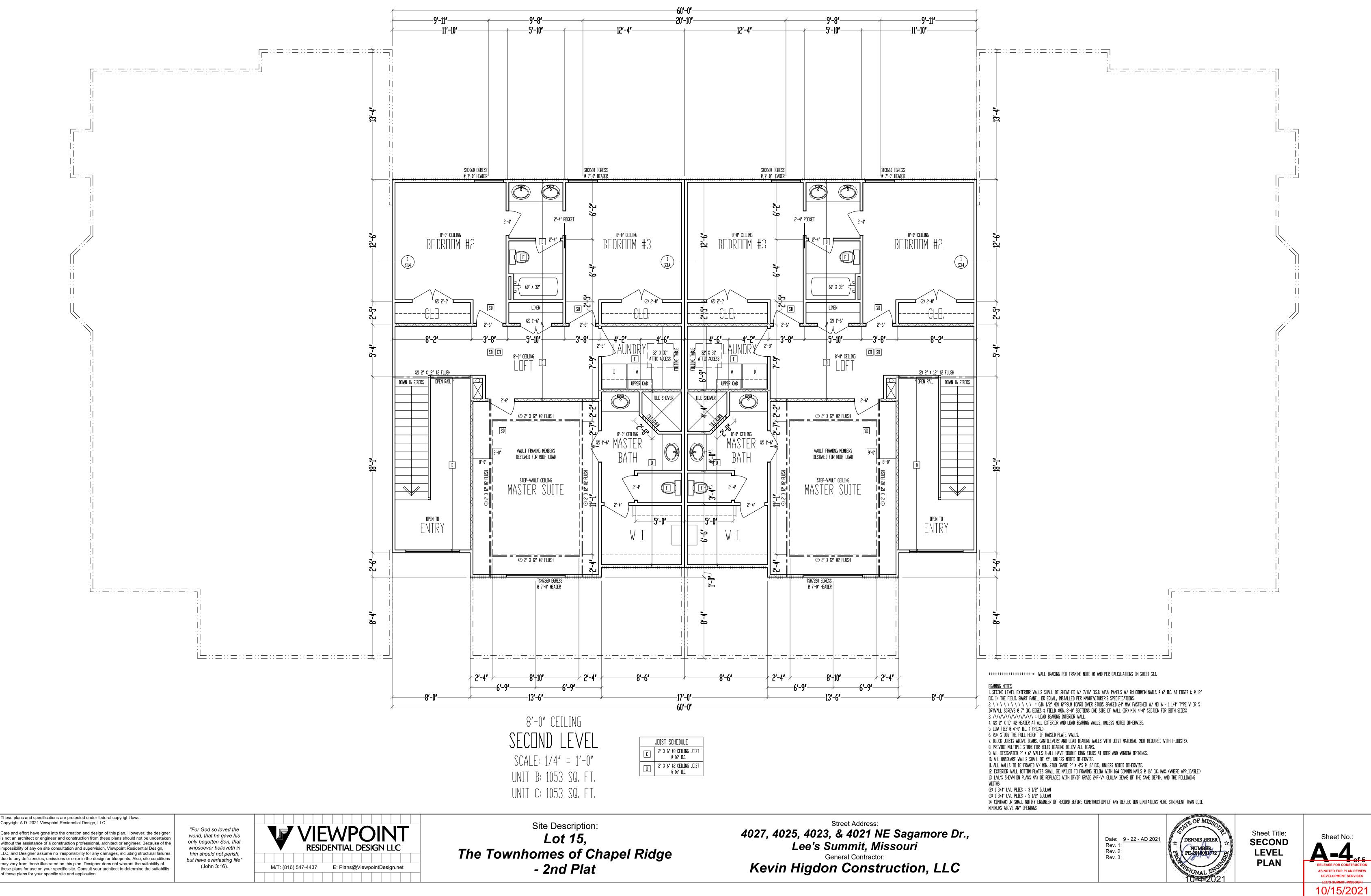


- 2nd Plat

of these plans for your specific site and application.

Kevin Higdon Construction, LLC AS NOTED FOR PLAN REVIEW DEVELOPMENT SERVICES

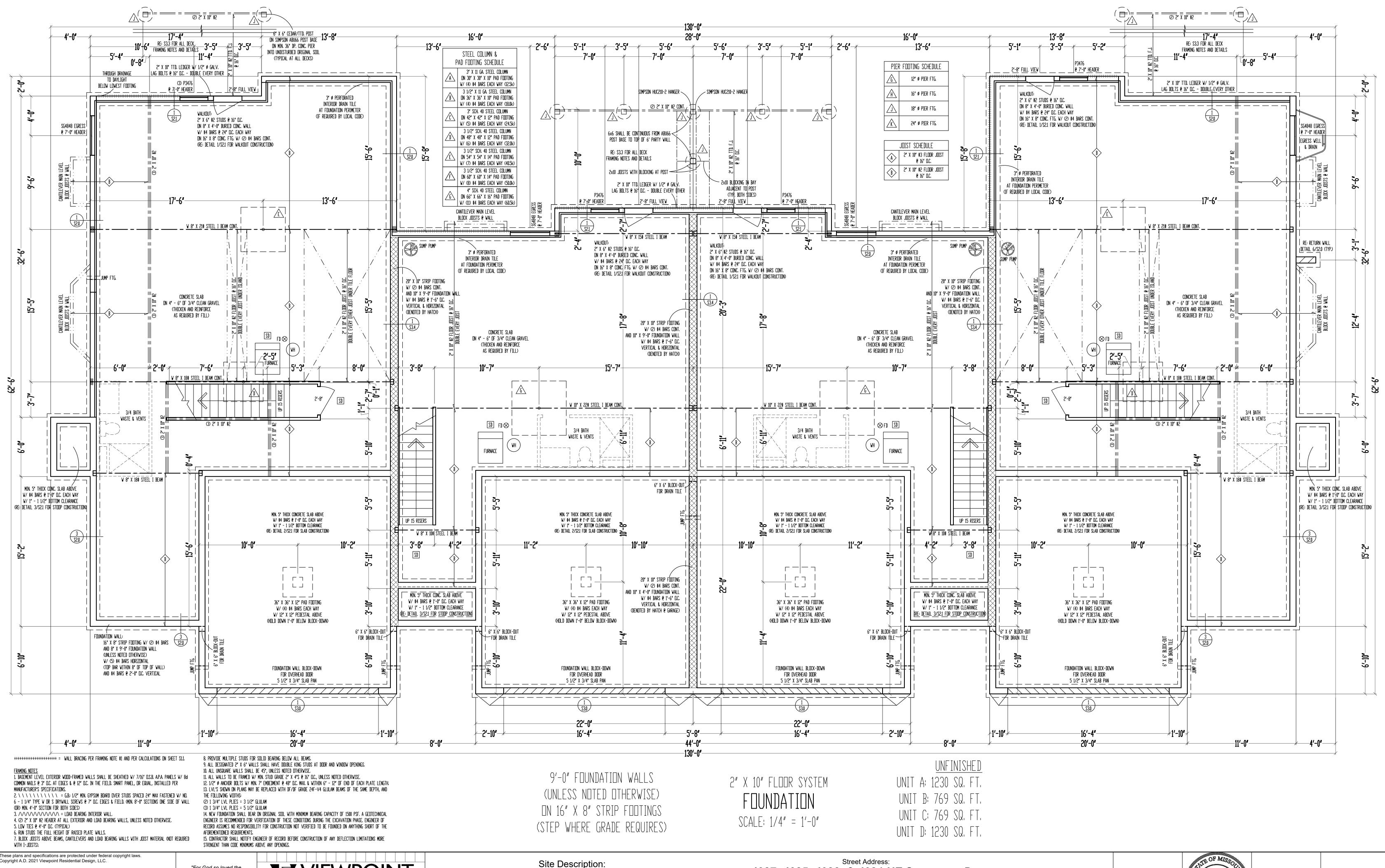
10/15/2021



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"For God so loved the world, that he gave his only begotten Son, that whosoever believeth in him should not perish, but have everlasting life" (John 3:16).



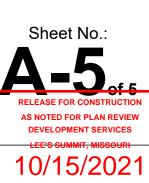
Site Description: Lot 15, The Townhomes of Chapel Ridge - 2nd Plat

Street Address: 4027, 4025, 4023, & 4021 NE Sagamore Dr., Lee's Summit, Missouri General Contractor: Kevin Higdon Construction, LLC

Date: 9 - 22 - AD 2021 Rev. 1 Rev. 2: Rev. 3:

DENNIS HEIE NUMBER E-20100017

Sheet Title: FOUNDATION PLAN



		FASTENER SCHEDULE FOR	STRUCTURAL MEMBERS		
DESCRIPTION OF BUILDING ELE	EMENTS	NUMBER AND TYP			SPACING OF FASTENERS
LOCKING BETWEEN JOISTS OR RAF	TERS TO TOP	ROO			
PLATE, TOE NAIL		3-8d (2½")			-
CEILING JOISTS TO PLATE, TO CEILING JOISTS NOT ATTACHED TO		3-8d (2½")	·		-
RAFTER, LAPS OVER PARTITIONS, COLLAR TIE TO RAFTER, FACE NAIL					-
GAGE RIDGE STRAP		3-10d (3" x 3-16d BOX NAILS (3½" x 0.		2 TOE NA	-
RAFTER OR ROOF TRUSS TO PLATE		NAILS (3/2 × 0. NAILS (3"			E SIDE OF EACH RAFTER OR TRUSS
ROOF RAFTERS TO RIDGE, VALLE RAFTERS: TOE NAIL FACE N		4-16d (3½" x 0.135"),			-
BUILT-UP STUDS - FACE NA	AIL	WA 10d (3" x			24" O.C.
ABUTTING STUDS AT INTERSECTI CORNERS, FACE NAIL		16d (3½" >			12" O.C.
UILT-UP HEADER, TWO PIECES WITH	H½" SPACER	16d (3½" >	(0.135")		16" O.C. ALONG EACH EDGE
CONTINUED HEADER, TWO PI		16d (3½" >	< 0.135")		16" O.C. ALONG EACH EDGE
· · · · · · · · · · · · · · · · · · ·		4-8d (2½" :	x 0.113")		-
CONTINUOUS HEADER TO STUD,		10d (3" x			24" O.C.
DOUBLE STUDS, FACE NA	IL	``````````````````````````````````````			
DOUBLE TOP PLATES, FACE		10d (3" x			24" O.C.
OOUBLE TOP PLATES, MINIMUM 24-IN OF END JOINTS, FACE NAIL IN LAP		8-16d (3½"	x 0.135")		-
SOLE PLATE TO JOIST OR BLOCKING	G, FACE NAIL	16d (3½" >	(0.135")		16" O.C.
SOLE PLATE TO JOIST OR BLOCKING WALL PANELS	AT BRACED	3-16d (3½"	x 0.135")		16" O.C.
STUD TO SOLE PLATE, TOE I	NAIL	3-8d (2½" x 0.113") OR	2-16d (3½" x 0.135")		-
TOP OR SOLE PLATE TO STUD, E	END NAIL	2-16d (3½"	x 0.135")		-
TOP PLATES, LAPS AT CORNER		2-10d (3" x	x 0.128")	-	
INTERSECTIONS, FACE NA BRACE TO EACH STUD AND PLATE		2-8d (2½" :	x 0.113")		-
I"x6" SHEATHING TO EACH BEARING		2-8d (2½" :	x 0.113") -		-
		2-8d (2½"	' x 0.113")		
1"x8" SHEATHING TO EACH BEARING 		3-8d (2½"			
FACE NAIL	CH BEARING,	3-ou (2/2)			-
		FLO	OR ¹	1	
JOIST TO SILL OR GIRDER, TO	E NAIL	3-8d (2½" :	x 0.113")		-
RIM JOIST TO TOP PLATE, TOE NA APPLICATIONS ALSO)	AIL (ROOF	8d (2½" ×	x 0.113"		6" O.C.
IM JOIST OR BLOCKING TO SILL PLA	TE, TOE NAIL	8d (2½" x 0.113")			6" O.C.
1"x6" SUBFLOOR OR LESS TO EACH . NAIL	JOIST, FACE	2-8d (2½" x 0.113")			-
2" SUBFLOOR TO JOIST OR GIRDER,	, BLIND AND	2-16d (3½"	x 0.135")		-
FACE NAIL PLANKS (PLANK AND BEAM - FLOOI		2-16d (3½" x 0.135")		AT EACH BEARING	
· · · · · · · · · · · · · · · · · · ·		10d (3" x		NAIL EACH LAYER AS FOLLOWS: 32" O.C. AT TOP	
BUILT-UP GIRDERS AND BEAMS, 2-IN LAYERS	ICH LUMBER			AND BOTTOM AND STAGGERED. TWO NAILS AT ENDS AND AT EACH SPLICE	
EDGER STRIP SUPPORTING JOISTS	OR RAFTERS	3-16d (3½"	x 0.135")		AT EACH JOIST OR RAFTER
SCRIPTION OF BUILDING MATERIALS	S DESCR	FASTNER SCHEDULE FOR	STRUCTURAL MEMBERS EDGE SPACING (IN(CHES)	INTERMEDIATE SUPPORTS (INCHES)
WOOD STRUCTURAL PANELS, SU			THING TO FRAMING AND F	PARTICLEBOA	RD WALL SHEATHING TO FRAMING ¹
3/8" - 1/2"		IMON (2" x 0.113") NAIL R, WALL) 8d COMMON NAIL (ROOF)	6		12
	84.00M	MON NAIL (2½" x 0.131")	6		12
¹⁹ ⁄ ₃₂ " - 1"	10d COMMON (3" x 0.148") NAIL OR 8d				12
¹⁹ / ₃₂ " - 1" 11/ ₈ " - 11/ ₄ "	10d COMMO	,	6		12
	10d COMMO	DN (3" x 0.148") NAIL OR 8d 131") DEFORMED NAIL OTHER WALL			12
	10d COMMO (2½" x 0. 1½" GALV	131") DEFORMED NAIL			7
11/8" - 11/4"	10d COMMC (2½" x 0. 1½" GALV STAPLE GA SCR	131") DEFORMED NAIL OTHER WALL ANIZED ROOFING NAIL; LVANIZED, 1½" LONG; 1¼" EWS, TYPE W OR S	SHEATHING ¹		
11⁄8" - 11⁄4"	10d COMMO (2½" x 0. 1½" GALV STAPLE GA SCR 1¾" GALV STAPLE GA	131") DEFORMED NAIL OTHER WALL ANIZED ROOFING NAIL; LVANIZED, 1½" LONG; 1¼"	SHEATHING ¹		
1 ¹ / ₈ " - 1 ¹ / ₄ " ¹ / ₂ " GYPSUM SHEATHING ⁵ / ₈ " GYPSUM SHEATHING	10d COMMO (2½" x 0. 1½" GALV STAPLE GA SCR 1¾" GALV STAPLE GA SCR	131") DEFORMED NAIL OTHER WALL ANIZED ROOFING NAIL; LVANIZED, 1½" LONG; 1¼" EWS, TYPE W OR S ANIZED ROOFING NAIL; LVANIZED, 15%" LONG; 15%" EWS, TYPE W OR S	SHEATHING ¹ 7 7	ΕΝΤ ΤΟ FRAM	7 7
1 ¹ / ₈ " - 1 ¹ / ₄ " ¹ / ₂ " GYPSUM SHEATHING ⁵ / ₈ " GYPSUM SHEATHING	10d COMMC (2½" x 0. 1½" GALV STAPLE GA SCR 1¾" GALV STAPLE GA SCR	131") DEFORMED NAIL OTHER WALL ANIZED ROOFING NAIL; LVANIZED, 1½" LONG; 1¼" EWS, TYPE W OR S ANIZED ROOFING NAIL; LVANIZED, 15%" LONG; 15%"	SHEATHING ¹ 7 7	ENT TO FRAM	7 7
1 ¹ / ₈ " - 1 ¹ / ₄ " ¹ / ₂ " GYPSUM SHEATHING ⁵ / ₈ " GYPSUM SHEATHING	10d COMMC (2½" x 0. 1½" GALV STAPLE GA SCR 1¾" GALV STAPLE GA SCR 7000D STRUCTU	131") DEFORMED NAIL OTHER WALL ANIZED ROOFING NAIL; LVANIZED, 1½" LONG; 1¼" EWS, TYPE W OR S ANIZED ROOFING NAIL; LVANIZED, 15%" LONG; 15%" EWS, TYPE W OR S	SHEATHING ¹ 7 7	ENT TO FRAM	7 7
1 ¹ / ₈ " - 1 ¹ / ₄ " ¹ / ₂ " GYPSUM SHEATHING ⁵ / ₈ " GYPSUM SHEATHING ³ / ₄ " AND LESS	10d COMMC (2½" x 0. 1½" GALV STAPLE GA SCR 1¾" GALV STAPLE GA SCR 000D STRUCTU 6d DEFORM COMM	131") DEFORMED NAIL OTHER WALL (ANIZED ROOFING NAIL; LVANIZED, 1½" LONG; 1¼" EWS, TYPE W OR S (ANIZED ROOFING NAIL; LVANIZED, 15%" LONG; 15%" EWS, TYPE W OR S IRAL PANELS, COMBINATION ED (2" x 0.120") NAIL OR 8d ON (2½" x 0.131") NAIL OR 8d	SHEATHING ¹ 7 7 N SUBFLOOR UNDERLAYM	ENT TO FRAM	7 7 ING ¹ 12
1 ¹ / ₈ " - 1 ¹ / ₄ " ¹ / ₂ " GYPSUM SHEATHING ⁵ / ₈ " GYPSUM SHEATHING	10d COMMC (2½" x 0. 1½" GALV STAPLE GA SCR 1¾" GALV STAPLE GA SCR 000D STRUCTU 6d DEFORM COMM	131") DEFORMED NAIL OTHER WALL (ANIZED ROOFING NAIL; LVANIZED, 1½" LONG; 1¼" EWS, TYPE W OR S (ANIZED ROOFING NAIL; LVANIZED, 15%" LONG; 15%" EWS, TYPE W OR S IRAL PANELS, COMBINATION ED (2" x 0.120") NAIL OR 8d ON (2½" x 0.131") NAIL	SHEATHING ¹ 7 7 N SUBFLOOR UNDERLAYM	ENT TO FRAM	7 7 ING ¹

1. IF INFORMATION LISTED ON PLAN SHEETS CONTRADICTS INFORMATION IN THIS TABLE, INFORMATION ON PLANS TAKES PRECEDENCE OVER INFORMATION LISTED IN THIS TABLE

FOUNDATION NOTES

CONCRETE SHALL BE AIR-ENTRAINED BETWEEN 5%-7% WITH A MINIMUM 28-DAY COMPRESSIVE STRENGTH OF 2500 PSI FOR BASEMENT AND INTERIOR FLOOR SLABS-ON-GRADE, 3000 PSI FOR FOUNDATION WALLS, AND 3500 PSI FOR PORCHES AND GARAGE FLOOR SLABS

THE FOUNDATION DESIGN SHALL COMPLY WITH THE ENFORCING JURISDICTION'S RESIDENTIAL FOUNDATION STANDARDS

PROVIDE A MINIMUM 4"-DIAMETER PERFORATED DRAIN PIPE ALONG PERIMETER OF USABLE SPACE AT FOOTING LEVEL OR OTHER EQUIVALENT MATERIALS PER IRC SECTION R405.1. THE PIPE SHALL BE COVERED WITH A MINIMUM OF 6" OF GRAVEL OR CRUSHED ROCK. THE DRAIN SHALL DAYLIGHT BELOW FOOTING LEVEL OR TERMINATE IN A MINIMUM 20 GALLON SUMP PIT.

