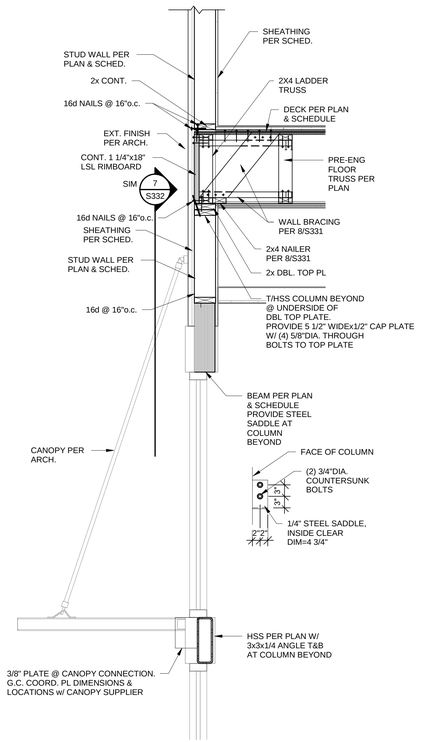
TR¡ PROJECT NO. 20-001

SHEET NO.

S332

WOOD FLOOR FRAMING SECTIONS



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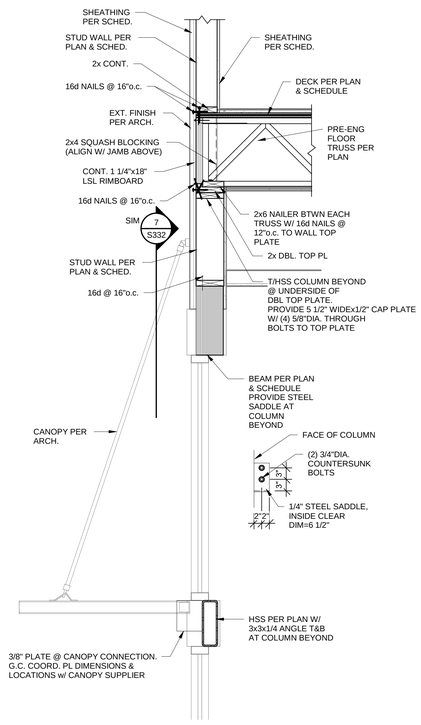

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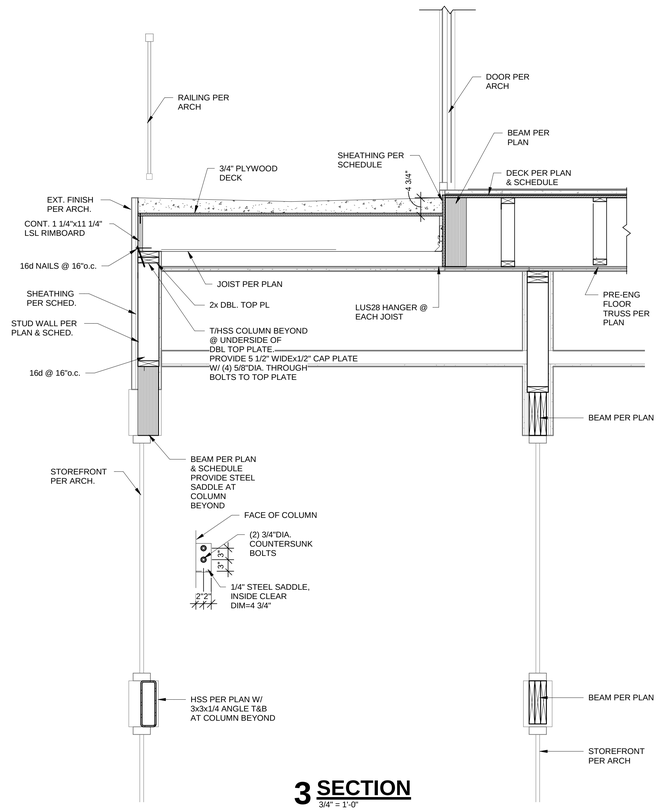

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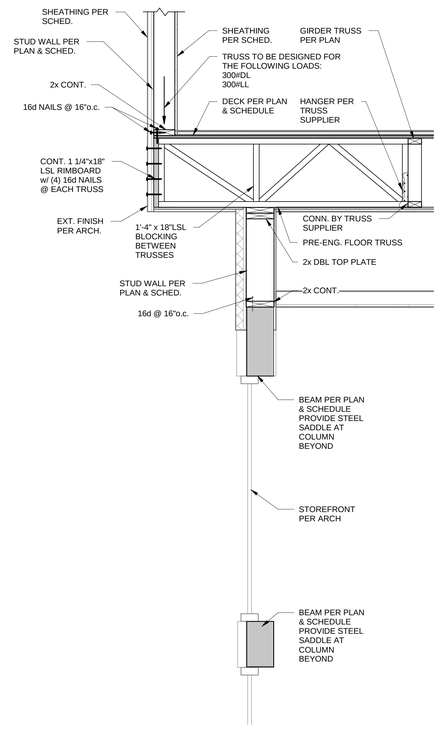

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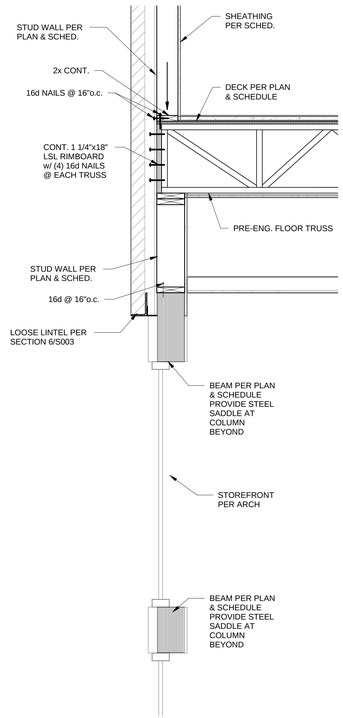

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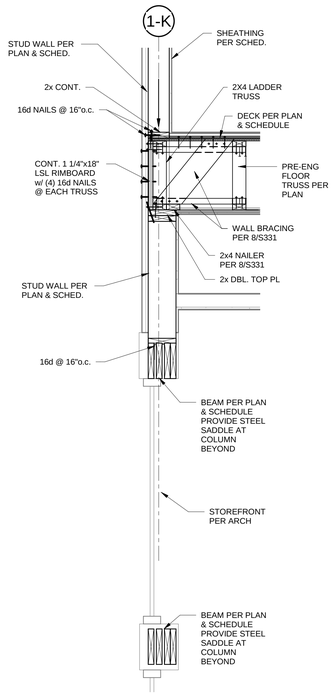

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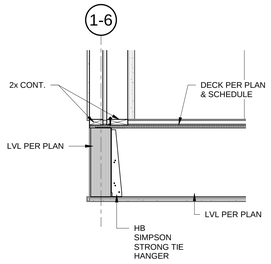

7 SECTION
3/4" = 1'-0"

ARCHITECT
STRUCTURAL ENGINEER
CIVIL ENGINEER
GENERAL CONTRACTOR
MECHANICAL ENGINEER
PLUMBING ENGINEER
ELECTRICAL ENGINEER

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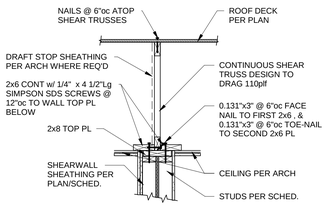
TR¡ PROJECT NO. 20-001

SHEET NO.

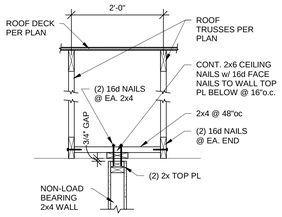
S333

WOOD FLOOR FRAMING SECTIONS



TYPICAL SHEAR TRUSS ATOP SHEARWALL

1 SECTION 3/4" = 1'-0"



TYPICAL NON-LOAD BEARING WALL/TRUSS AT ROOF

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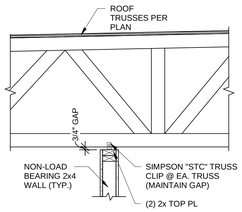
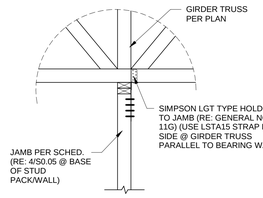


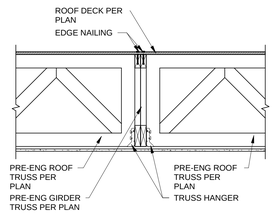
TYPICAL NON-LOAD BEARING WALL/TRUSS AT ROOF

2A SECTION 3/4" = 1'-0"



TYPICAL DETAIL AT ROOF GIRDER TRUSS BEARING

3 SECTION 3/4" = 1'-0"



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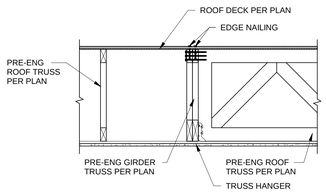
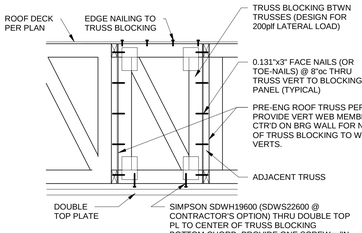


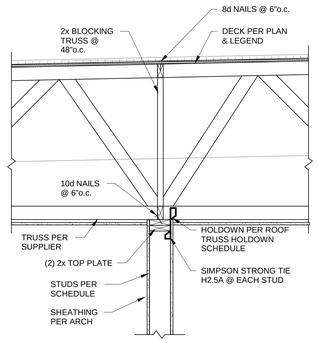
5 SECTION 3/4" = 1'-0"



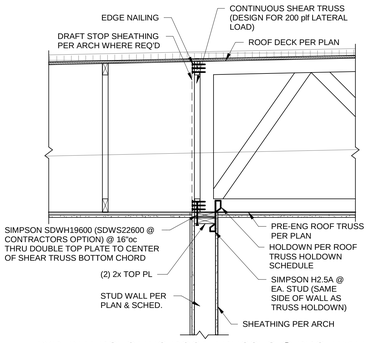
NOTES:
1. PROVIDE TRUSS BLOCKING @ ROOF LEVEL ATOP ALL SHEARWALLS WHEN A CONTINUOUS SHEAR/GIRDER TRUSS IS NOT USED.
2. TRUSS BLOCK MAY CONSIST OF A FRAMED WOOD STRUCTURAL PANEL OR PREFAB TRUSS BLOCK.
3. TRUSS MANUFACTURER TO DESIGN PREFAB TRUSS BLOCK.

TYPICAL ROOF TRUSS BLOCKING AT SHEARWALLS

6 SECTION 3/4" = 1'-0"

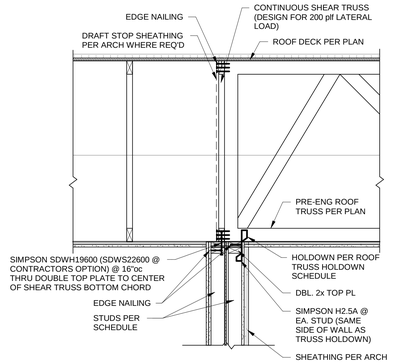


8 SECTION 3/4" = 1'-0"



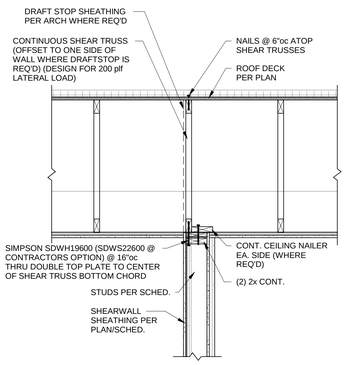
TYP. INTERSECTING ROOF TRUSSES @ BRG. WALL

9 SECTION 3/4" = 1'-0"



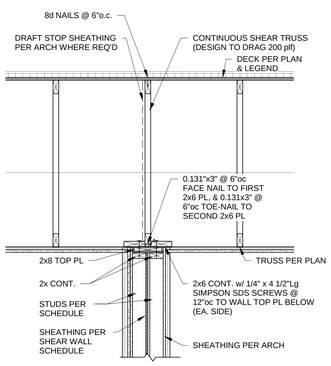
TYP. INTERSECTING ROOF TRUSSES @ BRG. WALL

10 SECTION 3/4" = 1'-0"



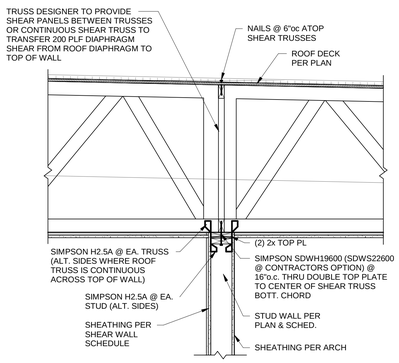
TYPICAL LATERAL TRUSS ATOP PARALLEL SHEARWALL

11 SECTION 3/4" = 1'-0"



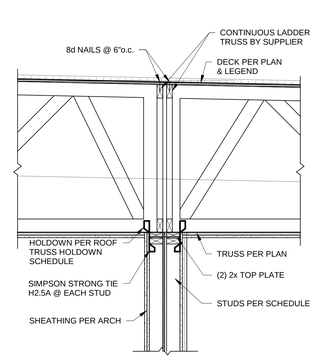
TYPICAL LATERAL TRUSS ATOP PARALLEL SHEARWALL

12 SECTION 3/4" = 1'-0"



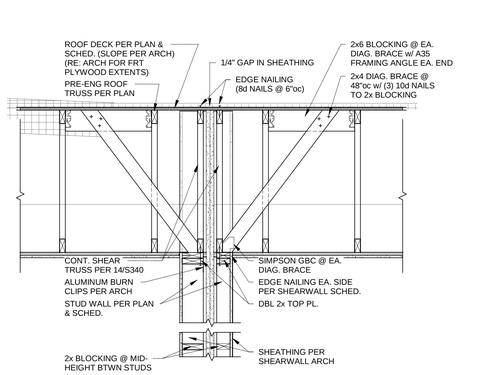
TYPICAL LATERAL TRUSS ATOP SHEARWALL PERPENDICULAR TO TRUSS SPAN

13 SECTION 3/4" = 1'-0"



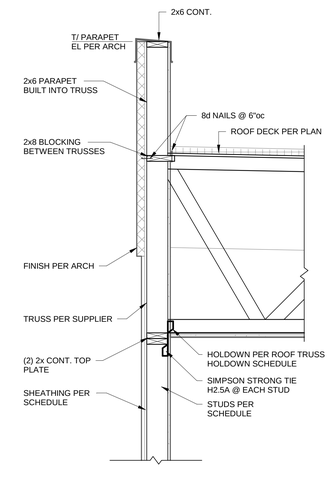
TYPICAL LATERAL TRUSS ATOP SHEARWALL PERPENDICULAR TO TRUSS SPAN

14 SECTION 3/4" = 1'-0"

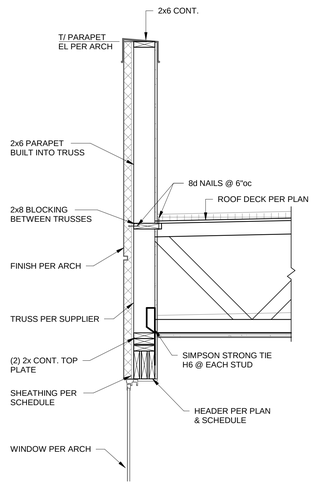


TYPICAL FIRE WALL AT ROOF

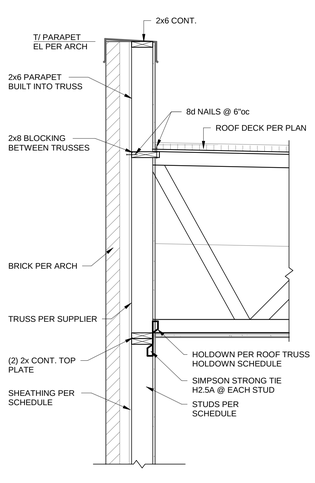
15 SECTION 3/4" = 1'-0"



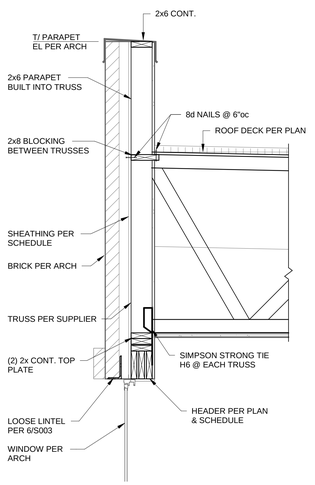
16 SECTION 3/4" = 1'-0"



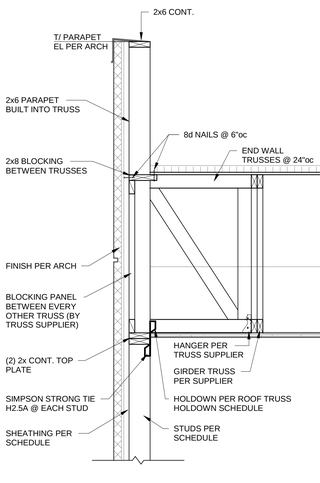
16A SECTION 3/4" = 1'-0"



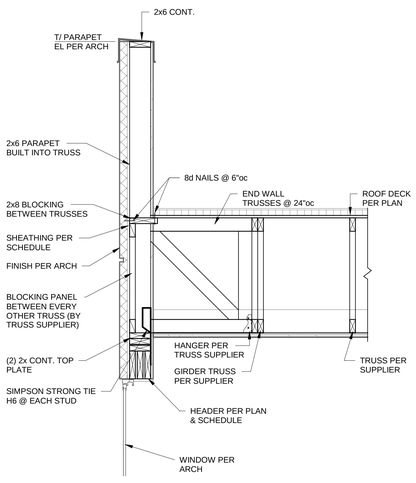
17 SECTION 3/4" = 1'-0"



17A SECTION 3/4" = 1'-0"



18 SECTION 3/4" = 1'-0"



18A SECTION 3/4" = 1'-0"

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BRINKMANN CONSTRUCTORS
LATIMER SOMMERS & ASSOCIATES
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ARCHITECT
STRUCTURAL ENGINEER
CIVIL ENGINEER
GENERAL CONTRACTOR
MECHANICAL ENGINEER
PLUMBING ENGINEER
ELECTRICAL ENGINEER

STATE OF MISSOURI — CRAIG A. SPURR — BUSINESS — NUMBER PE-2010026837 — PROFESSIONAL ENGINEER — 12/14/2021

Tegethoff DEVELOPMENT

The Signature at West Pryor

2100 AND 2150 NW LOWENSTEIN DR.
LEE'S SUMMIT, MISSOURI 64081

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DATE: 7.15.2021

REVISIONS 4

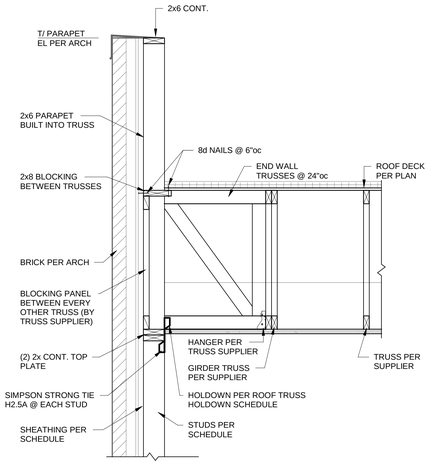
DWG BY CB

TR¡ PROJECT NO. 20-001

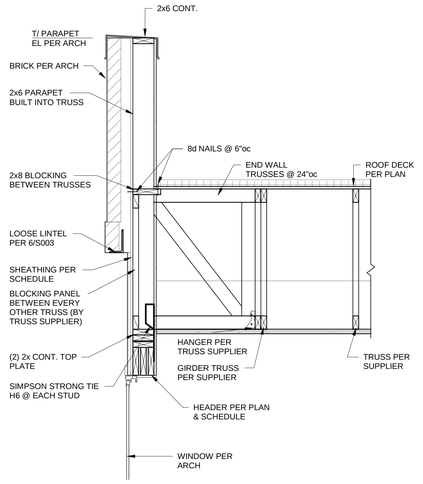
SHEET NO.

