

Title Block Line 1
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 Title Block Line 6

Project Title: 2366 SW OLD PORT RD.
 Engineer:
 Project ID:
 Project Descr:

Printed: 27 SEP 2021, 10:25AM

Wood Beam

Lic. #: KW-06001844

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 HD ENGINEERING & DESIGN INC.

DESCRIPTION: BEAMS OVER FRONT OF GARAGE

CODE REFERENCES

Calculations per NDS 2018, IBC 2018, CBC 2019, ASCE 7-16
 Load Combination Set : ASCE 7-16

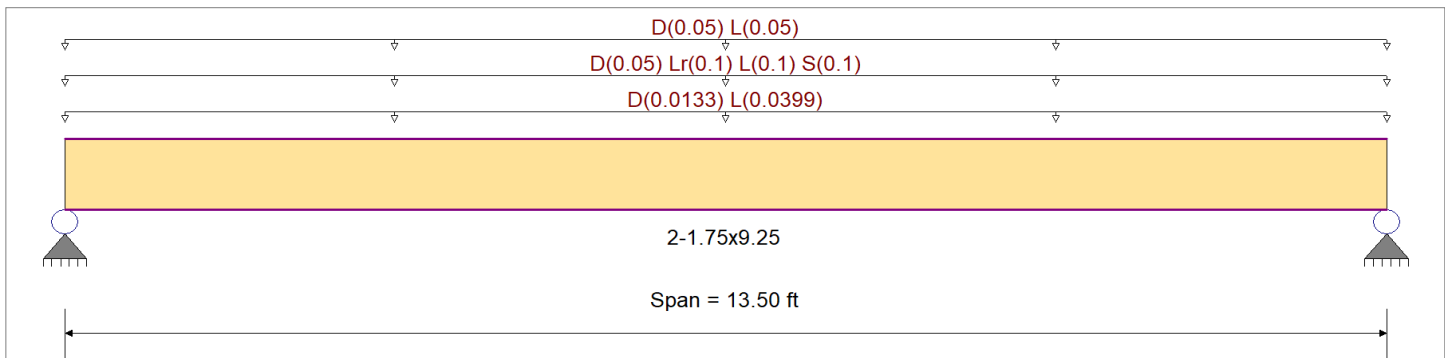
Material Properties

Analysis Method : Allowable Stress Design
 Load Combination : ASCE 7-16

Wood Species : iLevel Truss Joist
 Wood Grade : MicroLam LVL 1.9 E

Beam Bracing : Beam is Fully Braced against lateral-torsional buckling

Fb + 2,600.0 psi E : Modulus of Elasticity
 Fb - 2,600.0 psi Ebend-xx 1,900.0
 Fc - Prll 2,510.0 psi Eminend-xx 965.71 ksi
 Fc - Perp 750.0 psi
 Fv 285.0 psi
 Ft 1,555.0 psi Density 42.010pcf



Applied Loads

Service loads entered. Load Factors will be applied for calculations.

Uniform Load : D = 0.010, L = 0.030 ksf, Tributary Width = 1.330 ft, (FLOOR)
 Uniform Load : D = 0.010, Lr = 0.020, L = 0.020, S = 0.020 ksf, Tributary Width = 5.0 ft, (ROOF)
 Uniform Load : D = 0.010, L = 0.010 ksf, Tributary Width = 5.0 ft, (CEILING)

DESIGN SUMMARY

Design OK

| | | | | | | | |
|-----------------------------------|---|-------------|-----|-----------------------------|---|-------------|-------|
| Maximum Bending Stress Ratio | = | 0.639 | 1 | Maximum Shear Stress Ratio | = | 0.296 | 1 |
| Section used for this span | | 2-1.75x9.25 | | Section used for this span | | 2-1.75x9.25 | |
| fb: Actual | = | 1,660.69 | psi | fv: Actual | = | 84.44 | psi |
| Fb: Allowable | = | 2,600.00 | psi | Fv: Allowable | = | 285.00 | psi |
| Load Combination | | +D+L | | Load Combination | | +D+L | |
| Location of maximum on span | = | 6.750 | ft | Location of maximum on span | = | 0.000 | ft |
| Span # where maximum occurs | = | Span # 1 | | Span # where maximum occurs | = | Span # 1 | |
| Maximum Deflection | | | | | | | |
| Max Downward Transient Deflection | | 0.325 | in | Ratio = | | 497 | >=360 |
| Max Upward Transient Deflection | | 0.000 | in | Ratio = | | 0 | <360 |
| Max Downward Total Deflection | | 0.567 | in | Ratio = | | 285 | >=180 |
| Max Upward Total Deflection | | 0.000 | in | Ratio = | | 0 | <180 |