FOUNDATION SHALL BE DESIGNED FOR A BEARING CAPACITY OF 1500 PSF AND FOUNDED ON COMPETENT ORIGINAL SOIL AS DETERMINED AND CONFIRMED BY A LICENSED GEOTECHNICAL ENGINEER OR ENGINEERING GEOLOGIST. ENGINEER OF RECORD ASSUMES NO RESPONSIBILITY FOR CONSTRUCTION NOT VERIFIED TO BE FOUNDED ON ANY SOIL WITH THE AFOREMENTIONED MINIMUM PROPERTIES.

FOOTINGS SHALL BE A MINIMUM OF 16" WIDE x 8" DEEP AND SHALL HAVE A MINIMUM OF (2) CONTINUOUS GRADE 40 #4 BARS WITH 3" BOTTOM CLERANCE. BOTTOM OF FOOTING SHALL BE LOCATED A MINIMUM OF 3'-0" BELOW GRADE FOR FROST PROTECTION.

CONCRETE PADS SUP0PORTING COLUMN LOADS SHALL BE NO SMALLER THAN 2'-0" x 2'-0" x 1'-0" DEEP WITH A MINIMUM OF (4) GRADE 40 #4 BARS EACH WAY WITH 3" BOTTOM CLEARANCE

FOUNDATION WALLS SHALL BE A MINIMUM OF 8" NOMINAL WIDTH AND SHALL HAVE HOIZONTAL GRADE 40 #4 BARS AT 2'-0" O.C. MAX. WITH VERTICAL #4 BARS AS REQUIRED ON FOUNDATION CROSS SECTION ON SHEET S2.0 REINFORCEMENT SHALL LAP A MINIMUM OF 2'-0" (CLASS B SPLICE)

- INTERIOR BEARING WALLS AND COLUMNS SHALL BE ISOLATED FROM THE BASEMENT FLOOR SLAB
- BASEMENT FLOOR SLAB SHALL BE A MINIMUM OF 4" THICK ON A MINIMUM BASE COURSE OF 4" TO 6" OF SAND,

GRAVEL OR CRUSHED ROCK. BETWEEN THE BASE COURSE AND FLOOR SLAB SHALL BE PLACED A 6-MIL POLY VAPOR RETARDER WITH MINIMUM OVERLAP OF 6" AT DISCONTINUITIES 11. IF A FLOOR IS TO BE SUPPORTED BY A MINIMUM OF 2'-0" OF GRANULAR FILL OR 8" OF EARTH, BASEMENT SLAB

SHALL BE DESIGNED BY A LICENSED ENGINEER SILL PLATES SHALL BE ANCHORED TO THE FOUNDATION WALL WITH $\frac{1}{2}$ " Ø ANCHOR BOLTS EMBEDDED A MINIMUM OF 7" INTO CENTER OF WALL STEM AND SHALL BE INSTALLED AT A MAXIMUM OF 6'-0" O.C. (OR AS NOTED ON PLANS) AND SHALL BE INSTALLED WITHIN 6" TO 12" OF EACH END OF EACH SILL PLATE LENGTH, PER IRC SECTION R403.1.6 FOUNDATION WINDOW WELLS SHALL BE PROVIDED WITH MINIMUM DIMENSIONS AS SHOWN IN DETAIL ON SHEET

14. THE GARAGE FLOOR SHALL SLOPE TOWARD THE VEHICLE DOORS OR TO A TRENCH OR UNTRAPPED DRAIN THAT DISCHARGES TO THE EXTERIOR, ABOVE GRADE

FRAMING NOTES

S2.0

ALL DIMENSIONAL LUMBER SHALL BE DOUGLAS-FIR-LARCH GRADE #2, UNLESS NOTED OTHERWISE ON PLANS ALL INTERIOR LOAD-BEARING AND EXTERIOR WALL HEADERS SHALL BE (2) #2 - 2x10's, UNLESS NOTED OTHERWISE ON PLANS

BLOCK OVER BEAMS AND AT CANTILEVERS AND DOOR JAMBS

INTERIOR NON-BEARING WALLS RESTING ON BASEMENT SLAB SHALL BE ISOLATED FROM ABOVE FRAMING BY A MINIMUM OF 3/5" ALL HEADERS/BEAMS SHALL BEAR ON A MINIMUM OF (2) 2x4 POSTS (KING AND JACK STUDS), UNLESS NOTED

OTHERWISE

20. WHERE JOISTS SPAN PARALLEL TO FOUNDATION, BLOCKING SHALL BE PROVIDED IN THE TWO SPACES MOST ADJACENT TO THE FOUNDATION WALL AT 4'-0" O.C. FOR THE PURPOSE OF TRANSFERRING LATERAL FOUNDATION WALL LOAD TO THE FLOOR DIAPHRAGM. FASTEN JOISTS AND BLOCKING TO SILL PLATE WITH (4) 10d NAILS. IF MECHANICAL DUCTWORK IS INSTALLED IN ONE OF THESE FIRST TWO BAYS, FASTEN 2x4's FLAT AT 4'-0" O.C. BETWEEN JOIST(S) AND/OR SILL AND PROVIDE BLOCKING AS PRESCRIBED ABOVE IN THE NEXT TWO JOIST BAYS. SECURE 2x4's TO JOIST(S)/SILL PLATE WITH (4) 10d NAILS. ALL WOOD MATERIAL SUPPORTED ON CONCRETE OR MASONRY SHALL BE TREATED OR OF DECAY-RESISTANT

MATERIAL JOISTS UNDER BEARING PARTITIONS ON PLANS HAVE BEEN SIZED TO SUPPORT THE DESIGN LOAD.

23. JOISTS FRAMING INTO THE FACE OF A STEEL OR WOOD BEAM SHALL BE SUPPORTED WITH APPROPRIATE COLD-FORMED STEEL JOIST HANGERS

JOISTS FRAMED ON TOP OF STRUCTURAL MEMBER SHALL BE SUPPORTED AT EN DS BY FULL-DEPTH SOLID BLOCKING MIN. 1/4" IN THICKNESS OR BY FASTENING RIM TO JOISTS PER FASTENING TABLE TO LEFT ALL WALL COVERINGS SHALL COMPLY WITH IRC SECTION R702.3

ALL RAFTERS AND COLLAR TIES SHALL COMPLY WITH IRC SECTION R802.3.

ALL RAFTERS SHALL HAVE 2x4 COLLAR TIES @ 4'-0" O.C. IN UPPER $\frac{1}{3}$ OF VERTICAL DISTANCE BETWEEN CEILING AND ROOF

BLOCKING BETWEEN JOISTS UNDER A LOAD-BEARING WALL IS NOT REQUIRED PER IRC SECTION 501.3, BOTTOM OF ALL FLOOR ASSEMBLIES ABOVE UNFINISHED AREAS SHALL BE PROVIDED WITH A $\frac{1}{2}$ " GYPSUM BOARD MEMBRANE OR RESIDENTIAL FIRE SPRINKLER SYSTEM WHEN FLOOR SYSTEM IS CONSTRUCTED OF OTHER THAN DIMENSION LUMBER OR STRUCTURAL COMPOSITE LUMBER EQUAL TO OR GREATER THAN 2x10 NOMINAL DIMENSION(WHERE REQUIRED BY ENFORCING JURISDICTION)

30. ENGINEERED LVL's SHALL HAVE MINIMUM PROPERTIES OF Fb = 2600 psi, E=1900 ksi, AND Fv=285 psi

ENGINEERED PARALLAMS SHALL HAVE MINIMUM PROPERTIES OF Fb = 2600 psi, E = 2000 ksi, AND Fv = 290 psi COLUMN CONNECTION TO STEEL BEAMS SHALL BE WITH A CLIP POST CAP WITH ALL FOUR TAB EARS BENT AROUND THE BOTTOM FLANGE OF THE BEAM. FOR A BEARING PLATE, FOUR HOLES SHALL BE DRILLED IN THE BOTTOM FLANGE OF THE STEEL BEAM TO MATCH THE HOLE PATTERN OF THE PLATE. ¹/₂" x 2" BOLTS SHALL THEN BE INSTALLED WITH A FLAT WASHER, LOCK WASHER, AND A NUT IN EACH OF THE HOLES. THE POST CAP MAY BE WELDED TO THE STEEL BEAM IN ACCORDANCE WITH AWS D1.1-92 AS AN ALTERNATIVE, AND WOULD NEED TO BE INSPECTED BY AN AWS-CERTIFIED INSPECTOR.

WHEN MECHANICAL EQUIPMENT IS LOCATED IN AN ENCLOSED ROOM, THERE SHALL BE (2) 14"x12" VENTS LOCATED IN A WALL COMMON WITH ADDITIONAL LIVING AREA. ONE VENT SHALL BE LOCATED SUCH THAT THE BOTTOM OF THE VENT BEGINS 12" FROM THE FLOOR AND THE OTHER VENT SHALL BE LOCATED SUCH THAT THE TOP OF THE VENT BEGINS 12" FROM THE CEILING.