S340

WOOD ROOF FRAMING SECTIONS



1 SECTION
3/4" = 1'-0"



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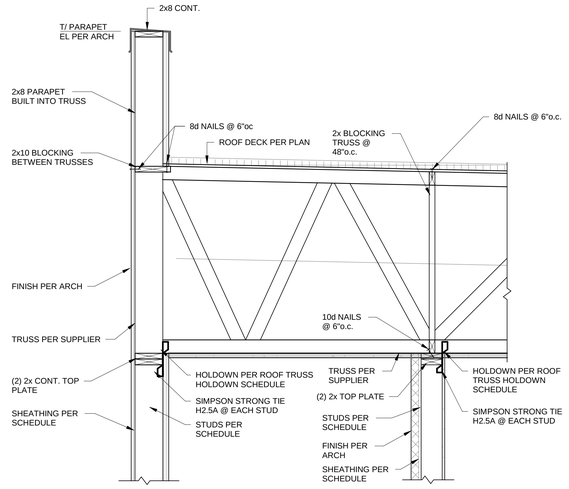

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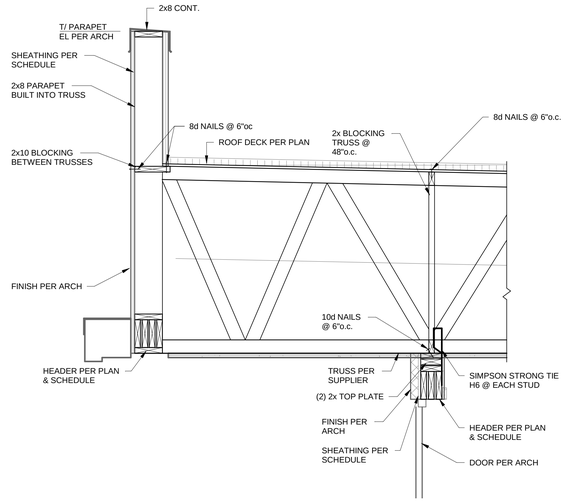

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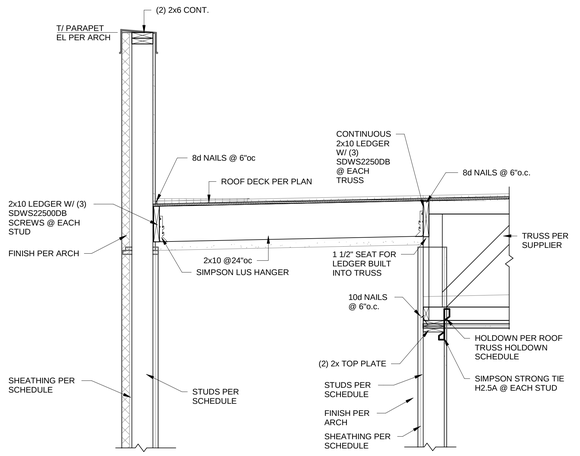
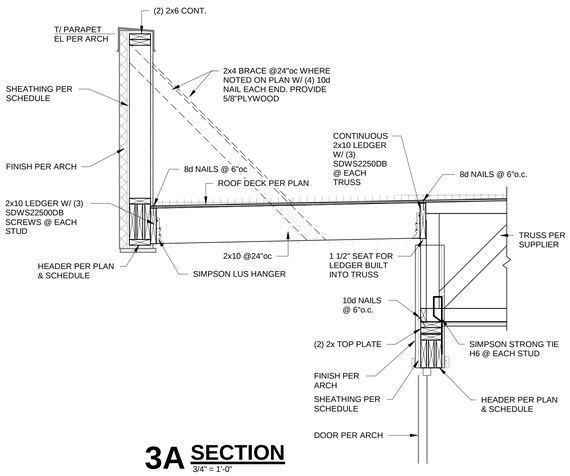
3/4" = 1'-0"



3 SECTION
3/4" = 1'-0"



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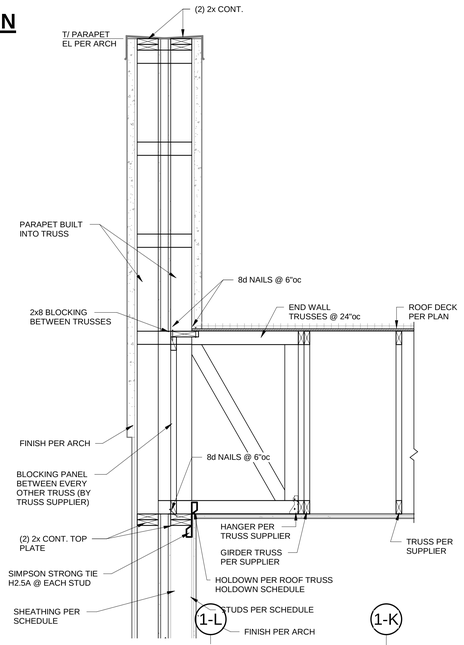

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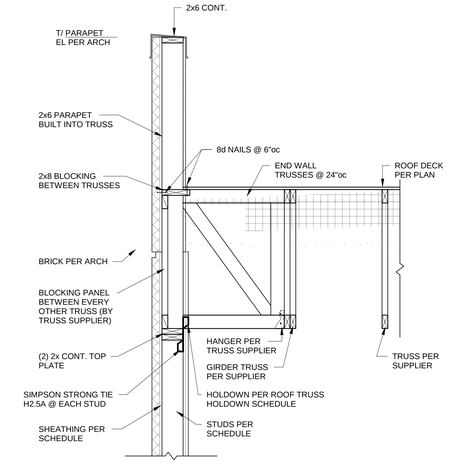

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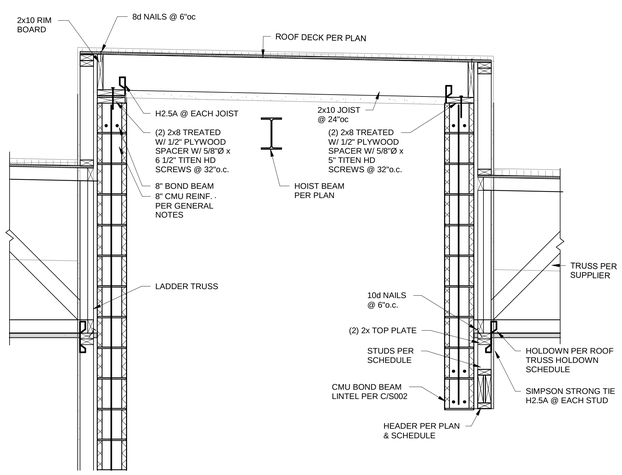
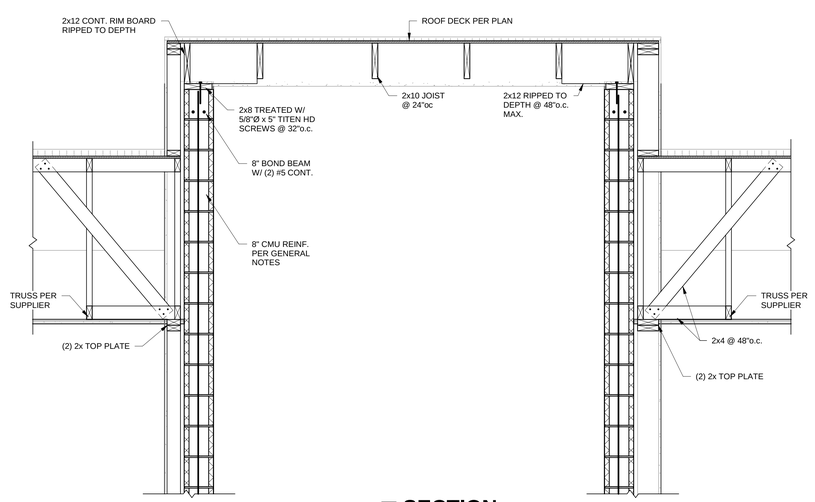


6 SECTION
3/4" = 1'-0"



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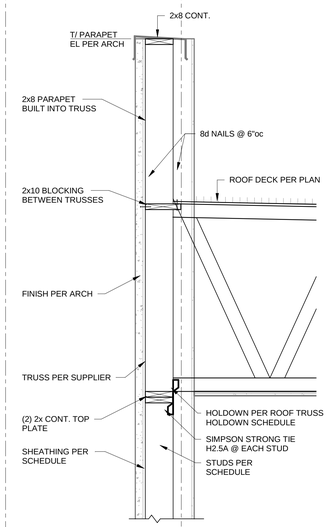

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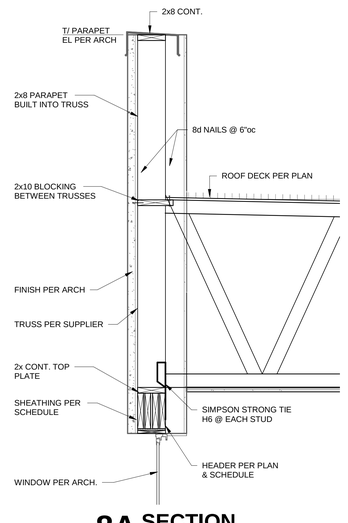
3/4" = 1'-0"



8A SECTION
3/4" = 1'-0"

ARCHITECT — TR¡ ARCHITECTS
STRUCTURAL ENGINEER — BOB D CAMPBELL & COMPANY
CIVIL ENGINEER — SM ENGINEERING
GENERAL CONTRACTOR — BRINKMANN CONSTRUCTORS
MECHANICAL ENGINEER — LATIMER SOMMERS & ASSOCIATES
PLUMBING ENGINEER — LATIMER SOMMERS & ASSOCIATES
ELECTRICAL ENGINEER — LATIMER SOMMERS & ASSOCIATES

Together Development

The Signature at West Pryor

2100 AND 2150 NW LOWENSTEIN DR.
LEE'S SUMMIT, MISSOURI 64081

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TR¡ Architects
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T: 314-395-9750
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DATE: 7.15.2021

REVISIONS 2

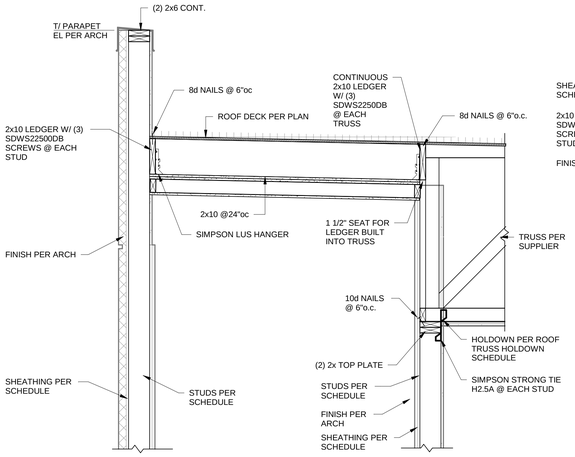
DWG BY CB

TR¡ PROJECT NO. 20-001

SHEET NO.

S341

WOOD ROOF FRAMING SECTIONS



1 SECTION
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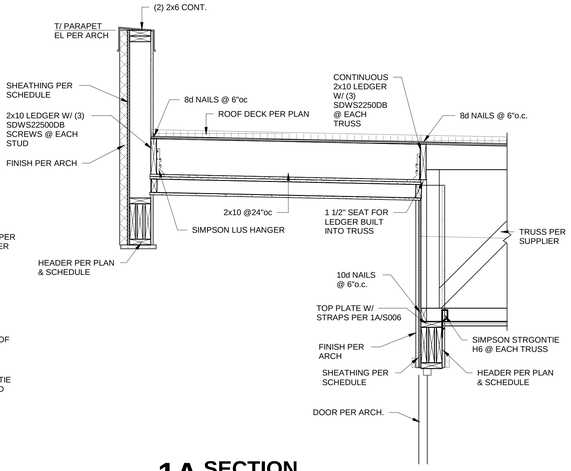

1A SECTION
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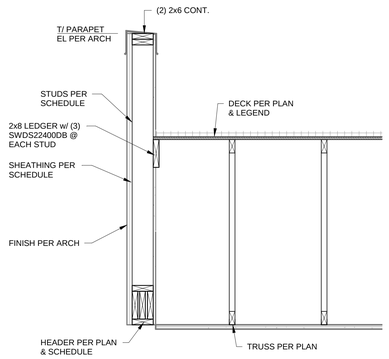

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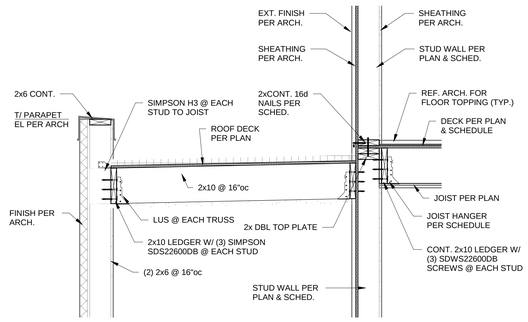
3/4" = 1'-0"



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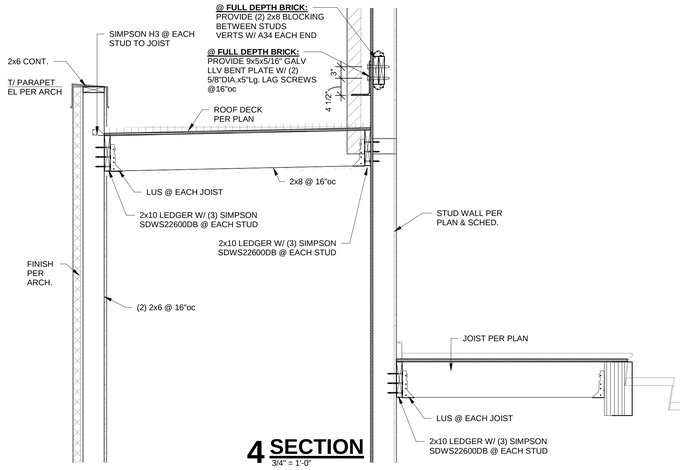
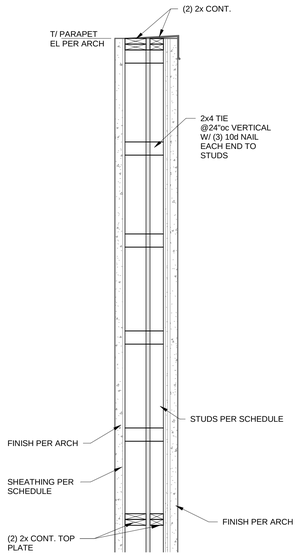
3/4" = 1'-0"



4 SECTION
3/4" = 1'-0"

5 SECTION
3/4" = 1'-0"



6 SECTION
3/4" = 1'-0"

ARCHITECT
STRUCTURAL ENGINEER
CIVIL ENGINEER
GENERAL CONTRACTOR
MECHANICAL ENGINEER
PLUMBING ENGINEER
ELECTRICAL ENGINEER

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REVISIONS 4

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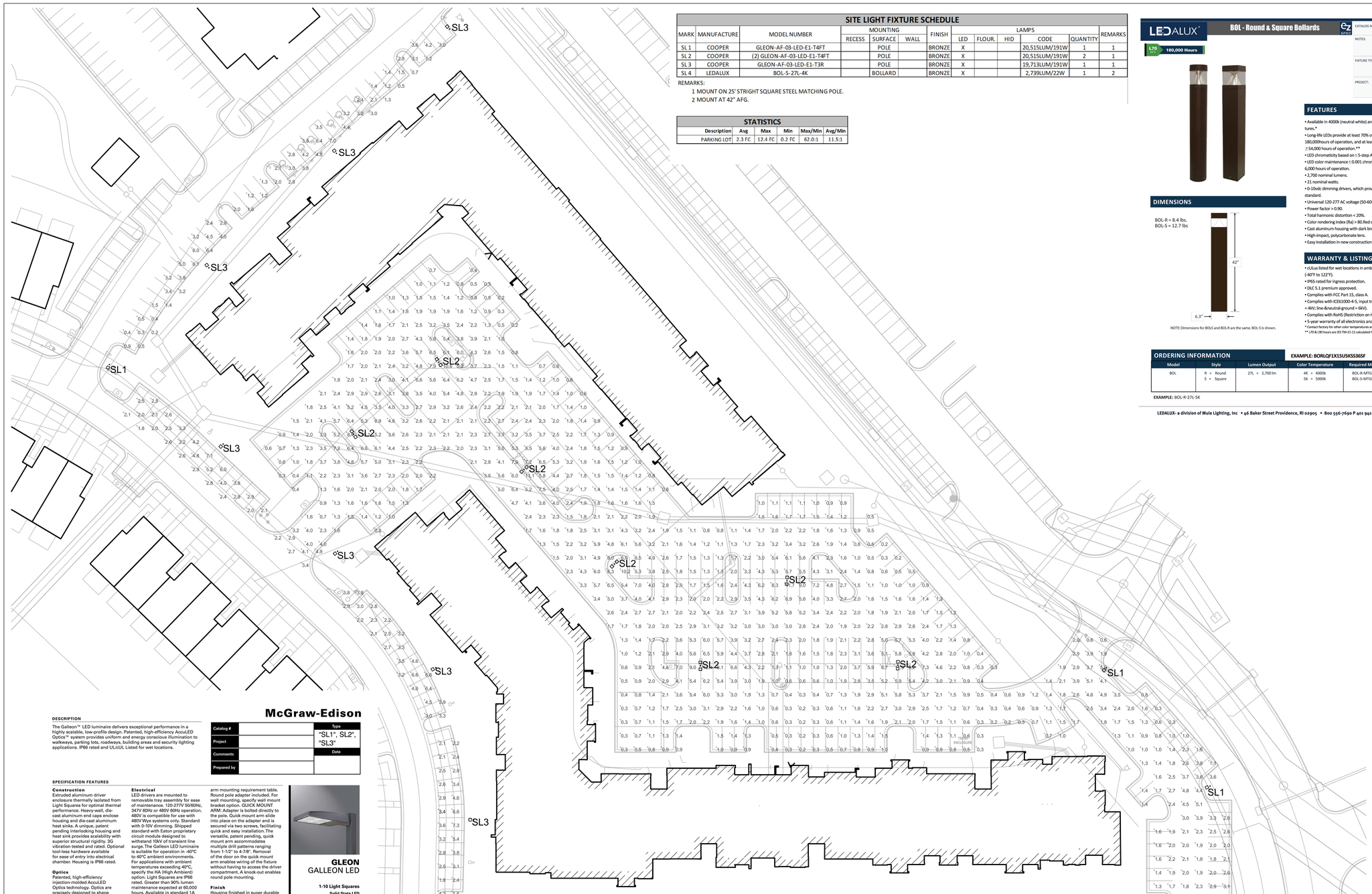
S342

WOOD ROOF FRAMING SECTIONS



SITE LIGHT FIXTURE SCHEDULE

MARK	MANUFACTURE	MODEL NUMBER	MOUNTING			FINISH	LAMPS					REMARKS
			RECESS	SURFACE	WALL		LED	FLOUR.	HID	CODE	QUANTITY	
SL 1	COOPER	GLEON-AF-03-LED-E1-T4FT		POLE		BRONZE	X			20,515LUM/191W	1	1
SL 2	COOPER	(2) GLEON-AF-03-LED-E1-T4FT		POLE		BRONZE	X			20,515LUM/191W	2	1
SL 3	COOPER	GLEON-AF-03-LED-E1-T3R		POLE		BRONZE	X			19,713LUM/191W	1	1
SL 4	LEDALUX	BOL-S-27L-4K		BOLLARD		BRONZE	X			2,739LUM/22W	1	2

REMARKS:
1 MOUNT ON 25' STRIGHT SQUARE STEEL MATCHING POLE.
2 MOUNT AT 42" AFG.

