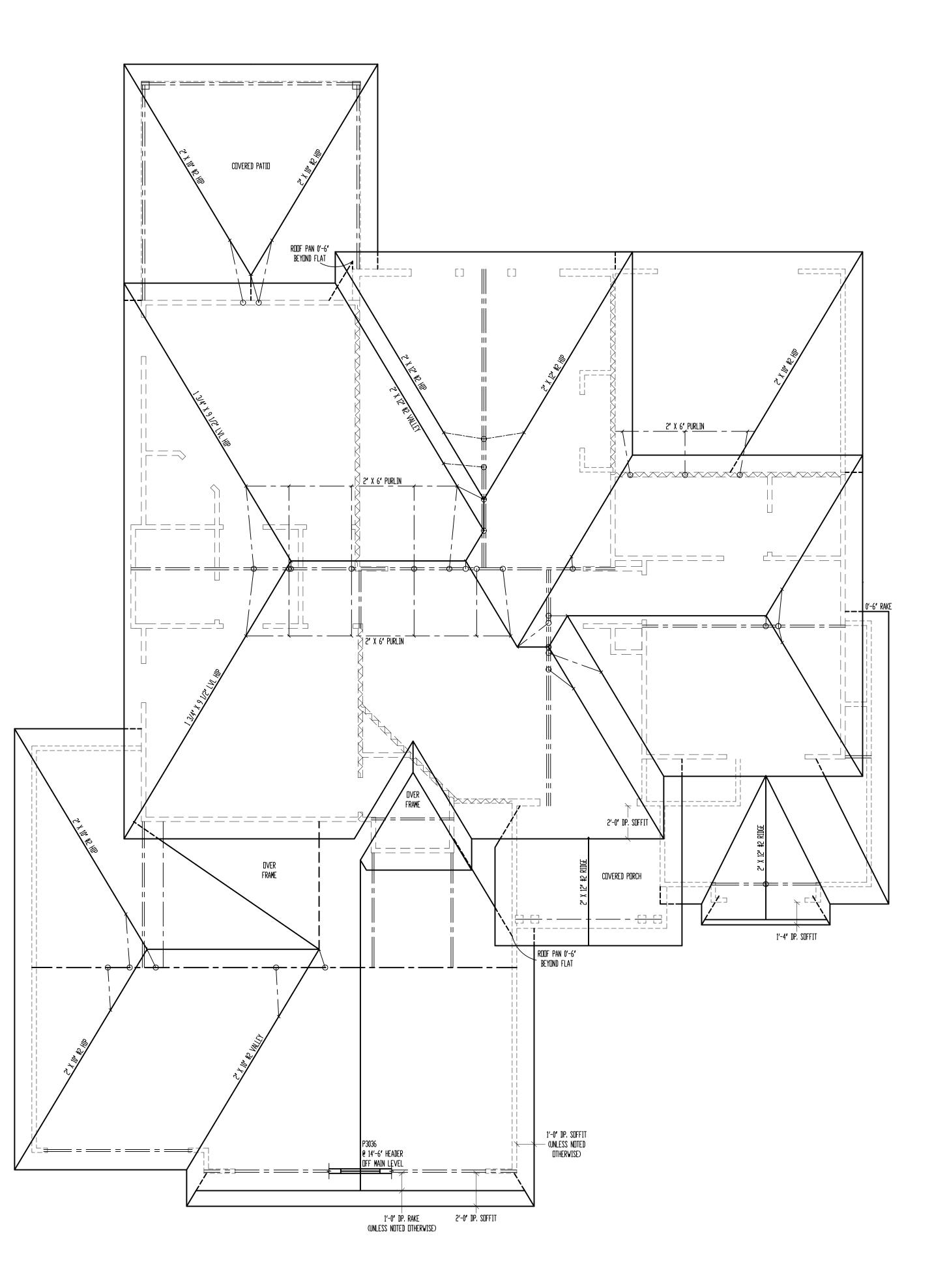


RELEASE FOR CONSTRUCTION AS NOTED ON PLANS REVIEW Development Services LEE'S SUMMIT, MISSOURI



RODF NOTES:

ROOF DESIGNED FOR LIGHT ROOF COVERING 30psf Total LOAD (10psf DL, 20psf LL (SL)) * RAFTERS (HEM-FIR, DDUG-FIR, DR EQUAL): SEE SPAN CHARTS BELDW

 HIGHER PERFORMANCE (RECOMMENDED)

 RAFTERS
 SPACING
 MAX
 HDRIZDNTAL
 CLEARSPAN

 #2-2x6
 @24'
 D.C.
 8'-6'

 #2-2x6
 @16'
 D.C.
 9'-9'

 #2-2x8
 @24'
 D.C.
 11'-3'

 #2-2x8
 @16'
 D.C.
 12'-9'

 #2-2x10
 @24'
 D.C.
 14'-3'

 #2-2x10
 @16'
 D.C.
 16'-3'

 DEFLECTION = L/360
 LIVE
 LDAD, L/240
 TDTAL

* VAULTS TO BE 2x10 DEPTH * RIDGE BDARDS ARE: (UNLESS OTHERWISE NOTED) - #2- 2X8 UP TO 10/12 PITCH - #2- 2X10 DVER 10/12 PITCH * All HIPS & VALLEYS ARE: (UNLESS OTHERWISE NOTED) - #2- 2X8 UP TO 10/12 PITCH - #2- 2X10 DVER 10/12 PITCH * PURLINS ARE 2X6 MIN. - PURLIN STRUTS ARE AT 4'-0" D.C. - PURLIN STRUTS SHALL BE INSTALLED AT NOT LESS THAN A 45 DEGREE ANGLE WITH THE HDRIZONTAL - ALL PURLINS STRUTS SHALL HAVE A MAXIMUM UNBRACED LENGTH DF 8'-0' - Purlins struts shall be constructed in a 'T' Configuration and per the following chart:

purlin strut (2) 2x4 (1) 2x4 & (1) 2x6 (1) 2x6 & (1) 2x8 (2) 2x6 & (1) 2x8 CDNSULT ARCH,/ENGR, >

* RIDGE BRACES ARE SAME AS PURLIN BRACES-SPACING, SIZE, CONFIGURATION, & INSTALLATION (SEE PURLIN BRACE NOTES ABOVE) * HIP & VALLEY BRACES ARE SAME AS PURLIN SIZE, CONFIGURATION, & INSTALLATION (see purlin brace notes above)

* VERTICAL BRACE IF DOT IS UNDER HIP DR VALLEY * SLASH IS TOP END OF BRACE (/), DDT IS BOTTOM OF BRACE (o). * ~~~~~ DENDTES BEARING WALL *----- Denotes roof brace *----- DENDTES PURLIN *----- DENDTES BEARING STRUCTURE



*ALL RAFTERS SHALL BE 2" X 6" #2 @ 16" D.C., UNLESS NOTED OTHERWISE. see detail 7/s3.2 For alternate rafter bearing detail when rafters are required to bear higher than the wall double top plate.

Flashing note: Drip Edge, valleys and flashings to be metal clad.

	CODE MINI	MUM		
	RAFTERS	SPACING	MAX HORIZONTAL CLEARSPAN	
	#2-2x6	024 " D.C.	11′-7 ′	
>>>	#2-2x6	0 16″ D.C.	14′-2 ″) <<<
	#2-2x8	024 " D.C.	14′-8 ″	
	#2-2x8	0 16″ D.C.	17'-11 '	
	#2-2x10	024 " D.C.	17'-10 '	
	#2-2x10	0 16″ D.C.	21'-11 '	
	NDTE: CODI	e minimum all	ows for a rafter deflection	_ DF L/180 TOTAL LOAD

8'-6 '
9'-9 '
11'-3 '
12'-9 '
14'-3 '
16'-3 '
'E LOAD, L/240 TOTAL LOAD

	MAX PURLIN STRUT LENGTH
	8'-0"
	12'-0'
	20'-0 '
	30'-0 '
>	30'-0"

"For God so loved the world, that he gave his only begotten Son, that whosoever believeth in him should not perish, but have everlasting life" (John 3:16). VIEWPOINT RESIDENTIAL DESIGN LLC l at Plat ο 0 ot

Copyright C.D. 2021 we gone into the creation and design of the creation and effort have gone into the creation and design of the designer is not an architect or engineer and construction from undertaken without the assistance of a construction profess Because of the impossibility of any on site consultation and Residential Design, LLC, and Designer assume no responincluding structural failures, due to any deficiencies, omiss blueprints. Also, site conditions may vary from those illustridoes not warrant the suitability of these plans for use on y architect to determine the suitability of these plans for you





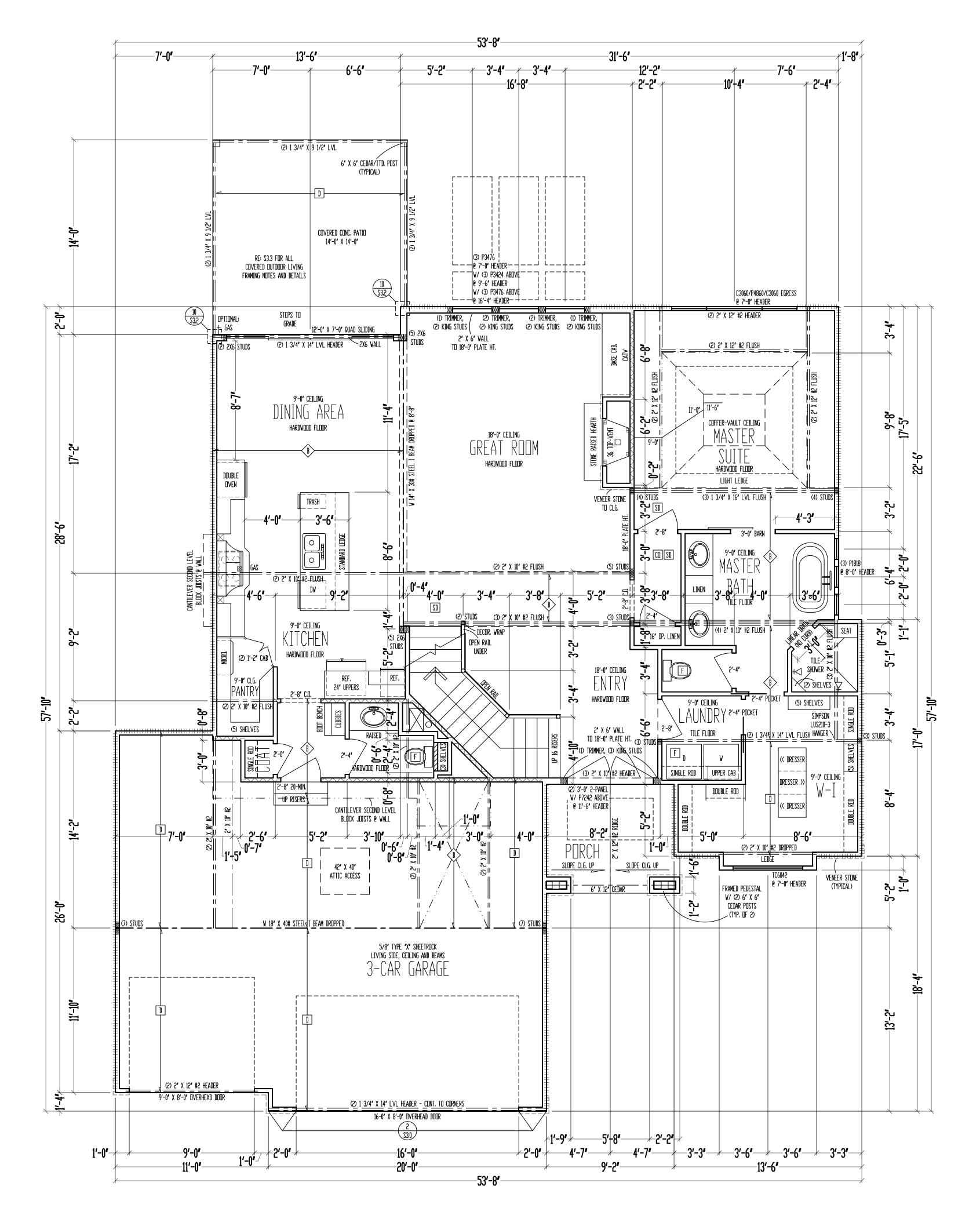


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Sheet Title: ROOF PLAN

Sheet No.: 5 **A-2**_{of 5}





COV. DUT/LIV: 195 SQ. FT. UNFIN. BASEMENT: 1426 SQ. FT.

Calculations on sheet \$1.1.

FOR BOTH SIDES) NOTED OTHERWISE. 5. LOW TIES @ 4'-0" D.C. (TYPICAL) JDIST MATERIAL (NOT REQUIRED WITH I-JOISTS). 8. PROVIDE MULTIPLE STUDS FOR SOLID BEARING BELOW ALL BEAMS. 9. All designated 2" X 6" walls shall have double king studs at door and VINDOV OPENINGS. 10. ALL UNSQUARE WALLS SHALL BE 45°, UNLESS NOTED OTHERWISE. 11. ALL WALLS TO BE FRAMED W/ MIN. STUD GRADE 2' X 4'S @ 16' D.C., UNLESS NOTED OTHERWISE. 12. EXTERIDR WALL BOTTOM PLATES SHALL BE NAILED TO FRAMING BELOW WITH 16d Common Nails @ 8" D.C. 13. LVL'S SHOWN ON PL BEAMS OF THE SAME DE (2) 1 3/4" LVL PLIES (3) 1 3/4" LVL PLIES 14. Contractor shall DEFLECTION LIMITATIONS OPENINGS.

> $\langle A \rangle$

C. Max. (Where Applicable.) Plans May be replaced with df/df)Epth, and the following widths: = 3 1/2" glulam = 5 1/2" glulam . Notify engineer of record before NS More stringent than code minimu	: Construction of Any
JOIST SCHEDULE	
2" X 10" #2 TTD. FLOOR JOIST @ 16" D.C.	
2" X 10" #2 Flddr Jdist @ 16" D.C.	
2" X 10" #2 FLOOR JOIST @ 16" D.C DOUBLE EVERY DTHER	
2" X 6" #2 CEILING JOIST @ 16" D.C.	

6. RUN STUDS THE FULL HEIGHT OF RAISED PLATE WALLS. 7. BLOCK JOISTS ABOVE BEAMS, CANTILEVERS AND LOAD BEARING WALLS WITH

edges & Field, (Min. 8'-0" sections one side of Wall (OR) Min. 4'-0" section 3. //////////// = LOAD bearing interior wall. 4. (2) 2" X 10" #2 Header at all exterior and load bearing Walls, unless

24" MAX FASTENED V/ ND. 6 - 1 1/4" TYPE V DR S DRYVALL SCREWS @ 7" D.C.

Framing Notes 1. Main Level Exterior Walls Shall be sheathed V/ 7/16' D.S.B. A.P.A. Panels V/ 8d Common Nails @ 4' D.C. At edges & @ 12' D.C. In the Field. Smart Panel, OR EQUAL, INSTALLED PER MANUFACTURER'S SPECIFICATIONS.

SECOND LEVEL: 827 SQ. FT. TOTAL: 2500 SQ. FT. GARAGE: 751 SQ. FT.

MAIN LEVEL: 1673 SQ, FT,

SCALE: 1/4'' = 1'-0''

MAIN LEVEL

2" X 10" FLOOR SYSTEM ABOVE

9'-0" CEILING

taken with ise of the ential De Care a design underta Becaus Reside includir bluepri does n archite "For God so loved the world, that he gave his only begotten Son, that whosoever believeth in him should not perish, but have everlasting life" (John 3:16).

