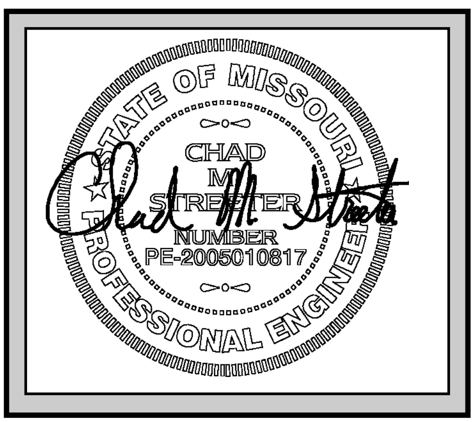
Oakview Lot 4
Lee's Summit, Missouri

VAN DEURZEN AND ASSOCIATES, P.A.
CONSULTING STRUCTURAL ENGINEERS
11011 KING STREET, SUITE 130
COMMERCE TERRACE BUILDING D
OVERLAND PARK, KANSAS 66210
(913) 451 - 6305   FAX (913) 451 - 1021

Reinforced Soil Retaining Wall Design



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Segmental Retaining Wall Design Calculations per NCMA

Wall Geometry

Height	Backslope	Dead Load	Live Load	Distance to Slope	Wall below grade at toe
$H := 5.00 \cdot ft$	$\beta := 0.0 \cdot deg$	$q_d := 0 \cdot psf$	$q_l := 150 \cdot psf$	$Z := 1.0 \cdot ft$	$H_{emb} := .67 \cdot ft$

Soil Properties

Reinforced Soil (Internal)	Retained Soil (External)	Drainage Fill	Foundation Soil	Pullout
$\gamma_i := 110 \cdot pcf$	$\gamma_e := 120 \cdot pcf$	$\gamma_d := 110 \cdot pcf$	$\gamma_f := 120 \cdot pcf$	$C_i := 0.7$
$\phi_i := 32 \cdot deg$	$\phi_e := 26 \cdot deg$	$\phi_d := 34 \cdot deg$	$\phi_f := 26 \cdot deg$	
$C_{dsi} := 0.8$	$C_{dse} := 1.0$		$c_f := 0.0psf$	

Segmental Unit Properties

Height	Length	Width	Setback	Center of Gravity	Batter
$H_u := 8 \cdot in$	$L_u := 18 \cdot in$	$W_u := 12 \cdot in$	$\Delta_u := 0.75 \cdot in$	$G_u := 6 \cdot in$	$\omega := atan\left(\frac{\Delta_u}{H_u}\right)$

$\omega = 5.356 \cdot deg$

Infilled Unit Weight

$\gamma_u := 120 \cdot pcf$

Hinge Height

$$H_h := if\left[\tan(\omega) = 0, H, \left[2 \cdot \frac{(W_u - G_u)}{\tan(\omega)}\right]\right] \Rightarrow H_h = 10.667\ ft \quad \text{[Eq. 4-1]}$$

Internal Interface Friction Angle

$$\delta_i := \frac{2}{3} \cdot \phi_i \qquad \delta_i = 21.33 \cdot deg \qquad \text{[Eq. 3-17]}$$

Internal Active Earth Pressure

$$Ka_i := \frac{\cos(\phi_i + \omega)^2}{\cos(\omega)^2 \cdot \cos(\omega - \delta_i) \cdot \left[1 + \sqrt{\left(\frac{\sin(\phi_i + \delta_i) \cdot \sin(\phi_i - \beta)}{\cos(\omega - \delta_i) \cdot \cos(\omega + \beta)}\right)}\right]^2}$$

[Eq. 3-11] $Ka_i = 0.239$

External Interface Friction Angle

$$\delta_e := if(\phi_i > \phi_e, \phi_e, \phi_i) \qquad \delta_e = 26 \cdot deg \qquad \text{[Eq. 3-16]}$$

External Active Earth Pressure

$$Ka_e := \frac{\cos(\phi_e + \omega)^2}{\cos(\omega)^2 \cdot \cos(\omega - \delta_e) \cdot \left[1 + \sqrt{\left[\frac{\sin(\phi_e + \delta_e) \cdot \sin((\phi_e - \beta))}{\cos(\omega - \delta_e) \cdot \cos(\omega + \beta)}\right]}\right]^2}$$