STATISTICS

Description	Avg	Max	Min	Max/Min	Avg/Min
PARKING LOT	2.3 FC	12.4 FC	0.2 FC	62.0:1	11.5:1

LEDALUX

BOL - Round & Square Bollards

CATALOG NUMBER: "SL4"
NOTES:
FIXTURE TYPE:
PROJECT:

L70 180,000 Hours

FEATURES

- Available in 4000k (neutral white) and 5000 (cool white) color temperatures.*
- Long-life LEDs provide at least 70% of initial lumen output (L70) for ≥ 180,000hours of operation, and at least 90% of initial lumen output (L90) for ≥ 54,000 hours of operation.**
- LED chromaticity based on ≤ 5-step ANSI quadrangles.
- LED color maintenance ≤ 0.001 chromaticity shift (Δu'v') over the initial 6,000 hours of operation.
- 2,700 nominal lumens.
- 21 nominal watts.
- 0-10vdc dimming drivers, which provide 10% continuous dimming are standard.
- Universal 120-277 AC voltage (50-60Hz) is standard.
- Power factor > 0.90.
- Total harmonic distortion < 20%.
- Color rendering index (Ra) > 80.Red color rendering of at least 15.
- Cast aluminum housing with dark bronze, powder coat finish.
- High-impact, polycarbonate lens.
- Easy installation in new construction or retrofit applications.

DIMENSIONS

BOL-R = 8.4 lbs.
BOL-S = 12.7 lbs

NOTE: Dimensions for BOLS and BOL-R are the same. BOL-S is shown.

WARRANTY & LISTINGS

- cULus listed for wet locations in ambient temperatures from -40°C to 50°C (-40°F to 122°F).
- IP65 rated for ingress protection.
- DLC 5.1 premium approved.
- Complies with FCC Part 15, class A.
- Complies with ICE61000-4-5, input transient surge protection (line-neutral = 4kV; line-&neutral-ground = 6kV).
- Complies with RoHS (Restriction on Hazardous Substances) requirements.
- 5-year warranty of all electronics and housing.

* Contact factory for other color temperatures and lumen packages.
** L70 & L90 hours are IES TM-21-11 calculated hours.

ORDERING INFORMATION

EXAMPLE: BORLQF1X15USKSS365F

Model	Style	Lumen Output	Color Temperature	Required Mounting Means Order Separately
BOL	R = Round S = Square	27L = 2,700 lm	4K = 4000k 5K = 5000k	BOL-R-MTGKIT = Anchor bolt for round bollard BOL-S-MTGKIT = Anchor bolt for square bollard

EXAMPLE: BOL-R-27L-5K

LEDALUX- a division of Mule Lighting, Inc • 46 Baker Street Providence, RI 02905 • 800 556-7690 P 401 941-2929 F • www.LEDALUX.com

McGraw-Edison

DESCRIPTION

The Galleon™ LED luminaire delivers exceptional performance in a highly scalable, low-profile design. Patented, high-efficiency AccuLED Optics™ system provides uniform and energy conscious illumination to walkways, parking lots, roadways, building areas and security lighting applications. IP66 rated and UL/cUL Listed for wet locations.

Catalog #	
Project	
Comments	
Prepared by	
Type	"SL1", SL2", "SL3"
Date	

SPECIFICATION FEATURES

Construction
Extruded aluminum driver enclosure thermally isolated from Light Squares for optimal thermal performance. Heavy-wall, die-cast aluminum end caps enclose housing and die-cast aluminum heat sinks. A unique, patent pending interlocking housing and heat sink provides scalability with superior structural rigidity. 3G vibration tested and rated. Optional tool-less hardware available for ease of entry into electrical chamber. Housing is IP66 rated.

Optics
Patented, high-efficiency injection-molded AccuLED Optics technology. Optics are precisely designed to shape the distribution maximizing efficiency and application spacing. AccuLED Optics create consistent distributions with the scalability to meet customized application requirements. Offered standard in 4000K (+/- 275K) CCT 70 CRI. Optional 3000K, 5000K and 6000K CCT.

Electrical
LED drivers are mounted to removable tray assembly for ease of maintenance. 120-277V 50/60Hz, 347V 60Hz or 480V 60Hz operation. 480V is compatible for use with 480V Wye systems only. Standard with 0-10V dimming. Shipped standard with Eaton proprietary circuit module designed to withstand 10kV of transient line surge. The Galleon LED luminaire is suitable for operation in -40°C to 40°C ambient environments. For applications with ambient temperatures exceeding 40°C, specify the HA (High Ambient) option. Light Squares are IP66 rated. Greater than 90% lumen maintenance expected at 60,000 hours. Available in standard 1A drive current and optional 600mA, 800mA and 1200mA drive currents (nominal).

Mounting
STANDARD ARM MOUNT:
Extruded aluminum arm includes internal bolt guides allowing for easy positioning of fixture during mounting. When mounting two or more luminaires at 90° and 120° apart, the EA extended arm may be required. Refer to the arm mounting requirement table. Round pole adapter included. For wall mounting, specify wall mount bracket option. QUICK MOUNT ARM: Adapter is bolted directly to the pole. Quick mount arm slide into place on the adapter and is secured via two screws, facilitating quick and easy installation. The versatile, patent pending, quick mount arm accommodates multiple drill patterns ranging from 1-1/2" to 4-7/8". Removal of the door on the quick mount arm enables wiring of the fixture without having to access the driver compartment. A knock-out enables round pole mounting.

Finish
Housing finished in super durable TGIC polyester powder coat paint, 2.5 mil nominal thickness for superior protection against fade and wear. Heat sink is powder coated black. Standard housing colors include black, bronze, grey, white, dark platinum and graphite metallic. RAL and custom color matches available.

Warranty
Five-year warranty.

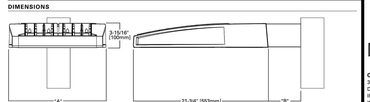
GLEON
GALLEON LED

1-10 Light Squares
Solid State LED

AREA/SITE LUMINAIRE

LumenGuide Technology CLICK HERE

DIMENSIONS



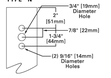
DIMENSION DATA

Number of Light Squares	"A" Width	"B" Standard Arm Length	"B" Optional Arm Length †	Weight with Arm (lbs.)	EPA with Arm † (Sq. Ft.)
1-4	15-1/2" (394mm)	7" (178mm)	10" (254mm)	33 (15.0 kgs.)	0.96
5-6	21-5/8" (549mm)	7" (178mm)	10" (254mm)	44 (20.0 kgs.)	1.00
7-8	27-5/8" (702mm)	7" (178mm)	13" (330mm)	54 (24.5 kgs.)	1.07
9-10	33-3/4" (857mm)	7" (178mm)	16" (406mm)	83 (28.6 kgs.)	1.12

NOTES: 1. Optional arm length to be used when mounting two fixtures at 90° on a single pole. **2.** EPA calculated with optional arm length.

DRILLING PATTERN

TYPE "N"



*www.designlights.org

DLC DLC PREMIUM

CERTIFICATION DATA
3G Vibration Rated
DesignLights Consortium® Qualified*
IP66 Rated
ISO 9001
LM79 / LM80 Compliant
UL/cUL Wet Location Listed

ENERGY DATA
Electronic LED Driver
>0.9 Power Factor
<20% Total Harmonic Distortion
120V-277V 50/60Hz
347V, 480V 60Hz
-40°C Min. Temperature
40°C Max. Temperature
50°C Max. Temperature (HA Option)

TD500020EN
June 3, 2019 11:11 AM

1 **SITE PLAN - PHOTOMETRIC**
1"=30'-0" PHOTOMETRIC

NORTH

STATE OF MISSOURI
RICHARD R. BEARDMORE
NUMBER A-2004018
PROFESSIONAL ENGINEER
7-15-21

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CIVIL ENGINEER
GENERAL CONTRACTOR
MECHANICAL ENGINEER
PLUMBING ENGINEER
ELECTRICAL ENGINEER

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CIVIL
GENERAL CONTRACTOR
ENGINEER
PLUMBING
ELECTRICAL

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REVISIONS 1

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TR¡ PROJECT NO. 20-001

SHEET NO.

E001

SITE PLAN - PHOTOMETRIC

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Email: lsapa@lsapa.com

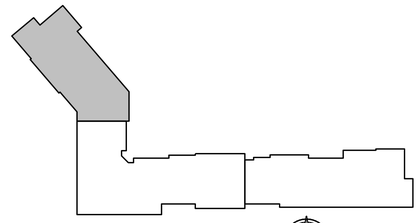
LSA PROJECT NO. 2104008

NOTES:
1. ALL STAIR LIGHTS ARE TYPE "A2" UNLESS NOTED OTHERWISE.
2. ALL ⊕ ARE TYPE "X1" UNLESS NOTED OTHERWISE.
3. ALL ⊕ ARE TYPE "X2" UNLESS NOTED OTHERWISE.
4. ALL ⊕ ARE TYPE "X5" UNLESS NOTED OTHERWISE.
5. CONNECT FIRE/SMOKE DAMPERS SHOWN ON MECHANICAL PLANS TO 120V. AND FIRE ALARM RELAY TO SHUT UPON SMOKE DETECTION IN CORRIDORS.
6. SEE UNIT PLANS FOR FURTHER WORK.
7. LIGHTS AT APT. ENTRY DOORS CONNECT TO APT. HOT CIRCUIT.
8. ALL AHU'S HAVE AUTO DAMPER/THERMOSTAT SYSTEM TO WIRE. SEE HVAC PLANS.
9. EXIT SIGNS SHALL BE READILY VISIBLE TO OCCUPANTS.
10. SEE ARCH. REFLECTED CEILING PLANS FOR DIMENSIONS.
11. "DP-1" DENOTES DISTRIBUTION PANEL.

LEGEND:
(1) ROUTE CIRCUIT THRU PHOTOCELL.

1 **BLDG 1-A, PARTIAL FIRST FLOOR PLAN**
1/8"=1'-0" ELECTRICAL

NORTH



KEY PLAN
NO SCALE

NORTH

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ELECTRICAL ENGINEER

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REVISIONS (1)

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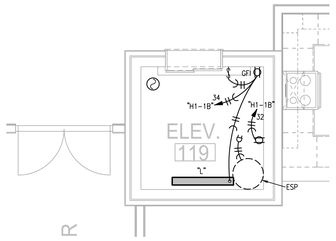
TR¡ PROJECT NO. 20-001

SHEET NO.

E101

BLDG 1-A, PARTIAL FIRST FLOOR PLAN - ELECTRICAL



2 BLDG 1-B, PARTIAL FIRST FLOOR PLAN
1/8"=1'-0"

ELECTRICAL

NORTH

NOTES:
1. ALL STAIR LIGHTS ARE TYPE "A2" UNLESS NOTED OTHERWISE.
2. ALL ● ARE TYPE "A1" UNLESS NOTED OTHERWISE.
3. ALL ● ARE TYPE "A2" UNLESS NOTED OTHERWISE.
4. ALL ● ARE TYPE "A5" UNLESS NOTED OTHERWISE.
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7. LIGHTS AT APT. ENTRY DOORS CONNECT TO APT. HOT CIRCUIT.
8. ALL AHU'S HAVE AUTO DAMPER/THERMOSTAT SYSTEM TO WIRE. SEE HVAC PLANS.
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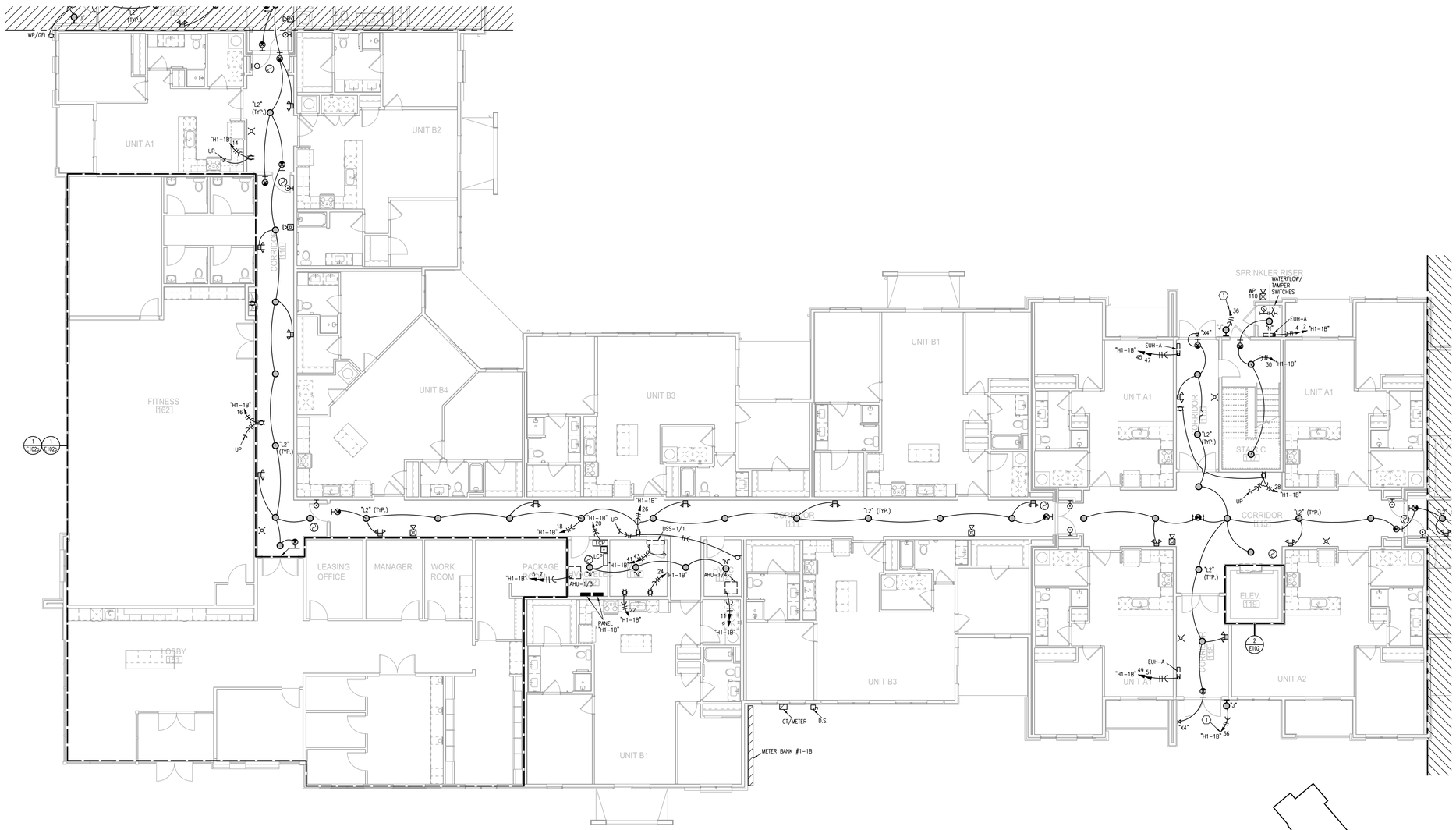
11. "DP-1" DENOTES DISTRIBUTION PANEL.

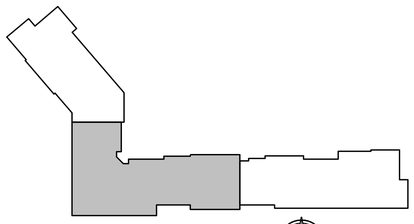
LEGEND:
(1) ROUTE CIRCUIT THRU PHOTOCELL.



