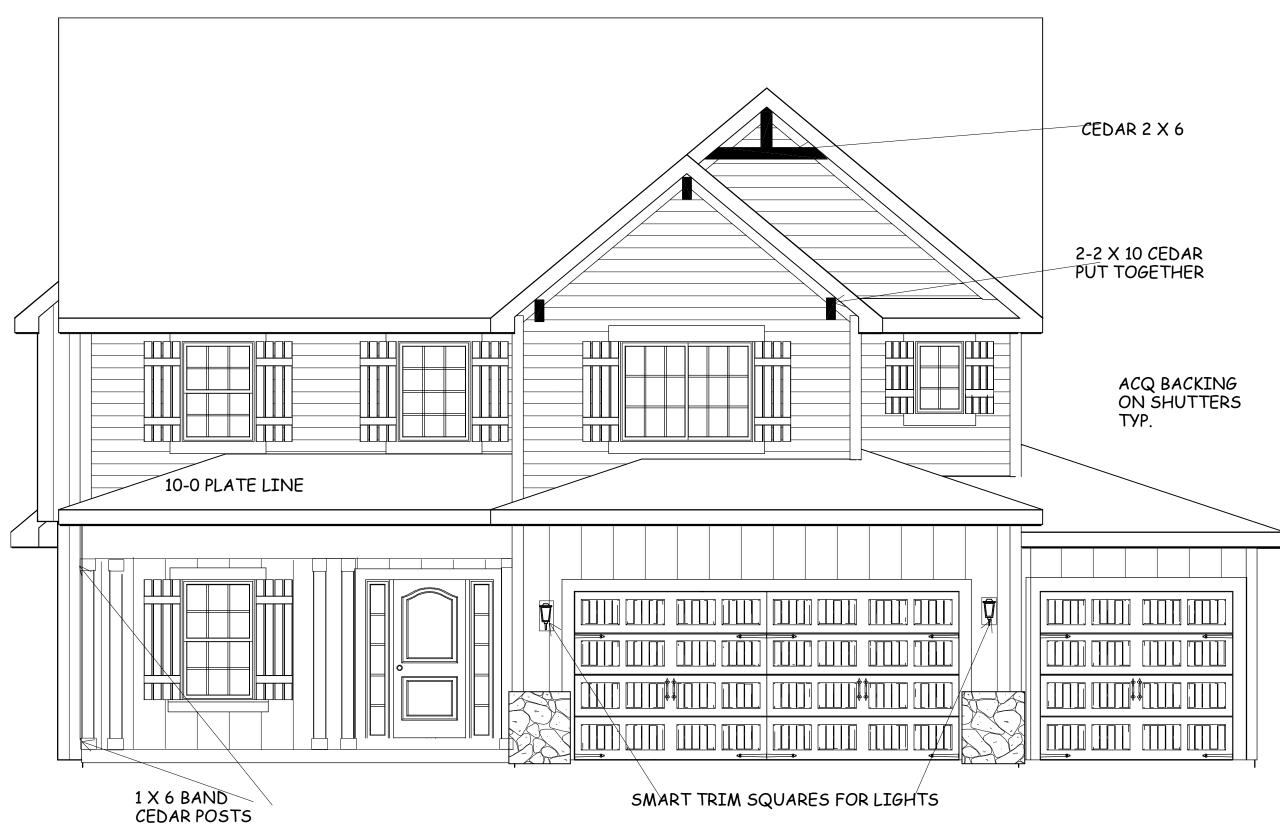


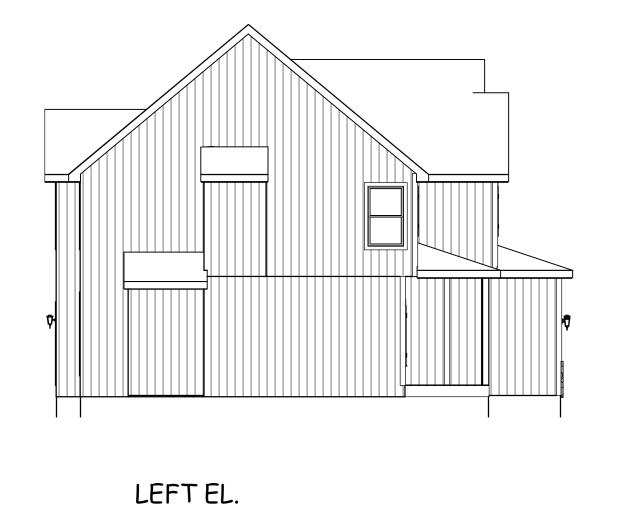
ROOF PLAN ELEVATION 1/8 = 1-0 TYP. ROOF PITCH 10/12 RAFTERS 2 X 6 DF NO 2 @ 16" OC TYP. HIPS AND RIDGES 2 X 10 DF NO 2 U.N.O.