Maximum Forces & Stresses for Load Combinations

| Load Combination | Segment Length | Span # | Max Stress Ratios | | | | | | | | Moment Values | | | Shear Values | | | |
|---------------------|----------------|--------|-------------------|-------|----------------|------------------|----------------|----------------|----------------|----------------|----------------|------|----------|--------------|------|-------|--------|
| | | | M | V | C _d | C _{F/V} | C _i | C _r | C _m | C _t | C _L | M | fb | F'b | V | fv | F'v |
| D Only | | | | | | | | | | | | | 0.00 | 0.00 | 0.00 | 0.00 | |
| Length = 13.451 ft | 1 | | 0.265 | 0.123 | 0.90 | 1.000 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 2.58 | 620.57 | 2340.00 | 0.68 | 31.55 | 256.50 |
| Length = 0.04927 ft | 1 | | 0.004 | 0.123 | 0.90 | 1.000 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.04 | 9.03 | 2340.00 | 0.68 | 31.55 | 256.50 |
| +D+L | | | | | | 1.000 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | | | 0.00 | 0.00 | 0.00 | 0.00 |
| Length = 13.451 ft | 1 | | 0.639 | 0.296 | 1.00 | 1.000 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 6.91 | 1,660.69 | 2600.00 | 1.82 | 84.44 | 285.00 |
| Length = 0.04927 ft | 1 | | 0.009 | 0.296 | 1.00 | 1.000 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.10 | 24.16 | 2600.00 | 1.82 | 84.44 | 285.00 |
| +D+Lr | | | | | | 1.000 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | | | 0.00 | 0.00 | 0.00 | 0.00 |
| Length = 13.451 ft | 1 | | 0.359 | 0.167 | 1.25 | 1.000 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 4.86 | 1,168.29 | 3250.00 | 1.28 | 59.40 | 356.25 |
| Length = 0.04927 ft | 1 | | 0.005 | 0.167 | 1.25 | 1.000 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.07 | 16.99 | 3250.00 | 1.28 | 59.40 | 356.25 |
| +D+S | | | | | | 1.000 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | | | 0.00 | 0.00 | 0.00 | 0.00 |
| Length = 13.451 ft | 1 | | 0.391 | 0.181 | 1.15 | 1.000 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 4.86 | 1,168.29 | 2990.00 | 1.28 | 59.40 | 327.75 |
| Length = 0.04927 ft | 1 | | 0.006 | 0.181 | 1.15 | 1.000 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.07 | 16.99 | 2990.00 | 1.28 | 59.40 | 327.75 |

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 DEVELOPMENT SERVICES
 LEE'S SUMMIT, MISSOURI

09/28/2021

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Wood Beam

File: 41286.ec6
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 HD ENGINEERING & DESIGN INC

