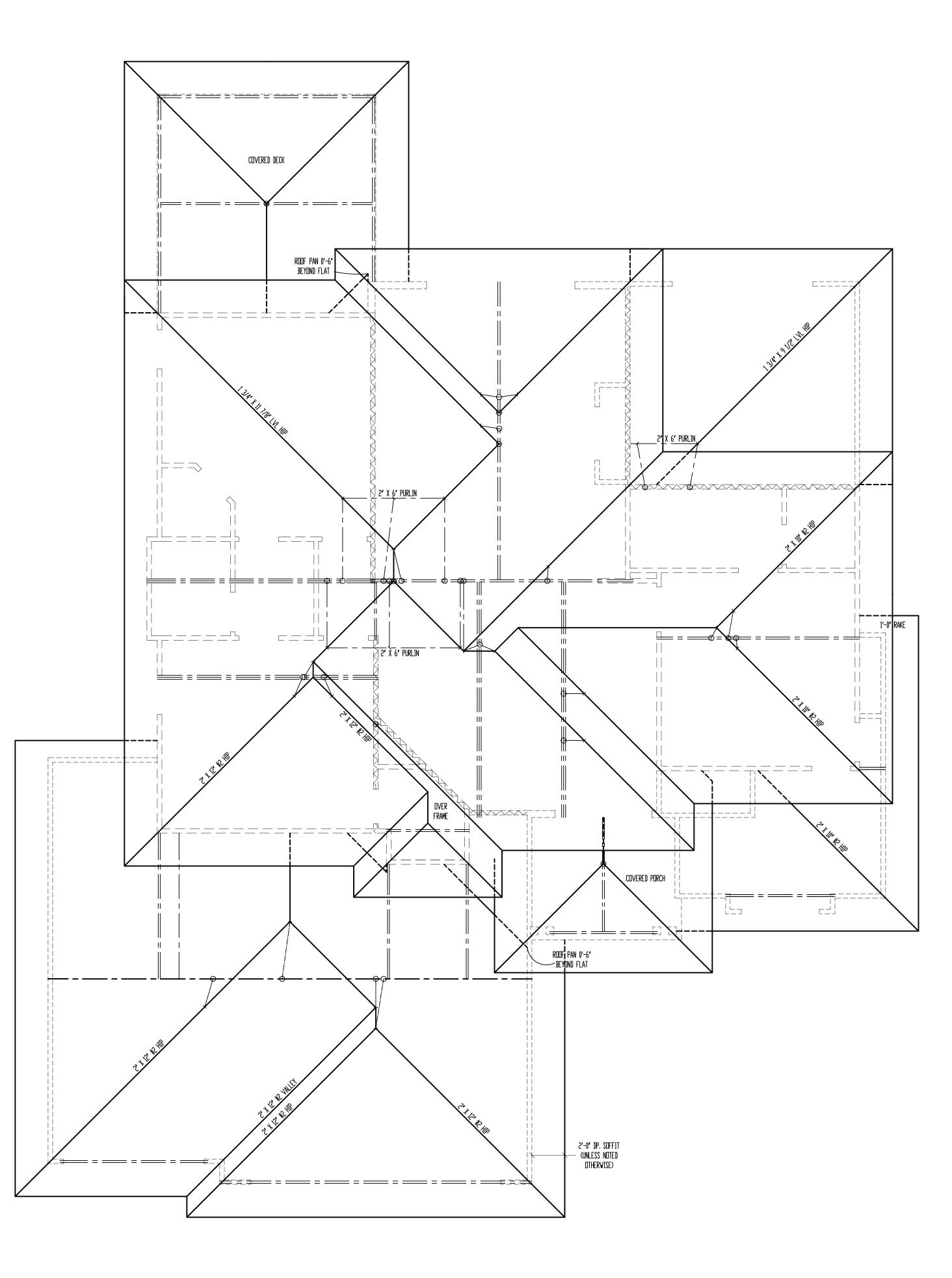


RELEASE FOR CONSTRUCTION AS NOTED FOR PLAN REVIEW DEVELOPMENT SERVICES LEE'S SUMMIT, MISSOURI 10/20/2021



Flashing Note:

RODF NOTES: RODF DESIGNED FOR LIGHT RODF COVERING 30psf TOTAL LOAD [10psf DL, 20psf LL (SL)]

* RAFTERS (HEM-FIR, DDUG-FIR, DR EQUAL): SEE SPAN CHARTS BELDW

| | CODE MINI | MUM |
|-------------------------|-----------|---------------|
| | RAFTERS | SPACING |
| | #2-2x6 | 024″ D.C. |
| $\rangle\rangle\rangle$ | #2-2x6 | @16″ D.C. |
| | #2-2x8 | 024″ D.C. |
| | #2-2x8 | @16″ D.C. |
| | #2-2x10 | 024″ D.C. |
| | #2-2x10 | @16″ D.C. |
| | NOTE: COD | e minimum ali |

| HIGHER PE | RFORMANCE (R | ECOMME |
|-----------|------------------|--------|
| RAFTERS | SPACING | MAX |
| #2-2x6 | @24″ D.C. | |
| #2-2x6 | @16″ D.C. | |
| #2-2x8 | @24″ D.C. | |
| #2-2x8 | @16″ D.C. | |
| #2-2x10 | @24″ D.C. | |
| #2-2x10 | @16' D.C. | |
| DEFLECTIO | N = L/360 LI | VE LOA |

* VAULTS TO BE 2x10 DEPTH * RIDGE BOARDS ARE: (UNLESS OTHERWISE NOTED) - #2- 2X8 UP TO 10/12 PITCH - #2- 2X10 DVER 10/12 PITCH * ALL HIPS & VALLEYS ARE: (UNLESS DTHERWISE NOTED) - #2- 2X8 UP TO 10/12 PITCH - #2- 2X10 OVER 10/12 PITCH * PURLINS ARE 2X6 MIN. - Purlin struts are at 4'-0" D.C. – PURLIN STRUTS SHALL BE INSTALLED AT NOT LESS THAN A 45 Degree angle with the Horizontal - ALL PURLINS STRUTS SHALL HAVE A MAXIMUM UNBRACED LENGTH DF 8'-0' - PURLINS STRUTS SHALL BE CONSTRUCTED IN A 'T' Configuration and per the following chart:

PURLIN STRUT (2) 2x4 (1) 2x4 & (1) 2x6 1) 2x6 & (1) 2x8 (2) 2x6 & (1) 2x8 CONSULT ARCH./ENGR. >

* RIDGE BRACES ARE SAME AS PURLIN BRACES-SPACING, SIZE, CONFIGURATION, & INSTALLATION (SEE PURLIN BRACE NOTES ABOVE) * HIP & VALLEY BRACES ARE SAME AS PURLIN SIZE, CONFIGURATION, & INSTALLATION (SEE PURLIN BRACE NOTES ABOVE)

* Vertical brace if dot is under hip or valley * Slash is top end of brace (/), DOT IS BOTTOM OF BRACE (o). * ~~~~~ DENDITES BEARING WALL *----- DENDTES ROOF BRACE *----- Denotes purlin *----- DENDTES BEARING STRUCTURE



*ALL RAFTERS SHALL BE 2" X 6" #2 @ 16" D.C., UNLESS NOTED OTHERWISE. see detail 7/s3.2 for alternate rafter bearing detail when rafters are required to bear higher than the wall double top plate.

DRIP EDGE, VALLEYS AND FLASHINGS TO BE METAL CLAD.

| MAX HORIZONTAL CLEARSPAN | |
|--------------------------|-------|
| 11′-7 ′ | |
| 14'-2 ' |) (((|
| 14′-8 ″ | |
| 17'-11 ' | |
| 17′-10 ″ | |

| 21'-11" | | | | | |
|----------------------------|----|-------|-------|------|--|
| /s for a rafter deflection | ٦F | L/180 | TOTAL | LDAD | |

| OMME | ENDED) | |
|------|----------------|-----------|
| MAX | HORIZONTAL | CLEARSPAN |
| | 8'-6 ' | |
| | 9′-9 ′ | |
| | 11'-3 ' | |
| | 12'-9' | |

. <u>14'-3'</u> . <u>16'-3'</u> LIVE LOAD, L/240 TOTAL LOAD

| MAX PURLIN STRUT LENGTH |
|-------------------------|
| 8′-0 ′ |
| 12'-0 ' |
| 20'-0 ' |
| 30'-0 ' |
| 30'-0 ' |



ne PI. souri

0k

Site Description: Lot 53, The Retreat at Hoo

street Address: **2812 SW Hearthston** Lee's Summit, Miss

LLC

Wa

DENNIS HEIER

10-18-202

Date: <u>10 - 14 - AD 2021</u> Rev. 1:

Sheet Title: **ROOF PLAN**

Sheet No.:

A-2of 5

5

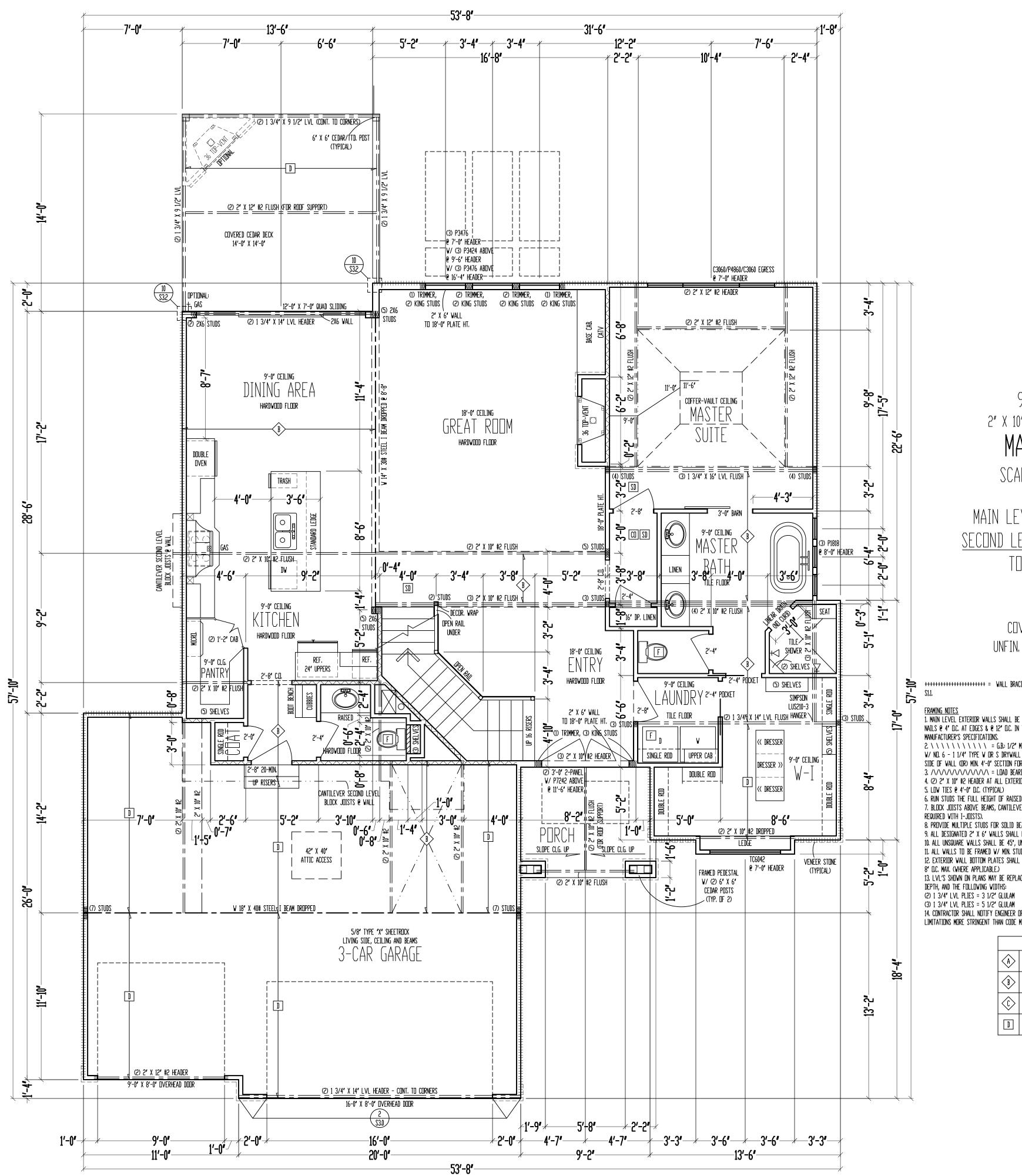
Rev. 2: Rev. 3:

Plan Name: RHF053 Spec General Contractor: Iker Custom Homes, L

"For God so loved the world, that he gave his only begotten Son, that whosoever believeth in him should not perish, but have everlasting life" (John 3:16).

rt have gone into the c t an architect or engin thout the assistance c e impossibility of any c ssign, LLC, and Desig stural failures, due to a Care and effort P designer is not a undertaken witho Because of the in Residential Desi including structu blueprints. Also, does not warram architect to detei





| 9'-0" CEILING | |
|---|---|
| IO" FLOOR SYSTEM ABOVE AIN LEVEL ALE: 1/4" = 1'-0" | |
| EVEL: 1673 SQ. FT. <u>EVEL: 827 SQ. FT.</u> DTAL: 2500 SQ. FT. | |
| GARAGE: 751 SQ. FT. JV. DUT/LIV: 196 SQ. FT. N. BASEMENT 1419 SQ. FT. | |
| acing per framing note #1 and per calculations on sheet | Site Description |
| BE SHEATHED W/ 7/16' D.S.B. A.P.A. PANELS W/ 8d COMMON In The Field. Smart Panel, or Equal, installed per " Min. Gypsum Board over Studs spaced 24" Max fastened All Screws @ 7" D.C. Edges & Field. (Min. 8'-0" sections one For Both Sides) Caring Interior Wall. Erior and Load Bearing Walls, unless noted otherwise. | |
| SED PLATE WALLS. Evers and LDAD bearing walls with JDIST Material (NDT | |
| Bearing Below All Beams. Ll have double king studs at door and window openings. , unless noted otherwise. Stud grade 2" X 4"s @ 16" D.C., unless noted otherwise. Ll Be nailed to framing Below with 16d common nails @ | |
| laced with dF/dF grade 24F-V4 glulam beams of the same | |
| R OF RECORD BEFORE CONSTRUCTION OF ANY DEFLECTION E MINIMUMS ABOVE ANY OPENINGS. | |
| JOIST SCHEDULE 2" X 10" #2 TTD. FLOOR JOIST | |
| e 16" D.C. | |
| 2" X 10" #2 FLOOR JOIST e 16" D.C. | |
| 2" X 10" #2 FLOOR JOIST e 16" D.C DOUBLE EVERY OTHER | |
| 2" X 6" #2 CEILING JOIST @ 16" D.C. | |
| | The I among the second |
| | |

| | Site Description: | | | These plans and specifications are protected under federal copyright laws. Copyright A.D. 2021 Viewpoint Residential Design, LLC. |
|-------------|---|---|--|---|
| RHF053 Spec | Lot 53, The Retreat at Hook Farms | | "For God so loved the world, that he gave his only begotten Son, that | Care and effort have gone into the creation and design of this plan. However, the designer is not an architect or engineer and construction from these plans should not be undertaken without the assistance of a construction professional, architect or engineer. |
| | Street Address: | | whosoever belleveth in him should not perish, but have everlasting life" | Because of the impossibility of any on site consultation and supervision, Viewpoint Residential Design, LLC, and Designer assume no responsibility for any damages, including structural failures, due to any deficiencies, omissions or error in the design or |
| 2 2 2 | zorz Sw nearmstone Pr., Lee's Summit. Missouri | Office: (816) 554-0400 Email: admin@viewpointdesign.net | (John 3:16). | blueprints. Also, site conditions may vary from those illustrated on this plan. Designer does not warrant the suitability of these plans for use on your specific site. Consult your |

Date: <u>10 - 14 - AD 2021</u> Rev. 1:

