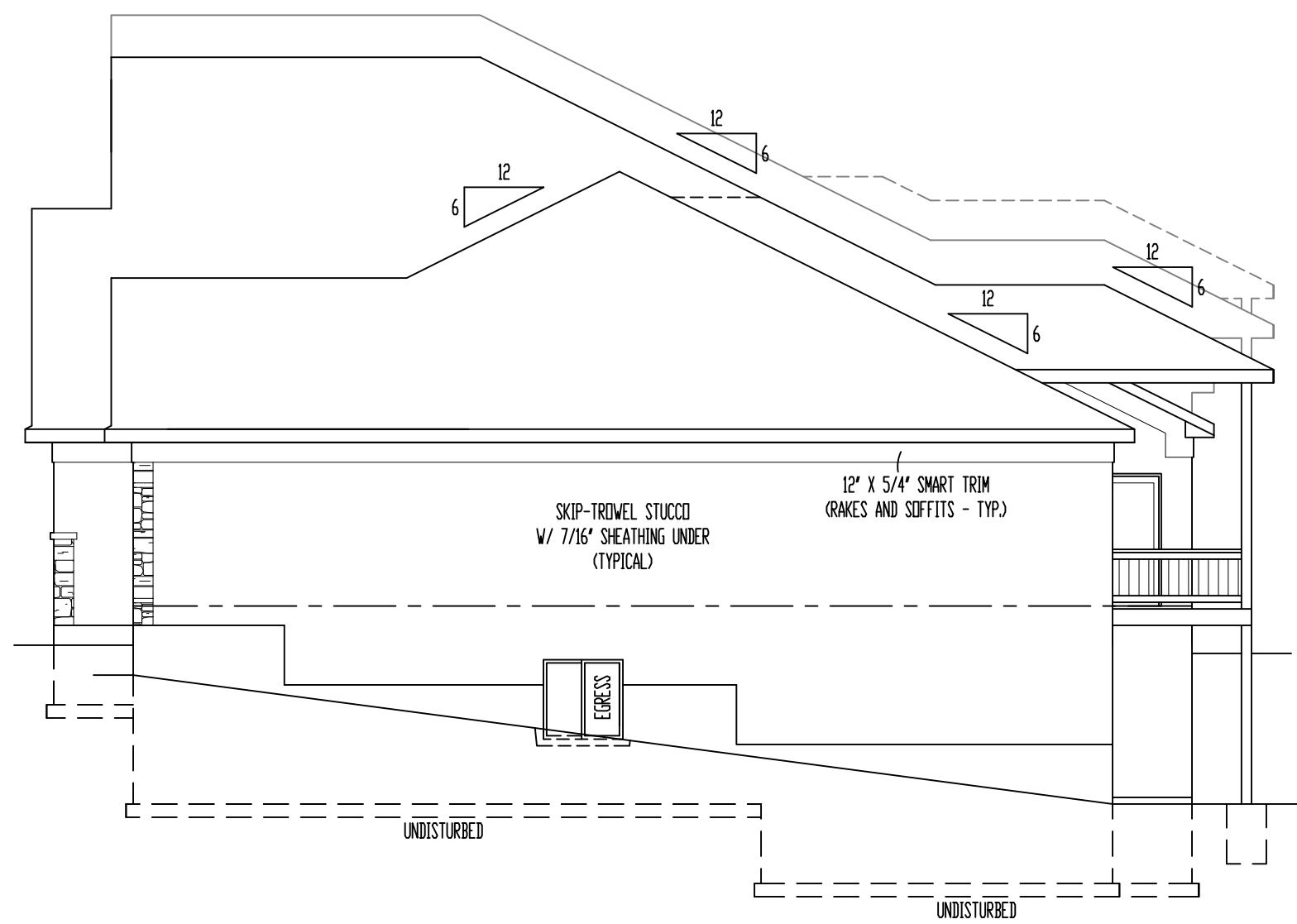
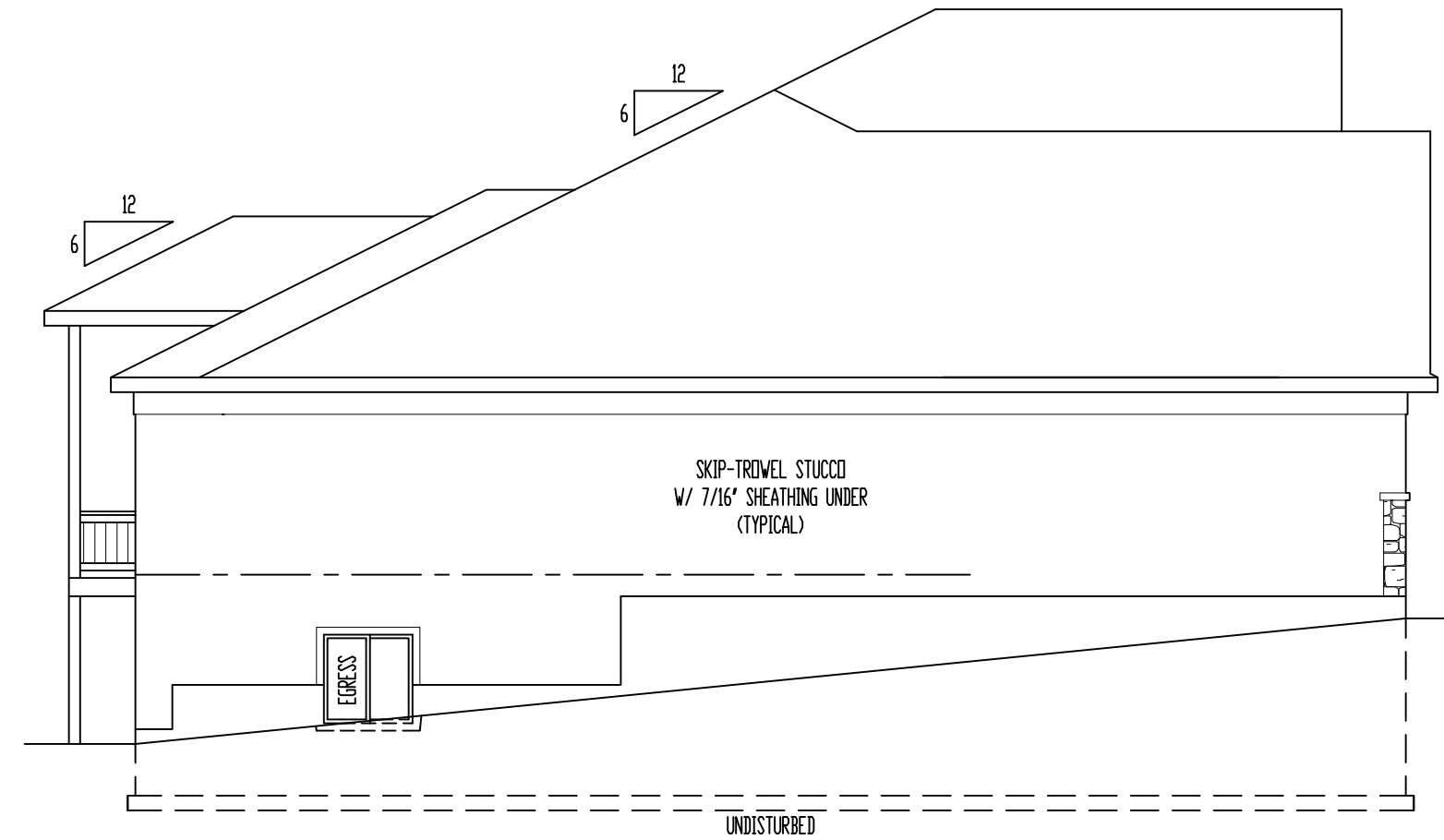


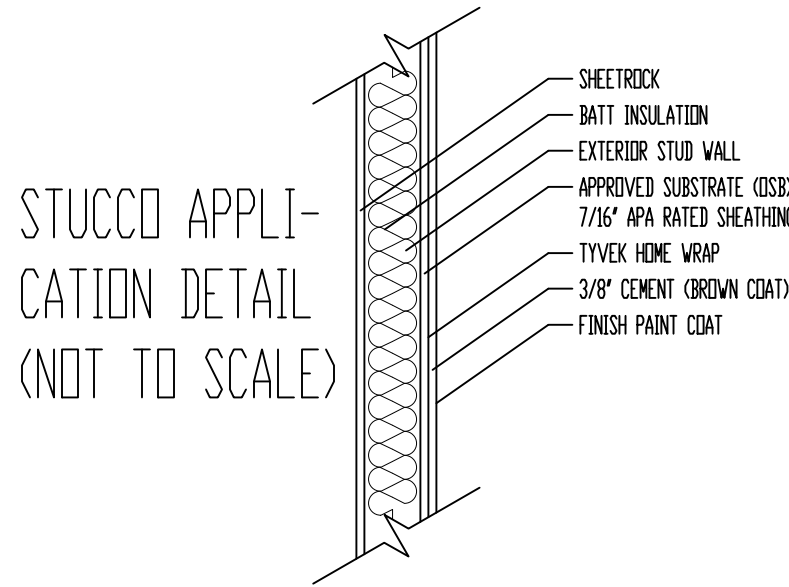
FRONT ELEVATION
SCALE: 1/4" = 1'-0"



RIGHT ELEVATION
SCALE: 1/8" = 1'-0"



LEFT ELEVATION
SCALE: 1/8" = 1'-0"



REAR ELEVATION
SCALE: 1/8" = 1'-0"

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"For God so loved the world, that he gave his only begotten Son, that whosoever should not perish, but have everlasting life."
(John 3:16)

VIEWPOINT
RESIDENTIAL DESIGN LLC

Office: (816) 554-0400 Email: admin@viewpointdesign.net

Site Description:
Lot 11, The Townhomes of Chapel Ridge - 2nd Plat
Street Address:
805, 807, and 809 NE Algonquin St., Lee's Summit, Missouri

Project Title:
TCR011 Triplex
General Contractor:
Kevin Higdon Construction, LLC

STATE OF MISSOURI
DENNIS REIER
NUMBER
PE-201600172
12-12-2021
Professional Engineer

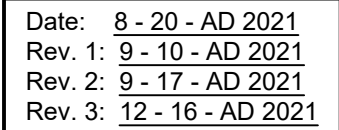
Date: 8 - 20 - AD 2021
Rev. 1: 9 - 10 - AD 2021
Rev. 2: 9 - 17 - AD 2021
Rev. 3: 12 - 16 - AD 2021

Sheet Title:
ELEVATIONS

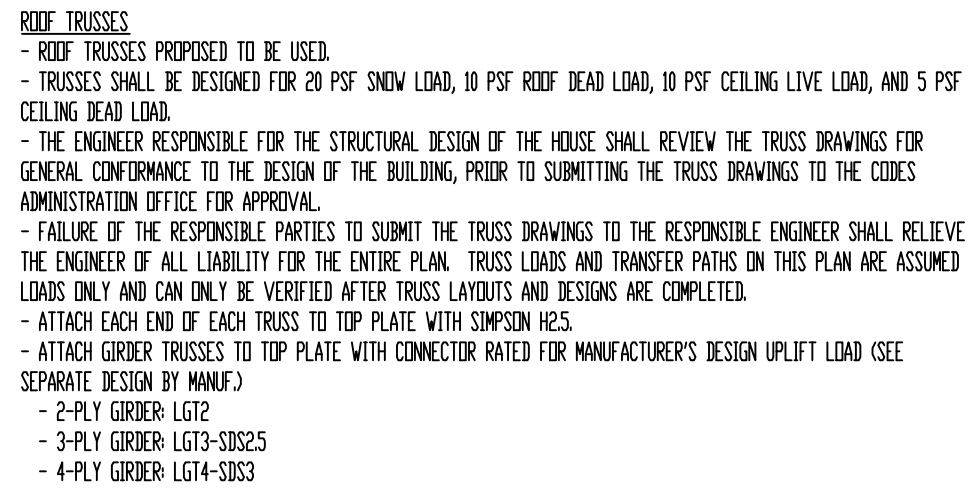
Sheet No.:
A-1 of 4

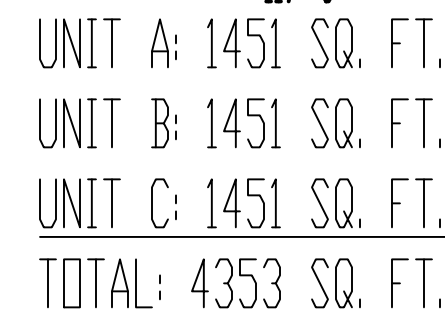
VIEWPOINT
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Office: (816) 554-0400 Email: admin@viewpointdesign.net

Project Title: ***TCR011 Triplex***
General Contractor: ***Kevin Higdon Construction, LLC***



Sheet No.:
A-2 of





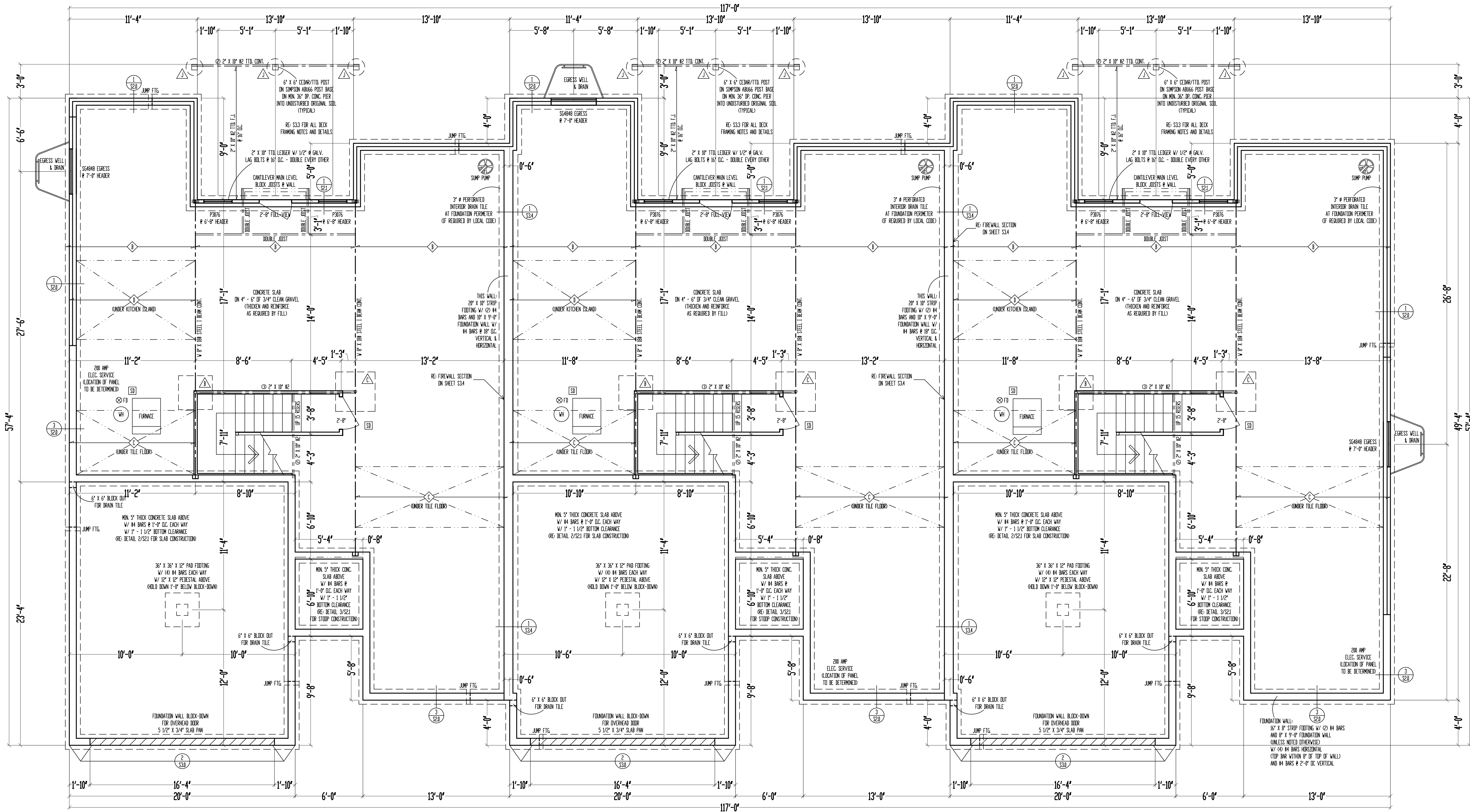
GARAGE A: 472 SQ. FT.
GARAGE B: 472 SQ. FT.
GARAGE C: 472 SQ. FT.
COV. OUT/LIV A: 171 SQ. FT.
COV. OUT/LIV B: 171 SQ. FT.
COV. OUT/LIV C: 171 SQ. FT.

- ROOF TRUSSES
 - ROOF TRUSSES PROPOSED TO BE USED.
 - TRUSSES SHALL BE DESIGNED FOR 20 PSF SNOW LOAD, 10 PSF ROOF DEAD LOAD, 10 PSF CEILING LIVE LOAD, AND 5 PSF CEILING DEAD LOAD.
- THE ENGINEER RESPONSIBLE FOR THE STRUCTURAL DESIGN OF THE HOUSE SHALL REVIEW THE TRUSS DRAWINGS FOR CONFORMANCE WITH THE DESIGN OF THE BUILDING, PRIOR TO SUBMITTING THE TRUSS DRAWINGS TO THE CHIEF OF ADMINISTRATION OFFICE FOR APPROVAL.
- FAILURE OF THE RESPONSIBLE PARTIES TO SUBMIT THE TRUSS DRAWINGS TO THE RESPONSIBLE ENGINEER SHALL RELIEVE THE ENGINEER OF ALL LIABILITY FOR THE ENTIRE PLAN. TRUSS LOADS AND TRANSFER PATHS IN THIS PLAN ARE ASSUMED LOADS ONLY AND CAN ONLY BE VERIFIED AFTER TRUSS LAYOUTS AND DESIGNS ARE COMPLETED.
- ATTACH EACH CHAIN OF EACH TRUSS TO TOP PLATE WITH SPOON NAILS.
- ATTACH GORDER TRUSSES TO TOP PLATE WITH CONNECTOR PLATES PER MANUFACTURER'S DESIGN (LIFT LOAD (SEE SEPARATE LIFTING BY NAME)).
- 2-PLY GORDER LGT2
- 2-PLY GORDER LGT3-S0D25
- 4-PLY GORDER LGT4-S0D3

+++++ = WALL BRACING PER FRAMING NOTE #1 AND PER CALCULATIONS ON SHEET SLL

MAINING NOTES

1. MINIMUM EXTERIOR WALLS SHALL BE SHEATHED W/ 7/16" O.S.B. OR PANELS W/ 84 COMMON NAILS @ 6" O.C. AT EDGES & 12" O.C. IN THE FIELD. SMART PANEL, OR EQUIV., INSTALLED PER MANUFACTURER'S SPECIFICATIONS.
2. 1" X 4" X 1/2" G-120 STIFFENING STRIPS OVER STUDS SPACED 24" MAX FASTENED W/ NO. 6 - 1 1/4" TYPE W OR 3" RIVWALL SCREWS @ 7" O.C. EDGES & FIELD.
3. 4" X 8" SECTIONS ONE SIDE OF WALL (DO NOT 4" X 8" SECTION FOR BOTH SIDES)
4. ^^^^^^^^^^^^^ - LOAD BEARING INTERIOR WALL
5. (2) 2" X 10" H2 HEADER AT INTERIOR AND LOAD BEARING WALLS, UNLESS NOTED OTHERWISE.
6. WOOD TIES @ 4'-0" O.C. (TYPICAL)
7. RUN STUDS THE FULL HEIGHT OF RAISED PLATE WALLS.
8. BLOCK TIES ABOVE BEAMS, CHIMNEYS AND LOAD BEARING WALLS WITH JOIST MATERIAL (NOT REQUIRED WITH I-JOISTS)
9. PROVIDE MULTIPLE STUDS TOP SIDES BEARING BELOW ALL BEAMS
10. ALL DESIGNED 2" X 6" WALLS SHALL HAVE DOUBLE KING STUDS AT DOOR AND WINDOW OPENINGS.
11. ALL UNSQUARE WALLS SHALL BE .45", UNLESS NOTED OTHERWISE.
12. ALL WALLS TO BE FRAMED W/ MIN STUD GRADE 2" X 4" @ 16" O.C., UNLESS NOTED OTHERWISE.
13. EXTERIOR WALL BOTTOM PLATES SHALL BE NAILLED TO FRAMING BELOW WITH 16d COMMON WALLS @ 8" O.C. MAX. (WHERE APPLICABLE).
14. LVLS SHOWN ON PLANS MAY BE REPLACED WITH 3/4" DIF. GRADE 2x4-4x4 GULLUM JOISTS OF THE SAME DEPTH, AND THE FOLLOWING WIDTHS:
 - (2) 1.33x4" LVLS @ 12" O.C. GULLUM
 - (2) 1.33x4" LVLS @ 15" O.C. GULLUM
15. CONTRACTOR SHALL NOTIFY ENGINEER OF RECORD BEFORE CONSTRUCTION OF ANY DEFLECTION LIMITATIONS MORE STRINGENT THAN CODE. MINIMUM ABOVE ANY PENGENS.



9'-0" FOUNDATION WALLS
(UNLESS NOTED OTHERWISE)
ON 16" X 8" STRIP FOOTINGS
(STEP WHERE GRADE REQUIRES)

2" X 10" FLOOR SYSTEM
FOUNDATION
SCALE: 1/4" = 1'-0"

UNFINISHED A: 1333 SQ. FT.
UNFINISHED B: 1333 SQ. FT.
UNFINISHED C: 1329 SQ. FT.

STEEL COLUMN & PAD FOOTING SCHEDULE	
	3" X 11 GA. STEEL COLUMN (ON 30" X 30" X 10" PAD FOOTING W/ (4) #4 BARS EACH WAY (C250))
	3 1/2" X 11 GA. STEEL COLUMN (ON 30" X 30" X 10" PAD FOOTING W/ (4) #4 BARS EACH WAY (C250))
	3" SCH. 40 STEEL COLUMN (ON 42" X 42" X 12" PAD FOOTING W/ (5) #4 BARS EACH WAY (C2450))
	3 1/2" SCH. 40 STEEL COLUMN (ON 48" X 48" X 12" PAD FOOTING W/ (5) #4 BARS EACH WAY (C2500))
	3 1/2" SCH. 40 STEEL COLUMN (ON 54" X 54" X 14" PAD FOOTING W/ (7) #4 BARS EACH WAY (C4630))
	3 1/2" SCH. 40 STEEL COLUMN (ON 60" X 60" X 14" PAD FOOTING W/ (8) #4 BARS EACH WAY (C5080))

PIER FOOTING SCHEDULE	
	12" Ø PIER FTG.
	16" Ø PIER FTG.
	18" Ø PIER FTG.
	24" Ø PIER FTG.

