GENERAL NOTES:

A. CODES AND STANDARDS
1. THE INTERNATIONAL BUILDING CODE/2015 EDITION
2. BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE ACI 318-05)
3. ASTM STANDARDS IN BUILDING CODES, VOLUMES I, II, AND III. AMERICAN SOCIETY FOR TESTING AND MATERIALS

B. DESIGN LOADS
1. LIVE LOADS:
   a. FLOORS.....40 PSF
   b. ROOF (MIN.).....30 PSF
   c. STAIR AND HALLWAYS.....100 PSF
2. SUPERIMPOSED DEAD LOADS
   a. HUNG LOADS (CEILINGS, LIGHTS, PIPES, ETC).....20 PSF
3. ROOF LOADS
   a. UNREDUCEABLE SNOW LOAD.....30 PSF
   b. EXPOSURE FACTOR..... 0.7
   c. IMPORTANCE FACTOR (I)..... 1.0
   d. ROOF LIVE LOAD (MIN).....30 PSF
   e. NET UPLIFT (MIN).....10 PSF
4. WIND
   a. BASIC WIND SPEED (V)..... 115 MPH
   b. IMPORTANCE FACTOR (I)..... 1.00
   c. EXPOSURE.....EXPOSURE C
   d. INTERNAL PRESSURE COEFFICIENT.....GCPI = +0.18

C. EXCAVATION AND BACKFILLING
1. PROVIDE ADEQUATE SHORING AND BRACING FOR ALL EXCAVATIONS AS REQUIRED TO MAINTAIN SAFETY AND PROTECTION FOR ADJACENT STRUCTURES AND FINISHES.
2. FILL IN STRUCTURAL AREAS OR PAVEMENT AREAS SHOULD BE COMPACTED TO AT LEAST 95 PERCENT OF THE MAXIMUM MODIFIED DRY DENSITY

D. FOUNDATIONS
1. ALL FOOTINGS SHALL BEAR ON NATURAL SOILS OR PROPERLY COMPACTED FILL HAVING A BEARING CAPACITY OF 2.0 K.S.F. AS APPROVED BY THE GEOTECHNICAL ENGINEER
2. DEWATERING: PROVIDE PUMPING EQUIPMENT TO KEEP THE EXCAVATION FREE OF WATER BEFORE PLACING CONCRETE.
3. FOOTINGS SHALL NOT BE PLACED IN WATER OR ON FROZEN SOIL
4. FROST PROTECTION: 30" MINIMUM MEASURED FROM BOTTOM OF FOOTING
5. THE ARCHITECT AND ENGINEER ASSUME NO RESPONSIBILITY FOR THE VALIDITY IF SUBSURFACE CONDITIONS DESCRIBED ON THE DRAWINGS OR IN THE SPECIFICATIONS. THESE DATA ARE INCLUDED ONLY TO ASSIST THE CONTRACTOR DURING BIDDING AND SUBSEQUENT CONSTRUCTION, AND REPRESENT CONDITIONS ONLY AT THESE SPECIFIC LOCATIONS AT THE PARTICULAR TIME THEY WERE MADE.

E. CAST-IN-PLACE CONCRETE
1. CONCRETE WORK SHALL CONFORM TO THE INTERNATIONAL BUILDING CODE/2015, THE ACI BUILDING CODE (ACI 318-05) AND TO THE SPECIFICATIONS. IN CASE OF CONFLICT, THE MOST STRINGENT REQUIREMENTS SHALL GOVERN.
2. ALL STRUCTURAL REINFORCED CONCRETE MEMBERS ARE DESIGNED USING "STRENGTH" DESIGN PROVISIONS
3. CONCRETE WITH MINIMUM 28 DAY COMPRESSIVE STRENGTH SHALL BE USED IN THE FOLLOWING AREAS:
   A) 3000 PSI NORMAL WEIGHT CONCRETE: IN ALL AREAS
4. ALL CONSTRUCTION, CONTROL AND EXPANSION JOINTS SHOWN ON THE DRAWINGS SHALL BE INCORPORATED INTO THE FINAL STRUCTURE UNLESS THEIR ELIMINATION IS APPROVED BY THE ARCHITECT
5. IF NOT SHOWN ON DRAWING, PROVIDE CONTROL JOINTS IN SLABS AT COLUMN LINES, AND MAXIMUM SPACING NOT TO EXCEED 12'-0" BETWEEN JOINTS.
6. ALL KEYS SHALL BE A NOMINAL 2 INCHES BY 4 INCHES UNLESS OTHERWISE NOTED ON THE DRAWINGS.
7. CONTRACTOR SHALL SUBMIT TO ARCHITECT, FOR APPROVAL, A SCHEDULE OF FLOOR POURS AT LEAST TWO WEEKS PRIOR TO INITIAL POUR
8. ALL CONCRETE EXPOSED TO THE WEATHER SHALL BE AIR ENTRAINED WITH 5% MIN. AIR