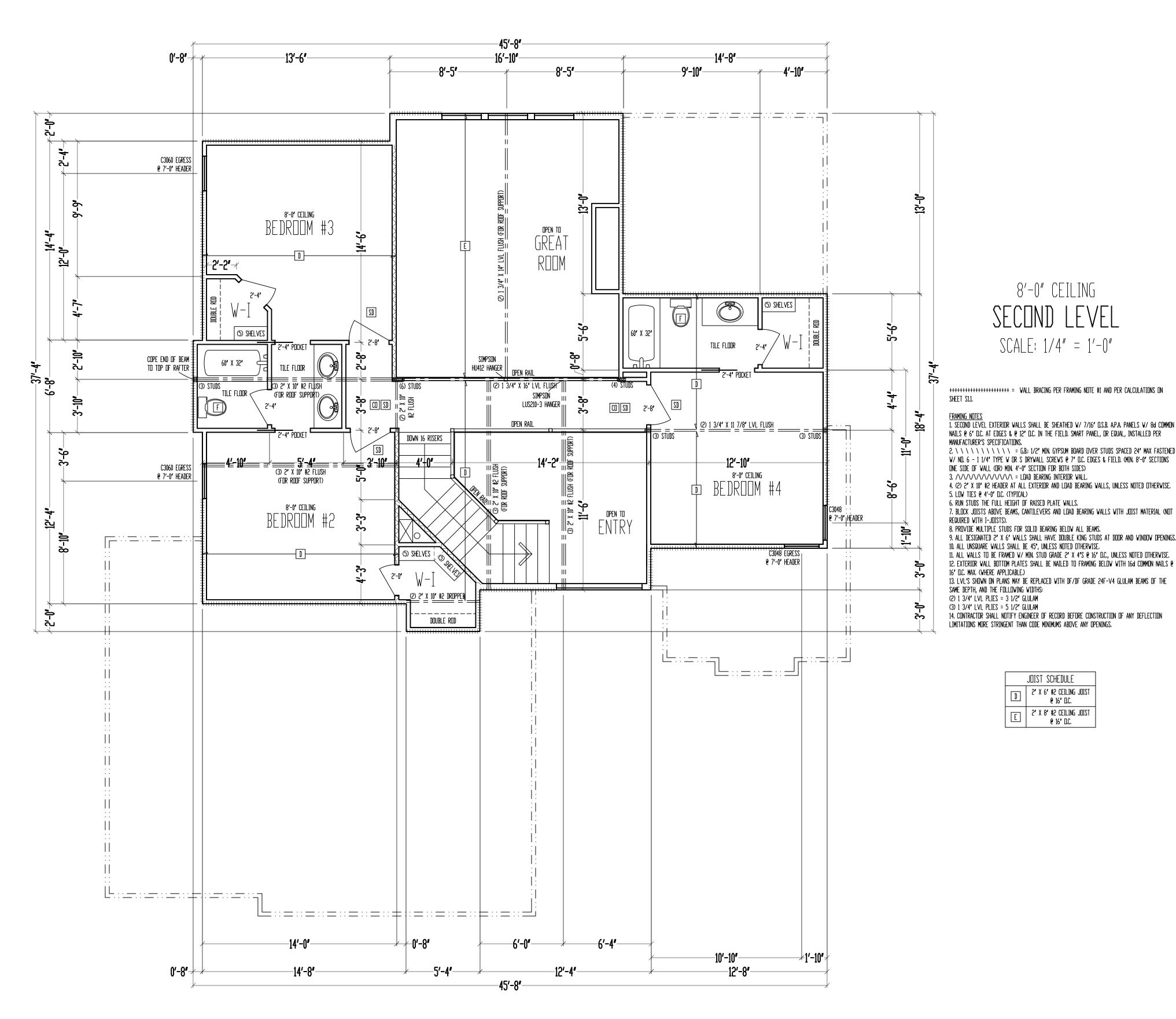
Sheet Title: MAIN LEVEL PLAN

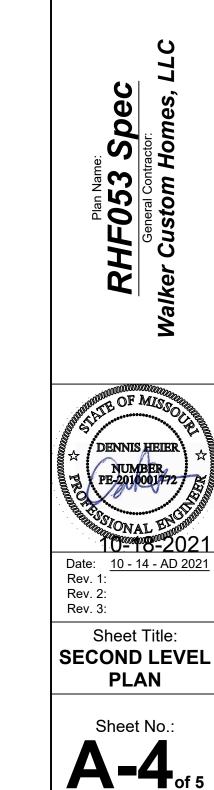
Sheet No.:

A-3 of 5

Rev. 2: Rev. 3:







JOIST SCHEDULE
 D
 2' X 6' #2 CEILING JDIST

 @ 16' L.C.

14. CONTRACTOR SHALL NOTIFY ENGINEER OF RECORD BEFORE CONSTRUCTION OF ANY DEFLECTION LIMITATIONS MORE STRINGENT THAN CODE MINIMUMS ABOVE ANY DPENINGS.

8. PROVIDE MULTIPLE STUDS FOR SOLID BEARING BELOW ALL BEAMS. 9. All designated 2" X 6" Walls shall have double king studs at door and window openings. 10. ALL UNSQUARE VALLS SHALL BE 45°, UNLESS NOTED OTHERWISE. 11. ALL VALLS TO BE FRAMED V/ MIN. STUD GRADE 2′ X 4′S @ 16′ D.C., UNLESS NOTED OTHERWISE. 12. Exterior Vall Bottom Plates Shall be Nailed to Framing Below with 16d common NAILS @

3. ////////////// = LOAD BEARING INTERIOR WALL. 4. (2) 2' X 10' #2 Header at all exterior and Load Bearing Walls, unless noted otherwise. 7. BLOCK JOISTS ABOVE BEAMS, CANTILEVERS AND LOAD BEARING WALLS WITH JOIST MATERIAL (NOT

<u>Framing Notes</u> 1. Second Level Exterior Walls Shall be sheathed W/ 7/16' D.S.B. A.P.A. Panels W/ 8d Common Nails @ 6' D.C. At Edges & @ 12' D.C. In the Field. Smart Panel, or Equal, installed Per

+++++ = Vall bracing per framing note #1 and per calculations on

8'-0" CEILING SECOND LEVEL SCALE: 1/4'' = 1'-0''

"For God so loved the world, that he gave his only begotten Son, that whosoever believeth in him should not perish, but have everlasting life" (John 3:16). **/IEWPOINT** RESIDENTIAL DESIGN LLC

ne PI. souri

Street Address: 2812 SW Hearthsto Lee's Summit, Mis

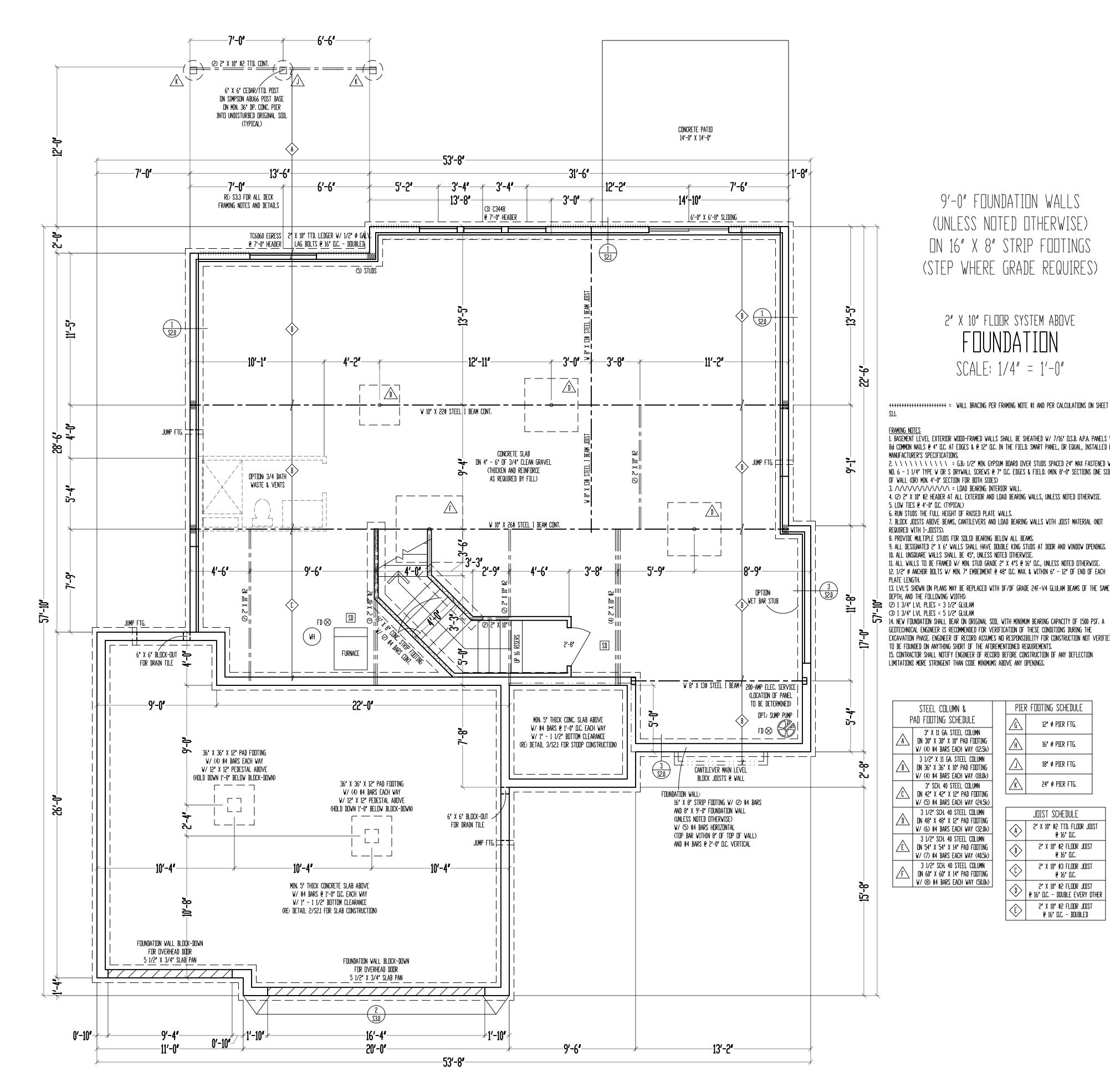
Site Deso Lot a

Retre

The

Care and effort l designer is not a undertaken with Because of the Residential Desi including structu blueprints. Also, does not warran architect to dete





9'-0" FOUNDATION WALLS (UNLESS NOTED OTHERWISE) ON 16" X 8" STRIP FOOTINGS (STEP WHERE GRADE REQUIRES)

> 2" X 10" FLOOR SYSTEM ABOVE FOUNDATION SCALE: 1/4'' = 1'-0''

++ = Wall bracing per framing note #1 and per calculations on sheet

<u>Framing notes</u> 1. Basement level exterior wood-framed walls shall be sheathed w/ 7/16" D.S.B. A.P.A. Panels w/ 8d CDMMDN NAILS @ 4' D.C. AT EDGES & @ 12' D.C. IN THE FIELD. SMART PANEL, DR EQUAL, INSTALLED PER

ND. 6 - 1 1/4" TYPE W DR S DRYWALL SCREWS @ 7" D.C. EDGES & FIELD. (MIN. 8'-0" SECTIONS DNE SIDE

7. BLOCK JOISTS ABOVE BEAMS, CANTILEVERS AND LOAD BEARING WALLS WITH JOIST MATERIAL (NOT

9. All designated 2' X 6' Walls shall have double king studs at door and window openings.

13. LVL'S SHOWN ON PLANS MAY BE REPLACED WITH DF/DF GRADE 24F-V4 GLULAM BEAMS OF THE SAME

14. NEV FOUNDATION SHALL BEAR ON ORIGINAL SOIL WITH MINIMUM BEARING CAPACITY OF 1500 PSF. A Gedtechnical engineer is recommended for verification of these conditions during the excavation phase, engineer of record assumes no responsibility for construction not verified

15, Contractor shall notify engineer of record before construction of any deflection

| | PIER | R FOOTING SCHEDULE | | | |
|------------------------|------------------------------|---|-----|--|--|
| N | Ś | 12 " ø pier ftg. | | | |
| TING 12.5k) | | 16 ' ø pier ftg. | | | |
| JMN ITING 18.0k) | \bigtriangleup | 18' ø pier ftg. | | | |
| N JTING | Ŕ | 24' ø pier ftg. | | | |
| 24.5k) | | | | | |
| JMN Iting | | JOIST SCHEDULE | | | |
| 32.0k) | $\langle A \rangle$ | 2" X 10" #2 TTD, FLOOR JOI @ 16" D.C. | IST | | |
| JMN ITING 40.5k) | $\langle \mathbf{B} \rangle$ | | | | |
| umn Iting | $\langle c \rangle$ | 2" X 10" #3 FLOOR JOIST 0 16" 0.C. | | | |
| 50.0k) | $\langle \mathbb{D} \rangle$ | 2" X 10" #2 Floor Joist @ 16" D.C Double every d | | | |
| | E | 2" X 10" #2 Floor Joist @ 16" 0.C Doubled | | | |



Care and effort designer is not a undertaken with Because of the Residential Des including structu blueprints. Also, does not warrar architect to dete

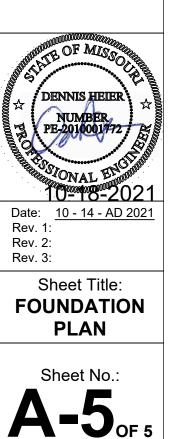
"For God so loved the world, that he gave his only begotten Son, that whosoever believeth in him should not perish, but have everlasting life" (John 3:16).

ne PI. souri 2812 SW Hearthsto Lee's Summit, Mis 53, Ho Site Desc Lot { reat at

Re

The







| DESCRIPTION OF BUILDING ELEN | IENTS | NUMBER AND TYP | PE OF FASTENER | SPAC | SPACING AND LOCATION | |
|--|--|--|--|---|--|--|
| | I | RO | OF ¹ | | | |
| BLOCKING BETWEEN JOISTS OR RAFT PLATE, TOE NAIL | ERS TO TOP | 4-8d (2½" | x 0.113") | | TOENAIL | |
| CEILING JOISTS TO PLATE, TOE | NAIL | 4-8d (2½" | x 0.113") | PE | R JOIST, TOENAIL | |
| CEILING JOISTS NOT ATTACHED TO RAFTER, LAPS OVER PARTITIONS, F | | 4-10d (3" | x 0.128") | | FACE NAIL | |
| CEILING JOIST TO PARALLEL RAFTER (| HEEL JOINT) | TBLE R | 802.5.2 | | FACE NAIL | |
| COLLAR TIE TO RAFTER, FACE NAIL OR RIDGE STRAP TO RAFTER | 1 ¹ / ₄ " x 20 GA. | 4-10d (3" | x 0.128") | FACE | NAIL, EACH RAFTER | |
| RAFTER OR ROOF TRUSS TO P | _ATE | 3-16d BOX NAILS (3½" x 0. NAILS (3" | 135") OR 3-10d COMMON x 0.148") | | NONE SIDE AND 1 TOE NAIL ON E OF EACH RAFTER OR TRUSS | |
| ROOF RAFTERS TO RIDGE, VALLEY RAFTERS OR ROOF RAFTER TO MINIMI BEAM | | 4-16d (3 <u>1</u> " x 0.135") - TOI 0.135") - E | | Т | DENAIL, END NAIL | |
| | · | WA | LL | | | |
| STUD TO STUD (NOT AT BRACED WAL | L PANELS) | 10d (3" x | 0.128") | 16 | 5" O.C. FACE NAIL | |
| STUD TO STUD AND ABUTTING ST NTERSECTING WALL CORNERS (AT BF PANELS) | | 16d (3½" : | x 0.135") | 12 | 2" O.C. FACE NAIL | |
| BUILT-UP HEADER, TWO PIECES WITH | ½" SPACER | 16d (3½" : | x 0.135") | 12" O.C. | EACH EDGE FACE NAIL | |
| CONTINUOUS HEADER TO ST | UD | 4-8d (2½" | x 0.131") | | TOENAIL | |
| TOP PLATE TO TOP PLATE | | 10d (3" x | 0.128") | 12 | 2" O.C. FACE NAIL | |
| DOUBLE TOP PLATE SPLICE | <u>=</u> | 8-16d COMMON | N (3 ¹ / ₂ " x 0.162") | | CH SIDE OF END JOINT (MIN. 24" GTH EACH SIDE OF END JOINT) | |
| BOTTOM PLATE TO JOIST, RIM JOIST, E OR BLOCKING (NOT AT BRACED WAL | | 16d COMMON | (3 ½" x 0.162") | 16 | 5" O.C. FACE NAIL | |
| BOTTOM PLATE TO JOIST, RIM JOIST, E OR BLOCKING (AT BRACED WALL | | 3-16d BOX (3 | 3 ¹ / ₂ " x 0.135") | 3 EAC | H 16" O.C. FACE NAIL | |
| TOP OR SOLE PLATE TO STUD, EN | ID NAIL | 4-8d BOX (2 ½" x 0.113") - T 0.135") - E | | TOENAI | L, END NAIL (SEE LEFT) | |
| TOP PLATES, LAPS AT CORNERS INTERSECTIONS | S AND | 3-10d BOX (| 3" x 0.128") | | FACE NAIL | |
| 1" BRACE TO EACH STUD AND P | LATE | 3-8d BOX (2 | ¹ / ₂ " x 0.113") | FACE NAIL | | |
| 1"x6" SHEATHING TO EACH BEARING | | 3-8d BOX (2 | ¹ / ₂ " x 0.113") | | FACE NAIL | |
| 1"x8" SHEATHING TO EACH BEARING | | 3-8d BOX (2 ½" x 0.113") - F 1"x8" - 4-8d BOX | | | FACE NAIL | |
| | | FLO | OR | | | |
| JOIST TO SILL, TOP PLATE, OR G | IRDER | 4-8d BOX (2 | ¹ / ₂ " x 0.113") | | TOE NAIL | |
| RIM JOIST, BAND JOIST, OR BLOCKING TOP PLATE (ROOF APPLICATIONS | | 8d BOX (2 ¹ / ₂ " x 0.113") | | 4" O.C. TOE NAIL | | |
| 1" x 6" SUBFLOOR OR LESS TO EAC | , | 3-8d BOX (2 ½" x 0.113") | | FACE NAIL | | |
| 2" SUBFLOOR TO JOIST OR GIR | | 3-16d BOX (3 ¹ / ₂ " x 0.135") | | BLIND AND FACE NAIL | | |
| 2" PLANKS (PLAN & BEAM - FLOOR A | ND ROOF) | 3-16d BOX (3 | 3 ¹ / ₂ " x 0.135") | AT EACH BEARING, FACE NAIL | | |
| BAND OR RIM JOIST TO JOIS | т | 3-16d COMMON (3 ¹ / ₂ " x 0.162") | | END NAIL | | |
| BUILT-UP GIRDERS AND BEAMS, 2-INC LAYERS | H LUMBER | 10d BOX (3" x 0.128") | | 24" O.C. FACE NAIL AT TOP AND BOTTOM STAGGERED ON OPPOSITE SIDES | | |
| LEDGER STRIP SUPPORTING JOISTS C | R RAFTERS | 4-16d BOX (3 | 3 ¹ / ₂ " x 0.135") | | IST OR RAFTER, FACE NAIL | |
| BRIDGING OR BLOCKING TO JO | DIST | 2-10d BOX (| 3" x 0.128") | EA | CH END, TOENAIL | |
| ESCRIPTION OF BUILDING MATERIALS WOOD STRUCTURAL PANELS, SUB | • | FASTNER SCHEDULE FOR PTION OF FASTENER | EDGE SPACING (INC | · · · · | ERMEDIATE SUPPORTS (INCHES) ALL SHEATHING TO FRAMING ¹ | |
| 3⁄8" - 1⁄2" | | MON (2" x 0.113") NAIL WALL) 8d COMMON NAIL (ROOF) | 6 | | 12 | |
| | | | | | | |
| ¹⁹ ⁄ ₃₂ " - 1" | 8d COMN | 10N NAIL (2½" x 0.131") | 6 | | 12 | |