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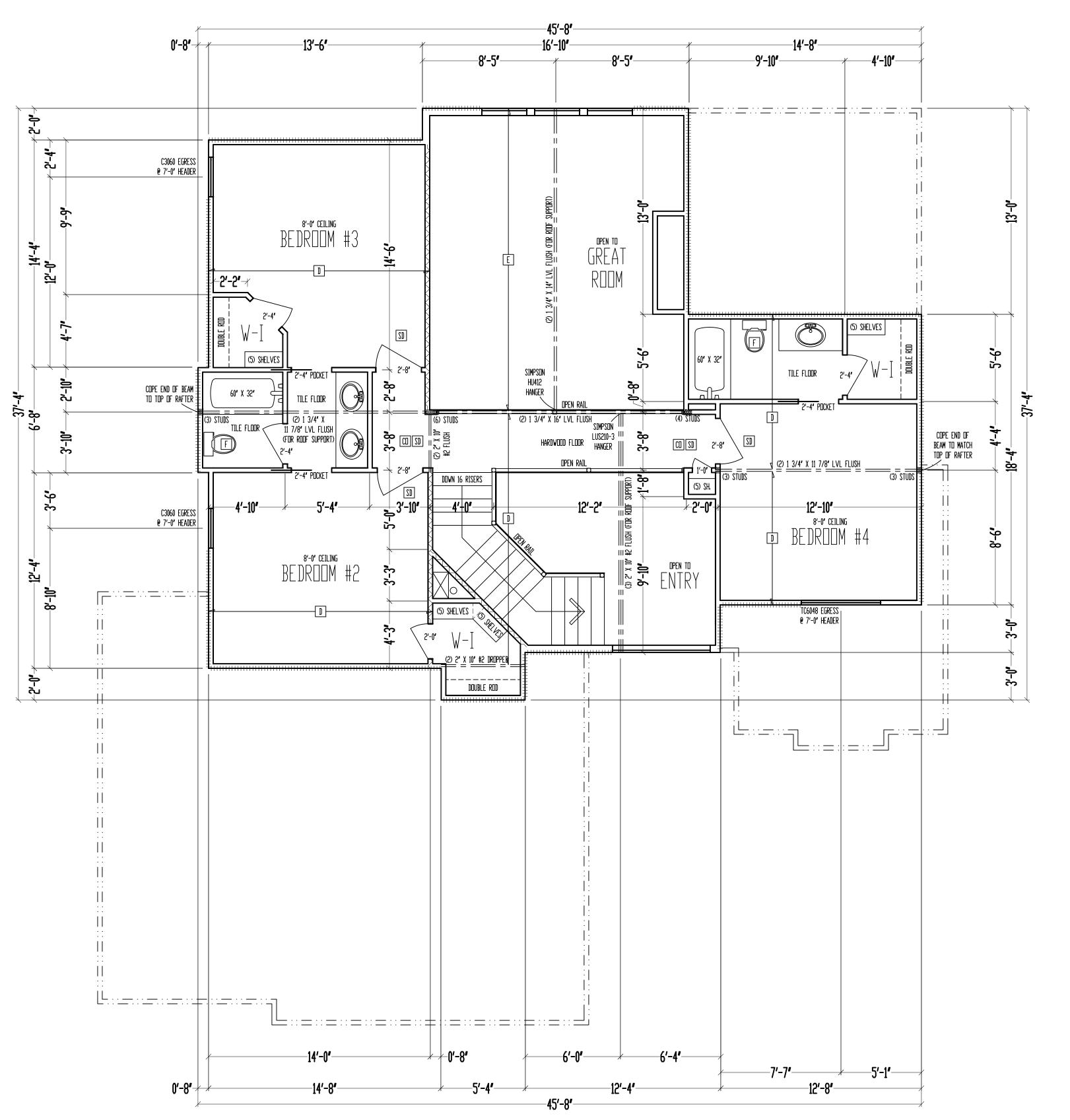
Date: <u>10 - 04 - AD 2021</u> Rev. 1: 10 - 5 - AD 2021 Rev. 2: Rev. 3:

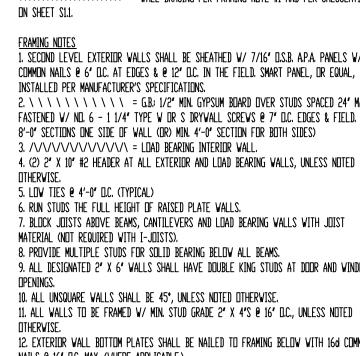
Sheet Title: MAIN LEVEL PLAN

Sheet No.: 2

A-3

RELEASE FOR CONSTRUCTION AS NOTED ON PLANS REVIEW Development Services LEE'S SUMMIT, MISSOURI





(2) 1 3/4' LVL PLIES = 3 1/2' GLULAM (3) 1 3/4' LVL PLIES = 5 1/2' GLULAM



+++++ = Vall bracing per framing note #1 and per calculations

3. /\/\/\/\/\/\/\/\/\/ = LDAD BEARING INTERIDR WALL. 4. (2) 2' X 10' #2 HEADER AT ALL EXTERIDR AND LDAD BEARING WALLS, UNLESS NOTED

9. All designated 2" X 6" walls shall have double king studs at door and window

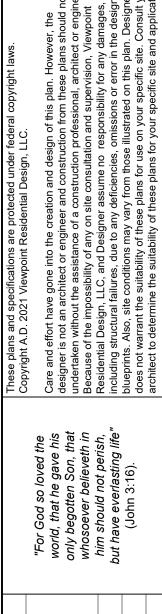
10. All unsquare Walls Shall be 45°, unless noted otherwise. 11. All Walls to be framed W/ Min. Stud grade 2″ X 4′S @ 16′ D.C., unless noted

17. EXTERIOR WALL BOTTOM PLATES SHALL BE NAILED TO FRAMING BELOW WITH 160 COMMON NAILS @ 16' D.C. MAX. (WHERE APPLICABLE.) 13. LVL'S SHOWN ON PLANS MAY BE REPLACED WITH DF/DF GRADE 24F-V4 GLULAM BEAMS OF THE SAME DEPTH, AND THE FOLLOWING WIDTHS:

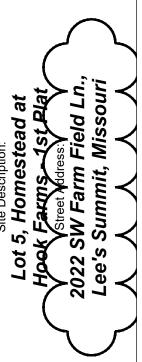
14. Contractor shall notify engineer of record before construction of any deflection limitations more stringent than code minimums above any openings.

JOIST SCHEDULE
 D
 2' X 6' #2 CEILING JDIST

 @ 16' D.C.
 E 2' X 8' #2 CEILING JOIST @ 16' D.C.



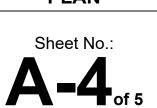
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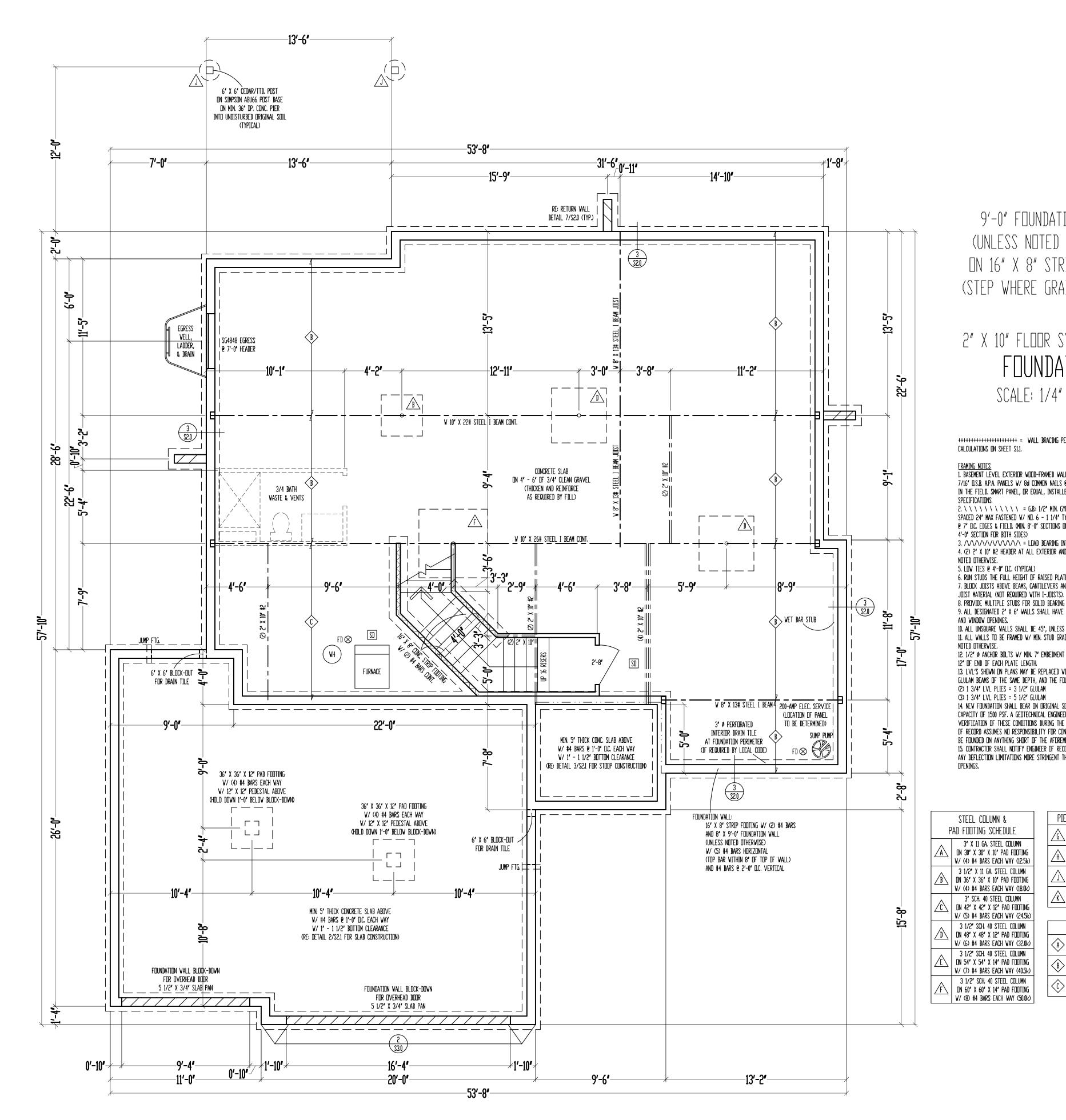
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0-3-20 Date: <u>10 - 04 - AD 2021</u> Rev. 1: <u>10 - 5 - AD 2021</u> Rev. 2: Rev. 3: Sheet Title: SECOND LEVEL PLAN







9'-0" FOUNDATION WALLS (UNLESS NOTED OTHERWISE) ON 16" X 8" STRIP FOOTINGS (STEP WHERE GRADE REQUIRES)

2" X 10" FLOOR SYSTEM ABOVE FOUNDATION SCALE: 1/4'' = 1'-0''

1. BASEMENT LEVEL EXTERIOR WODD-FRAMED WALLS SHALL BE SHEATHED W/ 7/16' D.S.B. A.P.A. PANELS W/ 8d COMMON NAILS @ 6' D.C. AT EDGES & @ 12' D.C. IN THE FIELD, SMART PANEL, OR EQUAL, INSTALLED PER MANUFACTURER'S

3. //////////// = LOAD bearing interior wall. 4. (2) 2" X 10" #2 Header at all exterior and load bearing Walls, unless

6. RUN STUDS THE FULL HEIGHT OF RAISED PLATE WALLS. 7. BLOCK JOISTS ABOVE BEAMS, CANTILEVERS AND LOAD BEARING WALLS WITH

8. PROVIDE MULTIPLE STUDS FOR SOLID BEARING BELOW ALL BEAMS. 9. All designated 2" X 6" walls shall have double king studs at door

10. ALL UNSQUARE WALLS SHALL BE 45°, UNLESS NOTED OTHERWISE. 11. All Walls to be framed W/ Min. Stud grade 2" X 4"S @ 16" D.C., UNLESS

12. 1/2" Ø ANCHOR BOLTS W/ MIN. 7" EMBEDMENT @ 48" D.C. MAX. & WITHIN 6" -13. LVL'S SHOWN ON PLANS MAY BE REPLACED WITH DF/DF GRADE 24F-V4

glulam beams of the same depth, and the following vidths:

14. New Foundation shall bear on original soll with minimum bearing CAPACITY DF 1500 PSF. A GEDTECHNICAL ENGINEER IS RECOMMENDED FOR verification of these conditions during the excavation phase. Engineer OF RECORD ASSUMES NO RESPONSIBILITY FOR CONSTRUCTION NOT VERIFIED TO be founded on anything short of the aforementioned requirements. 15. CONTRACTOR SHALL NOTIFY ENGINEER OF RECORD BEFORE CONSTRUCTION OF ANY DEFLECTION LIMITATIONS MORE STRINGENT THAN CODE MINIMUMS ABOVE ANY

> PIER FOOTING SCHEDULE 12" ø pier ftg. ∕₀∖∣ 16" ø pier ftg. /#\|

	\triangle	18 " ø pier ftg.	
	Ŕ	24' ø pier ftg.	
1			
		JOIST SCHEDULE	
	$\langle A \rangle$	2" X 10" #2 TTD. FLODR JOI @ 16" D.C.	ST
	$\langle \mathbf{B} \rangle$	2" X 10" #2 Floor joist € 16" 0.C.	
	$\langle \hat{z} \rangle$	2" X 10" #2 Floor Joist @ 16" D.C Double every d	THER

Care and effort I designer is not a undertaken with Because of the i Residential Desi including structu blueprints. Also, does not warran architect to dete "For God so loved the world, that he gave his only begotten Son, that whosoever believeth in him should not perish, but have everlasting life" (John 3:16). VIEWPOID RESIDENTIAL DESIGN l at Pla 5

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<u>U-3-20</u>. Date: <u>10 - 04 - AD 2021</u>

Rev. 1: 10 - 5 - AD 2021

Sheet Title: FOUNDATION PLAN

Sheet No.:

A-5_{OF 5}

Rev. 2: Rev. 3:

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DESCRIPTION OF BUILDING ELEN	IENTS	NUMBER AND TYP	PE OF FASTENER	SPAC	CING AND LOCATION
	I	RO	OF ¹		
BLOCKING BETWEEN JOISTS OR RAFT PLATE, TOE NAIL	ERS TO TOP	4-8d (2½"	x 0.113")		TOENAIL
CEILING JOISTS TO PLATE, TOE	NAIL	4-8d (2½"	x 0.113")	PE	R JOIST, TOENAIL
CEILING JOISTS NOT ATTACHED TO RAFTER, LAPS OVER PARTITIONS, F		4-10d (3"	x 0.128")		FACE NAIL
CEILING JOIST TO PARALLEL RAFTER (HEEL JOINT)	TBLE R	802.5.2		FACE NAIL
COLLAR TIE TO RAFTER, FACE NAIL OR RIDGE STRAP TO RAFTER	1 ¹ / ₄ " x 20 GA.	4-10d (3"	x 0.128")	FACE	NAIL, EACH RAFTER
RAFTER OR ROOF TRUSS TO P	_ATE	3-16d BOX NAILS (3½" x 0. NAILS (3"	135") OR 3-10d COMMON x 0.148")		NONE SIDE AND 1 TOE NAIL ON E OF EACH RAFTER OR TRUSS
ROOF RAFTERS TO RIDGE, VALLEY RAFTERS OR ROOF RAFTER TO MINIMI BEAM		4-16d (3 <u>1</u> " x 0.135") - TOI 0.135") - E		Т	DENAIL, END NAIL
	·	WA	LL		
STUD TO STUD (NOT AT BRACED WAL	L PANELS)	10d (3" x	0.128")	16	5" O.C. FACE NAIL
STUD TO STUD AND ABUTTING ST NTERSECTING WALL CORNERS (AT BF PANELS)		16d (3½")	x 0.135")	12	2" O.C. FACE NAIL
BUILT-UP HEADER, TWO PIECES WITH	½" SPACER	16d (3½" :	x 0.135")	12" O.C.	EACH EDGE FACE NAIL
CONTINUOUS HEADER TO ST	UD	4-8d (2½"	x 0.131")		TOENAIL
TOP PLATE TO TOP PLATE		10d (3" x	0.128")	12	2" O.C. FACE NAIL
DOUBLE TOP PLATE SPLICE	<u>=</u>	8-16d COMMON	N (3 ¹ / ₂ " x 0.162")		CH SIDE OF END JOINT (MIN. 24" GTH EACH SIDE OF END JOINT)
BOTTOM PLATE TO JOIST, RIM JOIST, E OR BLOCKING (NOT AT BRACED WAL		16d COMMON	(3 ½" x 0.162")	16	5" O.C. FACE NAIL
BOTTOM PLATE TO JOIST, RIM JOIST, E OR BLOCKING (AT BRACED WALL		3-16d BOX (3	3 ¹ / ₂ " x 0.135")	3 EAC	H 16" O.C. FACE NAIL
TOP OR SOLE PLATE TO STUD, EN	ID NAIL	4-8d BOX (2 ½" x 0.113") - T 0.135") - E		TOENAI	L, END NAIL (SEE LEFT)
TOP PLATES, LAPS AT CORNERS INTERSECTIONS	S AND	3-10d BOX (3" x 0.128")		FACE NAIL
1" BRACE TO EACH STUD AND P	LATE	3-8d BOX (2	¹ / ₂ " x 0.113")		FACE NAIL
1"x6" SHEATHING TO EACH BEA	RING	3-8d BOX (2	¹ / ₂ " x 0.113")		FACE NAIL
1"x8" SHEATHING TO EACH BEA	RING	3-8d BOX (2 ½" x 0.113") - F 1"x8" - 4-8d BOX			FACE NAIL
		FLO	OR		
JOIST TO SILL, TOP PLATE, OR G	IRDER	4-8d BOX (2	¹ / ₂ " x 0.113")		TOE NAIL
RIM JOIST, BAND JOIST, OR BLOCKING TOP PLATE (ROOF APPLICATIONS		8d BOX (2 2	<u>-</u> " x 0.113")		4" O.C. TOE NAIL
1" x 6" SUBFLOOR OR LESS TO EAC	,	3-8d BOX (2	¹ / ₂ " x 0.113")		FACE NAIL
2" SUBFLOOR TO JOIST OR GIR		3-16d BOX (3	3 ¹ / ₂ " x 0.135")	BLI	ND AND FACE NAIL
2" PLANKS (PLAN & BEAM - FLOOR A	ND ROOF)	3-16d BOX (3	3 ¹ / ₂ " x 0.135")	AT EAC	H BEARING, FACE NAIL
BAND OR RIM JOIST TO JOIS	т	3-16d COMMON	√ (3 ½" x 0.162")		END NAIL
BUILT-UP GIRDERS AND BEAMS, 2-INC LAYERS	H LUMBER	10d BOX (3	" x 0.128")		E NAIL AT TOP AND BOTTOM RED ON OPPOSITE SIDES
LEDGER STRIP SUPPORTING JOISTS C	R RAFTERS	4-16d BOX (3	3 ¹ / ₂ " x 0.135")		IST OR RAFTER, FACE NAIL
BRIDGING OR BLOCKING TO JO	DIST	2-10d BOX (3" x 0.128")	EA	CH END, TOENAIL
ESCRIPTION OF BUILDING MATERIALS WOOD STRUCTURAL PANELS, SUB	•	FASTNER SCHEDULE FOR PTION OF FASTENER	EDGE SPACING (INC	· · · ·	ERMEDIATE SUPPORTS (INCHES) ALL SHEATHING TO FRAMING ¹
3⁄8" - 1⁄2"		MON (2" x 0.113") NAIL WALL) 8d COMMON NAIL (ROOF)	6		12
¹⁹ ⁄ ₃₂ " - 1"	8d COMN	10N NAIL (2½" x 0.131")	6		12

	(2/2 X 0.131) DEFORMED NAIL		
	OTHER WALL		
¹ / ₂ " STRUCTURAL CELLULOSIC FIBERBOARD SHEATHING	1 $\frac{1}{2}$ " GALVANIZED ROOFING NAIL, $\frac{7}{16}$ " HEAD DIAMETER, OR 1 $\frac{1}{4}$ " LONG 16 GA. STAPLE WITH $\frac{7}{16}$ " OR 1" CROWN	3	6
²⁵ ³² STRUCTURAL CELLULOSIC FIBERBOARD SHEATHING	1 $\frac{3}{4}$ " GALVANIZED ROOFING NAIL, $\frac{7}{16}$ " HEAD DIAMETER, OR 1 $\frac{1}{2}$ " LONG 16 GA. STAPLE WITH $\frac{7}{16}$ " OR 1" CROWN	3	6
½" GYPSUM SHEATHING	1½" GALVANIZED ROOFING NAIL; STAPLE GALVANIZED, 1½" LONG; 1¼" SCREWS, TYPE W OR S	7	1 7
5∕8" GYPSUM SHEATHING	1¾" GALVANIZED ROOFING NAIL; STAPLE GALVANIZED, 1%" LONG; 1%" SCREWS, TYPE W OR S	7	7

WOOD STRUCTURAL PANELS, COMBINATION SUBFLOOR UNDERLAYMENT TO FRAMING

¾" AND LESS	6d DEFORMED (2" x 0.120") NAIL OR 8d COMMON (2½" x 0.131") NAIL	6	12
7⁄8" - 1"	8d COMMON (2½" x 0.131") NAIL OR 8d DEFORMED (2½" x 0.120") NAIL	6	12
11⁄8" - 11⁄4"	10d COMMON (3" x 0.148") NAIL OR 8d DEFORMED (2½" x 0.120") NAIL	6	12

1. IF INFORMATION LISTED ON PLAN SHEETS CONTRADICTS INFORMATION IN THIS TABLE, INFORMATION ON PLANS TAKES PRECEDENCE OVER INFORMATION

FOUNDATION NOTES

2.

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CONCRETE SHALL BE AIR-ENTRAINED BETWEEN 5%-7% WITH A MINIMUM 28-DAY COMPRESSIVE STRENGTH OF 2500 PSI FOR BASEMENT AND INTERIOR FLOOR SLABS-ON-GRADE, 3000 PSI FOR FOUNDATION WALLS, AND 3500 PSI FOR PORCHES AND GARAGE FLOOR SLABS

THE FOUNDATION DESIGN SHALL COMPLY WITH THE ENFORCING JURISDICTION'S RESIDENTIAL FOUNDATION STANDARDS

PROVIDE A MINIMUM 4"-DIAMETER PERFORATED DRAIN PIPE ALONG PERIMETER OF USABLE SPACE AT FOOTING LEVEL OR OTHER EQUIVALENT MATERIALS PER IRC SECTION R405.1. THE PIPE SHALL BE COVERED WITH A MINIMUM OF 6" OF GRAVEL OR CRUSHED ROCK. THE DRAIN SHALL DAYLIGHT BELOW FOOTING LEVEL OR TERMINATE IN A MINIMUM 20 GALLON SUMP PIT.

FOUNDATION SHALL BE DESIGNED FOR A BEARING CAPACITY OF 1500 PSF AND FOUNDED ON COMPETENT ORIGINAL SOIL AS DETERMINED AND CONFIRMED BY A LICENSED GEOTECHNICAL ENGINEER OR ENGINEERING GEOLOGIST. ENGINEER OF RECORD ASSUMES NO RESPONSIBILITY FOR CONSTRUCTION NOT VERIFIED TO BE FOUNDED ON ANY SOIL WITH THE AFOREMENTIONED MINIMUM PROPERTIES.

FOOTINGS SHALL BE A MINIMUM OF 16" WIDE x 8" DEEP AND SHALL HAVE A MINIMUM OF (2) CONTINUOUS GRADE 40 #4 BARS WITH 3" BOTTOM CLERANCE. BOTTOM OF FOOTING SHALL BE LOCATED A MINIMUM OF 3'-0" BELOW GRADE FOR FROST PROTECTION.

CONCRETE PADS SUP0PORTING COLUMN LOADS SHALL BE NO SMALLER THAN 2'-0" x 2'-0" x 1'-0" DEEP WITH A MINIMUM OF (4) GRADE 40 #4 BARS EACH WAY WITH 3" BOTTOM CLEARANCE

FOUNDATION WALLS SHALL BE A MINIMUM OF 8" NOMINAL WIDTH AND SHALL HAVE HOIZONTAL GRADE 40 #4 BARS AT 2'-0" O.C. MAX. WITH VERTICAL #4 BARS AS REQUIRED ON FOUNDATION CROSS SECTION ON SHEET S2.0 REINFORCEMENT SHALL LAP A MINIMUM OF 2'-0" (CLASS B SPLICE)

- INTERIOR BEARING WALLS AND COLUMNS SHALL BE ISOLATED FROM THE BASEMENT FLOOR SLAB
- BASEMENT FLOOR SLAB SHALL BE A MINIMUM OF 4" THICK ON A MINIMUM BASE COURSE OF 4" TO 6" OF SAND,

GRAVEL OR CRUSHED ROCK. BETWEEN THE BASE COURSE AND FLOOR SLAB SHALL BE PLACED A 6-MIL POLY VAPOR RETARDER WITH MINIMUM OVERLAP OF 6" AT DISCONTINUITIES 11. IF A FLOOR IS TO BE SUPPORTED BY A MINIMUM OF 2'-0" OF GRANULAR FILL OR 8" OF EARTH, BASEMENT SLAB

SHALL BE DESIGNED BY A LICENSED ENGINEER SILL PLATES SHALL BE ANCHORED TO THE FOUNDATION WALL WITH ½" Ø ANCHOR BOLTS EMBEDDED A MINIMUM OF 7" INTO CENTER OF WALL STEM AND SHALL BE INSTALLED AT A MAXIMUM OF 6'-0" O.C. (OR AS NOTED ON PLANS) AND SHALL BE INSTALLED WITHIN 6" TO 12" OF EACH END OF EACH SILL PLATE LENGTH, PER IRC SECTION R403.1.6 FOUNDATION WINDOW WELLS SHALL BE PROVIDED WITH MINIMUM DIMENSIONS AS SHOWN IN DETAIL ON SHEET

14. THE GARAGE FLOOR SHALL SLOPE TOWARD THE VEHICLE DOORS OR TO A TRENCH OR UNTRAPPED DRAIN THAT DISCHARGES TO THE EXTERIOR, ABOVE GRADE

FRAMING NOTES

ALL DIMENSIONAL LUMBER SHALL BE DOUGLAS-FIR-LARCH GRADE #2, UNLESS NOTED OTHERWISE ON PLANS ALL INTERIOR LOAD-BEARING AND EXTERIOR WALL HEADERS SHALL BE (2) #2 - 2x10's, UNLESS NOTED OTHERWISE ON PLANS

BLOCK OVER BEAMS AND AT CANTILEVERS AND DOOR JAMBS

INTERIOR NON-BEARING WALLS RESTING ON BASEMENT SLAB SHALL BE ISOLATED FROM ABOVE FRAMING BY A MINIMUM OF 3/5" ALL HEADERS/BEAMS SHALL BEAR ON A MINIMUM OF (2) 2x4 POSTS (KING AND JACK STUDS), UNLESS NOTED

OTHERWISE

20. WHERE JOISTS SPAN PARALLEL TO FOUNDATION, BLOCKING SHALL BE PROVIDED IN THE TWO SPACES MOST ADJACENT TO THE FOUNDATION WALL AT 4'-0" O.C. FOR THE PURPOSE OF TRANSFERRING LATERAL FOUNDATION WALL LOAD TO THE FLOOR DIAPHRAGM. FASTEN JOISTS AND BLOCKING TO SILL PLATE WITH (4) 10d NAILS. IF MECHANICAL DUCTWORK IS INSTALLED IN ONE OF THESE FIRST TWO BAYS, FASTEN 2x4's FLAT AT 4'-0" O.C. BETWEEN JOIST(S) AND/OR SILL AND PROVIDE BLOCKING AS PRESCRIBED ABOVE IN THE NEXT TWO JOIST BAYS. SECURE 2x4's TO JOIST(S)/SILL PLATE WITH (4) 10d NAILS. ALL WOOD MATERIAL SUPPORTED ON CONCRETE OR MASONRY SHALL BE TREATED OR OF DECAY-RESISTANT

MATERIAL JOISTS UNDER BEARING PARTITIONS ON PLANS HAVE BEEN SIZED TO SUPPORT THE DESIGN LOAD.

23. JOISTS FRAMING INTO THE FACE OF A STEEL OR WOOD BEAM SHALL BE SUPPORTED WITH APPROPRIATE COLD-FORMED STEEL JOIST HANGERS

JOISTS FRAMED ON TOP OF STRUCTURAL MEMBER SHALL BE SUPPORTED AT EN DS BY FULL-DEPTH SOLID BLOCKING MIN. 1/4" IN THICKNESS OR BY FASTENING RIM TO JOISTS PER FASTENING TABLE TO LEFT ALL WALL COVERINGS SHALL COMPLY WITH IRC SECTION R702.3

ALL RAFTERS AND COLLAR TIES SHALL COMPLY WITH IRC SECTION R802.3.

ALL RAFTERS SHALL HAVE 2x4 COLLAR TIES @ 4'-0" O.C. IN UPPER $\frac{1}{3}$ OF VERTICAL DISTANCE BETWEEN CEILING AND ROOF

BLOCKING BETWEEN JOISTS UNDER A LOAD-BEARING WALL IS NOT REQUIRED PER IRC SECTION 501.3, BOTTOM OF ALL FLOOR ASSEMBLIES ABOVE UNFINISHED AREAS SHALL BE PROVIDED WITH A $\frac{1}{2}$ " GYPSUM BOARD MEMBRANE OR RESIDENTIAL FIRE SPRINKLER SYSTEM WHEN FLOOR SYSTEM IS CONSTRUCTED OF OTHER THAN DIMENSION LUMBER OR STRUCTURAL COMPOSITE LUMBER EQUAL TO OR GREATER THAN 2x10 NOMINAL DIMENSION(WHERE REQUIRED BY ENFORCING JURISDICTION)

30. ENGINEERED LVL's SHALL HAVE MINIMUM PROPERTIES OF Fb = 2600 psi, E=1900 ksi, AND Fv=285 psi

ENGINEERED PARALLAMS SHALL HAVE MINIMUM PROPERTIES OF Fb = 2600 psi, E = 2000 ksi, AND Fv = 290 psi COLUMN CONNECTION TO STEEL BEAMS SHALL BE WITH A CLIP POST CAP WITH ALL FOUR TAB EARS BENT AROUND THE BOTTOM FLANGE OF THE BEAM. FOR A BEARING PLATE, FOUR HOLES SHALL BE DRILLED IN THE BOTTOM FLANGE OF THE STEEL BEAM TO MATCH THE HOLE PATTERN OF THE PLATE. ¹/₂" x 2" BOLTS SHALL THEN BE INSTALLED WITH A FLAT WASHER, LOCK WASHER, AND A NUT IN EACH OF THE HOLES. THE POST CAP MAY BE WELDED TO THE STEEL BEAM IN ACCORDANCE WITH AWS D1.1-92 AS AN ALTERNATIVE, AND WOULD NEED TO BE INSPECTED BY AN AWS-CERTIFIED INSPECTOR.

WHEN MECHANICAL EQUIPMENT IS LOCATED IN AN ENCLOSED ROOM, THERE SHALL BE (2) 14"x12" VENTS LOCATED IN A WALL COMMON WITH ADDITIONAL LIVING AREA. ONE VENT SHALL BE LOCATED SUCH THAT THE BOTTOM OF THE VENT BEGINS 12" FROM THE FLOOR AND THE OTHER VENT SHALL BE LOCATED SUCH THAT THE TOP OF THE VENT BEGINS 12" FROM THE CEILING.