F. CONCRETE TESTING
1. TESTING SERVICES: TESTING OF COMPOSITE SAMPLES OF FRESH CONCRETE OBTAINED ACCORDING TO ASTM C 172 SHALL BE PERFORMED ACCORDING TO THE FOLLOWING REQUIREMENTS:
   A) TESTING FREQUENCY: OBTAIN ONE COMPOSITE SAMPLE FOR EACH DAY'S POUR OF EACH CONCRETE MIX EXCEEDING 5 CU. YD. (4 CU. M.), BUT LESS THAN 25 CU. YD. (19 CU. M.), PLUS ONE SET FOR EACH ADDITIONAL 50 CU. YD. (38 CU. M.) OR FRACTION THEREOF.
2. WHEN FREQUENCY OF TESTING WILL PROVIDE FEWER THAN FIVE COMPRESSIVE-STRENGTH TESTS FOR EACH CONCRETE MIX, TESTING SHALL BE CONDUCTED FROM AT LEAST FIVE RANDOMLY SELECTED BATCHES OR FROM EACH BATCH IF FEWER THAN FIVE ARE USED.
3. SLUMP: ASTM C 143; ONE TEST AT POINT OF PLACEMENT FOR EACH COMPOSITE SAMPLE, BUT NOT LESS THAN ONE TEST FOR EACH DAY'S POUR OF EACH CONCRETE MIX. PERFORM ADDITIONAL TESTS WHEN CONCRETE CONSISTENCY APPEARS TO CHANGE.
4. AIR CONTENT: ASTM C 231, PRESSURE METHOD, FOR NORMAL-WEIGHT CONCRETE; ASTM C 173, VOLUMETRIC METHOD, FOR STRUCTURAL LIGHTWEIGHT CONCRETE; ONE TEST FOR EACH COMPOSITE SAMPLE, BUT NOT LESS THAN ONE TEST FOR EACH DAY'S POUR OF EACH CONCRETE MIX.
5. CONCRETE TEMPERATURE: ASTM C 1064; ONE TEST HOURLY WHEN AIR TEMPERATURE IS 40 DEG F. (4.4 DEG. C.) AND BELOW, AND 80 DEG. F. (27 DEG. C.) AND ABOVE, AND ONE TEST FOR EACH COMPOSITE SAMPLE.
6. COMPRESSION TEST SPECIMENS: ASTM C 31/C 31M; CAST AND LABORATORY CURE ONE SET OF FOUR STANDARD CYLINDER SPECIMENS FOR EACH COMPOSITE SAMPLE. FIELD-CURED SPECIMENS BELOW MAYBE REQUIRED TO VERIFY ADEQUANCY OF CURING AND PROTECTION OF CONCRETE OR TO VERIFY STRENGTH FOR REMOVAL OF SHORING AND RESHORING IN MULTISTORY CONSTRUCTION.
7. COMPRESSIVE-STRENGTH TESTS: ASTM 39; TEST TWO LABORATORY CURED SPECIMENS AT 7 DYAS AND TWO AT 28 DAYS. A COMPRESSIVE -STRENGTH TEST SHALL BE THE AVERAGE COMPRESSIVE STRENGTH FROM TWO SPECIMENS OBTAINED FROM THE SAME COMPOSITE SAMPLE AND TESTED AT THE AGE INDICATED.