| | (2/2 X 0.131) DEFORMED NAIL | | | | |
|--|--|---|-----|--|--|
| OTHER WALL SHEATHING ¹ | | | | | |
| ¹ / ₂ " STRUCTURAL CELLULOSIC FIBERBOARD SHEATHING | 1 $\frac{1}{2}$ " GALVANIZED ROOFING NAIL, $\frac{7}{16}$ " HEAD DIAMETER, OR 1 $\frac{1}{4}$ " LONG 16 GA. STAPLE WITH $\frac{7}{16}$ " OR 1" CROWN | 3 | 6 | | |
| ²⁵ ³² STRUCTURAL CELLULOSIC FIBERBOARD SHEATHING | 1 $\frac{3}{4}$ " GALVANIZED ROOFING NAIL, $\frac{7}{16}$ " HEAD DIAMETER, OR 1 $\frac{1}{2}$ " LONG 16 GA. STAPLE WITH $\frac{7}{16}$ " OR 1" CROWN | 3 | 6 | | |
| ½" GYPSUM SHEATHING | 1½" GALVANIZED ROOFING NAIL; STAPLE GALVANIZED, 1½" LONG; 1¼" SCREWS, TYPE W OR S | 7 | 1 7 | | |
| 5∕8" GYPSUM SHEATHING | 1¾" GALVANIZED ROOFING NAIL; STAPLE GALVANIZED, 1%" LONG; 1%" SCREWS, TYPE W OR S | 7 | 7 | | |

WOOD STRUCTURAL PANELS, COMBINATION SUBFLOOR UNDERLAYMENT TO FRAMING

| ¾" AND LESS | 6d DEFORMED (2" x 0.120") NAIL OR 8d COMMON (2½" x 0.131") NAIL | 6 | 12 |
|---------------|---|---|----|
| 7⁄8" - 1" | 8d COMMON (2½" x 0.131") NAIL OR 8d DEFORMED (2½" x 0.120") NAIL | 6 | 12 |
| 11⁄8" - 11⁄4" | 10d COMMON (3" x 0.148") NAIL OR 8d DEFORMED (2½" x 0.120") NAIL | 6 | 12 |

1. IF INFORMATION LISTED ON PLAN SHEETS CONTRADICTS INFORMATION IN THIS TABLE, INFORMATION ON PLANS TAKES PRECEDENCE OVER INFORMATION

FOUNDATION NOTES

2.

6.

10

12.

15.

16.

17

18.

19.

21

22.

25.

26.

27

28.

29

31.

32

CONCRETE SHALL BE AIR-ENTRAINED BETWEEN 5%-7% WITH A MINIMUM 28-DAY COMPRESSIVE STRENGTH OF 2500 PSI FOR BASEMENT AND INTERIOR FLOOR SLABS-ON-GRADE, 3000 PSI FOR FOUNDATION WALLS, AND 3500 PSI FOR PORCHES AND GARAGE FLOOR SLABS

THE FOUNDATION DESIGN SHALL COMPLY WITH THE ENFORCING JURISDICTION'S RESIDENTIAL FOUNDATION STANDARDS

PROVIDE A MINIMUM 4"-DIAMETER PERFORATED DRAIN PIPE ALONG PERIMETER OF USABLE SPACE AT FOOTING LEVEL OR OTHER EQUIVALENT MATERIALS PER IRC SECTION R405.1. THE PIPE SHALL BE COVERED WITH A MINIMUM OF 6" OF GRAVEL OR CRUSHED ROCK. THE DRAIN SHALL DAYLIGHT BELOW FOOTING LEVEL OR TERMINATE IN A MINIMUM 20 GALLON SUMP PIT.

FOUNDATION SHALL BE DESIGNED FOR A BEARING CAPACITY OF 1500 PSF AND FOUNDED ON COMPETENT ORIGINAL SOIL AS DETERMINED AND CONFIRMED BY A LICENSED GEOTECHNICAL ENGINEER OR ENGINEERING GEOLOGIST. ENGINEER OF RECORD ASSUMES NO RESPONSIBILITY FOR CONSTRUCTION NOT VERIFIED TO BE FOUNDED ON ANY SOIL WITH THE AFOREMENTIONED MINIMUM PROPERTIES.

FOOTINGS SHALL BE A MINIMUM OF 16" WIDE x 8" DEEP AND SHALL HAVE A MINIMUM OF (2) CONTINUOUS GRADE 40 #4 BARS WITH 3" BOTTOM CLERANCE. BOTTOM OF FOOTING SHALL BE LOCATED A MINIMUM OF 3'-0" BELOW GRADE FOR FROST PROTECTION.

CONCRETE PADS SUP0PORTING COLUMN LOADS SHALL BE NO SMALLER THAN 2'-0" x 2'-0" x 1'-0" DEEP WITH A MINIMUM OF (4) GRADE 40 #4 BARS EACH WAY WITH 3" BOTTOM CLEARANCE

FOUNDATION WALLS SHALL BE A MINIMUM OF 8" NOMINAL WIDTH AND SHALL HAVE HOIZONTAL GRADE 40 #4 BARS AT 2'-0" O.C. MAX. WITH VERTICAL #4 BARS AS REQUIRED ON FOUNDATION CROSS SECTION ON SHEET S2.0 REINFORCEMENT SHALL LAP A MINIMUM OF 2'-0" (CLASS B SPLICE)

- INTERIOR BEARING WALLS AND COLUMNS SHALL BE ISOLATED FROM THE BASEMENT FLOOR SLAB
- BASEMENT FLOOR SLAB SHALL BE A MINIMUM OF 4" THICK ON A MINIMUM BASE COURSE OF 4" TO 6" OF SAND,

GRAVEL OR CRUSHED ROCK. BETWEEN THE BASE COURSE AND FLOOR SLAB SHALL BE PLACED A 6-MIL POLY VAPOR RETARDER WITH MINIMUM OVERLAP OF 6" AT DISCONTINUITIES 11. IF A FLOOR IS TO BE SUPPORTED BY A MINIMUM OF 2'-0" OF GRANULAR FILL OR 8" OF EARTH, BASEMENT SLAB

SHALL BE DESIGNED BY A LICENSED ENGINEER SILL PLATES SHALL BE ANCHORED TO THE FOUNDATION WALL WITH $\frac{1}{2}$ " Ø ANCHOR BOLTS EMBEDDED A MINIMUM OF 7" INTO CENTER OF WALL STEM AND SHALL BE INSTALLED AT A MAXIMUM OF 6'-0" O.C. (OR AS NOTED ON PLANS) AND SHALL BE INSTALLED WITHIN 6" TO 12" OF EACH END OF EACH SILL PLATE LENGTH, PER IRC SECTION R403.1.6 FOUNDATION WINDOW WELLS SHALL BE PROVIDED WITH MINIMUM DIMENSIONS AS SHOWN IN DETAIL ON SHEET 13.

14. THE GARAGE FLOOR SHALL SLOPE TOWARD THE VEHICLE DOORS OR TO A TRENCH OR UNTRAPPED DRAIN THAT DISCHARGES TO THE EXTERIOR, ABOVE GRADE

FRAMING NOTES

S2.0

ALL DIMENSIONAL LUMBER SHALL BE DOUGLAS-FIR-LARCH GRADE #2, UNLESS NOTED OTHERWISE ON PLANS ALL INTERIOR LOAD-BEARING AND EXTERIOR WALL HEADERS SHALL BE (2) #2 - 2x10's, UNLESS NOTED OTHERWISE ON PLANS

BLOCK OVER BEAMS AND AT CANTILEVERS AND DOOR JAMBS

INTERIOR NON-BEARING WALLS RESTING ON BASEMENT SLAB SHALL BE ISOLATED FROM ABOVE FRAMING BY A MINIMUM OF 3/5" ALL HEADERS/BEAMS SHALL BEAR ON A MINIMUM OF (2) 2x4 POSTS (KING AND JACK STUDS), UNLESS NOTED

OTHERWISE

20. WHERE JOISTS SPAN PARALLEL TO FOUNDATION, BLOCKING SHALL BE PROVIDED IN THE TWO SPACES MOST ADJACENT TO THE FOUNDATION WALL AT 4'-0" O.C. FOR THE PURPOSE OF TRANSFERRING LATERAL FOUNDATION WALL LOAD TO THE FLOOR DIAPHRAGM. FASTEN JOISTS AND BLOCKING TO SILL PLATE WITH (4) 10d NAILS. IF MECHANICAL DUCTWORK IS INSTALLED IN ONE OF THESE FIRST TWO BAYS, FASTEN 2x4's FLAT AT 4'-0" O.C. BETWEEN JOIST(S) AND/OR SILL AND PROVIDE BLOCKING AS PRESCRIBED ABOVE IN THE NEXT TWO JOIST BAYS. SECURE 2x4's TO JOIST(S)/SILL PLATE WITH (4) 10d NAILS. ALL WOOD MATERIAL SUPPORTED ON CONCRETE OR MASONRY SHALL BE TREATED OR OF DECAY-RESISTANT

MATERIAL JOISTS UNDER BEARING PARTITIONS ON PLANS HAVE BEEN SIZED TO SUPPORT THE DESIGN LOAD.

23. JOISTS FRAMING INTO THE FACE OF A STEEL OR WOOD BEAM SHALL BE SUPPORTED WITH APPROPRIATE COLD-FORMED STEEL JOIST HANGERS

24 JOISTS FRAMED ON TOP OF STRUCTURAL MEMBER SHALL BE SUPPORTED AT EN DS BY FULL-DEPTH SOLID BLOCKING MIN. 1/4" IN THICKNESS OR BY FASTENING RIM TO JOISTS PER FASTENING TABLE TO LEFT ALL WALL COVERINGS SHALL COMPLY WITH IRC SECTION R702.3

ALL RAFTERS AND COLLAR TIES SHALL COMPLY WITH IRC SECTION R802.3.

ALL RAFTERS SHALL HAVE 2x4 COLLAR TIES @ 4'-0" O.C. IN UPPER $\frac{1}{3}$ OF VERTICAL DISTANCE BETWEEN CEILING AND ROOF

BLOCKING BETWEEN JOISTS UNDER A LOAD-BEARING WALL IS NOT REQUIRED PER IRC SECTION 501.3, BOTTOM OF ALL FLOOR ASSEMBLIES ABOVE UNFINISHED AREAS SHALL BE PROVIDED WITH A $\frac{1}{2}$ " GYPSUM BOARD MEMBRANE OR RESIDENTIAL FIRE SPRINKLER SYSTEM WHEN FLOOR SYSTEM IS CONSTRUCTED OF OTHER THAN DIMENSION LUMBER OR STRUCTURAL COMPOSITE LUMBER EQUAL TO OR GREATER THAN 2x10 NOMINAL DIMENSION(WHERE REQUIRED BY ENFORCING JURISDICTION)

30. ENGINEERED LVL's SHALL HAVE MINIMUM PROPERTIES OF Fb = 2600 psi, E=1900 ksi, AND Fv=285 psi

ENGINEERED PARALLAMS SHALL HAVE MINIMUM PROPERTIES OF Fb = 2600 psi, E = 2000 ksi, AND Fv = 290 psi COLUMN CONNECTION TO STEEL BEAMS SHALL BE WITH A CLIP POST CAP WITH ALL FOUR TAB EARS BENT AROUND THE BOTTOM FLANGE OF THE BEAM. FOR A BEARING PLATE, FOUR HOLES SHALL BE DRILLED IN THE BOTTOM FLANGE OF THE STEEL BEAM TO MATCH THE HOLE PATTERN OF THE PLATE. $\frac{1}{2}$ " x 2" BOLTS SHALL THEN BE INSTALLED WITH A FLAT WASHER, LOCK WASHER, AND A NUT IN EACH OF THE HOLES. THE POST CAP MAY BE WELDED TO THE STEEL BEAM IN ACCORDANCE WITH AWS D1.1-92 AS AN ALTERNATIVE, AND WOULD NEED TO BE INSPECTED BY AN AWS-CERTIFIED INSPECTOR.

WHEN MECHANICAL EQUIPMENT IS LOCATED IN AN ENCLOSED ROOM, THERE SHALL BE (2) 14"x12" VENTS LOCATED 33. IN A WALL COMMON WITH ADDITIONAL LIVING AREA. ONE VENT SHALL BE LOCATED SUCH THAT THE BOTTOM OF THE VENT BEGINS 12" FROM THE FLOOR AND THE OTHER VENT SHALL BE LOCATED SUCH THAT THE TOP OF THE VENT BEGINS 12" FROM THE CEILING.

34. ALL ROOF SHEATHING SHALL BE ⁷/₁₆" OSB WITH 8d COMMON NAILS @ 6" O.C. AT PANEL EDGES AND @ 12" O.C. IN FIELD

GLAZING NOTES

35. GLAZING IN HAZARDOUS LOCATIONS AS IDENTIFIED IN IRC SECTION R308.4 SHALL BE OF APPROVED SAFETY GLAZING MATERIALS. GLASS IN STORM DOORS, INDIVIDUAL FIXED OR OPENABLE PANELS ADJACENT TO A DOOR WHERE THE NEAREST VERTICAL EDGE IS WITHIN A 2'-0" ARC OF THE DOOR IN A CLOSED POSITION AND FOR WHICH THE BOTTOM EDGE IS WITHIN 5'-0" OF THE FLOOR, WALLS ENCLOSING STAIRWAYS AND LANDINGS WHERE THE GLAZING IS WITHIN 5'-0" OF THE TOP OR BOTTOM OF THE STAIR, ENCLOSURES FOR SPAS, TUBS, SHOWERS, AND WHIRLPOOLS, GLAZING IN FIXED OR OPENABLE PANELS EXCEEDING NINE SQUARE FEET AND FOR WHICH THE BOTTOM EDGE IS LESS THAN 1'-6" ABOVE THE FLOOR OR WALKING SURFACE WITHIN 3'-0" 36. ALL OPERABLE WINDOWS SHALL HAVE FALL PROTECTION PER IRC SECTION R612.2

ATTIC VENTILATION

ENCLOSED ATTICS SHALL HAVE CROSS VENTILATION FOR EACH SEPARATE SPACE BY VENTILATING OPENINGS PROTECTED AGAINST THE ENTRANCE OF RAIN OR SNOW. VENTILATING OPENINGS SHALL BE PROVIDED WITH CORROSION-RESISTANT WIRE MESH, WITH $\frac{1}{6}$ " TO $\frac{1}{4}$ " OPENINGS. THE TOTAL FREE VENTILATING AREA SHALL NOT BE LESS THAN $\frac{1}{150}$ OF THE AREA OF SPACE VENTILATED, EXCEPT WHERE THE VENTILATORS ARE LOCATED IN THE UPPER PORTION OF THE SPACE TO BE VENTILATED - THE REQUIRED AREA MAY BE REDUCED TO 1/300.