34. ALL ROOF SHEATHING SHALL BE $\frac{7}{16}$ " OSB WITH 8d COMMON NAILS @ 6" O.C. AT PANEL EDGES AND @ 12" O.C. IN FIELD

GLAZING NOTES

35. GLAZING IN HAZARDOUS LOCATIONS AS IDENTIFIED IN IRC SECTION R308.4 SHALL BE OF APPROVED SAFETY GLAZING MATERIALS. GLASS IN STORM DOORS, INDIVIDUAL FIXED OR OPENABLE PANELS ADJACENT TO A DOOR WHERE THE NEAREST VERTICAL EDGE IS WITHIN A 2'-0" ARC OF THE DOOR IN A CLOSED POSITION AND FOR WHICH THE BOTTOM EDGE IS WITHIN 5'-0" OF THE FLOOR, WALLS ENCLOSING STAIRWAYS AND LANDINGS WHERE THE GLAZING IS WITHIN 5'-0" OF THE TOP OR BOTTOM OF THE STAIR, ENCLOSURES FOR SPAS, TUBS, SHOWERS, AND WHIRLPOOLS, GLAZING IN FIXED OR OPENABLE PANELS EXCEEDING NINE SQUARE FEET AND FOR WHICH THE BOTTOM EDGE IS LESS THAN 1'-6" ABOVE THE FLOOR OR WALKING SURFACE WITHIN 3'-0" 36. ALL OPERABLE WINDOWS SHALL HAVE FALL PROTECTION PER IRC SECTION R612.2

ATTIC VENTILATION

ENCLOSED ATTICS SHALL HAVE CROSS VENTILATION FOR EACH SEPARATE SPACE BY VENTILATING OPENINGS PROTECTED AGAINST THE ENTRANCE OF RAIN OR SNOW. VENTILATING OPENINGS SHALL BE PROVIDED WITH CORROSION-RESISTANT WIRE MESH, WITH $\frac{1}{6}$ " TO $\frac{1}{4}$ " OPENINGS. THE TOTAL FREE VENTILATING AREA SHALL NOT BE LESS THAN $\frac{1}{150}$ OF THE AREA OF SPACE VENTILATED, EXCEPT WHERE THE VENTILATORS ARE LOCATED IN THE UPPER PORTION OF THE SPACE TO BE VENTILATED - THE REQUIRED AREA MAY BE REDUCED TO 1/300.

EMERGENCY EGRESS

PROVIDE A MINIMUM OF ONE WINDOW FOR EACH BEDROOM THAT HAS A MINIMUM OPENABLE AREA OF 5.7 SQUARE FEET WITH A MINIMUM OPENABLE HEIGHT OF 2'-0" AND A MINIMUM WIDTH OF 1'-9". IN ADDITION, THE OPENABLE PORTION OF EGRESS WINDOWS SHALL NOT EXCEED 3'-8" ABOVE THE ADJOINING FLOOR OR PERMANENT STEP. PROVIDE SMOKE ALARMS IN EACH SLEEPING ROOM, OUTSIDE OF EACH SLEEPING AREA AND ON EACH FLOOR, INCLUDING BASEMENT (IF APPLICABLE). ALARMS SHALL BE HARDWIRED TOGETHER SO THAT THE ACTIVATION OF ONE SMOKE ALARM WILL ACTIVATE ALL SMOKE ALARMS IN THE DWELLING. PROVIDE CARBON MONOXIDE DETECTORS OUTSIDE EACH SLEEPING AREA.

MASONRY VENEER

40. MASONRY VENEER SHALL BE ANCHORED TO THE SUPPORTING WALL STUDS WITH CORROSION-RESISTANT METAL TIES EMBEDDED IN MORTAR OR GROUT AND EXTENDING INTO THE VENEER A MINIMUM OF 1¹/₂", WITH NOT LESS THAN $\frac{5}{8}$ " MORTAR OR GROUT COVER TO OUTSIDE FACE.

- VENEER TIES, IF STRAND WIRE, SHALL NOT BE LESS IN THICKNESS THAN NO. 9 U.S. GAGE WIRE AND SHALL HAVE A 41. HOOK EMBEDDED IN THE MORTAR JOINT, OR IF SHEET METAL, SHALL BE NOT LESS THAN NO. 22 U.S. GAGE BY 7/8" CORRUGATED
- 42. EACH TIE SHALL SUPPORT NOT MORE THAN 2.67 SQUARE FEET OF WALL AREA AND SHALL BE SPACED NOT MORE THAN 32 INCHES ON CENTER HORIZONTALLY AND 24 INCHES ON CENTER VERTICALLY.
- 43. VENEER TIES AROUND WALL OPENINGS: ADDITIONAL METAL TIES SHALL BE PROVIDED AROUND ALL WALL OPENINGS GREATER THAN 16 INCHES IN EITHER DIMENSION. METAL TIES AROUND THE PERIMETER OF OPENINGS SHALL BE SPACED NOT MORE THAN 3 FEET ON CENTER AND PLACED WITHIN 12 INCHES OF THE WALL OPENING.

GARAGE NOTES

- DOOR(S) BETWEEN THE GARAGE AND DWELLING SHALL BE MINIMUM 1%" SOLID CORE OR HONEY-COMBED STEEL DOOR WITH 20-MINUTE FIRE RATING EQUIPPED WITH A SELF-CLOSING DEVICE 45. VEHICLE DOORS AND FRAMES SHALL BE DESIGNED AND INSTALLED TO MEET THE 90-MPH 3-SECOND GUST LOADING
- PER DASMA 108 AND ASTM E 330-96 PER IRC SECTION R301.2.1

GARAGE NOTES (CONTINUED)

- THE GARAGE SHALL BE SEPARATED FROM THE DWELL MINIMUM 5/8" GYP. BOARD APPLIED TO THE GARAGE SID HABITABLE SPACE OCCURS ABOVE THE GARAGE, THE SHALL BE PROTECTED WITH A MINIMUM 5/1 TYPE X GYP FLOOR/CEILING SPACE IS PROVIDED ABOVE THE GARA SUPPORTING THE SEPARATION SHALL ALSO BE PROTE
- 45 GARAGE DOOR H-FRAME FOR THE ATTACHMENT OF TH BALANCE SHALL CONSIST OF THE FOLLOWING: 2x6 VER FLOOR TO CEILING AND SHALL BE FASTENED WITH 21/2" STAGGERED WITH (7) 3¹/₄" x 0.120" NAILS THROUGH THE MINIMUM 2x8 HEADER FOR ATTACHMENT OF COUNTER

DESIGN LOADING (PER TABLE R301.5)

MINIMUM UNIFORMLY DISTRIB USE	LIVE LOAD	DEAD LOAD
UNINHABITABLE ATTICS WITHOUT STORAGE	10	10
UNINHABITABLE ATTICS WITH LIMITED STORAGE	20	10
HABITABLE ATTICS AND ATTICS SERVED WITH FIXED STAIRS	30	10
BALCONIES (EXTERIOR) AND DECKS	40	10 ^d
FIRE ESCAPES	40	10
GUARDRAILS AND HANDRAILS ^a	200 [°]	-
GUARDRAIL IN-FILL COMPONENTS ^b	50 [°]	-
PASSENGER VEHICLE GARAGES	50	DEPENDENT UPON SLAB CONSTRUCTION
ROOMS OTHER THAN SLEEPING ROOM	40	10 ^d
SLEEPING ROOM	30	10 ^d
STAIRS	40	10 ^d

a. A single concentrated load applied in any direction at any point along the top. b. Guard in-fill components (all those except the handrail), ballusters and panel fillers shall be designed to withstand a horizontally applied normal load of 50 pounds on an area equal to one square foot. This load need not be assumed to act concurrently with any other live load requirement. c. Glazing used in handrail assemblies and guards shall be designed with a safety factor of 4. The safety factor shall be applied to each of the concentrated loads applied to the top of the rail, and to the load on the infill components. These loads shall be determined independently of one another, and loads are assumed not to occur with any other live load. d. An additional dead loading of 10 psf shall be applied where thinset tile floor is to be installed. An additional dead loading of 50 psf shall be applied where mudset tile floor is to be installed. INSULATION/EFFICIENCY

BUILDING ENVELOPE INSULATION SHALL COMPLY WITH IRC TABLE N1102.1.1 OR THE 2012

- IECC (SEE SHEET S3.1 FOR FRAMING DETAILS AND TABLES ON THIS SHEET FOR MORE INFORMATION)
- CATHEDRAL -VAULTED CEILING FRAMING SHALL BE FRAMED WITH A MINIMUM INSULATION VALUE OF R-38. IF VAULTED RAFTERS DO NOT PROVIDE REQUIRED DEPTH TO ACHIEVE R-38 INSULATION BUILDER SHALL FUR DOWN RAFTERS PER DETAILS PROVIDED ON SHEET S3.1.

INSULATION AND FENESTRATION REQUIREMENTS BY COMPONENT (TABLE N1102.1.1)

CLIMATE ZONE	4-A
FENESTRATION U-FACTOR	0.35
SKYLIGHT U-FACTOR	0.55
GLAZED FENSTRATION SHGC	0.40
CEILING R-VALUE	49
WOOD FRAME WALL R-VALUE	13
MASS WALL R-VALUE	8 / 13
FLOOR R-VALUE	19
BASEMENT WALL R-VALUE	10-CONTINUOUS OR 13-CAVITY
SLAB R-VALUE AND DEPTH	10 AT 2'-0"
CRAWL SPACE WALL R-VALUE	10-CONTINUOUS OR 13-CAVITY
DUCTWORK EXPOSED TO OUTSIDE AIR R-VALUE	8
DUCTWORK NOT EXPOSED TO OUTSIDE AIR R-VALUE	6
CATHEDRAL VAULTED CEILING R-VALUE	38

DUCT SEALING

N1103.2.2 (R403.2.2) SEALING (MANDATORY). DUCTS, AIR HANDLERS, AND FILTER BOXES SHALL BE SEALED. JOINTS AND SEAMS SHALL COMPLY WITH SECTION M1601.4.1 OF 2012 IRC. EXCEPTIONS: AIR-IMPERMEABLE SPRAY FOAM PRODUCTS SHALL BE PERMITTED TO BE APPLIED 1. WITHOUT ADDITIONAL JOINT SEALS.