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3653

FRONT ELEVATION **ELEVATION B**



1/8" = 1-0



ELEVATE DESIGN & BUILD BRIGHTWOOD LOT 48 HOOK FARMS 2036 WHEATFIELD CT LEE SUMMIT MO

SCALE 1/4" = 1-0

DATE 10-25-21

PLAN NO.

3653

SHEET NO.

Kansas City, MO 64116

ALL HEADERS 6-9 ON THIS FLOOR

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SSIONAL ENGI. David Mezger Engineering LLC 212 NE Circle Dr. Kansas City, MO 64116

DAVID E. MEZGER

PE-2018009531

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& BUILD ELEVATE DESIGN & BUILL BRIGHTWOOD LOT 48 HOOK FARMS 2036 WHEATFIELD CT LEE SUMMIT MO

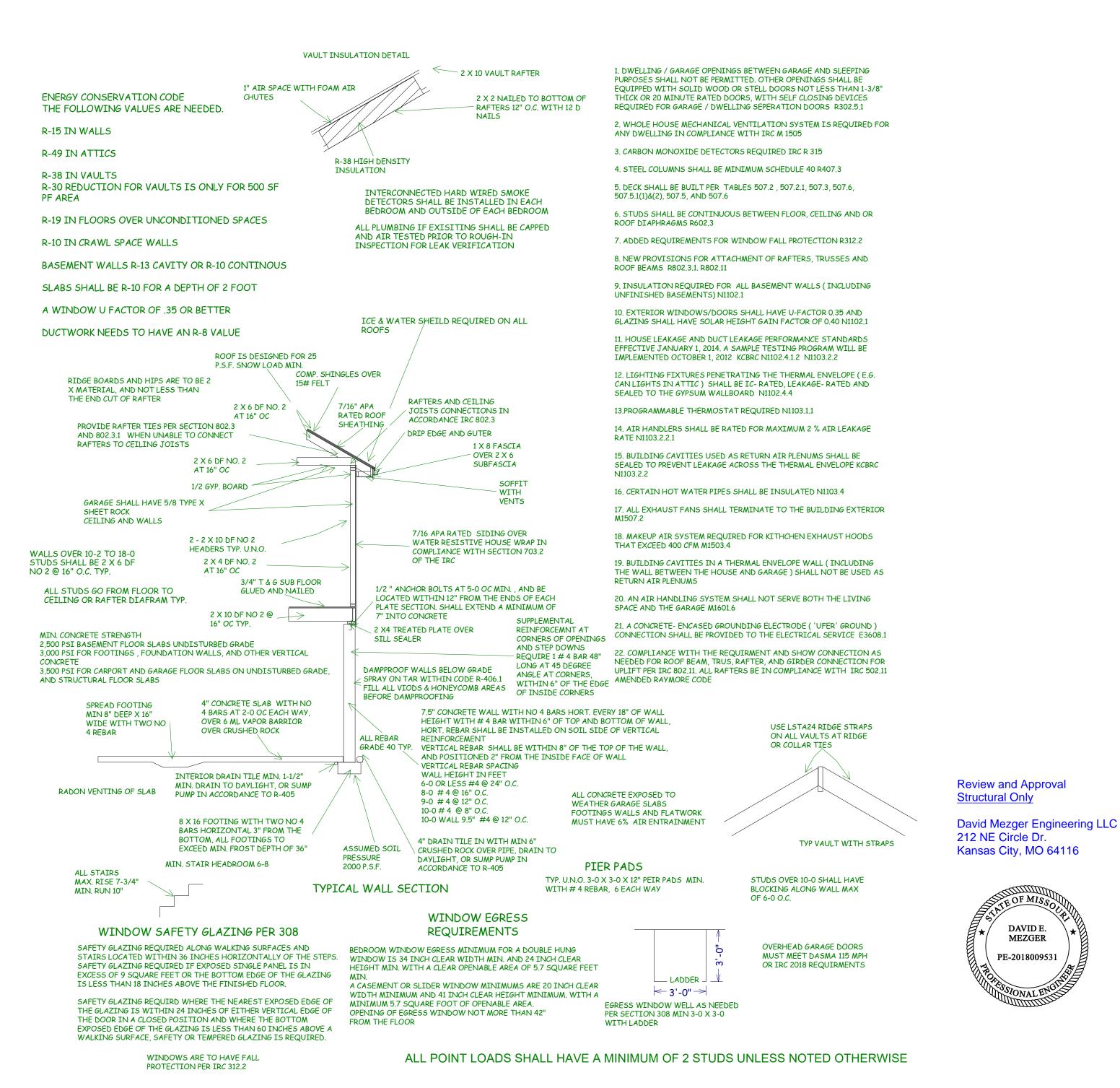
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DATE 10-25-21