JOIST SCHEDULE	
	2" X 10" R2 JOIST Ø 16" O.C.
	2" X 10" R2 JOIST Ø 18" O.C.
	2" X 10" R2 JOIST Ø 18" O.C. DOUBLE EVERY OTHER
	2" X 10" R2 JOIST Ø 16" O.C. DOUBLED

***** = WALL BRACING PER FRAMING NOTE #1 AND PER CALCULATIONS ON SHEET S31.

- FOUNDING NOTES
1. BASEMENT LEVEL EXTERIOR WOOD-FRAMED WALLS SHALL BE SHEATHED W/ 7/16" D.S.B. APA PANELS W/ 8d COMMON NAILS Ø 6" O.C. AT EDGES & Ø 12" O.C. IN THE FIELD. SMALT PANEL, OR EQUAL, INSTALLED PER MANUFACTURER'S SPECIFICATIONS.
 2. 1" MIN. GYPSUM BOARD OVER STUDS SPACED 24" MAX FASTENED W/ NO. 6 - 1 1/4" TYPE W OR S DRYWALL SCREWS Ø 7" O.C. EDGES & FIELD. MIN. 8'-0" SECTIONS ONE SIDE OF WALL (OR MIN. 4'-0" SECTION FOR BOTH SIDES).
 3. 1" MIN. GYPSUM BOARD OVER STUDS SPACED 24" MAX FASTENED W/ NO. 6 - 1 1/4" TYPE W OR S DRYWALL SCREWS Ø 7" O.C. EDGES & FIELD. MIN. 8'-0" SECTIONS ONE SIDE OF WALL (OR MIN. 4'-0" SECTION FOR BOTH SIDES).
 4. 2" X 10" R2 JOIST AT ALL EXTERIOR AND LOAD BEARING WALLS, UNLESS NOTED OTHERWISE.
 5. LVL TIES Ø 4" O.C. (TYPICAL).
 6. RIM JOISTS ABOVE BEAMS, CANTILEVERS AND LOAD BEARING WALLS WITH JOIST MATERIAL (NOT REQUIRED WITH I-JOISTS).
 7. BLOCK JOISTS ABOVE BEAMS, CANTILEVERS AND LOAD BEARING WALLS WITH JOIST MATERIAL (NOT REQUIRED WITH I-JOISTS).
 8. PROVIDE MULTIPLE STUDS FOR SOLID BEARING BELOW ALL BEAMS.
 9. ALL DESIGNATED 2" X 6" WALLS SHALL HAVE DOUBLE KING STUDS AT DOOR AND WINDOW OPENINGS.
 10. ALL UNDESIGATED 2" X 6" WALLS SHALL BE 45° UNLESS NOTED OTHERWISE.
 11. ALL WALLS TO BE FRAMED W/ MIN. STUD GRADE 2" X 4" S OR 16" O.C. UNLESS NOTED OTHERWISE.
 12. 1/2" Ø ANCHOR BOLTS W/ MIN. 7" EMBEDMENT Ø 48" O.C. MAX & WITHIN 6" - 12" OF END OF EACH PLATE LENGTH.
 13. LVL'S SHOWN ON PLANS MAY BE REPLACED WITH 16" O.C. GRADE 24K-V4 GULAN BEAMS OF THE SAME DEPTH, AND THE FOLLOWING WIDTHS:
Ø 1 3/4" LVL PLIES = 3 1/2" GULAN
Ø 1 3/4" LVL PLIES = 5 1/2" GULAN
 14. NEW FOUNDATION SHALL BEAR ON ORIGINAL SOIL WITH MINIMUM BEARING CAPACITY OF 1500 PSF. A GEOTECHNICAL ENGINEER IS RECOMMENDED FOR VERIFICATION OF THESE CONDITIONS DURING THE EXCAVATION PHASE. ENGINEER OF RECORD ASSUMES NO RESPONSIBILITY FOR CONSTRUCTION NOT VERIFIED TO BE FOUND ON ANYTHING SHORT OF THE aforementioned REQUIREMENTS.
 15. CONTRACTOR SHALL NOTIFY ENGINEER OF RECORD BEFORE CONSTRUCTION OF ANY DEFLECTION LIMITATIONS MORE STRINGENT THAN CODE MINIMUMS ABOVE ANY OPENINGS.

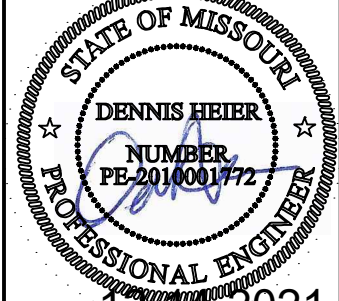
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(John 3:16)

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Date: 8 - 20 - AD 2021
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Sheet Title:
FOUNDATION PLAN

Sheet No.:
A-4 of 4

RESIDENTIAL SEISMIC & WIND ANALYSIS

DETERMINE WEIGHT OF HOUSE:

INPUT			
CALCULATED VALUE			
LOCATION		DEAD LOAD (psf)	AREA (ft²)
ROOF		10	6349
CEILING		10	6349
FIRST FLOOR		10	6349
	WALL LENGTH (ft)	WALL HEIGHT (ft)	WALL UNIT WT. (psf)
FIRST FLOOR EXT. WALL DL	354.66	10	10
		DEAD LOAD (psf)	AREA (ft²)
FIRST FLOOR INT. PARTITION WALL DL		6	6349

PROJECTED AREAS (WIND DESIGN PER 115 MPH 3-SECOND GUST, EXPOSURE C AND MEAN ROOF HEIGHT <= 30 FT ASSUMED)							
FRONT-TO-BACK				SIDE-TO-SIDE			
	AREA	LOAD			AREA	LOAD	
SLOPED ROOF	555	4515		SLOPED ROOF	708	6024	
VERT. ROOF	853	10129	CUMULATIVE	VERT. ROOF	30	373	CUMULATIVE
1ST	1287	15282	30007	1ST	663.63	8250	14728
			PRESSURE (PSF) - PER ASCE CH. 6				
	SLOPED ROOF	ZONE B	9.7		ZONE C	11.3	2a (FIG. 28.6-1, ASCE7)
	WALL/VERT. ROOF	ZONE A	14.2		ZONE D	7.7	12.068
	MEAN ROOF HT., h		24				

a) If there is a walkout wall to be sheathed, determine tributary wind area and enter here. If no walkout, enter 0 for area.

$q_{z=10} = 0.00256 K_z K_{zt} K_d V^2$ (ASCE7-10 Velocity Pressure)

$q_{z=10, ASD} = 0.6 q_{z=10}$ (Design Velocity Pressure for ASD analysis under ASCE7-10 and IRC/IBC 2012)

1ST FLOOR TRIBUTARY WEIGHT

S_g (SITE GROUND MOTION - %g - FROM ASCE7 SEISMIC MAP)

F_s (from ASCE7 Table 11.4-1)

S_{DS} (= 2/3 * S_g * F_s)

R (from ASCE7 Table 12.2-1)

144713

12.0%

1.6

0.128

6.5

SEISMIC SHEAR

LOCATION	From ASCE7 (Eq. 12.8-1):	V (= 1.2 * S _{DS} * W / R) (lbs.)
1ST FLOOR		3420

Sheathing Location	Min. Sheathing Schedule	Fastening Schedule	Allowable Shear (#/LF)	Code Reference
Exterior (Option #1)	7/16" APA Rated Plywood/OSB	1-1/2" 18ga. Staples w/ 1" penetration @ 8" O.C. Edges, 8" O.C. Field For 24" stud spacing, 12" O.C. Field For 16" stud spacing	155	per IBC, Table 2306.3(1)
Exterior (Option #2)	7/16" APA Rated Plywood/OSB	1-1/2" 18ga. Staples w/ 1" penetration @ 4" O.C. Edges, 8" O.C. Field For 24" stud spacing, 12" O.C. Field For 16" stud spacing	230	per IBC, Table 2306.3(1)
Exterior (Option #3)	7/16" APA Rated Plywood/OSB	1-1/2" 18ga. Staples w/ 1" penetration @ 3" O.C. Edges, 8" O.C. Field For 24" stud spacing, 12" O.C. Field For 16" stud spacing	310	per IBC, Table 2306.3(1)
Exterior (Option #4)	7/16" APA Rated Plywood/OSB or shiplap panel sheathing, or 3/8" shiplap panel sheathing with tighter nail spacing	8d Common Nails w/ 1-3/8" penetration @ 6" O.C. Edges, 12" O.C. Field for 7/16" APA-rated plywood/OSB or shiplap panel sheathing OR @ 4" O.C. Edges, 12" O.C. Field for 3/8" shiplap panel sheathing	220	AF&PA SDPWS Table 4.3A
Exterior (Option #5)	7/16" APA Rated Plywood/OSB or shiplap panel sheathing, or 3/8" shiplap panel sheathing with tighter nail spacing	8d Common Nails w/ 1-3/8" penetration @ 4" O.C. Edges, 12" O.C. Field for 7/16" APA-rated plywood/OSB or shiplap panel sheathing OR @ 3" O.C. Edges, 12" O.C. Field for 3/8" shiplap panel sheathing	320	AF&PA SDPWS Table 4.3A
Exterior (Option #6)	7/16" APA Rated Plywood/OSB or shiplap panel sheathing, or 3/8" shiplap panel sheathing with tighter nail spacing and double studs at each panel edge	8d Common Nails w/ 1-3/8" penetration @ 3" O.C. Edges, 12" O.C. Field	410	AF&PA SDPWS Table 4.3A
Interior	1/2" Gypsum Board	No. 6- 1 1/4" Type W or S Screws @ 8" O.C. Edges, 12" O.C. Field	60	per IBC, Table 2306.4.4
Interior	16 Ga. Simpson/USP Type WB Steel X-Brace (or equal)	(3) 16d @ end studs & (1) 8d @ intermediate studs (per manufacturer specifications - see detail on sheet S3)	325	

EXTERIOR SHEATHING OPTION FOR FIRST FLOOR	4
EXTERIOR SHEATHING OPTION FOR BASEMENT WALLS	4

WIDTH OF 1ST STORY (FT.)	117
DEPTH OF 1ST STORY (FT.)	60.33
BACK WALL OF GARAGE (FT.)	0
GAR. WALL: 1=F-B, 2=S-S	2

WIDTH OF 2ND STORY (FT.) 1
DEPTH OF 2ND STORY (FT.) 1

	SEISMIC				WIND			
	FRONT-TO-BACK	RESISTANCE (lbs.)	SIDE-TO-SIDE	RESISTANCE (lbs.)	FRONT-TO-BACK	RESISTANCE (lbs.)	SIDE-TO-SIDE	RESISTANCE (lbs.)
1ST FLOOR	114	31920	49.5	13860	114	44688	49.5	19404

1ST FLOOR FRONT-TO-BACK 1ST FLOOR SIDE-TO-SIDE BASEMENT FRONT-TO-BACK BASEMENT SIDE-TO-SIDE	ADDITIONAL RESISTANCE REQUIRED		Anchor Bolt Spacing (in.)		16d Nail Spacing req'd at bottom plate (in.)	
	SEISMIC	WIND	diameter (in.)	0.5	1st Floor F-B	11
	0	0	Shear value (per NDS)	944	1st Floor S-S	43
	0	0	Spacing F-B (inches)	72.9		
			spacing S-S (inches)	288.0		

RESISTANCE REQUIRED IN ADDITION TO RESISTANCE PROVIDED BY EXTERIOR WALLS**						
	ADDITIONAL RESISTANCE REQUIRED (POUNDS)	PORTAL FRAMES OR PERF. SHEAR WALL RESISTANCE	INTERIOR X-BRACES (325#/BRACE)	INTERIOR WALL LENGTH W/ 1/2" GYPSUM BOARD PER TABLE (FT.)	INT. WALL LENGTH SHEATHED W/ OSB (TOTAL LENGTH, ONE SIDE, FT.)	RESISTANCE PROVIDED BY ADDITIONAL METHODS (POUNDS)
1ST FLOOR FRONT-TO-BACK	0					0
1ST FLOOR SIDE-TO-SIDE	0					0

**NOTES: 1) SEE ATTACHED CALCULATIONS FOR PORTAL FRAME OR PERFORATED SHEAR WALL RESISTANCE CAPACITIES (IF APPLICABLE).

2) SEE SHEET S1 FOR INTERIOR STEEL X-BRACE INSTALLATION. 3) INTERIOR WALLS SHEATHED WITH OSB SHALL BE ATTACHED WITH SAME STAPLE/NAILING PATTERN AS EXTERIOR OSB ON SAME FLOOR (SEE TABLE ABOVE) AND ARE ONLY APPLICABLE FOR FULL-HEIGHT SECTIONS OF 2'-8" OR LONGER

ALL LATERAL BRACING ACHIEVED AT EXTERIOR WALLS AND WALLS DIRECTLY ON FOUNDATIONS; THEREFORE, NO INTERIOR BRACING PER 2012 IRC SECTION R502.2.1 IS REQUIRE!

WIND UPLIFT ANALYSIS							
	X/12	DEGREES					
ROOF PITCH (MAX)	12	45.0	PITCH OF 6 OR LESS: EOH -13.3, E -7.2, G -5.2				
		ASCE 7					
	LENGTH (FT.)	PRESSURE (PSF)	LINEAL FT. OF OH	UPLIFT PER FT* (LBS)			
OVERHANG	1	-1.08	356.66	-1.08			
	TOTAL AREA (FT²)	ZONE E AREA (FT²)	ZONE G AREA (FT²)	PRESSURE ZN. E (PSF)	PRESSURE ZN. G (PSF)	TOTAL FORCE (LBS)	FORCE PER LINEAL FT @ PERIMETER (LBS)
MAIN ROOF**	7058.61	-534.089424	7592.699424	-1.08	-0.36	-2157	-6.1
*ALONG PERIMETER	TOTAL UPLIFT PER LINEAL FOOT ALONG EXTERIOR (POUNDS)			-7.2	UPLIFT OK		
**INSIDE EXTERIOR WALLS	RESISTANCE DUE TO DEAD WEIGHT & (3) 16d TOENAILS			251.6			

NOTE FOR CONSTRUCTION:

THE CONTINUOUS STRUCTURAL PANEL SHEATHING BRACING METHOD REQUIRES USE OF THE ABOVE TABLE FOR SHEATHING OF THE ENTIRE STRUCTURE. IN ADDITION, FRAMING MEMBERS SHALL BE @ 16" O.C. MAX., UNBLOCKED, AND W/ SHEATHING APPLIED DIRECTLY TO FRAMING MEMBERS

NOTE FOR DESIGN:

ALL WALLS USED IN THE CALCULATION OF THE RESISTANCE FOR THIS STRUCTURE SHALL HAVE A MINIMUM UNINTERRUPTED HEIGHT OF 8'-0" AND LENGTH OF 2'-8". ALLOWABLE RESISTANCES HAVE BEEN #/FT AND INCREASED BY 40% FOR WIND LOADS, PER VALUES IN 2012 IBC SECTION 2306 AND AF&PA SDPWS TABLE 4.3A. FOR EXAMPLE, 7/16" APA-RATED SHEATHING WITH 8d @ 6" & 12" HAS A SEISMIC SHEAR VALUE OF 240 A WIND SHEAR VALUE OF 335#/FT - 40% GREATER THAN THAT OF SEISMIC)

NOTE: SOIL SITE CLASS ASSUMED TO BE CLASS D. IF SITE CONDITIONS ARE DETERMINED TO BE CLASS E OR F, CONSULT ENGINEER BEFORE PROCEEDING WITH CONSTRUCTION



14718 NW DELIA STREET * PORTLAND, OREGON 97229
OFFICE: 971.645.0901 * MOBILE: 971.645.0901 *
* DENNIS@VISTASTRUCTURAL.COM * VISTASTRUCTURAL.COM

CLIENT: KEVIN HIGDON CONSTRUCTION

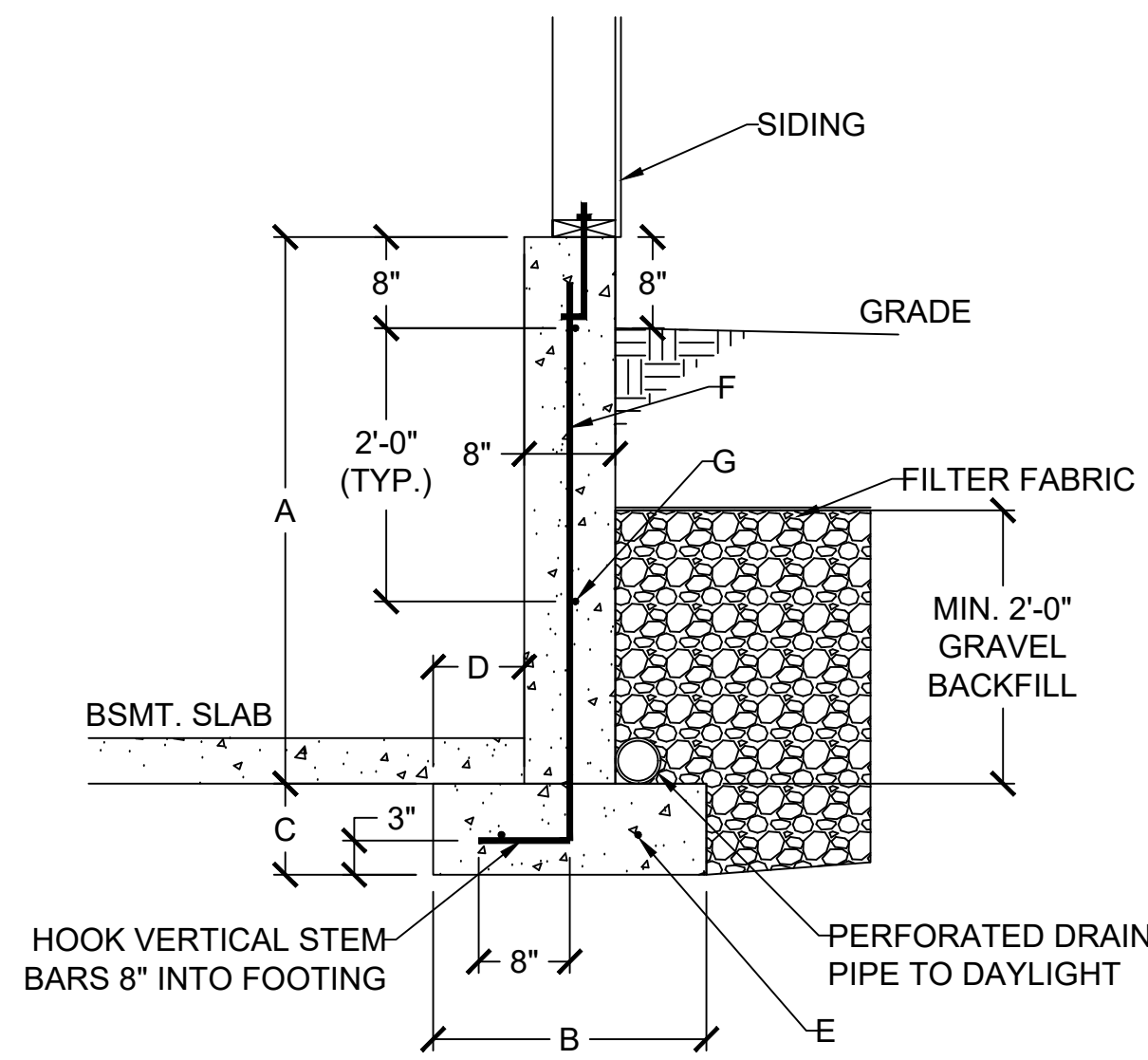
JOB TITLE: TCR011 TRIPLEX
LOT 111, THE TOWNHOMES OF CHAPEL RIDGE
2ND PLAT

LOCATION: 805, 807, 809 NE ALCONQUIN ST.
LEE'S SUMMIT, MISSOURI



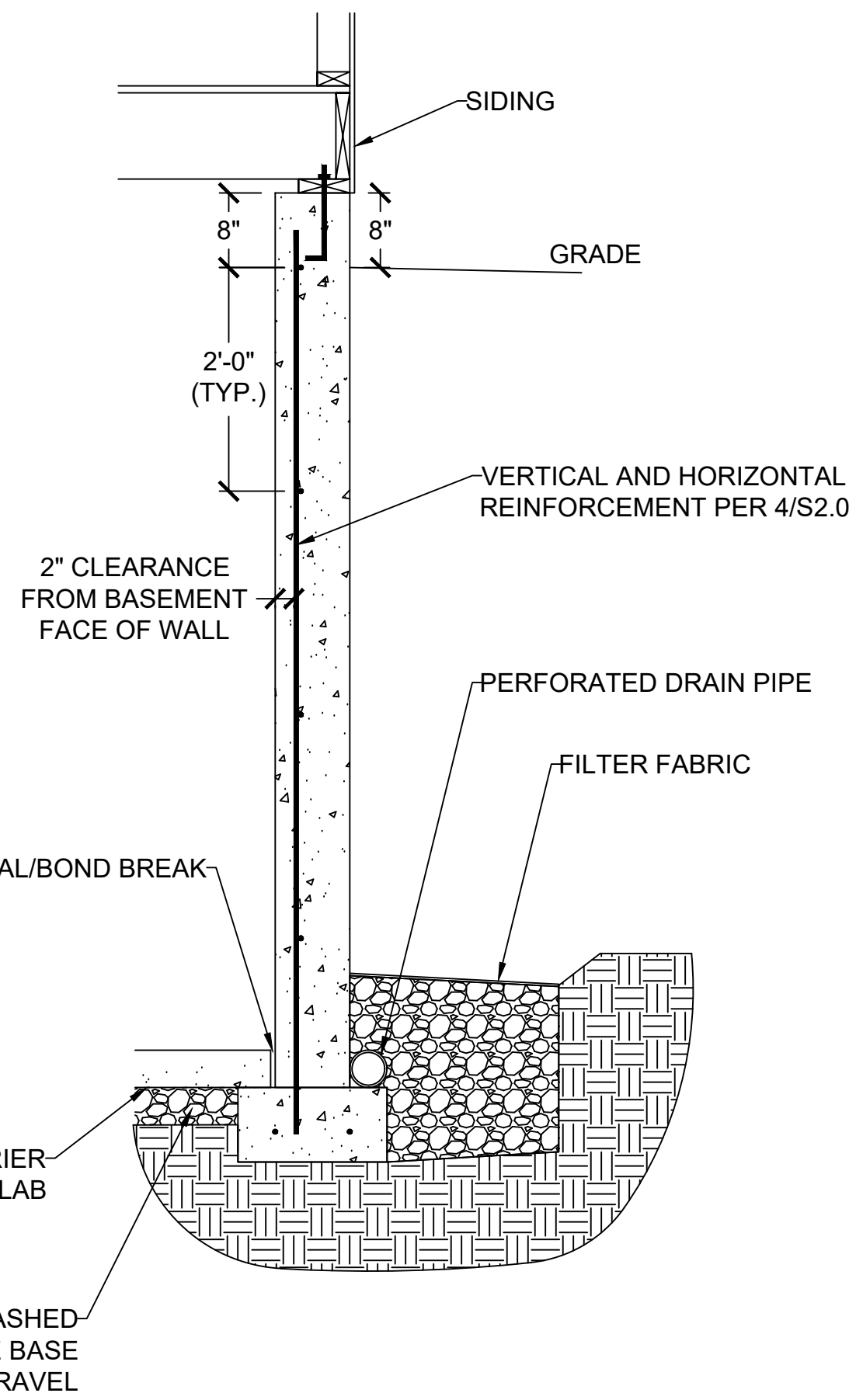
NO.	DATE	REVISION	BY
DRAWING TITLE			
STRUCTURAL CALCULATION			
ENGINEER: DMH		CHECKED BY: DMH	
JOB NO. 3812		DRAWN BY: DMH	
DATE: 12-14-21			
SHEET NUMBER			