1 BLDG 1-B, PARTIAL FIRST FLOOR PLAN
1/8"=1'-0"

ELECTRICAL

NORTH



KEY PLAN
NO SCALE

NORTH

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REVISIONS 2

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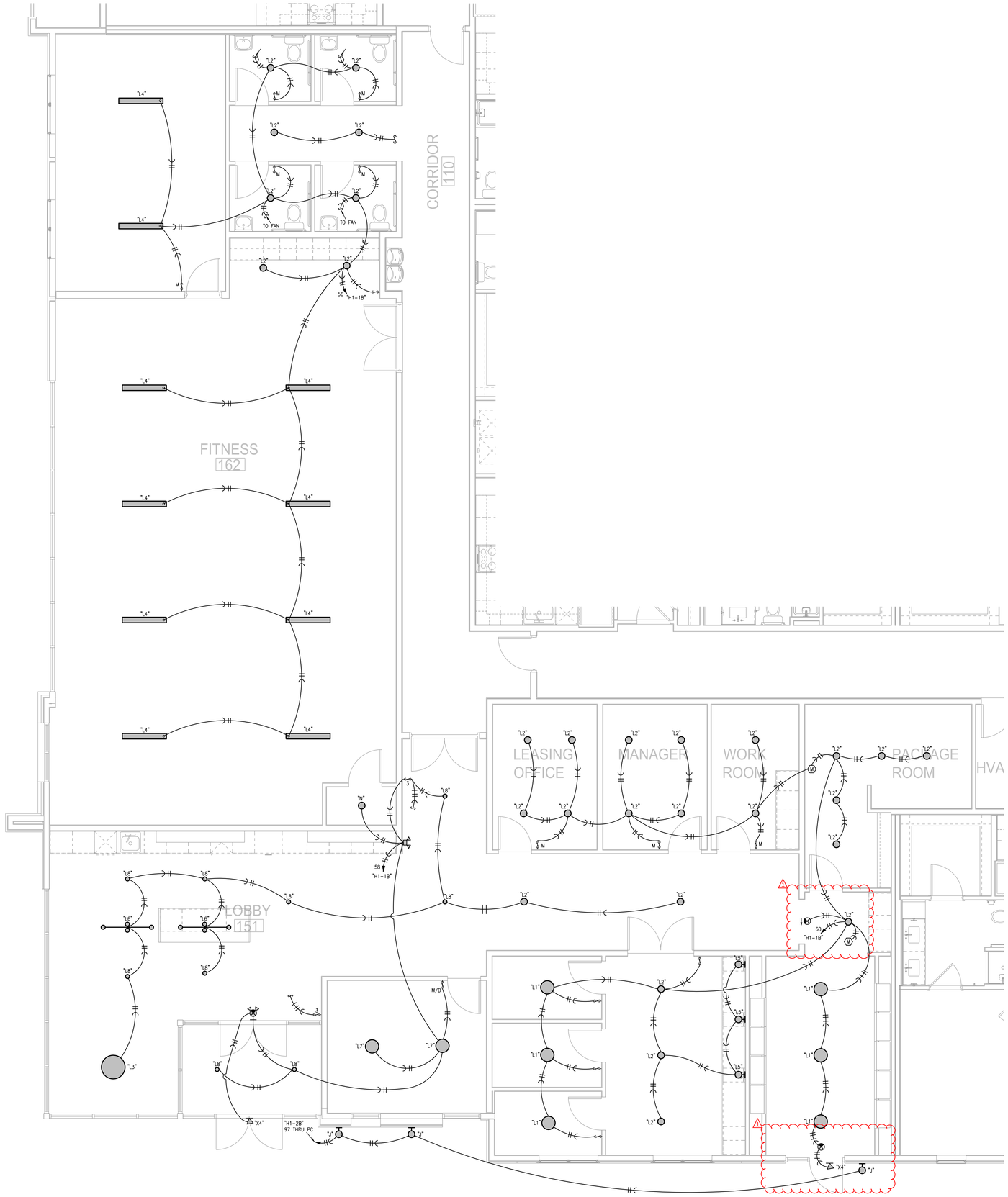
SHEET NO.

E102

BLDG 1-B, PARTIAL FIRST FLOOR PLAN - ELECTRICAL



NOTES:
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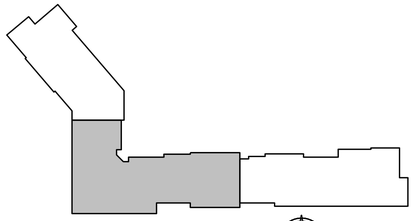
11. "DP-1" DENOTES DISTRIBUTION PANEL.

LEGEND:
(1) ROUTE CIRCUIT THRU PHOTOCELL.

1 **BLDG 1-B, PARTIAL FIRST FLOOR PLAN** LIGHTING
1/4" = 1'-0"

NORTH



KEY PLAN
NO SCALE

NORTH

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ELECTRICAL ENGINEER

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REVISIONS 2

2 PERMIT REVIEW COMMENTS 08-17-21

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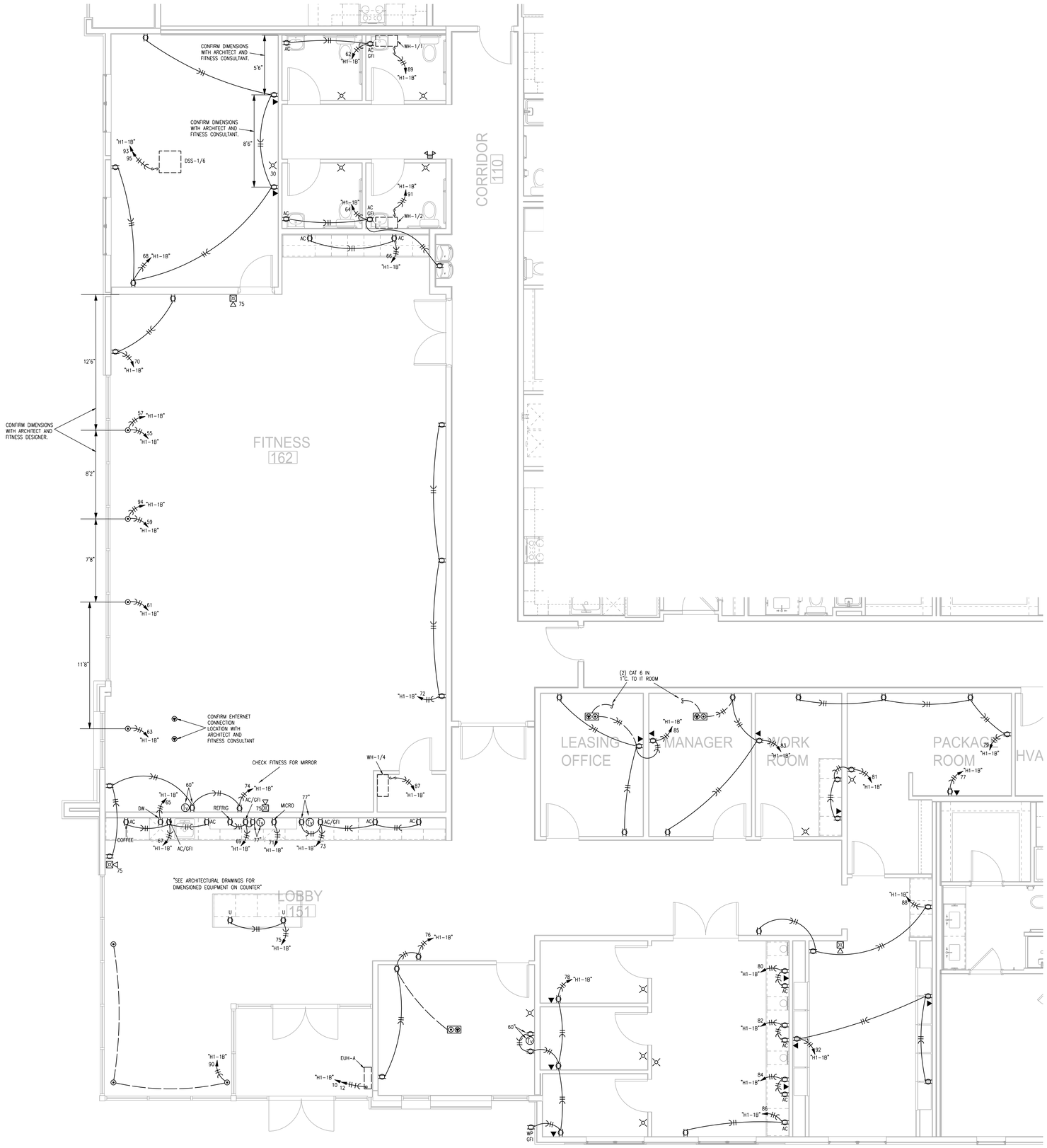
TR.i PROJECT NO. 20-001

SHEET NO.

E102a

BLDG 1-B, PARTIAL FIRST FLOOR PLAN - ELECTRICAL

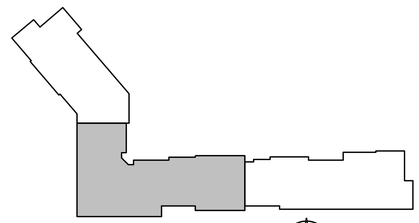


BLDG 1-B, PARTIAL FIRST FLOOR PLAN

1/4"=1'-0"

POWER

NORTH



KEY PLAN

NO SCALE

NORTH

NOTES:

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2. ALL ⓔ ARE TYPE "X1" UNLESS NOTED OTHERWISE.
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11. "DP-1" DENOTES DISTRIBUTION PANEL.

LEGEND:

(1) ROUTE CIRCUIT THRU PHOTOCELL.

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REVISIONS 4

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SHEET NO.

E102b

BLDG 1-B, PARTIAL FIRST FLOOR PLAN - ELECTRICAL

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Email: lsapa@lsapa.com
LSA PROJECT NO. 2104008

NOTES:
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2. ALL ● ARE TYPE "X1" UNLESS NOTED OTHERWISE.
3. ALL ◉ ARE TYPE "X2" UNLESS NOTED OTHERWISE.
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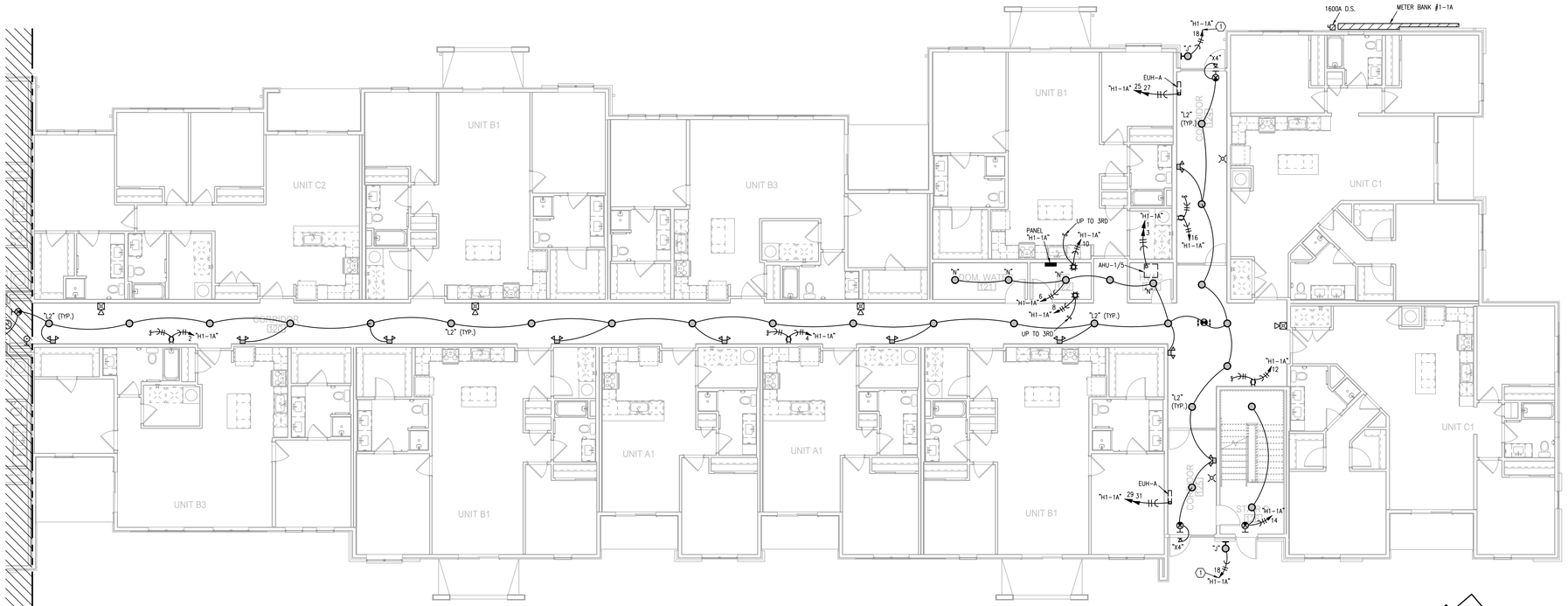
11. "DP-1" DENOTES DISTRIBUTION PANEL.

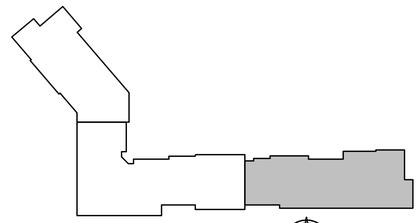
LEGEND:
(1) ROUTE CIRCUIT THRU PHOTOCELL.



1 BLDG 1-C, PARTIAL FIRST FLOOR PLAN

1/8"=1'-0" ELECTRICAL

NORTH



KEY PLAN
NO SCALE

NORTH

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REVISIONS 1

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SHEET NO.

E103

BLDG 1-C, PARTIAL FIRST FLOOR PLAN - ELECTRICAL

NOTES:
1. ALL STAIR LIGHTS ARE TYPE "A2" UNLESS NOTED OTHERWISE.
2. ALL ⓥ ARE TYPE "X1" UNLESS NOTED OTHERWISE.
3. ALL ⓦ ARE TYPE "X2" UNLESS NOTED OTHERWISE.
4. ALL ⓨ ARE TYPE "X5" UNLESS NOTED OTHERWISE.
5. CONNECT FIRE/SMOKE DAMPERS SHOWN ON MECHANICAL PLANS TO 120V. AND FIRE ALARM RELAY TO SHUT UPON SMOKE DETECTION IN CORRIDORS.
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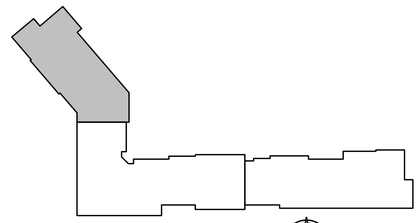
9. EXIT SIGNS SHALL BE READILY VISIBLE TO OCCUPANTS.
10. SEE ARCH. REFLECTED CEILING PLANS FOR DIMENSIONS.
11. "DP-1" DENOTES DISTRIBUTION PANEL.

LEGEND:
(1) ROUTE CIRCUIT THRU PHOTOCELL.

1 **BLDG 1-A, PARTIAL SECOND FLOOR PLAN**
1/8"=1'-0" ELECTRICAL

NORTH



KEY PLAN
NO SCALE

NORTH

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SHEET NO.

E104

BLDG 1-A, PARTIAL SECOND FLOOR PLAN - ELECTRICAL

NOTES:
1. ALL STAIR LIGHTS ARE TYPE "A2" UNLESS NOTED OTHERWISE.
2. ALL ● ARE TYPE "X1" UNLESS NOTED OTHERWISE.
3. ALL ● ARE TYPE "X2" UNLESS NOTED OTHERWISE.
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11. "DP-1" DENOTES DISTRIBUTION PANEL.

LEGEND:
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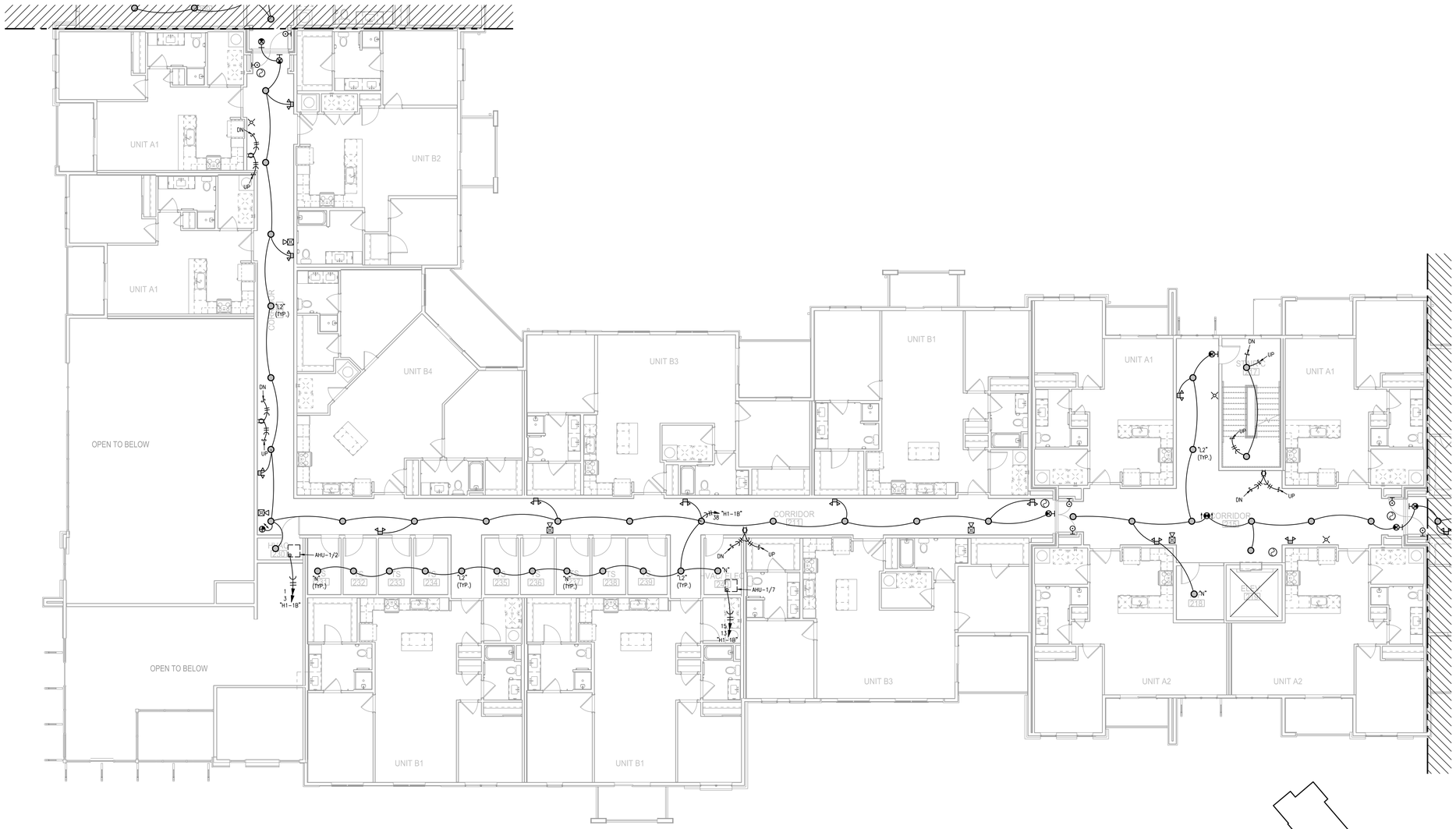
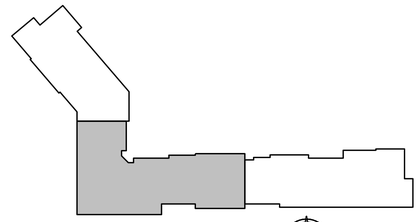


1 **BLDG 1-B, PARTIAL SECOND FLOOR PLAN** — ELECTRICAL
1/8"=1'-0"

NORTH



KEY PLAN
NO SCALE

NORTH

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SHEET NO.

E105

BLDG 1-B, PARTIAL SECOND FLOOR PLAN - ELECTRICAL

NOTES:
1. ALL STAIR LIGHTS ARE TYPE "A2" UNLESS NOTED OTHERWISE.
2. ALL ● ARE TYPE "X1" UNLESS NOTED OTHERWISE.
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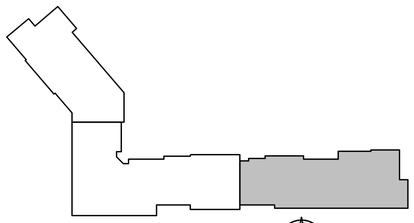
LEGEND:
(1) ROUTE CIRCUIT THRU PHOTOCELL.