G. REINFORCING
1. ALL REINFORCING BAR DETAILS NOT OTHERWISE SHOWN OR SPECIFIED SHALL CONFORM TO THE LATEST ACI CODE AND DETAILING MANUAL
2. NEW BILLET STEEL, ASTM A615, GRADE 60 SHALL BE USED FOR REINFORCING FOR ALL CONCRETE
3. ALL WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185
4. ALL ACCESSORIES NECESSARY TO MAINTAIN EXACT BAR POSITIONS SHALL BE SHOWN ON REINFORCING SHOP DRAWINGS.
5. UNLESS OTHERWISE SPECIFIED IN THE DRAWINGS, THE CONCRETE COVER FOR THE REINFORCING SHALL BE AS FOLLOWS
   a) UNIFORM SURFACES IN CONTACT WITH THE GROUND.....3"
   b) FORMED SURFACES IN CONTACT WITH THE GROUND.....2"
   c) SLABS ON GRADE.....2" UNDER
   d) INTERIOR SURFACES OTHER THAN SLABS.....1"
   e) IN ALL CASES CLEARANCE NOT LESS THAN THE DIAMETER THAN THE BAR
   f) MAXIMUM DEVIATION FROM THESE REQUIREMENTS SHALL BE +1/4" FOR SECTIONS TEN INCHES OR LESS AND + 1/2" FOR SECTIONS OVER 10"
   g) LAP SPLICES SEE SCHEDULE ON THIS SHEET
   h) WHERE REINFORCEMENT IS NOT SHOWN ON THE DRAWINGS, PROVIDE THE MINIMUM REINFORCEMENT PERMITTED BY A.C.I. CODES
   i) WHEN CONSTRUCTION JOINTS, OTHER THAN THOSE SHOWN ON THE DRAWINGS, ARE REQUIRED BY THE CONTRACTOR, THE REINFORCEMENT SHALL RUN CONTINUOUSLY THROUGH THE JOINT AND ADEQUATE SHEAR TRANSFER REINFORCEMENT SHALL BE ADDED
   j) PROVIDE THE EXTRA REINFORCING SHOWN IN TYPICAL DETAILS WHEREVER THE DETAIL CAN APPLY