S1.1

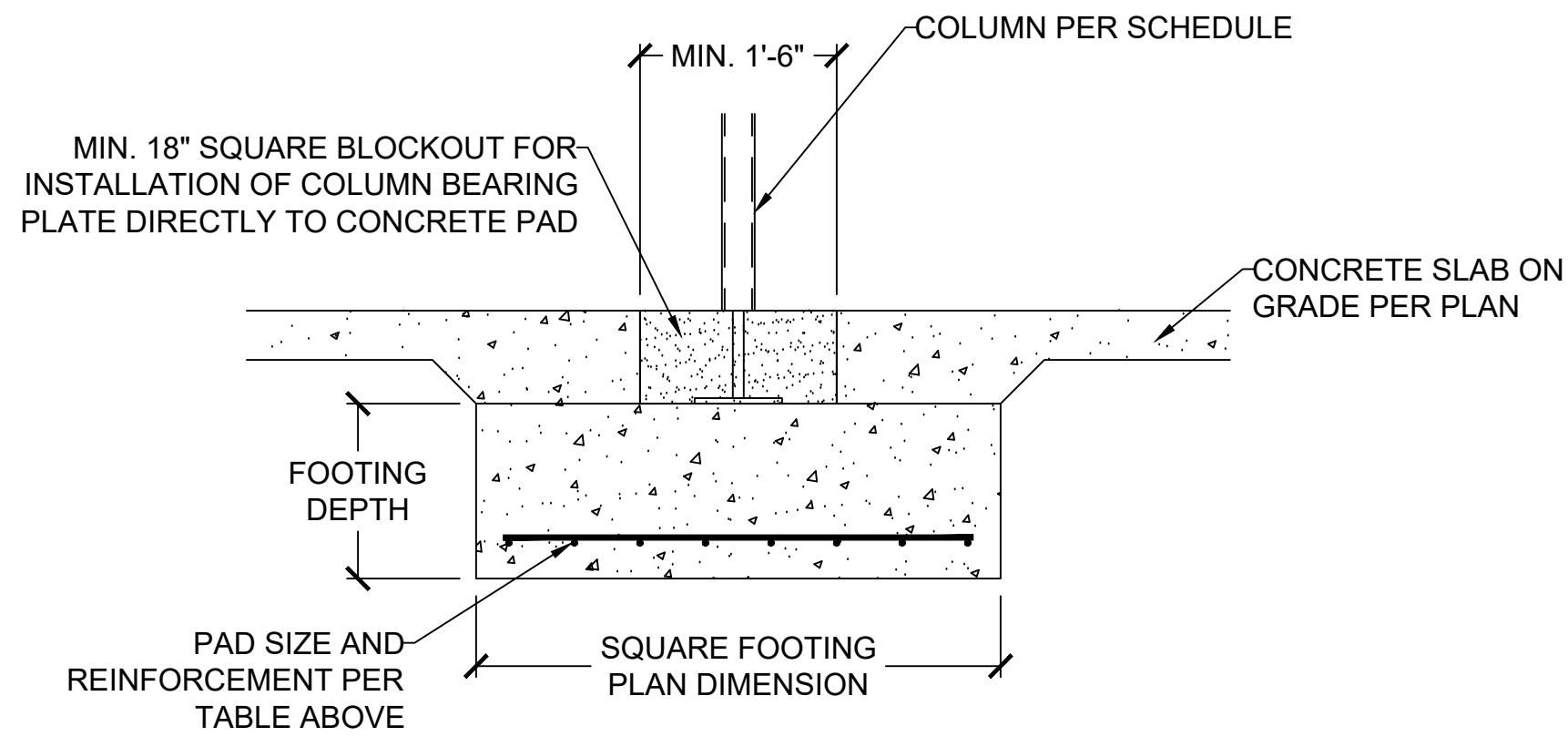


1 DAYLIGHT WALL CONSTRUCTION
S2.0 SCALE: 1/2" = 1'-0" (18x24) OR 3/4" = 1'-0" (24x36)

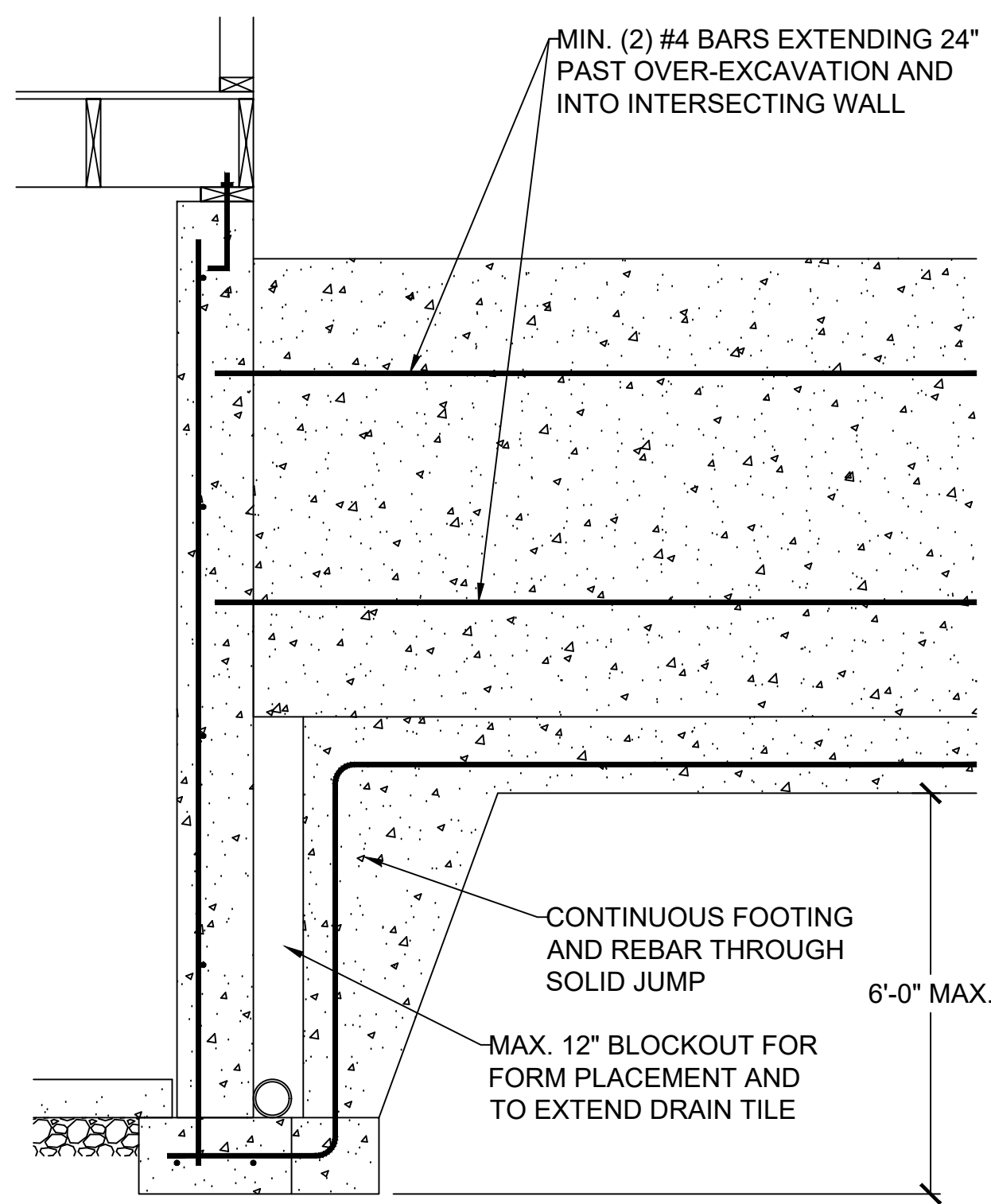
DAYLIGHT BASEMENT WALL SCHEDULE						
A	B	C	D	E	F	G
4'-0"	1'-6"	0'-8"	0'-5"	(2) #4	#4 VERT. @ 12" O.C.	(2) #4 HORIZ.
5'-0"	2'-0"	0'-8"	0'-7"	(2) #4	#4 VERT. @ 12" O.C.	(3) #4 HORIZ.
6'-0"	2'-6"	0'-8"	0'-10"	(3) #4	#4 VERT. @ 12" O.C.	(3) #4 HORIZ.



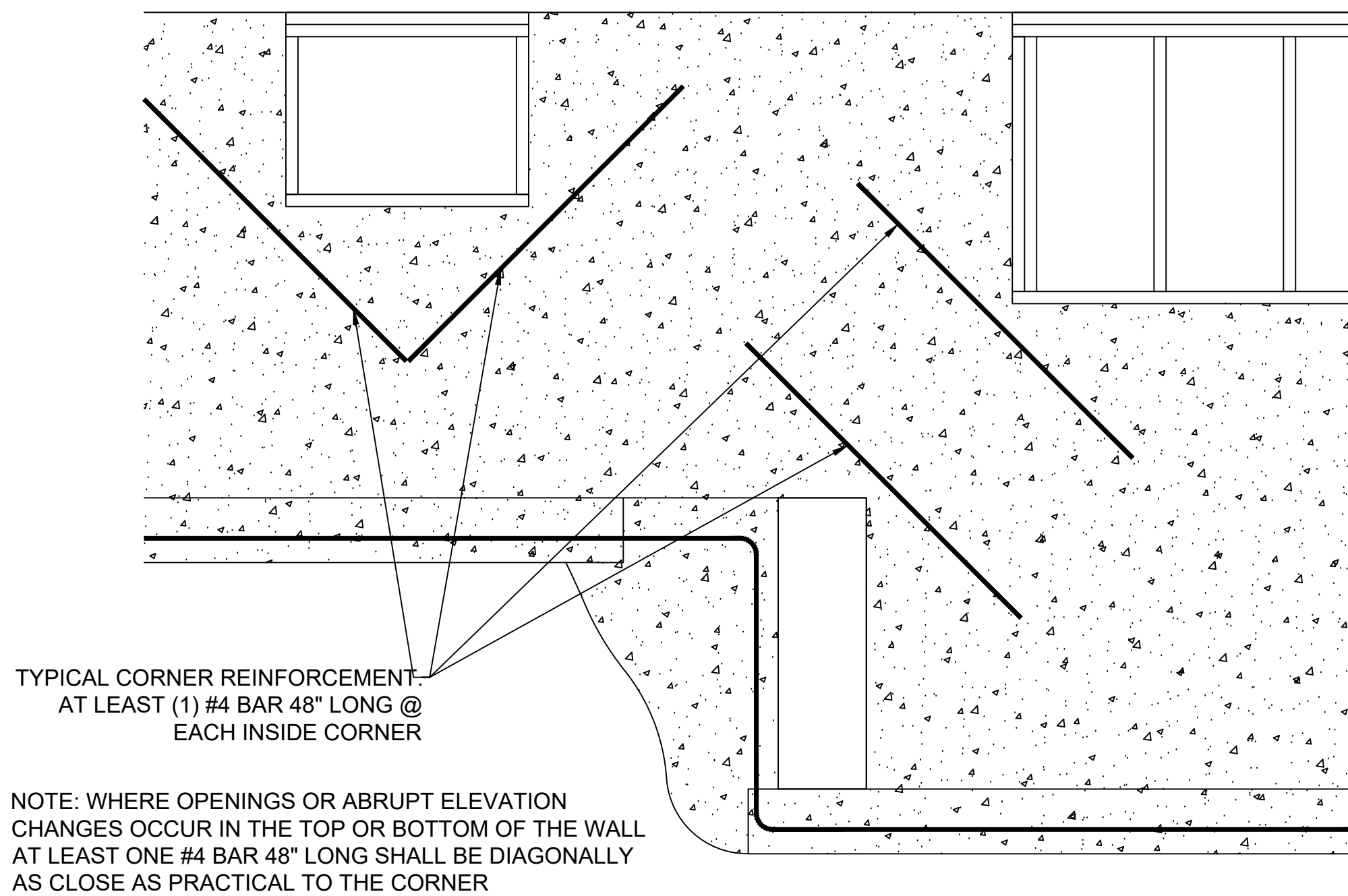
3 CONCRETE WALL SECTION
S2.0 SCALE: 1/2" = 1'-0" (18x24) OR 3/4" = 1'-0" (24x36)



2 COLUMN AND BEARING PAD SCHEDULE
S2.0 SCALE: 1/2" = 1'-0" (18x24) OR 3/4" = 1'-0" (24x36)

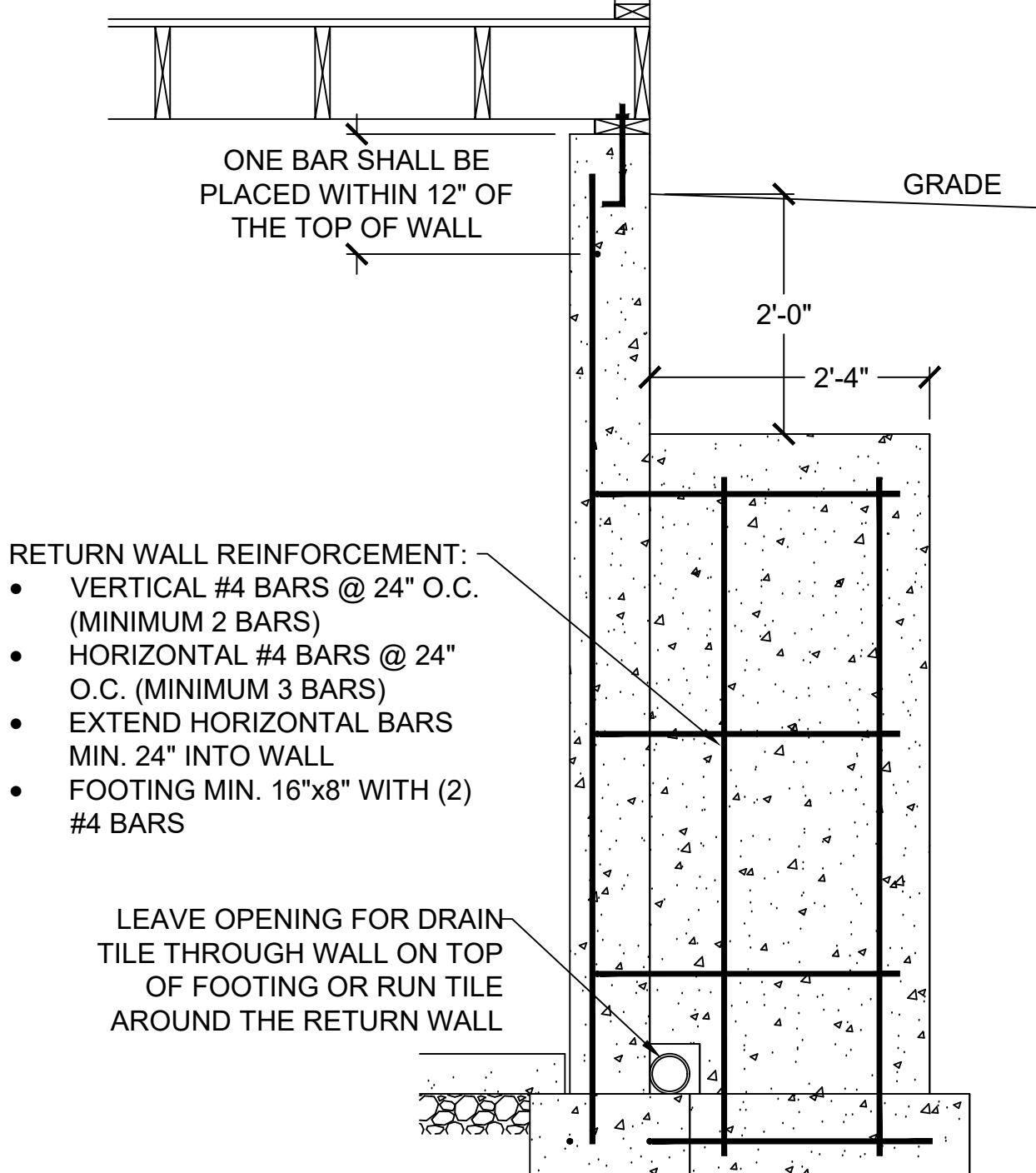


5 SOLID JUMP
S2.0 SCALE: 1/2" = 1'-0" (18x24) OR 3/4" = 1'-0" (24x36)



6 REINFORCEMENT AT OPENING CORNERS AND STEP CORNERS @ INSIDE CORNERS
S2.0 SCALE: 1/2" = 1'-0" (18x24) OR 3/4" = 1'-0" (24x36)

NOTE: WHERE FLOOR JOIST RUNS PARALLEL TO FDN WALL, SOLID BLOCK OUTSIDE 3 JOIST SPACES @ 36" OC ALIGNING BLOCKING WITH THE ANCHOR BOLT

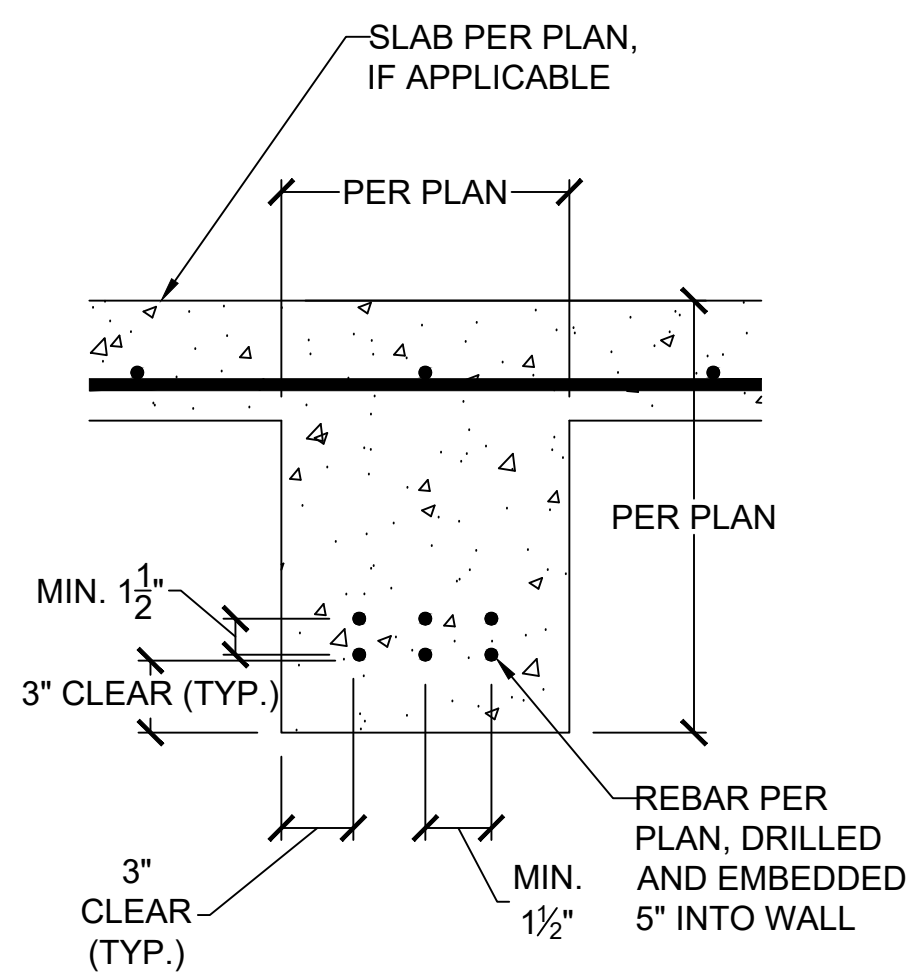


7 RETURN WALL DETAIL
S2.0 SCALE: 1/2" = 1'-0" (18x24) OR 3/4" = 1'-0" (24x36)

VERTICAL REINFORCEMENT SPACING						
CONCRETE STRENGTH/GRADE REINFORCEMENT (#4 BARS)	8" THICK WALL			10" THICK WALL		
	8'	9'	10'	8'	9'	10'
3,000 PSI/ GRADE 40	24	24	16	24	24	18
3,500 PSI/ GRADE 40	24	24	16	24	24	18
3,000 PSI/ GRADE 60	24	24	16	24	24	18
3,500 PSI/ GRADE 60	24	24	16	24	24	18
HORIZONTAL REINFORCEMENT - MINIMUM GRADE 40 STEEL						
ONE BAR 12" FROM TOP OF WALL; MAX. SPACING 24" OC	4-#4	5-#4	6-#4	4-#4	5-#4	6-#4

- FOOTNOTES:
- 1) WALL HEIGHT IS MEASURED FROM THE TOP OF THE WALL TO THE TOP OF THE FLOOR SLAB
 - 2) VERTICAL REINFORCEMENT FOR CONCRETE WALLS THAT ARE NOT FULL HEIGHT, AND FOR REINFORCEMENT SPACING 24" OC, REINFORCEMENT MAY BE PLACED IN THE MIDDLE OF THE WALL. OTHER WALLS SHALL HAVE VERTICAL REINFORCEMENT AS FOLLOWS:
 - A) 8" WALL - MINIMUM 5" FROM THE OUTSIDE FACE
 - B) 10" WALL - MINIMUM 6 3/4" FROM THE OUTSIDE FACE
 - C) EXTEND BARS TO WITHIN 8" OF THE TOP OF THE WALL
 - 3) REINFORCEMENT CLEARANCES:
 - A) CONCRETE EXPOSED TO EARTH - MINIMUM 1 1/2"
 - B) NOT EXPOSED TO WEATHER (INTERIOR SIDE OF WALLS) - 3/4"
 - C) CONCRETE EXPOSED TO WEATHER (TOP CLEARANCE IN GARAGE AND DRIVEWAY SLABS) - 1 1/2"
 - 4) HORIZONTAL REINFORCEMENT:
 - A) ONE BAR SHALL BE PLACED WITHIN 12" OF THE TOP OF THE WALL
 - B) OTHER BARS SHALL BE EQUALLY SPACED WITH SPACING NOT TO EXCEED 24" OC
 - C) HORIZONTAL BARS SHOULD BE AS CLOSE TO THE TENSION FACE AS POSSIBLE (INTERIOR) AND BEHIND THE VERTICAL REINFORCEMENT (I.E. 2" TOWARD THE INSIDE)
 - D) SUPPLEMENTAL REINFORCEMENT AT CORNERS - PLACE (1) #4 BAR 48" LONG AT 45 DEGREE ANGLE AT CORNERS OF OPENINGS. PLACE REINFORCEMENT WITHIN 6" OF THE EDGE OF INSIDE CORNERS.
 - 5) REINFORCEMENT SHALL BE LAPPED A MINIMUM 24" AT ENDS, SPLICES, AND AROUND CORNERS.
 - 6) AT MASONRY LEDGES THE MINIMUM WALL THICKNESS SHALL BE 3 1/2". LEDGES SHALL NOT EXCEED A DEPTH OF MORE THAN 24" BELOW THE TOP OF THE WALL. FOR WALL THICKNESSES LESS THAN 4" PROVIDE #4 BARS AT MAX. 24" OC TO WITHIN 8" OF THE TOP OF THE WALL.
 - 7) STRAIGHT WALLS MORE THAN 5' TALL AND MORE THAN 16 FEET LONG SHALL BE PROVIDED WITH EXTERIOR BRACED RETURN WALLS. WALL LENGTH SHALL BE MEASURED USING INSIDE THE SHORTEST DIMENSION BETWEEN INTERSECTING WALLS