1 BLDG 1-C, PARTIAL SECOND FLOOR PLAN

1/8"=1'-0" ELECTRICAL

NORTH



KEY PLAN
NO SCALE

NORTH

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NUMBER 2004018
PROFESSIONAL ENGINEER
7-15-21

ARCHITECT
STRUCTURAL ENGINEER
CIVIL ENGINEER
GENERAL CONTRACTOR
MECHANICAL ENGINEER
PLUMBING ENGINEER
ELECTRICAL ENGINEER

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ENGINEER
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REVISIONS (1)

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TR¡ PROJECT NO. 20-001

SHEET NO.

E106

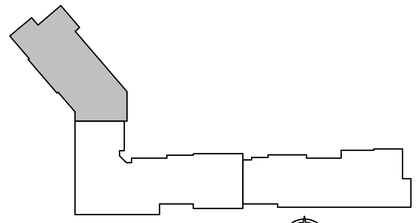
BLDG 1-C, PARTIAL SECOND FLOOR PLAN - ELECTRICAL

NOTES:
1. ALL STAIR LIGHTS ARE TYPE "A2" UNLESS NOTED OTHERWISE.
2. ALL ● ARE TYPE "X1" UNLESS NOTED OTHERWISE.
3. ALL ● ARE TYPE "X2" UNLESS NOTED OTHERWISE.
4. ALL ● ARE TYPE "X5" UNLESS NOTED OTHERWISE.
5. CONNECT FIRE/SMOKE DAMPERS SHOWN ON MECHANICAL PLANS TO 120V. AND FIRE ALARM RELAY TO SHUT UPON SMOKE DETECTION IN CORRIDORS.
6. SEE UNIT PLANS FOR FURTHER WORK.
7. LIGHTS AT APT. ENTRY DOORS CONNECT TO APT. HOT CIRCUIT.
8. ALL AHU'S HAVE AUTO DAMPER/THERMOSTAT SYSTEM TO WIRE. SEE HVAC PLANS.
9. EXIT SIGNS SHALL BE READILY VISIBLE TO OCCUPANTS.
10. SEE ARCH. REFLECTED CEILING PLANS FOR DIMENSIONS.
11. "DP-1" DENOTES DISTRIBUTION PANEL.

LEGEND:
(1) ROUTE CIRCUIT THRU PHOTOCELL.

1 **BLDG 1-A, PARTIAL THIRD FLOOR PLAN**
1/8"=1'-0" ELECTRICAL

NORTH



KEY PLAN
NO SCALE

NORTH

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ELECTRICAL

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SHEET NO.

E107

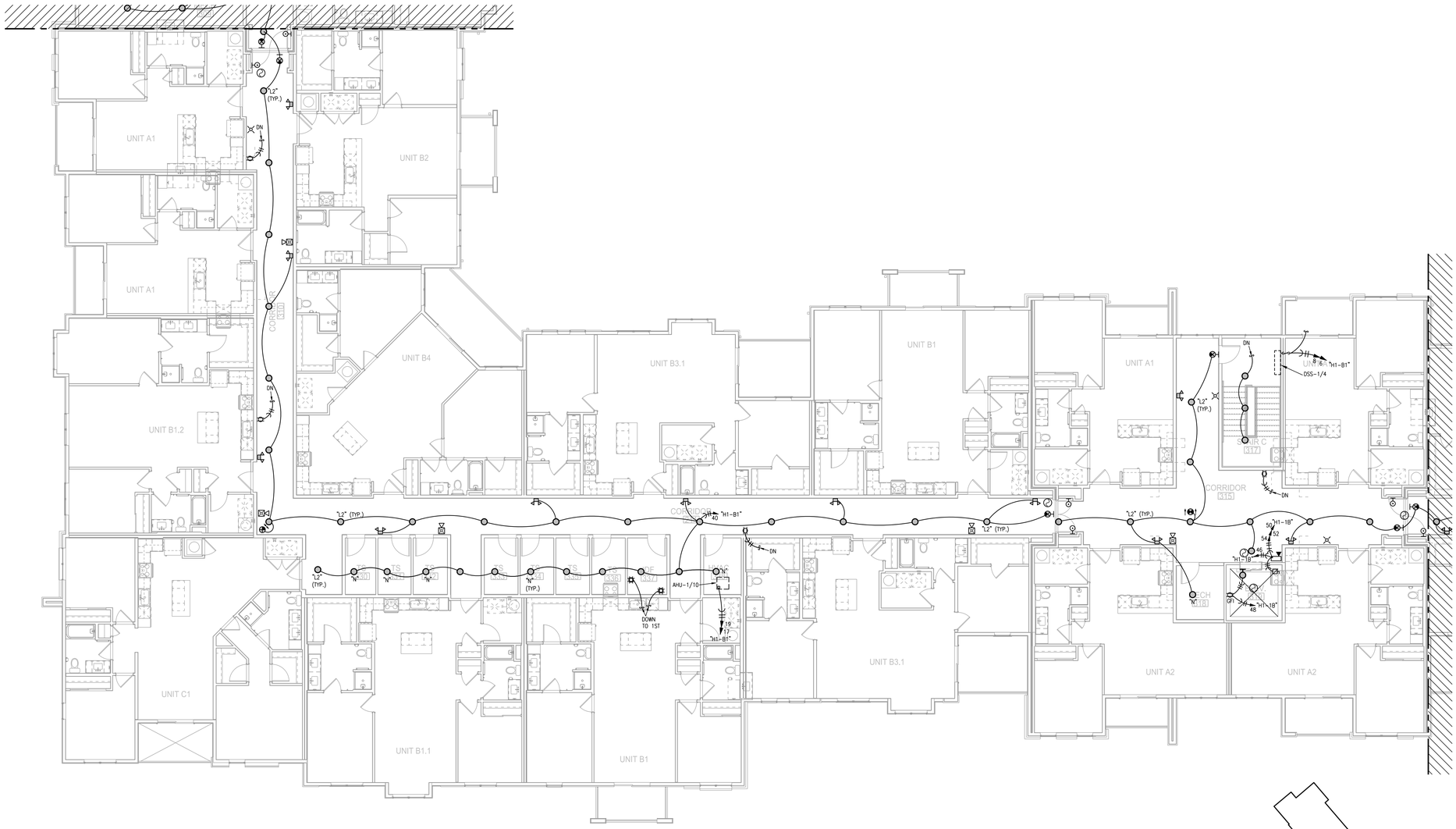
BLDG 1-A, PARTIAL THIRD FLOOR PLAN - ELECTRICAL

NOTES:

1. ALL STAIR LIGHTS ARE TYPE "A2" UNLESS NOTED OTHERWISE.
2. ALL ⊕ ARE TYPE "X1" UNLESS NOTED OTHERWISE.
3. ALL ⊗ ARE TYPE "X2" UNLESS NOTED OTHERWISE.
4. ALL ⊕ARE TYPE "X5" UNLESS NOTED OTHERWISE.
5. CONNECT FIRE/SMOKE DAMPERS SHOWN ON MECHANICAL PLANS TO 120V. AND FIRE ALARM RELAY TO SHUT UPON SMOKE DETECTION IN CORRIDORS.
6. SEE UNIT PLANS FOR FURTHER WORK.
7. LIGHTS AT APT. ENTRY DOORS CONNECT TO APT. HOT CIRCUIT.
8. ALL AHU'S HAVE AUTO DAMPER/THERMOSTAT SYSTEM TO WIRE. SEE HVAC PLANS.
9. EXIT SIGNS SHALL BE READILY VISIBLE TO OCCUPANTS.
10. SEE ARCH. REFLECTED CEILING PLANS FOR DIMENSIONS.
11. "DP-1" DENOTES DISTRIBUTION PANEL.

LEGEND:

(1) ROUTE CIRCUIT THRU PHOTOCELL.



NOTES:
1. ALL STAIR LIGHTS ARE TYPE "A2" UNLESS NOTED OTHERWISE.
2. ALL ● ARE TYPE "X1" UNLESS NOTED OTHERWISE.
3. ALL ● ARE TYPE "X2" UNLESS NOTED OTHERWISE.
4. ALL ● ARE TYPE "X5" UNLESS NOTED OTHERWISE.
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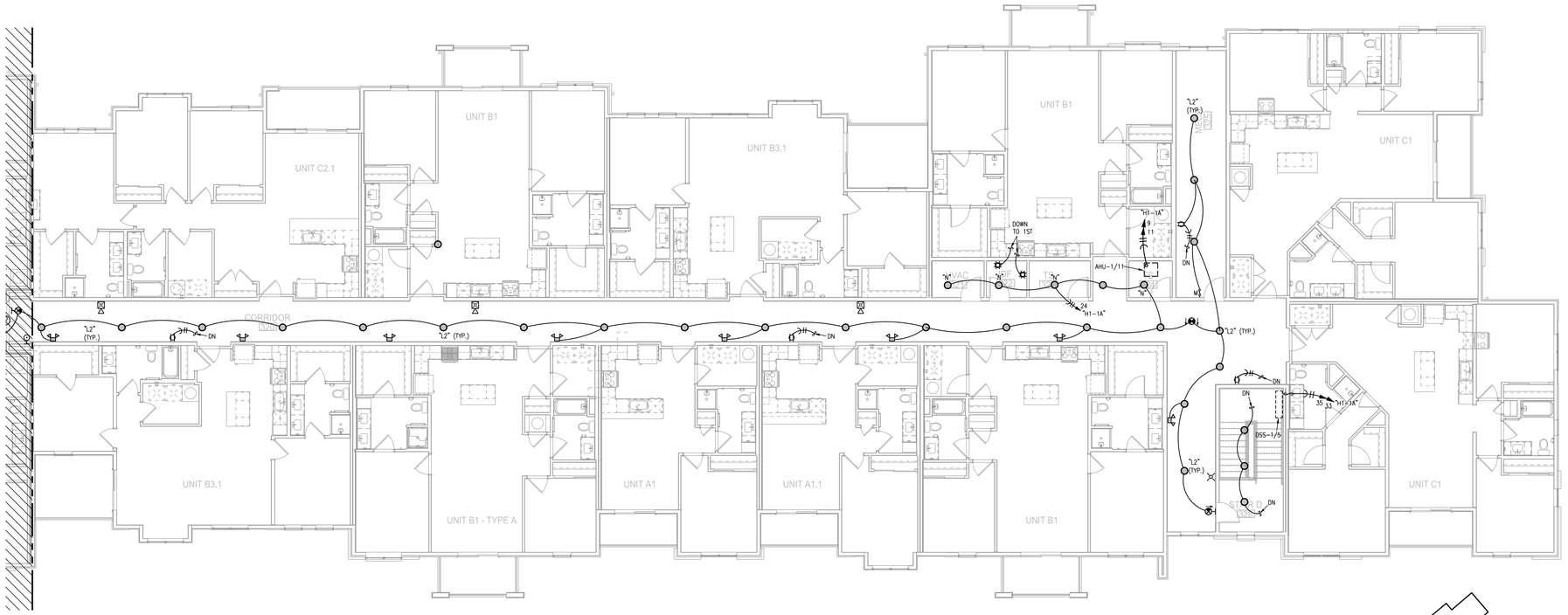
11. "DP-1" DENOTES DISTRIBUTION PANEL.

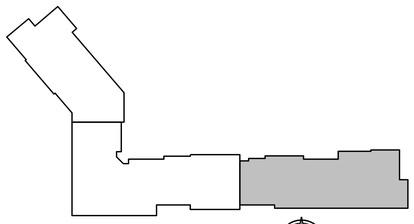
LEGEND:
(1) ROUTE CIRCUIT THRU PHOTOCELL.



1 BLDG 1-C, PARTIAL THIRD FLOOR PLAN

1/8"=1'-0" ELECTRICAL

NORTH



KEY PLAN

NO SCALE

NORTH

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TR;i PROJECT NO. 20-001

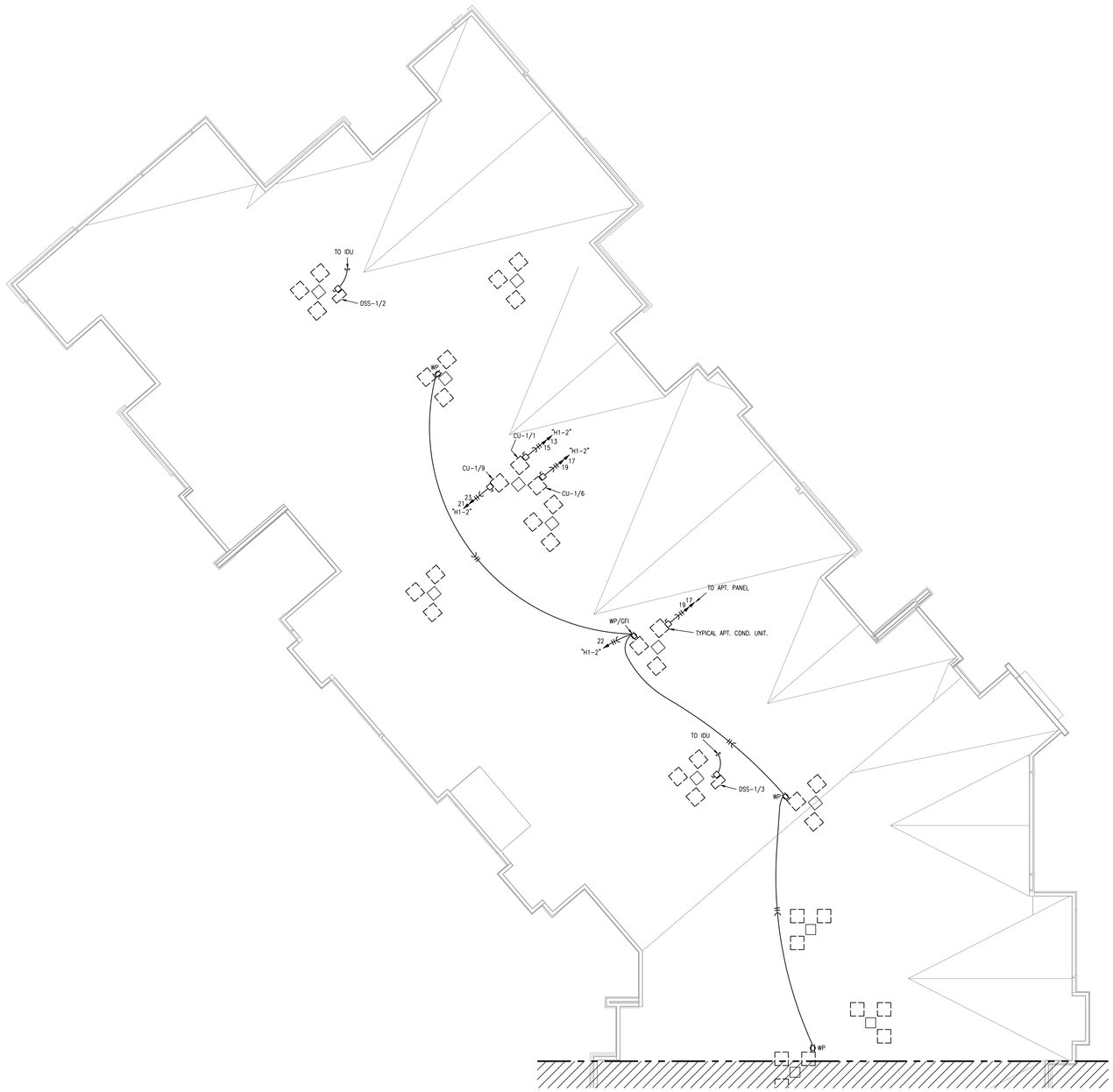
SHEET NO.

E109

BLDG 1-C, PARTIAL THIRD FLOOR PLAN - ELECTRICAL

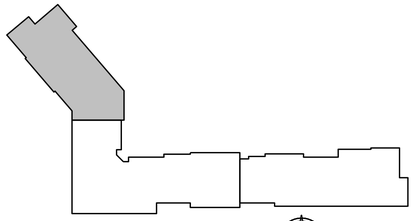
NOTES:
1. ALL STAIR LIGHTS ARE TYPE "A2" UNLESS NOTED OTHERWISE.
2. ALL ● ARE TYPE "A1" UNLESS NOTED OTHERWISE.
3. ALL ● ARE TYPE "A2" UNLESS NOTED OTHERWISE.
4. ALL ● ARE TYPE "A5" UNLESS NOTED OTHERWISE.
5. CONNECT FIRE/SMOKE DAMPERS SHOWN ON MECHANICAL PLANS TO 120V. AND FIRE ALARM RELAY TO SHUT UPON SMOKE DETECTION IN CORRIDORS.
6. SEE UNIT PLANS FOR FURTHER WORK.
7. LIGHTS AT APT. ENTRY DOORS CONNECT TO APT. HOT CIRCUIT.
8. ALL AHU'S HAVE AUTO DAMPER/THERMOSTAT SYSTEM TO WIRE. SEE HVAC PLANS.
9. EXIT SIGNS SHALL BE READILY VISIBLE TO OCCUPANTS.
10. SEE ARCH. REFLECTED CEILING PLANS FOR DIMENSIONS.
11. "DP-1" DENOTES DISTRIBUTION PANEL.

LEGEND:
(1) ROUTE CIRCUIT THRU PHOTOCELL.



1 **BLDG 1-A, PARTIAL ROOF PLAN** — ELECTRICAL
1/8"=1'-0"

NORTH



KEY PLAN
NO SCALE

NORTH

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TR¡I PROJECT NO. 20-001

SHEET NO.