[Eq. 3-11] $Ka_e = 0.304$

Orientation of Critical Internal Failure Surface

$$\alpha_i := atan\left[\frac{-\tan(\phi_i - \beta) + \sqrt{(\tan(\phi_i - \beta)) \cdot (\tan(\phi_i - \beta) + \cot(\phi_i + \omega)) \cdot (1 + \tan(\delta_i - \omega) \cdot \cot(\phi_i + \omega))}}{1 + \tan(\delta_i - \omega) \cdot (\tan(\phi_i - \beta) + \cot(\phi_i + \omega))}\right] + \phi_i$$

[Eq. 3-14]

$\alpha_i = 55.153 \cdot deg$

Orientation of Critical External Failure Surface

$$\alpha_e := atan\left[\frac{-\tan(\phi_e - \beta) + \sqrt{\tan(\phi_e - \beta) \cdot (\tan(\phi_e - \beta) + \cot(\phi_e + \omega)) \cdot (1 + \tan(\delta_e - \omega) \cdot \cot(\phi_e + \omega))}}{1 + \tan(\delta_e - \omega) \cdot (\tan(\phi_e - \beta) + \cot(\phi_e + \omega))}\right] + \phi_e$$

[Eq. 3-14]

$\alpha_e = 50.164 \cdot deg$

Sliding

External Stability Analysis

Given

$$1.5 = \frac{\min\left[\begin{array}{c} C_{dse}\cdot\left[q_d\cdot\left[(L-W_u-Z)+\frac{(L-W_u-Z)\cdot\tan(\beta)\cdot\tan(\omega)}{1-\tan(\beta)\cdot\tan(\omega)}\right]+L\cdot\gamma_e\cdot H \ldots \\ +\frac{1}{2}\cdot\gamma_e\cdot(L-W_u-Z)\cdot\left[(L-W_u-Z)+\frac{(L-W_u-Z)\cdot\tan(\beta)\cdot\tan(\omega)}{1-\tan(\beta)\cdot\tan(\omega)}\right]\cdot\tan(\beta)\right]\cdot\tan(\phi_e) \\ C_{dse}\cdot\left[q_d\cdot\left[(L-W_u-Z)+\frac{(L-W_u-Z)\cdot\tan(\beta)\cdot\tan(\omega)}{1-\tan(\beta)\cdot\tan(\omega)}\right]+L\cdot\gamma_e\cdot H \ldots \\ +\frac{1}{2}\cdot\gamma_e\cdot(L-W_u-Z)\cdot\left[(L-W_u-Z)+\frac{(L-W_u-Z)\cdot\tan(\beta)\cdot\tan(\omega)}{1-\tan(\beta)\cdot\tan(\omega)}\right]\cdot\tan(\beta)\right]\cdot\tan(\phi_d) \\ C_{dse}\cdot\left[c_f\cdot L+\left[q_d\cdot\left[(L-W_u-Z)+\frac{(L-W_u-Z)\cdot\tan(\beta)\cdot\tan(\omega)}{1-\tan(\beta)\cdot\tan(\omega)}\right]+L\cdot\gamma_e\cdot H \ldots \\ +\frac{1}{2}\cdot\gamma_e\cdot(L-W_u-Z)\cdot\left[(L-W_u-Z)+\frac{(L-W_u-Z)\cdot\tan(\beta)\cdot\tan(\omega)}{1-\tan(\beta)\cdot\tan(\omega)}\right]\cdot\tan(\beta)\right]\cdot\tan(\phi_f)\right] \end{array}\right]}{\left[\frac{1}{2}\cdot Ka_e\cdot\gamma_e\cdot\left[H+\left[(L-W_u-Z)+\frac{(L-W_u-Z)\cdot\tan(\beta)\cdot\tan(\omega)}{1-\tan(\beta)\cdot\tan(\omega)}\right]\cdot\tan(\beta)\right]^2\cdot\cos(\delta_e-\omega)\right] \ldots \\ +(q_d+q_l)\cdot Ka_e\cdot\left[H+\left[(L-W_u-Z)+\frac{(L-W_u-Z)\cdot\tan(\beta)\cdot\tan(\omega)}{1-\tan(\beta)\cdot\tan(\omega)}\right]\cdot\tan(\beta)\right]\cdot\cos(\delta_e-\omega)}$$