4 FOUNDATION WALL REINFORCEMENT TABLE
S2.0 NO SCALE



8 CONCRETE GRADE BEAM
S2.0 SCALE: 1" = 1'-0" (18x24) OR 1 1/2" = 1'-0" (24x36)



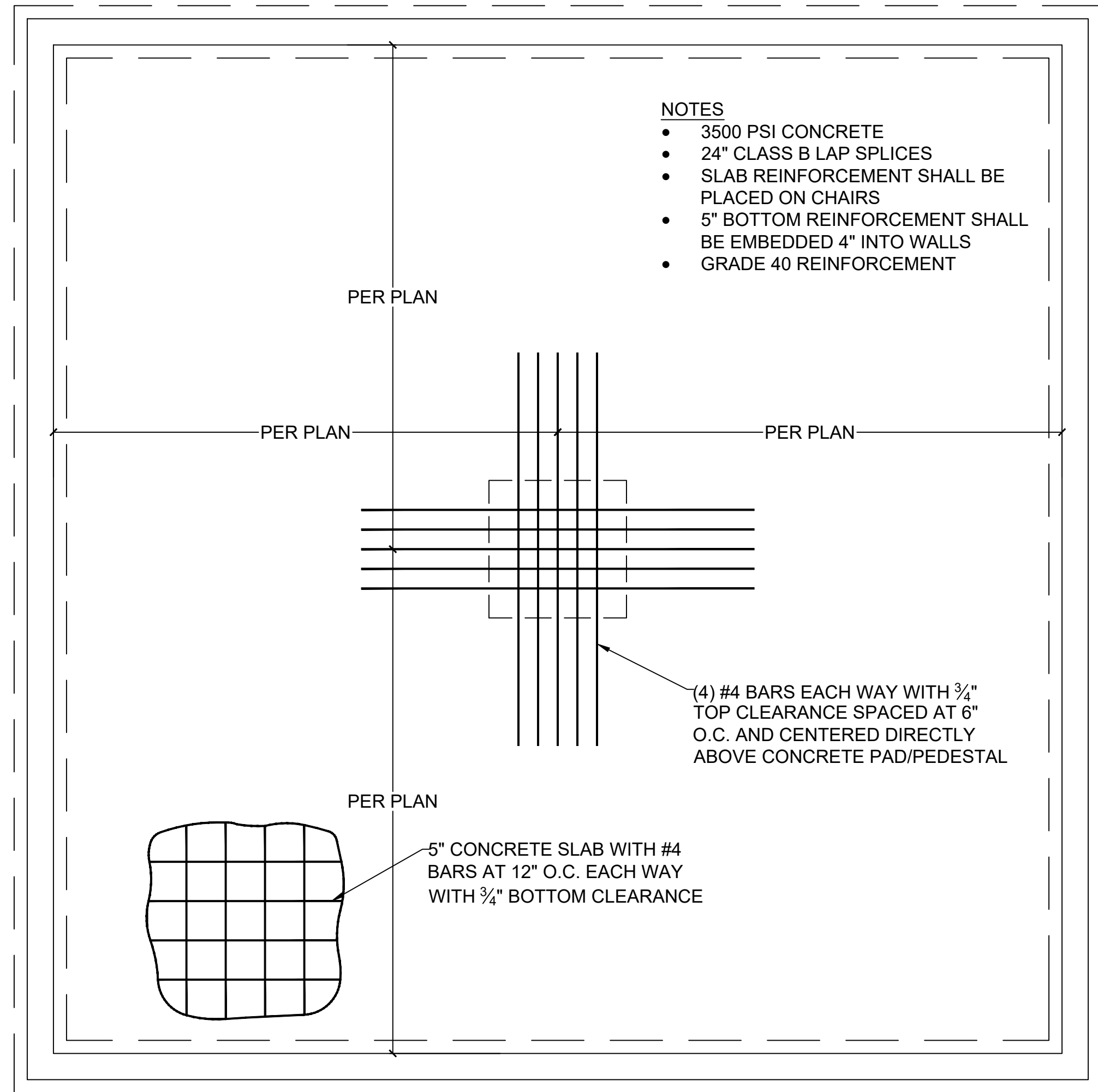
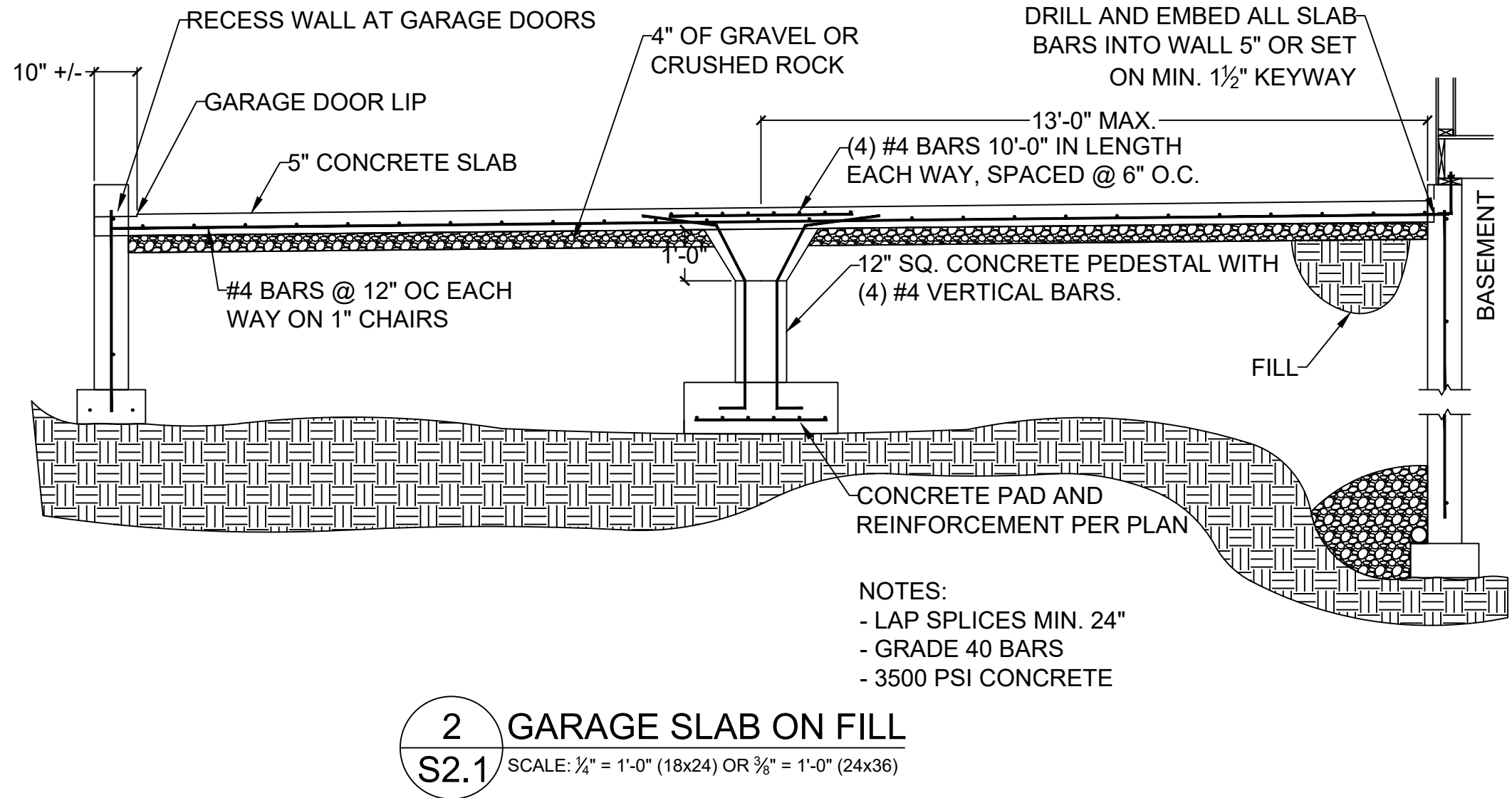
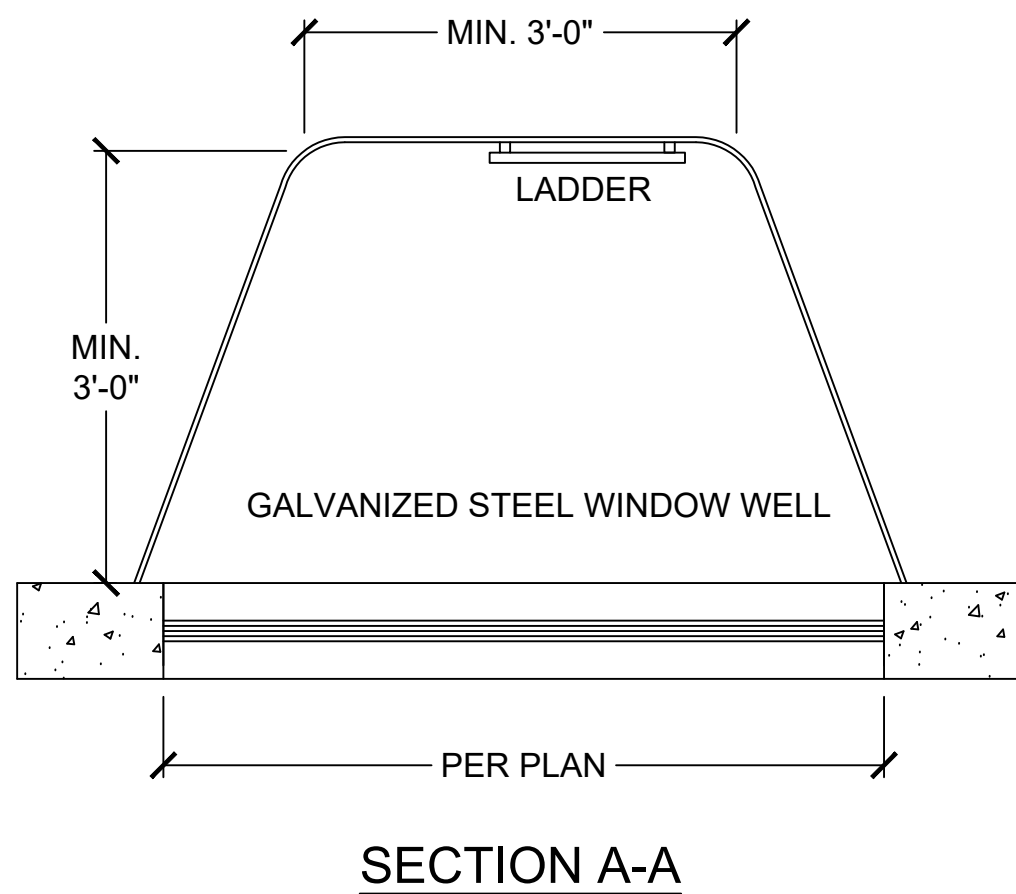
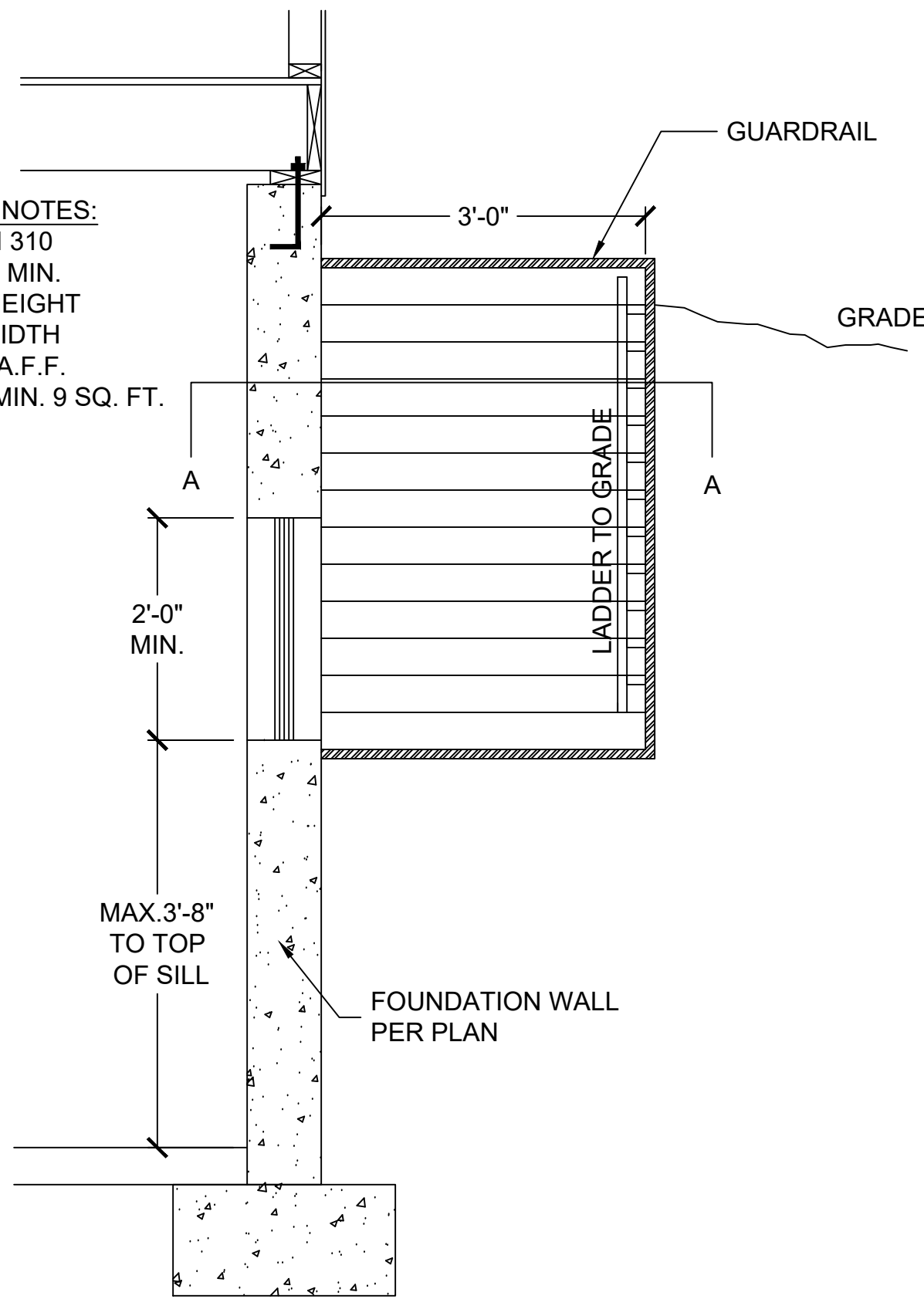
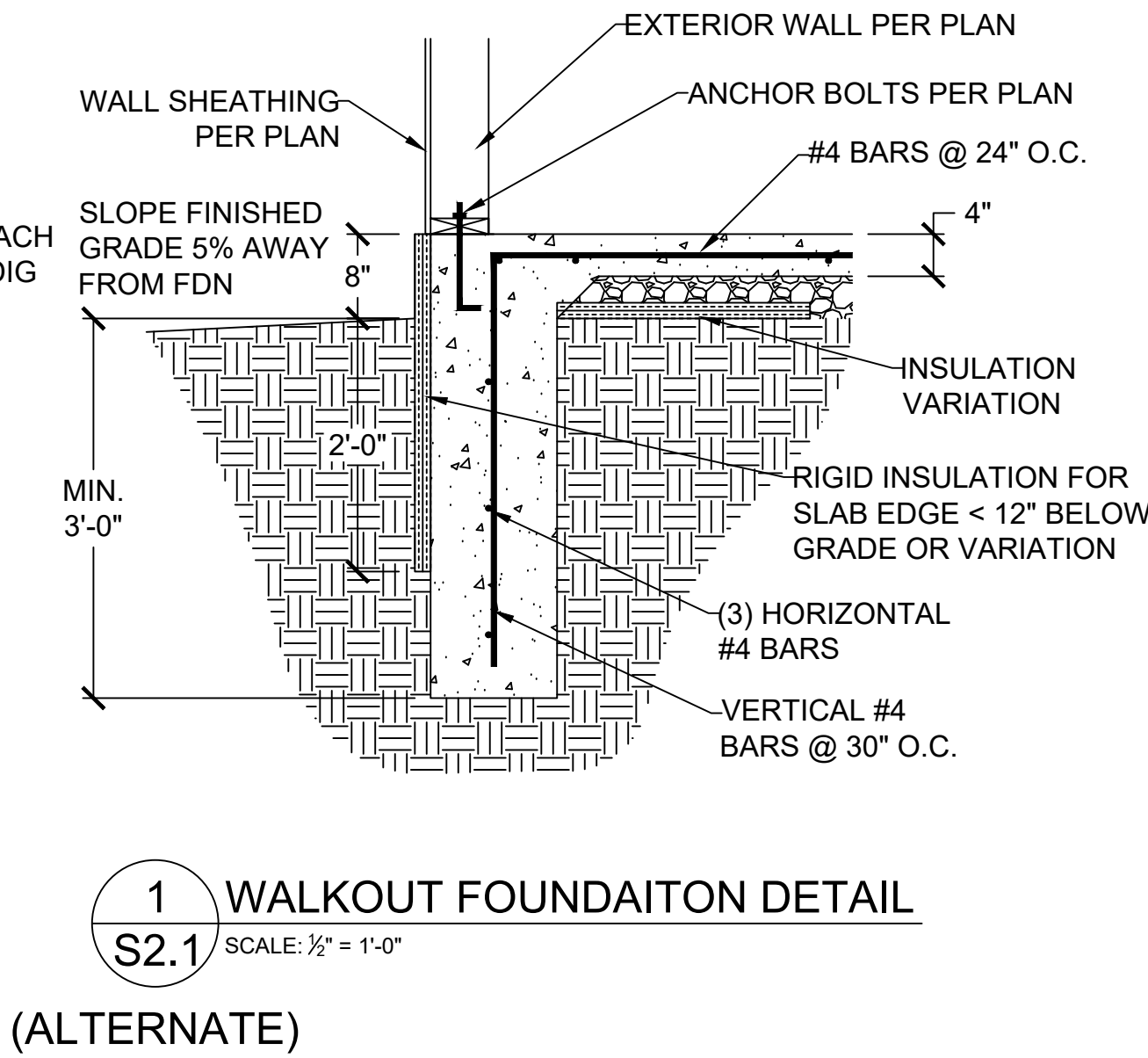
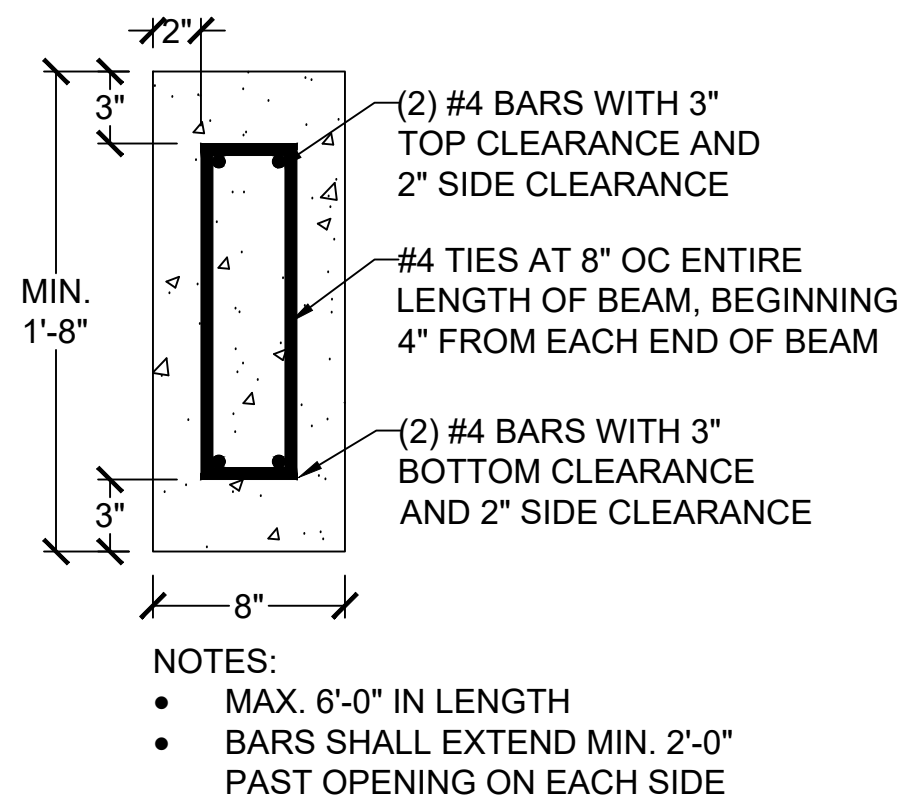
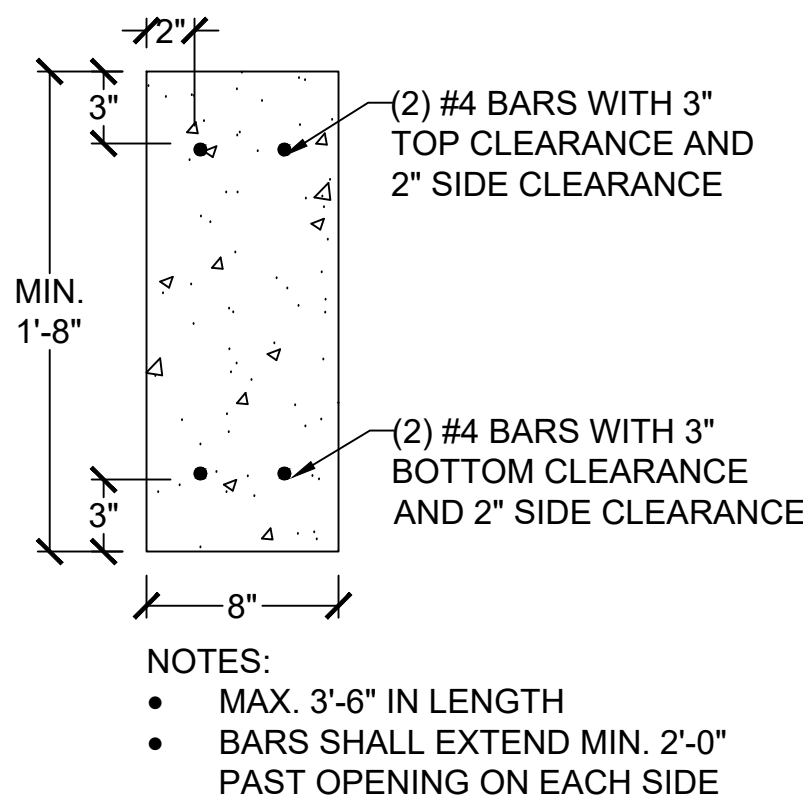
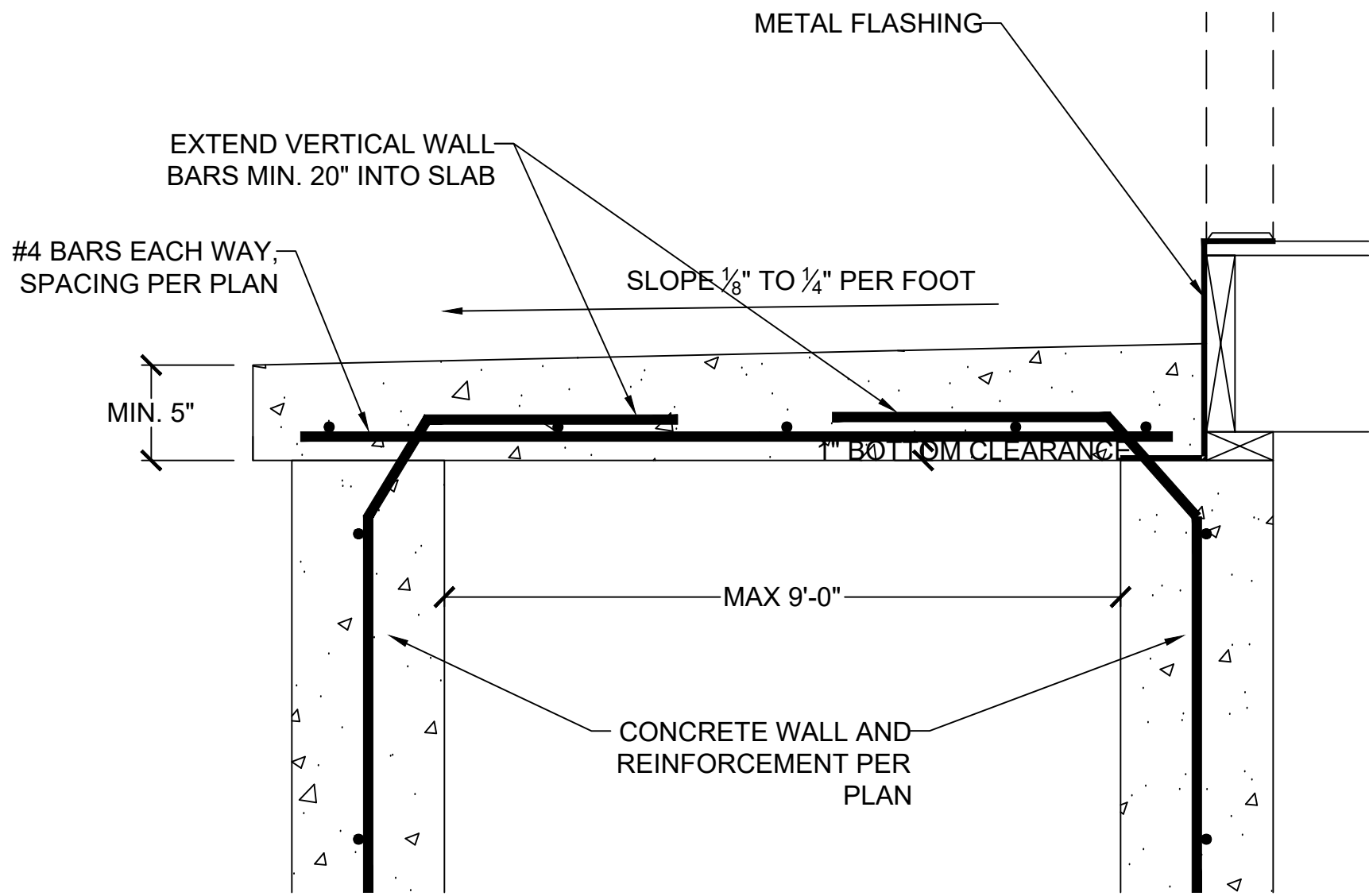
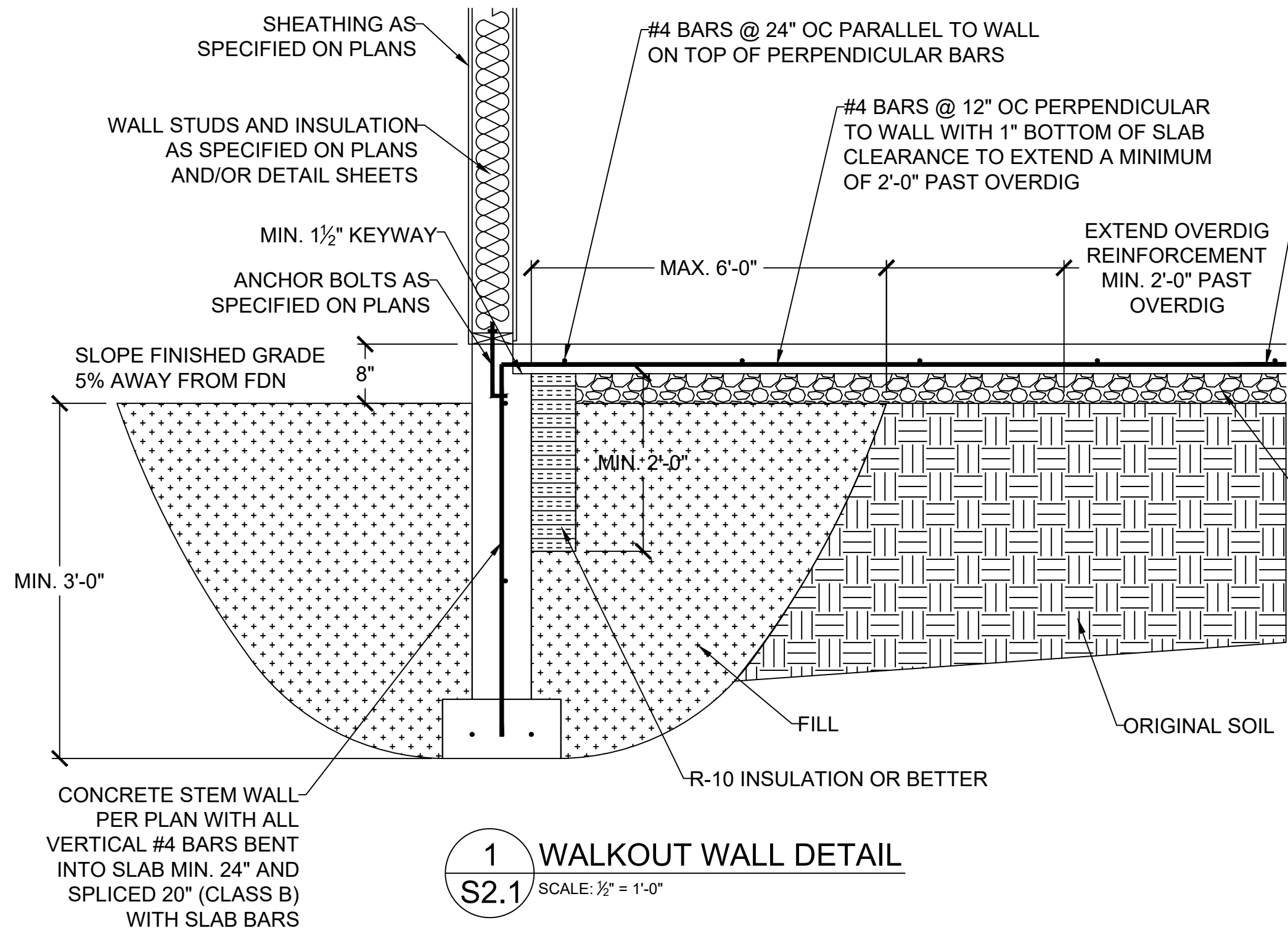
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CLIENT: KEVIN HIGDON CONSTRUCTION
JOB TITLE: TCR011 TRIPLEX
LOT 111, THE TOWNHOMES OF CHAPEL RIDGE
2ND PLAT
LOCATION: 805, 807, 809 NE ALCONQUIN ST.
LEE'S SUMMIT, MISSOURI