Lic. #: KW-06001844

DESCRIPTION: BEAMS OVER FRONT OF GARAGE

| Load Combination | Segment Length | Span # | Max Stress Ratios | | C _d | C _{F/V} | C _i | C _r | C _m | C _t | C _L | Moment Values | | | Shear Values | | |
|---------------------|----------------|--------|-------------------|-------|----------------|------------------|----------------|----------------|----------------|----------------|----------------|---------------|----------------|----------------|--------------|----------------|----------------|
| | | | M | V | | | | | | | | M | f _b | F _b | V | f _v | F _v |
| +D+0.750Lr+0.750L | | | | | 1.000 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | | | 0.00 | 0.00 | 0.00 | 0.00 |
| Length = 13.451 ft | 1 | | 0.557 | 0.259 | 1.25 | 1.000 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 7.53 | 1,811.45 | 3250.00 | 1.99 | 92.11 | 356.25 |
| Length = 0.04927 ft | 1 | | 0.008 | 0.259 | 1.25 | 1.000 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.11 | 26.35 | 3250.00 | 1.99 | 92.11 | 356.25 |
| +D+0.750L+0.750S | | | | | 1.000 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | | | 0.00 | 0.00 | 0.00 | 0.00 |
| Length = 13.451 ft | 1 | | 0.606 | 0.281 | 1.15 | 1.000 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 7.53 | 1,811.45 | 2990.00 | 1.99 | 92.11 | 327.75 |
| Length = 0.04927 ft | 1 | | 0.009 | 0.281 | 1.15 | 1.000 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.11 | 26.35 | 2990.00 | 1.99 | 92.11 | 327.75 |
| +0.60D | | | | | 1.000 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | | | 0.00 | 0.00 | 0.00 | 0.00 |
| Length = 13.451 ft | 1 | | 0.090 | 0.042 | 1.60 | 1.000 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.55 | 372.34 | 4160.00 | 0.41 | 18.93 | 456.00 |
| Length = 0.04927 ft | 1 | | 0.001 | 0.042 | 1.60 | 1.000 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.02 | 5.42 | 4160.00 | 0.41 | 18.93 | 456.00 |

Overall Maximum Deflections

| Load Combination | Span # | Max. "-" Defl | Location in Span | Load Combination | Max. "+" Defl | Location in Span |
|------------------|--------|---------------|------------------|------------------|---------------|------------------|
| +D+0.750L+0.750S | 1 | 0.5668 | 6.799 | | 0.0000 | 0.000 |

Vertical Reactions

Support notation : Far left is #1

Values in KIPS

| Load Combination | Support 1 | Support 2 |
|-------------------|-----------|-----------|
| Overall MAXimum | 2.232 | 2.232 |
| Overall MINimum | 0.675 | 0.675 |
| D Only | 0.765 | 0.765 |
| +D+L | 2.047 | 2.047 |
| +D+Lr | 1.440 | 1.440 |
| +D+S | 1.440 | 1.440 |
| +D+0.750Lr+0.750L | 2.232 | 2.232 |
| +D+0.750L+0.750S | 2.232 | 2.232 |
| +0.60D | 0.459 | 0.459 |
| Lr Only | 0.675 | 0.675 |
| L Only | 1.282 | 1.282 |
| S Only | 0.675 | 0.675 |