$L_{sliding} := \mathrm{Find}(L)$ $\quad$ $L_{sliding} = 3.278\ \mathrm{ft}$

Overturning

Given

$$2.0 = \frac{\left[(L\cdot\gamma_e\cdot H)\cdot\left[\frac{1}{2}\cdot(L+H\cdot\tan(\omega))\right]\right] \ldots \\ +\left[\frac{1}{2}\cdot\gamma_e\cdot(L-W_u-Z)\cdot\left[(L-W_u-Z)+\frac{(L-W_u-Z)\cdot\tan(\beta)\cdot\tan(\omega)}{1-\tan(\beta)\cdot\tan(\omega)}\right]\cdot\tan(\beta)\right]\cdot\left[\left[H\cdot\tan(\omega)+W_u+Z+\frac{2}{3}\cdot(L-W_u-Z)\right]\right. \\ +q_d\cdot\left[(L-W_u-Z)+\frac{(L-W_u-Z)\cdot\tan(\beta)\cdot\tan(\omega)}{1-\tan(\beta)\cdot\tan(\omega)}\right]\cdot\left[\frac{Z+\left[(L-W_u-Z)+\frac{(L-W_u-Z)\cdot\tan(\beta)\cdot\tan(\omega)}{1-\tan(\beta)\cdot\tan(\omega)}\right]}{2}+H\cdot\tan(\omega)+\right.}{\left[\frac{1}{2}\cdot Ka_e\cdot\gamma_e\cdot\left[H+\left[(L-W_u-Z)+\frac{(L-W_u-Z)\cdot\tan(\beta)\cdot\tan(\omega)}{1-\tan(\beta)\cdot\tan(\omega)}\right]\cdot\tan(\beta)\right]^2\cdot\cos(\delta_e-\omega)\right]\cdot\left[\frac{1}{3}\cdot\left[H+\left[(L-W_u-Z)+\frac{(L-W_u-Z)\cdot\tan(\beta)\cdot\mathrm{ta}}{1-\tan(\beta)\cdot\tan(\omega)}\right.\right.\right. \\ +(q_d+q_l)\cdot Ka_e\cdot\left[\left[H+\left[(L-W_u-Z)+\frac{(L-W_u-Z)\cdot\tan(\beta)\cdot\tan(\omega)}{1-\tan(\beta)\cdot\tan(\omega)}\right]\cdot\tan(\beta)\right]\cdot\cos(\delta_e-\omega)\right]\cdot\left[\frac{1}{2}\cdot\left[H+\left[(L-W_u-Z)+\frac{(L-W_u-Z)\cdot\tan(}{1-\tan(\beta)\cdot\mathrm{ta}}\right.\right.\right.}$$

$L_{overturning} := \mathrm{Find}(L)$ $\quad$ $L_{overturning} = 2.654\ \mathrm{ft}$

$$L := \max\left(\begin{pmatrix} L_{sliding} \\ L_{overturning} \\ 0.6\cdot H \end{pmatrix}\right)$$ $\quad$ $L = 3.278\ \mathrm{ft}$

Based on Overturning and Sliding: $L := 5.0\cdot\mathrm{ft}$ *(Round up L)*

Eccentricity

$L' := L - W_u - Z$ — $L' = 3 \cdot ft$ [Fig. 2-10] [Eq. 5-1]

$$L'' := \frac{(L - W_u - Z) \cdot \tan(\beta) \cdot \tan(\omega)}{1 - \tan(\beta) \cdot \tan(\omega)}$$ $L'' = 0\,ft$ [Fig. 2-10] [Eq. 5-2]

$L_\beta := L' + L''$ — $L_\beta = 3\,ft$ [Fig. 2-10] [Eq. 5-3]

$h := L_\beta \cdot \tan(\beta)$ — $h = 0\,ft$ [Fig. 2-10] [Eq. 5-4]

$W_{ri} := L \cdot \gamma_i \cdot H$ — $W_{ri} = 2750 \cdot plf$ [Eq. 5-15]

$X_{ri} := \frac{1}{2} \cdot (L + H \cdot \tan(\omega))$ — $X_{ri} = 2.734\,ft$ [Eq. 5-19]

$W_{r\beta} := \frac{1}{2} \cdot \gamma_i \cdot (L' - Z) \cdot h$ — $W_{r\beta} = 0 \cdot plf$ [Eq. 5-16]