NO.	DATE	REVISION	BY
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FOUNDATION DETAILS			
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DATE: 12-14-21			
SHEET NUMBER			

S2.0

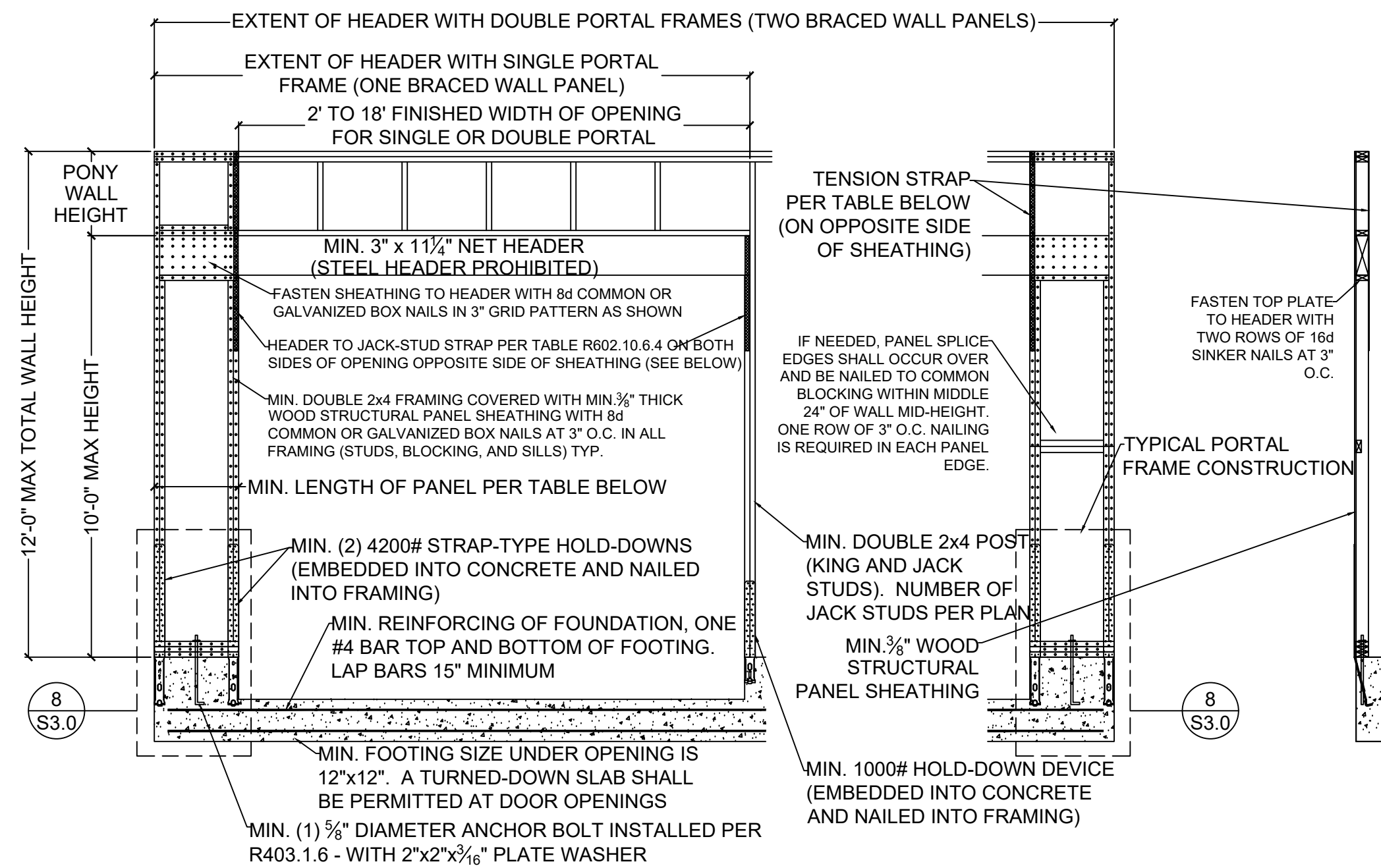


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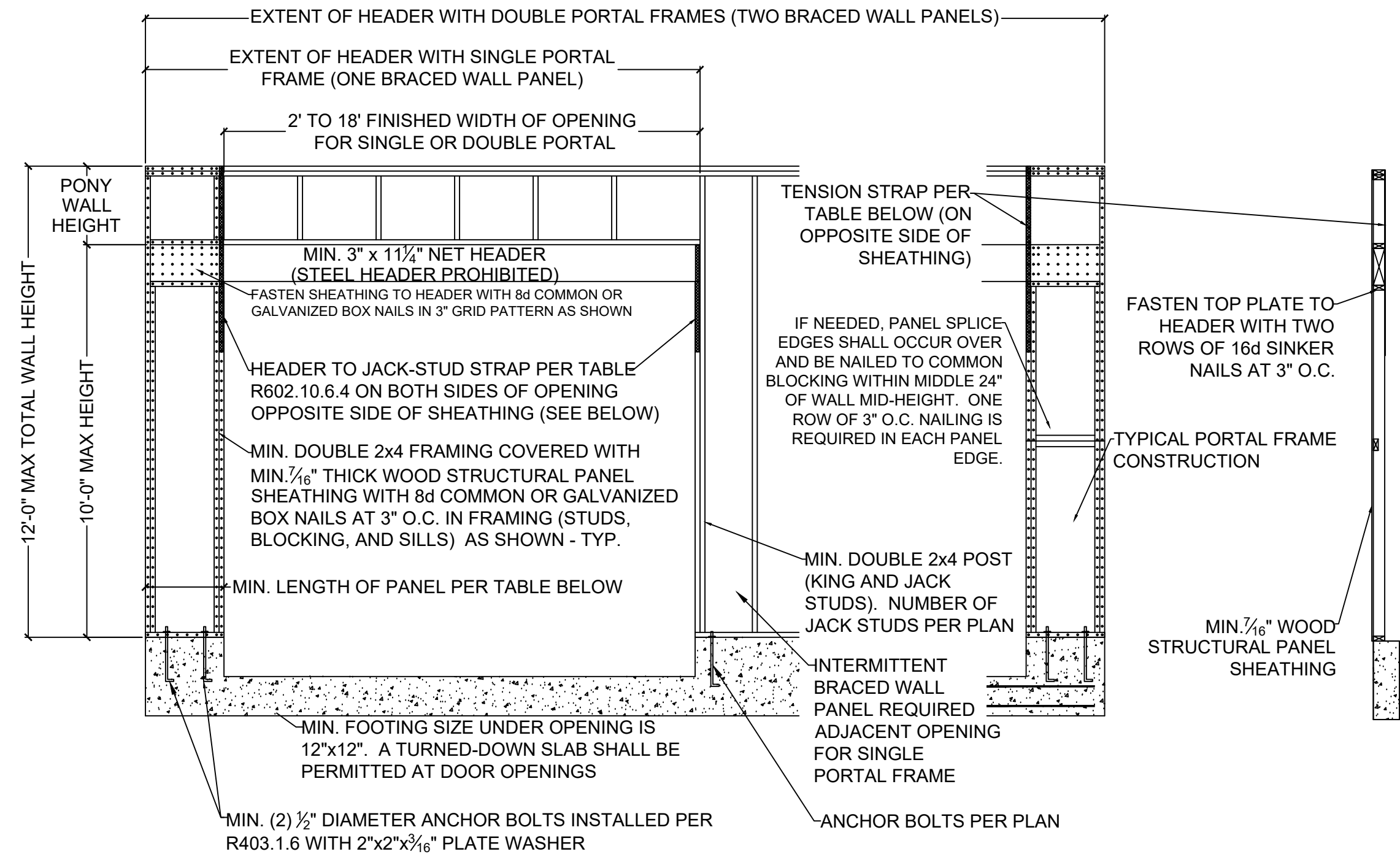
S2.1



1 METHOD PFH (PORTAL FRAME WITH S3.0 HOLD-DOWNS) - PER FIGURE IRC R602.10.6.2

SCALE: 1/2" = 1'-0" (18x24) OR 3/4" = 1'-0" (24x36)

	MINIMUM PANEL LENGTH FOR DETAIL 1/S3.0 (INCHES)				
	WALL HEIGHT				
	8 FEET	9 FEET	10 FEET	11 FEET	12 FEET
SUPPORTING ROOF ONLY	16	16	16	18	20
SUPPORTING ONE STORY AND ROOF	24	24	24	27	29

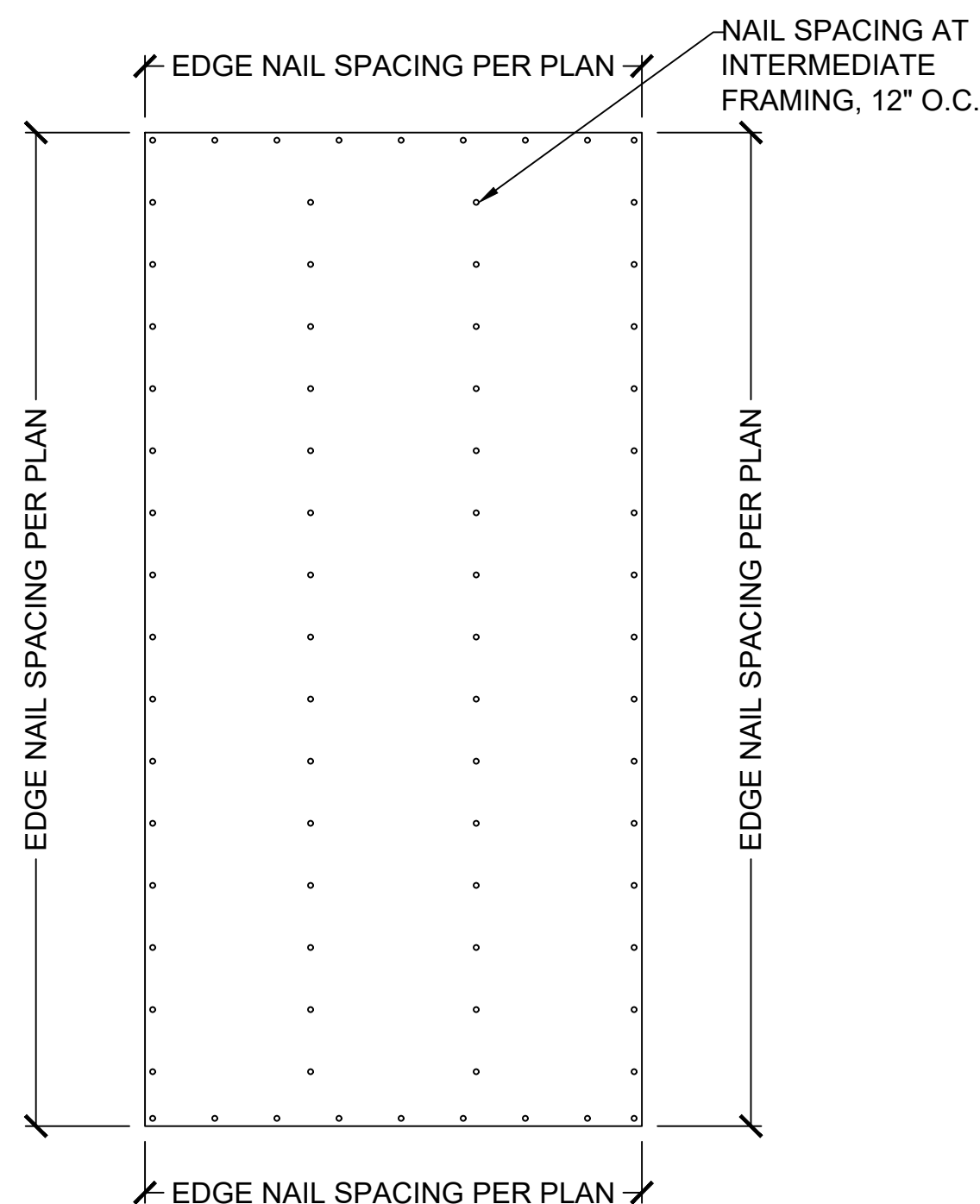


2 METHOD PFG (PORTAL FRAME AT GARAGE S3.0 DOOR) - PER FIGURE IRC R602.10.6.3

SCALE: 3/4" = 1'-0" (18x24) OR 5/8" = 1'-0" (24x36)

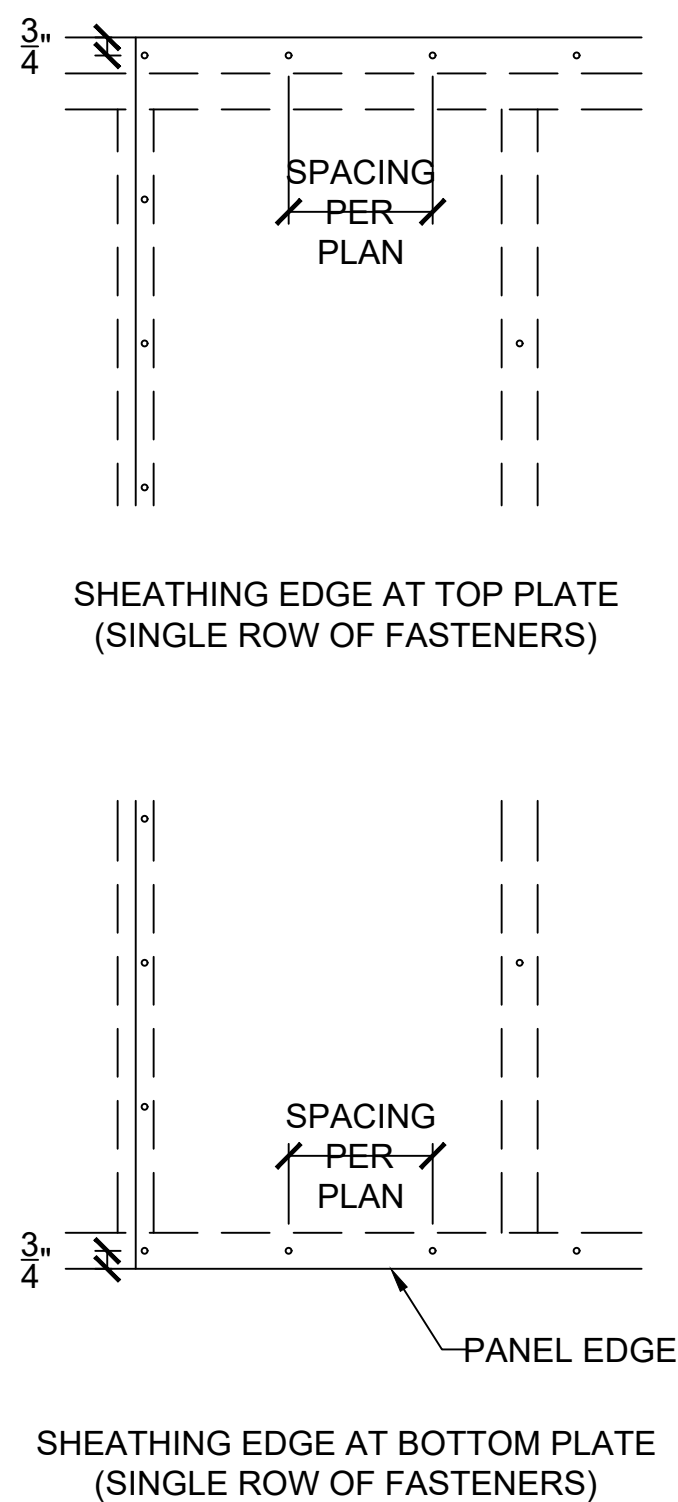
MINIMUM PANEL LENGTH FOR DETAIL 2/S3.0 (INCHES)				
WALL HEIGHT				
8 FEET	9 FEET	10 FEET	11 FEET	12 FEET
24	27	30	33 ^a	36 ^a

a. Maximum opening height for PFG is 10 feet in accordance with Figure R602.10.6.3, but wall height may be increased to 12 feet with pony wall



3 EXTERIOR WALL SHEATHING S3.0 PANEL ATTACHMENT

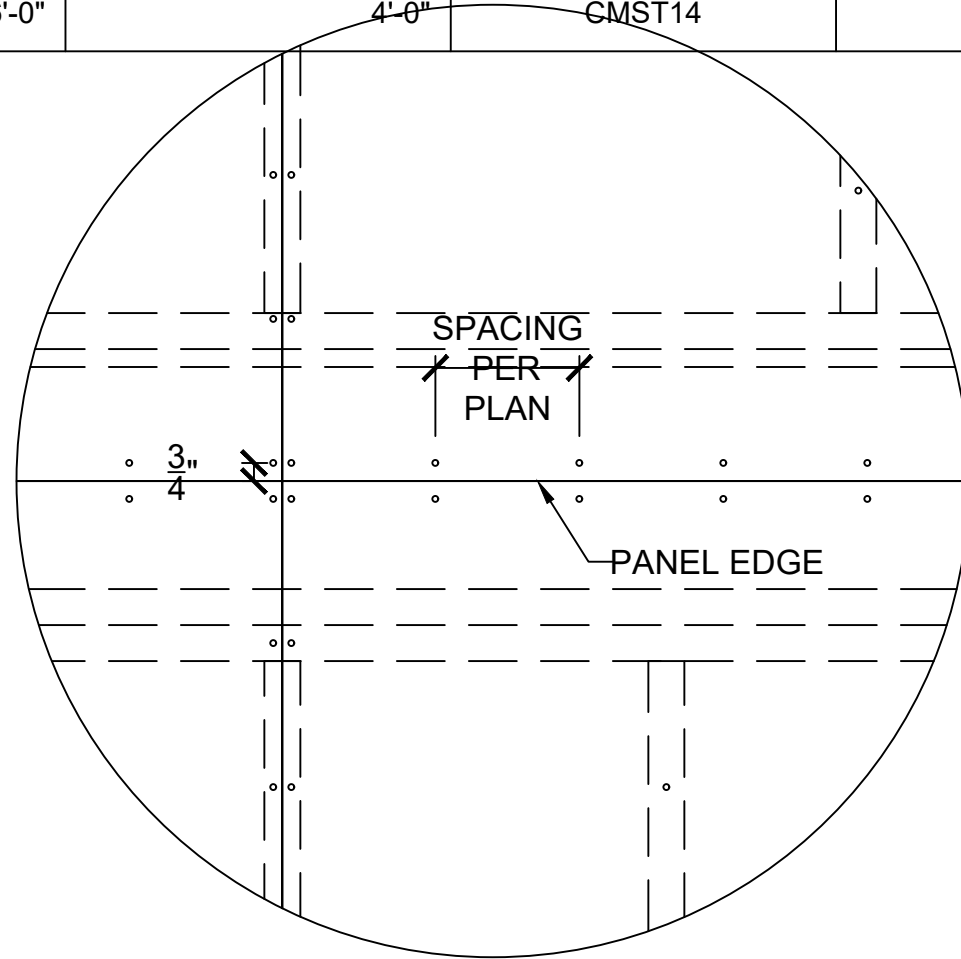
SCALE: 1/2" = 1'-0" (18x24) OR 3/4" = 1'-0" (24x36)



4 SHEATHING EDGE AT TOP S3.0 AND BOTTOM PLATES

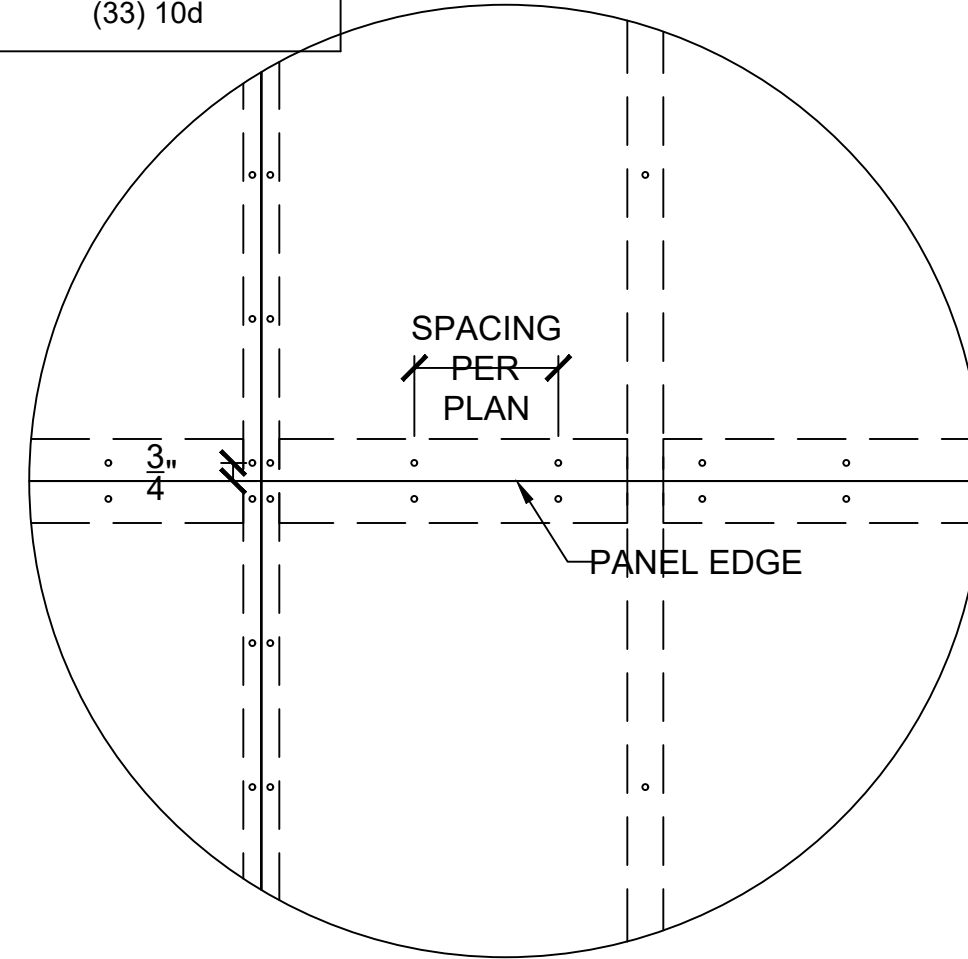
SCALE: 1" = 1'-0" (18x24) OR 1 1/2" = 1'-0" (24x36)