EMERGENCY EGRESS

PROVIDE A MINIMUM OF ONE WINDOW FOR EACH BEDROOM THAT HAS A MINIMUM OPENABLE AREA OF 5.7 SQUARE FEET WITH A MINIMUM OPENABLE HEIGHT OF 2'-0" AND A MINIMUM WIDTH OF 1'-9". IN ADDITION, THE OPENABLE PORTION OF EGRESS WINDOWS SHALL NOT EXCEED 3'-8" ABOVE THE ADJOINING FLOOR OR PERMANENT STEP. PROVIDE SMOKE ALARMS IN EACH SLEEPING ROOM, OUTSIDE OF EACH SLEEPING AREA AND ON EACH FLOOR, 39. INCLUDING BASEMENT (IF APPLICABLE). ALARMS SHALL BE HARDWIRED TOGETHER SO THAT THE ACTIVATION OF ONE SMOKE ALARM WILL ACTIVATE ALL SMOKE ALARMS IN THE DWELLING. PROVIDE CARBON MONOXIDE DETECTORS OUTSIDE EACH SLEEPING AREA.

MASONRY VENEER

40. MASONRY VENEER SHALL BE ANCHORED TO THE SUPPORTING WALL STUDS WITH CORROSION-RESISTANT METAL TIES EMBEDDED IN MORTAR OR GROUT AND EXTENDING INTO THE VENEER A MINIMUM OF 1¹/₂", WITH NOT LESS THAN $\frac{5}{8}$ " MORTAR OR GROUT COVER TO OUTSIDE FACE.

- VENEER TIES, IF STRAND WIRE, SHALL NOT BE LESS IN THICKNESS THAN NO. 9 U.S. GAGE WIRE AND SHALL HAVE A 41. HOOK EMBEDDED IN THE MORTAR JOINT, OR IF SHEET METAL, SHALL BE NOT LESS THAN NO. 22 U.S. GAGE BY 7/8" CORRUGATED
- 42. EACH TIE SHALL SUPPORT NOT MORE THAN 2.67 SQUARE FEET OF WALL AREA AND SHALL BE SPACED NOT MORE THAN 32 INCHES ON CENTER HORIZONTALLY AND 24 INCHES ON CENTER VERTICALLY.
- VENEER TIES AROUND WALL OPENINGS: ADDITIONAL METAL TIES SHALL BE PROVIDED AROUND ALL WALL 43. OPENINGS GREATER THAN 16 INCHES IN EITHER DIMENSION. METAL TIES AROUND THE PERIMETER OF OPENINGS SHALL BE SPACED NOT MORE THAN 3 FEET ON CENTER AND PLACED WITHIN 12 INCHES OF THE WALL OPENING.

GARAGE NOTES

- DOOR(S) BETWEEN THE GARAGE AND DWELLING SHALL BE MINIMUM 1%" SOLID CORE OR HONEY-COMBED STEEL DOOR WITH 20-MINUTE FIRE RATING EQUIPPED WITH A SELF-CLOSING DEVICE 45. VEHICLE DOORS AND FRAMES SHALL BE DESIGNED AND INSTALLED TO MEET THE 115-MPH 3-SECOND GUST
- LOADING PER DASMA 108 AND ASTM E 330-96 PER IRC 2018

GARAGE NOTES (CONTINUED)

- THE GARAGE SHALL BE SEPARATED FROM THE DWELLING AND ITS ATTIC AREAS BY 44 MINIMUM %" GYP. BOARD APPLIED TO THE GARAGE SIDE OF FRAMING. WHERE HABITABLE SPACE OCCURS ABOVE THE GARAGE, THE SHALL BE PROTECTED WITH A MINIMUM 5/8" TYPE X GYF FLOOR/CEILING SPACE IS PROVIDED ABOVE THE GARA
- SUPPORTING THE SEPARATION SHALL ALSO BE PROTE 45 GARAGE DOOR H-FRAME FOR THE ATTACHMENT OF TH BALANCE SHALL CONSIST OF THE FOLLOWING: 2x6 VEF FLOOR TO CEILING AND SHALL BE FASTENED WITH 21/2" STAGGERED WITH (7) 3¹/₄" x 0.120" NAILS THROUGH THE MINIMUM 2x8 HEADER FOR ATTACHMENT OF COUNTER

DESIGN LOADING (PER TABLE R301.5)

| MINIMUM UNIFORMLY DISTRIE | LIVE LOAD | DEAD LOAD | | | |
|---|------------------|-------------------------------------|--|--|--|
| UNINHABITABLE ATTICS WITHOUT STORAGE | 10 | 10 | | | |
| UNINHABITABLE ATTICS WITH LIMITED STORAGE | 20 | 10 | | | |
| HABITABLE ATTICS AND ATTICS SERVED WITH FIXED STAIRS | 30 | 10 | | | |
| BALCONIES (EXTERIOR) AND DECKS | 40 | 10 ^d | | | |
| FIRE ESCAPES | 40 | 10 | | | |
| GUARDRAILS AND HANDRAILS ^a | 200 [°] | - | | | |
| GUARDRAIL IN-FILL COMPONENTS ^b | 50 [°] | - | | | |
| PASSENGER VEHICLE GARAGES | 50 | DEPENDENT UPON SLAB CONSTRUCTION | | | |
| ROOMS OTHER THAN SLEEPING ROOM | 40 | 10 ^d | | | |
| SLEEPING ROOM | 30 | 10 ^d | | | |
| STAIRS | 40 | 10 ^d | | | |
| a. A single concentrated load applied in any direction at any point along the top. b. Guard in-fill components (all those except the handrail), ballusters and panel fillers shall be designed | | | | | |

withstand a horizontally applied normal load of 50 pounds on an area equal to one square foot. This load need not be assumed to act concurrently with any other live load requirement. c. Glazing used in handrail assemblies and guards shall be designed with a safety factor of 4. The safety factor shall be applied to each of the concentrated loads applied to the top of the rail, and to the load on the infill components. These loads shall be determined independently of one another, and loads are assumed not to occur with any other live load.

d. An additional dead loading of 10 psf shall be applied where thinset tile floor is to be installed. An additional dead loading of 50 psf shall be applied where mudset tile floor is to be installed.

INSULATION/EFFICIENCY

- BUILDING ENVELOPE INSULATION SHALL COMPLY WITH IRC TABLE N1102.1.1 OR THE 2012 IECC (SEE SHEET S3.1 FOR FRAMING DETAILS AND TABLES ON THIS SHEET FOR MORE INFORMATION)
- CATHEDRAL -VAULTED CEILING FRAMING SHALL BE FRAMED WITH A MINIMUM INSULATION VALUE OF R-38. IF VAULTED RAFTERS DO NOT PROVIDE REQUIRED DEPTH TO ACHIEVE R-38 INSULATION BUILDER SHALL FUR DOWN RAFTERS PER DETAILS PROVIDED ON SHEET S3.1.

INSULATION AND FENESTRATION REQUIREMENTS BY COMPONENT (TABLE N1102.1.1)

| CLIMATE ZONE | 4-A |
|--|----------------------------|
| FENESTRATION U-FACTOR | 0.35 |
| SKYLIGHT U-FACTOR | 0.55 |
| GLAZED FENSTRATION SHGC | 0.40 |
| CEILING R-VALUE | 49 |
| WOOD FRAME WALL R-VALUE | 15 |
| MASS WALL R-VALUE | 8 / 13 |
| FLOOR R-VALUE | 19 |
| BASEMENT WALL R-VALUE | 10-CONTINUOUS OR 13-CAVITY |
| SLAB R-VALUE AND DEPTH | 10 AT 2'-0" |
| CRAWL SPACE WALL R-VALUE | 10-CONTINUOUS OR 13-CAVITY |
| DUCTWORK EXPOSED TO OUTSIDE AIR R-VALUE | 8 |
| DUCTWORK NOT EXPOSED TO OUTSIDE AIR R-VALUE | 6 |
| CATHEDRAL VAULTED CEILING R-VALUE | 38 |

DUCT SEALING N1103.2.2 (R403.2.2) SEALING (MANDATORY). DUCTS, AIR HANDLERS, AND FILTER BOXES SHALL BE SEALED. JOINTS AND SEAMS SHALL COMPLY WITH SECTION M1601.4.1 OF 2018 IRC. **EXCEPTIONS:** AIR-IMPERMEABLE SPRAY FOAM PRODUCTS SHALL BE PERMITTED TO BE APPLIED 1. WITHOUT ADDITIONAL JOINT SEALS. WHERE A DUCT CONNECTION IS MADE THAT IS PARTIALLY INACCESSIBLE, THREE

- SCREWS OR RIVETS SHALL BE EQUALLY SPACED ON THE EXPOSED PORTION OF THE JOINT SO AS TO PREVENT A HINGE EFFECT.
- CONTINUOUSLY WELDED AND LOCKING-TYPE LONGITUDINAL JOINTS AND SEAMS IN DUCTS OPERATING AT STATIC PRESSURES LESS THAN 2 INCHES OF WATER COLUMN PRESSURE CLASSIFICATION SHALL NOT REQUIRE ADDITIONAL CLOSURE SYSTEMS.
- DUCT TIGHTNESS SHALL BE VERIFIED BY EITHER OF THE FOLLOWING: POST-CONSTRUCTION TEST: TOTAL LEAKAGE SHALL BE LESS THAN OR EQUAL TO 4 CFM 1. PER 100 SQUARE FEET OF CONDITIONED FLOOR AREA WHEN TESTED AT A PRESSURE DIFFERENTIAL OF 0.1 INCHES W.G. ACROSS THE ENTIRE SYSTEM, INCLUDING THE MANUFACTURER'S AIR HANDLER ENCLOSURE. ALL REGISTER BOOTS SHALL BE TAPED
- OR OTHERWISE SEALED DURING THE TEST. ROUGH-IN TEST: TOTAL LEAKAGE SHALL BE LESS THAN OR EQUAL TO 4 CFM PER 100 SQUARE FEET OF CONDITIONED FLOOR AREA WHEN TESTED AT A PRESSURE DIFFERENTIAL OF 0.1 INCHES W.G. ACROSS THE SYSTEM, INCLUDING THE MANUFACTURER'S AIR HANDLER ENCLOSURE. ALL REGISTERS SHALL BE TAPED OR OTHERWISE SEALED DURING THE TEST. IF THE AIR HANDLER IS NOT INSTALLED AT THE

TIME OF THE TEST, TOTAL LEAKAGE SHALL BE LESS THAN OR EQUAL TO 3 CFM PER 100 SQUARE FEET OF CONDITIONED FLOOR AREA. **EXCEPTION:** THE TOTAL LEAKAGE TEST IS NOT REQUIRED FOR DUCTS AND AIR HANDLERS LOCATED ENTIRELY WITHIN THE BUILDING THERMAL ENVELOPE.

| ME | ECHANICAL VENTILATIO | |
|---------------------------|----------------------|----------------|
| FAN LOCATION | AIR FLOW RATE | MINIMUM EFFICA |
| TAN ECCATION | MINIMUM (CFM) | (CFM/WATT) |
| RANGE HOODS | ANY | 2.8 |
| IN-LINE FAN | ANY | 2.8 |
| BATHROOM, UTILITY ROOM | 10 | 1.4 |
| BATHROOM, UTILITY ROOM | 90 | 2.8 |

| GARAGE CEILING ASSEMBLY |
|------------------------------|
| P. BOARD. WHERE A |
| AGE COLUMNS AND BEAMS |
| ECTED WITH 5/8" GYP. BOARD. |
| HE TRACK AND COUNTER |
| RTICAL JAMBS RUNNING FROM |
| "" x 0.120" NAILS AT 7" O.C. |
| E JAMB INTO THE HEADER. |
| R BALANCE SYSTEM. |
| |
| |

AIR FLOW RATE

MAXIMUM (CFM)

ANY

ANY

90

ANY

 \triangleleft ШЦ S Ш

(n U \sim L 28 DATE REVISION DRAWING TITLE STRUCTURAL NOTES ENGINEER: DMH CHECKED BY: DMH JOB NO. 3983 | DRAWN BY: DMH DATE: 10-18-21 SHEET NUMBER

10/20/2021

RESIDENTIAL SEISMIC & WIND ANALYSIS

| LOCATION DEAD LOAD (psf) AREA (ft²) ROOF 10 2692 0 CEILING 10 2692 0 SECOND FLOOR 10 827 0 827 FIRST FLOOR 10 2692 0 827 FIRST FLOOR 10 827 0 0 2692 SECOND FLOOR EXT. WALL DL 10 223 10 10 2692 SECOND FLOOR EXT. WALL DL 166 8 8 8 FIRST FLOOR EXT. WALL DL 223 10 10 10 SECOND FLOOR INT. PARTITION WALL DL 0 827 0 10 | DETERMINE WEIGHT | T OF HOUSE: | | | | | | CAL |
|--|---------------------|---------------------|----------|--------------------|-----------------------|-------------------------------|-------------------------|-------|
| CEILING 10 2692 SECOND FLOOR 10 827 FIRST FLOOR 10 2692 FIRST FLOOR 10 2692 WALL LENGTH (ft) WALL UNIT (psf) WALL UNIT (psf) SECOND FLOOR EXT. WALL DL 166 8 8 FIRST FLOOR EXT. WALL DL 166 8 8 SECOND FLOOR INT. PARTITION WALL DL 166 8 8 FIRST FLOOR INT. PARTITION WALL DL 6 827 FIRST FLOOR INT. PARTITION WALL DL 6 827 FIRST FLOOR INT. PARTITION WALL DL 6 827 FRONT-TO-BACK SIDE-TO-SIDE SIDE-TO-SIDE FRONT-TO-BACK SIDE-TO-SIDE 1261 VERT. ROOF 0 0 0 0 SLOPED ROOF 213 938 SLOPED ROOF 291 1261 VERT. ROOF 0 0 0 0 0 0 157 5024 1ST 590.37 8232 15067 157 2732 1732 | LOCATION | | | | | DEAD LOAD (psf) | AREA (ft ²) | |
| SECOND FLOOR 10 827 FIRST FLOOR 10 2692 SECOND FLOOR EXT. WALL DL WALL LENGTH (ft) WALL UNT WALL WALL WALL WALL WALL WALL WALL WAL | ROOF | | | | | 10 | 2692 | |
| FIRST FLOOR 10 2692 WALL LENGTH (ft) WALL HEIGHT (ft) WALL UNIT WT. (psf) SECOND FLOOR EXT. WALL DL 166 8 8 FIRST FLOOR EXT. WALL DL 123 10 10 SECOND FLOOR INT. PARTITION WALL DL 6 827 FIRST FLOOR INT. PARTITION WALL DL 6 827 FIRST FLOOR INT. PARTITION WALL DL 6 2692 PROJECTED AREAS (WIND DESIGN PER 115 MPH 3-SECOND GUST, EXPOSURE C AND MEAN ROOF HEIGHT <= 30 FT ASSUMED) | CEILING | | | | | 10 | 2692 | |
| WALL LENGTH (ft) WALL HEIGHT (ft) WALL UNIT WT. (psf) SECOND FLOOR EXT. WALL DL 166 8 8 FIRST FLOOR EXT. WALL DL 223 10 10 SECOND FLOOR INT. PARTITION WALL DL 0EAD LOAD (psf) AREA (ft2) SECOND FLOOR INT. PARTITION WALL DL 6 827 FIRST FLOOR INT. PARTITION WALL DL 6 2692 PROJECTED AREAS (WIND DESIGN PER 115 MPH 3-SECOND GUST, EXPOSURE C AND MEAN ROOF HEIGHT <= 30 FT ASSUMED) | SECOND FLOOR | | | | | 10 | 827 | |
| SECOND FLOOR EXT. WALL DL 166 8 8 FIRST FLOOR EXT. WALL DL 223 10 10 DEAD LOAD (psf) AREA (ft2) 0 0 SECOND FLOOR INT. PARTITION WALL DL 6 827 FIRST FLOOR INT. PARTITION WALL DL 6 2692 PROJECTED AREAS (WIND DESIGN PER 115 MPH 3-SECOND GUST, EXPOSURE C AND MEAN ROOF HEIGHT <= 30 FT ASSUMED) | FIRST FLOOR | | | | | 10 | | |
| FIRST FLOOR EXT. WALL DL 223 10 10 SECOND FLOOR INT. PARTITION WALL DL DEAD LOAD (psf) AREA (ft2) FIRST FLOOR INT. PARTITION WALL DL 6 827 FIRST FLOOR INT. PARTITION WALL DL 6 2692 PROJECTED AREAS (WIND DESIGN PER 115 MPH 3-SECOND GUST, EXPOSURE C AND MEAN ROOF HEIGHT <= 30 FT ASSUMED) | | | | | WALL LENGTH (ft) | WALL HEIGHT (ft) | WALL UNIT WT. (psf) | |
| DEAD LOAD (psf) AREA (ft2) SECOND FLOOR INT. PARTITION WALL DL 6 827 FIRST FLOOR INT. PARTITION WALL DL 6 2692 PROJECTED AREAS (WIND DESIGN PER 115 MPH 3-SECOND GUST, EXPOSURE C AND MEAN ROOF HEIGHT <= 30 FT ASSUMED) | SECOND FLOOR EXT | T. WALL DL | | | 166 | 8 | 8 | |
| SECOND FLOOR INT. PARTITION WALL DL 6 827 FIRST FLOOR INT. PARTITION WALL DL 6 2692 PROJECTED AREAS (WIND DESIGN PER 115 MPH 3-SECOND GUST, EXPOSURE C AND MEAN ROOF HEIGHT <= 30 FT ASSUMED) | FIRST FLOOR EXT. V | VALL DL | | | 223 | 10 | 10 | |
| FIRST FLOOR INT. PARTITION WALL DL 6 2692 PROJECTED AREAS (WIND DESIGN PER 115 MPH 3-SECOND GUST, EXPOSURE C AND MEAN ROOF HEIGHT <= 30 FT ASSUMED) | | | | | | DEAD LOAD (psf) | AREA (ft2) | |
| PROJECTED AREAS (WIND DESIGN PER 115 MPH 3-SECOND GUST, EXPOSURE C AND MEAN ROOF HEIGHT <= 30 FT ASSUMED) FRONT-TO-BACK SIDE-TO-SIDE AREA LOAD SLOPED ROOF 291 1261 VERT. ROOF 0 0 CUMULATIVE VERT. ROOF 0 0 2ND 411.03 5897 6835 2ND 335.97 5024 IST 590.37 8232 15067 1ST 636.13 8765 BSMT ^{al} 0 0 0 BSMT ^{al} 157 2732 | SECOND FLOOR INT | . PARTITION WALL DL | | 6 | 827 | | | |
| FRONT-TO-BACK SIDE-TO-SIDE AREA LOAD AREA LOAD SLOPED ROOF 213 938 SLOPED ROOF 291 1261 VERT. ROOF 0 0 CUMULATIVE VERT. ROOF 0 0 0 2ND 411.03 5897 6835 2ND 335.97 5024 0 1ST 590.37 8232 15067 1ST 636.13 8765 0 BSMT ^a 0 0 0 BSMT ^a 157 2732 0 | FIRST FLOOR INT. P/ | ARTITION WALL DL | | | | 6 | 2692 | |
| FRONT-TO-BACK SIDE-TO-SIDE AREA LOAD AREA LOAD SLOPED ROOF 213 938 SLOPED ROOF 291 1261 VERT. ROOF 0 0 CUMULATIVE VERT. ROOF 0 0 0 2ND 411.03 5897 6835 2ND 335.97 5024 0 1ST 590.37 8232 15067 1ST 636.13 8765 0 BSMT ^a 0 0 0 BSMT ^a 157 2732 0 | | | | | | | | |
| AREA LOAD AREA LOAD SLOPED ROOF 213 938 SLOPED ROOF 291 1261 VERT. ROOF 0 0 CUMULATIVE VERT. ROOF 0 0 0 2ND 411.03 5897 6835 2ND 335.97 5024 1ST 590.37 8232 15067 1ST 636.13 8765 BSMT ^a 0 0 0 BSMT ^a 157 2732 | | | | DESIGN PER 115 MPH | 3-SECOND GUST, EXPOSU | JRE C AND MEAN ROOF HEIGHT <= | = 30 FT ASSUMED) | |
| SLOPED ROOF 213 938 SLOPED ROOF 291 1261 VERT. ROOF 0 0 CUMULATIVE VERT. ROOF 0 0 2ND 411.03 5897 6835 2ND 335.97 5024 1ST 590.37 8232 15067 1ST 636.13 8765 BSMT ^a 0 0 0 BSMT ^a 157 2732 | | FRONT | -TO-BACK | | | SIDE-TO- | SIDE | |
| VERT. ROOF 0 0 CUMULATIVE VERT. ROOF 0 0 2ND 411.03 5897 6835 2ND 335.97 5024 1ST 590.37 8232 15067 1ST 636.13 8765 BSMT ^a 0 0 0 BSMT ^a 157 2732 | | AREA | LOAD | | | AREA | LOAD | |
| 2ND 411.03 5897 6835 2ND 335.97 5024 1ST 590.37 8232 15067 1ST 636.13 8765 BSMT ^a 0 0 0 BSMT ^a 157 2732 PRESSURE (PSF) - PER ASCE CH. 6 | SLOPED ROOF | 213 | 938 | | SLOPED ROOF | 291 | 1261 | |
| 1ST 590.37 8232 15067 1ST 636.13 8765 BSMT ^a 0 0 0 BSMT ^a 157 2732 PRESSURE (PSF) - PER ASCE CH. 6 | VERT. ROOF | 0 | 0 | CUMULATIVE | VERT. ROOF | 0 | 0 | |
| BSMT ^a 0 0 BSMT ^a 157 2732 PRESSURE (PSF) - PER ASCE CH. 6 | 2ND | 411.03 | 5897 | 6835 | 2ND | 335.97 | 5024 | |
| PRESSURE (PSF) - PER ASCE CH. 6 | | 590.37 | 8232 | 15067 | | 636.13 | 8765 | |
| | BSMT ^a | 0 | 0 | 0 | BSMT ^a | 157 | 2732 | |
| SLOPED ROOF ZONE B 5.9 ZONE C 11.6 2a (| | | | PRESSURE (PSI | F) - PER ASCE CH. 6 | | • | |
| | | SLOPED ROOF | ZONE B | | 5.9 | ZONE C | 11.6 | 2a (F |
| WALL/VERT. ROOF ZONE A 17.4 ZONE D 3.4 | | | ZONE A | | 17.4 | ZONE D | 3.4 | |
| MEAN ROOF HT., h 26 | | MEAN ROOF HT., h | | 26 | | | | |