$X_{r\beta} := H \cdot \tan(\omega) + W_u + \frac{2}{3} \cdot L_\beta + Z$ — $X_{r\beta} = 4.469\,ft$ [Eq. 5-20]

$X_{q\beta} := \frac{Z + L_\beta}{2} + [(H + h) \cdot \tan(\omega)] + W_u$ — $X_{q\beta} = 3.469\,ft$ [Eq. 5-21]

Actual Heigth of wall:

$H_s := (H + h)$ — $H_s = 5\,ft$

Earth Pressures:

$P_{sH} := \left[\frac{1}{2} \cdot Ka_e \cdot \gamma_e \cdot (H + h)^2 \cdot \cos(\delta_e - \omega)\right]$ — $P_{sH} = 426.284 \cdot plf$ [Eq. 5-6]

$Y_s := \frac{1}{3} \cdot (H + h)$ — $Y_s = 1.667\,ft$ [Eq. 5-9]

$P_{qH} := (q_d + q_l) \cdot Ka_e \cdot (H + h) \cdot \cos(\delta_e - \omega)$ — $P_{qH} = 213.142 \cdot plf$ [Eq. 5-8]

$Y_q := \frac{1}{2} \cdot (H + h)$ — $Y_q = 2.5\,ft$ [Eq. 5-10]

$$e := \frac{\left[P_{sH} \cdot Y_s + P_{qH} \cdot Y_q - W_{ri} \cdot \left(X_{ri} - \frac{L}{2}\right) - W_{r\beta} \cdot \left(X_{r\beta} - \frac{L}{2}\right) - q_d \cdot (L_\beta) \cdot \left(X_{q\beta} - \frac{L}{2}\right)\right]}{W_{ri} + W_{r\beta} + q_d \cdot (L_\beta)}$$ $e = 0.2177\,ft$ [Eq. 5-25]

Check $e := \text{if}(e \le 0, 0.075L, e)$ — $e = 0.218\,ft$

Surcharge is applied over: $(L' + L'') = 3\,ft$ — $B := L - 2 \cdot e$ — $B = 4.565\,ft$ [Eq. 5-24]

Bearing Capacity

$$Q_a := \frac{\left[W_{ri} + W_{r\beta} + (q_d + q_l)\cdot(L' + L'')\right]}{B}$$ $Q_a = 701.061 \cdot psf$

$$N_q := \tan\left(45\cdot deg + \frac{\phi_f}{2}\right)^2 \cdot \exp(\pi\cdot\tan(\phi_f))$$ $N_q = 11.854$ [Fig. 4-5]

$$N_c := if\left[\phi_f = 0, 5.14, (N_q - 1)\cdot\cot(\phi_f)\right]$$ $N_c = 22.254$ [Fig. 4-5]

$$N_\gamma := 2\cdot(N_q + 1)\cdot\tan(\phi_f)$$ $N_\gamma = 12.539$ [Fig. 4-5]

$$Q_{ult} := c_f\cdot N_c + \frac{1}{2}\cdot\gamma_f\cdot B\cdot N_\gamma + \gamma_f\cdot H_{emb}\cdot N_q$$ $Q_{ult} = 4387.095 \cdot psf$ [Eq. 4-20]

$$FS_{bearing} := \frac{Q_{ult}}{Q_a}$$ $FS_{bearing} = 6.258$ [Eq. 4-19]

Internal Stability

Reinforcement Properties

Geogrid Design Data

Backfill Soil $Type := (gravel)$

Geogrid Number

	1	2	3	4	5	6	7	8	9
$Type^T =$	411	834	1199	1336	2004	2508	3011	3873	7914
$inter^T =$	1145	1145	1145	1145	1145	1145	0		
$slope^T =$	38	38	38	38	38	38	0		
$maxc^T =$	4540	4540	4540	4540	4540	4540	0		

$GN1 := 2$ $GN2 := 2$

$x := 4..1$ x is the number of grids at the top of the wall of a different type

$T_a := Type_{GN1}\cdot plf$ $T_a = 834\cdot plf$ $T_{a2} := Type_{GN2}\cdot plf$ $T_{a2} = 834\cdot plf$

$a_{cs} := inter_{GN1}\cdot plf$ $a_{cs} = 1145\cdot plf$ $\lambda_{cs} := slope_{GN1}\cdot deg$ $\lambda_{cs} = 38\cdot deg$ $V_{csmax} := maxc_{GN1}\cdot plf$ $V_{csmax} = 4540\cdot plf$