PLAN NO.

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BUIL OD ARM: ELD C ō おる。 48 O Ē E

SCALE 1/4" = 1-0

DATE 10-25-21

E OF MISSO

DAVID E.

MEZGER

PE-2018009531

SSIONALE

PLAN NO.

3653

SHEET NO.

PURLIN PLAN SECOND FLOOR NO PURLINS ON FIRST FLOOR

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ELEVATE DESIGN & BUILD BRIGHTWOOD LOT 48 HOOK FARMS 2036 WHEATFIELD CT LEE SUMMIT MO

SCALE 1/4" = 1-0

DATE 10-25-21

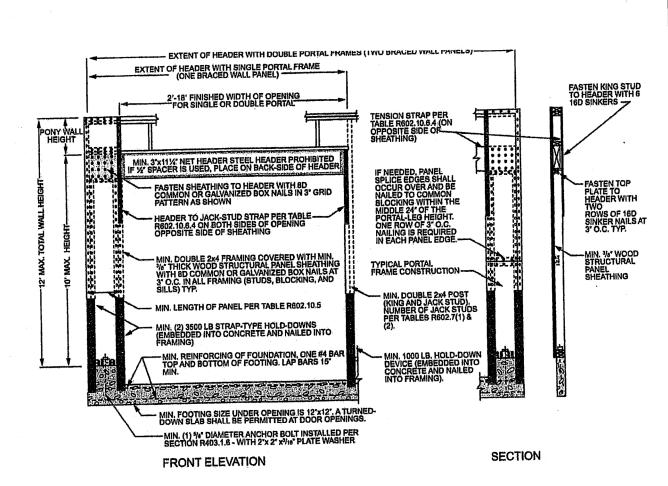
PLAN NO.

3653

SHEET NO.

	E	T RACING REQUIR	ABLE R602.10.3(1) EMENTS BASED C	N WIND SPEED			
EXPOSURE CATEGORY B 30-FOOT MEAN ROOF HEIGHT 10-FOOT WALL HEIGHT 2 BRACED WALL LINES			MINIMUM TOTAL LENGTH (FEET) OF BRACED WALL PANELS REQUIRED ALONG EACH BRACED WALL LINE'				
Ultimate Design Wind Speed (mph)	Story Location	Braced Wall Line Spacing ^e (feet)	Method LIB ^b	Method GB	Methods DWB, WSP, SFB, PBS, PCP, HPS, BV-WSP, ABW, PFH, PFC, CS-SFB	Methods CS-WSP, CS-G, CS-PF	
		10	3.5	3.5	2.0	2.0	
	^	20	6.5	6,5	3.5	3.5	
		30	9,5	9.5	5.5	4.5	
		40	12.5	12.5	7.0	6.0	
		50	15.0	15.0	9.0	7.5	
		60	18.0	18.0	10.5	9.0	
		10	7.0	7.0	4.0	3.5	
		20	12.5	12.5	7.5	6.5	
		30	18.0	18.0	10.5	9.0	
≤ 115		40	23.5	23.5	13.5	11.5	
		50	29.0	29.0	16.5	14.0	
		60	34.5	34.5	20.0	17.0	
	自	10	NP	10.0	6.0	5.0	
		20	NP	18.5	11.0	9.0	
		30	NP	27.0	15.5	13.0	
		40	NP	35.0	20.0	17.0	
		50	NP	43.0	24.5	21.0	
	(2)	60	NP	51.0	29.0	25.0	