a) If there is a walkout wall to be sheathed, determine tributary wind area and enter here. If no walkout, enter 0 for area. q_{z10}=0.00256K_zK_{zt}K_dV² (ASCE7-10 Velocity Pressure) q_{z10_ASD}=0.6q_{z10} (Design Velocity Pressure for ASD analysis under ASCE7-10 and IRC/IBC 2012)

2ND FLOOR TRIBUTARY WEIGHT 1ST FLOOR TRIBUTARY WEIGHT

BASEMENT TRIBUTARY WEIGHT S_s (SITE GROUND MOTION - %g - FROM ASCE7 SEISMIC MAP) F_a (from ASCE7 Table 11.4-1)

S_{DS} (= 2/3 * S_S * F_a) R (from ASCE7 Table 12.2-1)

| | | SEISMIC SHEAR | |
|------------------------------------|--|---|-------------------|
| ON OOR DOR | | From ASC | CE7 (Eq. 12.8-1): |
| ENT | | | |
| Sheathing Location | Min. Sheathing Schedule | Fastening Schedule | Allowable Shea |
| Exterior <u>(Option #1)</u> | 7/16" APA Rated Plywood/OSB | 1-1/2" 16ga. Staples w/ 1" penetration@ 6" OC Edges, 6" OC Field For 24" stud spacing, 12" OC Field For 16" stud spacing | 155 |
| Exterior (Option #2) | 7/16" APA Rated Plywood/OSB | 1-1/2" 16ga. Staples w/ 1" penetration@ 4" OC Edges, 5" OC Field For 24" stud spacing, 12" OC Field For 16" stud spacing | 230 |
| Exterior (Option #3) | 7/16" APA Rated Plywood/OSB | 1-1/2" 16ga. Staples w/ 1" penetration@ 3" OC Edges, 5" OC Field For 24" stud spacing, 12" OC Field For 16" stud spacing | 310 |
| Exterior <u>(Option #4)</u> | 7/16" APA Rated Plywood/OSB or shiplap panel sheathing, or 3/8" shiplap panel sheathing with tighter nail spacing | 8d Common Nails w/ 1-3/8" penetration @ 6" O.C. Edges, 12" O.C. Field for 7/16" APA-rated plywood/OSB or shiplap panel sheathing OR @ 4" O.C. Edges, 12" O.C. Field for 3/8" shiplap panel sheathing | 220 |
| Exterior <u>(<i>Option #5)</i></u> | 7/16" APA Rated Plywood/OSB or shiplap panel sheathing, or 3/8" shiplap panel sheathing with tighter nail spacing | 8d Common Nails w/ 1-3/8" penetration @ 4" O.C. Edges, 12" O.C. Field for 7/16" APA-rated plywood/OSB or shiplap panel sheathing OR @ 3" O.C. Edges, 12" O.C. Field for 3/8" shiplap panel sheathing | 320 |
| Exterior (<i>Option #6)</i> | 7/16" APA Rated Plywood/OSB or shiplap panel sheathing, or 3/8" shiplap panel sheathing with tighter nail spacing and double studs at each panel edge | 8d Common Nails w/ 1-3/8" penetration @ 3" O.C. Edges, 12" O.C. Field | 410 |
| Interior | 1/2" Gypsum Board | No. 6- 1 ¹ / ₄ " Type W or S Screws @ 8" O.C. Edges, 12" O.C. Field | 60 |
| Interior | 16 Ga. Simpson/USP Type WB Steel X-Brace (or equal) | (3) 16d @ end studs & (1) 8d @ intermediate studs (per manufacturer specifications - see detail on sheet S3) | 325 |

| EXTERIOR SHEATHING OPTION FOR SECOND FLOOR | 4 |
|--|---|
| EXTERIOR SHEATHING OPTION FOR FIRST FLOOR | 5 |
| EXTERIOR SHEATHING OPTION FOR BASEMENT WALLS | 5 |
| | |

| WIDTH OF 1ST STORY (FT.) | 53.67 | WIDTH O |
|---------------------------|-------|---------|
| DEPTH OF 1ST STORY (FT.) | 57.83 | DEPTH C |
| BACK WALL OF GARAGE (FT.) | 0 | |
| GAR. WALL: 1=F-B, 2=S-S | 2 | |

| | | | | | GAR. WALL: 1=F-B, 2=S-S | 2 | | |
|----------------------------|--------------------------------|-------------------|--------------|---------------------|-----------------------------|-------------------|--------------------------|----------------------|
| L | | | | | | | | |
| | | | | IOR STRUCTURAL WALL | LENGTHS (ft.) & RESISTANCES | | | |
| | | SE | ISMIC | | | WIND | | |
| | FRONT-TO-BACK | RESISTANCE (lbs.) | SIDE-TO-SIDE | RESISTANCE (lbs.) | FRONT-TO-BACK | RESISTANCE (lbs.) | SIDE-TO-SIDE | RESISTANCE (lbs.) |
| 2ND FLOOR | 30 | 8400 | 42 | 11760 | 30 | 11760 | 42 | 16464 |
| 1ST FLOOR | 70 | 26600 | 30 | 11400 | 70 | 37240 | 30 | 15960 |
| BASEMENT | 0 | 0 | 24.5 | 9310 | 0 | 0 | 24.5 | 13034 |
| L | | | | | | | | |
| | ADDITIONAL RESISTANCE REQUIRED | | |] | Anchor Bolt Spacing | (in.) | 16d Nail Spacing req'd a | t bottom plate (in.) |
| | | SEISMIC | WIND | | diameter (in.) | 0.5 | 2nd Floor F-B | 46 |
| 2ND FLOOR FRONT- | TO-BACK | 0 | 0 | 1 | Shear value (per NDS) | 944 | 2nd Floor S-S | 46 |
| 2ND FLOOR SIDE-TO-SIDE | | 0 | 0 | | Spacing F-B (inches) | 139.1 | 1st Floor F-B | 21 |
| 1ST FLOOR FRONT-TO-BACK 0 | | 0 | 0 | | spacing S-S (inches) | 129.3 | 1st Floor S-S | 19 |
| 1ST FLOOR SIDE-TO-SIDE 0 0 | | 0 | | | | | | |
| BASEMENT FRONT-TO-BACK 0 0 | | 0 | 1 | | | | | |
| BASEMENT SIDE-TO-SIDE | | 0 | 0 | 1 | | | | |

| BASEMENT SIDE-TO-SIDE | 0 | 0 | | | | | |
|--|---|--|-----------------------------------|--|---|--|-----|
| P | | | | | | | |
| | | RESISTANCE REQUI | RED IN ADDITION TO RES | SISTANCE PROVIDED BY EXTERIOR V | VALLS** | | |
| | ADDITIONAL RESISTANCE REQUIRED (POUNDS) | PORTAL FRAMES OR PERF. SHEAR WALL RESISTANCE | INTERIOR X-BRACES (325#/BRACE) | INTERIOR WALL LENGTH W/ 1/2" GYPSUM BOARD PER TABLE (FT.) | INT. WALL LENGTH SHEATHED W/ OSB (TOTAL LENGTH, ONE SIDE, FT.) | RESISTANCE PROVIDED BY ADDITIONAL METHODS (POUNDS) | OK? |
| 2ND FLOOR FRONT-TO-BACK | 0 | | | | | 0 | YES |
| 2ND FLOOR SIDE-TO-SIDE | 0 | | | | | 0 | YES |
| 1ST FLOOR FRONT-TO-BACK | 0 | | | | | 0 | YES |
| 1ST FLOOR SIDE-TO-SIDE | 0 | | | | | 0 | YES |
| BASEMENT FRONT-TO-BACK | 0 | | | | | 0 | YES |
| BASEMENT SIDE-TO-SIDE | 0 | | | | | 0 | YES |
| **NOTES: 1) SEE ATTACHED CALCULATIONS FOR PORTAL FRAME OR PERFORATED SHEAR WALL RESISTANCE CAPACITIES (IF APPLICABLE), | | | | | | | |

2) SEE SHEET S1 FOR INTERIOR STEEL X-BRACE INSTALLATION, 3) INTERIOR WALLS SHEATHED WITH OSB SHALL BE ATTACHED WITH SAME STAPLE/NAILING PATTERN AS EXTERIOR OSB ON SAME FLOOR (SEE TABLE ABOVE) AND ARE ONLY APPLICABLE FOR FULL-HEIGHT SECTIONS OF 2'-8" OR LONGER

| PATTERN AS EXTERIOR USB ON SAME FLOOR (SEE TABLE ABOVE) AND ARE ONLY APPLICABLE FOR FULL-HEIGHT SECTIONS OF 2'-8" OR LONGER | | | | | | | | |
|---|--|--------------------------------|--------------------------------|---|----------------------|-------------------|---------------------------------------|--|
| ALL LATERAL BRAC | ALL LATERAL BRACING ACHIEVED AT EXTERIOR WALLS AND WALLS DIRECTLY ON FOUNDATIONS; THEREFORE, NO INTERIOR BRACING PER 2012 IRC SECTION R502.2.1 IS REQUIRED | | | | | | | |
| | WIND UPLIFT ANALYSIS | | | | | | | |
| | X/12 | DEGREES | | | | | | |
| ROOF PITCH (MAX) | 5 | 22.6 | PITCH OF 6 OR LESS: | PITCH OF 6 OR LESS: EOH -13.3, E -7.2, G -5.2 | | | | |
| ASCE 7 | | | | | | | | |
| | LENGTH (FT.) | PRESSURE (PSF) | LINEAL FT. OF OH | LINEAL FT. OF OH UPLIFT PER FT* (LBS) | | | | |
| OVERHANG | 1 | 16.56 | 225 | 16.56 | | | | |
| | TOTAL AREA (FT ²) | ZONE E AREA (FT ²) | ZONE G AREA (FT ²) | PRESSURE ZN. E (PSF) | PRESSURE ZN. G (PSF) | TOTAL FORCE (LBS) | FORCE PER LINEAL FT @ PERIMETER (LBS) | |
| MAIN ROOF** | 3103.7361 | 1320.968976 | 1782.767124 | 15.12 | 10.5 | 38692 | 173.5 | |
| | | | | | | | | |
| *ALONG PERIMETER TOTAL UPLIFT PER LINEAL FOOT ALONG EXTERIOR (POUNDS) | | | | UNDS) | 190.1 | UPLIFT OK | | |
| **INSIDE EXTERIOR WALLS RESISTANCE DUE TO DEAD WEIGHT & (3) 10d TOENAILS | | | | 3 | 251.6 | | | |

NOTE FOR CONSTRUCTION: THE CONTINUOUS STRUCTURAL PANEL SHEATHING BRACING METHOD REQUIRES USE OF THE ABOVE TABLE FOR SHEATHING OF THE ENTIRE STRUCTURE. IN ADDITION, FRAMING MEMBERS SHALL BE @ 16" O.C. MAX., UNBLOCKED, AND W/ SHEATHING APPLIED DIRECTLY TO FRAMING MEMBERS

NOTE FOR DESIGN:

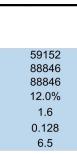
ALL WALLS USED IN THE CALCULATION OF THE RESISTANCE FOR THIS STRUCTURE SHALL HAVE A MINIMUM UNINTERRUPTED HEIGHT OF 8'-0" AND LENGTH OF 2'-8". ALLOWABLE RESISTANCES HAVE BEEN #/FT AND INCREASED BY 40% FOR WIND LOADS, PER VALUES IN 2012 IBC SECTION 2306 AND AF&PA SDPWS TABLE 4.3A. FOR EXAMPLE, 7/16" APA-RATED SHEATHING WITH 8d @ 6" & 12" HAS A SEISMIC SHEAR VALUE OF 240 A WIND SHEAR VALUE OF 335#/FT - 40% GREATER THAN THAT OF SEISMIC)

NOTE: SOIL SITE CLASS ASSUMED TO BE CLASS D. IF SITE CONDITIONS ARE DETERMINED TO BE CLASS E OR F, CONSULT ENGINEER BEFORE PROCEEDING