E110

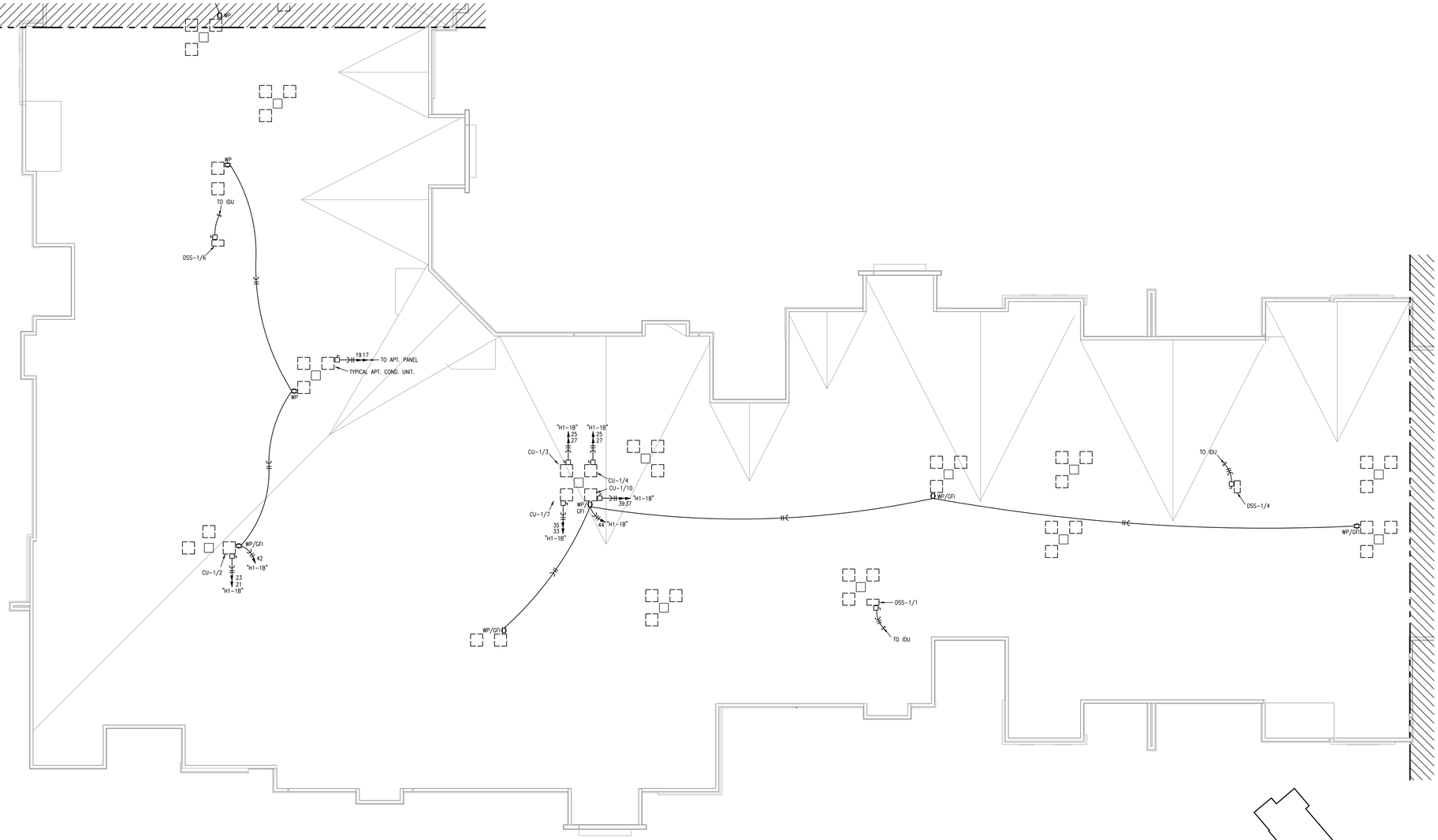
BLDG 1-A, PARTIAL ROOF PLAN - ELECTRICAL

NOTES:

1. ALL STAIR LIGHTS ARE TYPE "A2" UNLESS NOTED OTHERWISE.
2. ALL ● ARE TYPE "X1" UNLESS NOTED OTHERWISE.
3. ALL ● ARE TYPE "X2" UNLESS NOTED OTHERWISE.
4. ALL ● ARE TYPE "X5" UNLESS NOTED OTHERWISE.
5. CONNECT FIRE/SMOKE DAMPERS SHOWN ON MECHANICAL PLANS TO 120V. AND FIRE ALARM RELAY TO SHUT UPON SMOKE DETECTION IN CORRIDORS.
6. SEE UNIT PLANS FOR FURTHER WORK.
7. LIGHTS AT APT. ENTRY DOORS CONNECT TO APT. HOT CIRCUIT.
8. ALL AHU'S HAVE AUTO DAMPER/THERMOSTAT SYSTEM TO WIRE. SEE HVAC PLANS.
9. EXIT SIGNS SHALL BE READILY VISIBLE TO OCCUPANTS.
10. SEE ARCH. REFLECTED CEILING PLANS FOR DIMENSIONS.
11. "DP-1" DENOTES DISTRIBUTION PANEL.

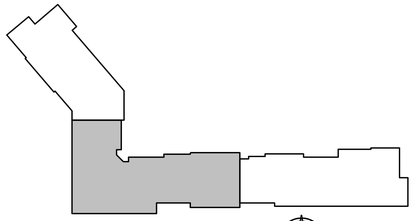
LEGEND:

(1) ROUTE CIRCUIT THRU PHOTOCELL.



1 **BLDG 1-B, PARTIAL ROOF PLAN** — ELECTRICAL
1/8"=1'-0"

NORTH



KEY PLAN
NO SCALE

NORTH

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ELECTRICAL

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REVISIONS 4

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TR¡ PROJECT NO. 20-001

SHEET NO.

E111

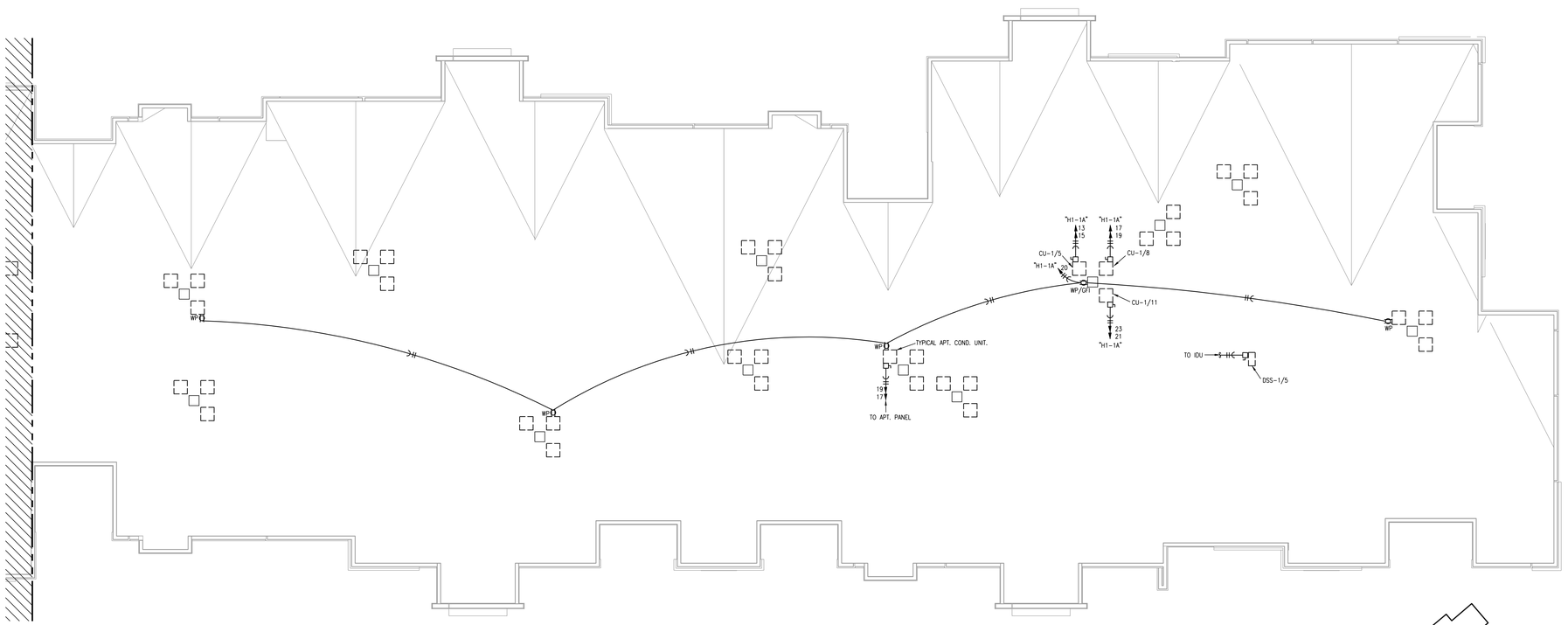
BLDG 1-B, PARTIAL ROOF PLAN - ELECTRICAL

NOTES:

1. ALL STAIR LIGHTS ARE TYPE "A2" UNLESS NOTED OTHERWISE.
2. ALL ● ARE TYPE "A1" UNLESS NOTED OTHERWISE.
3. ALL ● ARE TYPE "A2" UNLESS NOTED OTHERWISE.
4. ALL ● ARE TYPE "A5" UNLESS NOTED OTHERWISE.
5. CONNECT FIRE/SMOKE DAMPERS SHOWN ON MECHANICAL PLANS TO 120V. AND FIRE ALARM RELAY TO SHUT UPON SMOKE DETECTION IN CORRIDORS.
6. SEE UNIT PLANS FOR FURTHER WORK.
7. LIGHTS AT APT. ENTRY DOORS CONNECT TO APT. HOT CIRCUIT.
8. ALL AHU'S HAVE AUTO DAMPER/THERMOSTAT SYSTEM TO WIRE. SEE HVAC PLANS.
9. EXIT SIGNS SHALL BE READILY VISIBLE TO OCCUPANTS.
10. SEE ARCH. REFLECTED CEILING PLANS FOR DIMENSIONS.
11. "DP-1" DENOTES DISTRIBUTION PANEL.

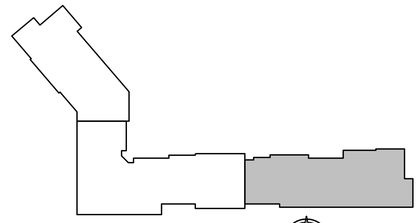
LEGEND:

(1) ROUTE CIRCUIT THRU PHOTOCELL.



1 **BLDG 1-C, PARTIAL ROOF PLAN** — ELECTRICAL
1/8"=1'-0"

NORTH



KEY PLAN
NO SCALE

NORTH

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ELECTRICAL ENGINEER

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REVISIONS 2

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TR¡ PROJECT NO. 20-001

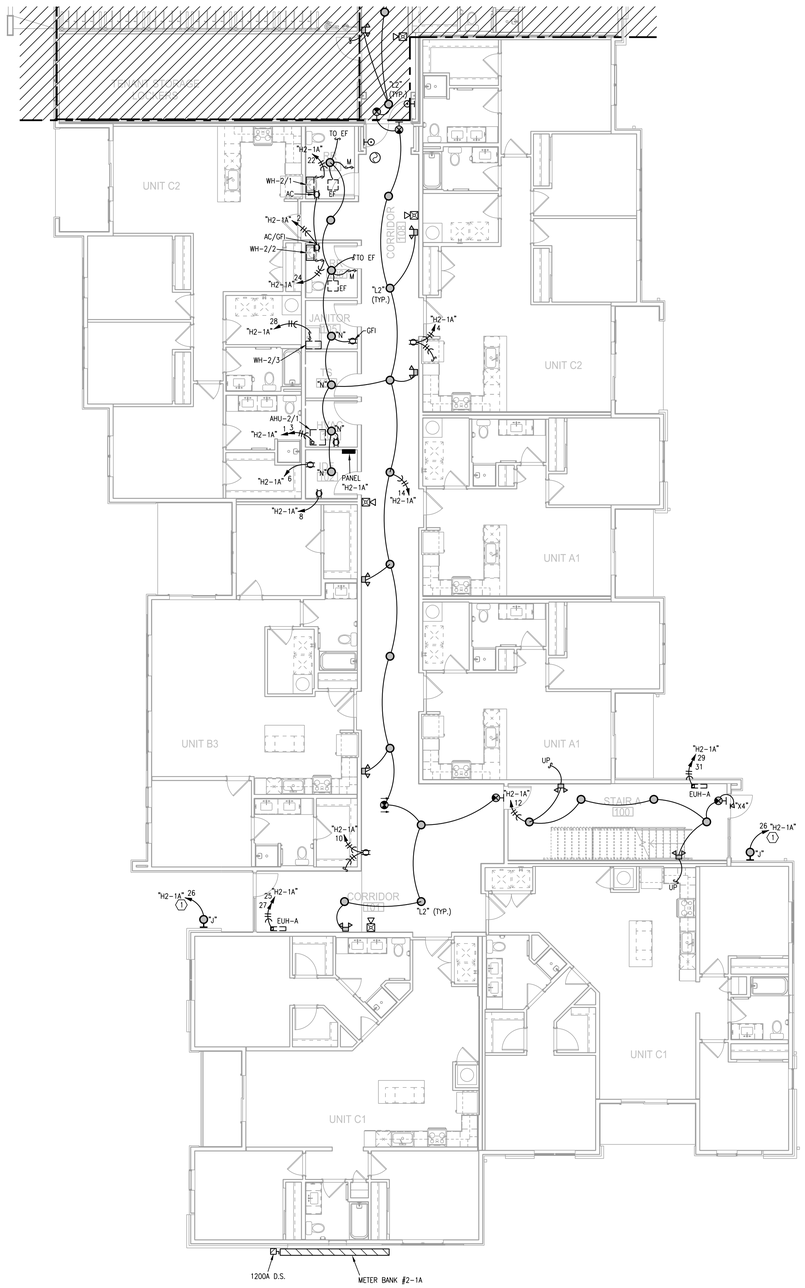
SHEET NO.

E112

BLDG 1-C, PARTIAL ROOF PLAN - ELECTRICAL

NOTES:
1. ALL ⊙ ARE TYPE "A2" UNLESS NOTED OTHERWISE.
2. ALL STAIR LIGHTS ARE TYPE "A" UNLESS NOTED OTHERWISE.
3. ALL ⊗ ARE TYPE "X1" UNLESS NOTED OTHERWISE.
4. ALL ⊗ ARE TYPE "X2" UNLESS NOTED OTHERWISE.
5. ALL ⊗ ARE TYPE "X5" UNLESS NOTED OTHERWISE.
6. ALL ▬ STAIR LIGHTS ARE TYPE "P" U.N.O.
7. CONNECT FIRE/SMOKE DAMPERS SHOWN ON MECHANICAL PLANS TO 120V. AND FIRE ALARM RELAY TO SHUT UPON SMOKE DETECTION IN CORRIDORS.
8. SEE UNIT PLANS FOR FURTHER WORK.
9. LIGHTS AT APT. ENTRY DOORS CONNECT TO APT. HOT CIRCUIT.
10. ALL AHU'S HAVE AUTO DAMPER/THERMOSTAT SYSTEM TO WIRE. SEE HVAC PLANS.
11. EXIT SIGNS SHALL BE READILY VISIBLE TO OCCUPANTS.
12. EXTERIOR LIGHTING TO BE ROUTED THRU PHOTOCELL.
13. "DP-1" DENOTES DISTRIBUTION PANEL.

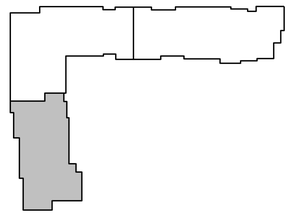
LEGEND:
(1) TO LIGHTING CONTROL PANEL.



1 BLDG 2-A, PARTIAL FIRST FLOOR PLAN - ELECTRICAL

1/8"=1'-0"

NORTH



KEY PLAN
NO SCALE

NORTH

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LSA PROJECT NO. 2104008

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MECHANICAL ENGINEER
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ELECTRICAL ENGINEER

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REVISIONS 4

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SHEET NO.

E113

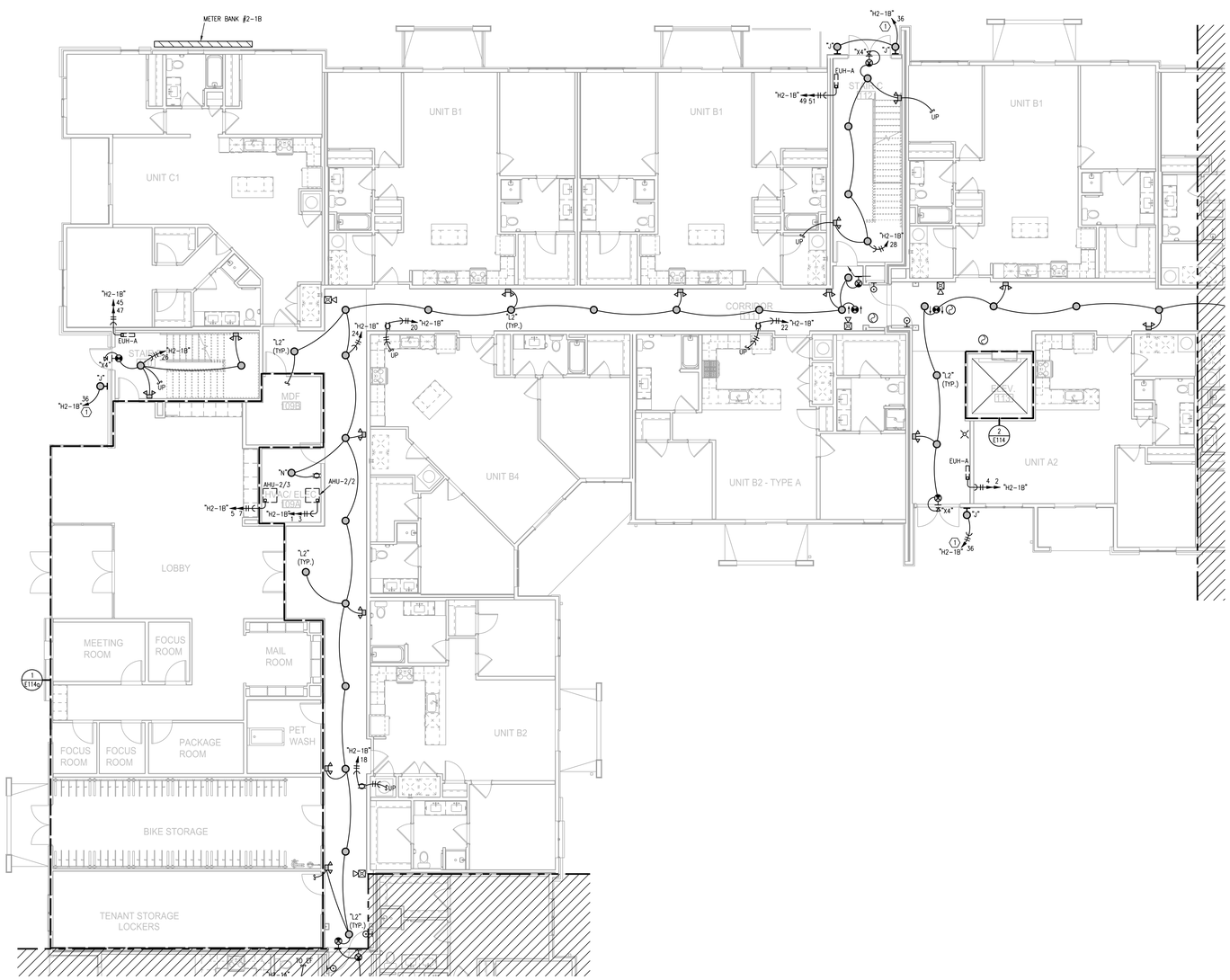
BLDG 2-A, PARTIAL FIRST FLOOR PLAN - ELECTRICAL

NOTES:

1. ALL ⊙ ARE TYPE "A2" UNLESS NOTED OTHERWISE.
2. ALL STAIR LIGHTS ARE TYPE "A" UNLESS NOTED OTHERWISE.
3. ALL ⊗ ARE TYPE "X1" UNLESS NOTED OTHERWISE.
4. ALL ⊗ ARE TYPE "X2" UNLESS NOTED OTHERWISE.
5. ALL ⊗ ARE TYPE "X5" UNLESS NOTED OTHERWISE.
6. ALL ▬ STAIR LIGHTS ARE TYPE "P" U.N.O.
7. CONNECT FIRE/SMOKE DAMPERS SHOWN ON MECHANICAL PLANS TO 120V. AND FIRE ALARM RELAY TO SHUT UPON SMOKE DETECTION IN CORRIDORS.
8. SEE UNIT PLANS FOR FURTHER WORK.
9. LIGHTS AT APT. ENTRY DOORS CONNECT TO APT. HOT CIRCUIT.
10. ALL AHU'S HAVE AUTO DAMPER/THERMOSTAT SYSTEM TO WIRE. SEE HVAC PLANS.
11. EXIT SIGNS SHALL BE READILY VISIBLE TO OCCUPANTS.
12. EXTERIOR LIGHTING TO BE ROUTED THRU PHOTOCELL.
13. "DP-1" DENOTES DISTRIBUTION PANEL.