$a_{cs2} := inter_{GN2}\cdot plf$ $a_{cs2} = 1145\cdot plf$ $\lambda_{cs2} := slope_{GN2}\cdot deg$ $\lambda_{cs2} = 38\cdot deg$ $V_{csmax2} := maxc_{GN2}\cdot plf$ $V_{csmax2} = 4540\cdot plf$

Tension in Geogrid

Number of Grids: $n_g := 2$

Grig Spacing (ft): $Spacing1 := 2$ $Spacing2 := 1.67$

# of grids for that spacing: $n_1 := 2$ $n_2 := 0$

Make all zero when using one geogrid

Depth of first grid (ft): $h_1 := 1.67$

Length of grids: $L_1 := 5.0$ $L_2 := 5.0$ $L_r := \frac{L}{ft}$

Note: make sure that the elevations don't excide the height of the wall (H) $H = 5\,ft$

$top := length(E)$ $p := 2..\,top$ $top = 2$

$grids := length(E)$ $n := 1..\,top$ $l := 1..\,grids - 1$

$E = \begin{pmatrix} 1.67 \\ 3.67 \end{pmatrix} ft$

$T_{a_x} := T_{a2}$ $T_{a_x} = \begin{pmatrix} 834 \\ 834 \\ 834 \\ 834 \end{pmatrix} \cdot plf$

$T_a := \frac{\overrightarrow{L \cdot T_a}}{L}$ $T_a = \begin{pmatrix} 834 \\ 834 \end{pmatrix} \cdot plf$

$L = \begin{pmatrix} 5 \\ 5 \end{pmatrix} ft$

$T_a^{\,T} = (834 \quad 834 \quad 834 \quad 834) \cdot plf$

$D_p := \frac{E_{p-1} + E_p}{2}$ $D_1 := 0 \cdot ft$ $D_{grids+1} := H$

$D^T = (0 \quad 2.67 \quad 5)\,ft$

Total Applied Tensile Strength in the Geosynthetic reinf.:

$$F_{g_n} := \int_{D_n}^{D_{(n+1)}} \left(\gamma_i \cdot D + q_l + q_d\right) \cdot Ka_i \cdot \cos\left(\delta_i - \omega\right) dD$$ [Eq. 5-36]

$F_g^{\,T} = (181.928 \quad 305.837) \cdot plf$

Safety factor:

$FS_{ten_n} := \frac{T_{a_n}}{F_{g_n}}$ $FS_{ten}^{\,T} = (4.584 \quad 2.727)$

Pullout Capacity

Anchorage Length of Geosynthetic

$$La_n := L_n - W_u - \left[(H+h) - E_n\right]\cdot\tan(90\cdot deg - \alpha_i) + \left[(H+h) - E_n\right]\cdot\tan(\omega)$$ [Eq. 5-46]

$La^T = (1.994 \quad 3.199)\ ft$

<u>Note</u>: If the anchorage length is less than 1ft then there is not enough embedment length and it has to be increased. Note that in some cases it might just be the top two grids.

Average Depth of overburden on Anchorage length

$$d_n := E_n + \left[(H - E_n)\cdot\tan(90\cdot deg - \alpha_i) + \frac{La_n}{2} - (Z + H\cdot\tan(\omega) - \Delta_u)\right]\cdot\tan(\beta)$$ [Eq. 5-47]

$d^T = (1.67 \quad 3.67)\ ft$

Anchorage Capacity

$$AC_n := 2\cdot La_n\cdot C_i\cdot(d_n\cdot\gamma_i + q_d)\cdot\tan(\phi_i)$$ [Eq. 5-45]

$AC^T = (320.399 \quad 1129.663)\cdot plf$

$F_g^T = (181.928 \quad 305.837)\cdot plf$

Safety Factor

$$FS_{po} := \overrightarrow{\frac{AC}{F_g}}$$ [Eq. 5-44]

$FS_{po}^T = (1.761 \quad 3.694)$

Internal Sliding

Reduced reinforcement length

$$\Delta L_{l+1} := \begin{cases} \left[(E_{l+1} - E_l)\cdot\left(\frac{1}{\tan(\alpha_e)} - \tan(\omega)\right)\right] & \text{if } n_g > 2 \\ Spacing1\cdot ft\cdot\left(\frac{1}{\tan(\alpha_e)} - \tan(\omega)\right) & \text{if } n_g = 2 \\ 0 & \text{if } n_g = 1 \end{cases}$$ [Eq. 5-51]