I. STRUCTURAL WOOD FRAMING
1. EXCEPT WHERE NOTED OTHERWISE,ALL 2" LUMBER SHALL BE #2 OR BETTER S.P.F., AND ALL SOLID TIMBER BEAMS AND POSTS SHALL BE #2 S.P.F.
2. TOP AND BOTTOM PLATES SHALL BE #2 OR BETTER D.F. PLATES IN CONTACT WITH CONCRETE SHALL BE ACQ. TREATED. HANGERS AND ANCHORS IN CONTACT WITH ACQ. TREATED WOOD SHALL BE G185 CORROSION RESISTANT OR TYPE 304 OR 316 STAINLESS STEEL.
3. EXCEPT AS NOTED OTHERWISE, MINIMUM NAILING SHALL BE PROVIDED AS SPECIFIED IN TABLE NO. 2304.9.1 FASTENING SCHEDULE OF THE CURRENT INTERNATL. BUILDING CODE.
4. 2x4 STUDS SHALL BE NAILED TO TOP PLATE WITH 3-16d END NAILS AND TO SOLE PLATE WITH 6-8d TOENAILS OR 3-16d END NAILS.
5. ALL PLYWOOD OR O.S.B. BD. SHEATHING SHALL BE APA RATED, EXPOSURE 1. PANEL IDENTIFICATION INDEX AND/ OR THICKNESS SHALL BE 32/16, 1/2" MINIMUM, OR AS NOTED ON DRAWINGS.
6. TRUSSED RAFTERS AND MFG. JOISTS, AS WELL AS ALL CONNECTIONS SHALL BE DESIGNED BY A REGISTERED ENGINEER TO SUPPORTTHE FULL DEAD AND LIVE LOADS OF THE ROOF, FLOOR, CEILING AND ANY OTHER SUPERIMPOSED LOADS. WEB ARRANGMENT AND MEMBER FORCES SHALL BE DETERMINED BY THE TRUSSED RAFTER/JOIST FABRICATOR.STRESSES SHALL NOT EXCEED THOSE ALLOWED BY THE CURRENT BOCA NATL. BLDG. CODE. LOAD DURATION FACTOR MAY BE 1.15 FOR ROOF TRUSSES ONLY. CALCULATIONS AND SHOP DRAWINGS, INCLUDING MEMBER SIZES, LUMBER SPECIES AND GRADES AND SUBSTANTIATING DATA FOR CONNECTOR CAPACITIES, SHALL BE SUBMITTED TO THE ARCHITECT FOR REVIEW AND APPROVAL PRIOR TO FABRICATION.
7. LEAD HOLES FOR LAG BOLTS SHALL BE DRILLED IN ACCORDANCE WITH TABLE 5.1. OF THE A.I.T.C. TIMBER CONSTRUCTION MANUAL.
8. STUD NOTCHING MUST NOT EXCEED 20% OF STUD DEPTH.
9. HOLES DRILLED IN EXTERIOR WALL STUDS SHALL NOT EXCEED 1" DIA.
10. TEMPORARY CONSTRUCTION LOADS WHICH EXCEED THE DESIGN CAPACITY OF ANY ELEMENT ARE PROHIBITED. ERECTION BRACING SHALL BE PROVIDED TO ASSURE ADEQUATE LATERAL SUPPORT FOR THE INDIVIDUAL ELEMENTS, AS WELL AS THE ENTIRE SYSTEM UNTIL SHEATHING IS INSTALLED.
   (SHEATHING CAN BE EITHER PLYWOOD, OSB, OR GYP-BOARD)

J. DECKING
1. ROOF DECKING SHALL BE 5/8" OSB SHEATHING w/CLIPS FASTENED WITH 10d NAILS 12" O.C. AND 6" O.C. @ JOINTS

K. STRUCTURAL STEEL
1. ALL STRUCTURAL STEEL ASTM A 992, FOR WIDE FLANGE SHAPES, TEES AND CHANNELS. ETC.
2. STRUCTURAL TUBES ASTM A500, TYPE B (FY = 46 KSI)
3. STRUCTURAL STEEL CONNECTIONS SHALL BE COMPLETED BY THE STRUCTURAL STEEL CONTRACTOR
5. ALL BOLTED CONNECTIONS OF STRUCTURAL STEEL TO BE 3/4" DIA. A325-N UNLESS OTHERWISE NOTED
6. ANCHOR BOLTS SHALL CONFORM TO ASTM A325
7. PAINTING: PRIMER PAINT FOR EXPOSED STRUCTURAL STEEL: SSPC-25

L. GROUTS FOR BASEPLATES AND ANCHORS
1. A NON METALLIC SHRINKAGE RESISTANT GROUT UNDER BASEPLATES AND BEARING PLATES SHALL BE "MASTERFLOW 713" BY MASTER BUILDERS
2. EPOXY GROUT FOR GROUTING REBARS SHALL BE "SIKADUR 370" BY SIKA CHEMICAL COMPANY, OR APPROVED EQUAL FOR SURFACE PREPARATION ,PROPORTION, MIXING AND APPLICATION FOLLOW MANUFACTURER'S DIRECTIONS

K. SHEARWALL NOTES AND ROOF DIAPHRAGMS:
1. PROVIDE SHEARWALLS AT ALL EXTERIOR WALLS
2. 7/16" O.S.B. NAILED W/ 10d NAILS AT 4" O.C. AT EDGES STAGGERED, AND 10d NAILS @ 12" O.C. AT INTERIOR SUPPORTS. WITH MIN. 1 1/2" PENETRATION.
   PROVIDE SOLID BLOCKING AT ALL HORZ. JOINTS.
3. FOR ALL INTERIOR SHEAR WALLS USE 5/8" GYP BOARD FASTENED W/ #6 TYPE'S S OR W DRYWALL SCREWS 1 1/4" LONG. MAX SPACING 7" AND EDGE SPACING 4" O.C. WITH SOLID BLOCKING

