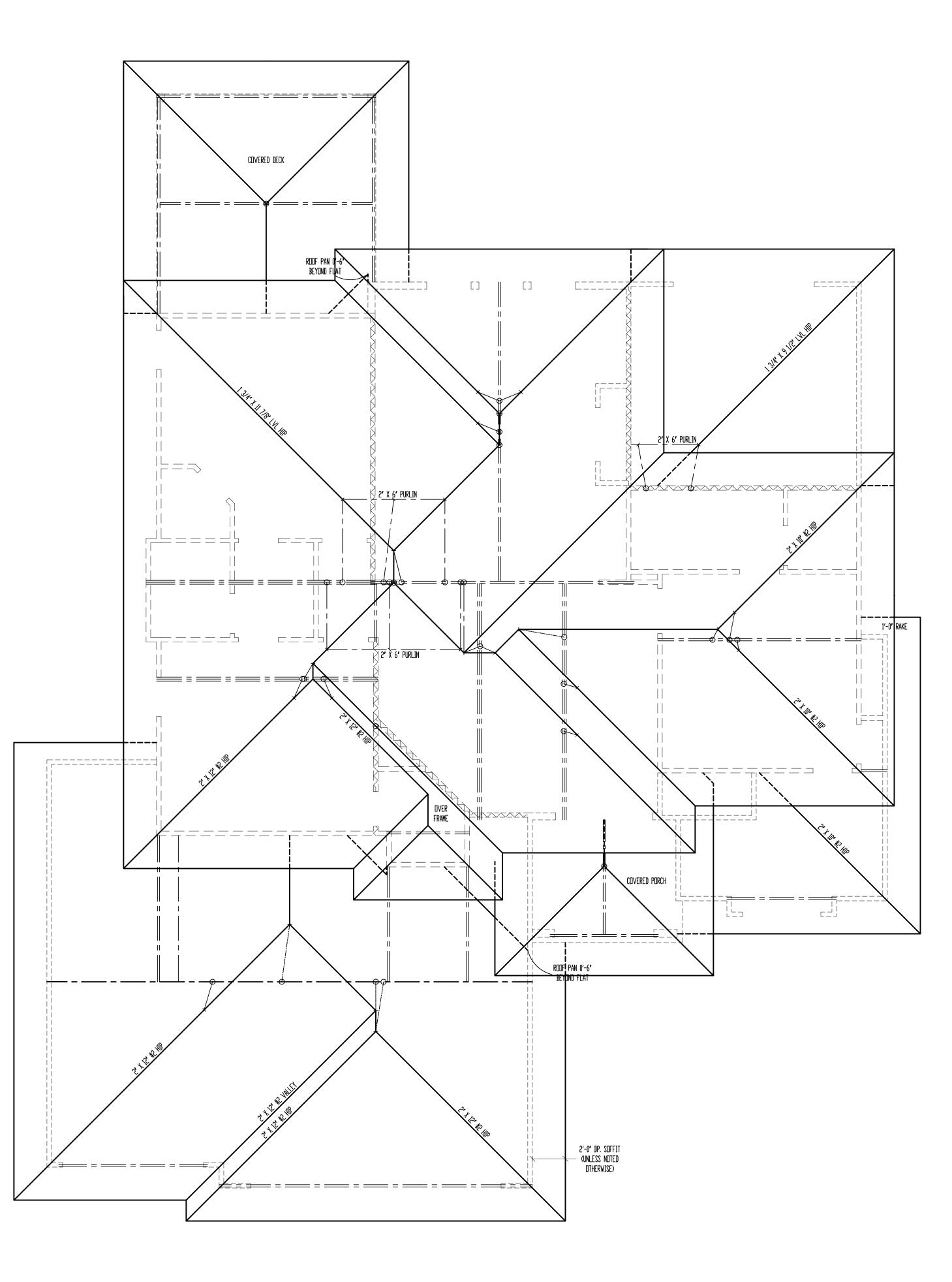


RELEASE FOR CONSTRUCTION AS NOTED FOR PLAN REVIEW DEVELOPMENT SERVICES LEE'S SUMMIT, MISSOURI 10/06/2021

SCALE: 1/8'' = 1'-0''



Flashing Note:

Roof Notes:

* RAFTERS (HEM-FIR, DDUG-FIR, DR EQUAL): see span charts below

	code minii	MUM
	RAFTERS	SPACING
	#2-2x6	024 " D.C.
$\rangle\rangle\rangle$	#2-2x6	@16″ D.C.
	#2-2x8	024″ D.C.
	#2-2x8	@16 * D.C.
	#2-2x10	024 " D.C.
	#2-2x10	@16″ D.C.
	NOTE: CODE	e minimum all

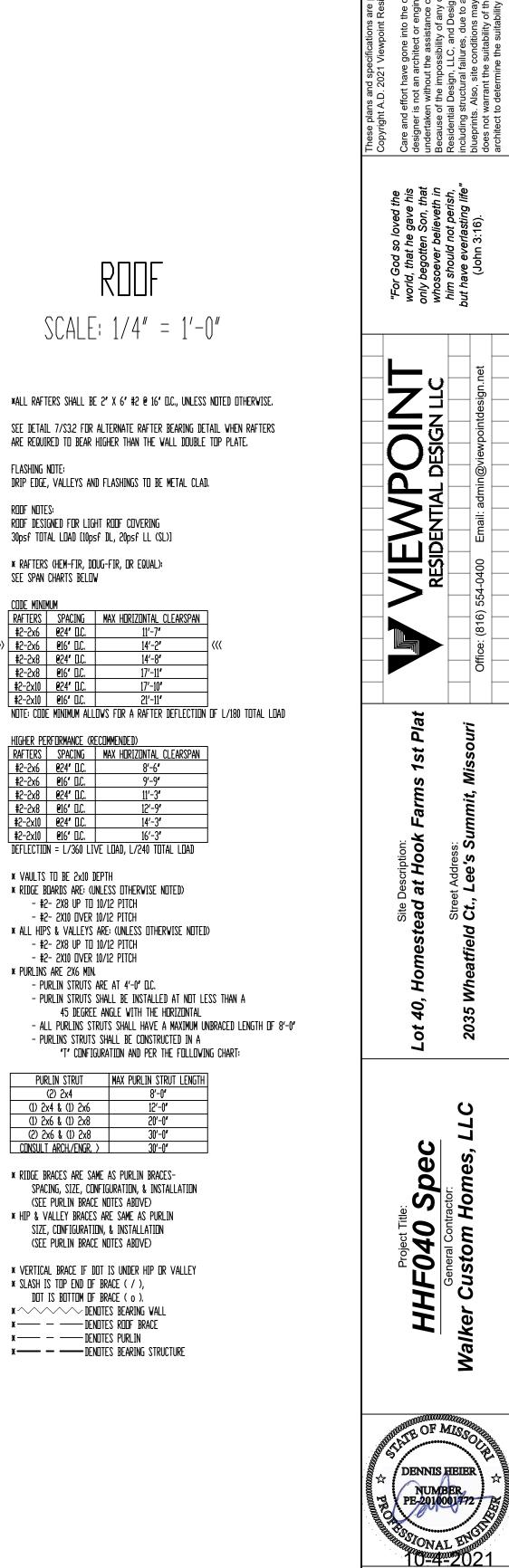
<u>HIUHEK PE</u>	<u>KFUKMANUE (KE</u>	
RAFTERS	SPACING	
#2-2x6	@24″ D.C.	
#2-2x6	@16 * D.C.	
#2-2x8	@24″ D.C.	
#2-2x8	@16″ D.C.	
#2-2x10	@24″ D.C.	
#2-2x10	@16 * D.C.	
DEFLECTIO	N = L/360 LIV	

- #2- 2X8 UP TO 10/12 PITCH - #2- 2X10 DVER 10/12 PITCH - #2- 2X8 UP TO 10/12 PITCH - #2- 2X10 OVER 10/12 PITCH * PURLINS ARE 2X6 MIN. - Purlin struts are at 4'-0" D.C.

PURLIN STRUT
(2) 2x4
(1) 2x4 & (1) 2x6
(1) 2x6 & (1) 2x8
(2) 2x6 & (1) 2x8
CONSULT ARCH./ENGR.

(see purlin brace notes above) * HIP & VALLEY BRACES ARE SAME AS PURLIN SIZE, CONFIGURATION, & INSTALLATION

* Slash is top end of brace (/), DOT IS BOTTOM OF BRACE (o). * ~~~~~ DENDITES BEARING WALL *----- DENDTES ROOF BRACE *----- Denotes purlin



Street Address: Ct., Lee's Sul

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Date: <u>9 - 22 - AD 2021</u> Rev. 1:

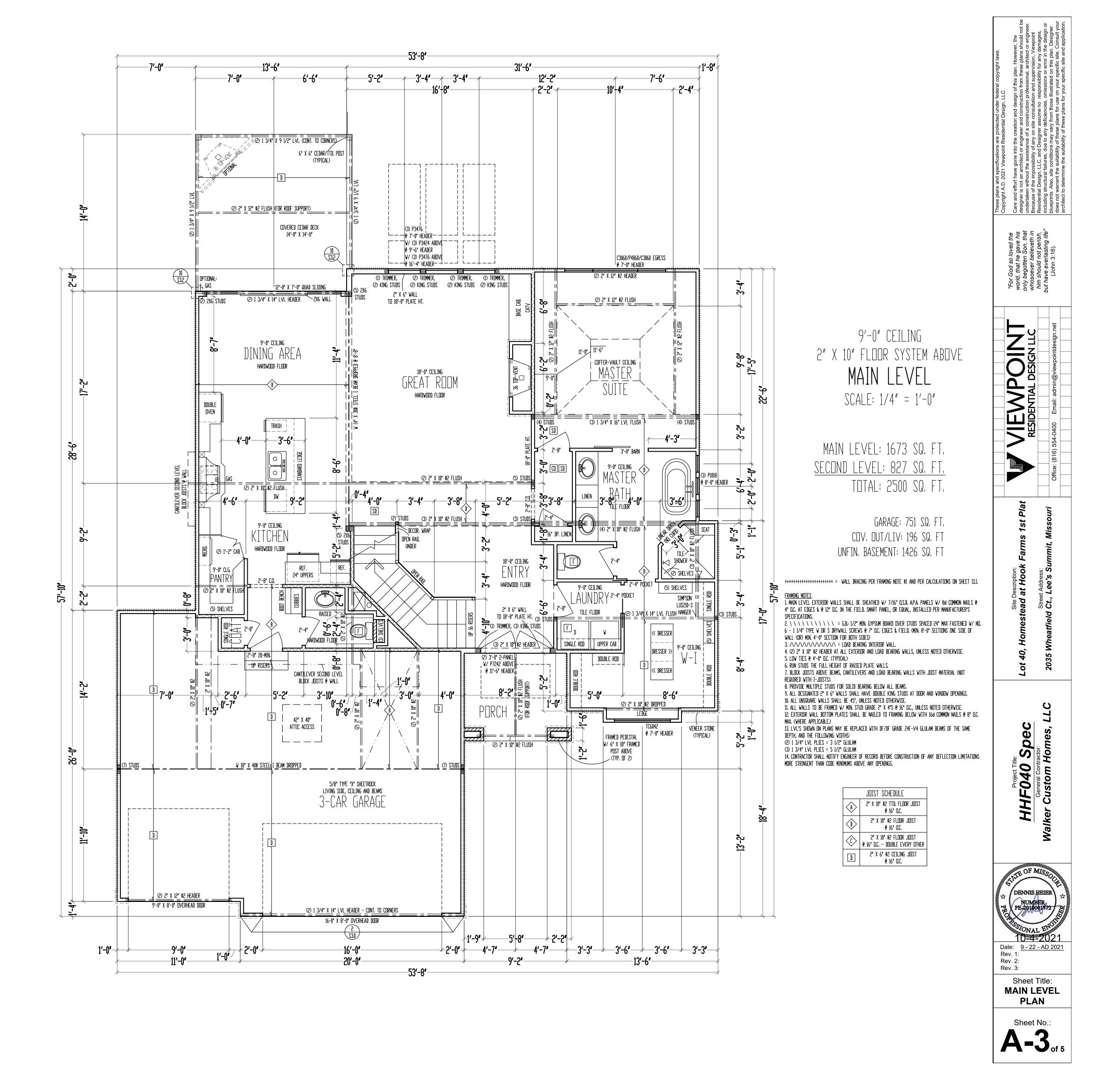
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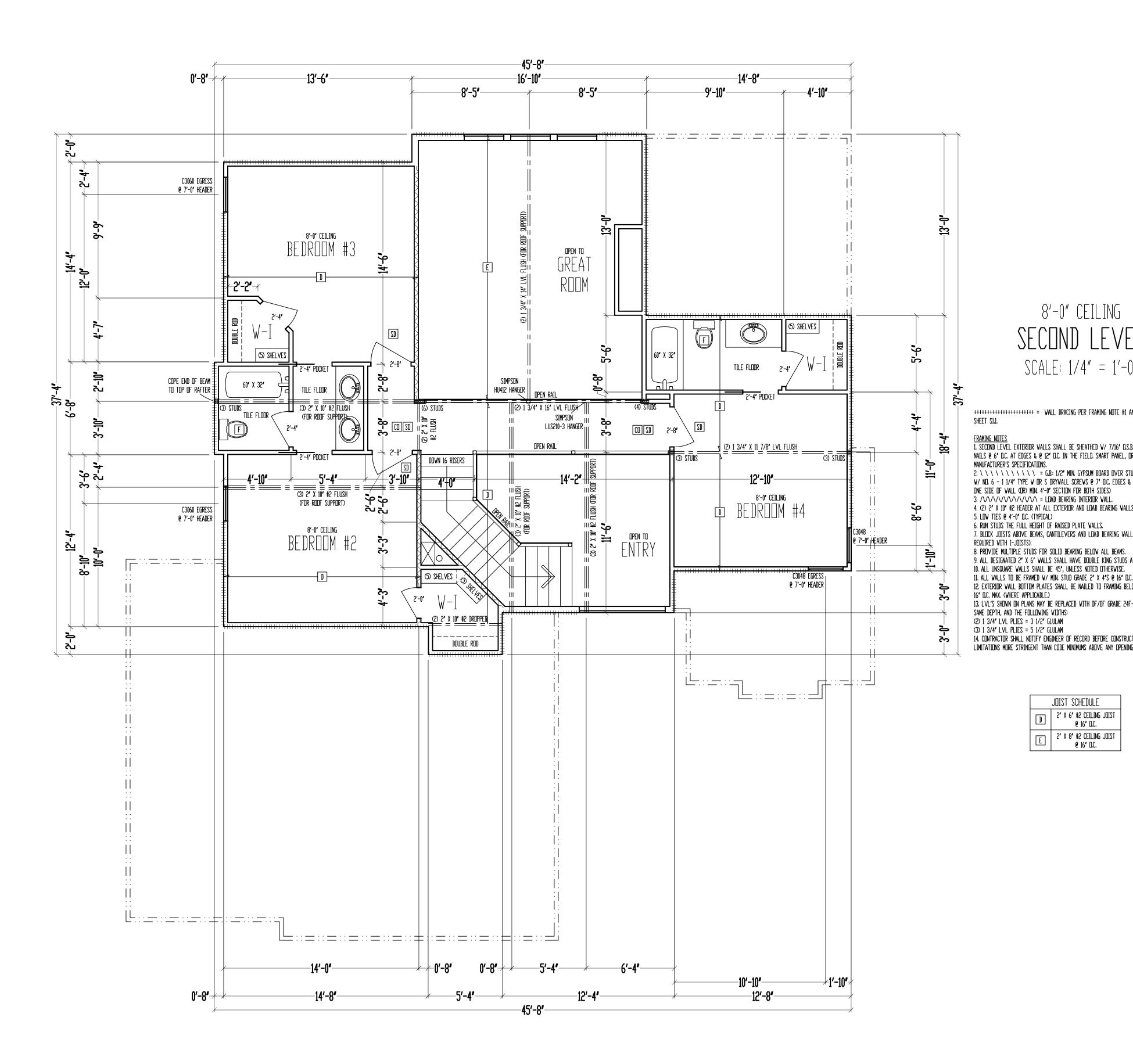
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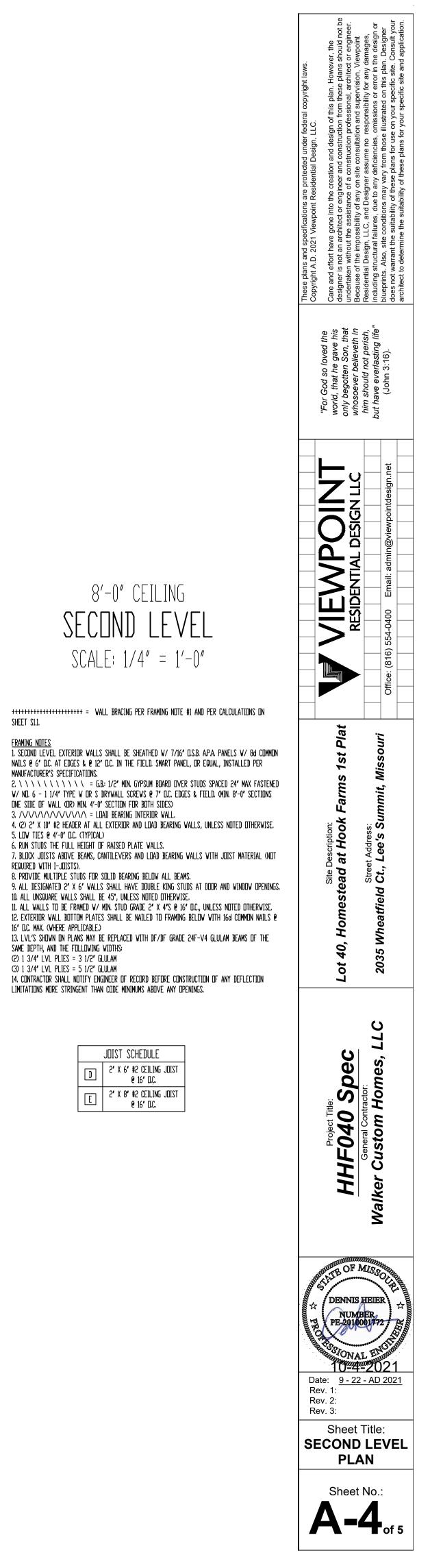
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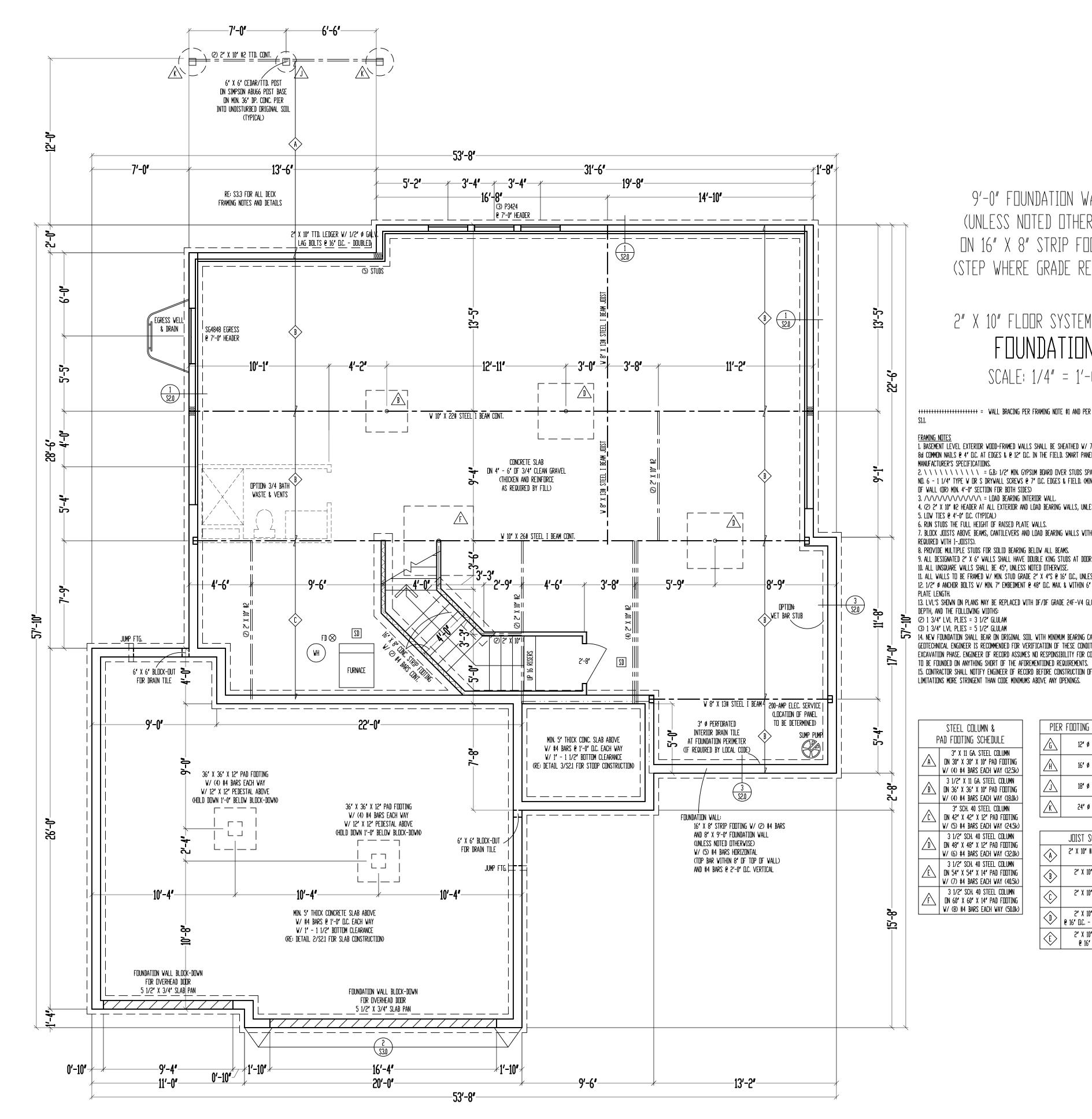


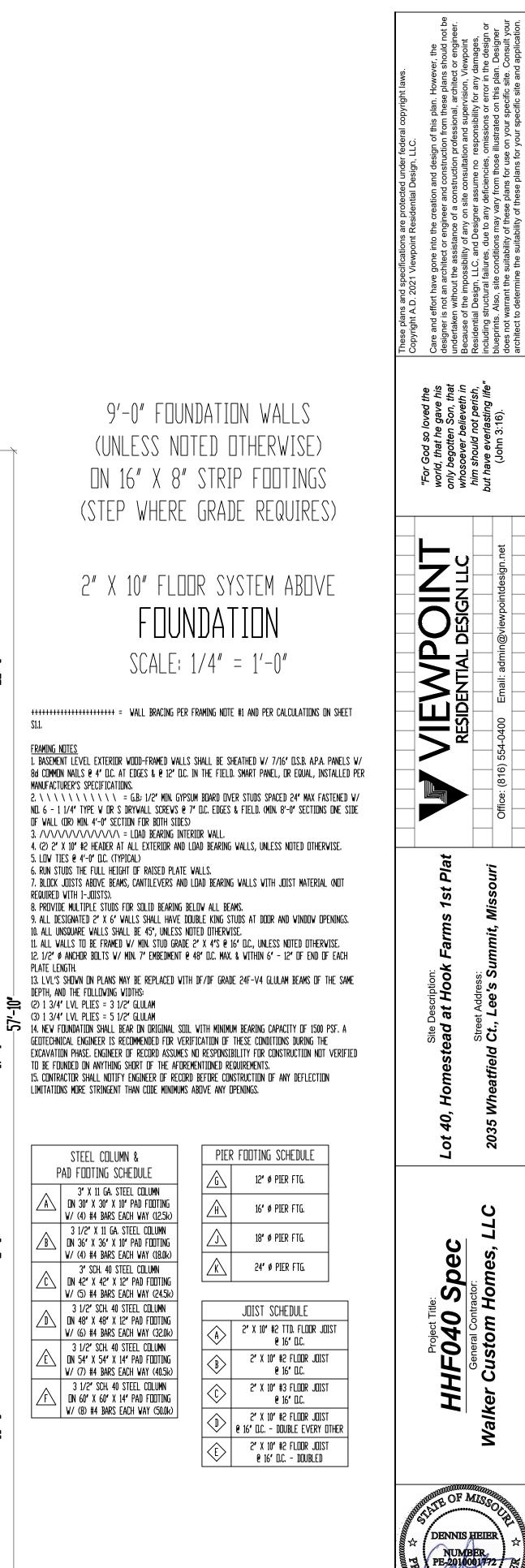
















DESCRIPTION OF BUILDING ELEN	IENTS	NUMBER AND TYP	PE OF FASTENER	SPAC	CING AND LOCATION
	I	RO	OF ¹		
BLOCKING BETWEEN JOISTS OR RAFT PLATE, TOE NAIL	ERS TO TOP	4-8d (2½"	x 0.113")		TOENAIL
CEILING JOISTS TO PLATE, TOE	NAIL	4-8d (2½"	x 0.113")	PE	R JOIST, TOENAIL
CEILING JOISTS NOT ATTACHED TO PARALLEL RAFTER, LAPS OVER PARTITIONS, FACE NAIL		4-10d (3" x 0.128")		FACE NAIL	
CEILING JOIST TO PARALLEL RAFTER (HEEL JOINT)					FACE NAIL
COLLAR TIE TO RAFTER, FACE NAIL OR 1 ¹ / ₄ " x 20 GA. RIDGE STRAP TO RAFTER RAFTER OR ROOF TRUSS TO PLATE		4-10d (3" x 0.128") 3-16d BOX NAILS (3½" x 0.135") OR 3-10d COMMON NAILS (3" x 0.148")		FACE NAIL, EACH RAFTER 2 TOE NAILS ON ONE SIDE AND 1 TOE NAIL ON OPPOSITE SIDE OF EACH RAFTER OR TRUSS	
	·	WA	LL		
STUD TO STUD (NOT AT BRACED WAL	L PANELS)	10d (3" x	0.128")	16	5" O.C. FACE NAIL
STUD TO STUD AND ABUTTING ST NTERSECTING WALL CORNERS (AT BF PANELS)		16d (3½")	x 0.135")	12	2" O.C. FACE NAIL
BUILT-UP HEADER, TWO PIECES WITH	½" SPACER	16d (3½" :	x 0.135")	12" O.C.	EACH EDGE FACE NAIL
CONTINUOUS HEADER TO ST	UD	4-8d (2½"	x 0.131")		TOENAIL
TOP PLATE TO TOP PLATE		10d (3" x	0.128")	12	2" O.C. FACE NAIL
DOUBLE TOP PLATE SPLICE	<u>=</u>	8-16d COMMON	N (3 ¹ / ₂ " x 0.162")		CH SIDE OF END JOINT (MIN. 24" GTH EACH SIDE OF END JOINT)
BOTTOM PLATE TO JOIST, RIM JOIST, E OR BLOCKING (NOT AT BRACED WAL		16d COMMON	(3 ½" x 0.162")	16	5" O.C. FACE NAIL
BOTTOM PLATE TO JOIST, RIM JOIST, E OR BLOCKING (AT BRACED WALL		3-16d BOX (3	3 ¹ / ₂ " x 0.135")	3 EAC	H 16" O.C. FACE NAIL
TOP OR SOLE PLATE TO STUD, EN	ID NAIL	4-8d BOX (2 ½" x 0.113") - T 0.135") - E		TOENAI	L, END NAIL (SEE LEFT)
TOP PLATES, LAPS AT CORNERS INTERSECTIONS	S AND	3-10d BOX (BOX (3" x 0.128") FACE NAIL		FACE NAIL
1" BRACE TO EACH STUD AND P	LATE	3-8d BOX (2	¹ / ₂ " x 0.113")		FACE NAIL
1"x6" SHEATHING TO EACH BEA	RING	3-8d BOX (2 ¹ / ₂ " x 0.113")			FACE NAIL
1"x8" SHEATHING TO EACH BEA	RING	3-8d BOX (2 ½" x 0.113") - FACE NAIL; WIDER THAN 1"x8" - 4-8d BOX (2 ½" x 0.113")			FACE NAIL
		FLO	OR		
JOIST TO SILL, TOP PLATE, OR G	IRDER	4-8d BOX (2	¹ / ₂ " x 0.113")		TOE NAIL
RIM JOIST, BAND JOIST, OR BLOCKING TOP PLATE (ROOF APPLICATIONS		8d BOX (2 ¹ / ₂ " x 0.113")			4" O.C. TOE NAIL
1" x 6" SUBFLOOR OR LESS TO EAC	,	3-8d BOX (2 ¹ / ₂ " x 0.113")			FACE NAIL
2" SUBFLOOR TO JOIST OR GIR		3-16d BOX (3 ¹ / ₂ " x 0.135")		BLI	ND AND FACE NAIL
2" PLANKS (PLAN & BEAM - FLOOR A	ND ROOF)	3-16d BOX (3 ¹ / ₂ " x 0.135")		AT EAC	H BEARING, FACE NAIL
BAND OR RIM JOIST TO JOIS	т	3-16d COMMON	ON (3 ¹ / ₂ " x 0.162") END NAIL		END NAIL
BUILT-UP GIRDERS AND BEAMS, 2-INC LAYERS	H LUMBER	10d BOX (3	10d BOX (3" x 0.128") 24" O.C. FACE NAIL AT TOP AND B		E NAIL AT TOP AND BOTTOM RED ON OPPOSITE SIDES
LEDGER STRIP SUPPORTING JOISTS C	R RAFTERS	4-16d BOX (3 ¹ / ₂ " x 0.135")		AT EACH JOIST OR RAFTER, FACE NAIL	
BRIDGING OR BLOCKING TO JO	DIST	2-10d BOX (3" x 0.128")		EA	CH END, TOENAIL
ESCRIPTION OF BUILDING MATERIALS WOOD STRUCTURAL PANELS, SUB	•	FASTNER SCHEDULE FOR PTION OF FASTENER	EDGE SPACING (INC	· · · ·	ERMEDIATE SUPPORTS (INCHES) ALL SHEATHING TO FRAMING ¹
3⁄8" - 1⁄2"		MON (2" x 0.113") NAIL WALL) 8d COMMON NAIL (ROOE)	6		12
	(ROOF)				
¹⁹ ⁄ ₃₂ " - 1"	8d COMN	· · · · ·	6		12