$\Delta L^T = (0 \quad 1.481)\ ft$

$$L'_{s_n} := L_n - W_u - \Delta L_n$$ [Eq. 5-50]

$L'^T_s = (4 \quad 2.519)\ ft$

Length of sloping ground

$$L_{s\beta_n} := L'_{s_n} + \frac{(L'_{s_n} - W_u)\cdot\tan(\beta)\cdot\tan(\omega)}{1 - \tan(\beta)\cdot\tan(\omega)} - Z$$ [Eq. 5-53 & 5-52]

$L_{s\beta}^{T} = (3 \quad 1.519)\ ft$

Height of slope above crest of wall

$h'_n := L_{s\beta_n} \cdot \tan(\beta)$ [Eq. 5-54]

$h'^{T} = (0 \quad 0)\ ft$

Weight of reduced reinforced area

$W'_{ri_n} := L'_{s_n} \cdot E_n \cdot \gamma_i$ [Eq. 5-55]

$W'_{ri}^{T} = (735 \quad 1017) \cdot plf$

Weight of wedge beyond reinforced soil zone

$W'_{r\beta_n} := \frac{1}{2} \cdot \left(\left(L_{s\beta_n} \cdot h'_n \right) \right) \cdot \gamma_i$ [Eq. 5-56]

$W'_{r\beta}^{T} = (0 \quad 0) \cdot plf$

Friction developed by weight

$R'_{s_n} := C_{dsi} \cdot \left[q_d \cdot \left(L_{s\beta_n} + Z \right) + W'_{ri_n} + W'_{r\beta_n} \right] \cdot \tan\left(\phi_i\right)$ [Eq. 5-49]

$R'_{s}^{T} = (367 \quad 508) \cdot plf$

Shear capacity of facing elements

$V_{u_n} := \min\left[V_{csmax}, a_{cs} + \left(\text{if}\left(E_n > H_h, H_h, E_n \right) \cdot \gamma_u \cdot W_u \right) \cdot \tan\left(\lambda_{cs} \right) \right]$ [Eq. 4-25]

$V_{u}^{T} = (1302 \quad 1489) \cdot plf$

Driving Forces

From retained soil

$P_{s_n} := \left[\frac{1}{2} \cdot Ka_e \cdot \gamma_e \cdot \left(E_n + h'_n \right)^2 \cdot \cos\left(\delta_e - \omega \right) \right]$ [Eq. 5-6]

$P_{s}^{T} = (48 \quad 230) \cdot plf$

From surcharge

$P_{q_n} := \left(q_d + q_l \right) \cdot Ka_e \cdot \left(E_n + h'_n \right) \cdot \cos\left(\delta_e - \omega \right)$ [Eq. 5-8]

$P_{q}^{T} = (71 \quad 156) \cdot plf$

Factor of safety against internal sliding

$P_{a_n} := P_{s_n} + P_{q_n}$ [Eq. 5-11]

$P_{a}^{T} = (119 \quad 386) \cdot plf$

$FS_{sl_n} := \frac{R'_{s_n} + V_{u_n}}{\left(P_{a_n} \right)}$ [Eq. 5-48]

$FS_{sl}^{T} = (14.055 \quad 5.173)$

Local Stability of Facing Units

Facing Connection Strength

$$T_{conn_n} := \min\left[V_{csmax_n}, a_{cs_n} + \left(\mathrm{if}\left(E_n > H_h, H_h, E_n\right)\cdot\gamma_u\cdot W_u\right)\cdot\tan\left(\lambda_{cs_n}\right)\right]$$ [Eq. 5-59]

$$T_{conn}^T = (1302 \quad 1489)\cdot plf$$

$$FS_{conn_n} := \frac{T_{conn_n}}{F_{g_n}}$$ $$FS_{conn}^T = (7.154 \quad 4.869)$$