SHEARWALL ANCHORING:
1. PROVIDE 2-2x4 (MIN.) AT ENDS OF SHEARWALL.
2. ALL REFERENCES TO PLYWOOD INDICATE ANY WOOD (EXTERIOR RATED) SHEATHING W/ APA EQUIVALENT SPAN RATING PER THE FOLLOWING:

APPROX. THICKNESS	SPAN RATING
7/16"	32/16

M. HEADER SCHEDULE

ROOF SPAN	3'-0	4'-0	5'-0	6'-0	7'-0	8'-0
	(2) 2x6	(2) 2x8	(2) 2x8	(2) 2x10	(2) 2x12	(2) 2x12

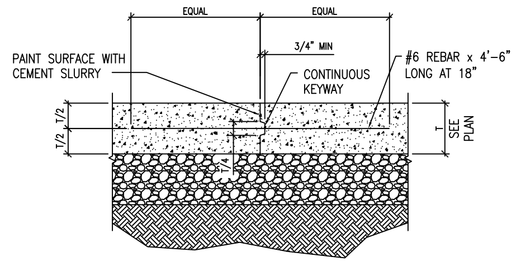
N. ROOF TRUSS LOADING

DL AT TOP CHORD	20.0	PSF
DL AT BOTTOM CHORD	10.0	
TOTAL DEAD LOAD	30.0	PSF
LIVE LOAD (MINIMUM)	30.0	
TOTAL DESIGN LOAD	60.0	PSF
NET UPLIFT (DUE TO WIND)	10.0	PSF (30.0 AT O.H.)

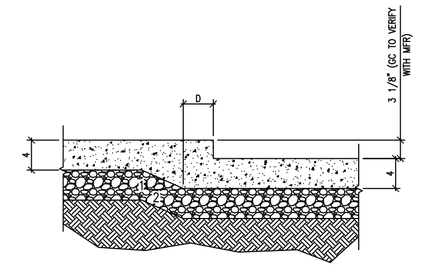
O. HVAC UNITS COORDINATION
1. OPENINGS AND LOCATION SHOWN FOR HVAC UNITS ETC. ON THE FRAMING PLANS ARE SUBJECT TO CHANGE. COORDINATE WITH MECHANICAL CONTRACTOR PRIOR TO PREPARING SHOPDRAWINGS FOR DECK, JOISTS AND STRUCTURAL STEEL

P. HAND / GUARD RAILS
1. HANDRAIL ASSEMBLIES AND GUARDS SHALL BE DESIGNED TO RESIST A LOAD OF 50 PLF APPLIED IN ANY DIRECTION AT THE TOP AND TO TRANSFER THIS LOAD THROUGH THE SUPPOSRTS TO THE STRUCTURE.
2. HANDRAIL ASSEMBLIES AND GUARDS SHALL BE ABLE TO RESIST A SINGLE CONCRETRATED LOAD OF 200 POUNDS APPLIED IN ANY DIRECTION AT ANY POINT ALONG THE TOP, AND HAVE ATTACHMENT DEVICES AND SUPPORTING STRUCTURE TO TRANSFER THIS LOADING TO APPROPRIATE STUCTURAL ELEMENTS OF THE BUILDING. THIS LOAD NEED NOT BE ASSUMED TO ACT CONCURRENTLY WITH THE LOADS SPECIFIED IN THE DESIGN LOADS PARAGRAPH.
3. INTERMEDIATE RAILS (ALL THOSE EXCEPT THE HANDRAILS), BALUSTERS AND PANEL FILLERS SHALL BE DESIGNED TO WITHSTAND A HORIZONTALLY APPLIED NORMAL LOAD OF 50 POUNDS ON AS AREA EQUAL TO 1 SQUARE FOOT, INCLUDING OPENINGS AND SPACE BETWEEN RAILS. REACTIONS DUE TO THIS LOADING ARE NOT REQUIRED TO BE SUPERIMPOSED WITH THOSE ABOVE.