34. ALL ROOF SHEATHING SHALL BE ⁷/₁₆" OSB WITH 8d COMMON NAILS @ 6" O.C. AT PANEL EDGES AND @ 12" O.C. IN FIELD

GLAZING NOTES

35. GLAZING IN HAZARDOUS LOCATIONS AS IDENTIFIED IN IRC SECTION R308.4 SHALL BE OF APPROVED SAFETY GLAZING MATERIALS. GLASS IN STORM DOORS, INDIVIDUAL FIXED OR OPENABLE PANELS ADJACENT TO A DOOR WHERE THE NEAREST VERTICAL EDGE IS WITHIN A 2'-0" ARC OF THE DOOR IN A CLOSED POSITION AND FOR WHICH THE BOTTOM EDGE IS WITHIN 5'-0" OF THE FLOOR, WALLS ENCLOSING STAIRWAYS AND LANDINGS WHERE THE GLAZING IS WITHIN 5'-0" OF THE TOP OR BOTTOM OF THE STAIR, ENCLOSURES FOR SPAS, TUBS, SHOWERS, AND WHIRLPOOLS, GLAZING IN FIXED OR OPENABLE PANELS EXCEEDING NINE SQUARE FEET AND FOR WHICH THE BOTTOM EDGE IS LESS THAN 1'-6" ABOVE THE FLOOR OR WALKING SURFACE WITHIN 3'-0" 36. ALL OPERABLE WINDOWS SHALL HAVE FALL PROTECTION PER IRC SECTION R612.2

ATTIC VENTILATION

ENCLOSED ATTICS SHALL HAVE CROSS VENTILATION FOR EACH SEPARATE SPACE BY VENTILATING OPENINGS PROTECTED AGAINST THE ENTRANCE OF RAIN OR SNOW. VENTILATING OPENINGS SHALL BE PROVIDED WITH CORROSION-RESISTANT WIRE MESH, WITH $\frac{1}{6}$ " TO $\frac{1}{4}$ " OPENINGS. THE TOTAL FREE VENTILATING AREA SHALL NOT BE LESS THAN $\frac{1}{150}$ OF THE AREA OF SPACE VENTILATED, EXCEPT WHERE THE VENTILATORS ARE LOCATED IN THE UPPER PORTION OF THE SPACE TO BE VENTILATED - THE REQUIRED AREA MAY BE REDUCED TO 1/300.

EMERGENCY EGRESS

PROVIDE A MINIMUM OF ONE WINDOW FOR EACH BEDROOM THAT HAS A MINIMUM OPENABLE AREA OF 5.7 SQUARE FEET WITH A MINIMUM OPENABLE HEIGHT OF 2'-0" AND A MINIMUM WIDTH OF 1'-9". IN ADDITION, THE OPENABLE PORTION OF EGRESS WINDOWS SHALL NOT EXCEED 3'-8" ABOVE THE ADJOINING FLOOR OR PERMANENT STEP. PROVIDE SMOKE ALARMS IN EACH SLEEPING ROOM, OUTSIDE OF EACH SLEEPING AREA AND ON EACH FLOOR, INCLUDING BASEMENT (IF APPLICABLE). ALARMS SHALL BE HARDWIRED TOGETHER SO THAT THE ACTIVATION OF ONE SMOKE ALARM WILL ACTIVATE ALL SMOKE ALARMS IN THE DWELLING. PROVIDE CARBON MONOXIDE DETECTORS OUTSIDE EACH SLEEPING AREA.

MASONRY VENEER

40. MASONRY VENEER SHALL BE ANCHORED TO THE SUPPORTING WALL STUDS WITH CORROSION-RESISTANT METAL TIES EMBEDDED IN MORTAR OR GROUT AND EXTENDING INTO THE VENEER A MINIMUM OF 1¹/₂", WITH NOT LESS THAN $\frac{5}{8}$ " MORTAR OR GROUT COVER TO OUTSIDE FACE.

- VENEER TIES, IF STRAND WIRE, SHALL NOT BE LESS IN THICKNESS THAN NO. 9 U.S. GAGE WIRE AND SHALL HAVE A 41. HOOK EMBEDDED IN THE MORTAR JOINT, OR IF SHEET METAL, SHALL BE NOT LESS THAN NO. 22 U.S. GAGE BY 7/8" CORRUGATED
- 42. EACH TIE SHALL SUPPORT NOT MORE THAN 2.67 SQUARE FEET OF WALL AREA AND SHALL BE SPACED NOT MORE THAN 32 INCHES ON CENTER HORIZONTALLY AND 24 INCHES ON CENTER VERTICALLY.
- VENEER TIES AROUND WALL OPENINGS: ADDITIONAL METAL TIES SHALL BE PROVIDED AROUND ALL WALL 43. OPENINGS GREATER THAN 16 INCHES IN EITHER DIMENSION. METAL TIES AROUND THE PERIMETER OF OPENINGS SHALL BE SPACED NOT MORE THAN 3 FEET ON CENTER AND PLACED WITHIN 12 INCHES OF THE WALL OPENING.

GARAGE NOTES

- DOOR(S) BETWEEN THE GARAGE AND DWELLING SHALL BE MINIMUM 1%" SOLID CORE OR HONEY-COMBED STEEL DOOR WITH 20-MINUTE FIRE RATING EQUIPPED WITH A SELF-CLOSING DEVICE 45. VEHICLE DOORS AND FRAMES SHALL BE DESIGNED AND INSTALLED TO MEET THE 115-MPH 3-SECOND GUST
- LOADING PER DASMA 108 AND ASTM E 330-96 PER IRC 2018

GARAGE NOTES (CONTINUED)

- THE GARAGE SHALL BE SEPARATED FROM THE DWELLING AND ITS ATTIC AREAS BY MINIMUM ⁵/₈" GYP. BOARD APPLIED TO THE GARAGE SIDE OF FRAMING. WHERE HABITABLE SPACE OCCURS ABOVE THE GARAGE, THE GARAGE CEILING ASSEMBLY SHALL BE PROTECTED WITH A MINIMUM 5/2" TYPE X GYP. BOARD. WHERE A FLOOR/CEILING SPACE IS PROVIDED ABOVE THE GARAGE COLUMNS AND BEAMS
- SUPPORTING THE SEPARATION SHALL ALSO BE PROTECTED WITH %" GYP. BOARD. 45 GARAGE DOOR H-FRAME FOR THE ATTACHMENT OF THE TRACK AND COUNTER BALANCE SHALL CONSIST OF THE FOLLOWING: 2x6 VERTICAL JAMBS RUNNING FROM FLOOR TO CEILING AND SHALL BE FASTENED WITH $2\frac{1}{2}$ "" x 0.120" NAILS AT 7" O.C. STAGGERED WITH (7) 3¹/₄" x 0.120" NAILS THROUGH THE JAMB INTO THE HEADER. MINIMUM 2x8 HEADER FOR ATTACHMENT OF COUNTER BALANCE SYSTEM.

DESIGN LOADING (PER TABLE R301.5)

	<i>i</i> _
MINIMUM UNIFORMLY DISTRIB	UTED LIVE LO
UNINHABITABLE ATTICS WITHOUT STORAGE	10
UNINHABITABLE ATTICS WITH LIMITED STORAGE	20
HABITABLE ATTICS AND ATTICS SERVED WITH FIXED STAIRS	30
BALCONIES (EXTERIOR) AND DECKS	40
FIRE ESCAPES	40
GUARDRAILS AND HANDRAILS ^a	200 [°]
GUARDRAIL IN-FILL COMPONENTS ^b	50 ^c
PASSENGER VEHICLE GARAGES	50
ROOMS OTHER THAN SLEEPING ROOM	40
SLEEPING ROOM	30
STAIRS	40

a. A single concentrated load applied in any direction at any point along the top. b. Guard in-fill components (all those except the handrail), ballusters and panel fillers shall be designed to withstand a horizontally applied normal load of 50 pounds on an area equal to one square foot. This load need not be assumed to act concurrently with any other live load requirement. c. Glazing used in handrail assemblies and guards shall be designed with a safety factor of 4. The safety factor shall be applied to each of the concentrated loads applied to the top of the rail, and to the load on the infill components. These loads shall be determined independently of one another, and loads are assumed not to occur with any other live load.

d. An additional dead loading of 10 psf shall be applied where thinset tile floor is to be installed. An additional dead loading of 50 psf shall be applied where mudset tile floor is to be installed.

INSULATION/EFFICIENCY

- BUILDING ENVELOPE INSULATION SHALL COMPLY WITH IRC TABLE N1102.1.1 OR THE 2012 IECC (SEE SHEET S3.1 FOR FRAMING DETAILS AND TABLES ON THIS SHEET FOR MORE INFORMATION)
- CATHEDRAL -VAULTED CEILING FRAMING SHALL BE FRAMED WITH A MINIMUM INSULATION VALUE OF R-38. IF VAULTED RAFTERS DO NOT PROVIDE REQUIRED DEPTH TO ACHIEVE R-38 INSULATION BUILDER SHALL FUR DOWN RAFTERS PER DETAILS PROVIDED ON SHEET S3.1.

CLIMATE ZONE

FENESTRATION U-FACTOR	
SKYLIGHT U-FACTOR	
GLAZED FENSTRATION SHGC	
CEILING R-VALUE	
WOOD FRAME WALL R-VALUE	
MASS WALL R-VALUE	
FLOOR R-VALUE	
BASEMENT WALL R-VALUE	10-CONTI
SLAB R-VALUE AND DEPTH	
CRAWL SPACE WALL R-VALUE	10-CONTI
DUCTWORK EXPOSED TO OUTSIDE AIR R-VALUE	
DUCTWORK NOT EXPOSED TO OUTSIDE AIR R-VALUE	
CATHEDRAL VAULTED CEILING R-VALUE	

DUCT SEALING

N1103.2.2 (R403.2.2) SEALING (MANDATORY). DUCTS, AIR HANDLERS, AND FILTER BOXES SHALL BE SEALED. JOINTS AND SEAMS SHALL COMPLY WITH SECTION M1601.4.1 OF 2018 IRC. **EXCEPTIONS:** AIR-IMPERMEABLE SPRAY FOAM PRODUCTS SHALL BE PERMITTED TO BE APPLIED

- 1. WITHOUT ADDITIONAL JOINT SEALS.
- WHERE A DUCT CONNECTION IS MADE THAT IS PARTIALLY INACCESSIBLE, THREE SCREWS OR RIVETS SHALL BE EQUALLY SPACED ON THE EXPOSED PORTION OF THE JOINT SO AS TO PREVENT A HINGE EFFECT.
- CONTINUOUSLY WELDED AND LOCKING-TYPE LONGITUDINAL JOINTS AND SEAMS IN DUCTS OPERATING AT STATIC PRESSURES LESS THAN 2 INCHES OF WATER COLUMN PRESSURE CLASSIFICATION SHALL NOT REQUIRE ADDITIONAL CLOSURE SYSTEMS.
- DUCT TIGHTNESS SHALL BE VERIFIED BY EITHER OF THE FOLLOWING: POST-CONSTRUCTION TEST: TOTAL LEAKAGE SHALL BE LESS THAN OR EQUAL TO 4 CFM 1. PER 100 SQUARE FEET OF CONDITIONED FLOOR AREA WHEN TESTED AT A PRESSURE DIFFERENTIAL OF 0.1 INCHES W.G. ACROSS THE ENTIRE SYSTEM, INCLUDING THE MANUFACTURER'S AIR HANDLER ENCLOSURE. ALL REGISTER BOOTS SHALL BE TAPED
- OR OTHERWISE SEALED DURING THE TEST. ROUGH-IN TEST: TOTAL LEAKAGE SHALL BE LESS THAN OR EQUAL TO 4 CFM PER 100 SQUARE FEET OF CONDITIONED FLOOR AREA WHEN TESTED AT A PRESSURE DIFFERENTIAL OF 0.1 INCHES W.G. ACROSS THE SYSTEM, INCLUDING THE MANUFACTURER'S AIR HANDLER ENCLOSURE. ALL REGISTERS SHALL BE TAPED OR OTHERWISE SEALED DURING THE TEST. IF THE AIR HANDLER IS NOT INSTALLED AT THE

TIME OF THE TEST, TOTAL LEAKAGE SHALL BE LESS THAN OR EQUAL TO 3 CFM PER 100 SQUARE FEET OF CONDITIONED FLOOR AREA. **EXCEPTION:** THE TOTAL LEAKAGE TEST IS NOT REQUIRED FOR DUCTS AND AIR HANDLERS LOCATED ENTIRELY WITHIN THE BUILDING THERMAL ENVELOPE.

ME	ECHANICAL VENTILATIO		
FAN LOCATION	AIR FLOW RATE MINIMUM (CFM)	MINIMUM EFFICACY (CFM/WATT)	AIR FLOW RATE MAXIMUM (CFM)
RANGE HOODS	ANY	2.8	ANY
IN-LINE FAN	ANY	2.8	ANY
BATHROOM, UTILITY ROOM	10	1.4	90
BATHROOM, UTILITY ROOM	90	2.8	ANY