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 LEE'S SUMMIT, MISSOURI

09/28/2021

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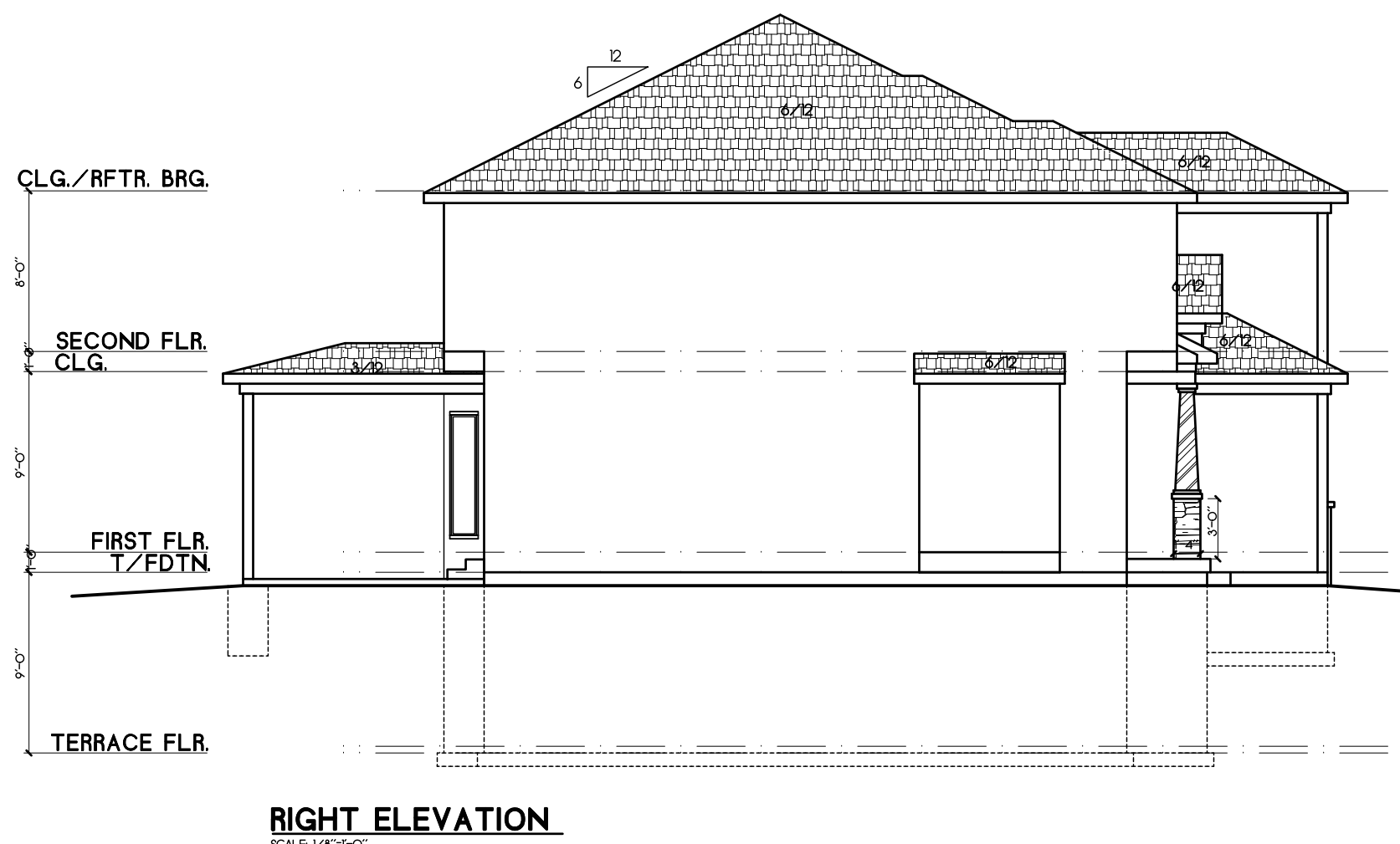