	(2/2 X 0.131) DEFORMED NAIL		
	OTHER WALL		
¹ / ₂ " STRUCTURAL CELLULOSIC FIBERBOARD SHEATHING	1 $\frac{1}{2}$ " GALVANIZED ROOFING NAIL, $\frac{7}{16}$ " HEAD DIAMETER, OR 1 $\frac{1}{4}$ " LONG 16 GA. STAPLE WITH $\frac{7}{16}$ " OR 1" CROWN	3	6
²⁵ ³² STRUCTURAL CELLULOSIC FIBERBOARD SHEATHING	1 $\frac{3}{4}$ " GALVANIZED ROOFING NAIL, $\frac{7}{16}$ " HEAD DIAMETER, OR 1 $\frac{1}{2}$ " LONG 16 GA. STAPLE WITH $\frac{7}{16}$ " OR 1" CROWN	3	6
½" GYPSUM SHEATHING	1½" GALVANIZED ROOFING NAIL; STAPLE GALVANIZED, 1½" LONG; 1¼" SCREWS, TYPE W OR S	7	1 7
5∕8" GYPSUM SHEATHING	1¾" GALVANIZED ROOFING NAIL; STAPLE GALVANIZED, 1%" LONG; 1%" SCREWS, TYPE W OR S	7	7

WOOD STRUCTURAL PANELS, COMBINATION SUBFLOOR UNDERLAYMENT TO FRAMING

¾" AND LESS	6d DEFORMED (2" x 0.120") NAIL OR 8d COMMON (2½" x 0.131") NAIL	6	12
7⁄8" - 1"	8d COMMON (2½" x 0.131") NAIL OR 8d DEFORMED (2½" x 0.120") NAIL	6	12
11⁄8" - 11⁄4"	10d COMMON (3" x 0.148") NAIL OR 8d DEFORMED (2½" x 0.120") NAIL	6	12

1. IF INFORMATION LISTED ON PLAN SHEETS CONTRADICTS INFORMATION IN THIS TABLE, INFORMATION ON PLANS TAKES PRECEDENCE OVER INFORMATION

FOUNDATION NOTES

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CONCRETE SHALL BE AIR-ENTRAINED BETWEEN 5%-7% WITH A MINIMUM 28-DAY COMPRESSIVE STRENGTH OF 2500 PSI FOR BASEMENT AND INTERIOR FLOOR SLABS-ON-GRADE, 3000 PSI FOR FOUNDATION WALLS, AND 3500 PSI FOR PORCHES AND GARAGE FLOOR SLABS

THE FOUNDATION DESIGN SHALL COMPLY WITH THE ENFORCING JURISDICTION'S RESIDENTIAL FOUNDATION STANDARDS

PROVIDE A MINIMUM 4"-DIAMETER PERFORATED DRAIN PIPE ALONG PERIMETER OF USABLE SPACE AT FOOTING LEVEL OR OTHER EQUIVALENT MATERIALS PER IRC SECTION R405.1. THE PIPE SHALL BE COVERED WITH A MINIMUM OF 6" OF GRAVEL OR CRUSHED ROCK. THE DRAIN SHALL DAYLIGHT BELOW FOOTING LEVEL OR TERMINATE IN A MINIMUM 20 GALLON SUMP PIT.

FOUNDATION SHALL BE DESIGNED FOR A BEARING CAPACITY OF 1500 PSF AND FOUNDED ON COMPETENT ORIGINAL SOIL AS DETERMINED AND CONFIRMED BY A LICENSED GEOTECHNICAL ENGINEER OR ENGINEERING GEOLOGIST. ENGINEER OF RECORD ASSUMES NO RESPONSIBILITY FOR CONSTRUCTION NOT VERIFIED TO BE FOUNDED ON ANY SOIL WITH THE AFOREMENTIONED MINIMUM PROPERTIES.

FOOTINGS SHALL BE A MINIMUM OF 16" WIDE x 8" DEEP AND SHALL HAVE A MINIMUM OF (2) CONTINUOUS GRADE 40 #4 BARS WITH 3" BOTTOM CLERANCE. BOTTOM OF FOOTING SHALL BE LOCATED A MINIMUM OF 3'-0" BELOW GRADE FOR FROST PROTECTION.

CONCRETE PADS SUP0PORTING COLUMN LOADS SHALL BE NO SMALLER THAN 2'-0" x 2'-0" x 1'-0" DEEP WITH A MINIMUM OF (4) GRADE 40 #4 BARS EACH WAY WITH 3" BOTTOM CLEARANCE

FOUNDATION WALLS SHALL BE A MINIMUM OF 8" NOMINAL WIDTH AND SHALL HAVE HOIZONTAL GRADE 40 #4 BARS AT 2'-0" O.C. MAX. WITH VERTICAL #4 BARS AS REQUIRED ON FOUNDATION CROSS SECTION ON SHEET S2.0 REINFORCEMENT SHALL LAP A MINIMUM OF 2'-0" (CLASS B SPLICE)

- INTERIOR BEARING WALLS AND COLUMNS SHALL BE ISOLATED FROM THE BASEMENT FLOOR SLAB
- BASEMENT FLOOR SLAB SHALL BE A MINIMUM OF 4" THICK ON A MINIMUM BASE COURSE OF 4" TO 6" OF SAND,

GRAVEL OR CRUSHED ROCK. BETWEEN THE BASE COURSE AND FLOOR SLAB SHALL BE PLACED A 6-MIL POLY VAPOR RETARDER WITH MINIMUM OVERLAP OF 6" AT DISCONTINUITIES 11. IF A FLOOR IS TO BE SUPPORTED BY A MINIMUM OF 2'-0" OF GRANULAR FILL OR 8" OF EARTH, BASEMENT SLAB

SHALL BE DESIGNED BY A LICENSED ENGINEER SILL PLATES SHALL BE ANCHORED TO THE FOUNDATION WALL WITH $\frac{1}{2}$ " Ø ANCHOR BOLTS EMBEDDED A MINIMUM OF 7" INTO CENTER OF WALL STEM AND SHALL BE INSTALLED AT A MAXIMUM OF 6'-0" O.C. (OR AS NOTED ON PLANS) AND SHALL BE INSTALLED WITHIN 6" TO 12" OF EACH END OF EACH SILL PLATE LENGTH, PER IRC SECTION R403.1.6 FOUNDATION WINDOW WELLS SHALL BE PROVIDED WITH MINIMUM DIMENSIONS AS SHOWN IN DETAIL ON SHEET 13.

14. THE GARAGE FLOOR SHALL SLOPE TOWARD THE VEHICLE DOORS OR TO A TRENCH OR UNTRAPPED DRAIN THAT DISCHARGES TO THE EXTERIOR, ABOVE GRADE

FRAMING NOTES

S2.0

ALL DIMENSIONAL LUMBER SHALL BE DOUGLAS-FIR-LARCH GRADE #2, UNLESS NOTED OTHERWISE ON PLANS ALL INTERIOR LOAD-BEARING AND EXTERIOR WALL HEADERS SHALL BE (2) #2 - 2x10's, UNLESS NOTED OTHERWISE ON PLANS

BLOCK OVER BEAMS AND AT CANTILEVERS AND DOOR JAMBS

INTERIOR NON-BEARING WALLS RESTING ON BASEMENT SLAB SHALL BE ISOLATED FROM ABOVE FRAMING BY A MINIMUM OF 3/5" ALL HEADERS/BEAMS SHALL BEAR ON A MINIMUM OF (2) 2x4 POSTS (KING AND JACK STUDS), UNLESS NOTED

OTHERWISE

20. WHERE JOISTS SPAN PARALLEL TO FOUNDATION, BLOCKING SHALL BE PROVIDED IN THE TWO SPACES MOST ADJACENT TO THE FOUNDATION WALL AT 4'-0" O.C. FOR THE PURPOSE OF TRANSFERRING LATERAL FOUNDATION WALL LOAD TO THE FLOOR DIAPHRAGM. FASTEN JOISTS AND BLOCKING TO SILL PLATE WITH (4) 10d NAILS. IF MECHANICAL DUCTWORK IS INSTALLED IN ONE OF THESE FIRST TWO BAYS, FASTEN 2x4's FLAT AT 4'-0" O.C. BETWEEN JOIST(S) AND/OR SILL AND PROVIDE BLOCKING AS PRESCRIBED ABOVE IN THE NEXT TWO JOIST BAYS. SECURE 2x4's TO JOIST(S)/SILL PLATE WITH (4) 10d NAILS. ALL WOOD MATERIAL SUPPORTED ON CONCRETE OR MASONRY SHALL BE TREATED OR OF DECAY-RESISTANT

MATERIAL JOISTS UNDER BEARING PARTITIONS ON PLANS HAVE BEEN SIZED TO SUPPORT THE DESIGN LOAD.

23. JOISTS FRAMING INTO THE FACE OF A STEEL OR WOOD BEAM SHALL BE SUPPORTED WITH APPROPRIATE COLD-FORMED STEEL JOIST HANGERS

24. JOISTS FRAMED ON TOP OF STRUCTURAL MEMBER SHALL BE SUPPORTED AT EN DS BY FULL-DEPTH SOLID BLOCKING MIN. 1// "IN THICKNESS OR BY FASTENING RIM TO JOISTS PER FASTENING TABLE TO LEFT ALL WALL COVERINGS SHALL COMPLY WITH IRC SECTION R702.3

ALL RAFTERS AND COLLAR TIES SHALL COMPLY WITH IRC SECTION R802.3.

ALL RAFTERS SHALL HAVE 2x4 COLLAR TIES @ 4'-0" O.C. IN UPPER $\frac{1}{3}$ OF VERTICAL DISTANCE BETWEEN CEILING AND ROOF

BLOCKING BETWEEN JOISTS UNDER A LOAD-BEARING WALL IS NOT REQUIRED PER IRC SECTION 501.3, BOTTOM OF ALL FLOOR ASSEMBLIES ABOVE UNFINISHED AREAS SHALL BE PROVIDED WITH A $\frac{1}{2}$ " GYPSUM BOARD MEMBRANE OR RESIDENTIAL FIRE SPRINKLER SYSTEM WHEN FLOOR SYSTEM IS CONSTRUCTED OF OTHER THAN DIMENSION LUMBER OR STRUCTURAL COMPOSITE LUMBER EQUAL TO OR GREATER THAN 2x10 NOMINAL DIMENSION(WHERE REQUIRED BY ENFORCING JURISDICTION)

30. ENGINEERED LVL's SHALL HAVE MINIMUM PROPERTIES OF Fb = 2600 psi, E=1900 ksi, AND Fv=285 psi

ENGINEERED PARALLAMS SHALL HAVE MINIMUM PROPERTIES OF Fb = 2600 psi, E = 2000 ksi, AND Fv = 290 psi COLUMN CONNECTION TO STEEL BEAMS SHALL BE WITH A CLIP POST CAP WITH ALL FOUR TAB EARS BENT AROUND THE BOTTOM FLANGE OF THE BEAM. FOR A BEARING PLATE, FOUR HOLES SHALL BE DRILLED IN THE BOTTOM FLANGE OF THE STEEL BEAM TO MATCH THE HOLE PATTERN OF THE PLATE. $\frac{1}{2}$ " x 2" BOLTS SHALL THEN BE INSTALLED WITH A FLAT WASHER, LOCK WASHER, AND A NUT IN EACH OF THE HOLES. THE POST CAP MAY BE WELDED TO THE STEEL BEAM IN ACCORDANCE WITH AWS D1.1-92 AS AN ALTERNATIVE, AND WOULD NEED TO BE INSPECTED BY AN AWS-CERTIFIED INSPECTOR.

WHEN MECHANICAL EQUIPMENT IS LOCATED IN AN ENCLOSED ROOM, THERE SHALL BE (2) 14"x12" VENTS LOCATED 33. IN A WALL COMMON WITH ADDITIONAL LIVING AREA. ONE VENT SHALL BE LOCATED SUCH THAT THE BOTTOM OF THE VENT BEGINS 12" FROM THE FLOOR AND THE OTHER VENT SHALL BE LOCATED SUCH THAT THE TOP OF THE VENT BEGINS 12" FROM THE CEILING.

34. ALL ROOF SHEATHING SHALL BE ⁷/₁₆" OSB WITH 8d COMMON NAILS @ 6" O.C. AT PANEL EDGES AND @ 12" O.C. IN FIELD

GLAZING NOTES

35. GLAZING IN HAZARDOUS LOCATIONS AS IDENTIFIED IN IRC SECTION R308.4 SHALL BE OF APPROVED SAFETY GLAZING MATERIALS. GLASS IN STORM DOORS, INDIVIDUAL FIXED OR OPENABLE PANELS ADJACENT TO A DOOR WHERE THE NEAREST VERTICAL EDGE IS WITHIN A 2'-0" ARC OF THE DOOR IN A CLOSED POSITION AND FOR WHICH THE BOTTOM EDGE IS WITHIN 5'-0" OF THE FLOOR, WALLS ENCLOSING STAIRWAYS AND LANDINGS WHERE THE GLAZING IS WITHIN 5'-0" OF THE TOP OR BOTTOM OF THE STAIR, ENCLOSURES FOR SPAS, TUBS, SHOWERS, AND WHIRLPOOLS, GLAZING IN FIXED OR OPENABLE PANELS EXCEEDING NINE SQUARE FEET AND FOR WHICH THE BOTTOM EDGE IS LESS THAN 1'-6" ABOVE THE FLOOR OR WALKING SURFACE WITHIN 3'-0" 36. ALL OPERABLE WINDOWS SHALL HAVE FALL PROTECTION PER IRC SECTION R612.2

ATTIC VENTILATION

ENCLOSED ATTICS SHALL HAVE CROSS VENTILATION FOR EACH SEPARATE SPACE BY VENTILATING OPENINGS PROTECTED AGAINST THE ENTRANCE OF RAIN OR SNOW. VENTILATING OPENINGS SHALL BE PROVIDED WITH CORROSION-RESISTANT WIRE MESH, WITH $\frac{1}{6}$ " TO $\frac{1}{4}$ " OPENINGS. THE TOTAL FREE VENTILATING AREA SHALL NOT BE LESS THAN $\frac{1}{150}$ OF THE AREA OF SPACE VENTILATED, EXCEPT WHERE THE VENTILATORS ARE LOCATED IN THE UPPER PORTION OF THE SPACE TO BE VENTILATED - THE REQUIRED AREA MAY BE REDUCED TO 1/300.

EMERGENCY EGRESS

PROVIDE A MINIMUM OF ONE WINDOW FOR EACH BEDROOM THAT HAS A MINIMUM OPENABLE AREA OF 5.7 SQUARE FEET WITH A MINIMUM OPENABLE HEIGHT OF 2'-0" AND A MINIMUM WIDTH OF 1'-9". IN ADDITION, THE OPENABLE PORTION OF EGRESS WINDOWS SHALL NOT EXCEED 3'-8" ABOVE THE ADJOINING FLOOR OR PERMANENT STEP. PROVIDE SMOKE ALARMS IN EACH SLEEPING ROOM, OUTSIDE OF EACH SLEEPING AREA AND ON EACH FLOOR, 39. INCLUDING BASEMENT (IF APPLICABLE). ALARMS SHALL BE HARDWIRED TOGETHER SO THAT THE ACTIVATION OF ONE SMOKE ALARM WILL ACTIVATE ALL SMOKE ALARMS IN THE DWELLING. PROVIDE CARBON MONOXIDE DETECTORS OUTSIDE EACH SLEEPING AREA.