19 INUOUS OR 13-CAVITY 10 AT 2'-0" INUOUS OR 13-CAVITY 8 6

0.35

0.55

0.40

49

15

8/13

38

-DEPENDENT UPON SLAB CONSTRUCTION 10 ^d

10 ^d

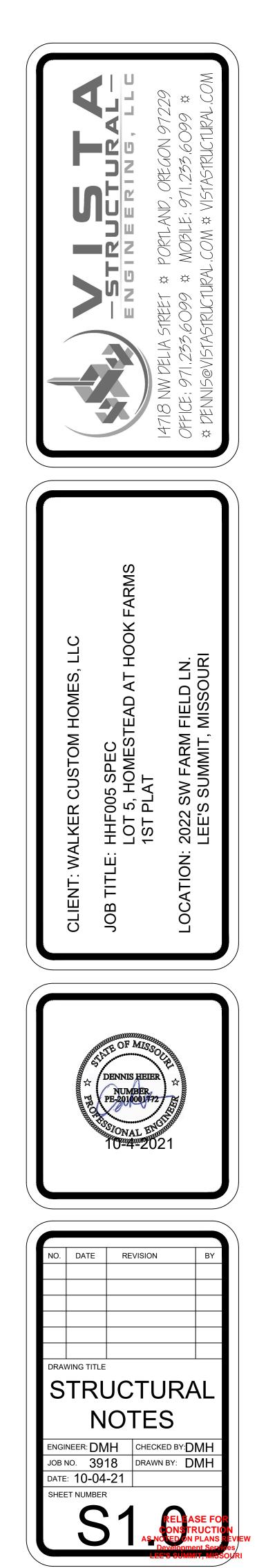
10 ^d

LOADS (PSF) DEAD LOAP 10 10

10

10 ^d

10



RESIDENTIAL SEISMIC & WIND ANALYSIS

OCATION					DEAD LOAD (psf)	AREA (ft ²)	WE
ROOF					10	2692	
EILING					10	2692	
SECOND FLOOR					10	827	
IRST FLOOR					10	2692	
				WALL LENGTH (ft)	WALL HEIGHT (ft)	WALL UNIT WT. (psf)	W
SECOND FLOOR EX				166	8	8	
IRST FLOOR EXT.	WALL DL			223	10	10	
					DEAD LOAD (psf)	AREA (ft2)	W
	T. PARTITION WALL DL				6	827	
					C	0000	
IK31 FLOOK INT. F	PARTITION WALL DL				6	2692	
IK31 FLOOK INT. F							
	PROJ		DESIGN PER 115 MPH 3	S-SECOND GUST, EXPOSURE	C AND MEAN ROOF HEIGHT	<= 30 FT ASSUMED)	
	PROJ FRONT-	TO-BACK	DESIGN PER 115 MPH 3	3-SECOND GUST, EXPOSURE	C AND MEAN ROOF HEIGHT SIDE-	<= 30 FT ASSUMED) FO-SIDE	
	PROJ FRONT- AREA	TO-BACK LOAD	DESIGN PER 115 MPH 3		C AND MEAN ROOF HEIGHT SIDE-T AREA	<= 30 FT ASSUMED) FO-SIDE LOAD	
SLOPED ROOF	PROJ FRONT- AREA 213	TO-BACK		SLOPED ROOF	C AND MEAN ROOF HEIGHT SIDE-T AREA 291	<= 30 FT ASSUMED) FO-SIDE LOAD 1261	
SLOPED ROOF VERT. ROOF	PROJ FRONT- AREA 213 0	TO-BACK LOAD 938 0	CUMULATIVE	SLOPED ROOF VERT. ROOF	C AND MEAN ROOF HEIGHT SIDE-T AREA 291 0	<= 30 FT ASSUMED) FO-SIDE LOAD 1261 0	CL
SLOPED ROOF VERT. ROOF 2ND	PROJ FRONT- AREA 213 0 411.03	TO-BACK LOAD		SLOPED ROOF VERT. ROOF 2ND	C AND MEAN ROOF HEIGHT SIDE-T AREA 291 0 335.97	<= 30 FT ASSUMED) FO-SIDE LOAD 1261 0 5024	CL
SLOPED ROOF VERT. ROOF 2ND 1ST	PROJ FRONT- AREA 213 0	TO-BACK LOAD 938 0	CUMULATIVE	SLOPED ROOF VERT. ROOF 2ND 1ST	C AND MEAN ROOF HEIGHT SIDE-T AREA 291 0	<= 30 FT ASSUMED) FO-SIDE LOAD 1261 0	CL
SLOPED ROOF VERT. ROOF 2ND	PROJ FRONT- AREA 213 0 411.03	TO-BACK LOAD 938 0 5897	CUMULATIVE 6835	SLOPED ROOF VERT. ROOF 2ND	C AND MEAN ROOF HEIGHT SIDE-T AREA 291 0 335.97	<= 30 FT ASSUMED) FO-SIDE LOAD 1261 0 5024	CL
SLOPED ROOF VERT. ROOF 2ND 1ST	PROJ FRONT- AREA 213 0 411.03 590.37	TO-BACK LOAD 938 0 5897 8232	CUMULATIVE 6835 15067 0	SLOPED ROOF VERT. ROOF 2ND 1ST	C AND MEAN ROOF HEIGHT SIDE-T AREA 291 0 335.97 636.13	<pre><= 30 FT ASSUMED) TO-SIDE LOAD 1261 0 5024 8765</pre>	CL
SLOPED ROOF VERT. ROOF 2ND 1ST	PROJ FRONT- AREA 213 0 411.03 590.37	TO-BACK LOAD 938 0 5897 8232	CUMULATIVE 6835 15067 0 PRESSURE (PSF	SLOPED ROOF VERT. ROOF 2ND 1ST BSMT ^a	C AND MEAN ROOF HEIGHT SIDE-T AREA 291 0 335.97 636.13	<pre><= 30 FT ASSUMED) TO-SIDE LOAD 1261 0 5024 8765</pre>	CL 2a (FIG
SLOPED ROOF VERT. ROOF 2ND 1ST	PROJ FRONT- AREA 213 0 411.03 590.37 0	TO-BACK LOAD 938 0 5897 8232 0	CUMULATIVE 6835 15067 0 PRESSURE (PSP	SLOPED ROOF VERT. ROOF 2ND 1ST BSMT ^a) - PER ASCE CH. 6	C AND MEAN ROOF HEIGHT SIDE-T AREA 0 335.97 636.13 157	<= 30 FT ASSUMED) O-SIDE LOAD 1261 0 5024 8765 2732	

q_{z10}=0.00256K_zK_{zt}K_dV² (ASCE7-10 Velocity Pressure) q_{z10_ASD}=0.6q_{z10} (Design Velocity Pressure for ASD analysis under ASCE7-10 and IRC/IBC 2012)

2ND FLOOR TRIBUTARY WEIGHT 1ST FLOOR TRIBUTARY WEIGHT

BASEMENT TRIBUTARY WEIGHT S_S (SITE GROUND MOTION - %g - FROM ASCE7 SEISMIC MAP)

F_a (from ASCE7 Table 11.4-1)

S_{DS} (= 2/3 * S_S * F_a) R (from ASCE7 Table 12.2-1)

N OR DR NT		From AS(CE7 (Eq. 12.8-1):	V (= 1.2 * S _{DS} * W / R) (lbs.) 1398 2099 2099
Sheathing Location	Min. Sheathing Schedule	Fastening Schedule	Allowable She	ar (#/LF) Code Refere
Exterior <u>(Option #1)</u>	7/16" APA Rated Plywood/OSB	1-1/2" 16ga. Staples w/ 1" penetration@ 6" OC Edges, 6" OC Field For 24" stud spacing, 12" OC Field For 16" stud spacing	155	per IBC, Tab 2306.3(1)
Exterior <u>(Option #2)</u>	7/16* APA Rated Plywood/OSB	1-1/2" 16ga. Staples w/ 1" penetration@ 4" OC Edges, 6" OC Field For 24" stud spacing, 12" OC Field For 16" stud spacing	230	per IBC, Tab 2306.3(1)
Exterior (Option #3)	7/16" APA Rated Plywood/OSB	1-1/2" 16ga. Staples w/ 1" penatration@ 3" OC Edges, 6" OC Field For 24" stud spacing, 12" OC Field For 16" stud spacing	310	per IBC, Tab 2306.3(1)
Exterior <u>(Option #4)</u>	7/16" APA Rated Plywood/OSB or shiplap panel sheathing, or 3/8" shiplap panel sheathing with tighter nail spacing	8d Common Nails w/ 1-3/8" penetration @ 6" O.C. Edges, 12" O.C. Field for 7/16" APA-rated plywood/OSB or shiplap panel sheathing OR @ 4" O.C. Edges, 12" O.C. Field for 3/8" shiplap panel sheathing	220	AF&PA SDP Table 4.3A
Exterior <u>(Option #5)</u>	7/16" APA Rated Plywood/OSB or shiplap panel sheathing, or 3/8" shiplap panel sheathing with tighter nail spacing	8d Common Nails w/ 1-3/8" penetration @ 4" O.C. Edges, 12" O.C. Field for 7/16" APA-rated plywood/OSB or shiplap panel sheathing OR @ 3" O.C. Edges, 12" O.C. Field for 3/8" shiplap panel sheathing	320	AF&PA SDP Table 4.3A
Exterior <u>(Option #6)</u>	7/16" APA Rated Plywood/OSB or shiplap panel sheathing, or 3/8" shiplap panel sheathing with tighter nail spacing and double studs at each panel edge	8d Common Nails w/ 1-3/8" penetration @ 3" O.C. Edges, 12" O.C. Field	410	AF&PA SDP Table 4.3A
Interior	1/2" Gypsum Board	No. 6- 1 ¹ / ₄ " Type W or S Screws @ 8" O.C. Edges, 12" O.C. Field	60	per IBC, Tab 2306.4.4
Interior	16 Ga. Simpson/USP Type WB Steel X-Brace (or equal)	(3) 16d @ end studs & (1) 8d @ intermediate studs (per manufacturer specifications - see detail on sheet S3)	325	

EXTERIOR SHEATHIN	NG OPTION FOR SECC	OND FLOOR	4					
EXTERIOR SHEATHIN	NG OPTION FOR FIRS	T FLOOR	5		WIDTH OF 1ST STORY (FT.)	53.67	WIDTH OF 2ND STORY (FT.)	45.67
EXTERIOR SHEATHIN	NG OPTION FOR BASE	MENT WALLS	5		DEPTH OF 1ST STORY (FT.)	57.83	DEPTH OF 2ND STORY (FT.)	37.33
					BACK WALL OF GARAGE (FT.)	0		
					GAR. WALL: 1=F-B, 2=S-S	2		
					LENGTHS (ft.) & RESISTANCES			
		SE	SMIC	IOR STRUCTURAL WALL	LENGTHS (IL) & RESISTANCES	WIND		
	FRONT-TO-BACK	RESISTANCE (lbs.)	SIDE-TO-SIDE	RESISTANCE (lbs.)	FRONT-TO-BACK	RESISTANCE (lbs.)	SIDE-TO-SIDE	RESISTANCE (lbs.)
2ND FLOOR	30	8400	42	11760	30	11760	42	16464
1ST FLOOR	70	26600	30	11400	70	37240	30	15960
BASEMENT	0	0	24.5	9310	0	0	24.5	13034
		ADDITIONAL RESIS			Anchor Bolt Spacing		16d Nail Spacing req'd at l	bottom plate (in.)
		SEISMIC	WIND		diameter (in.)	0.5	2nd Floor F-B	46
2ND FLOOR FRONT-1		0	0		Shear value (per NDS)	944	2nd Floor S-S	46
2ND FLOOR SIDE-TO-SIDE		0	0		Spacing F-B (inches)	139.1	1st Floor F-B	21
1ST FLOOR FRONT-T		0	0		spacing S-S (inches)	129.3	1st Floor S-S	19
1ST FLOOR SIDE-TO-		0	0					
BASEMENT FRONT-T		0	0					
BASEMENT SIDE-TO-	SIDE	0	0					

0 0

		RESISTANCE REQUIF	RED IN ADDITION TO RES	ISTANCE PROVIDED BY EXTERIOR V	VALLS**		
	ADDITIONAL RESISTANCE REQUIRED (POUNDS)	PORTAL FRAMES OR PERF. SHEAR WALL RESISTANCE	INTERIOR X-BRACES (325#/BRACE)	INTERIOR WALL LENGTH W/ 1/2" GYPSUM BOARD PER TABLE (FT.)	INT. WALL LENGTH SHEATHED W/ OSB (TOTAL LENGTH, ONE SIDE, FT.)	RESISTANCE PROVIDED BY ADDITIONAL METHODS (POUNDS)	OK?
2ND FLOOR FRONT-TO-BACK	0					0	YES
2ND FLOOR SIDE-TO-SIDE	0					0	YES
1ST FLOOR FRONT-TO-BACK	0					0	YES
1ST FLOOR SIDE-TO-SIDE	0					0	YES
BASEMENT FRONT-TO-BACK	0					0	YES
BASEMENT SIDE-TO-SIDE	0					Ô	YES

**NOTES: 1) SEE ATTACHED CALCULATIONS FOR PORTAL FRAME OR PERFORATED SHEAR WALL RESISTANCE CAPACITIES (IF APPLICABLE), 2) SEE SHEET S1 FOR INTERIOR STEEL X-BRACE INSTALLATION, 3) INTERIOR WALLS SHEATHED WITH OSB SHALL BE ATTACHED WITH SAME STAPLE/NAILING PATTERN AS EXTERIOR OSB ON SAME FLOOR (SEE TABLE ABOVE) AND ARE ONLY APPLICABLE FOR FULL-HEIGHT SECTIONS OF 2'-8" OR LONGER

	LL LATERAL BRACING ACHIEVED AT EXTERIOR WALLS AND WALLS DIRECTLY ON FOUNDATIONS; THEREFORE, NO INTERIOR BRACING PER 2012 IRC SECTION R502.2.1 IS REQUIRED									
	WIND UPLIFT ANALYSIS									
	X/12	DEGREES								
ROOF PITCH (MAX)	5	22.6	PITCH OF 6 OR LESS:	EOH -13.3, E -7.2, G -5.2						
ASCE 7										
	LENGTH (FT.)	PRESSURE (PSF)	LINEAL FT. OF OH	UPLIFT PER FT* (LBS)						
OVERHANG	1	16.56	225	16.56						
	TOTAL AREA (FT ²)	ZONE E AREA (FT ²)	ZONE G AREA (FT ²)	PRESSURE ZN. E (PSF)	PRESSURE ZN. G (PSF)	TOTAL FORCE (LBS)	FORCE PER LINEAL FT @ PERIMETER (LBS)			
MAIN ROOF**	3103.7361	1320.968976	1782.767124	15.12	10.5	38692	173.5			
*ALONG PERIMETER	*ALONG PERIMETER TOTAL UPLIFT PER LINEAL FOOT ALONG EXTERIOR (POUNDS)			190.1	UPLIFT OK					
**INSIDE EXTERIOR V	*INSIDE EXTERIOR WALLS RESISTANCE DUE TO DEAD WEIGHT & (3) 10d TOENAILS									

NOTE FOR CONSTRUCTION:

THE CONTINUOUS STRUCTURAL PANEL SHEATHING BRACING METHOD REQUIRES USE OF THE ABOVE TABLE FOR SHEATHING OF THE ENTIRE STRUCTURE. IN ADDITION, FRAMING MEMBERS SHALL BE @ 16" O.C. MAX., UNBLOCKED, AND W/ SHEATHING APPLIED DIRECTLY TO FRAMING MEMBERS

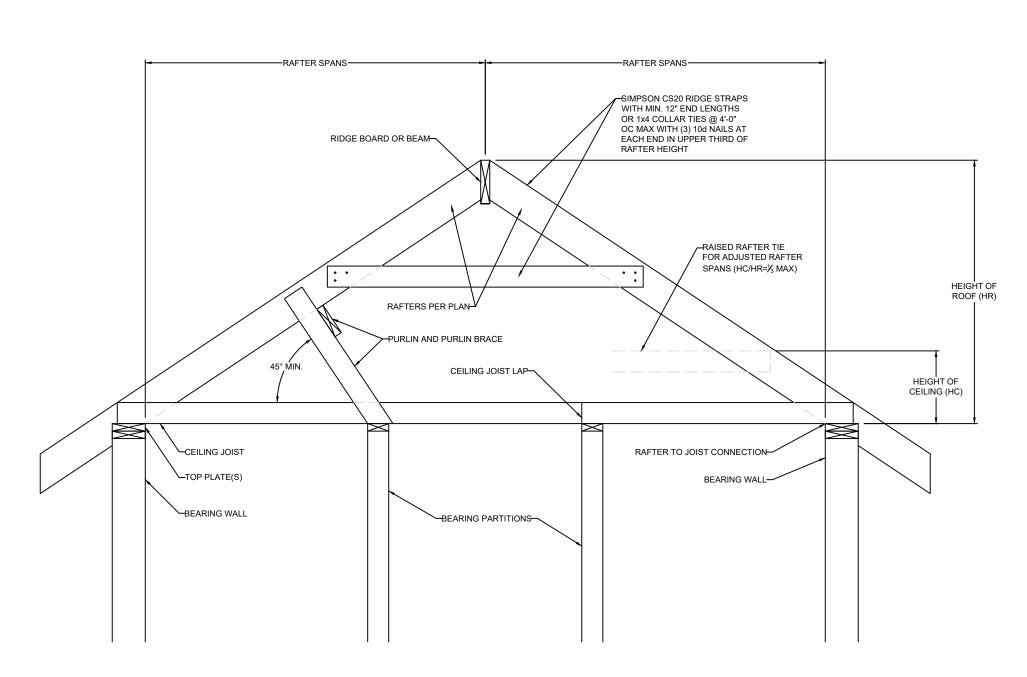
NOTE FOR DESIGN:

ALL WALLS USED IN THE CALCULATION OF THE RESISTANCE FOR THIS STRUCTURE SHALL HAVE A MINIMUM UNINTERRUPTED HEIGHT OF 8'-0" AND LENGTH OF 2'-8". ALLOWABLE RESISTANCES HAVE BEEN #/FT AND INCREASED BY 40% FOR WIND LOADS, PER VALUES IN 2012 IBC SECTION 2306 AND AF&PA SDPWS TABLE 4.3A. FOR EXAMPLE, 7/16" APA-RATED SHEATHING WITH 8d @ 6" & 12" HAS A SEISMIC SHEAR VALUE OF 240 A WIND SHEAR VALUE OF 335#/FT - 40% GREATER THAN THAT OF SEISMIC)

NOTE: SOIL SITE CLASS ASSUMED TO BE CLASS D. IF SITE CONDITIONS ARE DETERMINED TO BE CLASS E OR F, CONSULT ENGINEER BEFORE PROCEEDING WITH CONSTRUCTION