	PANEL LENGTH PER TABLE R602 10.5	
BRACED WALL PANEL HEIGHT	MIN. 3/8" WOOD STRUCTURAL PANEL SHEATHING ON ONE FACE MIN. 2 X4 FRAMING MIN. DOUBLE STUDS REQUIRED. (2) HOLD-DOWN OR (2) STRAP-TYPE ANICHORS PER TABLE R602 10.6.1 (ONE OF EACH SHOWN FOR CLARITY). STRAP-TYPE ANCHORS SHALL BE PERMITTED TO BE ATTACHED OVER THE WOOD STRUCTURAL PANEL PANEL MUST BE ATTACHED TO CONCRETE FOOTING OR CONCRETE FOOTING	FOR PANEL SPLICE (IF NEEDED) ADJOINING PANEL EDGES SHALL MEET OVER AND BE FASTENED TO COMMON FRAMING 8D COMMON OR GALV. BOX NAILS @ 6* O.C. AT PANEL EDGES. FOR SINGLE STORY AND @ 4* O.C. PANEL EDGES FOR THE FIRST OF 2 STORIES STUDS UNDER HEADER AS REQUIRED 8D COMMON OR GALV. BOX NAILS @ 12* O.C. AT INTERIOR SUPPORTS MIN. REINFORCING OF FOUNDATION, ONE #\$ BAR TOP AND BOTTOM. LAP BARS 15* MINIMUM.
	(2) 1/2* DIAMETER ANCHOR BOLTS LOCATED BETWEEN 6* AND 12* OF EACH END OF THE SEGMENT	_ MINIMUM FOOTING SIZE UNDER OPENING IS 12" X 12".A TURNED-DOWN SLAB SHALL BE PERMITTED AT DOOR OPENINGS.
25.4 mm.	FIGURE R602.10.6.1 METHOD ABW—ALTERNATE BRACED WALL PA	ANEL



4 mm, 1 foot = 304.8 mm.

FIGURE R602.10.6.2 METHOD PFH—PORTAL FRAME WITH HOLD-DOWNS

			TABLE R602.10 BRACING METHO				
METHODS, MATERIAL				CONNECTION CRITERIA*			
		MINIMUM THICKNESS	FIGURE	Fasteners	Spacing		
	LIB	1 × 4 wood or approved metal straps		Wood: 2-8d common nails or 3-8d (2 ¹ / ₂ " long x 0.113" dia.) nails	Wood: per stud and top and bottom plates		
Let-in-bracing		at 45° to 60° angles for maximum 16" stud spacing		Metal strap: per manufacturer	Metal: per manufacturer		
	DWB Diagonal wood boards	³ / ₄ " (1" nominal) for maximum 24" stud spacing		2-8d $(2^{1}/_{2}" \log \times 0.113" \text{ dia.})$ nails or $2 - 1^{3}/_{4}" \log \text{ staples}$	Per stud		
	WSP Wood			Exterior sheathing per Table R602.3(3)	6" edges 12" field		
	structural panel (See Section R604)	³/ ₈ "		Interior sheathing per Table R602.3(1) or R602.3(2)	Varies by fastener		
sthods	BV-WSP* Wood structural panels with stone or masonry veneer (See Section R602, 10.6.5)	7/ ₁₆ "	See Figure R602.10.6.5	8d common (2 ¹ / ₂ " × 0.131) nails	4" at panel edges 12" at intermediate supports 4" at braced wall panel end posts		
Intermittent Bracing Methods	SFB Structural fiberboard sheathing	1/2" or 25/32" for maximum 16" stud spacing		1^1l_2 " long × 0.12" dia. (for 1l_2 " thick sheathing) 1^3l_4 " long × 0.12" dia. (for $^{25}l_{32}$ " thick sheathing) galvanized roofing nails	3" edges 6" field		
Intermittent	GB Gypsum board	1/2"		Nails or screws per Table R602.3(1) for exterior locations Nails or screws per Table R702.3.5 for interior locations	panel locations: /"		
	PBS Particleboard sheathing (See Section R605)	³ / ₈ " or ¹ / ₂ " for maximum 16" stud spacing		For ³ / ₈ ", 6d common (2" long × 0.113" dia.) nails For ¹ / ₂ ", 8d common (2 ¹ / ₂ " long × 0.131" dia.) nails	3" edges 6" field		
	PCP Portland cement plaster	See Section R703.7 for maximum 16" stud spacing		1 ¹ / ₂ " long, 11 gage, ⁷ / ₁₆ " dia. head nails or ⁷ / ₈ " long, 16 gage staples	6" o.c. on all framing members		
	HPS Hardboard panel siding	7/16" for maximum 16' stud spacing		0.092" dia., 0.225" dia. head nails with length to accommodate 1 ½" penetration into studs	4" edges 8" field		
	ABW Alternate braced wall	3/8"		See Section R602.10.6.1	See Section R602.10.6.		