MASONRY VENEER

40. MASONRY VENEER SHALL BE ANCHORED TO THE SUPPORTING WALL STUDS WITH CORROSION-RESISTANT METAL TIES EMBEDDED IN MORTAR OR GROUT AND EXTENDING INTO THE VENEER A MINIMUM OF 1¹/₂", WITH NOT LESS THAN $\frac{5}{8}$ " MORTAR OR GROUT COVER TO OUTSIDE FACE.

- VENEER TIES, IF STRAND WIRE, SHALL NOT BE LESS IN THICKNESS THAN NO. 9 U.S. GAGE WIRE AND SHALL HAVE A 41. HOOK EMBEDDED IN THE MORTAR JOINT, OR IF SHEET METAL, SHALL BE NOT LESS THAN NO. 22 U.S. GAGE BY 7/8" CORRUGATED
- 42. EACH TIE SHALL SUPPORT NOT MORE THAN 2.67 SQUARE FEET OF WALL AREA AND SHALL BE SPACED NOT MORE THAN 32 INCHES ON CENTER HORIZONTALLY AND 24 INCHES ON CENTER VERTICALLY.
- VENEER TIES AROUND WALL OPENINGS: ADDITIONAL METAL TIES SHALL BE PROVIDED AROUND ALL WALL 43. OPENINGS GREATER THAN 16 INCHES IN EITHER DIMENSION. METAL TIES AROUND THE PERIMETER OF OPENINGS SHALL BE SPACED NOT MORE THAN 3 FEET ON CENTER AND PLACED WITHIN 12 INCHES OF THE WALL OPENING.

GARAGE NOTES

- DOOR(S) BETWEEN THE GARAGE AND DWELLING SHALL BE MINIMUM 1%" SOLID CORE OR HONEY-COMBED STEEL DOOR WITH 20-MINUTE FIRE RATING EQUIPPED WITH A SELF-CLOSING DEVICE 45. VEHICLE DOORS AND FRAMES SHALL BE DESIGNED AND INSTALLED TO MEET THE 115-MPH 3-SECOND GUST
- LOADING PER DASMA 108 AND ASTM E 330-96 PER IRC 2018

GARAGE NOTES (CONTINUED)

- THE GARAGE SHALL BE SEPARATED FROM THE DWELL MINIMUM 5/8" GYP. BOARD APPLIED TO THE GARAGE SID HABITABLE SPACE OCCURS ABOVE THE GARAGE, THE SHALL BE PROTECTED WITH A MINIMUM 5/11 TYPE X GYP FLOOR/CEILING SPACE IS PROVIDED ABOVE THE GARA
- SUPPORTING THE SEPARATION SHALL ALSO BE PROTE 45 GARAGE DOOR H-FRAME FOR THE ATTACHMENT OF TH BALANCE SHALL CONSIST OF THE FOLLOWING: 2x6 VEF FLOOR TO CEILING AND SHALL BE FASTENED WITH 2¹/₂" STAGGERED WITH (7) 3¹/₄" x 0.120" NAILS THROUGH THE MINIMUM 2x8 HEADER FOR ATTACHMENT OF COUNTER

DESIGN LOADING (PER TABLE R301.5)

MINIMUM UNIFORMLY DISTRIB		ADS (PSF)
USE	LIVE LOAD	DEAD LOAD
UNINHABITABLE ATTICS WITHOUT STORAGE	10	10
UNINHABITABLE ATTICS WITH LIMITED STORAGE	20	10
HABITABLE ATTICS AND ATTICS SERVED WITH FIXED STAIRS	30	10
BALCONIES (EXTERIOR) AND DECKS	40	10 ^d
FIRE ESCAPES	40	10
GUARDRAILS AND HANDRAILS ^a	200 [°]	-
GUARDRAIL IN-FILL COMPONENTS ^b	50 [°]	-
PASSENGER VEHICLE GARAGES	50	DEPENDENT UPON SLAB CONSTRUCTION
ROOMS OTHER THAN SLEEPING ROOM	40	10 ^d
SLEEPING ROOM	30	10 ^d
STAIRS	40	10 ^d

a. A single concentrated load applied in any direction at any point along the top. b. Guard in-fill components (all those except the handrail), ballusters and panel fillers shall be designed to withstand a horizontally applied normal load of 50 pounds on an area equal to one square foot. This load need not be assumed to act concurrently with any other live load requirement. c. Glazing used in handrail assemblies and guards shall be designed with a safety factor of 4. The safety factor shall be applied to each of the concentrated loads applied to the top of the rail, and to the load on the infill components. These loads shall be determined independently of one another, and loads are assumed not to occur with any other live load. d. An additional dead loading of 10 psf shall be applied where thinset tile floor is to be installed. An additional dead loading of 50 psf shall be applied where mudset tile floor is to be installed. INSULATION/EFFICIENCY BUILDING ENVELOPE INSULATION SHALL COMPLY WITH IRC TABLE N1102.1.1 OR THE 2012 IECC (SEE SHEET S3.1 FOR FRAMING DETAILS AND TABLES ON THIS SHEET FOR MORE INFORMATION) CATHEDRAL -VAULTED CEILING FRAMING SHALL BE FRAMED WITH A MINIMUM INSULATION VALUE OF R-38. IF VAULTED RAFTERS DO NOT PROVIDE REQUIRED DEPTH TO ACHIEVE R-38 INSULATION BUILDER SHALL FUR DOWN RAFTERS PER DETAILS PROVIDED ON SHEET S3.1. INSULATION AND FENESTRATION REQUIREMENTS BY COMPONENT (TABLE N1102.1.1)

CLIMATE ZONE	4-A
FENESTRATION U-FACTOR	0.35
SKYLIGHT U-FACTOR	0.55
GLAZED FENSTRATION SHGC	0.40
CEILING R-VALUE	49
WOOD FRAME WALL R-VALUE	15
MASS WALL R-VALUE	8 / 13
FLOOR R-VALUE	19
BASEMENT WALL R-VALUE	10-CONTINUOUS OR 13-CAVITY
SLAB R-VALUE AND DEPTH	10 AT 2'-0"
CRAWL SPACE WALL R-VALUE	10-CONTINUOUS OR 13-CAVITY
DUCTWORK EXPOSED TO OUTSIDE AIR R-VALUE	8
DUCTWORK NOT EXPOSED TO OUTSIDE AIR R-VALUE	6
CATHEDRAL VAULTED CEILING R-VALUE	38

DUCT SEALING

N1103.2.2 (R403.2.2) SEALING (MANDATORY). DUCTS, AIR HANDLERS, AND FILTER BOXES SHALL BE SEALED. JOINTS AND SEAMS SHALL COMPLY WITH SECTION M1601.4.1 OF 2018 IRC. **EXCEPTIONS:** AIR-IMPERMEABLE SPRAY FOAM PRODUCTS SHALL BE PERMITTED TO BE APPLIED 1. WITHOUT ADDITIONAL JOINT SEALS.

- WHERE A DUCT CONNECTION IS MADE THAT IS PARTIALLY INACCESSIBLE, THREE SCREWS OR RIVETS SHALL BE EQUALLY SPACED ON THE EXPOSED PORTION OF THE JOINT SO AS TO PREVENT A HINGE EFFECT.
- CONTINUOUSLY WELDED AND LOCKING-TYPE LONGITUDINAL JOINTS AND SEAMS IN DUCTS OPERATING AT STATIC PRESSURES LESS THAN 2 INCHES OF WATER COLUMN PRESSURE CLASSIFICATION SHALL NOT REQUIRE ADDITIONAL CLOSURE SYSTEMS.
- DUCT TIGHTNESS SHALL BE VERIFIED BY EITHER OF THE FOLLOWING: POST-CONSTRUCTION TEST: TOTAL LEAKAGE SHALL BE LESS THAN OR EQUAL TO 4 CFM 1. PER 100 SQUARE FEET OF CONDITIONED FLOOR AREA WHEN TESTED AT A PRESSURE DIFFERENTIAL OF 0.1 INCHES W.G. ACROSS THE ENTIRE SYSTEM, INCLUDING THE MANUFACTURER'S AIR HANDLER ENCLOSURE. ALL REGISTER BOOTS SHALL BE TAPED
- OR OTHERWISE SEALED DURING THE TEST. ROUGH-IN TEST: TOTAL LEAKAGE SHALL BE LESS THAN OR EQUAL TO 4 CFM PER 100 SQUARE FEET OF CONDITIONED FLOOR AREA WHEN TESTED AT A PRESSURE DIFFERENTIAL OF 0.1 INCHES W.G. ACROSS THE SYSTEM, INCLUDING THE MANUFACTURER'S AIR HANDLER ENCLOSURE. ALL REGISTERS SHALL BE TAPED OR OTHERWISE SEALED DURING THE TEST. IF THE AIR HANDLER IS NOT INSTALLED AT THE

TIME OF THE TEST, TOTAL LEAKAGE SHALL BE LESS THAN OR EQUAL TO 3 CFM PER 100 SQUARE FEET OF CONDITIONED FLOOR AREA. **EXCEPTION:** THE TOTAL LEAKAGE TEST IS NOT REQUIRED FOR DUCTS AND AIR HANDLERS LOCATED ENTIRELY WITHIN THE BUILDING THERMAL ENVELOPE.

ME	ECHANICAL VENTILATIO	
FAN LOCATION	AIR FLOW RATE	MINIMUM EFFICA
TAN ECCATION	MINIMUM (CFM)	(CFM/WATT)
RANGE HOODS	ANY	2.8
IN-LINE FAN	ANY	2.8
BATHROOM, UTILITY ROOM	10	1.4
BATHROOM, UTILITY ROOM	90	2.8

ING AND ITS ATTIC AREAS BY
DE OF FRAMING. WHERE
GARAGE CEILING ASSEMBLY
P. BOARD. WHERE A
AGE COLUMNS AND BEAMS
ECTED WITH 5⁄8" GYP. BOARD.
HE TRACK AND COUNTER
RTICAL JAMBS RUNNING FROM
"" x 0.120" NAILS AT 7" O.C.
E JAMB INTO THE HEADER.
R BALANCE SYSTEM.

AIR FLOW RATE

MAXIMUM (CFM)

ANY

ANY

90

ANY

38

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DATE REVISION DRAWING TITLE STRUCTURAL NOTES ENGINEER: DMH CHECKED BY: DMH JOB NO. 3920 | DRAWN BY: DMH DATE: 10-04-21 SHEET NUMBER

10/06/2021

RESIDENTIAL SEISMIC & WIND ANALYSIS

DETERMINE WEIGH							
LOCATION	ATION					AREA (ft ²)	WE
ROOF		· · · · ·			10	2692	
CEILING		i i	ł		10	2692	
SECOND FLOOR					10	827	
FIRST FLOOR					10	2692	
				WALL LENGTH (ft)	WALL HEIGHT (ft)	WALL UNIT WT. (psf)	WE
SECOND FLOOR EX	T. WALL DL			166	8	8	
FIRST FLOOR EXT. V	VALL DL			223	10	10	
					DEAD LOAD (psf)	AREA (ft2)	WE
SECOND FLOOR INT	. PARTITION WALL DL				6	827	
FIRST FLOOR INT. PARTITION WALL DL							
FIRST FLOOR INT. P	ARTITION WALL DL				6	2692	
FIRST FLOOR INT. F							
FIRST FLOOR INT. P	PROJ		DESIGN PER 115 MPH		RE C AND MEAN ROOF HEIGHT <	= 30 FT ASSUMED)	
	PROJ	ECTED AREAS (WIND I TO-BACK	DESIGN PER 115 MPH	3-SECOND GUST, EXPOSU		= 30 FT ASSUMED)	
	PROJ		DESIGN PER 115 MPH	-SECOND GUST, EXPOSU	RE C AND MEAN ROOF HEIGHT <	= 30 FT ASSUMED)	
SLOPED ROOF	PROJ FRONT-	TO-BACK	DESIGN PER 115 MPH	SLOPED ROOF	RE C AND MEAN ROOF HEIGHT < SIDE-TO	= 30 FT ASSUMED) SIDE	
	PROJ FRONT- AREA	TO-BACK LOAD	DESIGN PER 115 MPH		RE C AND MEAN ROOF HEIGHT < SIDE-TO AREA	= 30 FT ASSUMED) -SIDE LOAD	CU
SLOPED ROOF	PROJ FRONT- AREA 213	TO-BACK LOAD 938		SLOPED ROOF	RE C AND MEAN ROOF HEIGHT < SIDE-TO AREA 291	= 30 FT ASSUMED) SIDE LOAD 1261	CU
SLOPED ROOF VERT. ROOF	PROJ FRONT- AREA 213 0	TO-BACK LOAD 938 0	CUMULATIVE	SLOPED ROOF VERT. ROOF	RE C AND MEAN ROOF HEIGHT < SIDE-TO AREA 291 0	= 30 FT ASSUMED) SIDE LOAD 1261 0	CU
SLOPED ROOF VERT. ROOF 2ND	PROJ FRONT- AREA 213 0 411.03	TO-BACK LOAD 938 0 5897	CUMULATIVE 6835	SLOPED ROOF VERT. ROOF 2ND	RE C AND MEAN ROOF HEIGHT < SIDE-TO AREA 291 0 335.97	= 30 FT ASSUMED) -SIDE LOAD 1261 0 5024	CU
SLOPED ROOF VERT. ROOF 2ND 1ST	PROJ FRONT- AREA 213 0 411.03 590.37	TO-BACK LOAD 938 0 5897 8232	CUMULATIVE 6835 15067 0	SLOPED ROOF VERT. ROOF 2ND 1ST	RE C AND MEAN ROOF HEIGHT < SIDE-TO AREA 291 0 335.97 636.13	= 30 FT ASSUMED) -SIDE LOAD 1261 0 5024 8765	CU
SLOPED ROOF VERT. ROOF 2ND 1ST	PROJ FRONT- AREA 213 0 411.03 590.37	TO-BACK LOAD 938 0 5897 8232	CUMULATIVE 6835 15067 0	SLOPED ROOF VERT. ROOF 2ND 1ST BSMT ^a	RE C AND MEAN ROOF HEIGHT < SIDE-TO AREA 291 0 335.97 636.13	= 30 FT ASSUMED) -SIDE LOAD 1261 0 5024 8765	
SLOPED ROOF VERT. ROOF 2ND 1ST	PROJ FRONT- AREA 213 0 411.03 590.37 0	TO-BACK LOAD 938 0 5897 8232 0	CUMULATIVE 6835 15067 0 PRESSURE (PSI	SLOPED ROOF VERT. ROOF 2ND 1ST BSMT ^a -) - PER ASCE CH. 6	RE C AND MEAN ROOF HEIGHT < SIDE-TO AREA 291 0 335.97 636.13 157	= 30 FT ASSUMED) -SIDE LOAD 1261 0 5024 8765 2732	CU 2a (FIG.