TENSION STRAP REQUIRED FOR HEADER TO JACK STUD FOR DETAILS 1/S3.0 AND 2/S3.0 (FROM TABLE R602.10.6.4)				
MAX GARAGE OPENING (FT.)	PONY WALL WALL HT. (FT.)	REQUIRED SIMPSON STRAP	MIN. STRAP END LENGTH	NAILS REQUIRED IN EACH STRAP END LENGTH
18'-0"	0'-0"	CS20	0'-9"	(7) 8d
9'-0"	1'-0"	CS20	0'-9"	(7) 8d
18'-0"	1'-0"	CS14	1'-4"	(15) 8d
9'-0"	2'-0"	CS18	0'-11"	(9) 8d
18'-0"	2'-0"	CMSTC16	1'-8"	(25) 16d SINKER
9'-0"	4'-0"	CMSTC16	1'-8"	(25) 16d SINKER
16'-0"	4'-0"	CMST14	2'-6"	(33) 10d



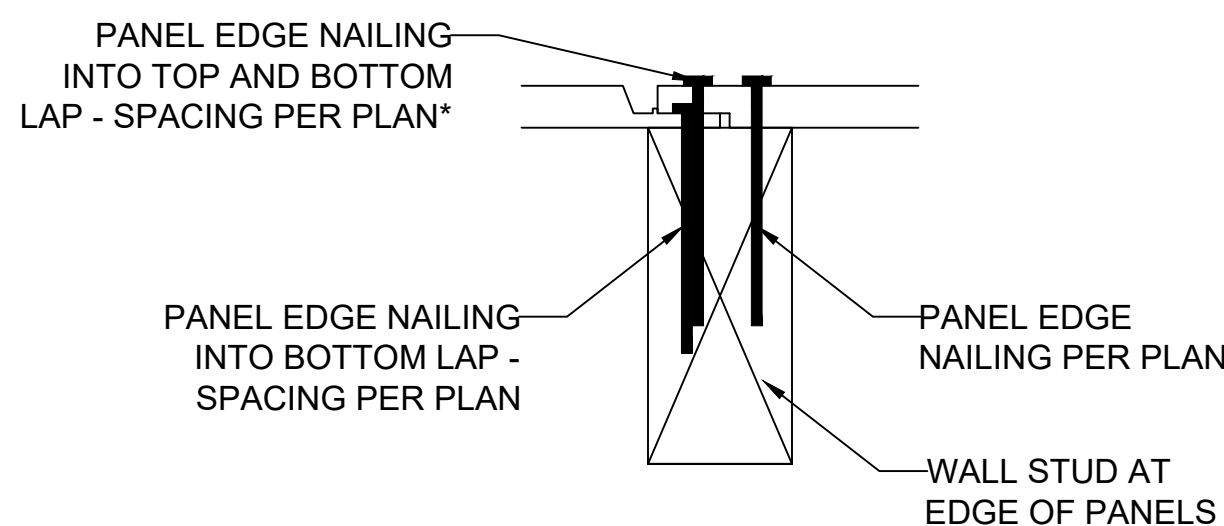
5 SHEATHING EDGE AT HORIZONTAL S3.0 FRAMING MEMBER

SCALE: 1" = 1'-0" (18x24) OR 1 1/2" = 1'-0" (24x36)



6 SHEATHING EDGE AT PANEL S3.0 SPLICE ACROSS STUDS

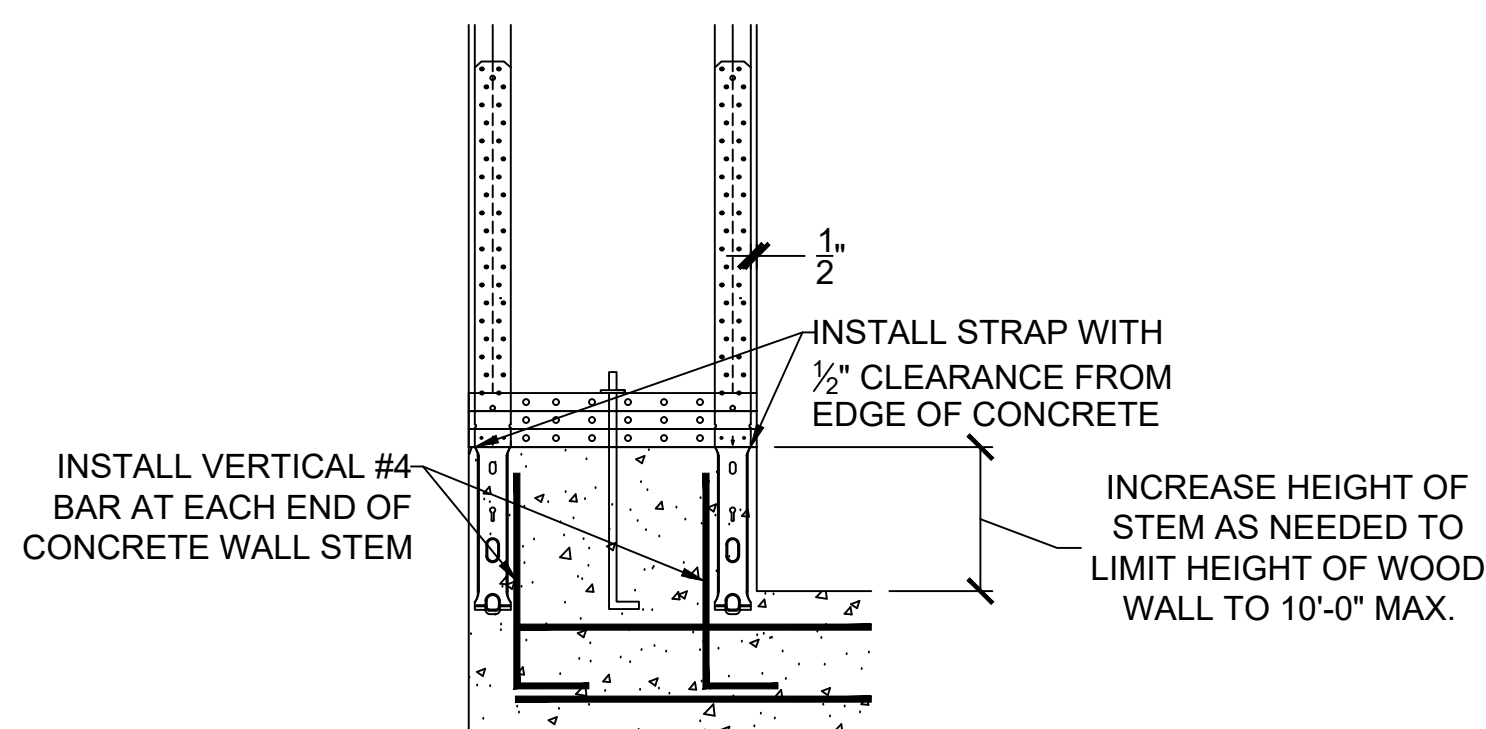
SCALE: 1" = 1'-0" (18x24) OR 1 1/2" = 1'-0" (24x36)



*NOTE: NAILING INTO TOP AND BOTTOM LAP IS IN ADDITION TO NAILING REQUIRED INTO BOTTOM LAP. FOR EXAMPLE, IF PLAN CALLS FOR NAILS @ 6" O.C. AT EDGES, BOTTOM LAP SHALL BE FASTENED AT 6" O.C AND, IN ADDITION, NAILING SHALL ALSO BE INSTALLED THROUGH TOP AND BOTTOM LAP @ 6" O.C. STAGGERED 3" FROM BOTTOM LAP NAILING

7 FASTENING INSTRUCTIONS FOR S3.0 SHIPLAP PANEL SHEATHING

SCALE: 4" = 1'-0" (18x24) OR 6" = 1'-0" (24x36)



8 GARAGE HOLD-DOWN S3.0 STRAP INSTALLATION

SCALE: 1/2" = 1'-0" (18x24) OR 3/4" = 1'-0" (24x36)



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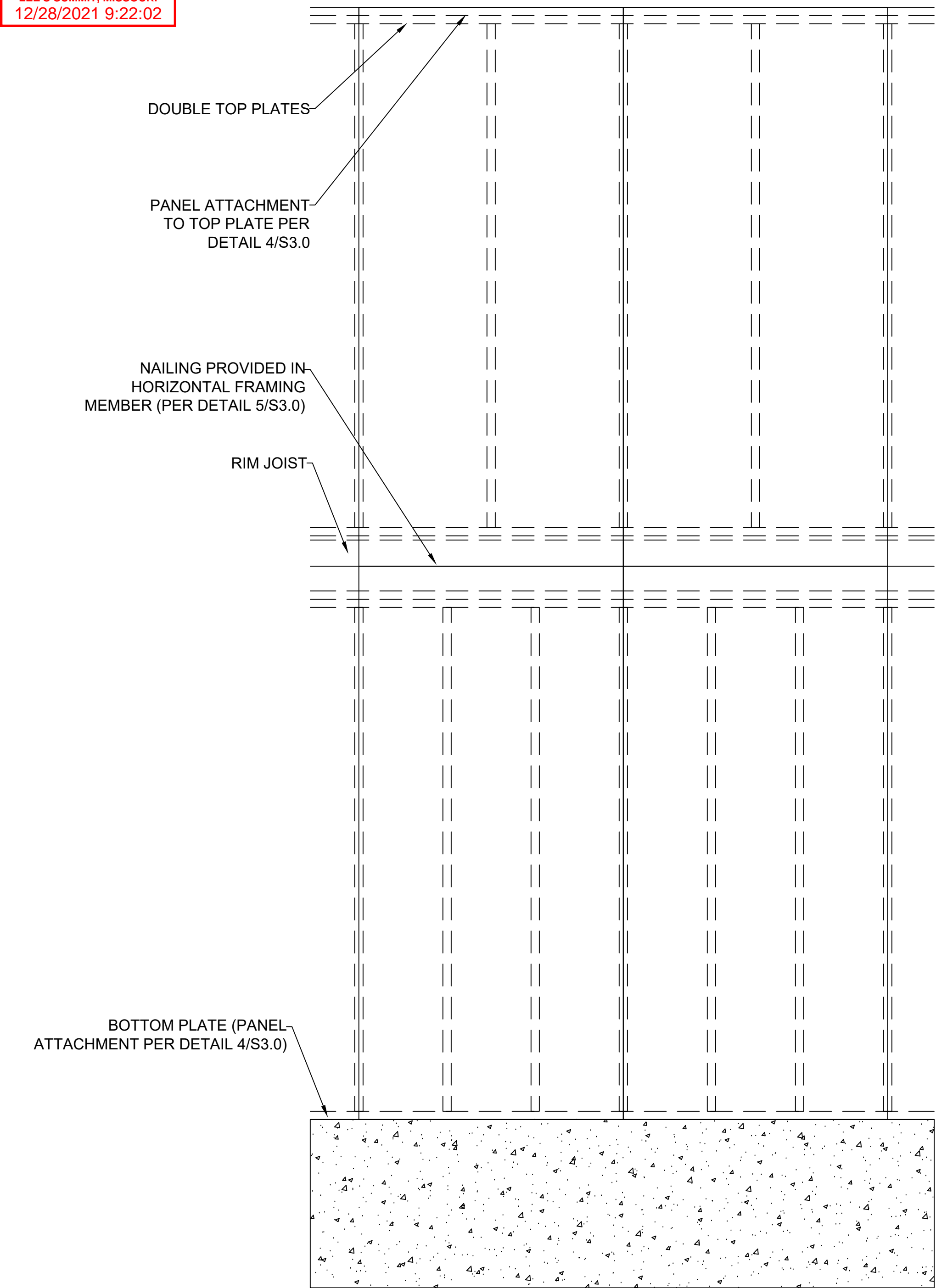
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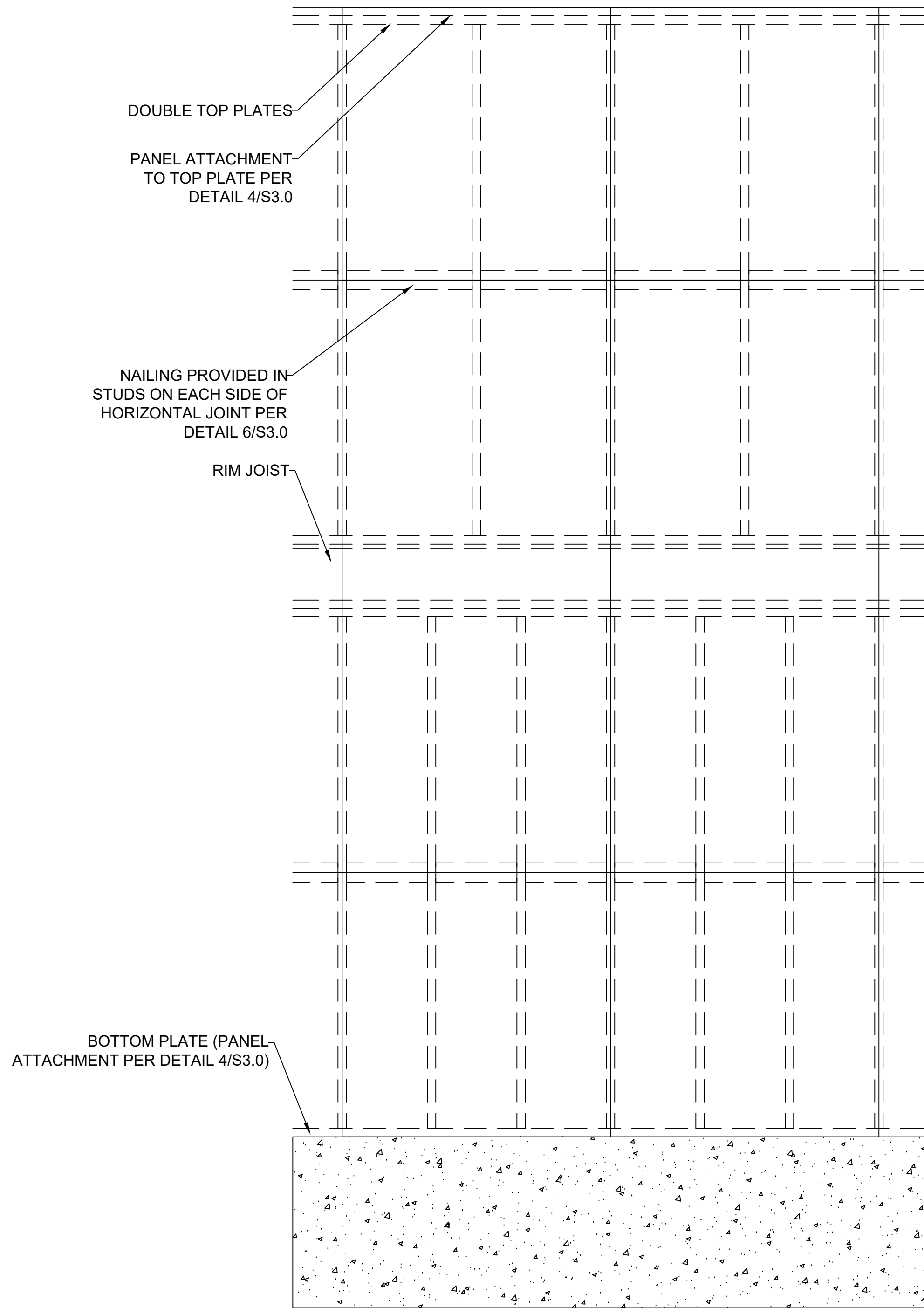


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S3.0

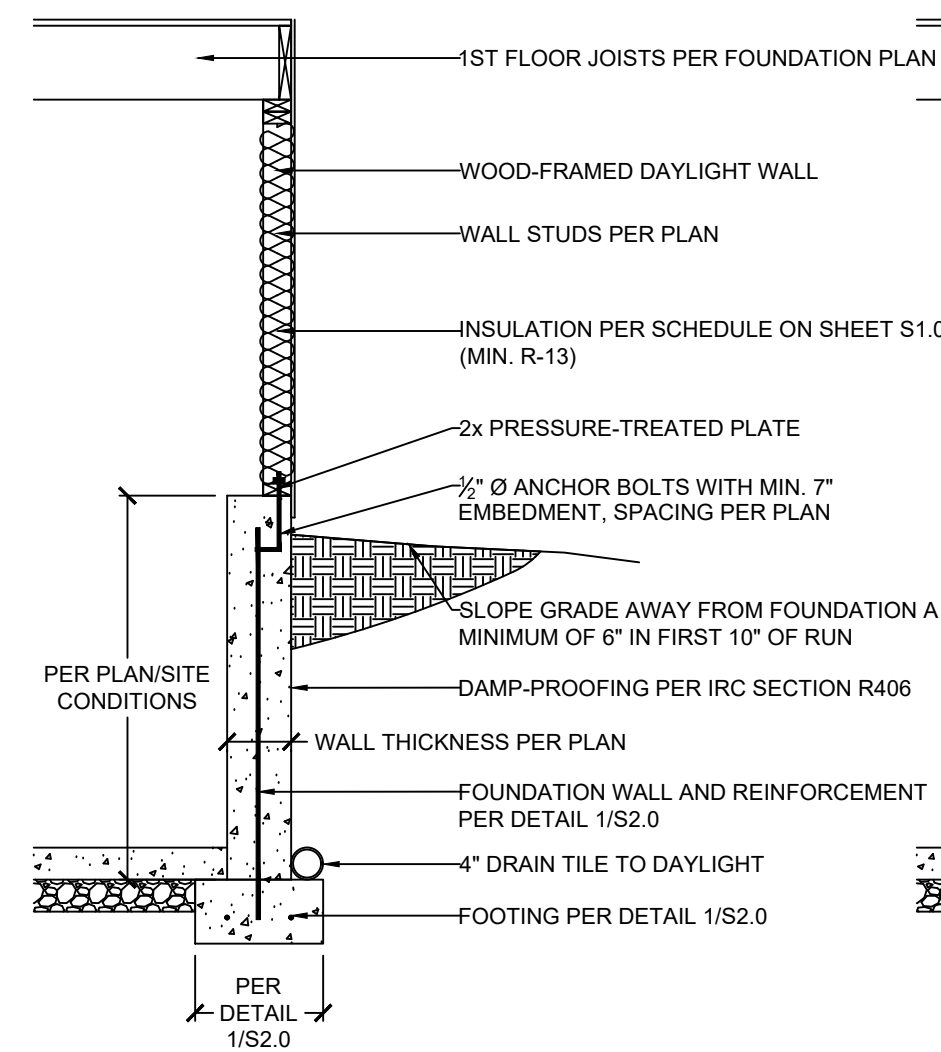


1 EXTERIOR WALL SHEATHING PANEL ATTACHMENT
S3.1 PANEL SPLICE OVER HORIZONTAL FRAMING MEMBER
SCALE: 1/2" = 1'-0" (18x24) OR 3/4" = 1'-0" (24x36)

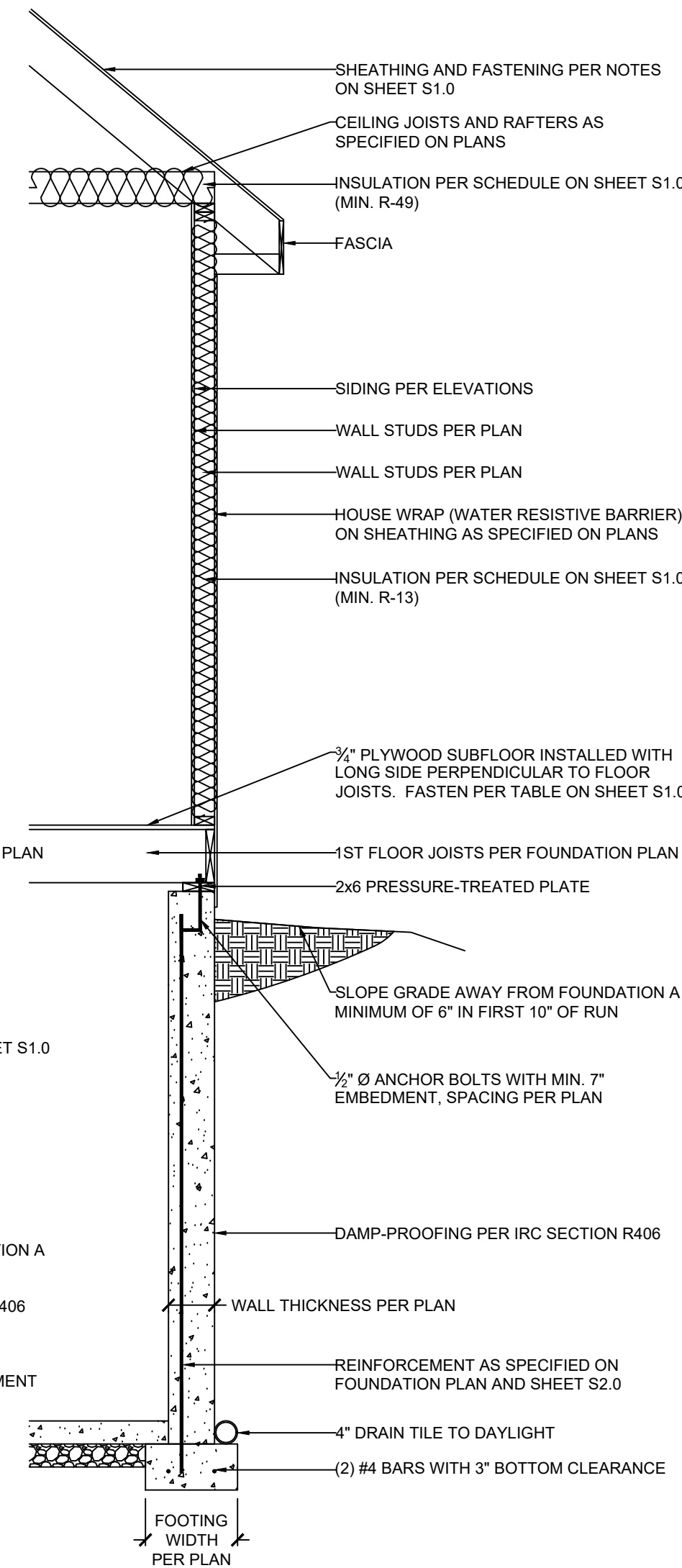


2 EXTERIOR WALL SHEATHING PANEL ATTACHMENT
S3.1 PANEL SPLICE OCCURRING ACROSS STUDS
SCALE: 1/2" = 1'-0" (18x24) OR 3/4" = 1'-0" (24x36)