METHOD			NGTH OF BRACED WALL PANELS MINIMUM LENGTH* (Inches)				CONTRIBUTING LENGTH (Inches)	
(See Table R602.10.4)		Wali Helght					(monos)	
		8 feet	9 feet	10 feet	11 feet 53	12 feet 58	Actual ^b	
DWB, WSP, SFB, P	BS, PCP, HPS, BV-WSP	48	48	48			Double sided = Actual	
	GB	48	48	48	53	58	Single sided = $0.5 \times Actua$	
	LIB	55	62	69	NP	NP	Actual ⁶	
ABW	SDC A, B and C, ultimate design wind speed < 140 mph	28	32	34	38	42	48	
	SDC D ₀ , D ₁ and D ₂ , ultimate design wind speed < 140 mph	32	32	34	NP	NP		
	CS-G	24	27	30	33	36	Actual ^b	
	Adjacent clear opening height (inches)							
	≤ 64	24	27	30	33	36		
	68	26	27	30	33	36		
	72	27	27	30	33	36		
	76	30	29	30	33	36]	
	80	32	30	30	33	36] .	
	84	35	32	32	33	36		
	88	38	35	33	33	36		
	92	43	37	35	35	36		
	96	48	41	38	36	36		
CS-WSP, CS-SFB	100		44	40	38	38		
	104		49	43	40	39	Actual ^b	
	108		54	46	43	41		
	112	 -	T =	50	45	43		
	116	 		55	48	45]	
	120	_	 	60	52	48		
	124		T -	-	56	51		
	128	 	 	T	61	54		
	132	 	-		66	58	1	
	136	 	† = -		1 -	62		
	140	1 -	1 -	-	T =	66		
	144	T =	1 =		T -	72		
METHOD			Po	rtal header				
-	able R602,10.4)	8 feet	9 feet	10 feet	11 feet	12 feet		
PFH	Supporting roof only	16	16	16	Note c	Note c	48	
	Supporting one story and roo	f 24	24	24	Note c	Note c		
PFG		24	27	30	Note d	Note d		
	SDC A, B and C	16	18	20	Note e	Note e		
CS-PF	SDC D ₀ , D ₁ and D ₂	16	18	20	Note e	Note e	Actual ^b	

For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm, 1 mile per hour = 0.447 m/s.

NP = Not Permitted.

a. Linear interpolation shall be permitted.

b. Use the actual length where it is greater than or equal to the minimum length.

c. Maximum header height for PFH is 10 feet in accordance with Figure R602.10.6.2, but wall height shall be permitted to be increased to 12 feet with pony wall.

d. Maximum header height for PFG is 10 feet in accordance with Figure R602.10.6.3, but wall height shall be permitted to be increased to 12 feet with pony wall.

e. Maximum header height for CS-PF is 10 feet in accordance with Figure R602.10.6.4, but wall height shall be permitted to be increased to 12 feet with pony wall.