- WHERE A DUCT CONNECTION IS MADE THAT IS PARTIALLY INACCESSIBLE, THREE SCREWS OR RIVETS SHALL BE EQUALLY SPACED ON THE EXPOSED PORTION OF THE JOINT SO AS TO PREVENT A HINGE EFFECT.
- CONTINUOUSLY WELDED AND LOCKING-TYPE LONGITUDINAL JOINTS AND SEAMS IN DUCTS OPERATING AT STATIC PRESSURES LESS THAN 2 INCHES OF WATER COLUMN PRESSURE CLASSIFICATION SHALL NOT REQUIRE ADDITIONAL CLOSURE SYSTEMS.
- DUCT TIGHTNESS SHALL BE VERIFIED BY EITHER OF THE FOLLOWING: POST-CONSTRUCTION TEST: TOTAL LEAKAGE SHALL BE LESS THAN OR EQUAL TO 4 CFM 1. PER 100 SQUARE FEET OF CONDITIONED FLOOR AREA WHEN TESTED AT A PRESSURE DIFFERENTIAL OF 0.1 INCHES W.G. ACROSS THE ENTIRE SYSTEM, INCLUDING THE MANUFACTURER'S AIR HANDLER ENCLOSURE. ALL REGISTER BOOTS SHALL BE TAPED
- OR OTHERWISE SEALED DURING THE TEST. ROUGH-IN TEST: TOTAL LEAKAGE SHALL BE LESS THAN OR EQUAL TO 4 CFM PER 100 SQUARE FEET OF CONDITIONED FLOOR AREA WHEN TESTED AT A PRESSURE DIFFERENTIAL OF 0.1 INCHES W.G. ACROSS THE SYSTEM, INCLUDING THE MANUFACTURER'S AIR HANDLER ENCLOSURE. ALL REGISTERS SHALL BE TAPED OR
- OTHERWISE SEALED DURING THE TEST. IF THE AIR HANDLER IS NOT INSTALLED AT THE TIME OF THE TEST, TOTAL LEAKAGE SHALL BE LESS THAN OR EQUAL TO 3 CFM PER 100 SQUARE FEET OF CONDITIONED FLOOR AREA.

EXCEPTION: THE TOTAL LEAKAGE TEST IS NOT REQUIRED FOR DUCTS AND AIR HANDLERS LOCATED ENTIRELY WITHIN THE BUILDING THERMAL ENVELOPE.

L ME	<u>ECHANICAL VENTILATIO</u>	
FAN LOCATION	AIR FLOW RATE	
	MINIMUM (CFM)	(CFM/WATT)
RANGE HOODS	ANY	2.8
IN-LINE FAN	ANY	2.8
BATHROOM, UTILITY ROOM	10	1.4
BATHROOM, UTILITY ROOM	90	2.8

ING AND ITS ATTIC AREAS BY
DE OF FRAMING. WHERE
GARAGE CEILING ASSEMBLY
P. BOARD. WHERE A
AGE COLUMNS AND BEAMS
ECTED WITH 5⁄8" GYP. BOARD.
HE TRACK AND COUNTER
RTICAL JAMBS RUNNING FROM
"" x 0.120" NAILS AT 7" O.C.
E JAMB INTO THE HEADER.
R BALANCE SYSTEM.

AIR FLOW RATE

MAXIMUM (CFM)

ANY

ANY

90

ANY

LO

Ш

DATE

DRAWING TITLE

DATE: 10-04-21

SHEET NUMBER

REVISION

STRUCTURAL

NOTES

ENGINEER: DMH CHECKED BY: DMH

JOB NO. 3905 DRAWN BY: DMH

10/15/2021

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RESIDENTIAL SEISMIC & WIND ANALYSIS

DETERMINE WEIGHT	OF HOUSE:					
LOCATION	DEAD LOAD (psf)					
ROOF	10					
CEILING					10	
FIRST FLOOR					10	
				WALL LENGTH (ft)	WALL HEIGHT (ft)	W
FIRST FLOOR EXT. W	ALL DL			354.66	10	
					DEAD LOAD (psf)	
FIRST FLOOR INT. PA	ARTITION WALL DL				6	
	PRO	JECTED AREAS (WIND	DESIGN PER 115 MPH 3	3-SECOND GUST, EXPOSU	RE C AND MEAN ROOF HEIGHT <= 30) FT
	FRONT	-TO-BACK			SIDE-TO-S	IDE
	AREA	LOAD			AREA	
SLOPED ROOF	555	4515		SLOPED ROOF	708	
VERT. ROOF	853	10129	CUMULATIVE	VERT. ROOF	30	
1ST	1287	15282	30007	1ST	663.63	
BSMT ^a	0	0	0	BSMT ^a	140	
			PRESSURE (PSF) - PER ASCE CH. 6		-
	SLOPED ROOF	ZONE B		9.7	ZONE C	
	WALL/VERT. ROOF	ZONE A		14.2	ZONE D	
	MEAN ROOF HT., h		24			
a) If there is a walkout	wall to be sheathed, det	ermine tributary wind area	and enter here. If no wa	alkout, enter 0 for a		
q_{z10} =0.00256K _z K _{zt} K _d V ² (ASCE7-10 Velocity Pressure) q_{z10} _ASD=0.6 q_{z10} (Design Velocity Pressure for ASD and the second s					analysis under ASCE7-10 and IRC/IBC 2	2012

1ST FLOOR TRIBUTARY WEIGHT

BASEMENT TRIBUTARY WEIGHT S_S(SITE GROUND MOTION - %g - FROM ASCE7 SEISMIC MAP) F_a (from ASCE7 Table 11.4-1)

S_{DS} (= 2/3 * S_S * F_a)

R (from ASCE7 Table 12.2-1)

OCATION T FLOOR				Fi
SEMENT				
Sheathing Location	Min. Sheath	ing Schedule	Fas	stening Schedule
Exterior <u>(Option #1)</u>	7/16" APA Rate	ed Plywood/OSB		penetration@ 6" OC Edges, 6" OC Fiel , 12" OC Field For 16" stud spacing
Exterior (Option #2)	7/16" APA Rate	ed Plywood/OSB		penetration@ 4" OC Edges, 6" OC Fiel , 12" OC Field For 16" stud spacing
Exterior <u>(Option #3)</u>	7/16" APA Rate	od Plywood/OSB		penetration@ 3" OC Edges, 6" OC Fiel , 12" OC Field For 16" stud spacing
Exterior (<i>Option #4)</i>	sheathing, or 3/8" ship	ood/OSB or shiplap panel lap panel sheathing with ail spacing	Field for 7/16" APA-rated	8" penetration @ 6" O.C. Edges, 12" O.C plywood/OSB or shiplap panel sheathin D.C. Field for 3/8" shiplap panel sheathi
Exterior (<i>Option #5)</i>	sheathing, or 3/8" ship	ood/OSB or shiplap panel lap panel sheathing with ail spacing	Field for 7/16" APA-rated	" penetration @ 4" O.C. Edges, 12" O.0 plywood/OSB or shiplap panel sheathin D.C. Field for 3/8" shiplap panel sheathi
Exterior (<i>Option #6)</i>	sheathing, or 3/8" ship tighter nail spacing and o	ood/OSB or shiplap panel lap panel sheathing with double studs at each pan lge	8d Common Nails w/ 1-3/8	" penetration @ 3" O.C. Edges, 12" O.C Field
Interior	1/2" Gyps	1/2" Gypsum Board		Screws @ 8" O.C. Edges, 12" O.C. Field
Interior		vpe WB Steel X-Brace (or ual)		3d @ intermediate studs (per manufactu is - see detail on sheet S3)
TERIOR SHEATHING OPTION FOR		4		WIDTH OF 1ST STORY (FT.)
TERIOR SHEATHING OPTION FOR	BASEMENT WALLS	4		DEPTH OF 1ST STORY (FT.)

BACK WALL OF GARAGE (FT.) GAR. WALL: 1=F-B, 2=S-S

			EXTER	RIOR STRUCTURAL WALL LE	NGTHS (ft.) & RESISTANCES		i i i i i i i i i i i i i i i i i i i	
	SEISMIC					WIND		
	FRONT-TO-BACK	RESISTANCE (lbs.)	SIDE-TO-SIDE	RESISTANCE (lbs.)	FRONT-TO-BACK	RESISTANCE (lbs.)	SIDE-TO-SIDE	RESISTANCE (lbs.)
1ST FLOOR	114	31920	49.5	13860	114	44688	49.5	19404
BASEMENT	0	0	75	21000	0	0	75	29400
				· · · · · · · · · · · · · · · · · · ·				
		ADDITIONAL RESIS	TANCE REQUIRED] [Anchor Bolt Spacir	ng (in.)	16d Nail Spacing req'd	at bottom plate (in
		SEISMIC	WIND	1 Г	diameter (in.)	0.5	1st Floor F-B	11
1ST FLOOR FRONT-	O-BACK	SEISMIC 0	WIND 0	F	diameter (in. Shear value (per NDS)	0.5 944	1st Floor F-B 1st Floor S-S	11
1ST FLOOR FRONT- 1ST FLOOR SIDE-TO		SEISMIC 0 0	WIND 0 0					11 11 43
	-SIDE	SEISMIC 0 0 0 0	WIND 0 0 0 0		Shear value (per NDS)	944		11 43