a) If there is a walkout wall to be sheathed, determine tributary wind area and enter here. If no walkout, enter 0 for area. q_{z10_ASD}=0.6q_{z10} (Design Velocity Pressure for ASD analysis under ASCE7-10 and IRC/IBC 2012) q_{z10}=0.00256K_zK_{zt}K_dV² (ASCE7-10 Velocity Pressure)

2ND FLOOR TRIBUTARY WEIGHT 1ST FLOOR TRIBUTARY WEIGHT

BASEMENT TRIBUTARY WEIGHT S_s (SITE GROUND MOTION - %g - FROM ASCE7 SEISMIC MAP)

F_a (from ASCE7 Table 11.4-1)

S_{DS} (= 2/3 * S_S * F_a)

R (from ASCE7 Table 12.2-1)

53.67

57.83

2ND FLOOR
1ST FLOOR
BASEMENT

N OR DR NT		From P	ASCE7 (Eq. 12.8-1):	V (= 1.2 * S _{DS} * W / R) (lbs.) 1398 2099 2099
Sheathing Location	Min. Sheathing Schedule	Fastening Schedule	Allowable Sh	ear (#/LF) Code Refe
Exterior (Option #1)	7/16" APA Rated Plywood/OSB	1-1/2" 16ga. Staples w/ 1" penetration@ 6" OC Edges, 6" OC Field For 24" stud spacing, 12" OC Field For 16" stud spacing	155	per IBC, T 2306.3(1)
Exterior <u>(Option #2)</u>	7/16" APA Rated Plywood/OSB	1-1/2" 16ga. Staples w/ 1" penetration@ 4" OC Edges, 6" OC Field For 24" stud spacing, 12" OC Field For 16" stud spacing	230	per IBC, T 2305.3(1)
Exterior (Option #3)	7/16" APA Rated Plywood/OSB	1-1/2" 16ga. Staples w/ 1" penetration@ 3" OC Edges, 6" OC Field For 24" stud spacing, 12" OC Field For 16" stud spacing	310	per IBC, 1 2305.3(1)
Exterior <u>(Option #4)</u>	7/16" APA Rated Plywood/OSB or shiplap panel sheathing, or 3/8" shiplap panel sheathing with tighter nail spacing	8d Common Nails w/ 1-3/8" penetration @ 6" O.C. Edges, 12" O.C. Field for 7/16" APA-rated plywood/OSB or shiplap panel sheathing OR @ 4" O.C. Edges, 12" O.C. Field for 3/8" shiplap panel sheathing	220	AF&PA SI Table 4.3/
Exterior <u>(Option #5)</u>	7/16" APA Rated Plywood/OSB or shiplap panel sheathing, or 3/8" shiplap panel sheathing with tighter nail spacing	8d Common Nails w/ 1-3/8" penetration @ 4" O.C. Edges, 12" O.C. Field for 7/16" APA-rated plywood/OSB or shiplap panel sheathing OR @ 3" O.C. Edges, 12" O.C. Field for 3/8" shiplap panel sheathing	320	AF&PA SI Table 4.3/
Exterior <u>(Option #6)</u>	7/16" APA Rated Plywood/OSB or shiplap panel sheathing, or 3/8" shiplap panel sheathing with tighter nail spacing and double studs at each panel edge	8d Common Nails w/ 1-3/8" penetration @ 3" O.C. Edges, 12" O.C. Field	410	AF&PA SI Table 4.3/
Interior	1/2" Gypsum Board	No. 6- 1 ¹ / ₄ " Type W or S Screws @ 8" O.C. Edges, 12" O.C. Field	60	per IBC, T 2306.4.4
Interior	16 Ga. Simpson/USP Type WB Steel X-Brace (or equal)	(3) 16d @ end studs & (1) 8d @ intermediate studs (per manufacturer specifications - see detail on sheet S3)	325	;

EXTERIOR SHEATHING OPTION FOR SECOND FLOOR	4
EXTERIOR SHEATHING OPTION FOR FIRST FLOOR	5
EXTERIOR SHEATHING OPTION FOR BASEMENT WALLS	5

					BACK WALL OF GARAGE (FT.)	0		
					GAR. WALL: 1=F-B, 2=S-S	2		
I								
				IOR STRUCTURAL WALL	LENGTHS (ft.) & RESISTANCES			
		SE	ISMIC			WIND		
	FRONT-TO-BACK	RESISTANCE (lbs.)	SIDE-TO-SIDE	RESISTANCE (lbs.)	FRONT-TO-BACK	RESISTANCE (lbs.)	SIDE-TO-SIDE	RESISTANCE (lbs.)
2ND FLOOR	30	8400	42	11760	30 11760		42	16464
1ST FLOOR	70	26600	30	11400	70 37240		30	15960
BASEMENT	0	0	24.5	9310	0	0	24.5	13034
			:	· · ·			· · · ·	
		ADDITIONAL RESIS	TANCE REQUIRED		Anchor Bolt Spacing (in.)		16d Nail Spacing req'd at bottom plate (in.)	
		SEISMIC	WIND		diameter (in.)	0.5	2nd Floor F-B	46
2ND FLOOR FRONT-1	TO-BACK	0	0		Shear value (per NDS)	944	2nd Floor S-S	46
2ND FLOOR SIDE-TO-SIDE		0	0		Spacing F-B (inches)	139.1	1st Floor F-B	21
1ST FLOOR FRONT-TO-BACK		0	0		spacing S-S (inches)	129.3	1st Floor S-S	19
1ST FLOOR SIDE-TO-SIDE		0	0					
BASEMENT FRONT-T	O-BACK	0	0					
BASEMENT SIDE-TO-	-SIDE	0	0					

WIDTH OF 1ST STORY (FT.)

DEPTH OF 1ST STORY (FT.)

	RESISTANCE REQUIRED IN ADDITION TO RESISTANCE PROVIDED BY EXTERIOR WALLS**								
	ADDITIONAL RESISTANCE REQUIRED (POUNDS)	PORTAL FRAMES OR PERF. SHEAR WALL RESISTANCE	INTERIOR X-BRACES (325#/BRACE)	INTERIOR WALL LENGTH W/ 1/2" GYPSUM BOARD PER TABLE (FT.)	INT. WALL LENGTH SHEATHED W/ OSB (TOTAL LENGTH, ONE SIDE, FT.)	RESISTANCE PROVIDED BY ADDITIONAL METHODS (POUNDS)	OK?		
2ND FLOOR FRONT-TO-BACK	0					0	YES		
2ND FLOOR SIDE-TO-SIDE	0					0	YES		
1ST FLOOR FRONT-TO-BACK	0					0	YES		
1ST FLOOR SIDE-TO-SIDE	0					0	YES		
BASEMENT FRONT-TO-BACK	0					0	YES		
BASEMENT SIDE-TO-SIDE	0					0	YES		

**NOTES: 1) SEE ATTACHED CALCULATIONS FOR PORTAL FRAME OR PERFORATED SHEAR WALL RESISTANCE CAPACITIES (IF APPLICABLE), 2) SEE SHEET S1 FOR INTERIOR STEEL X-BRACE INSTALLATION, 3) INTERIOR WALLS SHEATHED WITH OSB SHALL BE ATTACHED WITH SAME STAPLE/NAILING

PATTERN AS EXTERIOR OSB ON SAME FLOOR (SEE TABLE ABOVE) AND ARE ONLY APPLICABLE FOR FULL-HEIGHT SECTIONS OF 2'-8" OR LONGER ALL LATERAL BRACING ACHIEVED AT EXTERIOR WALLS AND WALLS DIRECTLY ON FOUNDATIONS; THEREFORE, NO INTERIOR BRACING PER 2012 IRC SECTION R502.2.1 IS REQUIRED

**INSIDE EXTERIOR \		RESISTANCE DUE TO DEAD	•	,	251.6	OPLIFTOR				
*ALONG PERIMETER TOTAL UPLIFT PER LINEAL FOOT ALONG EXTERIOR (POUNDS) 190.1 UPLIFT OK										
MAIN ROOF**	3103.7361	1320.968976	1782.767124	15.12	10.5	38692				
	TOTAL AREA (FT ²)	ZONE E AREA (FT ²)	ZONE G AREA (FT ²)	PRESSURE ZN. E (PSF)	PRESSURE ZN. G (PSF)	TOTAL FORCE (LBS)	FORCE			
OVERHANG	1	16.56	225	16.56						
	LENGTH (FT.)	PRESSURE (PSF)	LINEAL FT. OF OH	UPLIFT PER FT* (LBS)						
		ASCE 7			_					
ROOF PITCH (MAX)	5	22.6	PITCH OF 6 OR LESS:	EOH -13.3, E -7.2, G -5.2						
	X/12	DEGREES								
	WIND UPLIFT ANALYSIS									

NOTE FOR CONSTRUCTION:

THE CONTINUOUS STRUCTURAL PANEL SHEATHING BRACING METHOD REQUIRES USE OF THE ABOVE TABLE FOR SHEATHING OF THE ENTIRE STRUCTURE. IN ADDITION, FRAMING MEMBERS SHALL BE @ 16" O.C. MAX., UNBLOCKED, AND W/ SHEATHING APPLIED DIRECTLY TO FRAMING MEMBERS

NOTE FOR DESIGN:

ALL WALLS USED IN THE CALCULATION OF THE RESISTANCE FOR THIS STRUCTURE SHALL HAVE A MINIMUM UNINTERRUPTED HEIGHT OF 8'-0" AND LENGTH OF 2'-8". ALLOWABLE RESISTANCES HAVE BEEN #/FT AND INCREASED BY 40% FOR WIND LOADS, PER VALUES IN 2012 IBC SECTION 2306 AND AF&PA SDPWS TABLE 4.3A. FOR EXAMPLE, 7/16" APA-RATED SHEATHING WITH 8d @ 6" & 12" HAS A SEISMIC SHEAR VALUE OF 240 A WIND SHEAR VALUE OF 335#/FT - 40% GREATER THAN THAT OF SEISMIC)

NOTE: SOIL SITE CLASS ASSUMED TO BE CLASS D. IF SITE CONDITIONS ARE DETERMINED TO BE CLASS E OR F, CONSULT ENGINEER BEFORE PROCEEDING

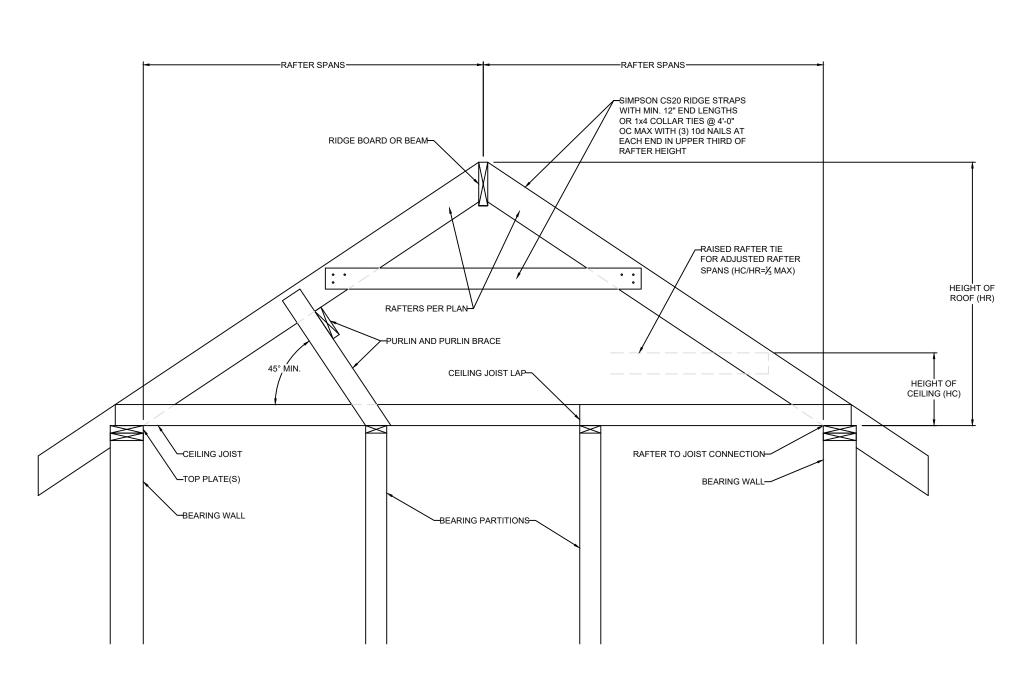
WITH CONSTRUCTION

INPUT
CULATED VALUE
VEIGHT (lbs.)
26920
26920
8270
26920
VEIGHT (lbs)
10624
22300
VEIGHT (lbs)
4962
16152
6285
15049
10256
G. 28.6-1, ASCE7)
10.734

59152 88846 88846 12.0% 1.6 0.128 6.5

WIDTH OF 2ND STORY (FT.)	45.67
DEPTH OF 2ND STORY (FT.)	37.33

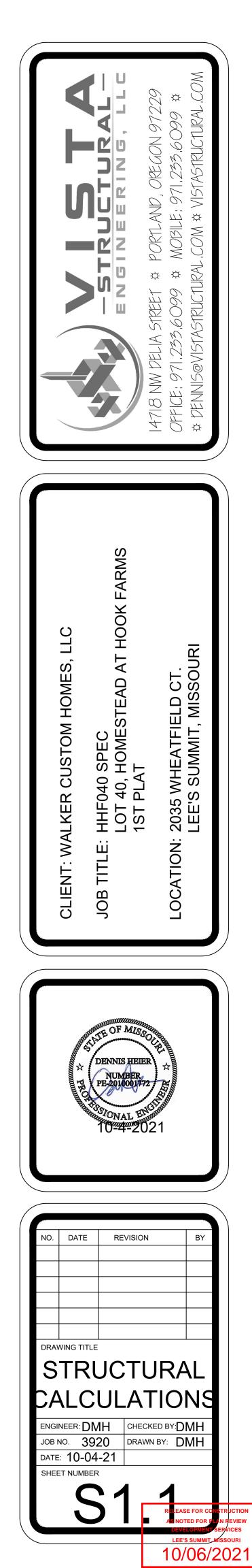
CE PER LINEAL FT @ PERIMETER (LBS) 173.5