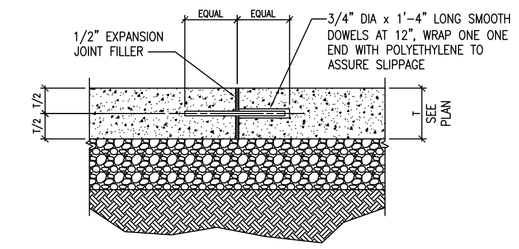
Q. MISCELLANEOUS CONSTRUCTION NOTES
1. THE CONTRACTOR SHALL VERIFY EXISTING CONDITIONS AND ALL DIMENSIONS IN THE FIELD, AND NOTIFY THE ARCHITECT IF CONDITIONS ENCOUNTERED ARE NOT AS SHOWN ON DRAWINGS
2. CONSULT ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS FOR VERIFICATION OF THE LOCATION AND DIMENSIONS OF THE CHASES, INSERTS, OPENINGS, SLEEVES, WASHES, DRIPS, REVEALS, DEPRESSIONS AND OTHER PROJECT REQUIREMENTS NOT SHOWN ON THE STRUCTURAL DRAWINGS.
3. NO CHANGE IN SIZE OR DIMENSIONS OF MEMBERS SHALL BE MADE, NOR ANY OPENINGS OR SLEEVES SHALL BE PERMITTED THROUGH ANY STRUCTURAL MEMBER WITHOUT THE APPROVAL OF THE ARCHITECT
4. OPENINGS IN SLABS OR WALLS LESS THAN 12" IN DIA. ARE NOT GENERALLY SHOWN
5. FURNISH AND INSTALL ALL SUPPORTS, TEMPORARY OR PERMANENT, NECESSARY TO BRACE WALLS, EXISTING ADJACENT STRUCTURES AND GRADE, SO THAT NO HORIZONTAL OR VERTICAL SETTLEMENT OCCURS TO THESE WALLS, STRUCTURES OR GRADES. BRACING SHALL BE LEFT IN PLACE UNTIL PERMANENT SUPPORTS ARE INSTALLED
6. WHERE DETAIL IS NOT SHOWN ON THE DRAWINGS, FOR SUCH A SITUATION, THE DETAIL SHALL BE ASSUMED IN ACCORDANCE WITH APPLICABLE DETAILS OR SIMILAR TO THAT SHOWN FOR MOST NEARLY SIMILIAR SITUATIONS, AS DETERMINED BY THE ARCHITECT. CONTRACTOR SHALL PROVIDE A COMPLETE AND FUNCTIONAL SYSTEM WHEATHER OR NOT A PARTICULAR ELEMENT IS SPECIFICALLY CALLED OUT.
7. CONTRACTORS AND SUBCONTRACTORS SHALL THOROUGHLY REVIEW ALL CONDITIONS AND RESPONSIBILITIES STATED IN THESE NOTES, PLANS SECTIONS AND DETAILS, AND SHALL NOTIFY THE ENGINEER AND ARCHITECT IN WRITING PRIOR TO BIDDING OF ANY DISCREPENCIES, ERRORS OR OMISSIONS FOUND ON PLANS. IF CONTACTOR FAILS TO NOTIFY SAID REPRESENTIVES, HE/SHE SHALL BE RESPONSIBLE FOR ALL COST OF LABOR AND MATERIALS NEEDED.