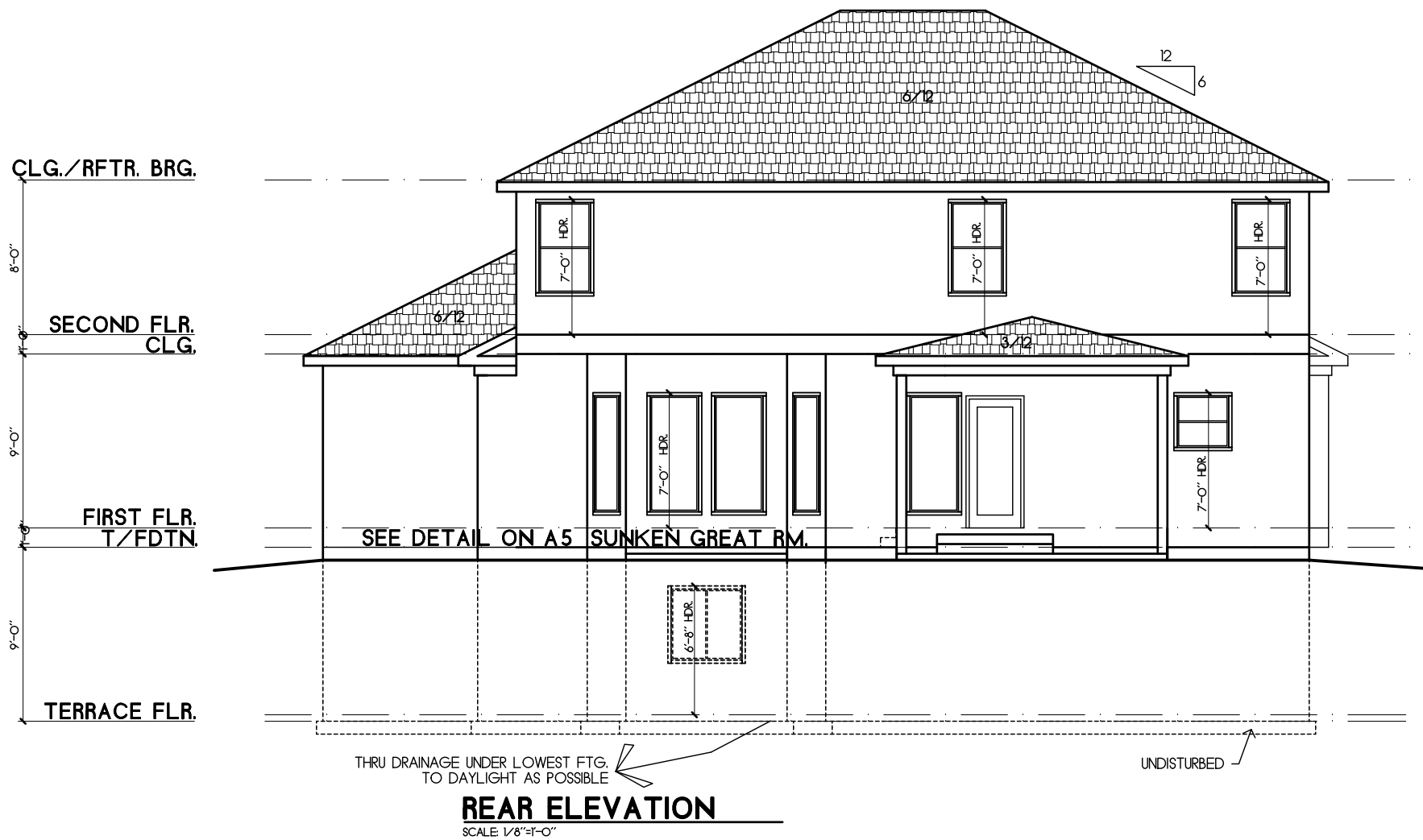
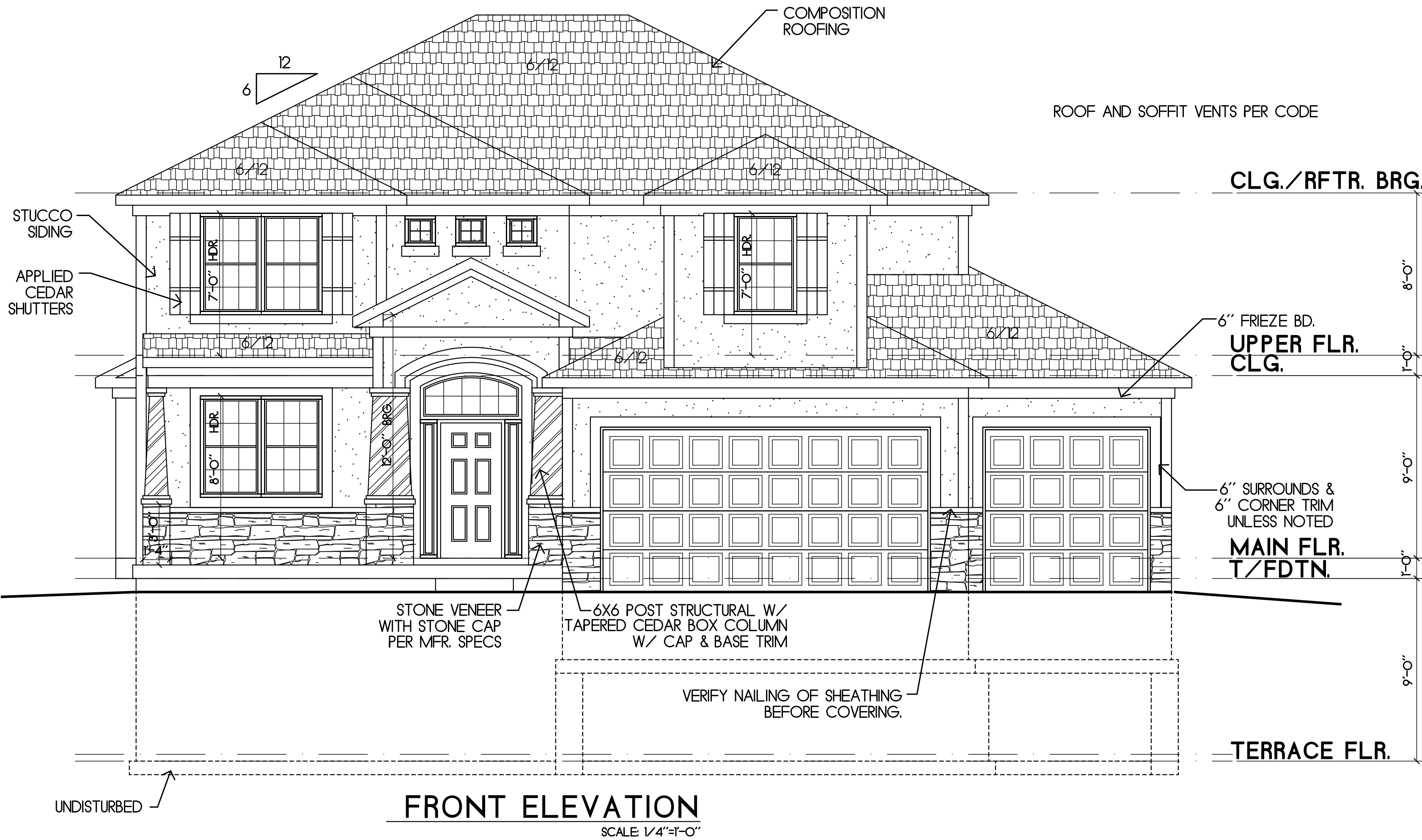
SAB HOMES, INC.
REDBUD E718
2366 SW. OLD PORT RD. LEE'S SUMMIT, MO
STRUCTURAL DETAILS & NOTES