BRACE WALL DETAILS WIND SPEED 115 MPH WIND EXPOSURE A SEISMIC DESIGN CAEGORY A

			TABLE R602.10.4—con	linued S	
METHODS, MATERIAL MINIMUM THICKNESS			CONNECTION CRITERIA		
		MINIMUM THICKNESS	FIGURE	Fasteners	Specing
, Methods	PFH Portal frame with hold-downs	³/g"		See Section R602.10.6.2	See Section R602.10.6.2
Intermittent Bracing Methods	PFG Portal frame at garage	7/ ₁₆ "		See Section R602.10.6.3	See Section R602.10.6.3
<u> </u>	CS-WSP	3/8"		Exterior sheathing per Table R602.3(3)	6" edges 12" field
20	Continuously sheathed wood structural panel			Interior sheathing per Table R602.3(1) or R602.3(2)	Varies by fastener
Sheathing Methods	CS-G ^{B,c} Continuously sheathed wood structural panel adjacent to garage openings	3/8″		See Method CS-WSP	See Method CS-WSP
Continuous Sh	CS-PF Continuously sheathed portal frame	7/ ₁₆ "		See Section R602.10.6.4	See Section R602.10.6.4
Conti	CS-SFB ^a Continuously sheathed structural fiberboard	1/2" or ²⁵ / ₃₂ " for maximum 16" stud spacing		1 $\frac{1}{2}$ " long × 0.12" dia. (for $\frac{1}{2}$ " thick sheathing) 1 $\frac{3}{4}$ " long × 0.12" dia. (for $\frac{25}{12}$ " thick sheathing)	3" edges 6" field

For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm, 1 degree = 0.0175 md, 1 pound per square foot = 47.8 N/m², 1 mile per hour = 0.447 m/s.

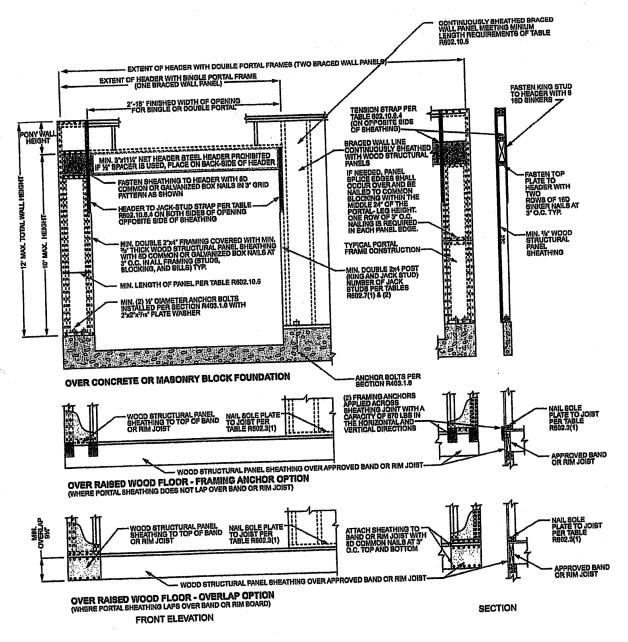
a. Adhesive attachment of wall sheathing, including Method GB, shall not be permitted in Seismic Design Categories C, D₀, D₁ and D₂.

b. Applies to panels next to garage door opening where supporting gable end wall or roof load only. Shall only be used on one wall of the garage. In Seismic Design Categories D₀, D₁ and D₂, roof covering dead load shall not exceed 3 psf.

c. Garage openings adjacent to a Method CS-G panel shall be provided with a header in accordance with Table R602.7(1). A full-height clear opening shall not be permitted adjacent to a Method CS-G panel.

d. Method CS-SFB does not apply in Seismic Design Categories D₀, D₁ and D₂.

e. Method applies to detached one- and two-family dwellings in Seismic Design Categories D₀ through D₂ only.



For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm.

FIGURE R802.10.6.4
METHOD CS-PF—CONTINUOUSLY SHEATHED PORTAL FRAME PANEL CONSTRUCTION

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BUILD ELEVATE DESIGN & BUILL BRIGHTWOOD LOT 48 HOOK FARMS 2036 WHEATFIELD CT LEE SUMMIT MO ELEVATE

SCALE 1/4" = 1-0

DATE 10-25-21

PLAN NO.

3653

SHEET NO.