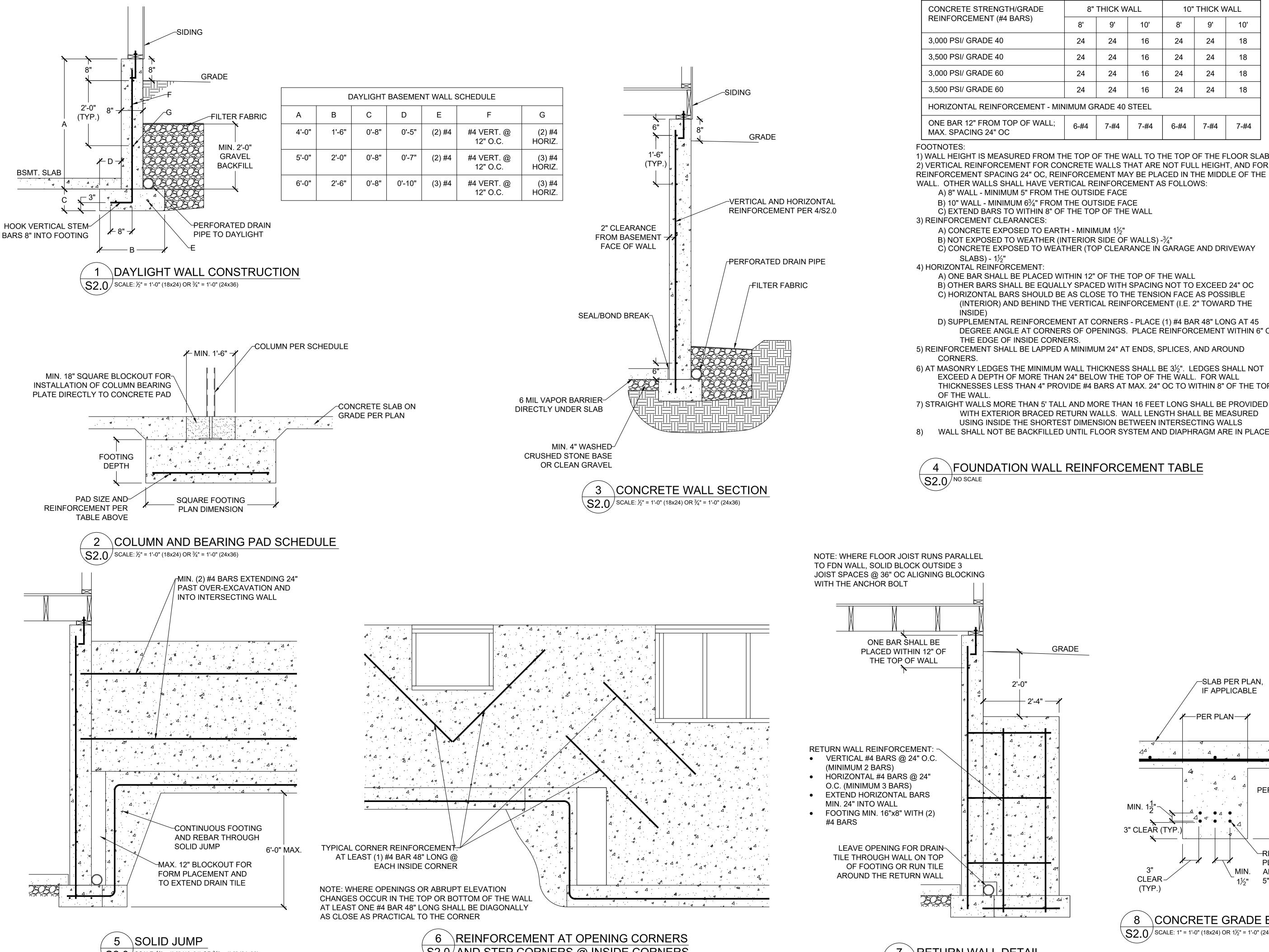


 BRACED RAFTER CONSTRUCTION

 \$1.1

 SCALE: 1" = 1'-0" (18x24) OR 1½" = 1'-0" (24x36)

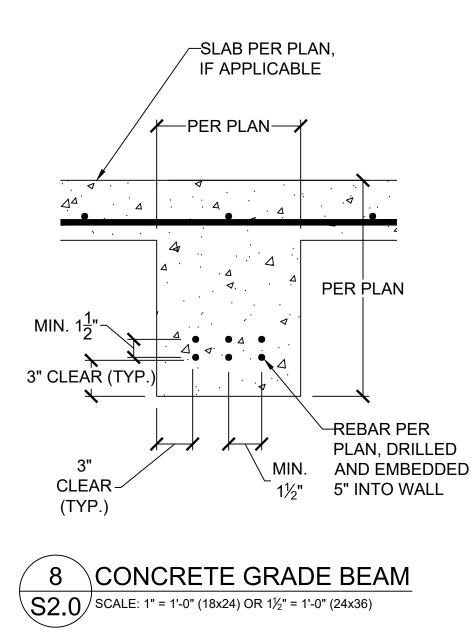




 $\overline{S2.0}$ SCALE: $\frac{1}{2}$ " = 1'-0" (18x24) OR $\frac{3}{4}$ " = 1'-0" (24x36)

S2.0/AND STEP CORNERS @ INSIDE CORNERS SCALE: ¹/₂" = 1'-0" (18x24) OR ³/₄" = 1'-0" (24x36)

ackslashRETURN WALL DETAIL SCALE: $\frac{1}{2}$ " = 1'-0" (18x24) OR $\frac{3}{4}$ " = 1'-0" (24x36) GRADE



AND EMBEDDED

WITH EXTERIOR BRACED RETURN WALLS. WALL LENGTH SHALL BE MEASURED USING INSIDE THE SHORTEST DIMENSION BETWEEN INTERSECTING WALLS WALL SHALL NOT BE BACKFILLED UNTIL FLOOR SYSTEM AND DIAPHRAGM ARE IN PLACE

6) AT MASONRY LEDGES THE MINIMUM WALL THICKNESS SHALL BE $3\frac{1}{2}$ ". LEDGES SHALL NOT EXCEED A DEPTH OF MORE THAN 24" BELOW THE TOP OF THE WALL. FOR WALL THICKNESSES LESS THAN 4" PROVIDE #4 BARS AT MAX. 24" OC TO WITHIN 8" OF THE TOP

5) REINFORCEMENT SHALL BE LAPPED A MINIMUM 24" AT ENDS, SPLICES, AND AROUND

D) SUPPLEMENTAL REINFORCEMENT AT CORNERS - PLACE (1) #4 BAR 48" LONG AT 45 DEGREE ANGLE AT CORNERS OF OPENINGS. PLACE REINFORCEMENT WITHIN 6" OF

C) HORIZONTAL BARS SHOULD BE AS CLOSE TO THE TENSION FACE AS POSSIBLE (INTERIOR) AND BEHIND THE VERTICAL REINFORCEMENT (I.E. 2" TOWARD THE

A) ONE BAR SHALL BE PLACED WITHIN 12" OF THE TOP OF THE WALL B) OTHER BARS SHALL BE EQUALLY SPACED WITH SPACING NOT TO EXCEED 24" OC

C) CONCRETE EXPOSED TO WEATHER (TOP CLEARANCE IN GARAGE AND DRIVEWAY

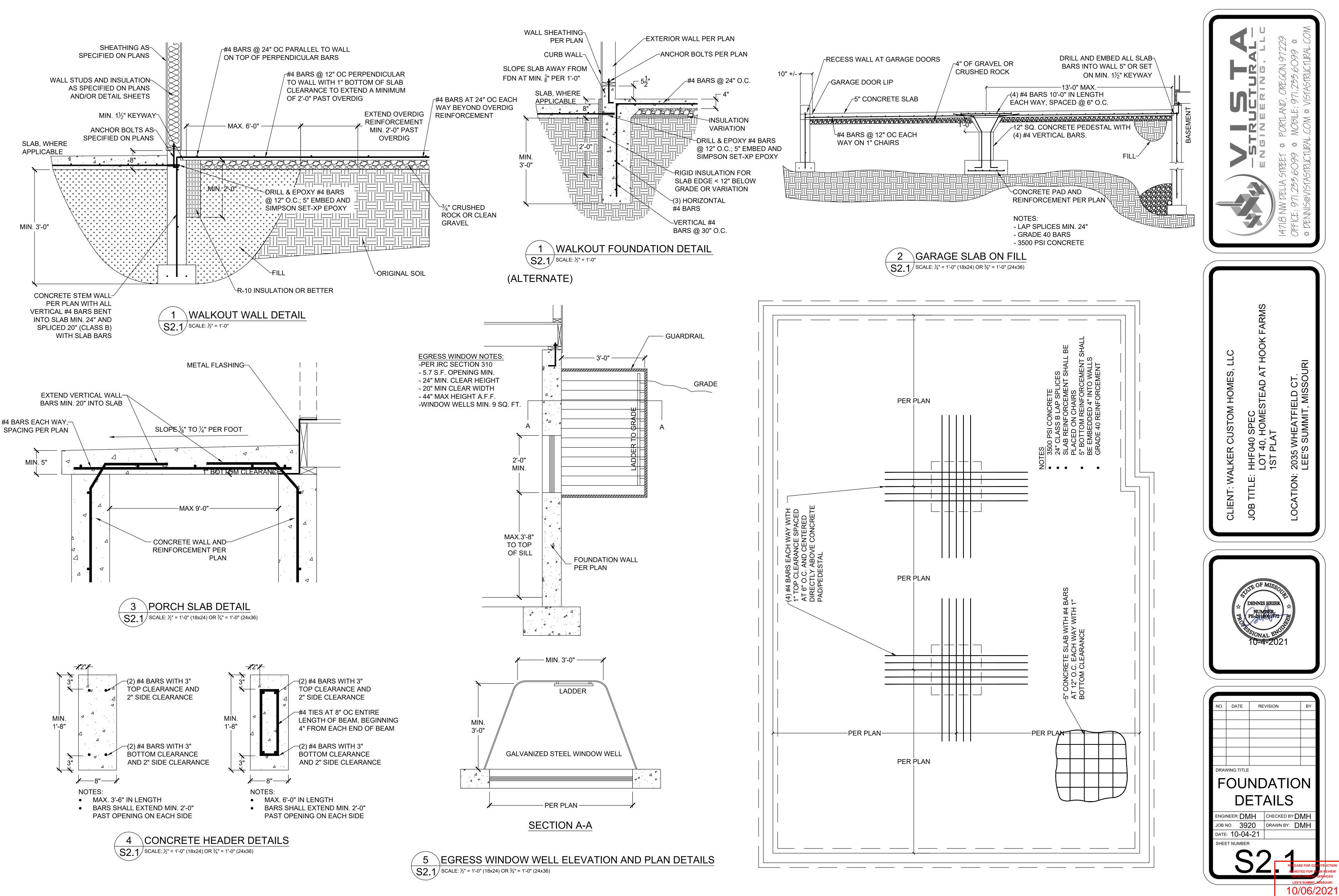
REINFORCEMENT SPACING 24" OC, REINFORCEMENT MAY BE PLACED IN THE MIDDLE OF THE

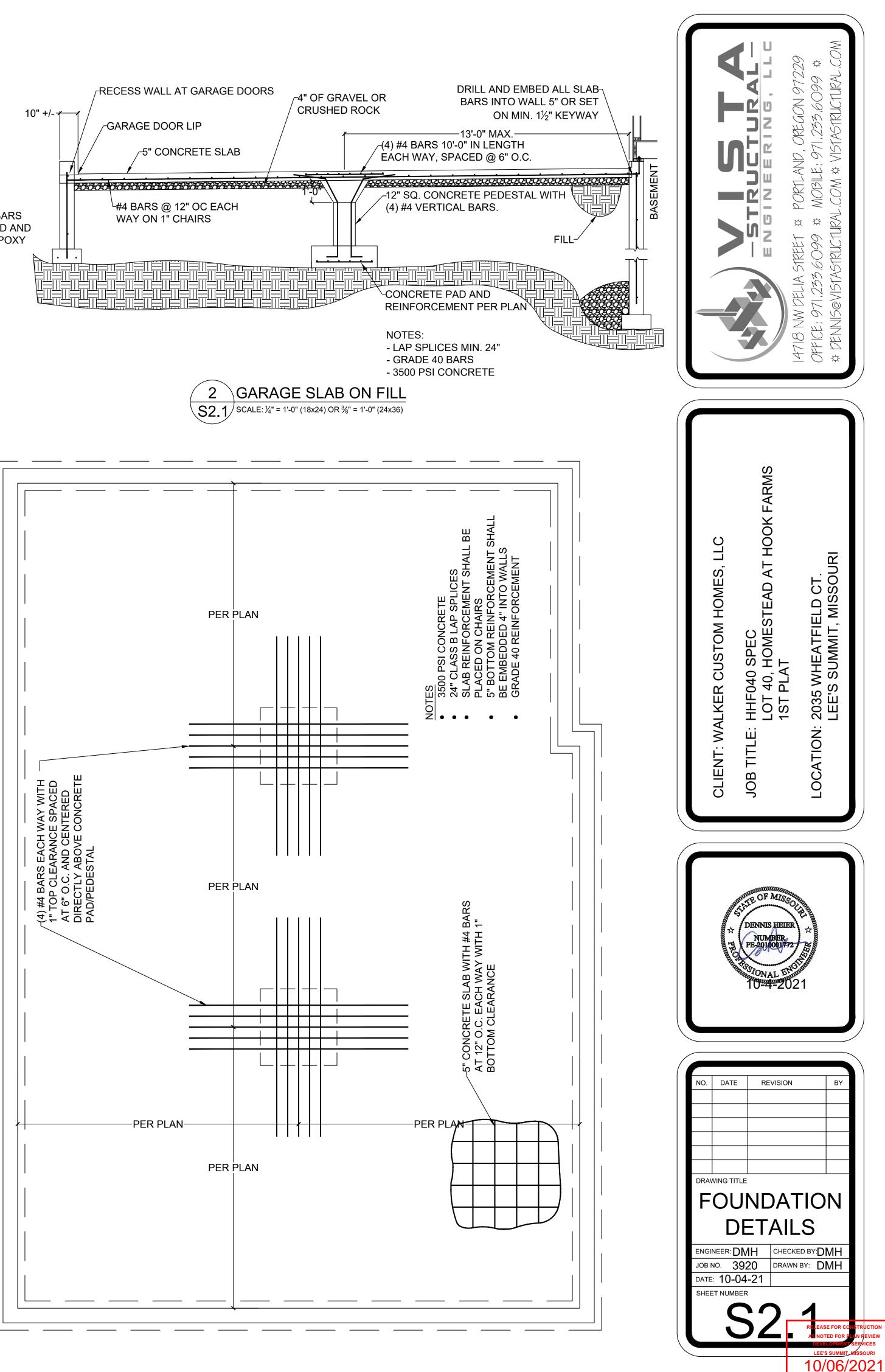
- MINIMUM GRADE 40 STEEL									
_L;	6-#4	7-#4	7-#4	6-#4	7-#4	7-#4			
OM THE TOP OF THE WALL TO THE TOP OF THE FLOOR SLAB									