HD#: 41286
DATE: 03/11/2021
CHECKED BY: CLS

| NO. | ISSUE/REVISION | Revision Date |
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RELEASE FOR CONSTRUCTION
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DEVELOPMENT SERVICES
LEE'S SUMMIT, MISSOURI
09/28/2021

| SQUARE FOOTAGE SUMMARY: : | |
|---------------------------|----------|
| MAIN FLOOR FINISH | 1,110 SF |
| UPPER FLOOR FINISH | 1,385 SF |
| LOWER FLOOR FINISH | 0 SF |
| LOWER FLOOR UN-FINISH | 1,128 SF |
| GARAGE SLAB | 630 SF |
| GARAGE AREA | 705 SF |
| FRONT PORCH | 105 SF |
| REAR PATIO | 168 SF |

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DECK PIER SCHEDULE

- MIN. 6X6 TRTD/CDR POST ON 12" CONC. PIER WITH USP PAU 66 BASE OR = (1177# MAX)
MIN. 6X6 TRTD/CDR POST ON 16" CONC. PIER WITH USP PAU 66 BASE OR = (2050# MAX)
MIN. 6X6 TRTD/CDR POST ON 18" CONC. PIER WITH USP PAU 66 BASE OR = (2649# MAX)
MIN. 6X6 TRTD/CDR POST ON 24" CONC. PIER WITH USP PAU 66 BASE OR = (4710# MAX)

PIERS TO TERMINATE ON ORIGINAL SOIL OF 1500 PSF MINIMUM BEARING.
PIERS TO TERMINATE AT A POINT 36" MINIMUM BELOW FINISH GRADE.
POST ARE NOT TO EXCEED AN UNBRACED LENGTH OF 12' WITHOUT CONTACTING HD ENGINEERING FOR GUIDANCE.