SEISMIC SHEAR

		RESISTANCE REQUI	RED IN ADDITION TO RES	ISTANCE PROVIDED BY EXTERIOR W	ALLS**		
	ADDITIONAL RESISTANCE REQUIRED (POUNDS)	PORTAL FRAMES OR PERF. SHEAR WALL RESISTANCE	INTERIOR X-BRACES (325#/BRACE)	INTERIOR WALL LENGTH W/ 1/2" GYPSUM BOARD PER TABLE (FT.)	INT. WALL LENGTH SHEATHED W/ OSB (TOTAL LENGTH, ONE SIDE, FT.)	RESISTANCE PROVIDED BY ADDITIONAL METHODS (POUNDS)	OK?
1ST FLOOR FRONT-TO-BACK	0					0	YES
1ST FLOOR SIDE-TO-SIDE	0					0	YES
BASEMENT FRONT-TO-BACK	0					0	YES
BASEMENT SIDE-TO-SIDE	0					0	YES
**NOTES: 1) SEE ATTACHED CALCULATION	IS FOR PORTAL FRAME	OR PERFORATED SHE	AR WALL RESISTANCE CA	APACITIES (IF APPLICABLE),			

2) SEE SHEET S1 FOR INTERIOR STEEL X-BRACE INSTALLATION, 3) INTERIOR WALLS SHEATHED WITH OSB SHALL BE ATTACHED WITH SAME STAPLE/NAILING PATTERN AS EXTERIOR OSB ON SAME FLOOR (SEE TABLE ABOVE) AND ARE ONLY APPLICABLE FOR FULL-HEIGHT SECTIONS OF 2'-8" OR LONGER ALL LATERAL BRACING ACHIEVED AT EXTERIOR WALLS AND WALLS DIRECTLY ON FOUNDATIONS; THEREFORE, NO INTERIOR BRACING PER 2012 IRC SECTION R502.2.1 IS REQUIRED

				WIND UPLIFT	ANALYSIS		
	X/12	DEGREES					
ROOF PITCH (MAX)	12	45.0	PITCH OF 6 OR LESS:	EOH -13.3, E -7.2, G -5.2			
		ASCE 7					
	LENGTH (FT.)	PRESSURE (PSF)	LINEAL FT. OF OH	UPLIFT PER FT* (LBS)			
OVERHANG	1	-1.08	356.66	-1.08			
	TOTAL AREA (FT ²)	ZONE E AREA (FT ²)	ZONE G AREA (FT ²)	PRESSURE ZN. E (PSF)	PRESSURE ZN. G (PSF)	TOTAL FORCE (LBS)	FORCE PER LINEAL FT @ PERIMETER (LBS)
MAIN ROOF**	7058.61	-534.089424	7592.699424	-1.08	-0.36	-2157	-6.1
						_	
*ALONG PERIMETER		TOTAL UPLIFT PER LINEAL	OOT ALONG EXTERIOR (PO	UNDS)	-7.2	UPLIFT OK	
**INSIDE EXTERIOR V	VALLS	RESISTANCE DUE TO DEAD	WEIGHT & (3) 10d TOENAILS		251.6		

NOTE FOR CONSTRUCTION: THE CONTINUOUS STRUCTURAL PANEL SHEATHING BRACING METHOD REQUIRES USE OF THE ABOVE TABLE FOR SHEATHING OF THE ENTIRE STRUCTURE. IN ADDITION, FRAMING MEMBERS SHALL BE @ 16" O.C. MAX., UNBLOCKED, AND W/ SHEATHING APPLIED DIRECTLY TO FRAMING MEMBERS

NOTE FOR DESIGN:

ALL WALLS USED IN THE CALCULATION OF THE RESISTANCE FOR THIS STRUCTURE SHALL HAVE A MINIMUM UNINTERRUPTED HEIGHT OF 8'-0" AND LENGTH OF 2'-8". ALLOWABLE RESISTANCES HAVE BEEN #/FT AND INCREASED BY 40% FOR WIND LOADS, PER VALUES IN 2012 IBC SECTION 2306 AND AF&PA SDPWS TABLE 4.3A. FOR EXAMPLE, 7/16" APA-RATED SHEATHING WITH 8d @ 6" & 12" HAS A SEISMIC SHEAR VALUE OF 240 A WIND SHEAR VALUE OF 335#/FT - 40% GREATER THAN THAT OF SEISMIC)

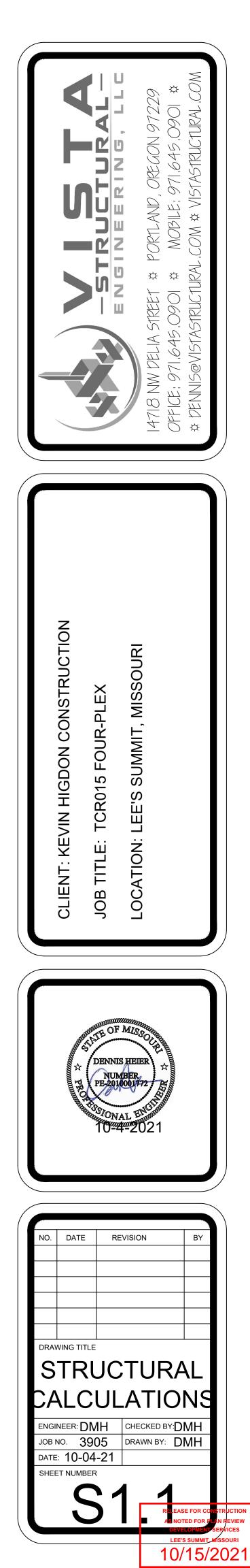
NOTE: SOIL SITE CLASS ASSUMED TO BE CLASS D. IF SITE CONDITIONS ARE DETERMINED TO BE CLASS E OR F, CONSULT ENGINEER BEFORE PROCEEDING WITH CONSTRUCTION

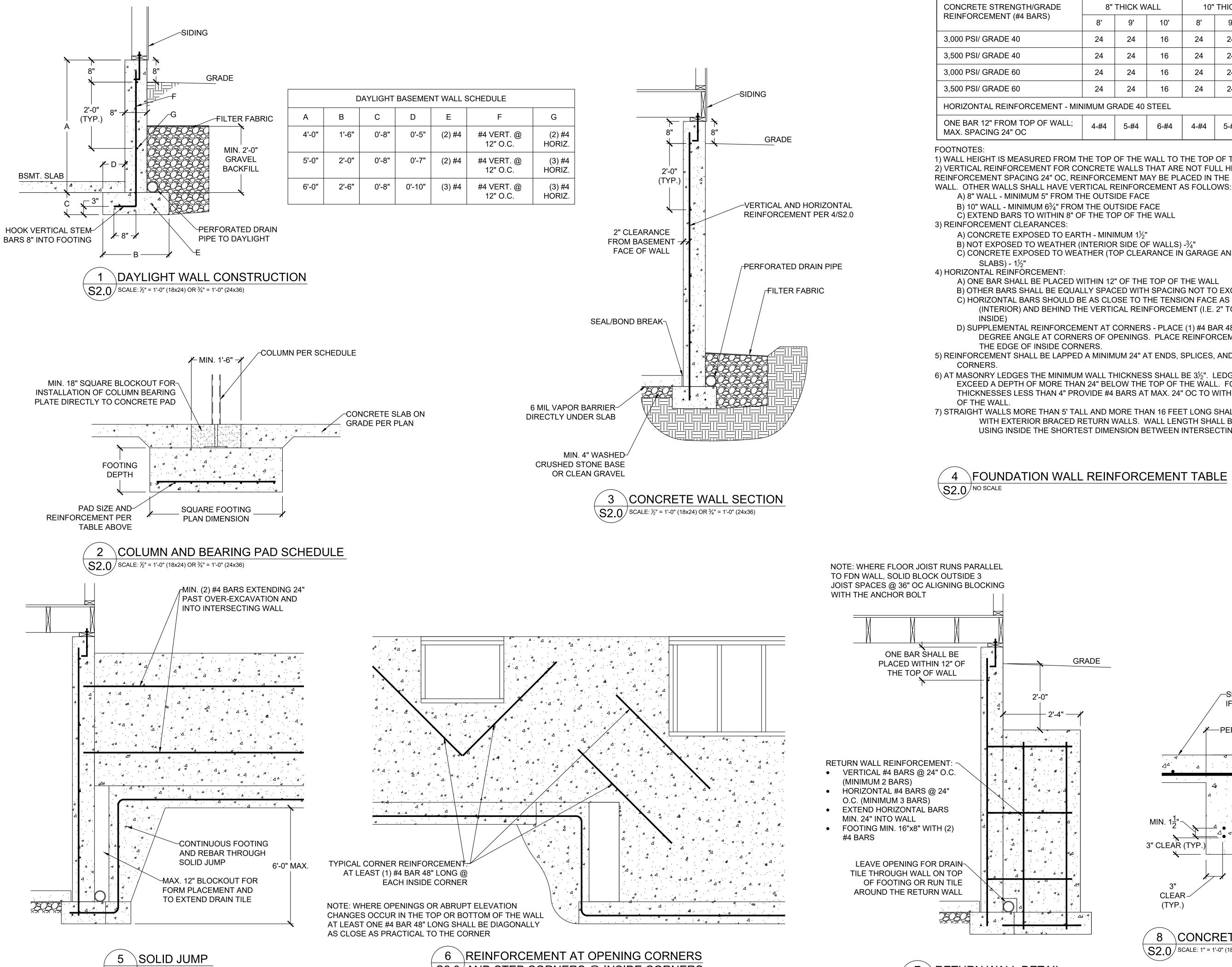
	INPUT
	CALCULATED VALUE
AREA (ft ²)	WEIGHT (lbs.)
6349	63490
6349	63490
6349	63490
WALL UNIT WT. (psf)	WEIGHT (lbs)
10	35466
AREA (ft2)	WEIGHT (lbs)
6349	38094
T ASSUMED)	
LOAD	
6024	
373	CUMULATIVE
8250	14728
1982	9346
11.3	2a (FIG. 28.6-1, ASCE7)
7.7	12.066
2)	