	INPUT
	CALCULATED VALUE
	WEIGHT (lbs.)
	26920
	26920
	8270
	26920
	WEIGHT (lbs)
	10624
	22300
	WEIGHT (lbs)
	4962
	16152
_	
	CUMULATIVE
	6285
	15049
	10256
_	
	2a (FIG. 28.6-1, ASCE7)
	10.734

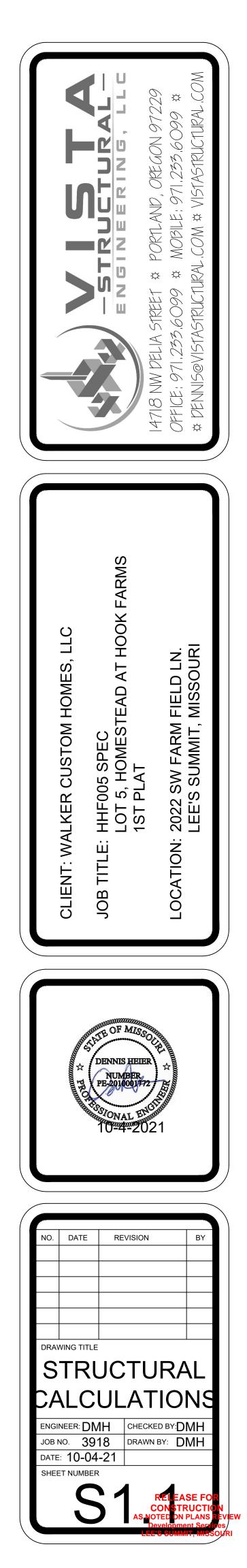
59152	
88846	
88846	
12.0%	
1.6	
0.128	
6.5	

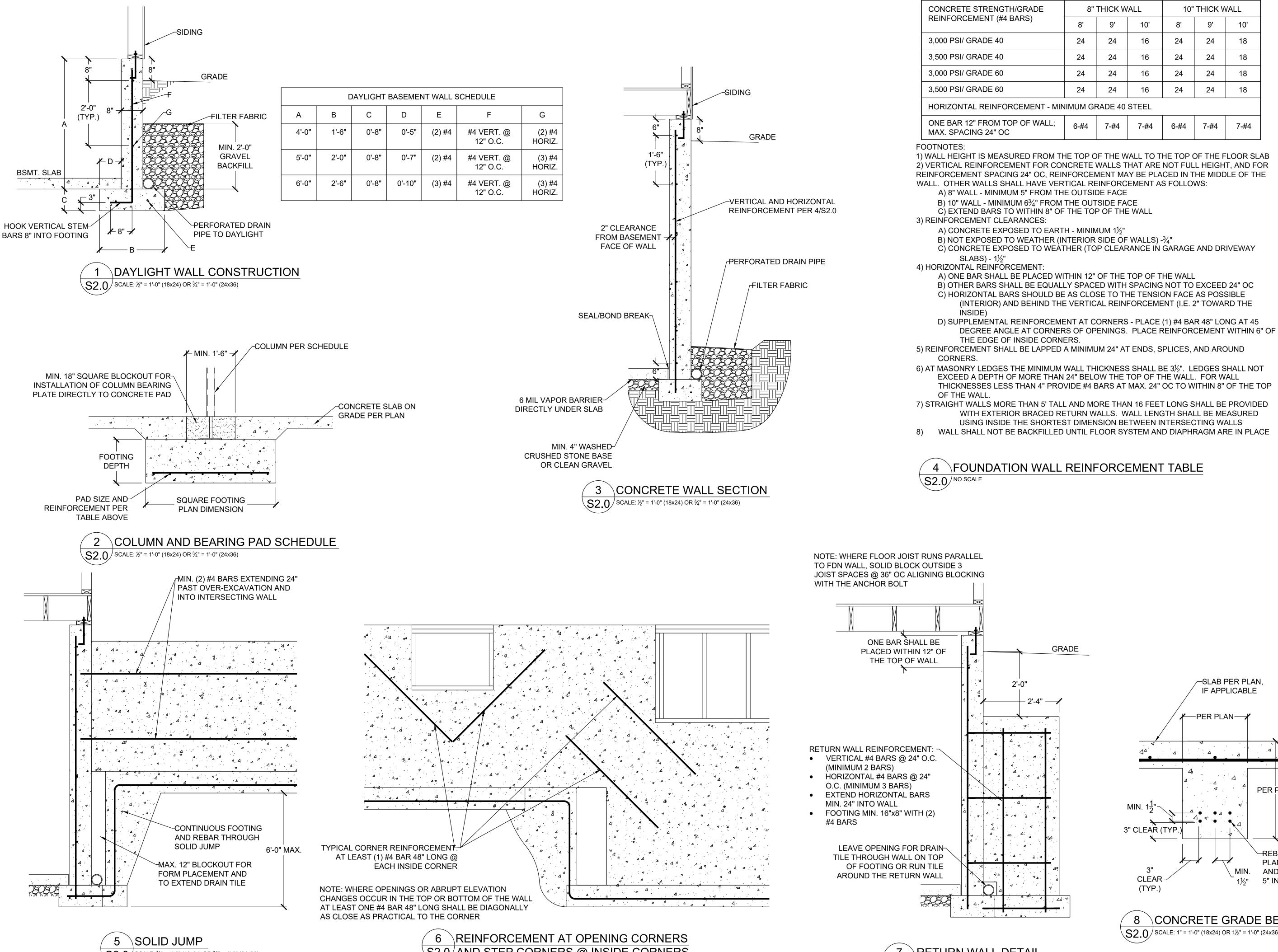


 BRACED RAFTER CONSTRUCTION

 \$1.1

 SCALE: 1" = 1'-0" (18x24) OR 1½" = 1'-0" (24x36)

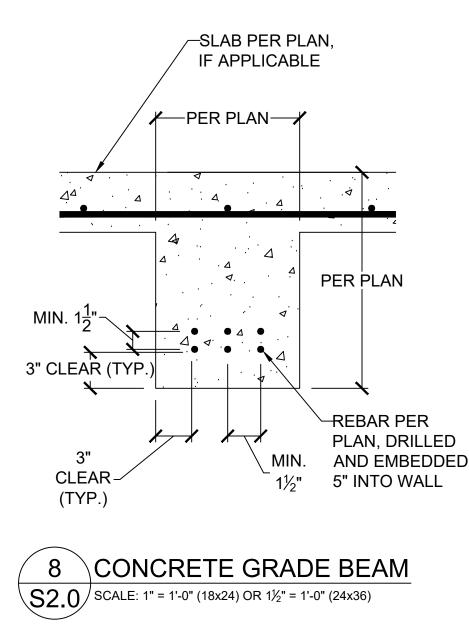




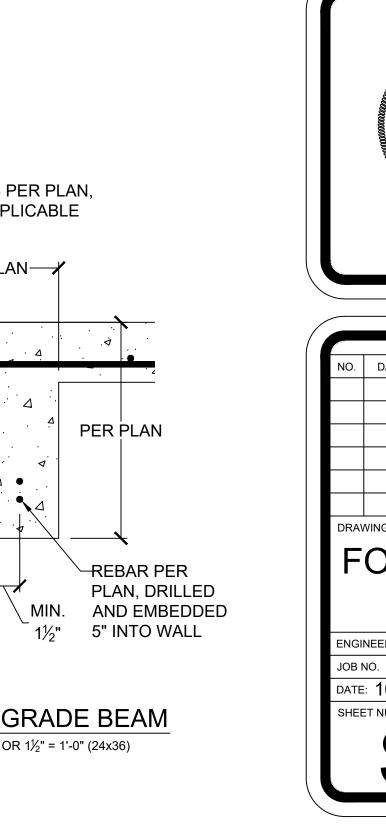
 $\overline{S2.0}$ SCALE: $\frac{1}{2}$ " = 1'-0" (18x24) OR $\frac{3}{4}$ " = 1'-0" (24x36)

S2.0/AND STEP CORNERS @ INSIDE CORNERS SCALE: ¹/₂" = 1'-0" (18x24) OR ³/₄" = 1'-0" (24x36)

ackslashRETURN WALL DETAIL SCALE: $\frac{1}{2}$ " = 1'-0" (18x24) OR $\frac{3}{4}$ " = 1'-0" (24x36) GRADE



AND EMBEDDED



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7) STRAIGHT WALLS MORE THAN 5' TALL AND MORE THAN 16 FEET LONG SHALL BE PROVIDED WITH EXTERIOR BRACED RETURN WALLS. WALL LENGTH SHALL BE MEASURED

USING INSIDE THE SHORTEST DIMENSION BETWEEN INTERSECTING WALLS WALL SHALL NOT BE BACKFILLED UNTIL FLOOR SYSTEM AND DIAPHRAGM ARE IN PLACE

A) ONE BAR SHALL BE PLACED WITHIN 12" OF THE TOP OF THE WALL B) OTHER BARS SHALL BE EQUALLY SPACED WITH SPACING NOT TO EXCEED 24" OC C) HORIZONTAL BARS SHOULD BE AS CLOSE TO THE TENSION FACE AS POSSIBLE (INTERIOR) AND BEHIND THE VERTICAL REINFORCEMENT (I.E. 2" TOWARD THE

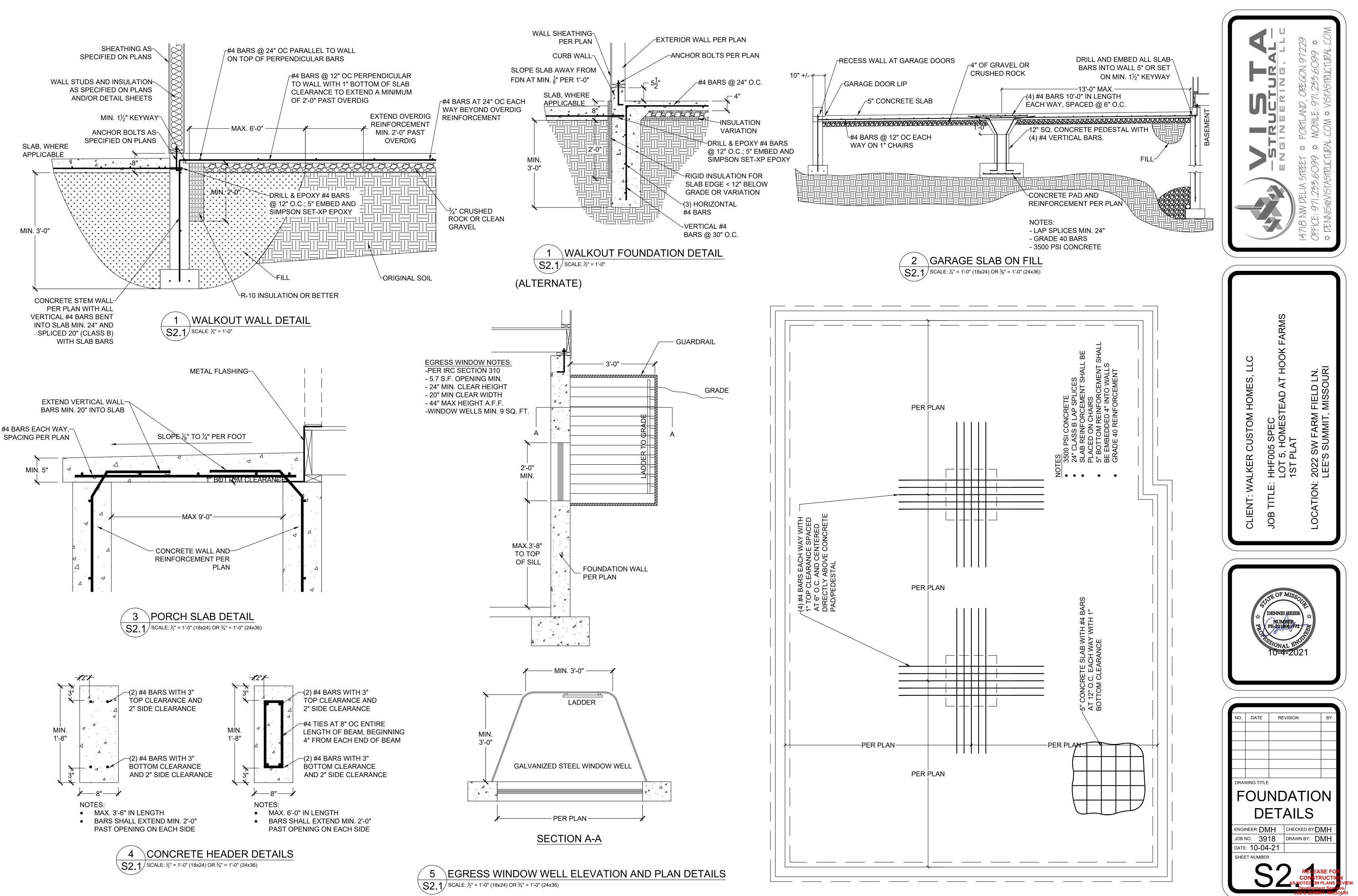
C) CONCRETE EXPOSED TO WEATHER (TOP CLEARANCE IN GARAGE AND DRIVEWAY

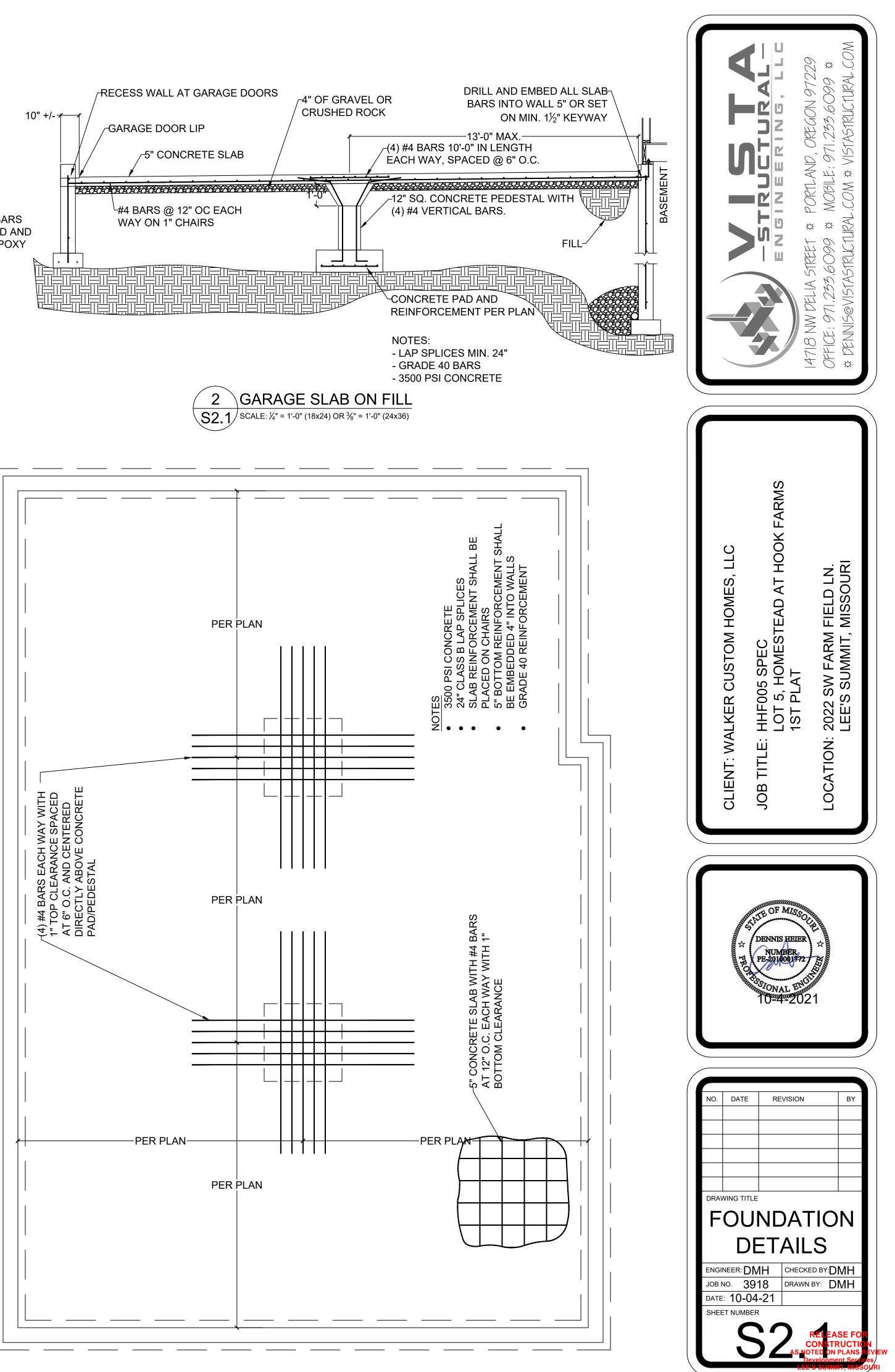
2) VERTICAL REINFORCEMENT FOR CONCRETE WALLS THAT ARE NOT FULL HEIGHT, AND FOR REINFORCEMENT SPACING 24" OC, REINFORCEMENT MAY BE PLACED IN THE MIDDLE OF THE