3 EXTERIOR WALL SECTION
S3.1 SCALE: 1/2" = 1'-0"



DAYLIGHT BASEMENT OPTION



FULL-HEIGHT CONCRETE WALL OPTION

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LEE'S SUMMIT, MISSOURI

STATE OF MISSOURI

DENNIS HEIER

NUMBER: PE-201001772

PROFESSIONAL ENGINEER

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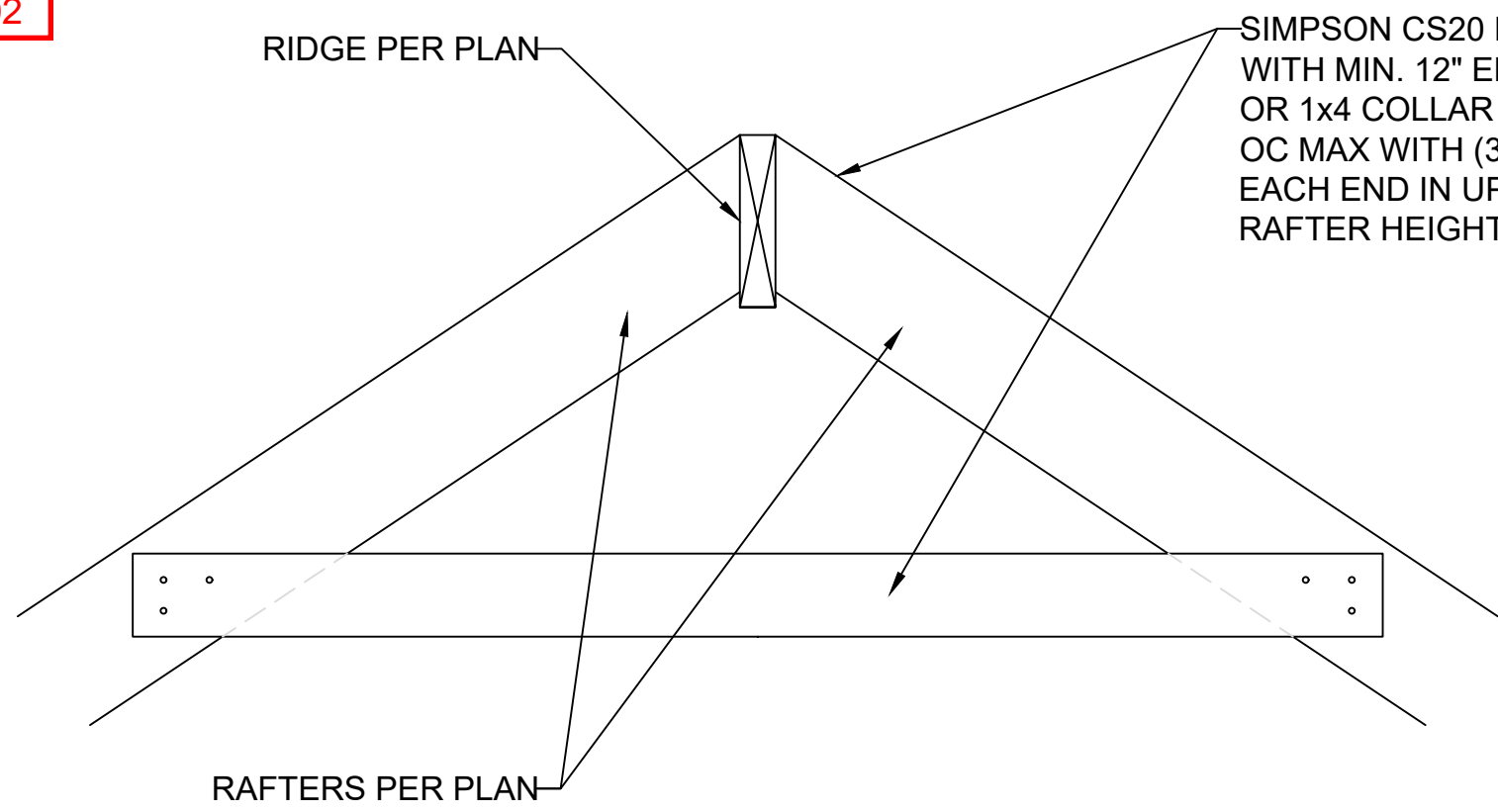
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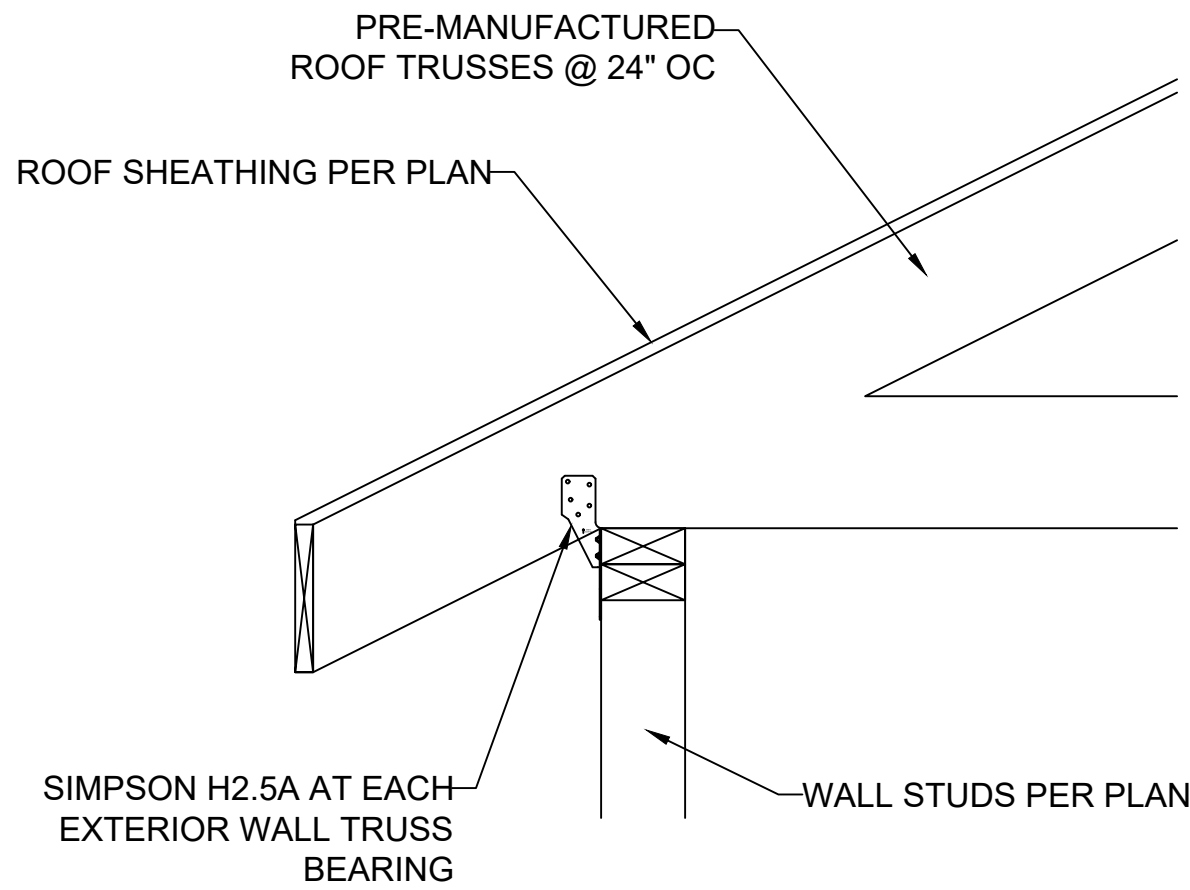
FRAMING
DETAILS

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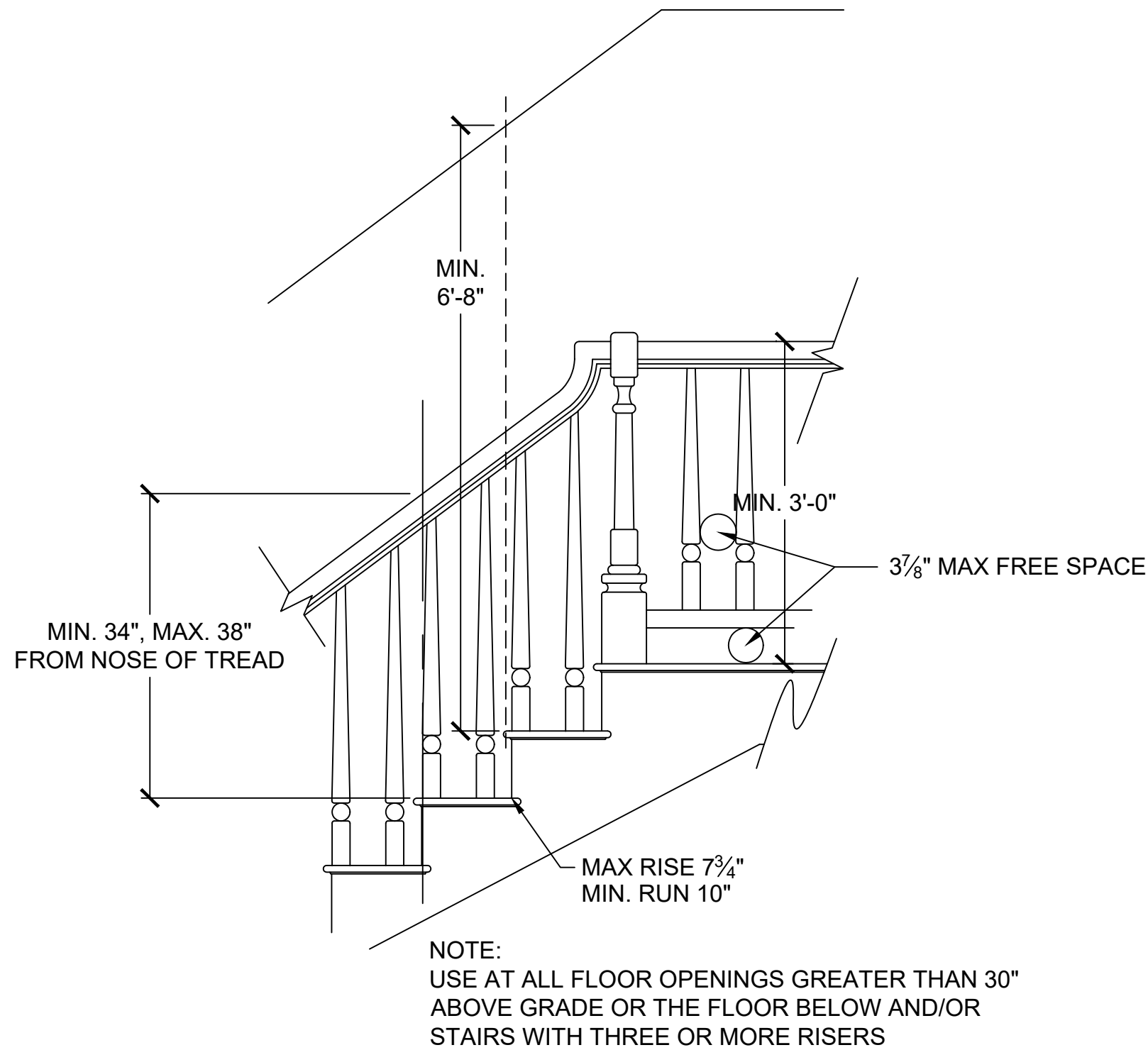
S3.1



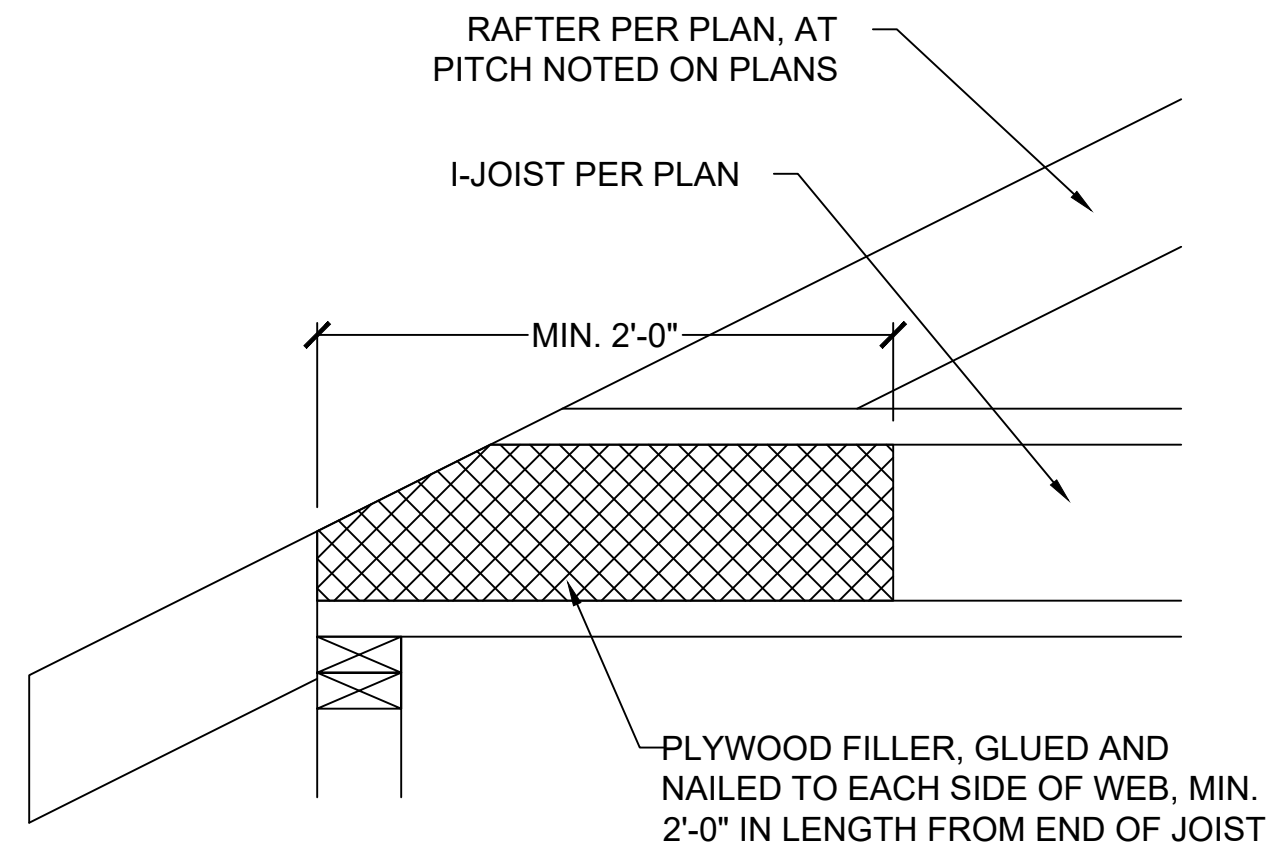
1 RIDGE FRAMING DETAIL
S3.2 SCALE: 1" = 1'-0" (18x24) OR 1/2" = 1'-0" (24x36)



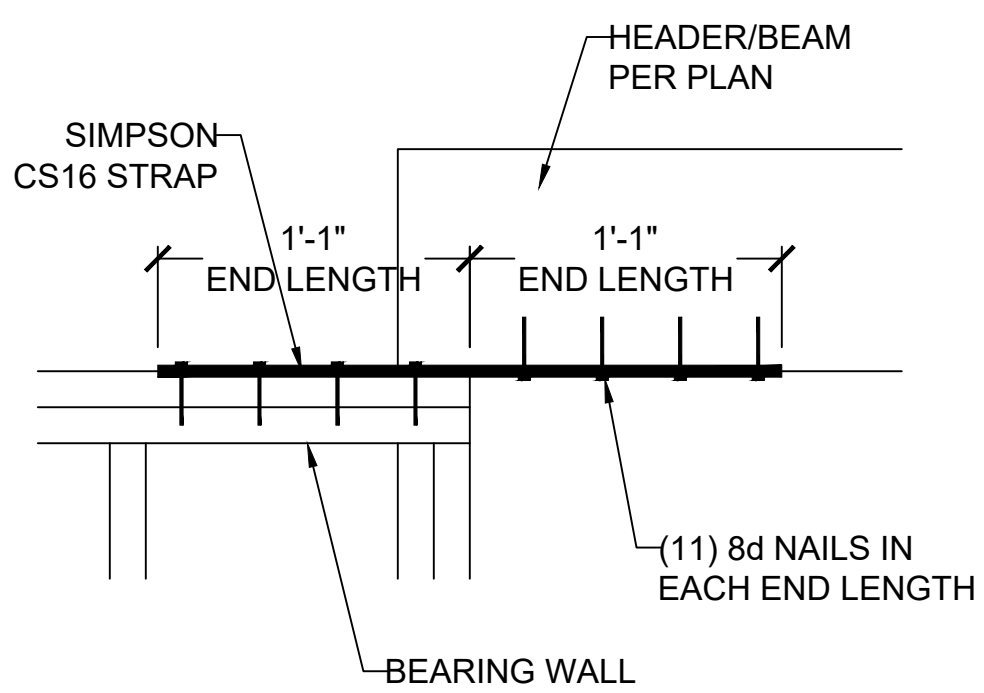
2 TRUSS CONNECTION TO EXT. WALL BEARING
S3.2 SCALE: 1" = 1'-0" (18x24) OR 1/2" = 1'-0" (24x36)



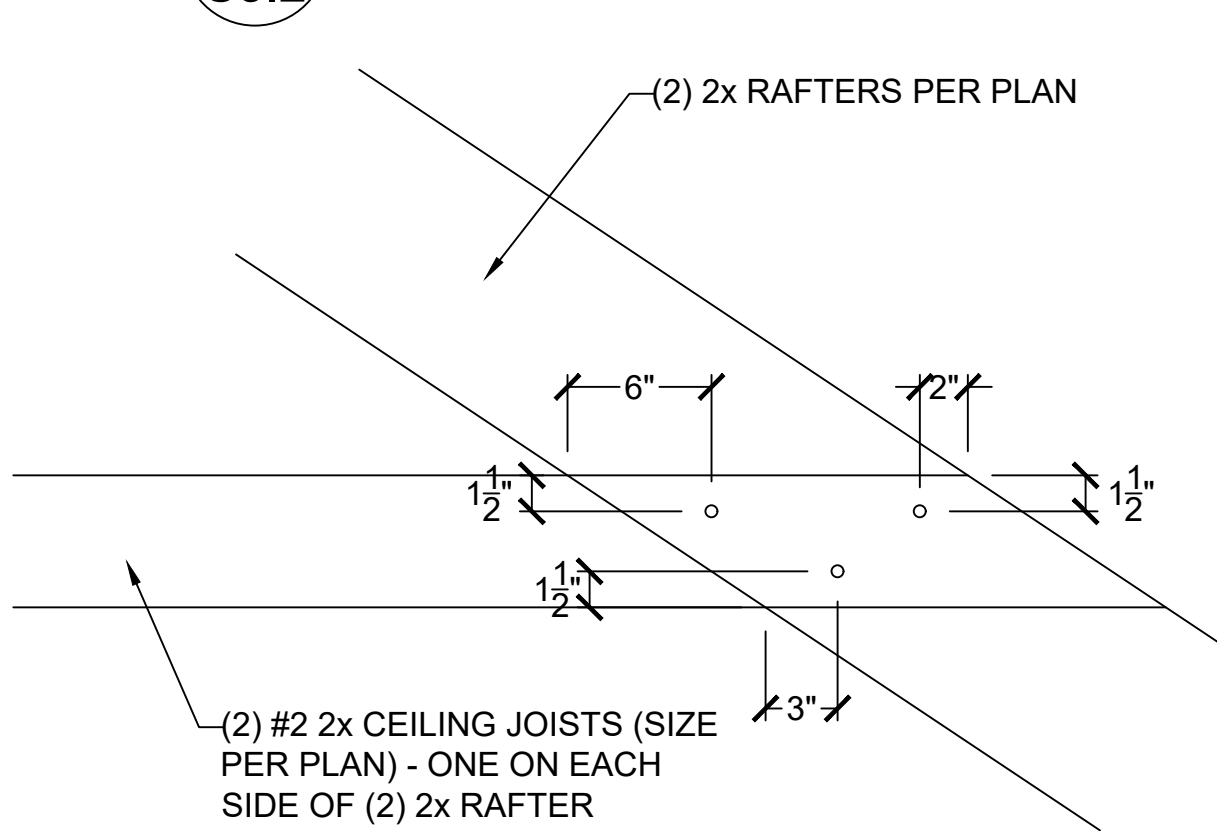
4 STAIR AND HANDRAIL/GUARDRAIL DETAIL
S3.2 SCALE: 1/2" = 1'-0" (18x24) OR 3/4" = 1'-0" (24x36)



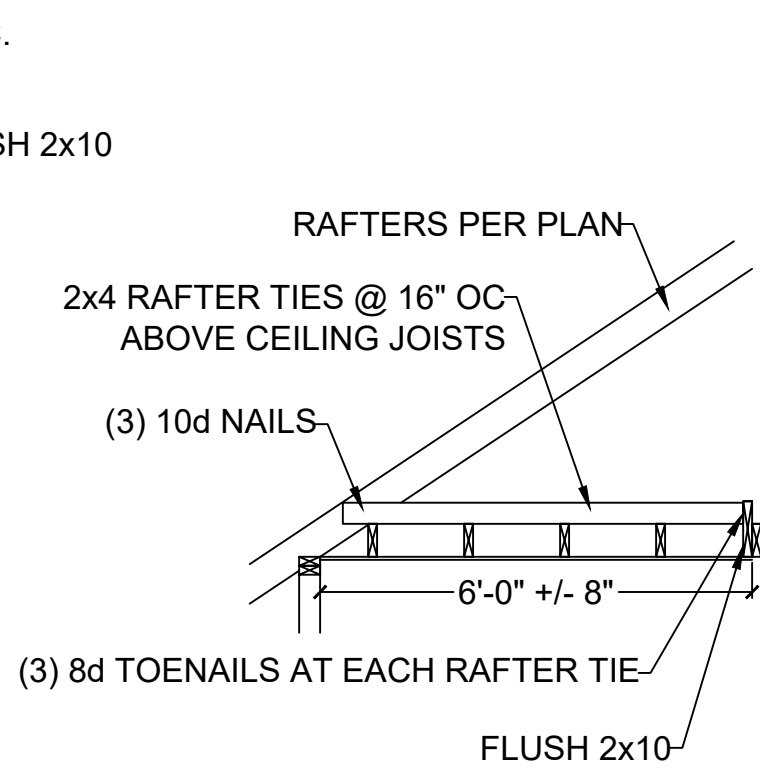
9 COPED I-JOIST REINFORCEMENT
S3.2 SCALE: 1" = 1'-0" (18x24) OR 1/2" = 1'-0" (24x36)



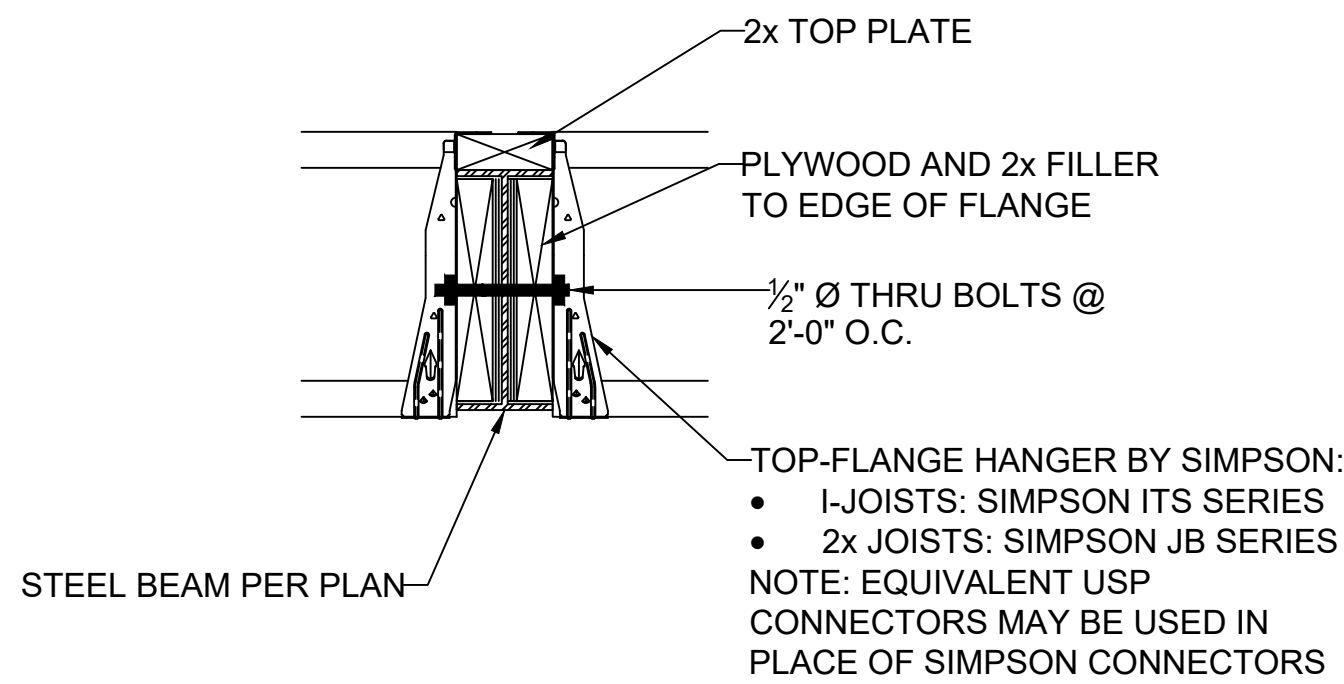
10 HEADER/BEAM CONNECTION OPTIONS AT OUTDOOR/OPEN SPACE
S3.2 SCALE: 1" = 1'-0" (18x24) OR 1/2" = 1'-0" (24x36)