FOUNDATION LIGHTING PLAN

ELEVATE DESIGN & BUILD BRIGHTWOOD LOT 48 HOOK FARMS 2036 WHEATFIELD CT LEE SUMMIT MO

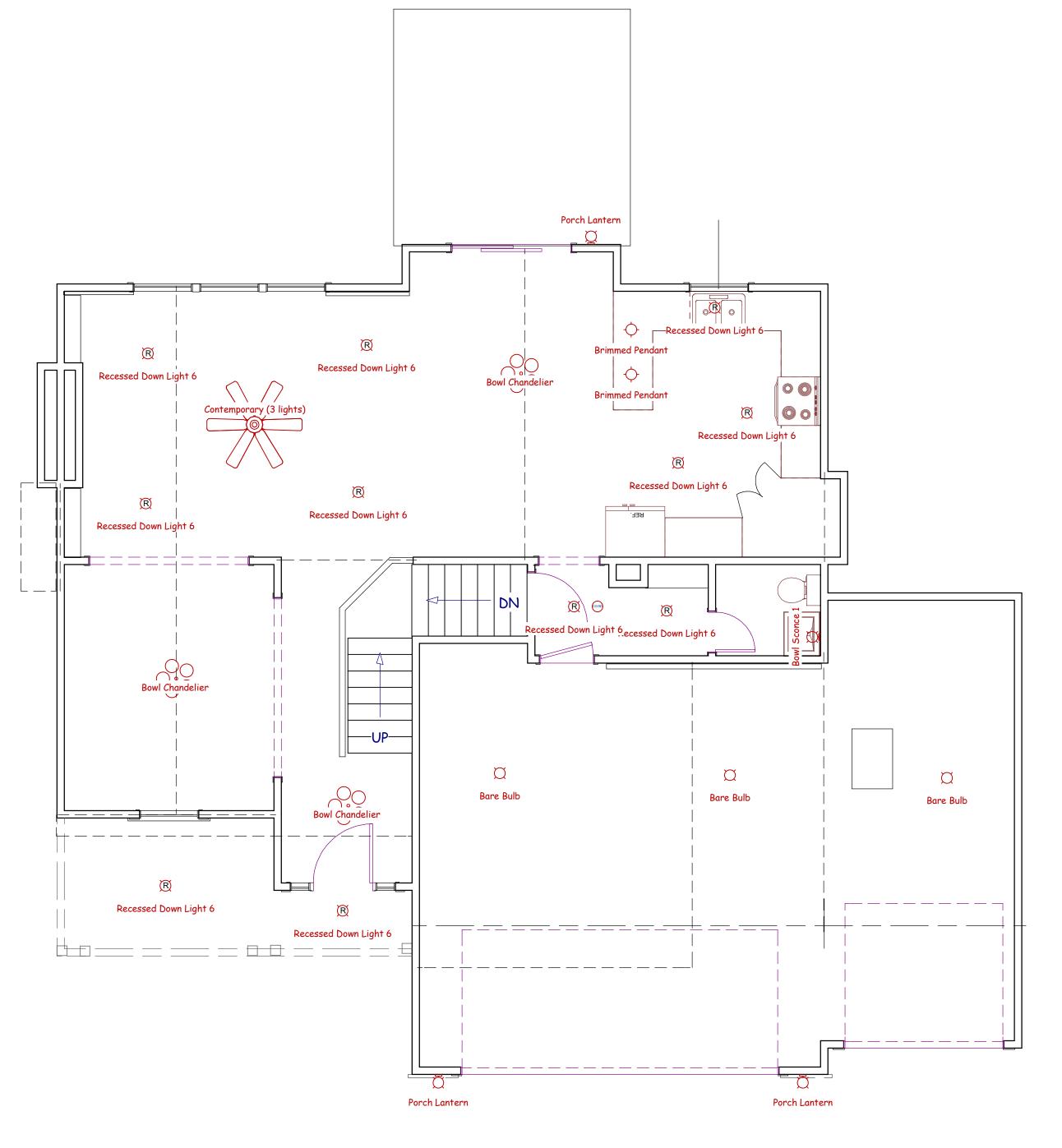
SCALE 1/4" = 1-0

DATE 10-25-21

PLAN NO.

3653

SHEET NO.