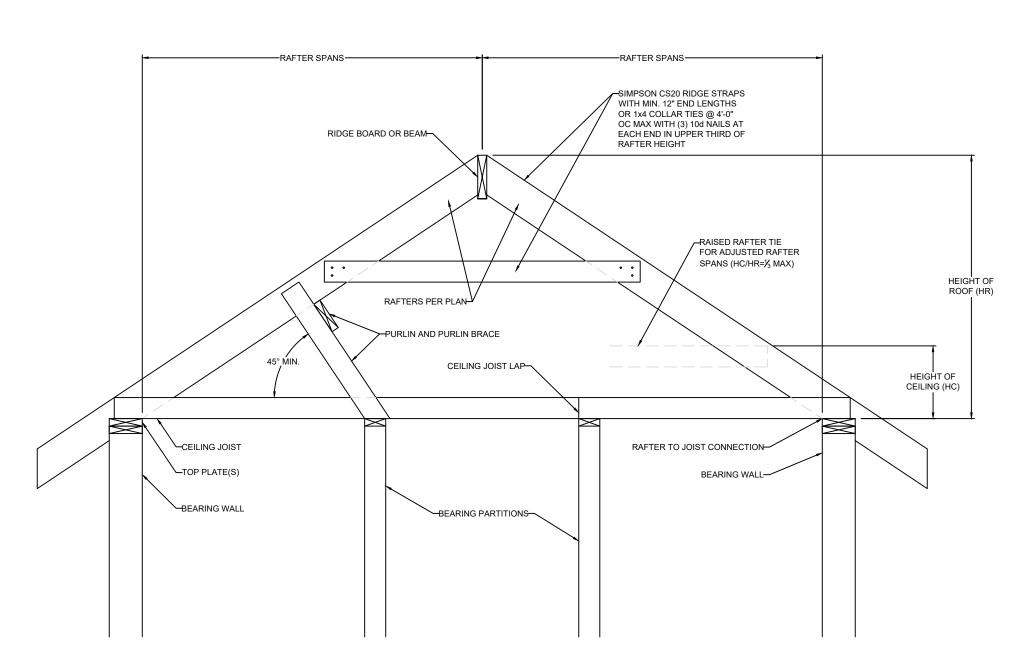
WITH CONSTRUCTION

| OF 2ND STORY (FT.) | 45.67 |
|--------------------|-------|
| OF 2ND STORY (FT.) | 37.33 |

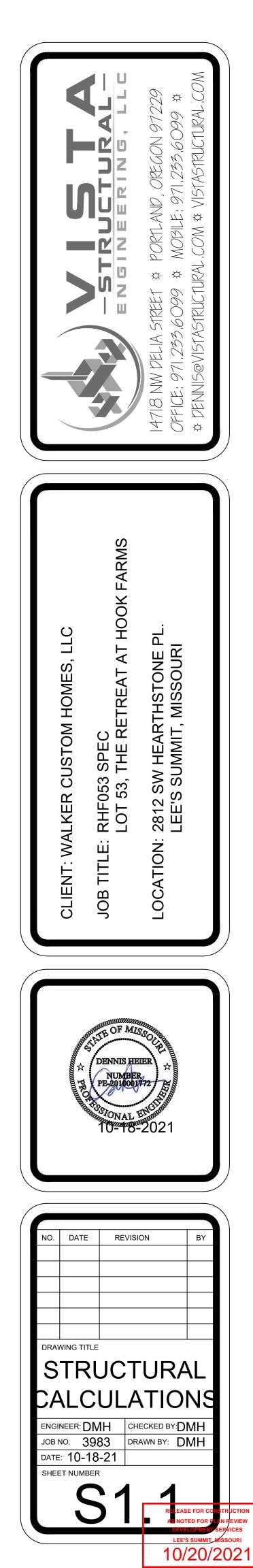
| V (= 1.2 * S _{DS} * W / | R) (lbs.) |
|----------------------------------|-----------------------------|
| 1398 | |
| 2099 | |
| 2099 | |
| | |
| (#/LF) | Code Reference |
| | per IBC, Table 2306.3(1) |
| | per IBC, Table 2306.3(1) |
| | per IBC, Table 2306.3(1) |
| | AF&PA SDPWS Table 4.3A |
| | AF&PA SDPWS Table 4.3A |
| | AF&PA SDPWS Table 4.3A |
| | per IBC, Table 2306.4.4 |
| | |

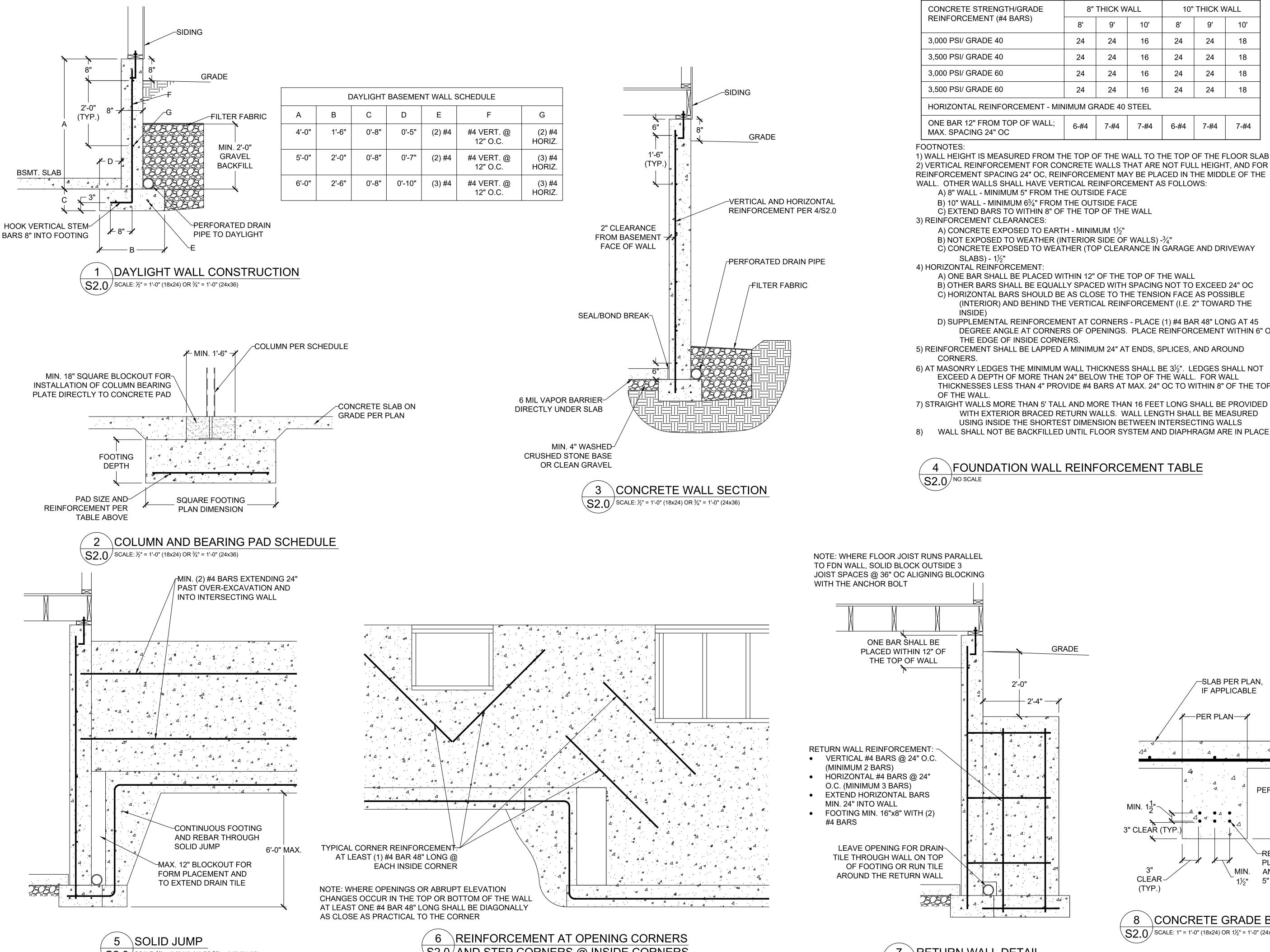


| INPUT |
|--------------------|
| CULATED VALUE |
| WEIGHT (lbs.) |
| 26920 |
| 26920 |
| 8270 |
| 26920 |
| WEIGHT (lbs) |
| 10624 |
| 22300 |
| WEIGHT (lbs) |
| 4962 |
| 16152 |
| |
| |
| |
| |
| CUMULATIVE |
| 6285 |
| 15049 |
| 10256 |
| |
| IG. 28.6-1, ASCE7) |
| 10.734 |
| |



1 BRACED RAFTER CONSTRUCTION S1.1 SCALE: 1" = 1'-0" (18x24) OR 1½" = 1'-0" (24x36)

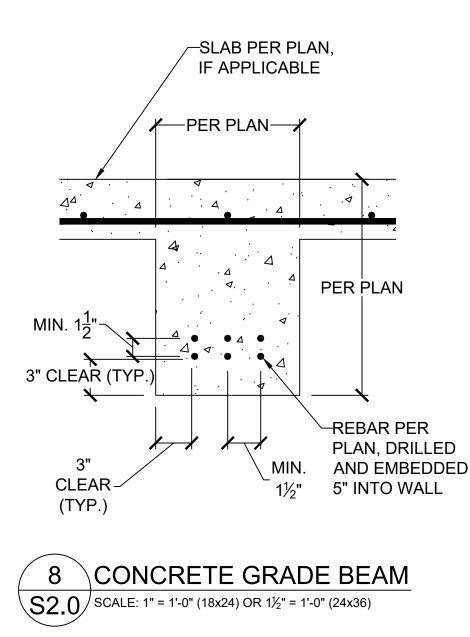




 $\overline{S2.0}$ SCALE: $\frac{1}{2}$ " = 1'-0" (18x24) OR $\frac{3}{4}$ " = 1'-0" (24x36)

S2.0/AND STEP CORNERS @ INSIDE CORNERS SCALE: ¹/₂" = 1'-0" (18x24) OR ³/₄" = 1'-0" (24x36)

ackslashRETURN WALL DETAIL SCALE: $\frac{1}{2}$ " = 1'-0" (18x24) OR $\frac{3}{4}$ " = 1'-0" (24x36) GRADE



AND EMBEDDED

THICKNESSES LESS THAN 4" PROVIDE #4 BARS AT MAX. 24" OC TO WITHIN 8" OF THE TOP 7) STRAIGHT WALLS MORE THAN 5' TALL AND MORE THAN 16 FEET LONG SHALL BE PROVIDED WITH EXTERIOR BRACED RETURN WALLS. WALL LENGTH SHALL BE MEASURED

EXCEED A DEPTH OF MORE THAN 24" BELOW THE TOP OF THE WALL. FOR WALL

6) AT MASONRY LEDGES THE MINIMUM WALL THICKNESS SHALL BE $3\frac{1}{2}$ ". LEDGES SHALL NOT

5) REINFORCEMENT SHALL BE LAPPED A MINIMUM 24" AT ENDS, SPLICES, AND AROUND

D) SUPPLEMENTAL REINFORCEMENT AT CORNERS - PLACE (1) #4 BAR 48" LONG AT 45 DEGREE ANGLE AT CORNERS OF OPENINGS. PLACE REINFORCEMENT WITHIN 6" OF

(INTERIOR) AND BEHIND THE VERTICAL REINFORCEMENT (I.E. 2" TOWARD THE

A) ONE BAR SHALL BE PLACED WITHIN 12" OF THE TOP OF THE WALL B) OTHER BARS SHALL BE EQUALLY SPACED WITH SPACING NOT TO EXCEED 24" OC C) HORIZONTAL BARS SHOULD BE AS CLOSE TO THE TENSION FACE AS POSSIBLE

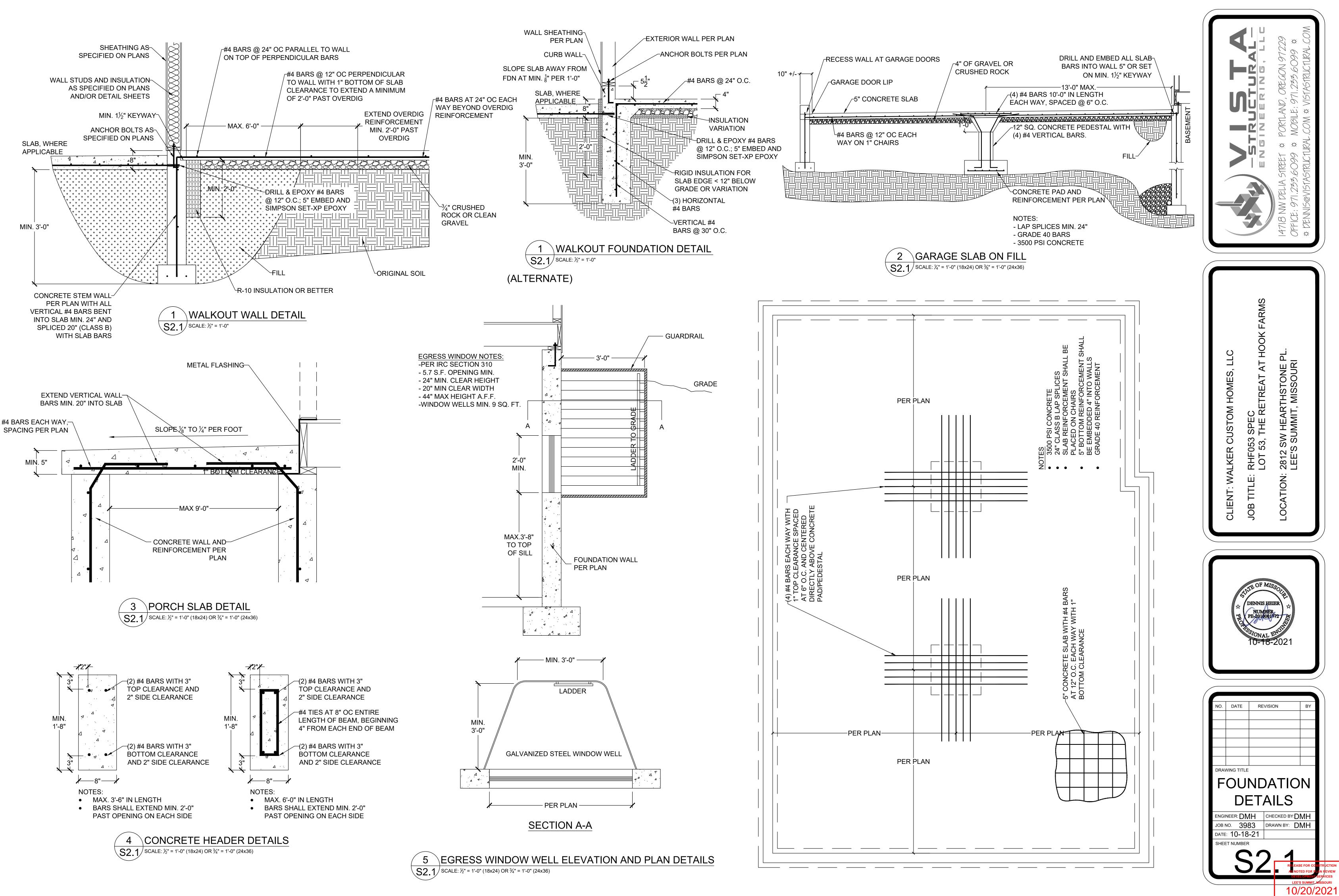
C) CONCRETE EXPOSED TO WEATHER (TOP CLEARANCE IN GARAGE AND DRIVEWAY

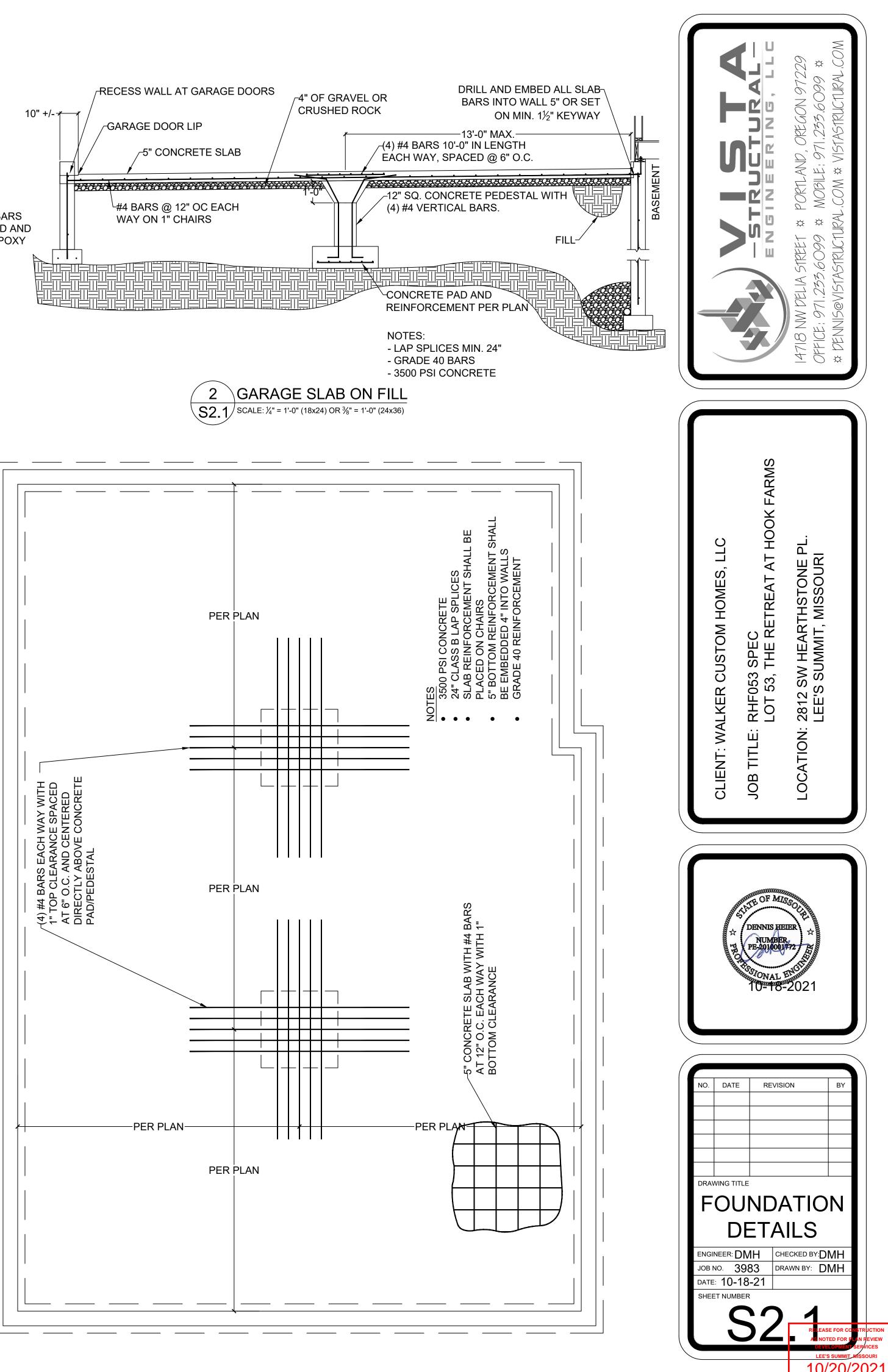
| - MINIMUM GRADE 40 STEEL | | | | | | | | |
|---|------|------|------|------|------|------|--|--|
| LL; | 6-#4 | 7-#4 | 7-#4 | 6-#4 | 7-#4 | 7-#4 | | |
| OM THE TOP OF THE WALL TO THE TOP OF THE FLOOR SLAB | | | | | | | | |