COLUMN PAD SCHEDULE

- 3" SCH. 40 STL. COL. ON 30"x30"x12" CONC. PAD W/ (5) #4 BARS E.W. (9.4K MAX.)
3" SCH. 40 STL. COL. ON 36"x36"x12" CONC. PAD W/ (6) #4 BARS E.W. (13.5K MAX.)
3 1/2" SCH. 40 STL. COL. ON 42"x42"x14" CONC. PAD W/ (7) #4 BARS E.W. (18.4K MAX.)
3 1/2" SCH. 40 STL. COL. ON 48"x48"x16" CONC. PAD W/ (8) #4 BARS E.W. (24K MAX.)
3 1/2" SCH. 40 STL. COL. ON 54"x54"x16" CONC. PAD W/ (9) #4 BARS E.W. (30.4K MAX.)
3 1/2" SCH. 40 STL. COL. ON 60"x60"x18" CONC. PAD W/ (10) #4 BARS E.W. (37.5K MAX.)

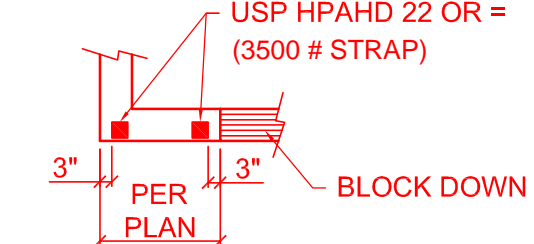
NOTES:

1. COLUMN AND PIER PAD SIZES SHOWN ARE FOR MAX. COLUMN HEIGHT OF 10'-0" TALL.
2. COLUMN AND PIER PAD SIZES SHOWN ARE BASED ON AN ASSUMED 1500 PSF. THIS IS THE CAPACITY REQUIRED BY A.H.J. UNDERLINED GENERAL NOTES ON S-1.0 FOR MORE DETAILS.
3. ALL STEEL COLUMNS SHALL BE ISOLATED FROM SLABS WITH APPROVED ISSOLATION DEVICE OR JOINT.

GENERAL NOTES:

- WINDOW SHALL HAVE FALL PROTECTION PER IRC 312.2.4
-HOUSE WILL BE PROVIDED WITH A "UFER" GROUND PER IRC SECTION 3608.1.5
-OVERHEAD GARAGE DOORS MUST MEET DASMA REQUIREMENTS SEE DETAIL SHEET S-1.0
-ALL HEADERS NOT LABELED SHALL BE MIN (2) #2-2X10 DFL
-DBL ALL JST UNDER ISLAND
-SOILS IN THIS AREA COMMONLY HAVE A VERY HIGH SHRINK SWELL CAPACITY. OUR FIRM RECOMMENDS ALL SITES BE EVALUATED BY A GEOTECHNICAL FIRM PRIOR TO PLACEMENT OF FOUNDATIONS
-PROVIDE CARBON MONOXIDE AND SMOKE DETECTORS PER IRC REQUIREMENTS
-ANY PORTION OF THESE PRINTS ISSUED WITHOUT A MIN. OF S-1.0 - S-4.0 SHALL NOT BE CONSIDERED A COMPLETE SET OF CONSTRUCTION DOCUMENTS
-INSTALL W8X15 STEEL BEAM MIN. UNDER ALL F.P. WALLS/HEARTHS (THAT WILL RECEIVE ROCK) UNLESS NOTED AS A LARGER BEAM. ANY STONE OVER 2" DEEP, NOTIFY ENG. TO VERIFY LOADS
-FOUNDATION SHALL BE CONSTRUCTED PER JOHNSON COUNTY RESIDENTIAL FOUNDATION GUIDELINE. SEE ATTACHED
-ICE AND WATER SHIELD AS REQUIRED PER IRC