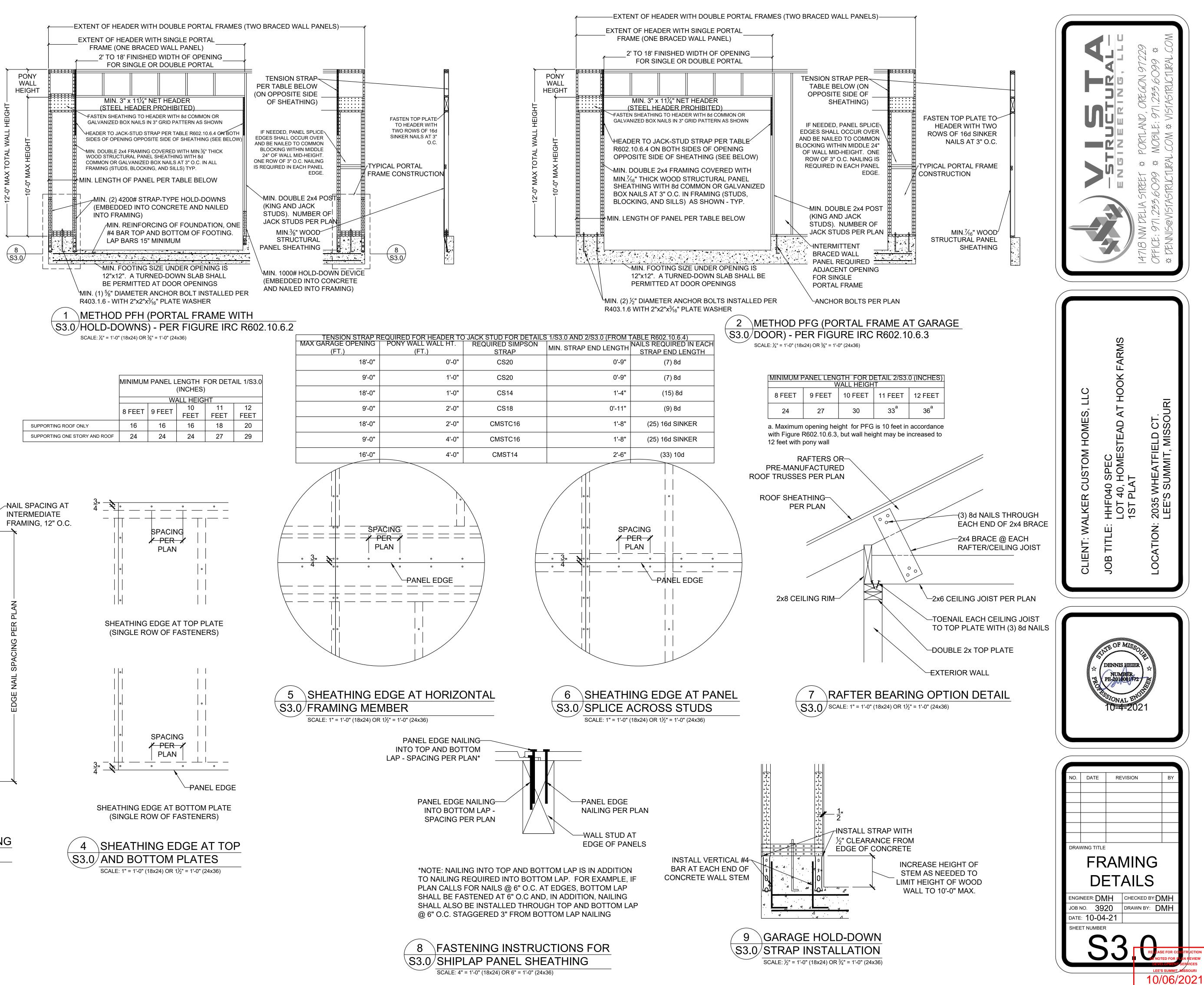
VERTICAL REINFORCEMENT SPACIN	IG						
CONCRETE STRENGTH/GRADE	8"	THICK W	'ALL	10" THICK WALL			
REINFORCEMENT (#4 BARS)	8'	9'	10'	8'	9'	10'	
3,000 PSI/ GRADE 40	24	24	16	24	24	18	
3,500 PSI/ GRADE 40	24	24	16	24	24	18	
3,000 PSI/ GRADE 60	24	24	16	24	24	18	
3,500 PSI/ GRADE 60	24	24	16	24	24	18	
HORIZONTAL REINFORCEMENT - MINIMUM GRADE 40 STEEL							
ONE BAR 12" FROM TOP OF WALL; MAX. SPACING 24" OC	6-#4	7-#4	7-#4	6-#4	7-#4	7-#4	

T JZ U ÷Ċ+ ARMS V ()D Ω Ũ \square Ē S SPEC \mathbf{C} CUS. 40 040 40 PL Ŷ 11 <u>우</u> [2] [1] Г 20 Г 20 CATION: CLIENT m Ο DATE REVISION DRAWING TITLE FOUNDATION DETAILS ENGINEER: DMH CHECKED BY:DMH JOB NO. 3920 DRAWN BY: DMH DATE: 10-04-21 SHEET NUMBER

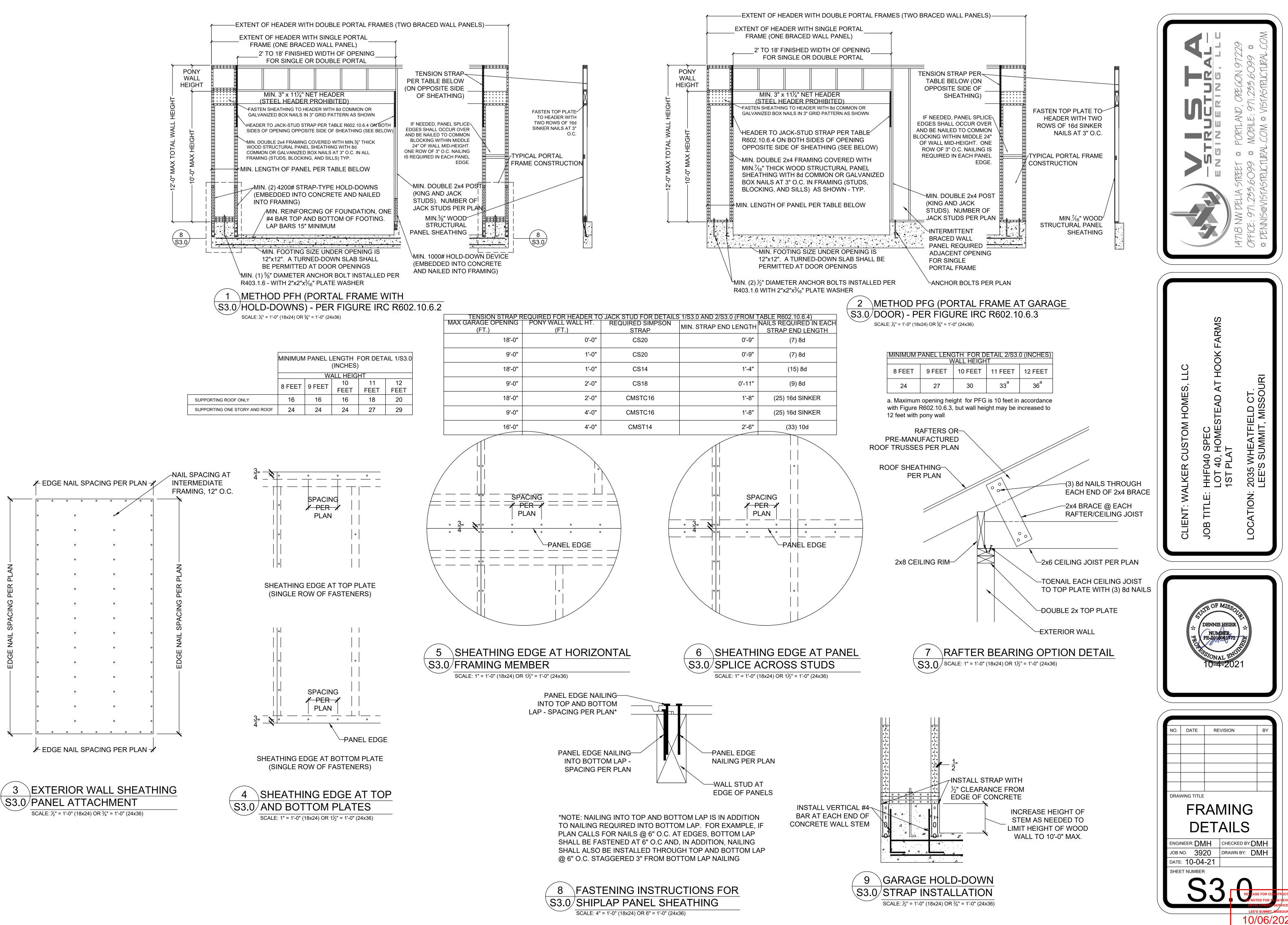
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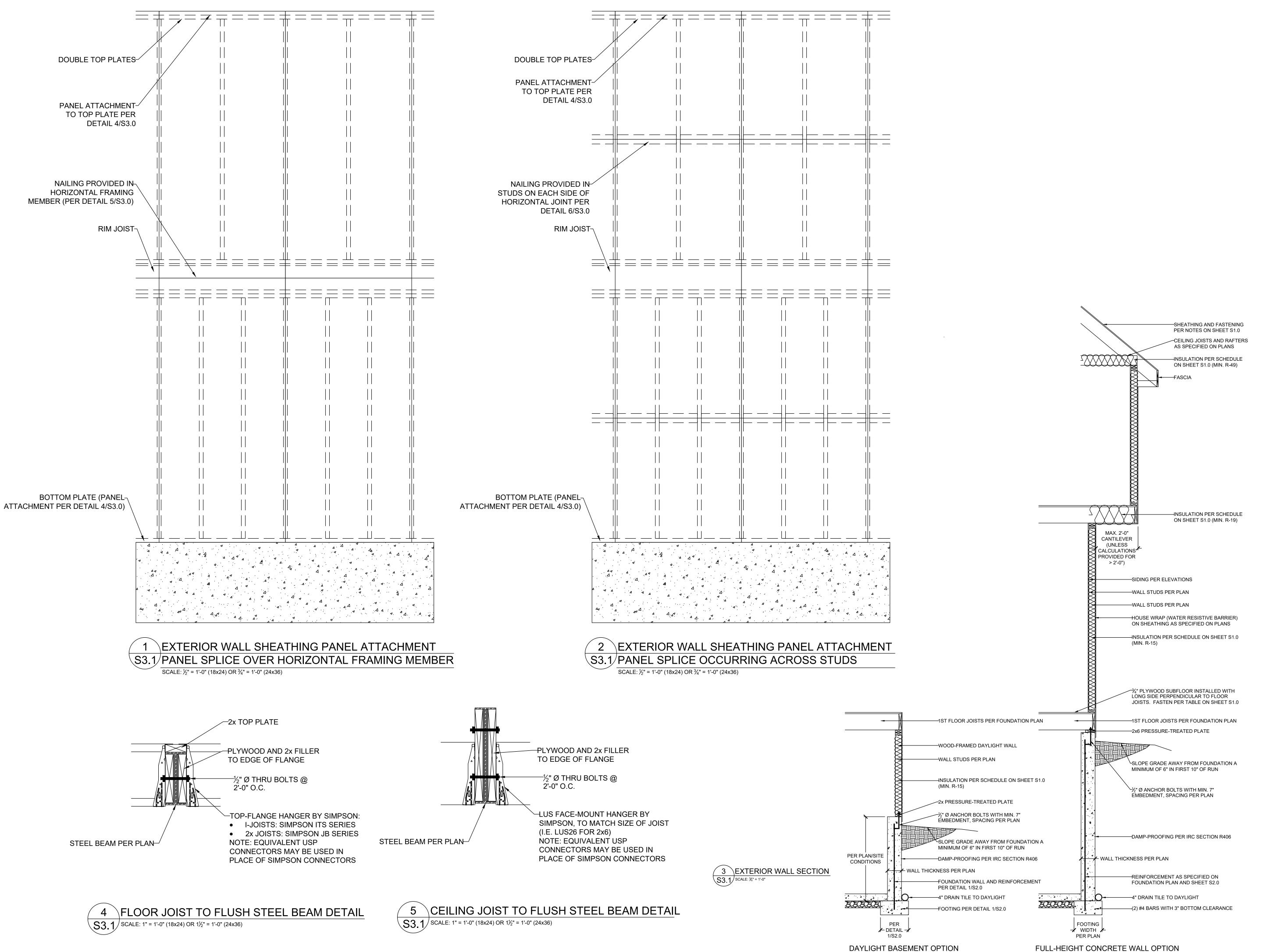