CONCRETE SLAB CONSTRUCTION JOINT



DEPRESSED SLAB ON GRADE



CONCRETE SLAB EXPANSION JOINT

MISC. LINTEL SCHEDULE

MAXIMUM MASONRY OPENING	WALL THICKNESS - NOM.		
	4" WALLS	6" WALLS	8" WALLS
3'-0"	L-3x2 1/2x1/4"	(2)L-3x2 1/2x1/4"	(2)L-3 1/2x3 1/2x1/4"
4'-0"	L-3x2 1/2x1/4"	(2)L- 3x2 1/2x1/4"	(2)L-4x3 1/2x1/4"
5'-0"	L-3 1/2x2 1/2x1/4"	(2)L-3 1/2x2 1/2x1/4"	(2)L-4x3 1/2x1/4"
6'-0"	L-3 1/2x2 1/2x1/4"	(2)L-3 1/2x2 1/2x1/4"	(2)L-5x3 1/2x1/4"
8'-0"	L-3 1/2x2 1/2x1/4"	(2)L- 3 1/2x2 1/2x1/4"	(2)L-6x3 1/2x1/4"

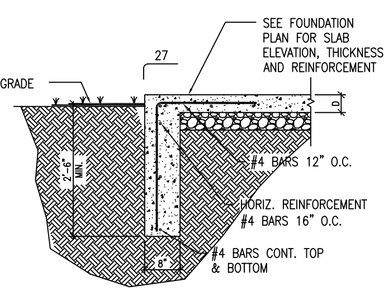
1. UNLESS OTHERWISE NOTED ON DRAWINGS, PROVIDE AND INSTALL LINTELS WITH LONG-LEGS VERTICALLY FOR MASONRY OPENINGS WHERE REQUIRED AND PER ARCHITECTS DRAWINGS.
2. WHERE LINTELS OCCUR IN EXTERIOR WALLS, MINIMUM THICKNESS OF ANGLES SHALL BE 5/16". LINTELS SHALL BEAR AT LEAST 6" (8" FOR OPENINGS OVER 6'-0" WIDE) AT EACH SIDE OF THE MASONRY OPENING. WHERE SUCH BEARING CANNOT BE PROVIDED, ATTACHED SECURELY TO ADJACENT STRUCTURAL MEMBERS OR PROVIDE SEPARATE SUPPORTS.

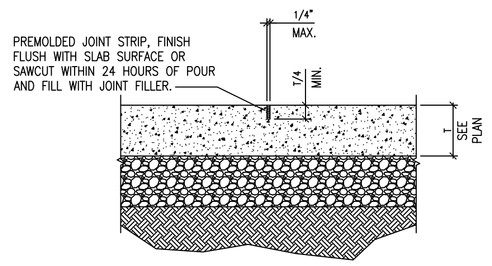
SCHEDULE OF SPLICE AND EMBEDMENT LENGTHS

f'c =3,000 PSI	BAR SIZE								
fy = 60,000 PSI	#3	#4	#5	#6	#7	#8	#9	#10	#11
COMPRESSION SPLICE (L)									
ALL BARS	12	15	19	23	26	30	34	38	42
TENSION SPLICE (L)									
BEAM TOP BARS AND WALL HORIZONTAL BARS	28	38	47	56	81	93	105	118	131
STRAIGHT BARS	22	29	36	43	63	72	81	91	101
EMBEDMENT (E)									
STRAIGHT TOP BARS	22	29	36	43	63	72	81	91	101
STRAIGHT BARS	17	22	28	33	48	55	62	70	78
HOOKED BARS	6	8	10	12	14	16	18	20	22

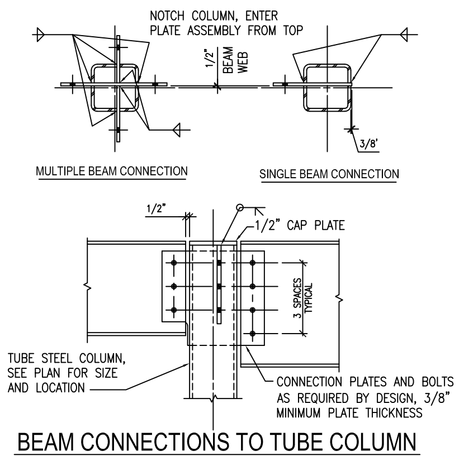
L = SPLICE LENGTH (INCHES) E = EMBEDMENT LENGTH (INCHE

TYPICAL EDGE DETAIL
(EXTERIOR SLAB ON GRADE)