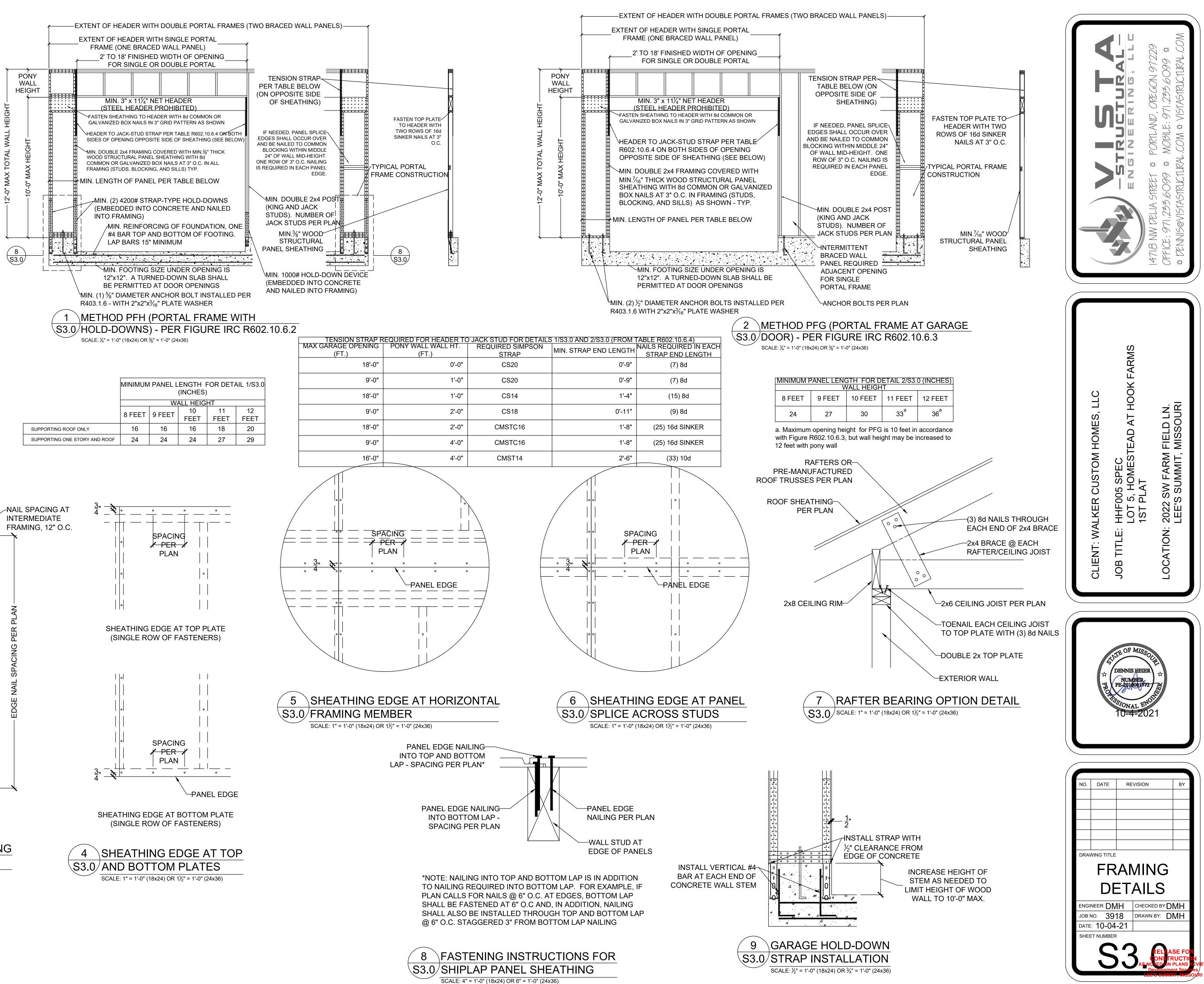
VERTICAL REINFORCEMENT SPACING									
CONCRETE STRENGTH/GRADE REINFORCEMENT (#4 BARS)	8"	THICK W	ALL	10"	VALL				
	8'	9'	10'	8'	9'	10'			
3,000 PSI/ GRADE 40	24	24	16	24	24	18			
3,500 PSI/ GRADE 40	24	24	16	24	24	18			
3,000 PSI/ GRADE 60	24	24	16	24	24	18			
3,500 PSI/ GRADE 60	24	24	16	24	24	18			
HORIZONTAL REINFORCEMENT - MINIMUM GRADE 40 STEEL									
ONE BAR 12" FROM TOP OF WALL; MAX. SPACING 24" OC	6-#4	7-#4	7-#4	6-#4	7-#4	7-#4			

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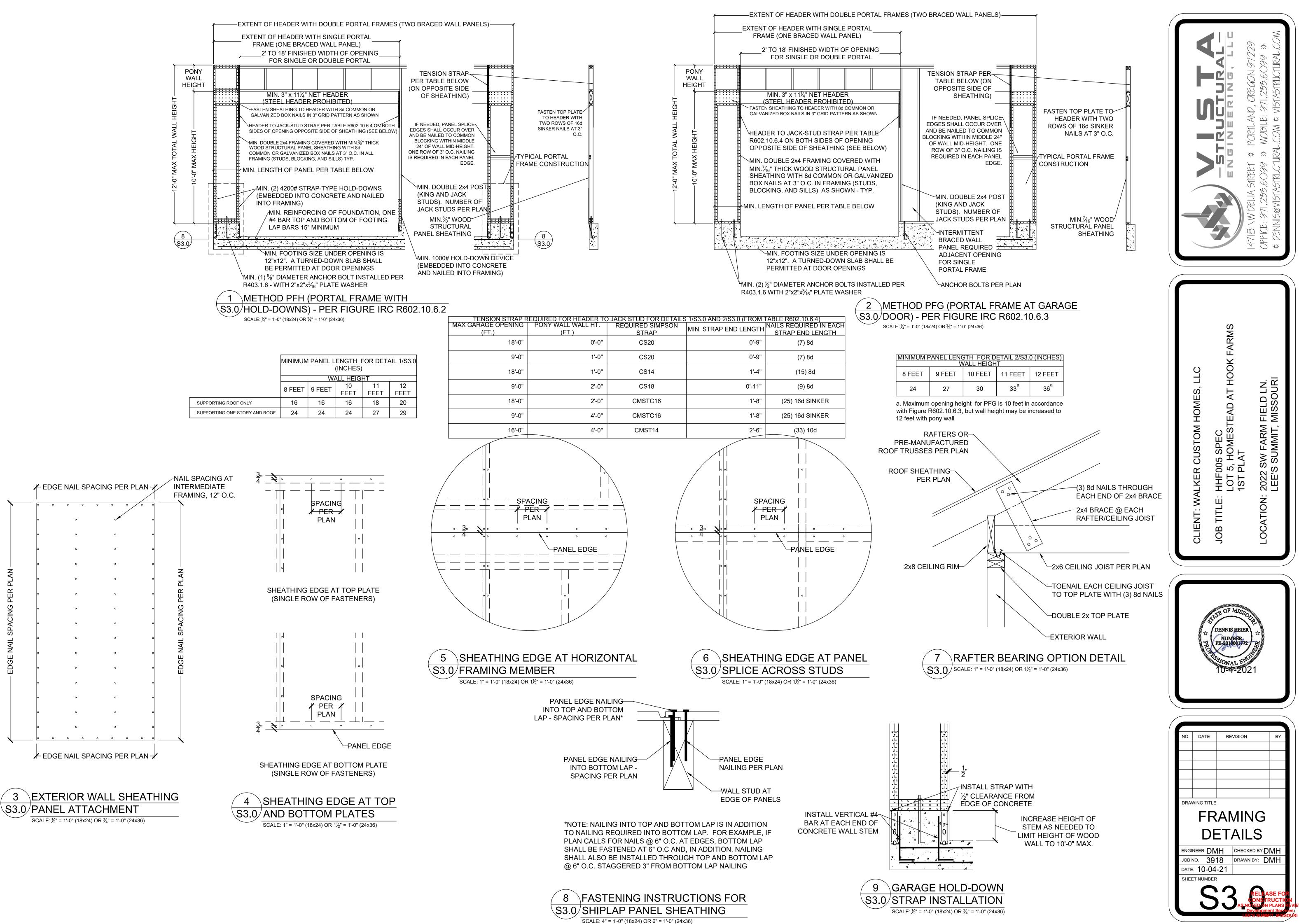
S