144713
144713
12.0%
1.6
0.128
6.5

		÷
From ASCE7 (Eq. 12.8-1):	V (= 1.2 * S _{DS} * W	/ R) (lbs.)
	3420	
	3420	
Allows	able Shear (#/LF)	Code Reference
Tield		per IBC, Table
1010	155	2306.3(1)
ield		per IBC, Table
	230	2306.3(1)
ield	310	per IBC, Table
		2306.3(1)
D.C.		
ning	220	AF&PA SDPWS Table 4.3A
thing		Table 4.5A
D.C.		AF&PA SDPWS
ning	320	Table 4.3A
thing		
D.C.	110	AF&PA SDPWS
	410	Table 4.3A
eld	60	per IBC, Table
		2306.4.4
cture	325	
	525	
117	WIDTH OF 2ND STORY (FT.)	1
60.33	DEPTH OF 2ND STORY (FT.)	1

2





 $\overline{S2.0}$ SCALE: $\frac{1}{2}$ " = 1'-0" (18x24) OR $\frac{3}{4}$ " = 1'-0" (24x36)

S2.0/AND STEP CORNERS @ INSIDE CORNERS SCALE: ¹/₂" = 1'-0" (18x24) OR ³/₄" = 1'-0" (24x36)

ackslashRETURN WALL DETAIL SCALE: $\frac{1}{2}$ " = 1'-0" (18x24) OR $\frac{3}{4}$ " = 1'-0" (24x36)

VERTICAL REINFORCEMENT SPACING							
CONCRETE STRENGTH/GRADE	8" THICK \		HICK WALL		10" THICK WALL		
REINFORCEMENT (#4 BARS)	8'	9'	10'	8'	9'	10'	
3,000 PSI/ GRADE 40	24	24	16	24	24	18	
3,500 PSI/ GRADE 40	24	24	16	24	24	18	
3,000 PSI/ GRADE 60	24	24	16	24	24	18	
3,500 PSI/ GRADE 60	24	24	16	24	24	18	
HORIZONTAL REINFORCEMENT - MINIMUM GRADE 40 STEEL							
ONE BAR 12" FROM TOP OF WALL;	4-#4	5-#4	6-#4	4-#4	5-#4	6-#4	

1) WALL HEIGHT IS MEASURED FROM THE TOP OF THE WALL TO THE TOP OF THE FLOOR SLAB 2) VERTICAL REINFORCEMENT FOR CONCRETE WALLS THAT ARE NOT FULL HEIGHT, AND FOR REINFORCEMENT SPACING 24" OC, REINFORCEMENT MAY BE PLACED IN THE MIDDLE OF THE

C) CONCRETE EXPOSED TO WEATHER (TOP CLEARANCE IN GARAGE AND DRIVEWAY

A) ONE BAR SHALL BE PLACED WITHIN 12" OF THE TOP OF THE WALL

B) OTHER BARS SHALL BE EQUALLY SPACED WITH SPACING NOT TO EXCEED 24" OC C) HORIZONTAL BARS SHOULD BE AS CLOSE TO THE TENSION FACE AS POSSIBLE (INTERIOR) AND BEHIND THE VERTICAL REINFORCEMENT (I.E. 2" TOWARD THE

D) SUPPLEMENTAL REINFORCEMENT AT CORNERS - PLACE (1) #4 BAR 48" LONG AT 45 DEGREE ANGLE AT CORNERS OF OPENINGS. PLACE REINFORCEMENT WITHIN 6" OF

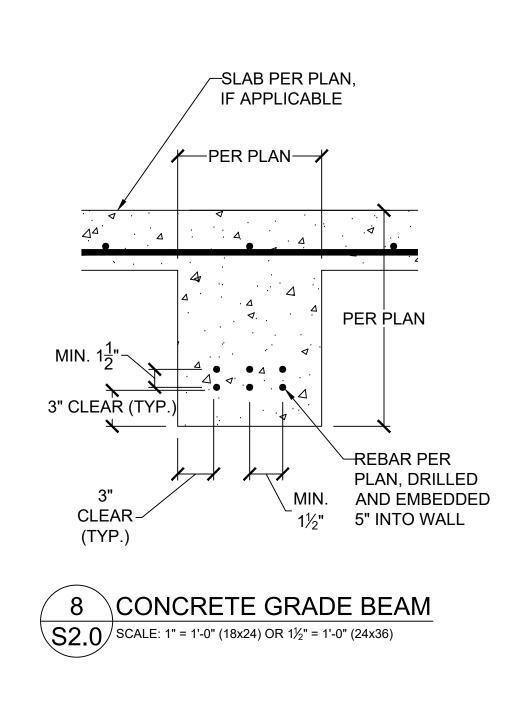
5) REINFORCEMENT SHALL BE LAPPED A MINIMUM 24" AT ENDS, SPLICES, AND AROUND

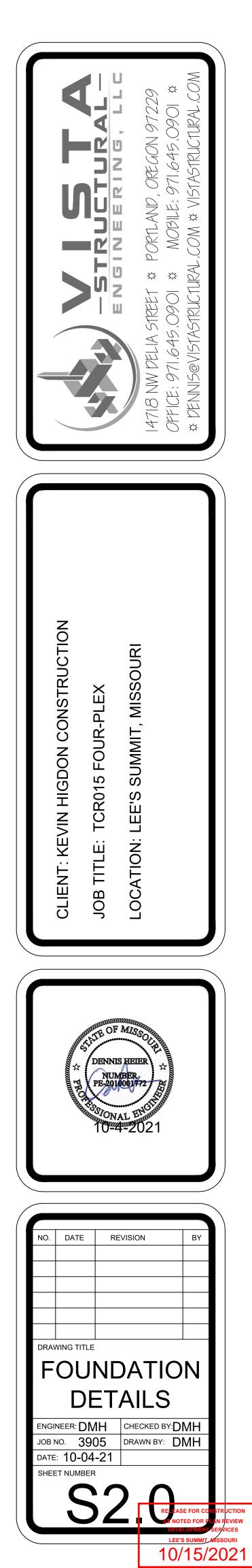
6) AT MASONRY LEDGES THE MINIMUM WALL THICKNESS SHALL BE 3½". LEDGES SHALL NOT EXCEED A DEPTH OF MORE THAN 24" BELOW THE TOP OF THE WALL. FOR WALL THICKNESSES LESS THAN 4" PROVIDE #4 BARS AT MAX. 24" OC TO WITHIN 8" OF THE TOP

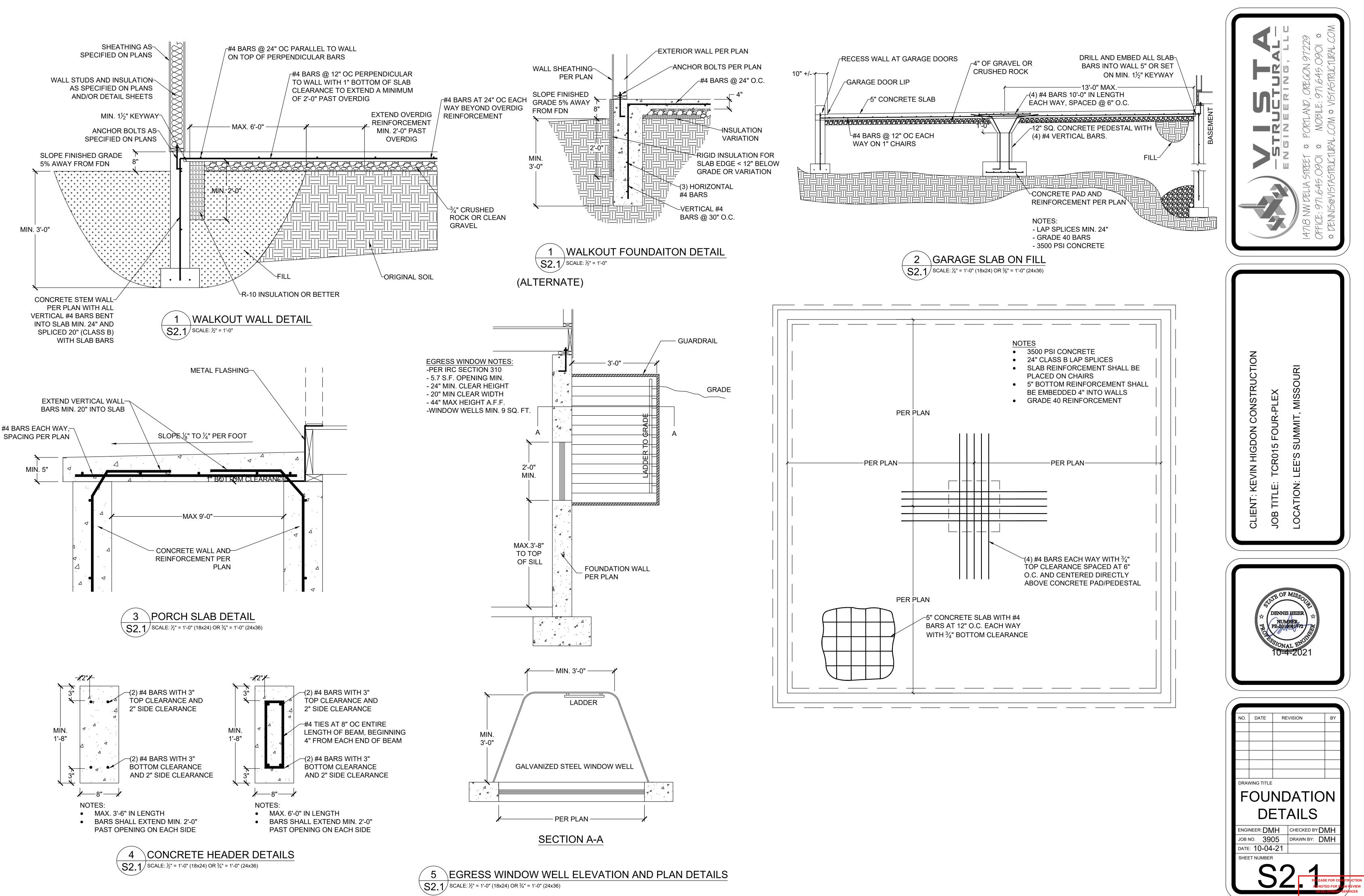
7) STRAIGHT WALLS MORE THAN 5' TALL AND MORE THAN 16 FEET LONG SHALL BE PROVIDED WITH EXTERIOR BRACED RETURN WALLS. WALL LENGTH SHALL BE MEASURED USING INSIDE THE SHORTEST DIMENSION BETWEEN INTERSECTING WALLS