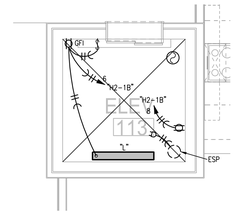
LEGEND:

① TO LIGHTING CONTROL PANEL.



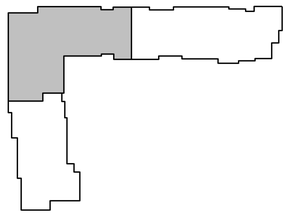
1 BLDG 2-B, PARTIAL FIRST FLOOR PLAN
1/8"=1'-0" ELECTRICAL

NORTH



2 BLDG 2-B, PARTIAL FIRST FLOOR PLAN
1/4"=1'-0" ELECTRICAL

NORTH



KEY PLAN
NO SCALE

NORTH

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GENERAL CONTRACTOR
MECHANICAL ENGINEER
PLUMBING ENGINEER
ELECTRICAL ENGINEER

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ELECTRICAL

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REVISIONS 4

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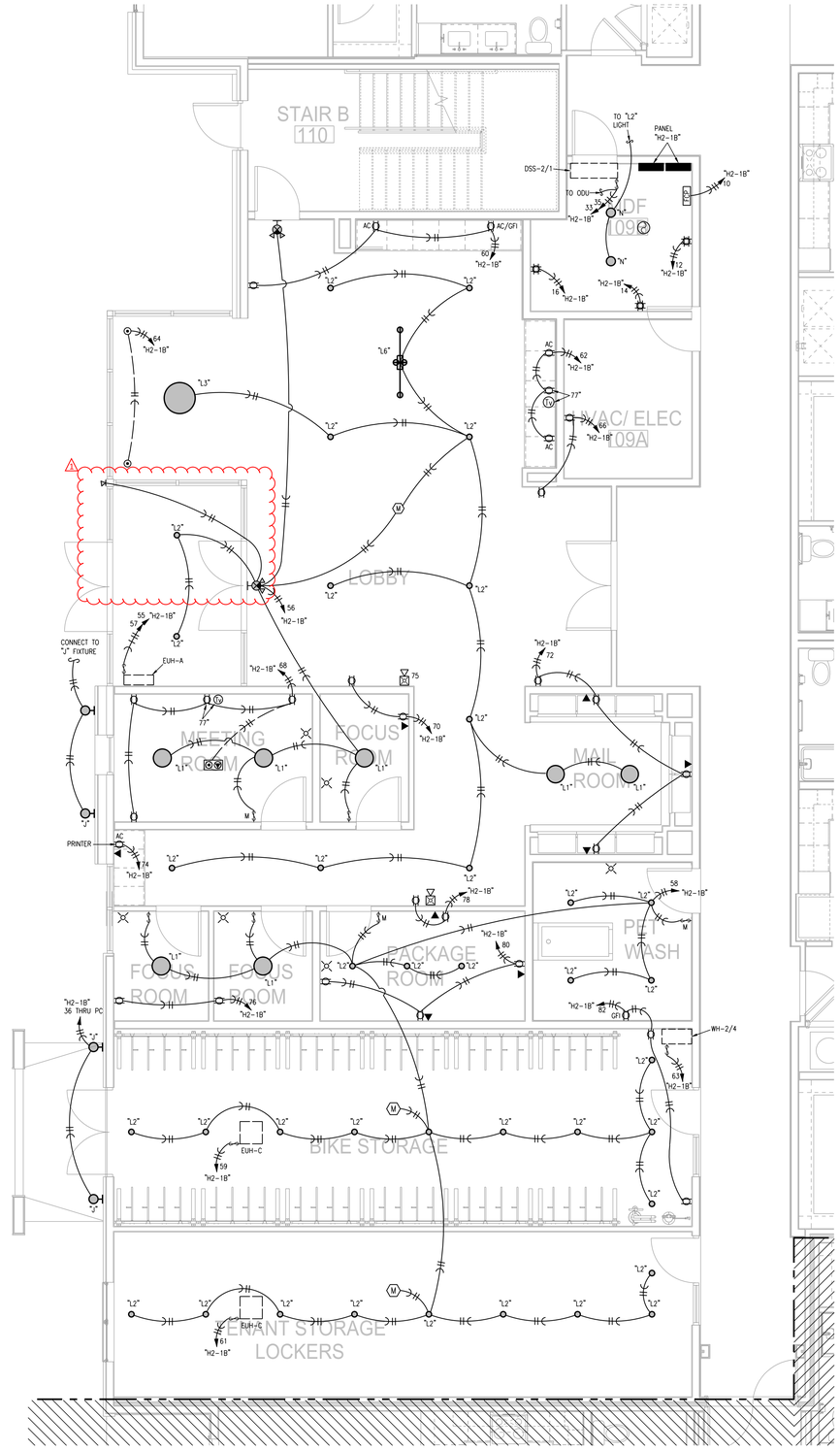
SHEET NO.

E114

BLDG 2-B, PARTIAL FIRST FLOOR PLAN - ELECTRICAL

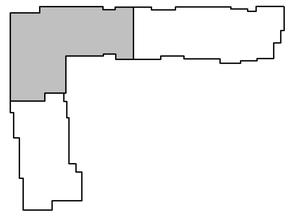


1 **BLDG 2-B, PARTIAL FIRST FLOOR PLAN** — ELECTRICAL
1/4"=1'-0"

NORTH

NOTES:
1. ALL ⊙ ARE TYPE "A2" UNLESS NOTED OTHERWISE.
2. ALL STAIR LIGHTS ARE TYPE "A" UNLESS NOTED OTHERWISE.
3. ALL ⊗ ARE TYPE "X1" UNLESS NOTED OTHERWISE.
4. ALL ⊗ ARE TYPE "X2" UNLESS NOTED OTHERWISE.
5. ALL ⊗ ARE TYPE "X5" UNLESS NOTED OTHERWISE.
6. ALL ▬ STAIR LIGHTS ARE TYPE "P" U.N.O.
7. CONNECT FIRE/SMOKE DAMPERS SHOWN ON MECHANICAL PLANS TO 120V. AND FIRE ALARM RELAY TO SHUT UPON SMOKE DETECTION IN CORRIDORS.
8. SEE UNIT PLANS FOR FURTHER WORK.
9. LIGHTS AT APT. ENTRY DOORS CONNECT TO APT. HOT CIRCUIT.
10. ALL AHU'S HAVE AUTO DAMPER/THERMOSTAT SYSTEM TO WIRE. SEE HVAC PLANS.
11. EXIT SIGNS SHALL BE READILY VISIBLE TO OCCUPANTS.
12. EXTERIOR LIGHTING TO BE ROUTED THRU PHOTOCELL.
13. "DP-1" DENOTES DISTRIBUTION PANEL.

LEGEND:
(1) TO LIGHTING CONTROL PANEL.



KEY PLAN
NO SCALE

NORTH

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REVISIONS 2

2 PERMIT REVIEW COMMENTS 08-17-21

DWG BY CAD/RRB

TR.i PROJECT NO. 20-001

SHEET NO.

E114a

BLDG 2-B, PARTIAL FIRST FLOOR PLAN - ELECTRICAL

NOTES:

1. ALL ⊙ ARE TYPE "A2" UNLESS NOTED OTHERWISE.
2. ALL STAIR LIGHTS ARE TYPE "A" UNLESS NOTED OTHERWISE.
3. ALL ⊗ ARE TYPE "X1" UNLESS NOTED OTHERWISE.
4. ALL ⊗ ARE TYPE "X2" UNLESS NOTED OTHERWISE.
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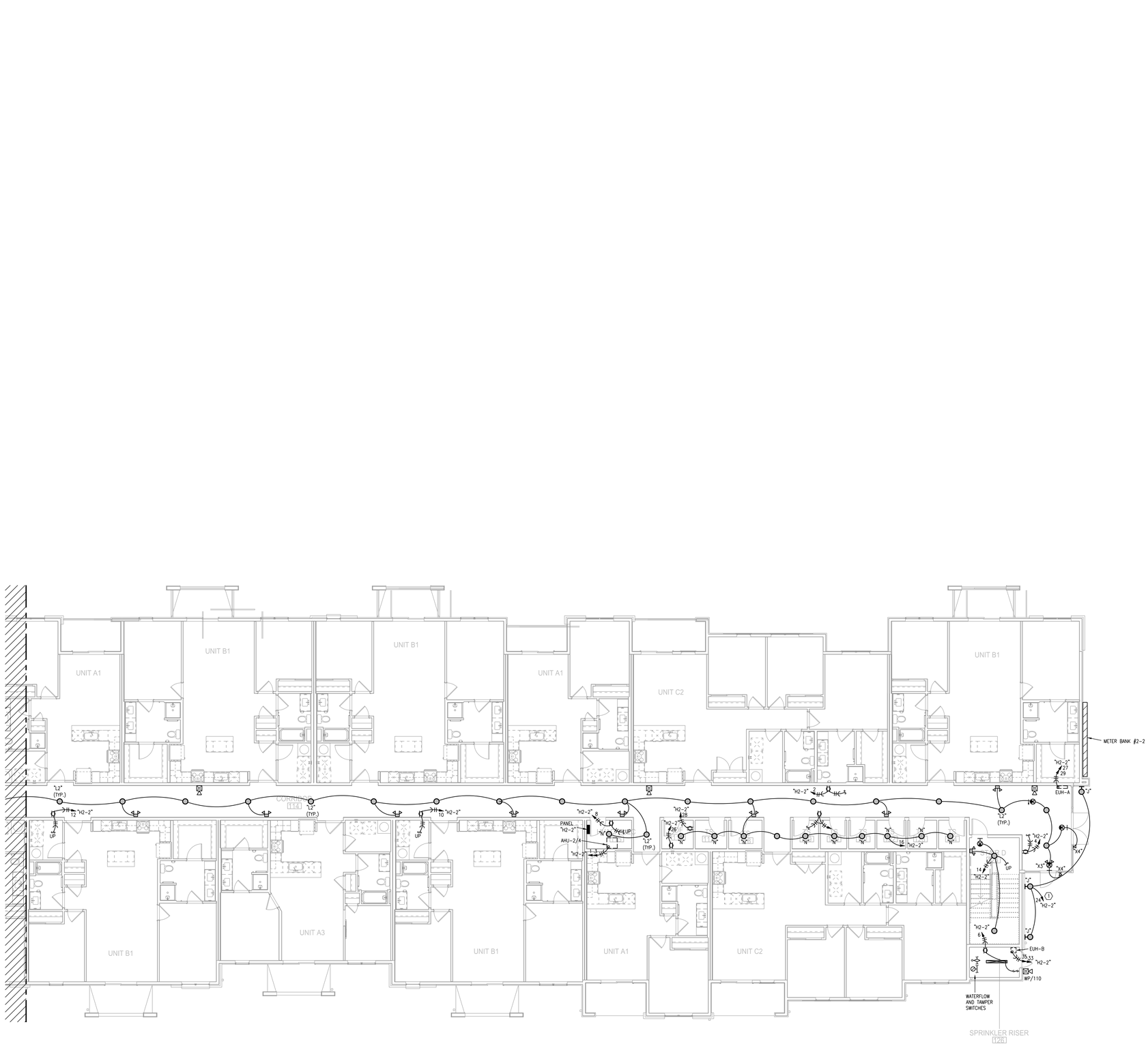
13. "DP-1" DENOTES DISTRIBUTION PANEL.

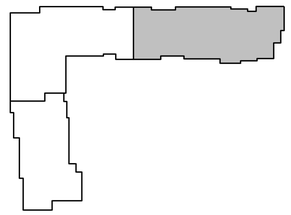
LEGEND:

(1) TO LIGHTING CONTROL PANEL.



1 BLDG 2-C, PARTIAL FIRST FLOOR PLAN
1/8"=1'-0" ELECTRICAL

NORTH



KEY PLAN
NO SCALE

NORTH

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REVISIONS 1

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TR¡I PROJECT NO. 20-001

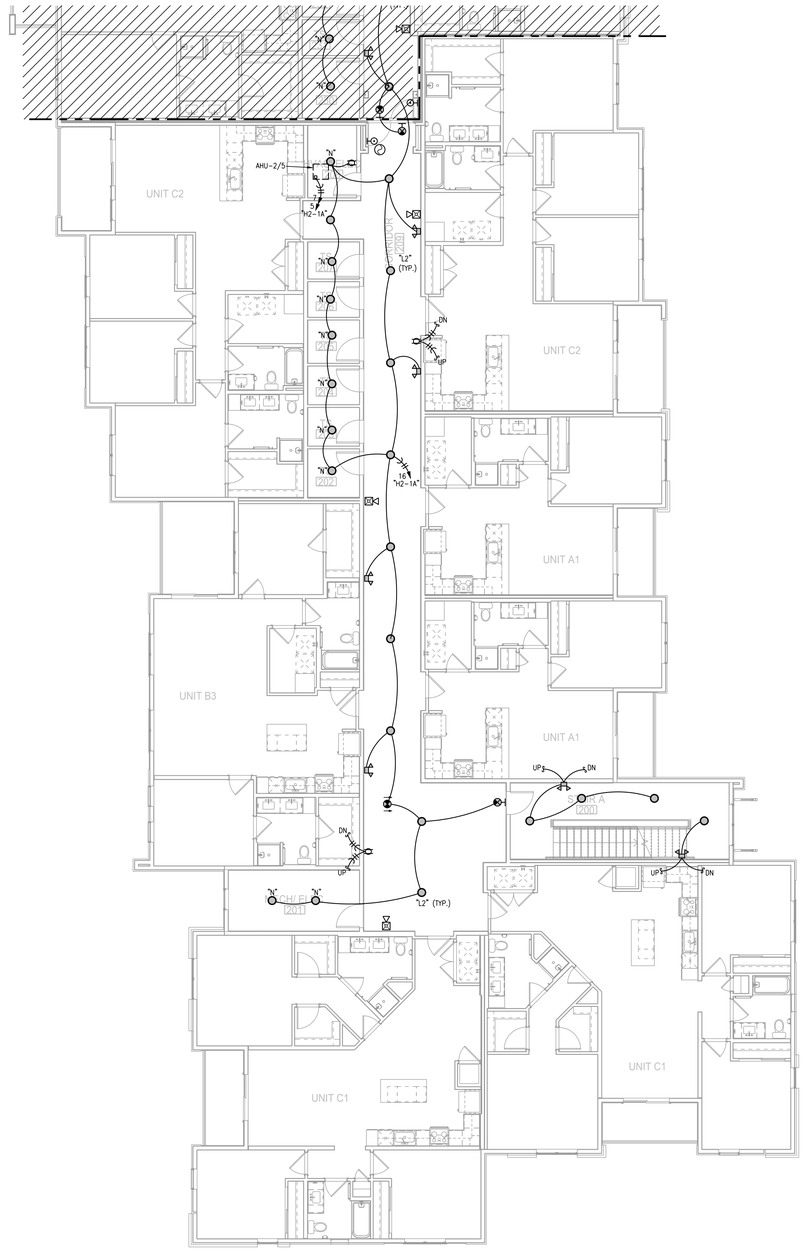
SHEET NO.

E115

BLDG 2-C, PARTIAL FIRST FLOOR PLAN - ELECTRICAL

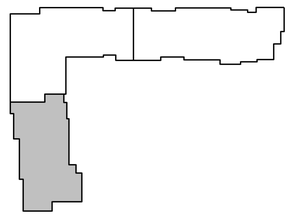
NOTES:
1. ALL ⊙ ARE TYPE "A2" UNLESS NOTED OTHERWISE.
2. ALL STAIR LIGHTS ARE TYPE "A" UNLESS NOTED OTHERWISE.
3. ALL ⊗ ARE TYPE "X1" UNLESS NOTED OTHERWISE.
4. ALL ⊗ ARE TYPE "X2" UNLESS NOTED OTHERWISE.
5. ALL ⊗ ARE TYPE "X5" UNLESS NOTED OTHERWISE.
6. ALL ▬ STAIR LIGHTS ARE TYPE "P" U.N.O.
7. CONNECT FIRE/SMOKE DAMPERS SHOWN ON MECHANICAL PLANS TO 120V. AND FIRE ALARM RELAY TO SHUT UPON SMOKE DETECTION IN CORRIDORS.
8. SEE UNIT PLANS FOR FURTHER WORK.
9. LIGHTS AT APT. ENTRY DOORS CONNECT TO APT. HOT CIRCUIT.
10. ALL AHU'S HAVE AUTO DAMPER/THERMOSTAT SYSTEM TO WIRE. SEE HVAC PLANS.
11. EXIT SIGNS SHALL BE READILY VISIBLE TO OCCUPANTS.
12. EXTERIOR LIGHTING TO BE ROUTED THRU PHOTOCELL.
13. "DP-1" DENOTES DISTRIBUTION PANEL.

LEGEND:
(1) TO LIGHTING CONTROL PANEL.



1 **BLDG 2-A, PARTIAL SECOND FLOOR PLAN**
1/8"=1'-0" ELECTRICAL

NORTH



KEY PLAN
NO SCALE

NORTH

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ELECTRICAL ENGINEER	

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PLUMBING	
ELECTRICAL	

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REVISIONS 2

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TR¡ PROJECT NO. 20-001

SHEET NO.