Resistance to Bulging

Shear capacity at each geogrid layer

$$V_{u_n} := \min\left[V_{csmax}, a_{cs} + \left(\mathrm{if}\left(E_n > H_h, H_h, E_n\right)\cdot\gamma_u\cdot W_u\right)\cdot\tan\left(\lambda_{cs}\right)\right]$$ [Eq. 4-25]

$$V_u^T = (1302 \quad 1489)\cdot plf$$

Driving Force at each geogrid layer

$$P_{a_n} := \left[\frac{1}{2}\cdot Ka_i\cdot\gamma_i\cdot\left(E_n\right)^2\cdot\cos\left(\delta_i - \omega\right)\right] + \left(q_d + q_l\right)\cdot Ka_i\cdot\left(E_n\right)\cdot\cos\left(\delta_i - \omega\right)$$ [Eq. 5-11]

$$P_a^T = (93 \quad 296)\cdot plf$$

Sum of tension in reinforcement layers above layer being considered

$$F_{n+1} := \sum_{i=1}^{n} F_{g_i}$$

$$F^T = (0 \quad 182 \quad 488)\cdot plf$$

$$FS_{sc_n} := \frac{V_{u_n}}{P_{a_n} - F_n}$$ [Eq. 5-61]

$$FS_{sc}^T = (14.04 \quad 13.008)$$

Maximum unreinforced height of SRW units

$y := E_1 = 1.67\ ft$ $q_l := 0\cdot psf$

Moment equilibrium

Driving Moments

$$P'_s := \left[\frac{1}{2}\cdot Ka_i\cdot\gamma_i\cdot(y)^2\cdot\cos\left(\delta_i - \omega\right)\right]$$ [Eq. 4-5] $$P'_s = 35.208\cdot plf$$

$P'_q := (q_d + q_l) \cdot Ka_i \cdot (y) \cdot \cos(\delta_i - \omega)$ [Eq. 4-6] $P'_q = 0 \cdot plf$

$P'_a := P'_s + P'_q$ [Eq. 4-4] $P'_a = 35.208 \cdot plf$

$Y'_s := \frac{1}{3} \cdot y$ [Eq. 4-7] $Y'_s = 0.557\ ft$

$Y'_q := \frac{1}{2} \cdot y$ [Eq. 4-8] $Y'_q = 0.84\ ft$

$M'_o := P'_s \cdot Y'_s + P'_q \cdot Y'_q$ [Eq. 4-17] $M'_o = 19.6 \cdot lbf$

Resisting Moments

$W'_w := y \cdot \gamma_u \cdot W_u$ [Eq. 4-9] $W'_w = 200.4 \cdot plf$

$X'_w := G_u + \frac{1}{2} \cdot (y) \cdot \tan(\omega)$ [Eq. 4-16] $X'_w = 0.578\ ft$

$M'_r := W'_w \cdot X'_w$ [Eq. 4-15] $M'_r = 115.888\ ft \cdot plf$

$FS_{ot} := \frac{M'_r}{M'_o}$ [Eq. 4-14] $FS_{ot} = 5.913$

Factor of Safety against Shear failure

$V'_u := a_{cs} + W'_w \cdot \tan(\lambda_{cs})$ [Eq. 4-25] $V'_u = \begin{pmatrix} 1301.57 \\ 1301.57 \\ \vdots \end{pmatrix}$

$FS_{sh} := \frac{V'_u}{P'_a}$ [Eq. 4-27] $FS_{sh} = \begin{pmatrix} 36.967 \\ 36.967 \\ 36.967 \\ 36.967 \end{pmatrix}$

Summary

Wall Height	$H = 5\ ft$	
Unreinforced Stability	$FS_{ot} = 5.913$	$FS_{bearing} = 6.258$
Applied Bearing Stress	$Q_a = 701 \cdot psf$	

Grid Elevation	Geogrid Length	Tensile Force	Geogrid Strength	Anch. Length	Anch. Capacity	FS Grid Tension (1.0)	FS Pullout (1.5)	FS Int Sliding (1.5)	FS Conn (1.5)	FS Bulging (1.5)
$E_n =$	$L_n =$	$\frac{Fg_n}{plf} =$	$\frac{Ta_n}{plf} =$	$La_n =$	$\frac{AC_n}{plf} =$	$FS_{ten_n} =$	$FS_{po_n} =$	$FS_{sl_n} =$	$FS_{conn_n} =$	$FS_{sc_n} =$
1.67 ft	5 ft	182	834	1.99 ft	320	4.58	1.76	14.05	7.15	14.04
3.67	5	306	834	3.2	1130	2.73	3.69	5.17	4.87	13.01