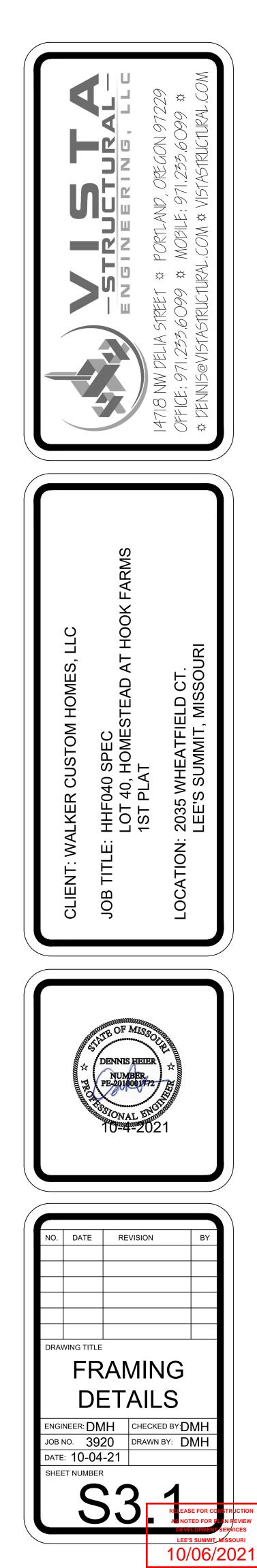


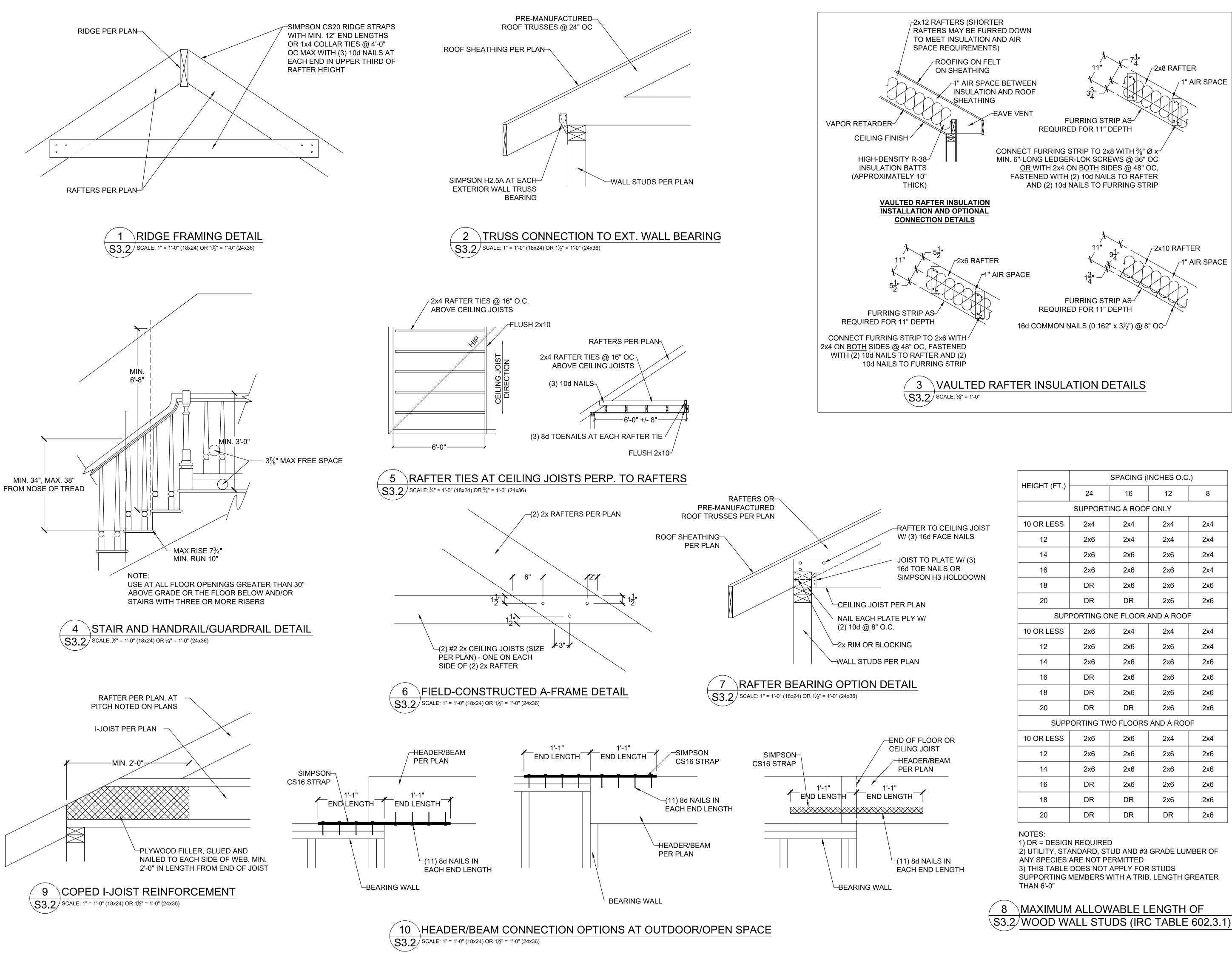


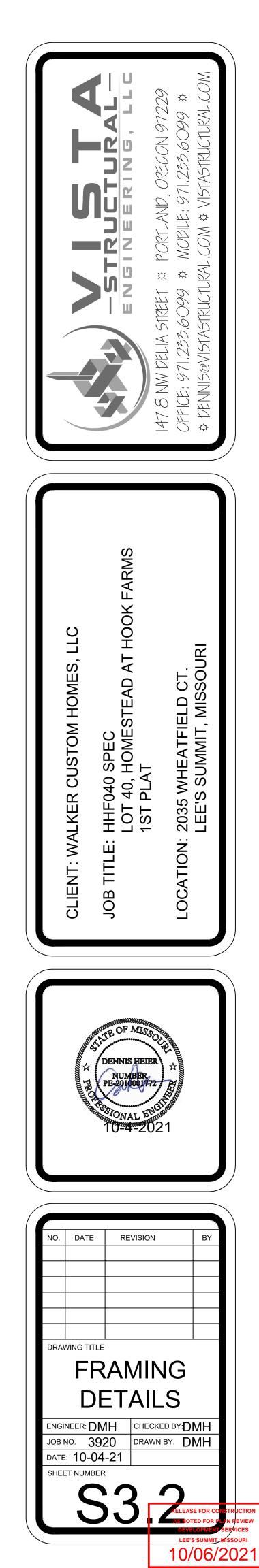
(INCHES)							
	WALL HEIGHT						
	8 FEET	9 FEET	10	11			
	_	_	FEEI	FEEI			
SUPPORTING ROOF ONLY	16	16	16	18			
SUPPORTING ONE STORY AND ROOF	24	24	24	27			
	SUPPORTING ROOF ONLY	SUPPORTING ROOF ONLY 16	W/ 8 FEET 9 FEET SUPPORTING ROOF ONLY 16	INCHES WALL HEIG 8 FEET 9 FEET SUPPORTING ROOF ONLY 16 16 16	WALL HEIGHT WALL HEIGHT 8 FEET 9 FEET 10 11 FEET 9 FEET FEET FEET SUPPORTING ROOF ONLY 16 16 18		

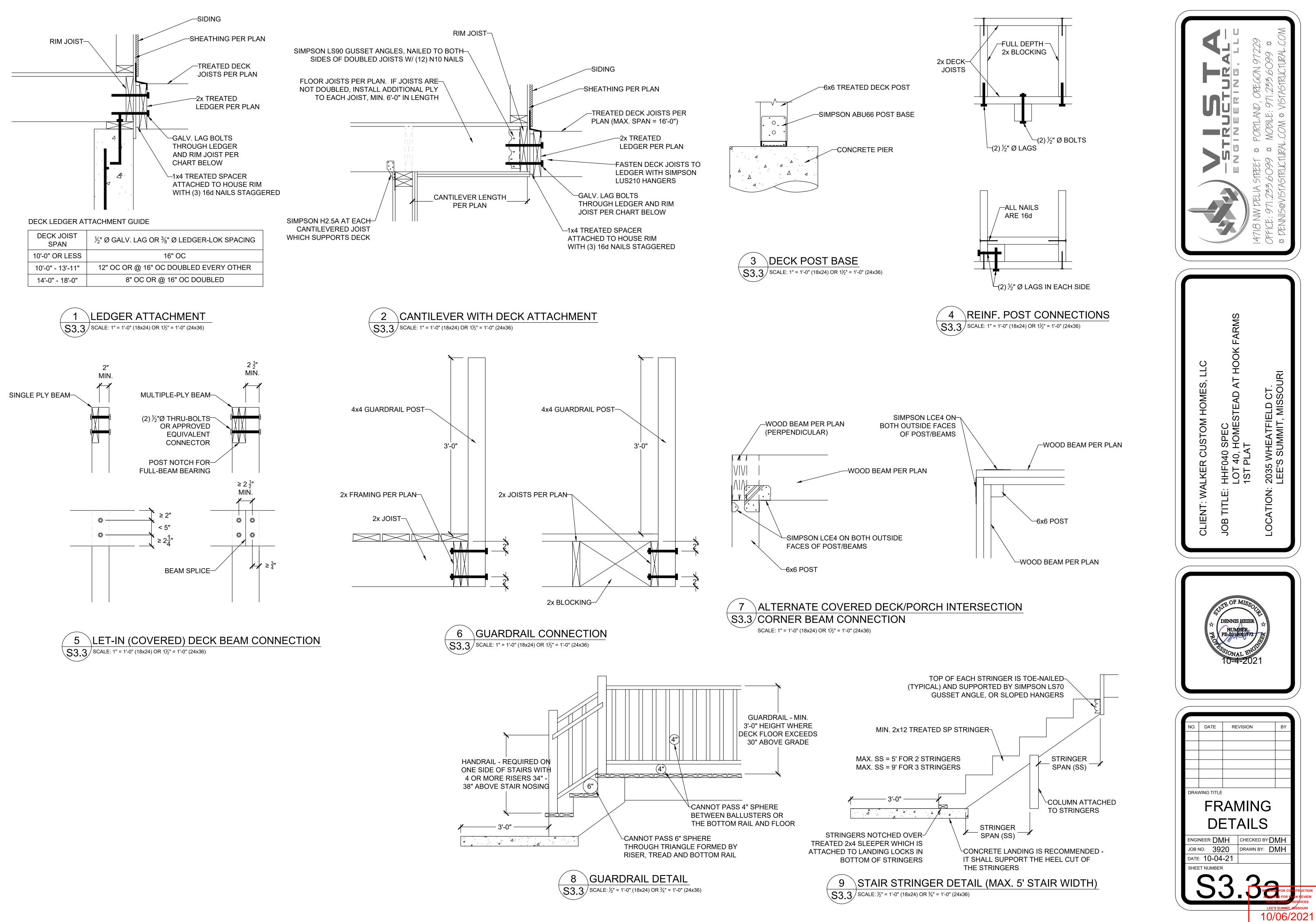


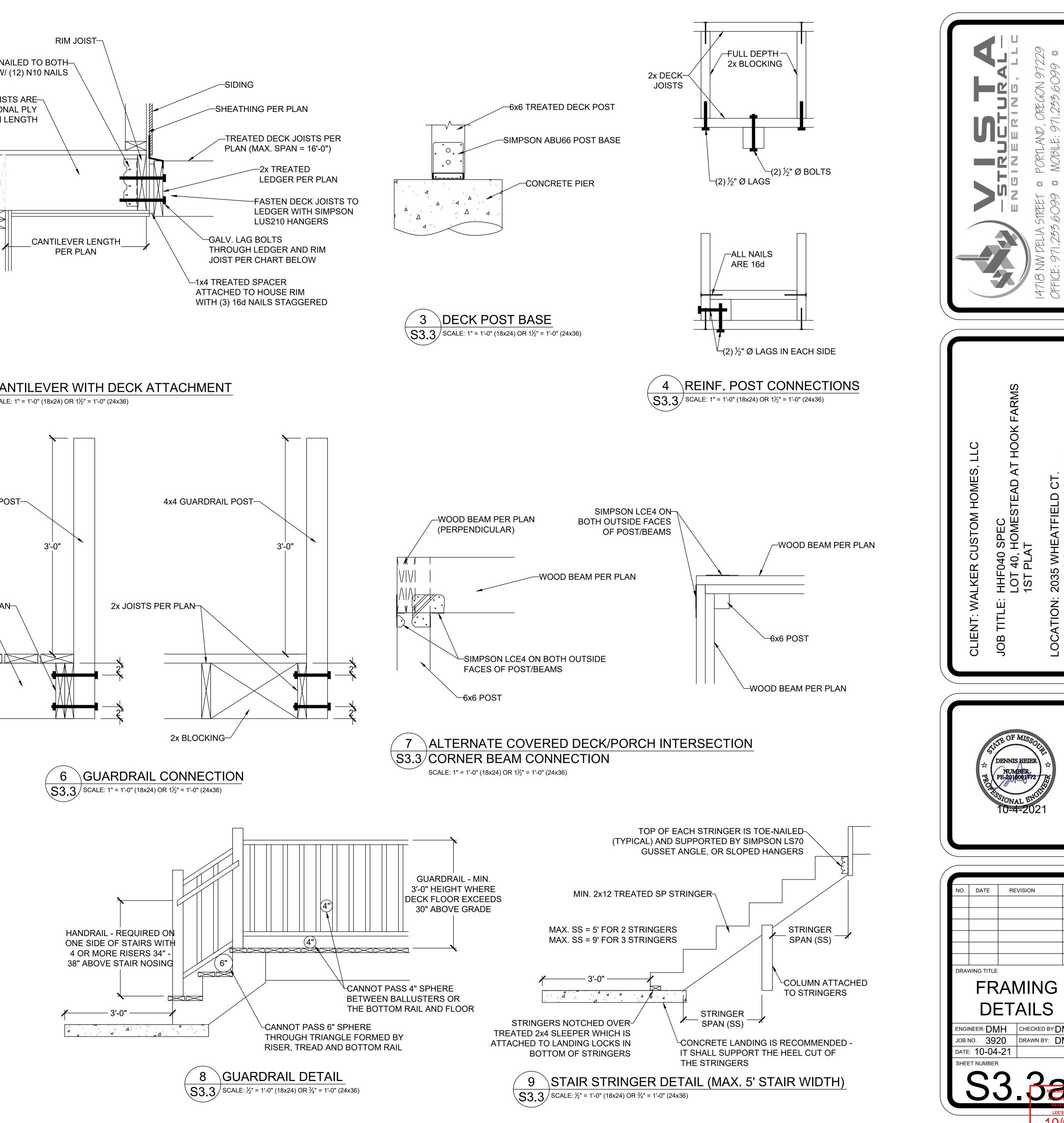


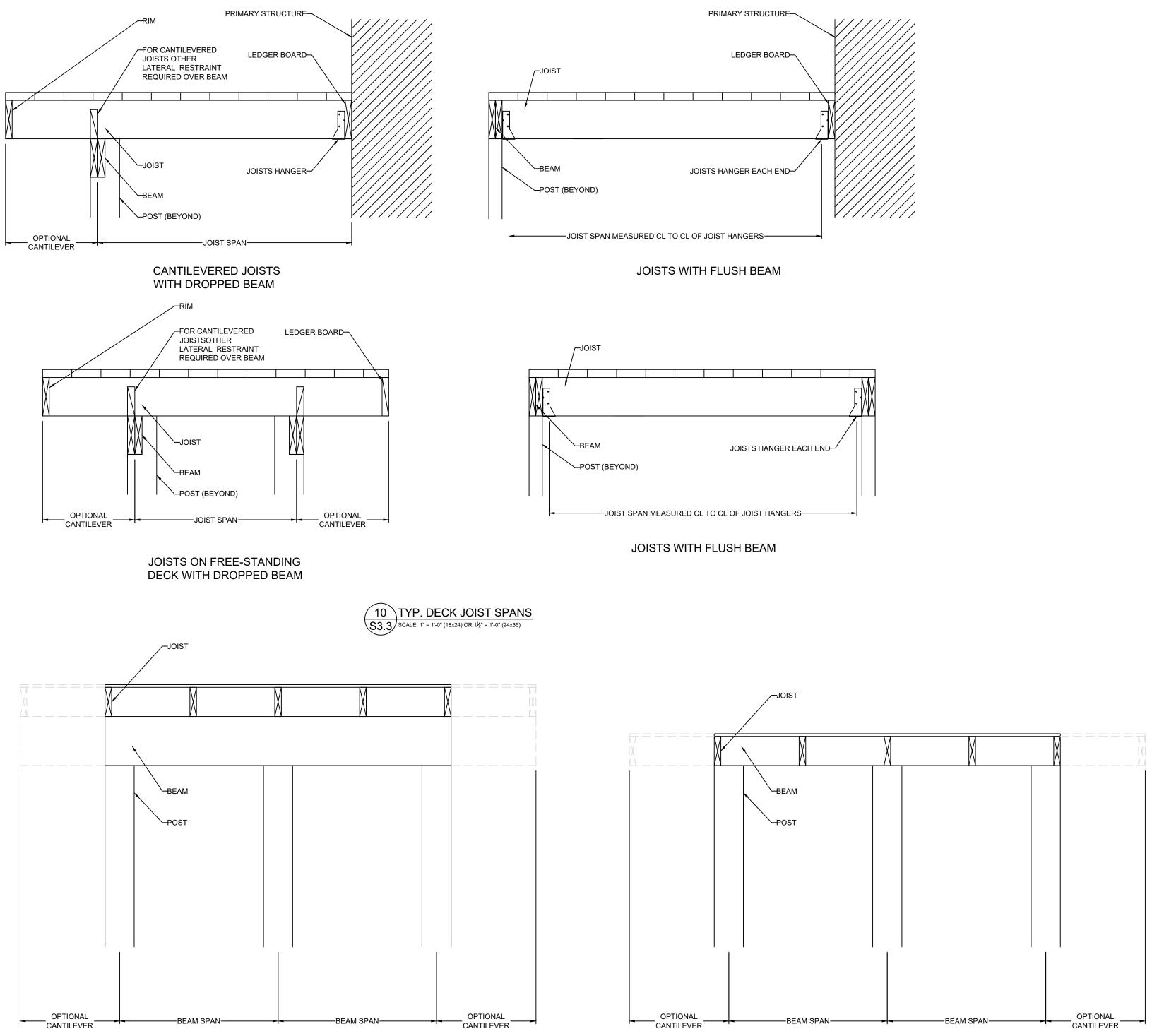












DROPPED BEAM



FLUSH BEAM