E116

BLDG 2-A, PARTIAL SECOND FLOOR PLAN - ELECTRICAL

NOTES:
1. ALL ⊙ ARE TYPE "A2" UNLESS NOTED OTHERWISE.
2. ALL STAIR LIGHTS ARE TYPE "A" UNLESS NOTED OTHERWISE.
3. ALL ⊗ ARE TYPE "X1" UNLESS NOTED OTHERWISE.
4. ALL ⊗ ARE TYPE "X2" UNLESS NOTED OTHERWISE.
5. ALL ⊽ ARE TYPE "X5" UNLESS NOTED OTHERWISE.
6. ALL ▭ STAIR LIGHTS ARE TYPE "P" U.N.O.
7. CONNECT FIRE/SMOKE DAMPERS SHOWN ON MECHANICAL PLANS TO 120V. AND FIRE ALARM RELAY TO SHUT UPON SMOKE DETECTION IN CORRIDORS.
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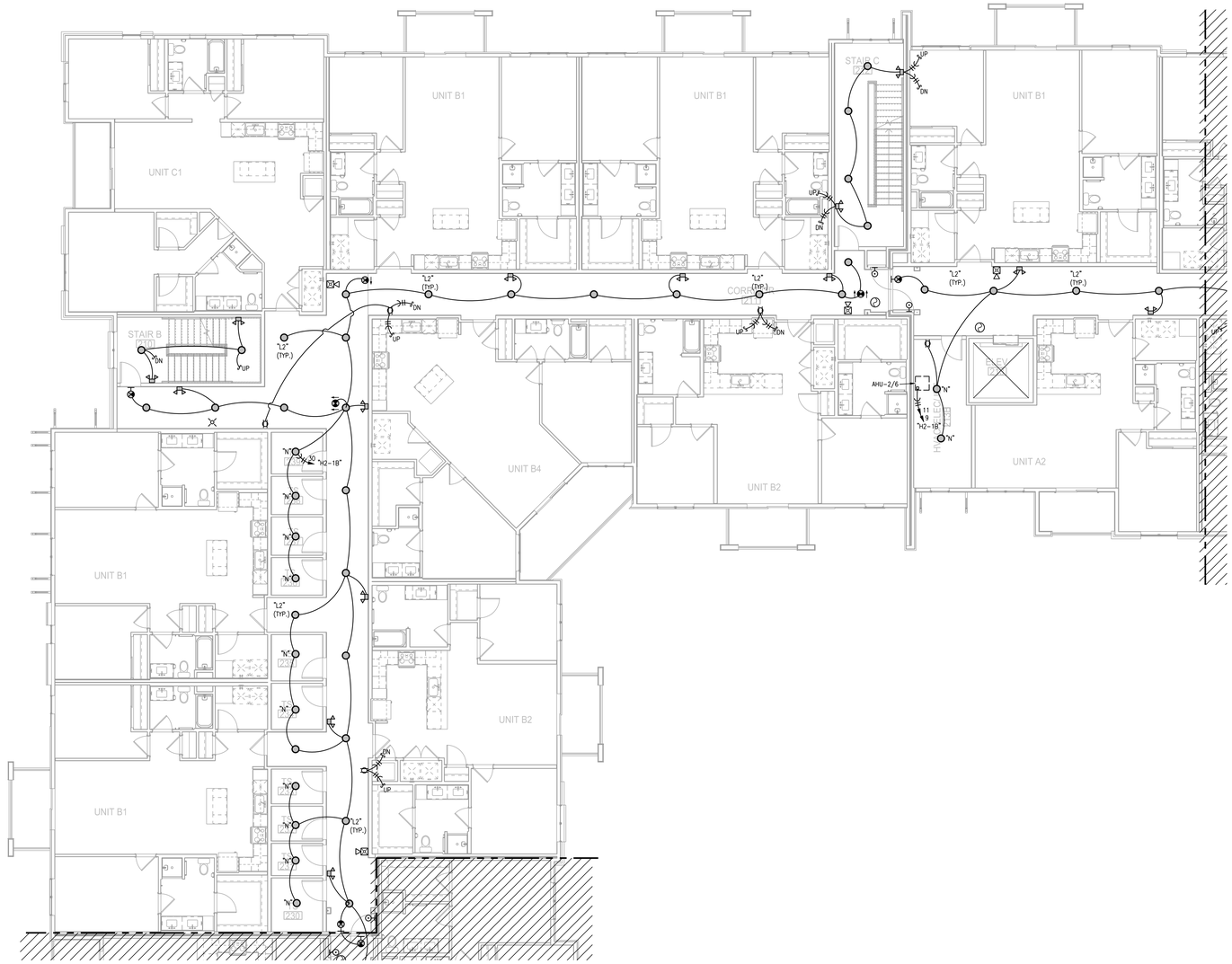
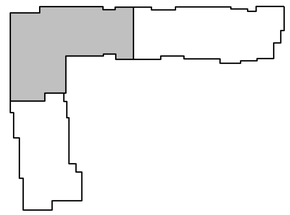
12. EXTERIOR LIGHTING TO BE ROUTED THRU PHOTOCELL.
13. "DP-1" DENOTES DISTRIBUTION PANEL.

LEGEND:
(1) TO LIGHTING CONTROL PANEL.



1 **BLDG 2-B, PARTIAL SECOND FLOOR PLAN**
1/8"=1'-0" ELECTRICAL

NORTH



KEY PLAN
NO SCALE

NORTH

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REVISIONS 4

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SHEET NO.

E117

BLDG 2-B, PARTIAL SECOND FLOOR PLAN - ELECTRICAL

NOTES:
1. ALL ⊙ ARE TYPE "A2" UNLESS NOTED OTHERWISE.
2. ALL STAIR LIGHTS ARE TYPE "A" UNLESS NOTED OTHERWISE.
3. ALL ⊗ ARE TYPE "X1" UNLESS NOTED OTHERWISE.
4. ALL ⊗ ARE TYPE "X2" UNLESS NOTED OTHERWISE.
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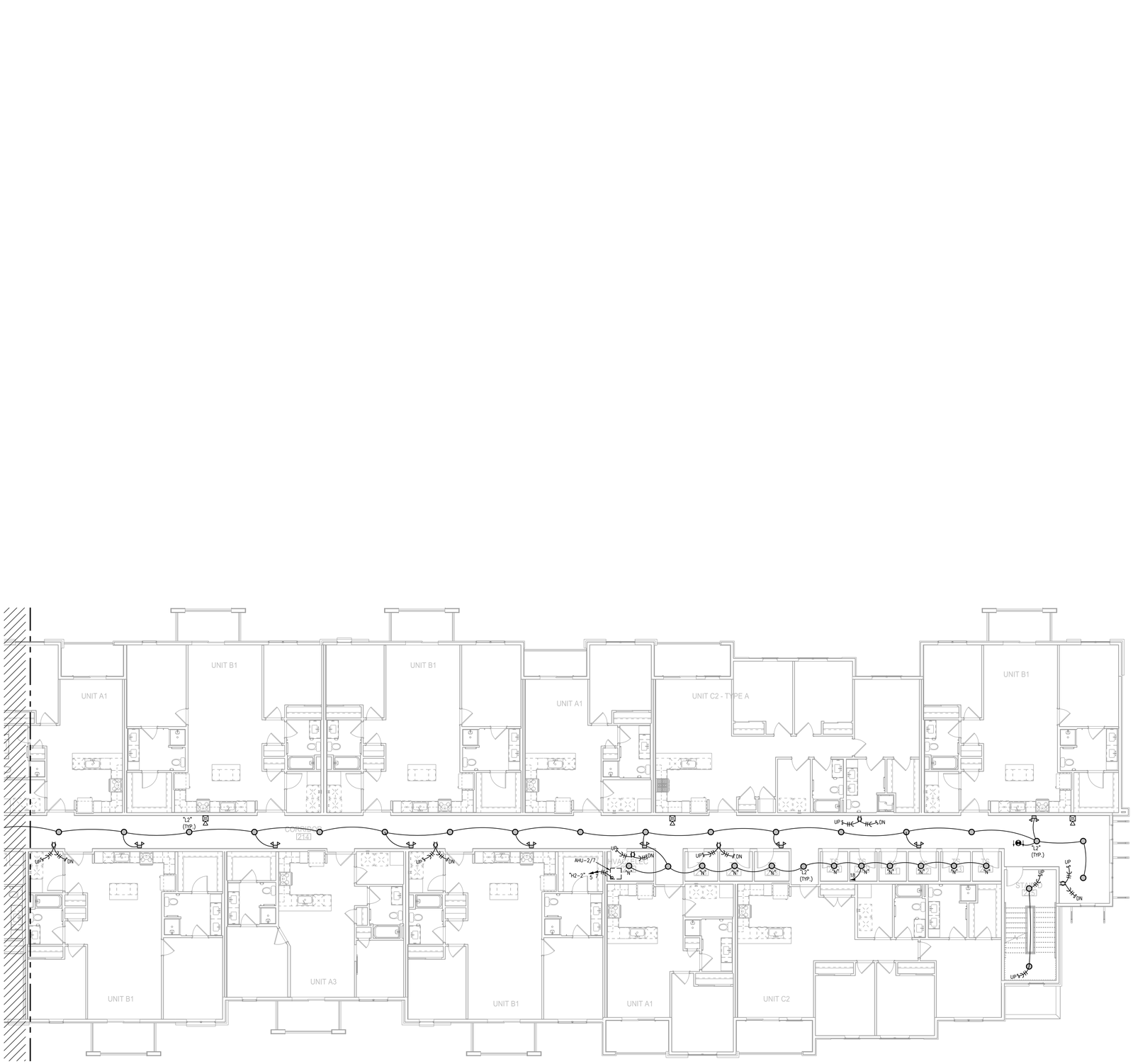
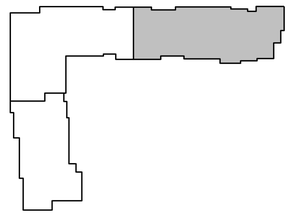
13. "DP-1" DENOTES DISTRIBUTION PANEL.

LEGEND:
(1) TO LIGHTING CONTROL PANEL.



1 **BLDG 2-C, PARTIAL SECOND FLOOR PLAN**
1/8"=1'-0" ELECTRICAL

NORTH



KEY PLAN
NO SCALE

NORTH

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PLUMBING ENGINEER
ELECTRICAL ENGINEER

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ELECTRICAL

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REVISIONS 4

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SHEET NO.

E118

BLDG 2-C, PARTIAL SECOND FLOOR PLAN - ELECTRICAL

NOTES:
1. ALL ⊙ ARE TYPE "A2" UNLESS NOTED OTHERWISE.
2. ALL STAIR LIGHTS ARE TYPE "A" UNLESS NOTED OTHERWISE.
3. ALL ⊗ ARE TYPE "X1" UNLESS NOTED OTHERWISE.
4. ALL ⊗ ARE TYPE "X2" UNLESS NOTED OTHERWISE.
5. ALL ⊗ ARE TYPE "X5" UNLESS NOTED OTHERWISE.
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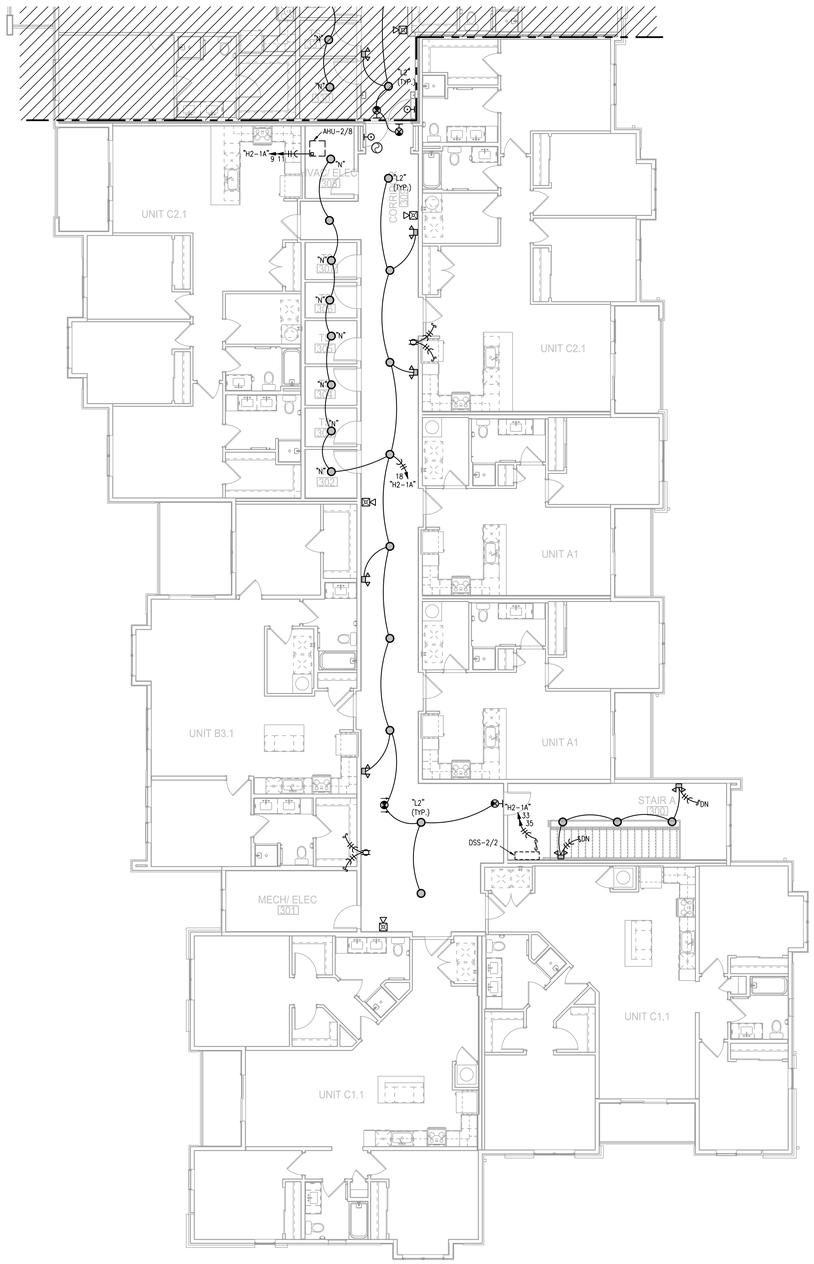
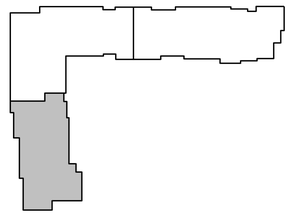
13. "DP-1" DENOTES DISTRIBUTION PANEL.

LEGEND:
(1) TO LIGHTING CONTROL PANEL.



1 **BLDG 2-A, PARTIAL THIRD FLOOR PLAN**
1/8"=1'-0" ELECTRICAL

NORTH



KEY PLAN
NO SCALE

NORTH

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ELECTRICAL ENGINEER

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REVISIONS 2

DWG BY CAD/RRB

TR¡ PROJECT NO. 20-001

SHEET NO.

E119

BLDG 2-A, PARTIAL THIRD FLOOR PLAN - ELECTRICAL

NOTES:

1. ALL ⊙ ARE TYPE "A2" UNLESS NOTED OTHERWISE.
2. ALL STAIR LIGHTS ARE TYPE "A" UNLESS NOTED OTHERWISE.
3. ALL ⊗ ARE TYPE "X1" UNLESS NOTED OTHERWISE.
4. ALL ⊗ ARE TYPE "X2" UNLESS NOTED OTHERWISE.
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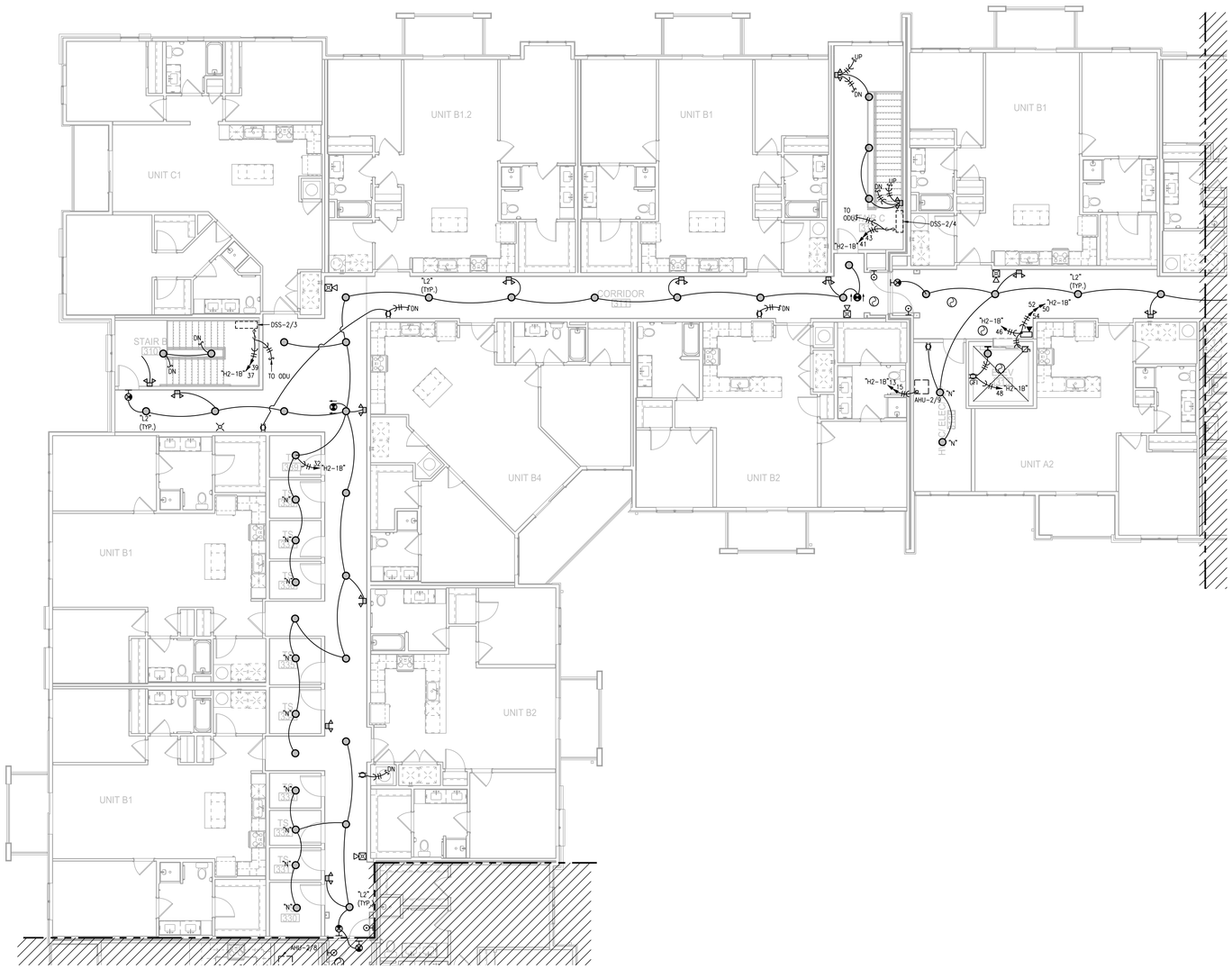
13. "DP-1" DENOTES DISTRIBUTION PANEL.

LEGEND:

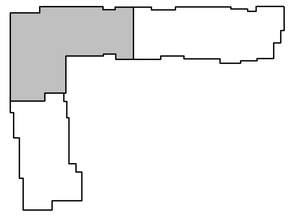
(1) TO LIGHTING CONTROL PANEL.



1 BLDG 2-B, PARTIAL THIRD FLOOR PLAN

1/8"=1'-0" ELECTRICAL

NORTH



KEY PLAN

NO SCALE

NORTH

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MECHANICAL ENGINEER
PLUMBING ENGINEER
ELECTRICAL ENGINEER

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REVISIONS 1

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TR.i PROJECT NO. 20-001

SHEET NO.

E120

BLDG 2-B, PARTIAL THIRD FLOOR PLAN - ELECTRICAL