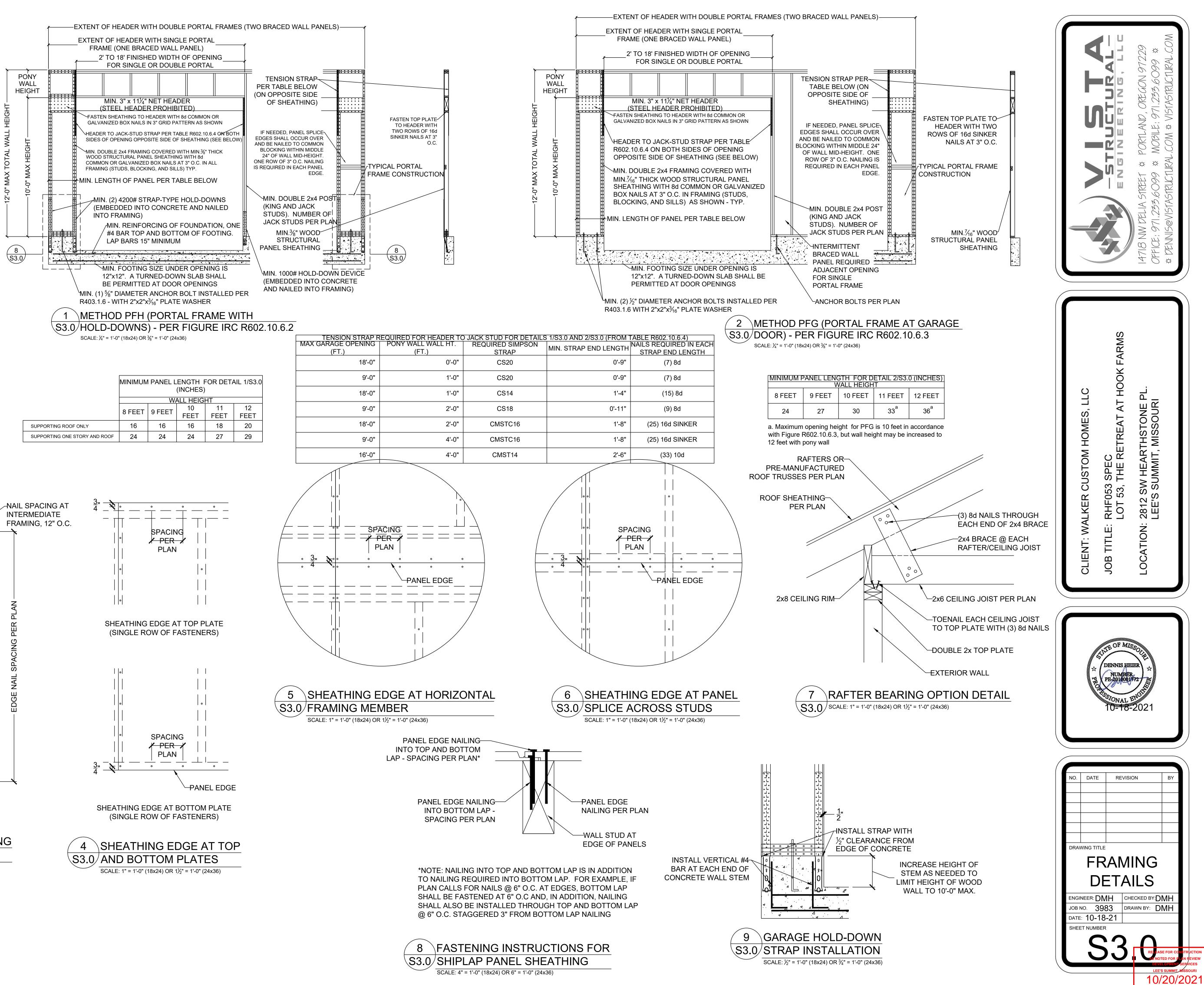
| VERTICAL REINFORCEMENT SPACIN | IG | | | | | |
|--|---------------|------|------|----------------|------|------|
| CONCRETE STRENGTH/GRADE REINFORCEMENT (#4 BARS) | 8" THICK WALL | | | 10" THICK WALL | | |
| | 8' | 9' | 10' | 8' | 9' | 10' |
| 3,000 PSI/ GRADE 40 | 24 | 24 | 16 | 24 | 24 | 18 |
| 3,500 PSI/ GRADE 40 | 24 | 24 | 16 | 24 | 24 | 18 |
| 3,000 PSI/ GRADE 60 | 24 | 24 | 16 | 24 | 24 | 18 |
| 3,500 PSI/ GRADE 60 | 24 | 24 | 16 | 24 | 24 | 18 |
| HORIZONTAL REINFORCEMENT - MINIMUM GRADE 40 STEEL | | | | | | |
| ONE BAR 12" FROM TOP OF WALL; | 6-#4 | 7-#4 | 7-#4 | 6-#4 | 7-#4 | 7-#4 |

T JZ U ÷Ċ+ ARMS \checkmark **O H** AT шЩ REAT ōō S S S Ŷ ō ĽЩ $\frac{1}{2}$ ທ ⊢ S S N S N က \mathfrak{C} ы ŝ КШ \sim 28 LE Ŕ :NOI CLIENT ന DATE REVISION DRAWING TITLE FOUNDATION DETAILS ENGINEER: DMH CHECKED BY:DMH JOB NO. 3983 DRAWN BY: DMH DATE: 10-18-21 SHEET NUMBER