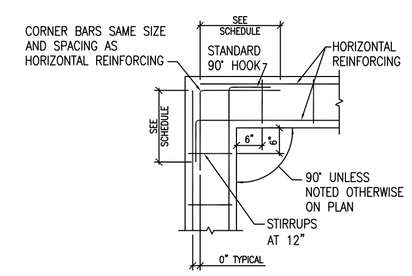




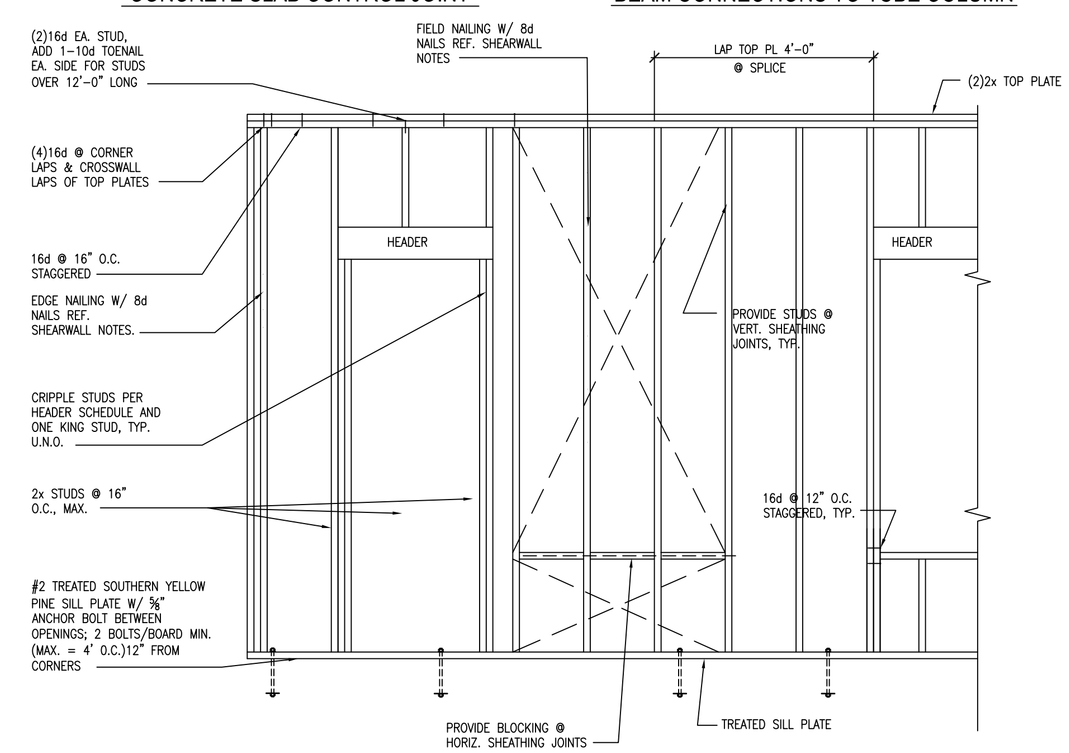
CONCRETE SLAB CONTROL JOINT



BEAM CONNECTIONS TO TUBE COLUMN



GRADE BEAM CORNER REINFORCING



FRAMING/SHEARWALL TYP. DETAIL

FOOTING REINFORCING SCHEDULE

FOOTING	SIZE	REINFORCING
F1	5'-0"x 5'-0"x 1'-0"	(5) #5 BARS @ EACH WAY, EACH FACE

12-08-2021

GENERAL NOTES

PINNACLE DESIGN CONSULTANTS
CIVIL - STRUCTURAL ENGINEERING
417-501-9820
PINNACLEDC.COM

3338 NE Ralph Powell Road
Lee's Summit, MO

Larry D. Nelson, Architect
3424 S. Rogers Ave. Springfield, MO 65804
Phone: 417.882.8122 Fax 417.882.8152
email: larrynelsonarchitect@mchsi.com

Date: 12-08-21
REV Date: 12-08-21

Sheet No.
S3