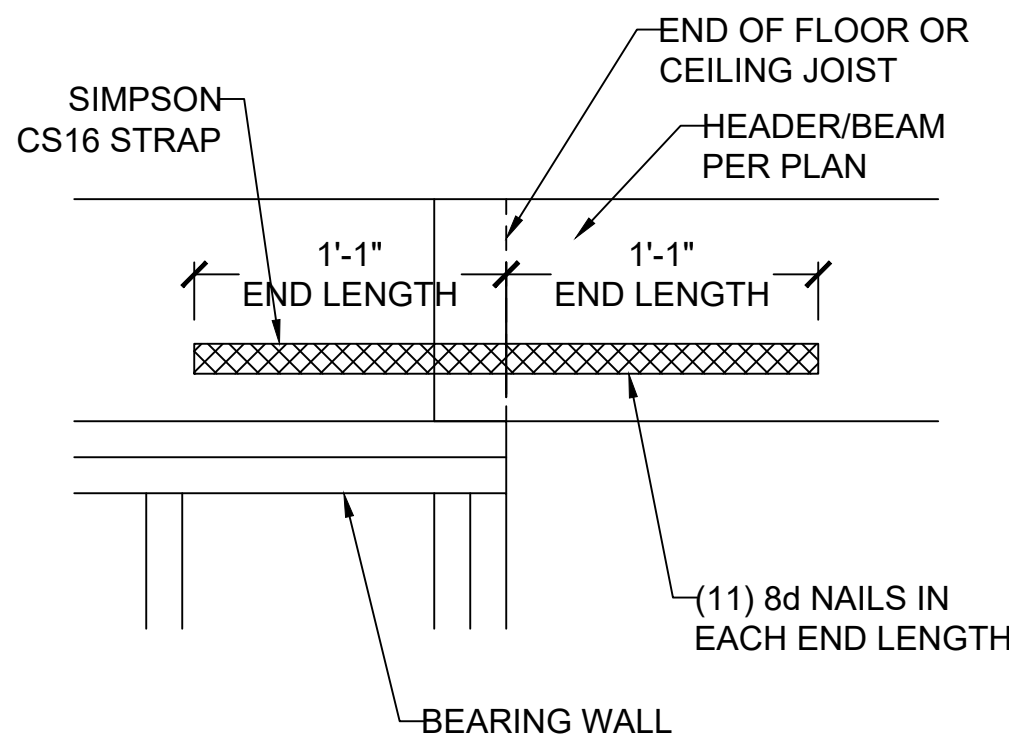
6 FIELD-CONSTRUCTED A-FRAME DETAIL
S3.2 SCALE: 1" = 1'-0" (18x24) OR 1/2" = 1'-0" (24x36)



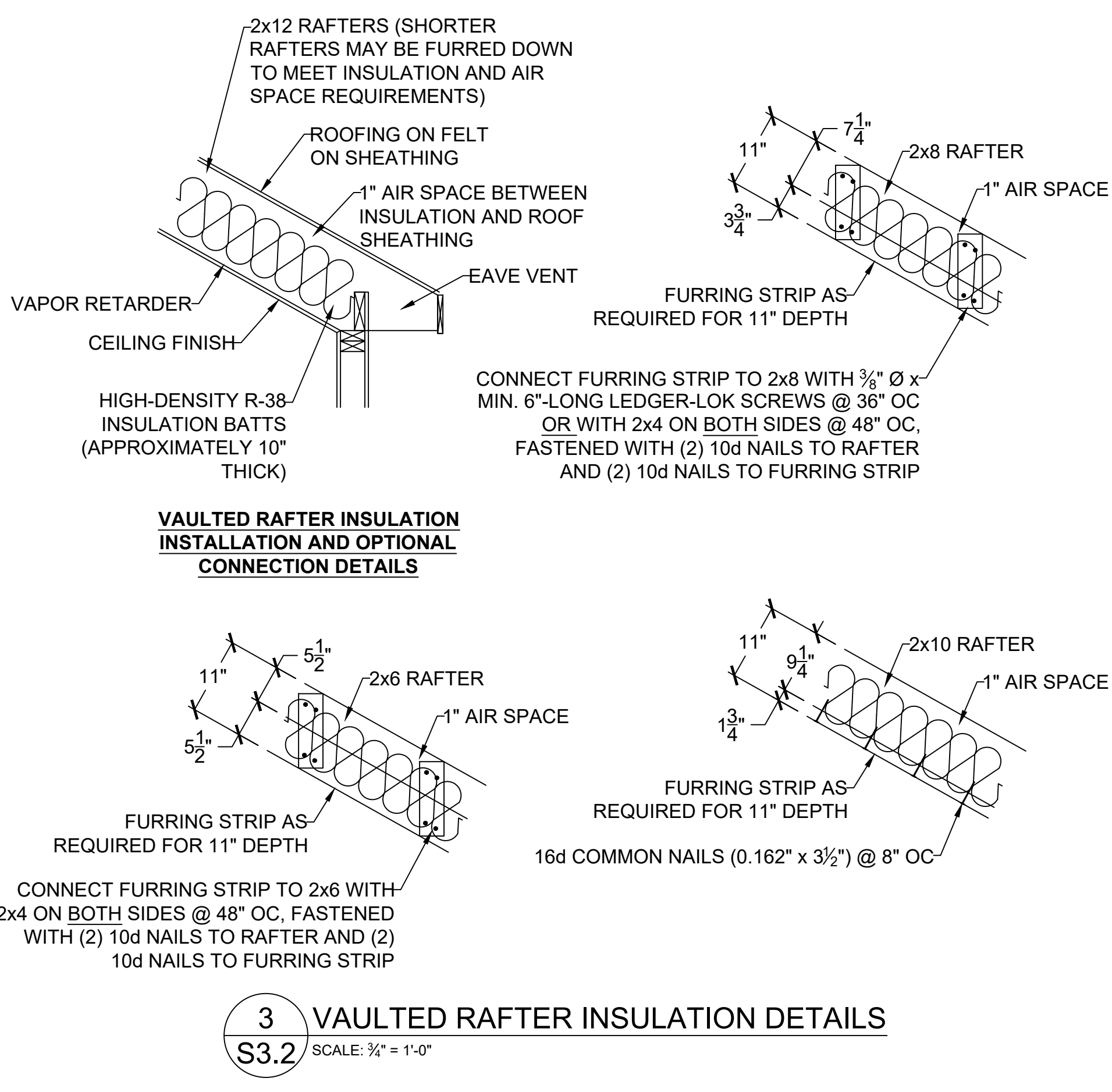
5 RAFTER TIES AT CEILING JOISTS PERP. TO RAFTERS
S3.2 SCALE: 3/4" = 1'-0" (18x24) OR 3/8" = 1'-0" (24x36)



7 FLOOR JOIST TO FLUSH STEEL BEAM DETAIL
S3.2 SCALE: 1" = 1'-0" (18x24) OR 1/2" = 1'-0" (24x36)



8 MAXIMUM ALLOWABLE LENGTH OF WOOD WALL STUDS (IRC TABLE 602.3.1)
S3.2



3 VAULTED RAFTER INSULATION DETAILS
S3.2 SCALE: 3/4" = 1'-0"

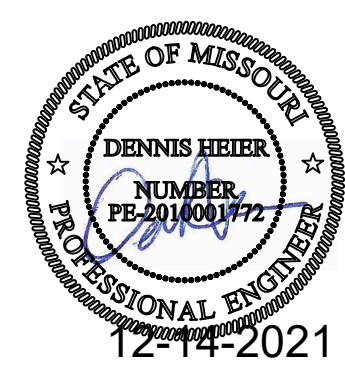
HEIGHT (FT.)	SPACING (INCHES O.C.)			
	24	16	12	8
SUPPORTING A ROOF ONLY				
10 OR LESS	2x4	2x4	2x4	2x4
12	2x6	2x4	2x4	2x4
14	2x6	2x6	2x6	2x4
16	2x6	2x6	2x6	2x4
18	DR	2x6	2x6	2x6
20	DR	DR	2x6	2x6
SUPPORTING ONE FLOOR AND A ROOF				
10 OR LESS	2x6	2x4	2x4	2x4
12	2x6	2x6	2x6	2x4
14	2x6	2x6	2x6	2x6
16	DR	2x6	2x6	2x6
18	DR	2x6	2x6	2x6
20	DR	DR	2x6	2x6
SUPPORTING TWO FLOORS AND A ROOF				
10 OR LESS	2x6	2x6	2x4	2x4
12	2x6	2x6	2x6	2x6
14	2x6	2x6	2x6	2x6
16	DR	2x6	2x6	2x6
18	DR	DR	2x6	2x6
20	DR	DR	DR	2x6

NOTES:
1) DR = DESIGN REQUIRED
2) UTILITY, STANDARD, STUD AND #3 GRADE LUMBER OF ANY SPECIES ARE NOT PERMITTED
3) THIS TABLE DOES NOT APPLY FOR STUDS SUPPORTING MEMBERS WITH A TRIB. LENGTH GREATER THAN 6'-0"



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S3.2

DECK JOIST SPAN	½" Ø GALV. LAG OR ¾" Ø LEDGER-LOK SPACING
10'-0" OR LESS	16" OC
10'-0" - 13'-11"	12" OC OR @ 16" OC DOUBLED EVERY OTHER
14'-0" - 18'-0"	8" OC OR @ 16" OC DOUBLED

1 1/4"

(2) 3/8" Ø x 5"-LONG LEDGERLOK SCREWS INTO EACH BEAM (FOUR TOTAL)

WOOD BEAM PER PLAN

6x6 CONTINUOUS POST

1 1/4"

(2) 3/8" Ø x 5"-LONG LEDGERLOK SCREWS INTO EACH BEAM (FOUR TOTAL)

WOOD BEAM PER PLAN

This technical drawing illustrates a 6x6 continuous post assembly. The post is shown in two sections, separated by a break symbol. Each section is 1 1/4" high. The post is secured to a horizontal beam using two 3/8" diameter by 5" long ledgerlok screws. The screws are shown in cross-section, with labels indicating that four screws are used per beam. The beam is labeled 'WOOD BEAM PER PLAN'. The post is labeled '6x6 CONTINUOUS POST'.

The image contains two technical diagrams illustrating the connection of a 6x6 post to a wood beam per plan using Simpson LCE4 brackets.

Left Diagram (Side View):

- WOOD BEAM PER PLAN (PERPENDICULAR):** Points to the horizontal wood beam on the left.
- WOOD BEAM PER PLAN:** Points to the horizontal wood beam on the right.
- SIMPSON LCE4 ON BOTH OUTSIDE FACES OF POST/BEAMS:** Points to the brackets connecting the post and beam.
- 6x6 POST:** Points to the vertical post.

Right Diagram (Top-Down View):

- WOOD BEAM PER PLAN:** Points to the horizontal wood beam at the top.
- 6x6 POST:** Points to the vertical post.
- WOOD BEAM PER PLAN:** Points to the horizontal wood beam at the bottom.
- SIMPSON LCE4 ON BOTH OUTSIDE FACES OF POST/BEAMS:** Points to the brackets connecting the post and beam.

SCALE: 1" = 1'-0" (18x24) OR 1½" = 1'-0" (24x36)

6x6 TREATED DECK POST

SIMPSON ABU66 POST BASE

CONCRETE PIER

4x4 GUARDRAIL POST

3'-0"

2x FRAMING PER PLAN

2x JOIST

2x JOISTS PER PLAN

2x BLOCKING

MIN. 2x12 TREATED SP STRINGER

MAX. SS = 5' FOR 2 STRINGERS
MAX. SS = 9' FOR 3 STRINGERS

3'-0"

STRINGERS NOTCHED OVER TREATED 2x4 SLEEPER WHICH IS ATTACHED TO LANDING LOCKS IN BOTTOM OF STRINGERS

TOP OF EACH STRINGER IS TOE-NAILED (TYPICAL) AND SUPPORTED BY SIMPSON LS70 GUSSET ANGLE, OR SLOPED HANGERS

STRINGER SPAN (SS)

COLUMN ATTACHED TO STRINGERS

STRINGER SPAN (SS)

CONCRETE LANDING IS RECOMMENDED - IT SHALL SUPPORT THE HEEL CUT OF THE STRINGERS

NO.	DATE	REVISION	BY

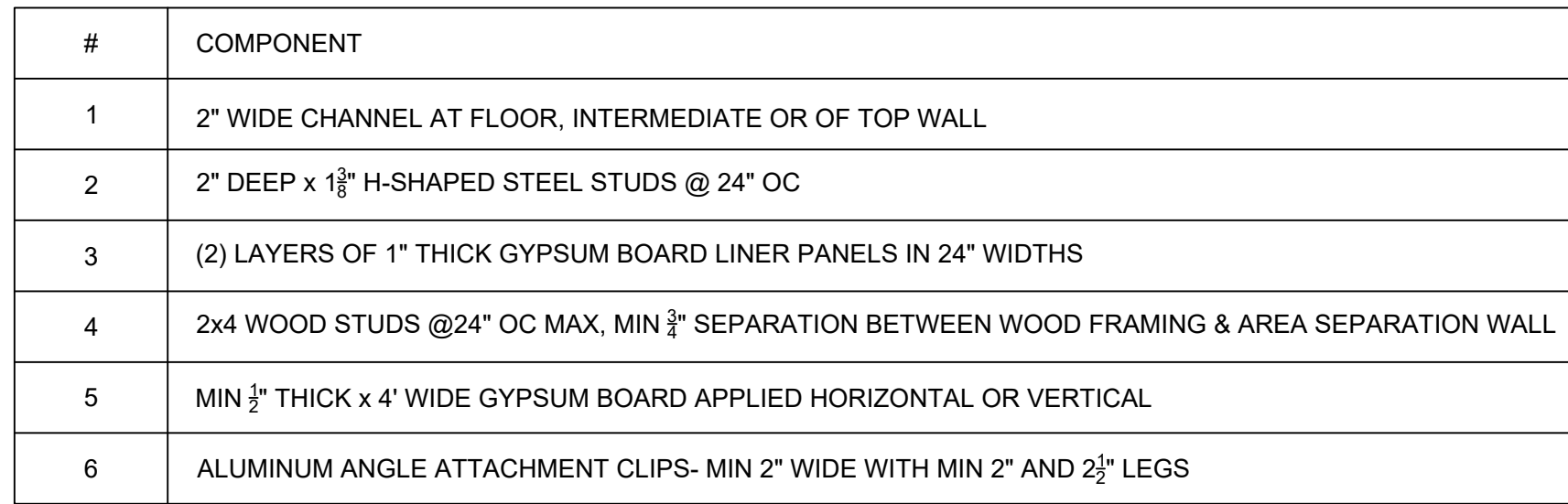
DRAWING TITLE

**FRAMING
DETAILS**

ENGINEER: DMH	CHECKED BY: DMH
JOB NO. 3812	DRAWN BY: DMH
DATE: 12-14-21	

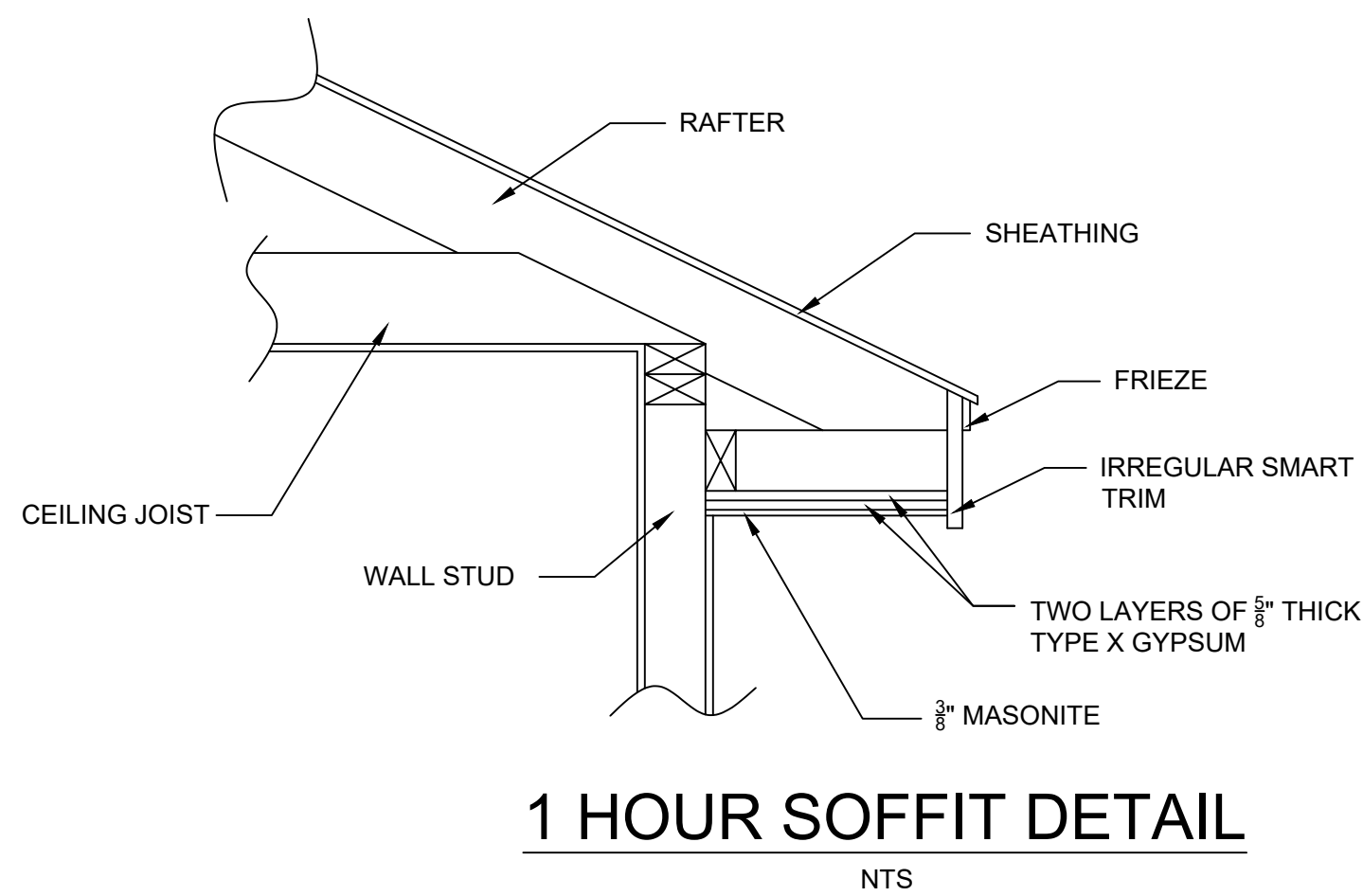
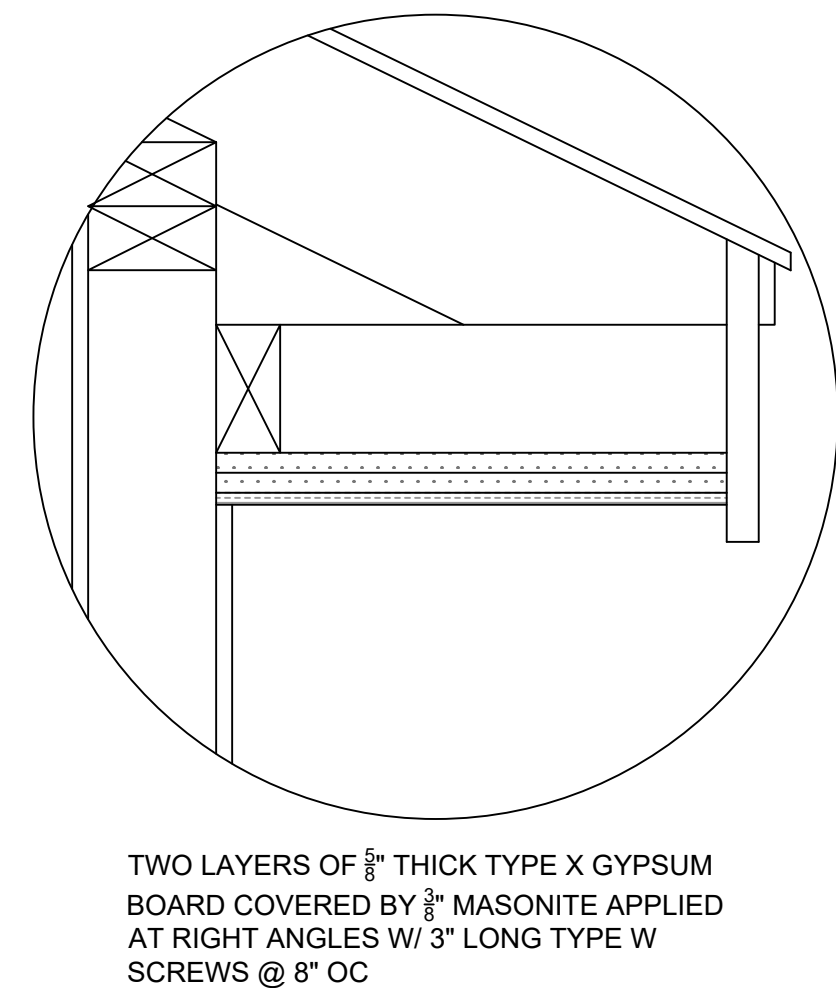
SHEET NUMBER

S3.3



*BEARING THE UL CLASSIFICATION MARK

- A. TWO HOUR FIRE WALL PER UL DESIGN # U366 SHOWN IN THE UL FIRE RESISTANCE DIRECTORY.
- B. INSULATE STUD CAVITIES WITH 3½" BATT INSULATION
- C. PLUMBING OR ELECTRICAL ALLOWED IN ADJOINING WALLS
- D. A SEPARATE FIRE SEPARATION WALL INSPECTION WILL BE REQUIRED
- E. ANY SHAFT WALL PENETRATIONS IN EXCESS OF 3" BUT LESS THAN 3½" TO BE FILLED WITH APPROVED FIRE CAULK OR FIRE FOAM. PENETRATIONS IN EXCESS OF 3½" TO BE FIRE PROOFED WITH OVERLAPPING LAYER OF 5½" TYPE X SHEET ROCK, PROPERLY NAILED AND GLUED. SEAL ADDITIONAL DRYWALL PATCH COMPLETELY WITH FIRE CAULK
- F. ATTIC FIRE SEPARATION WALL: (1)-2 HOUR SHAFT WALL FIRE TEST U366



STATE OF MISSOURI
DENNIS HEIER
NUMBER
PE-201000172
PROFESSIONAL ENGINEER
12-14-2021

NO.	DATE	REVISION	BY

DRAWING TITLE

**FRAMING
DETAILS**

ENGINEER: DMH	CHECKED BY: DMH
JOB NO. 3812	DRAWN BY: DMH
DATE: 12-14-21	

SHEET NUMBER

S3.4