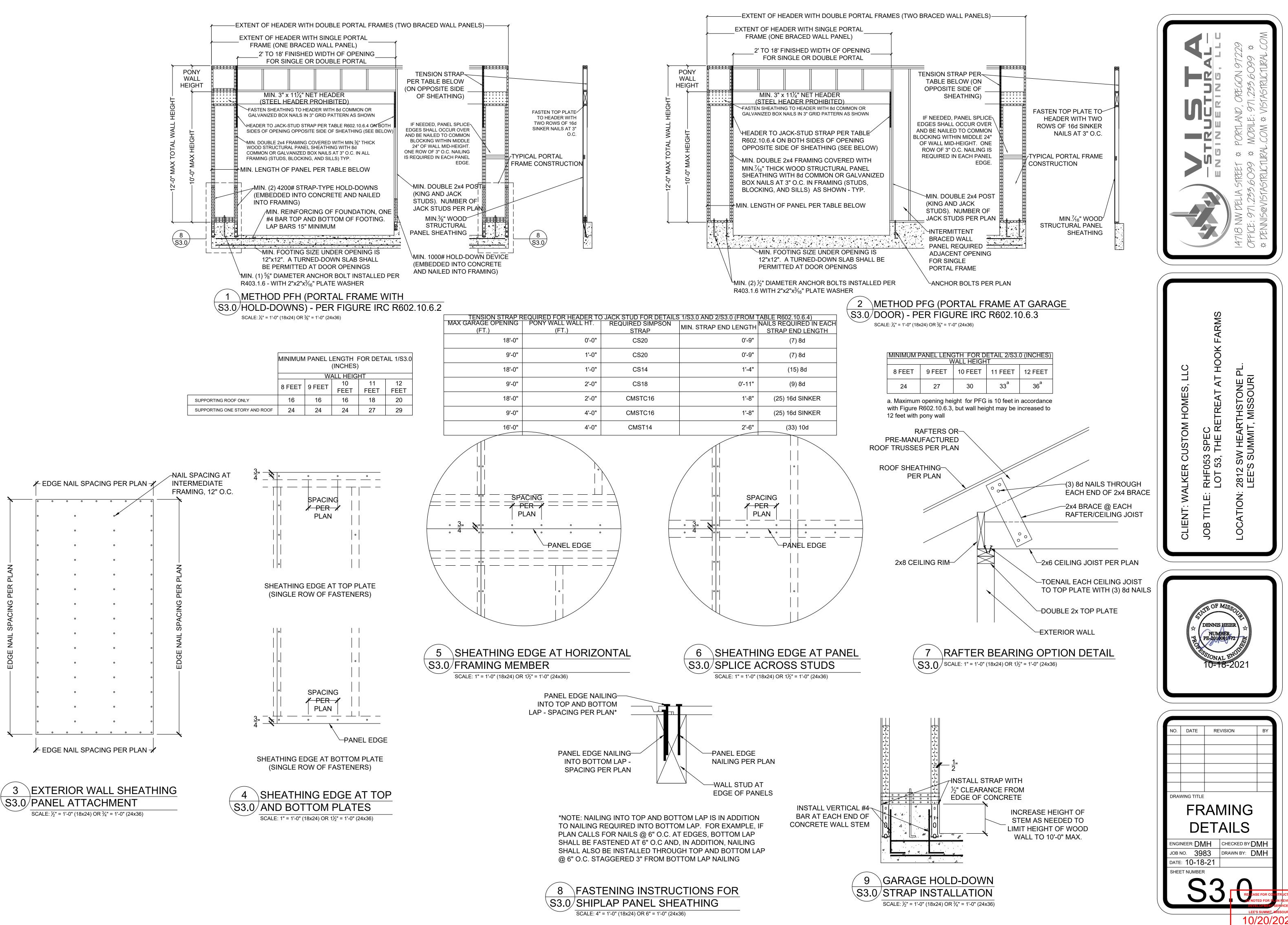
10/20/2021

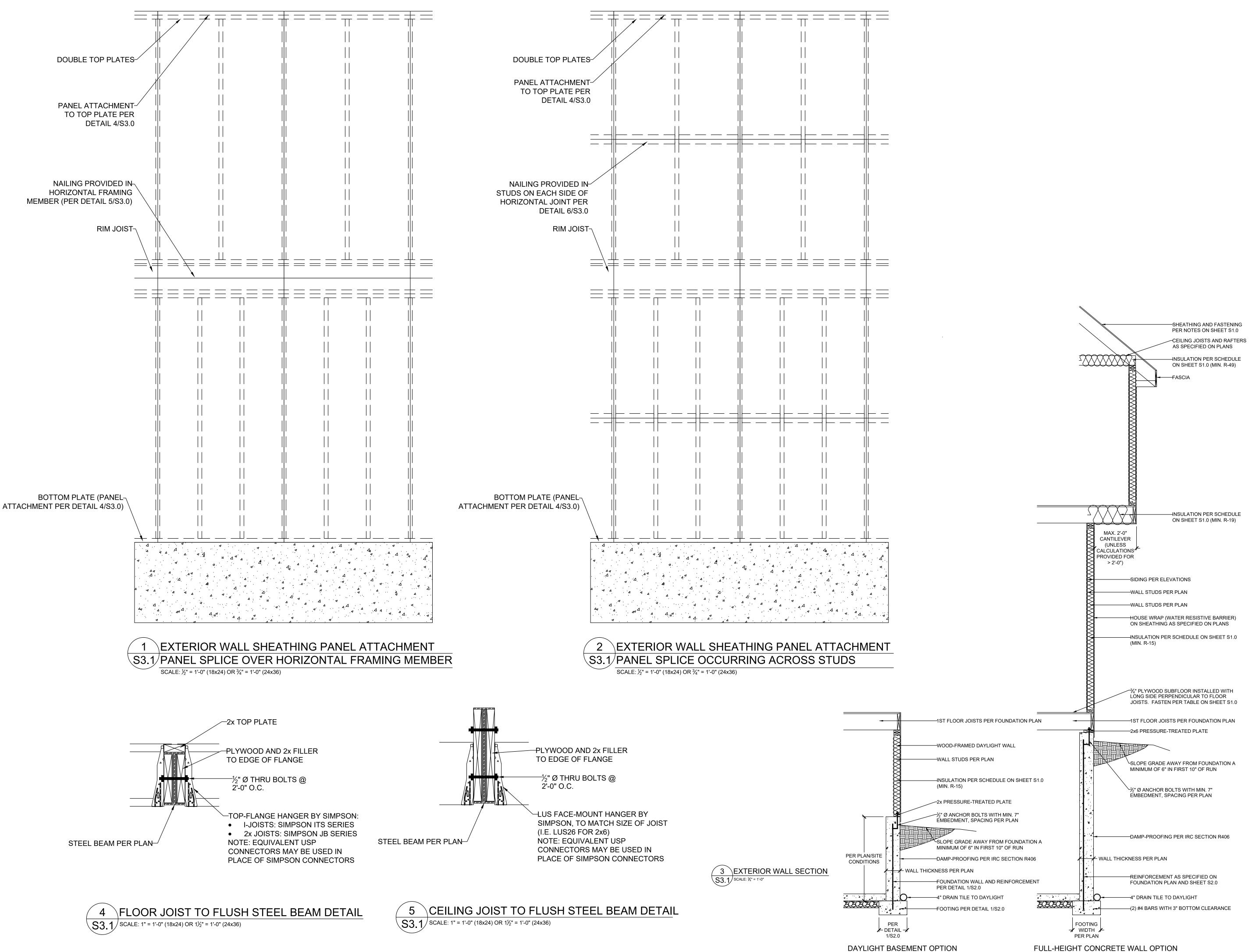


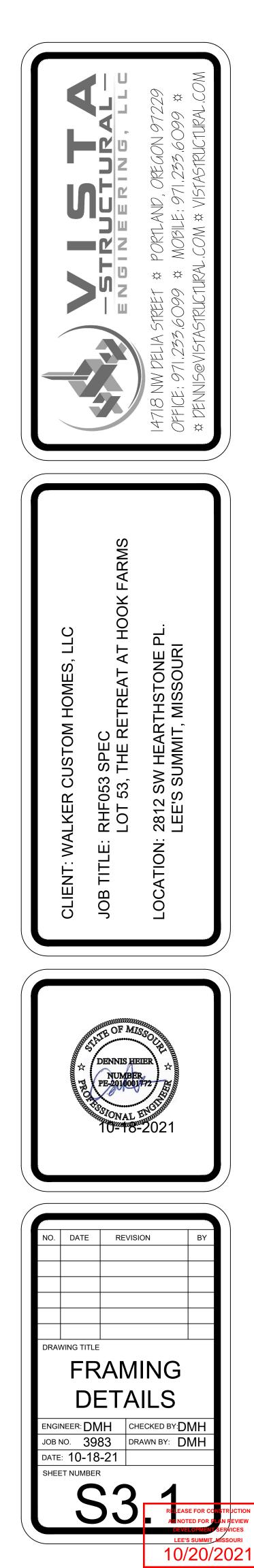


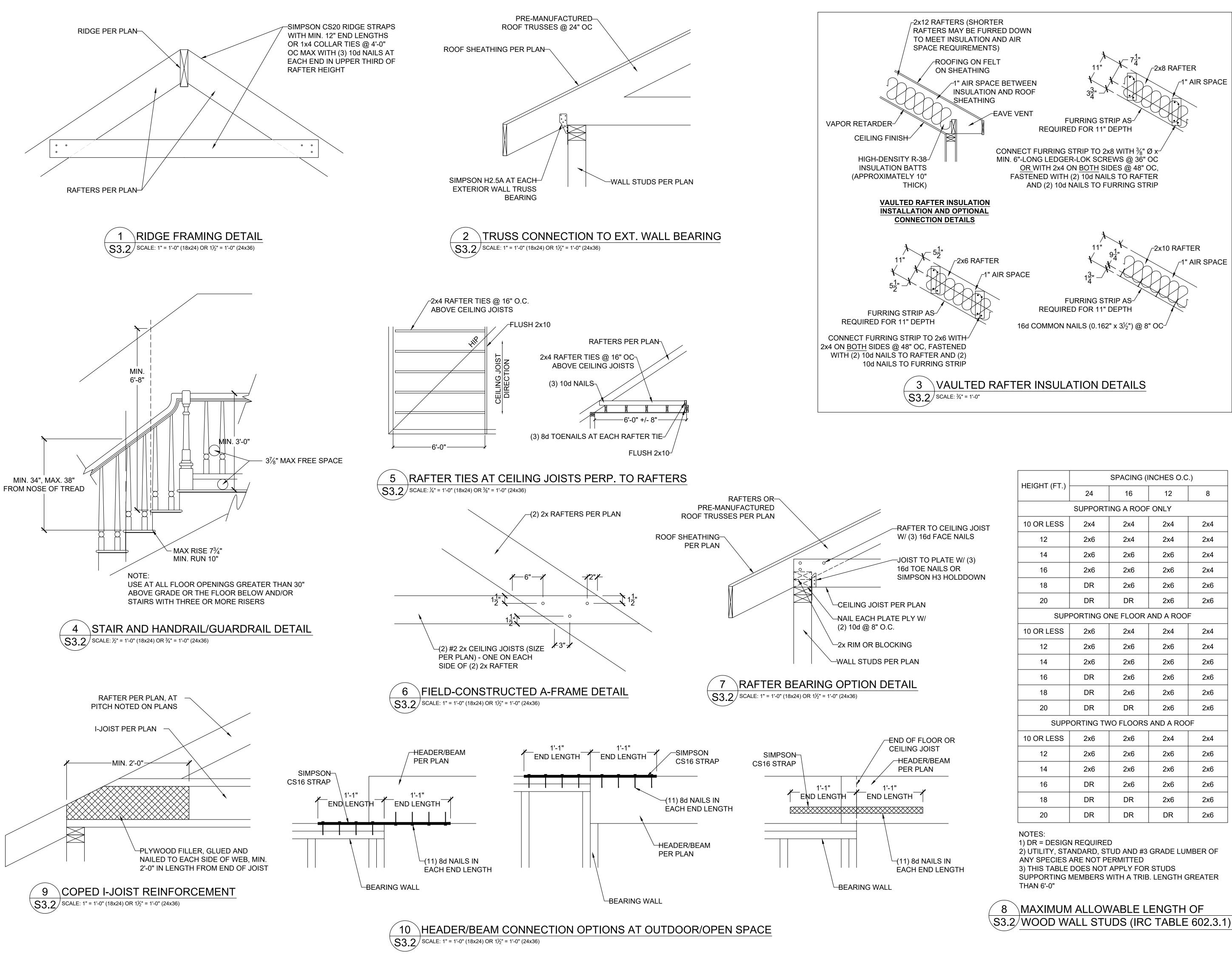


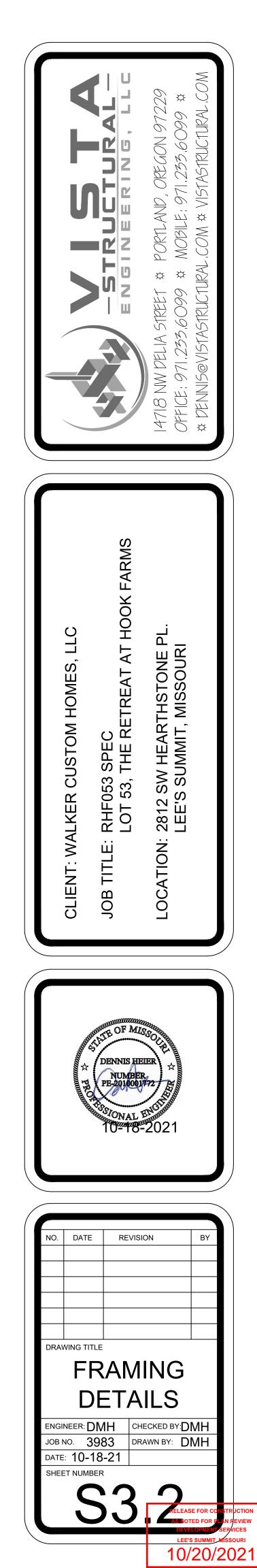
| | MINIMUM PANEL LENGTH FOR DETAIL (INCHES) | | | | |
|-------------------------------|---|-----------------------------|--|--|--|
| | WALL HEIGHT | | | | |
| | 8 FEET | 9 FEET | 10 | 11 | |
| | ••• | | FEEI | FEEI | |
| SUPPORTING ROOF ONLY | 16 | 16 | 16 | 18 | |
| SUPPORTING ONE STORY AND ROOF | 24 | 24 | 24 | 27 | |
| | SUPPORTING ROOF ONLY | SUPPORTING ROOF ONLY 16 | W/ 8 FEET 9 FEET SUPPORTING ROOF ONLY 16 | INCHES WALL HEIG 8 FEET 9 FEET SUPPORTING ROOF ONLY 16 16 16 | (INCHES) WALL HEIGHT 8 FEET 9 FEET 10 11 FEET 9 FEET 10 11 SUPPORTING ROOF ONLY 16 16 18 |

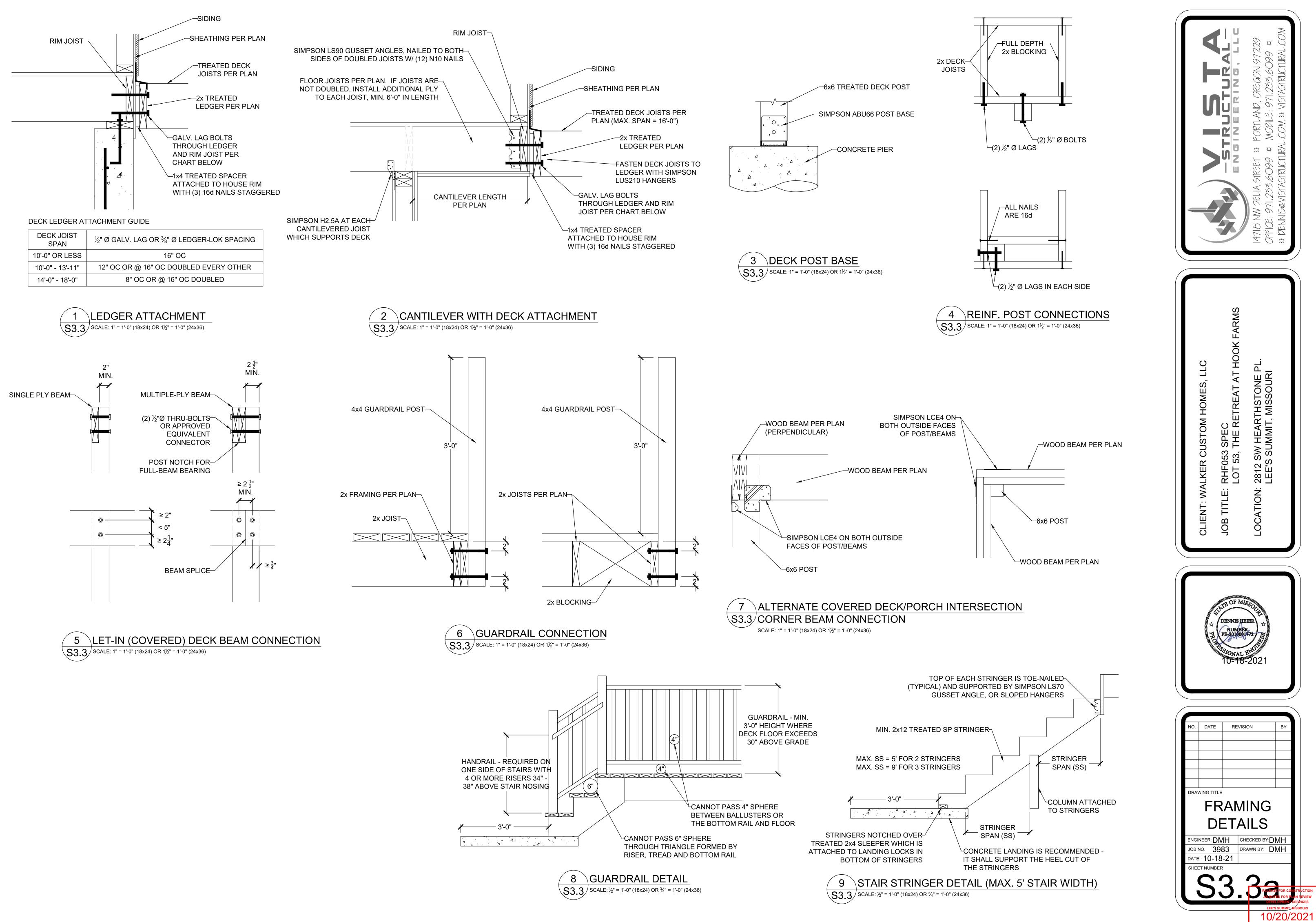


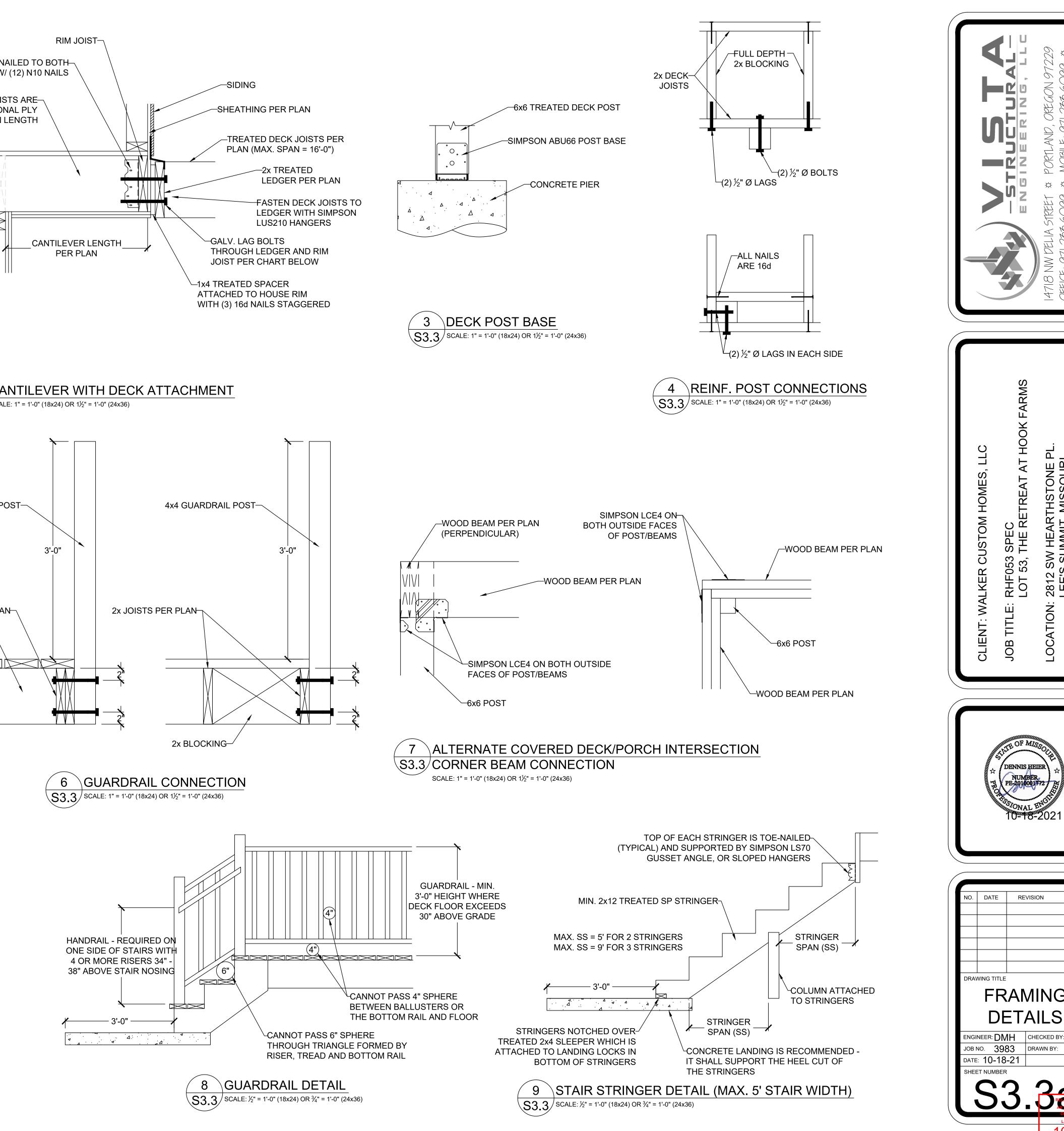


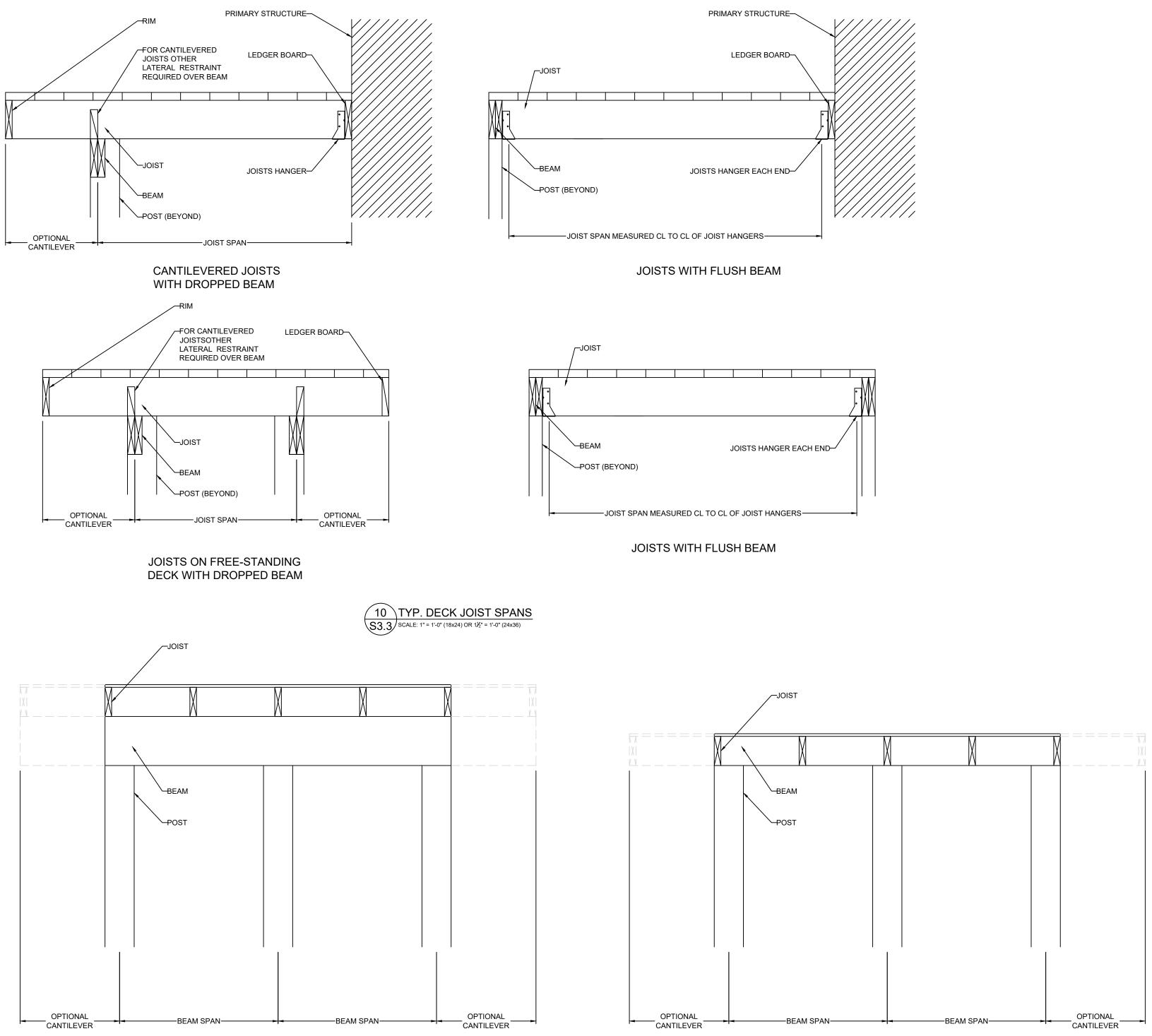












DROPPED BEAM



FLUSH BEAM