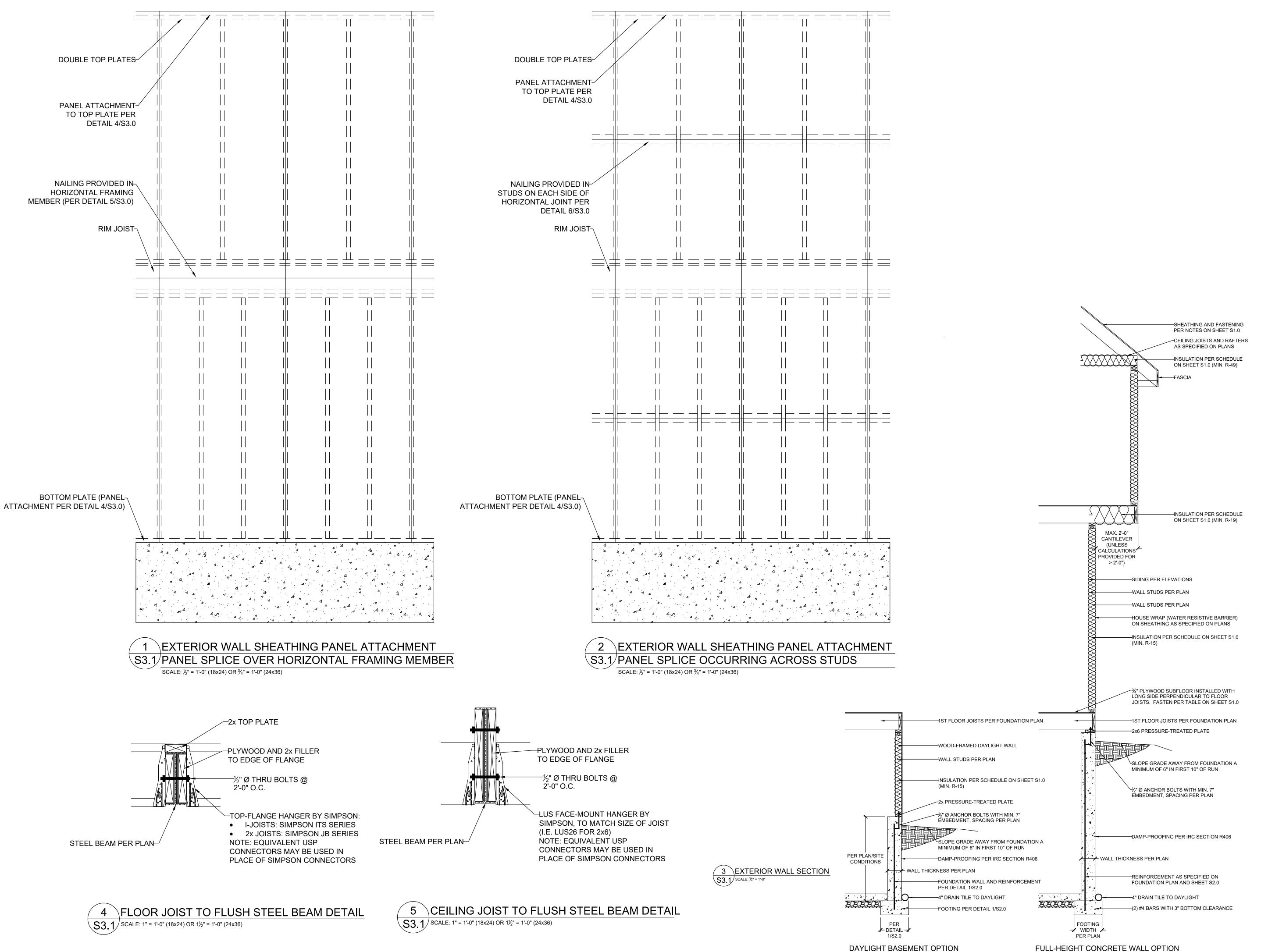


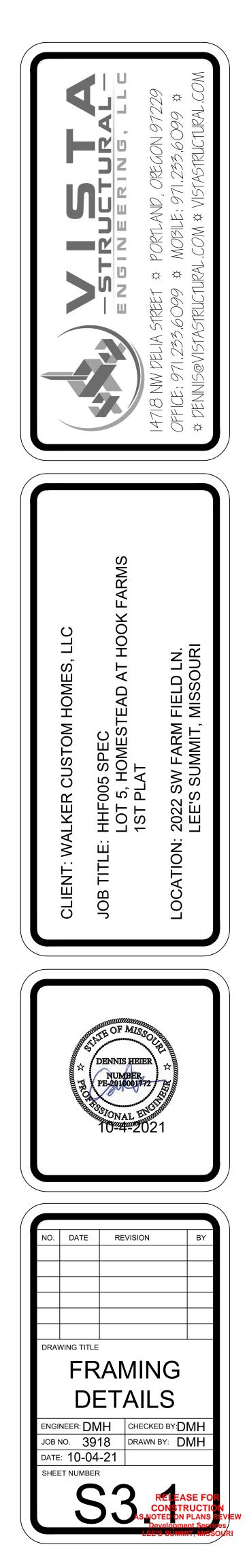


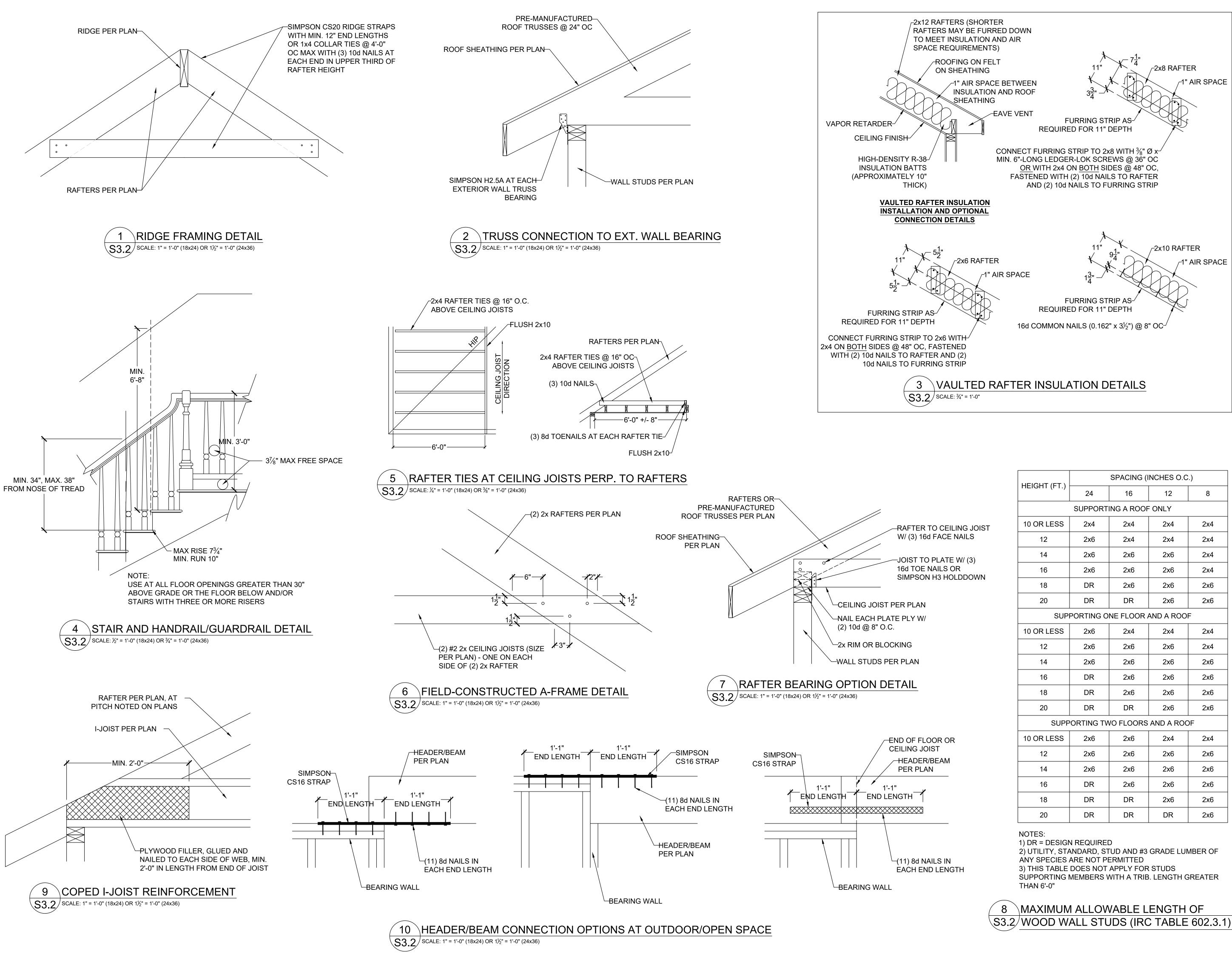


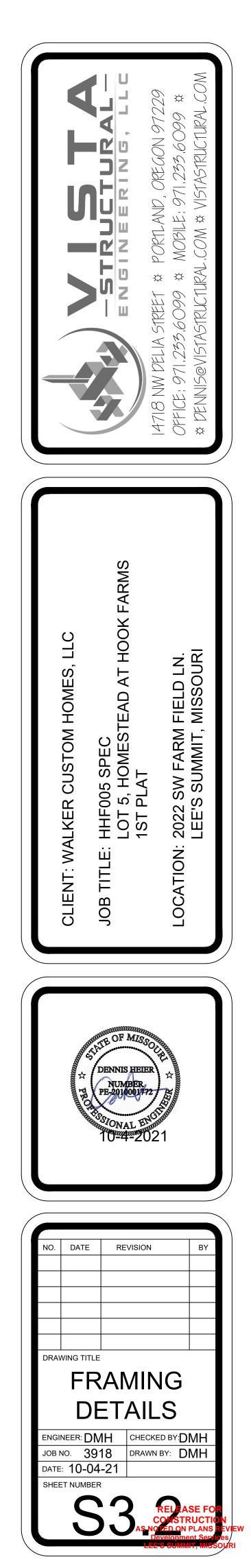
	MINIMUM PANEL LENGTH FOR DETA (INCHES)							
	ALL HEIG	HT						
	8 FEET 9 FEE	9 FEET	10	11				
	_	_	FEEI	FEEI				
SUPPORTING ROOF ONLY	16	16	16	18				
SUPPORTING ONE STORY AND ROOF	24	24	24	27				
	SUPPORTING ROOF ONLY	SUPPORTING ROOF ONLY 16	W/ 8 FEET 9 FEET SUPPORTING ROOF ONLY 16	INCHES WALL HEIG 8 FEET 9 FEET SUPPORTING ROOF ONLY 16 16 16	(INCHES) WALL HEIGHT 8 FEET 9 FEET 10 11 FEET 9 FEET 10 11 SUPPORTING ROOF ONLY 16 16 18			

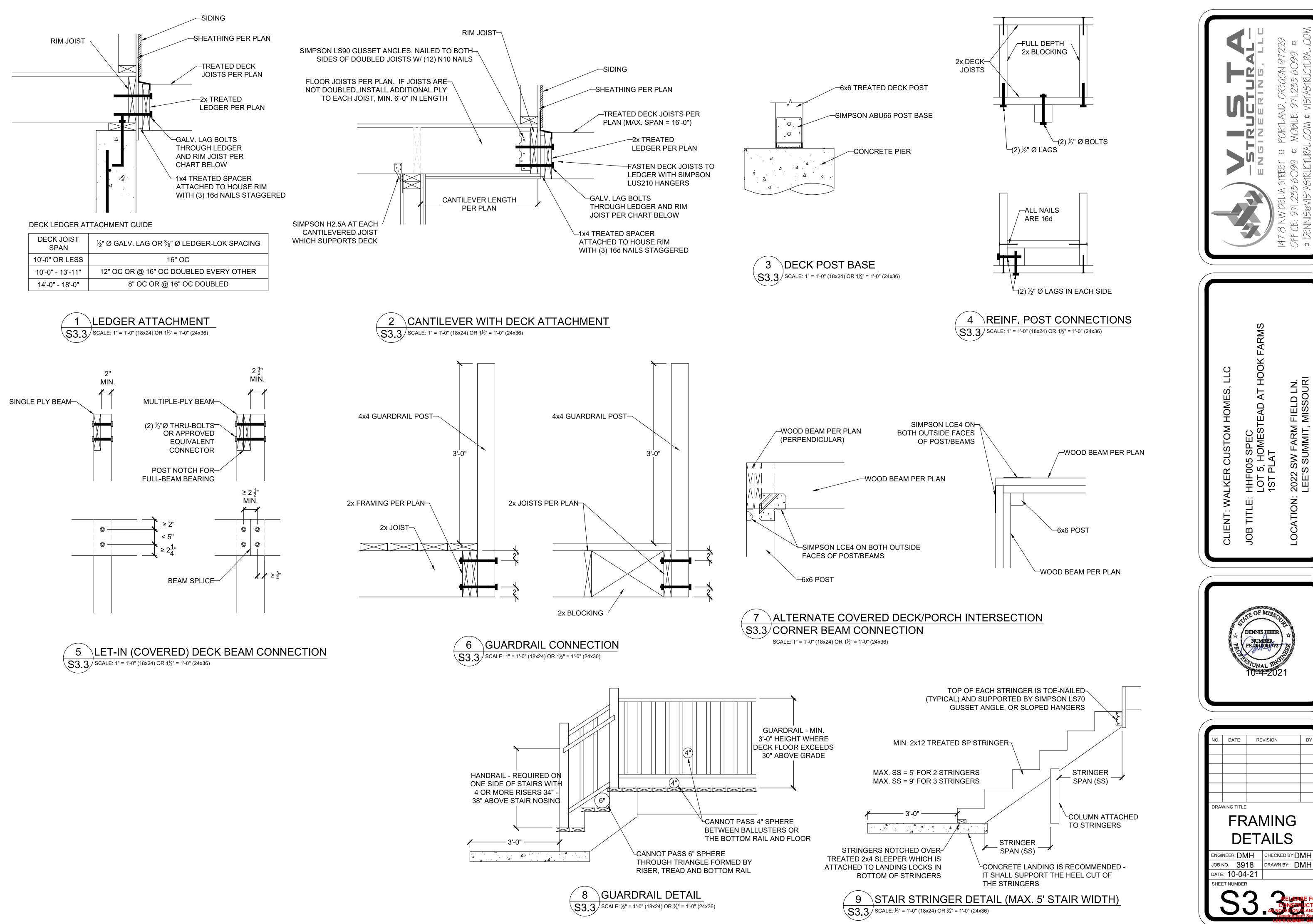


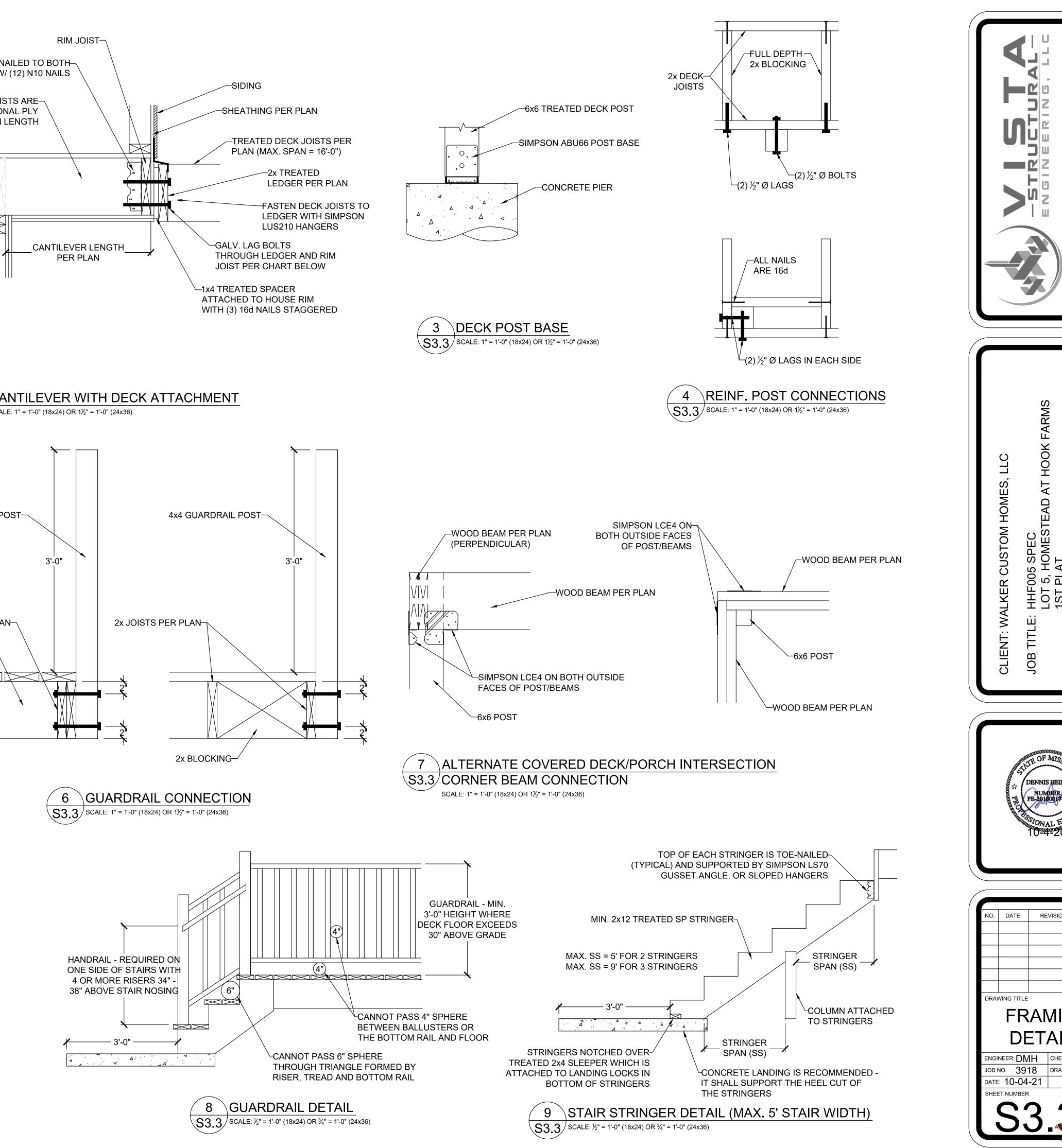












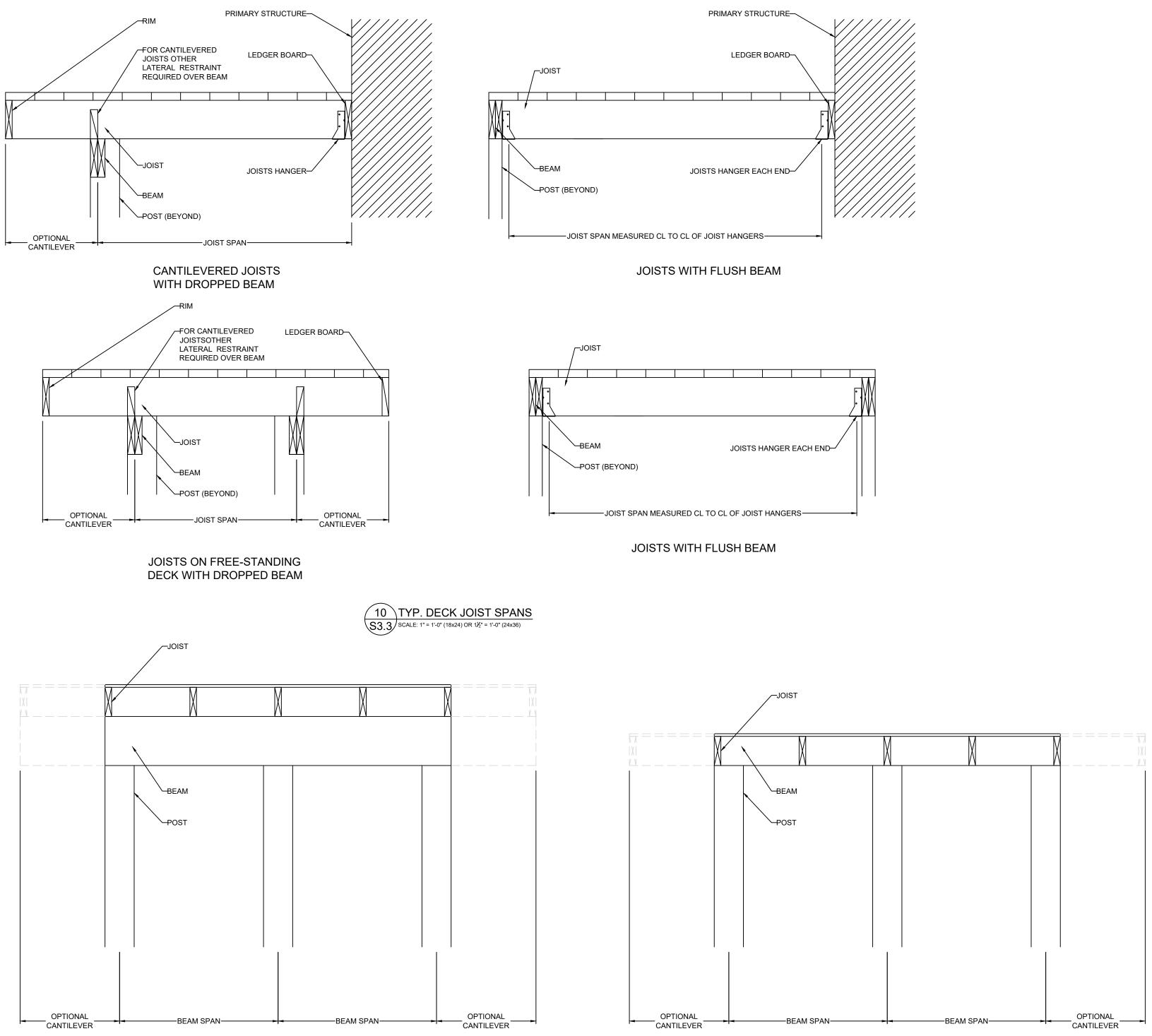
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FIELD LN. MISSOURI

SW SU

L 20

OCATION:



DROPPED BEAM



FLUSH BEAM