MAIN FLOOR LIGHTING PLAN

ELEVATE DESIGN & BUILD BRIGHTWOOD LOT 48 HOOK FARMS 2036 WHEATFIELD CT LEE SUMMIT MO

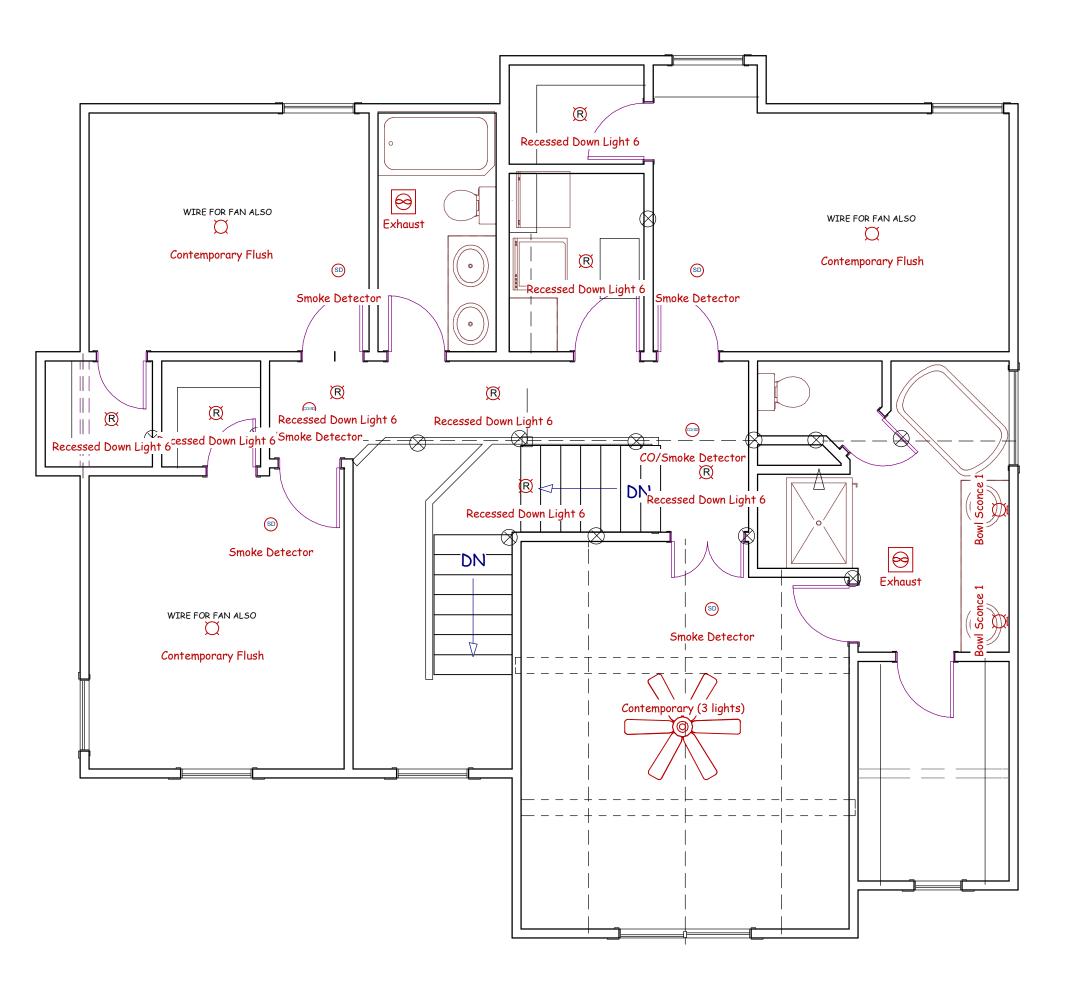
SCALE 1/4" = 1-0

DATE 10-25-21

PLAN NO.

3653

SHEET NO.



SECOND FLOOR LIGHTING PLAN

ELEVATE DESIGN & BUILD BRIGHTWOOD LOT 48 HOOK FARMS 2036 WHEATFIELD CT LEE SUMMIT MO

SCALE 1/4" = 1-0

DATE 10-25-21

PLAN NO.

3653

SHEET NO.