4 \FOUNDATION WALL REINFORCEMENT TABLE

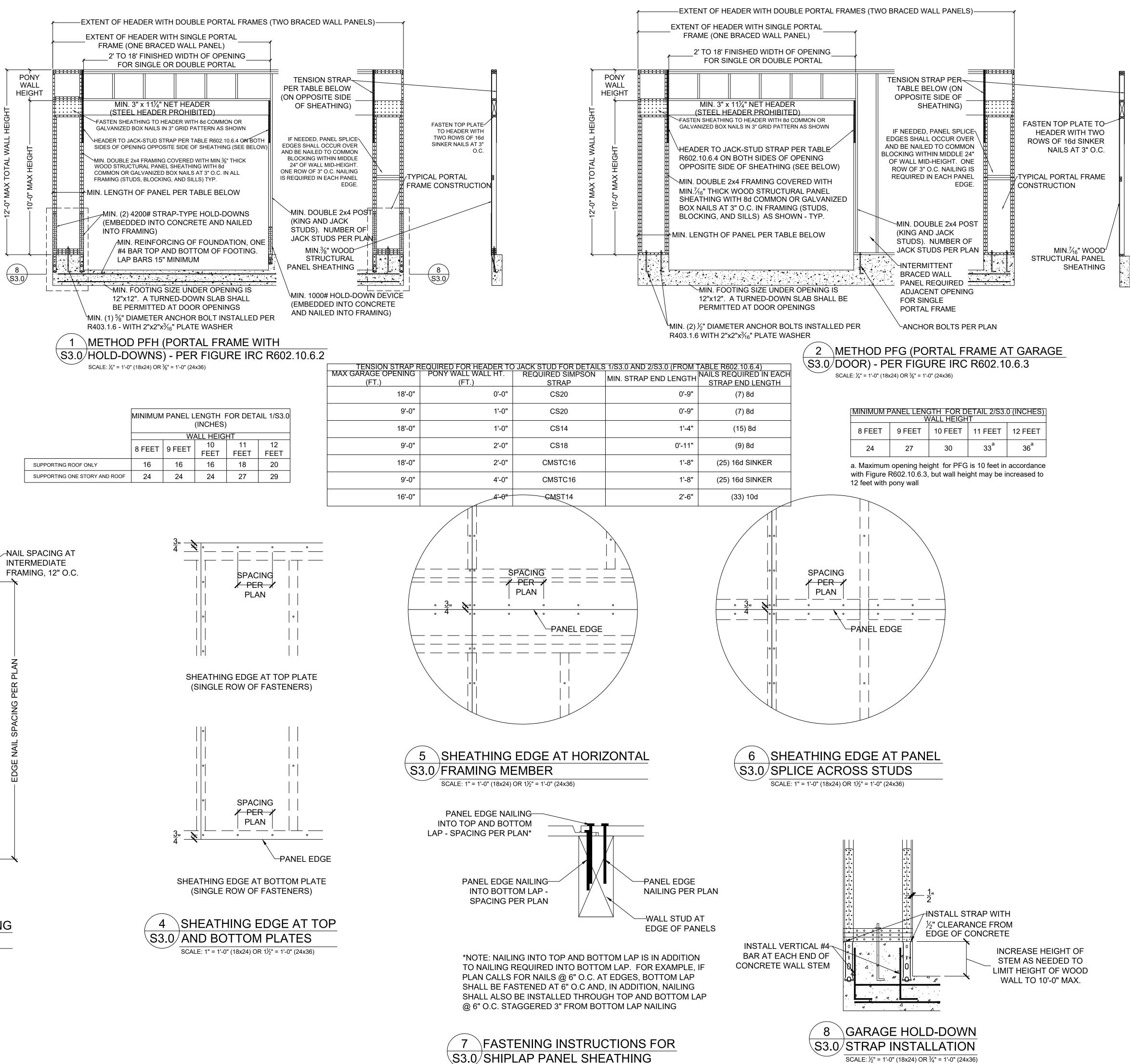
GRADE





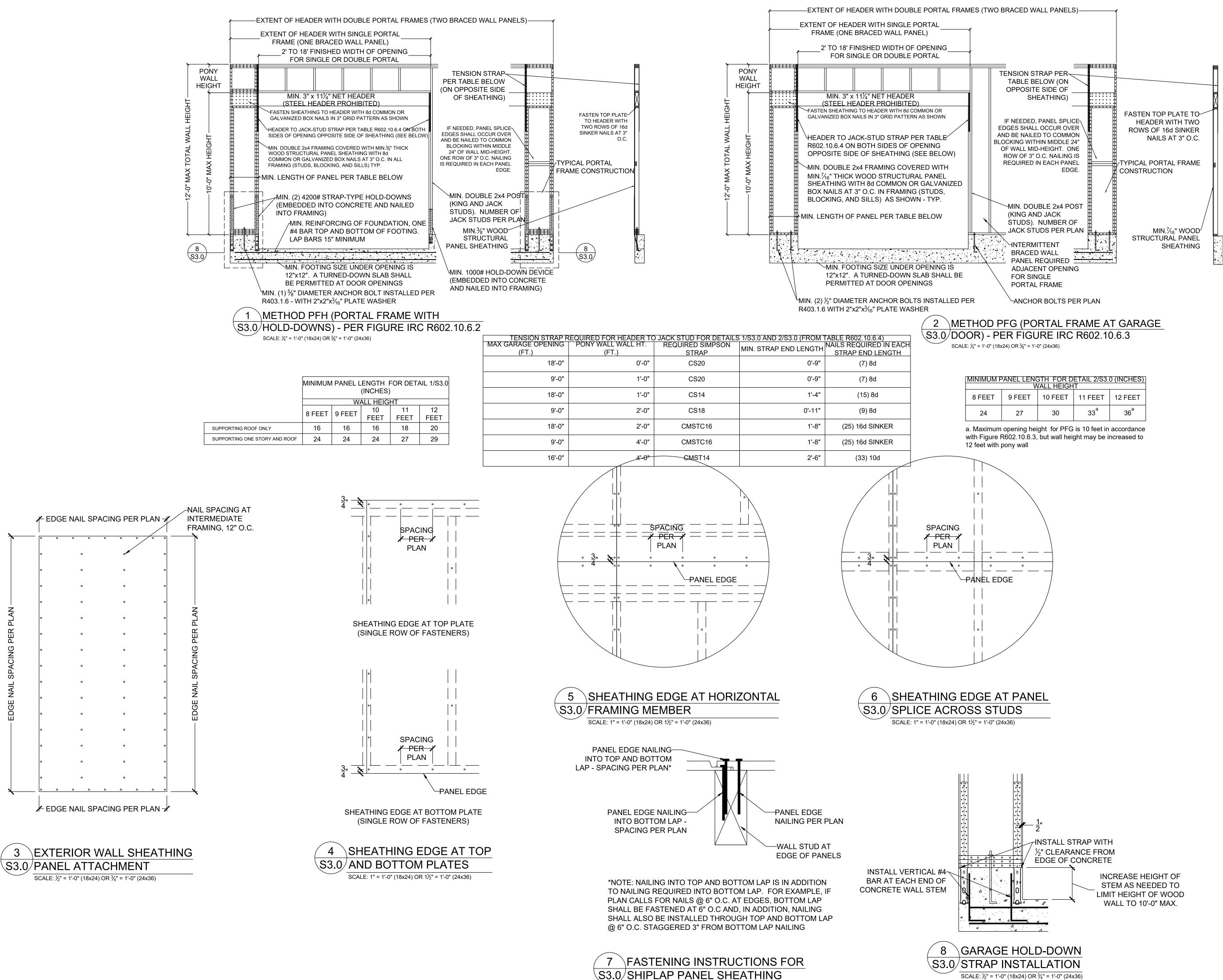


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SCALE: 4" = 1'-0" (18x24) OR 6" = 1'-0" (24x36)

	MINIMUM PANEL LENGTH FOR DETAIL (INCHES)				
	WALL HEIGHT				
	8 FEET	9 FEET	10	11	
			FEET	FEET	F
SUPPORTING ROOF ONLY	16	16	16	18	
SUPPORTING ONE STORY AND ROOF	24	24	24	27	



WALL HEIGHT						
8 FEET 9 FEET		10 FEET	11 FEET	12 FEET		
24	27	30	33 ^a	36 ^a		

¬℃Ŀ **C** Z Þ S Σ \vdash S S ŝ 20 Ξ Ш KEVI LOCATION: ш Ē CLIENT JOB REVISION DATE DRAWING TITLE FRAMING DETAILS ENGINEER: DMH CHECKED BY: DMH JOB NO. 3905 DRAWN BY: DMH DATE: 10-04-21 SHEET NUMBER 10/15/2021