TYPICAL TIE DOWN AT NARROW WALL

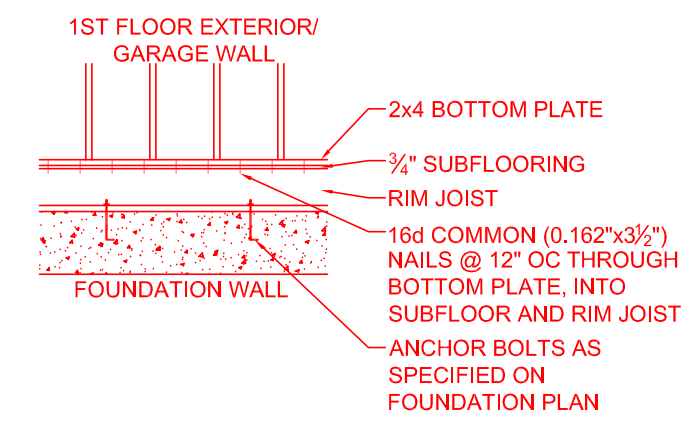


BRACED WALLS:

SEE CALCULATIONS ON SHEET S-2.0, PER ASC7-10 REQUIREMENTS AS ALLOWED BY IRC 2018 R301.2.1

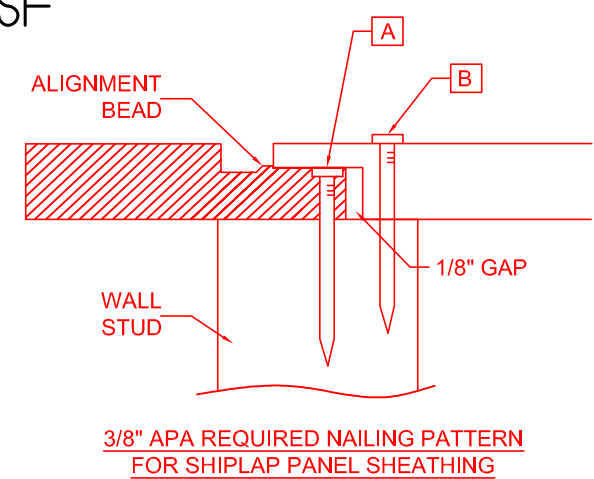
ALL EXTERIOR WALLS SHALL BE SHEATHED PER ANY ONE OF THE FOLLOWING OPTIONS:
-7/16" APA-RATED PLYWOOD/OSB WITH 8d NAILS @ 6" O.C. AT EDGES AND @ 12" O.C. IN THE FIELD
-7/16" SHIP LAP PANEL SHEATHING (I.E. LP SMARTSIDE OR EQUIVALENT) WITH 6d NAILS @ 6" O.C. AT EDGES AND @ 12" O.C. IN THE FIELD
-3/8" SHIP LAP PANEL SHEATHING (I.E. LP SMARTSIDE OR EQUIVALENT) WITH 6d NAILS @ 4" O.C. AT EDGES AND @ 12" O.C. IN THE FIELD

INTERIOR BRACED WALL LOCATIONS ONLY SHOWN WHEN REQUIRED BY ADDITIONAL BRACING SECTION OF CALCULATIONS ON SHEET S-2.0



FOUNDATION ANCHORING NOTES

- MIN. 1/2" ANCHOR BOLTS SHALL BE INSTALLED @ 36" O.C. MAX AND WITHIN 6"-12" FROM THE END OF EACH SECTION OF SILL PLATE ALONG ENTIRE PERIMETER OF FOUNDATION



NAILING WITH SPACING AS SPECIFIED PER PLAN. FOR EXAMPLE, IF REQUIRED SPACING IS 4" O.C., BOTTOM LAP SHALL FIRST BE NAILED AT 4" O.C. (NAIL "A"), THEN FULL DEPTH SECTION OF OVERLAP PANEL SHALL BE NAILED @ 4" O.C. (NAIL "B")

HD ENGINEERING & DESIGN, INC

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SHAWNEE, KS 66214

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913.631.2222

SERVICE@HDENGINEERS.COM



07/21/2021 PLAN CLARIFICATION

SAB HOMES, INC.
REDBUD E718
2366 SW. OLD PORT RD. LEE'S SUMMIT, MO

STRUCTURAL DETAILS & NOTES

HD#: 41286

DATE: 03/11/2021

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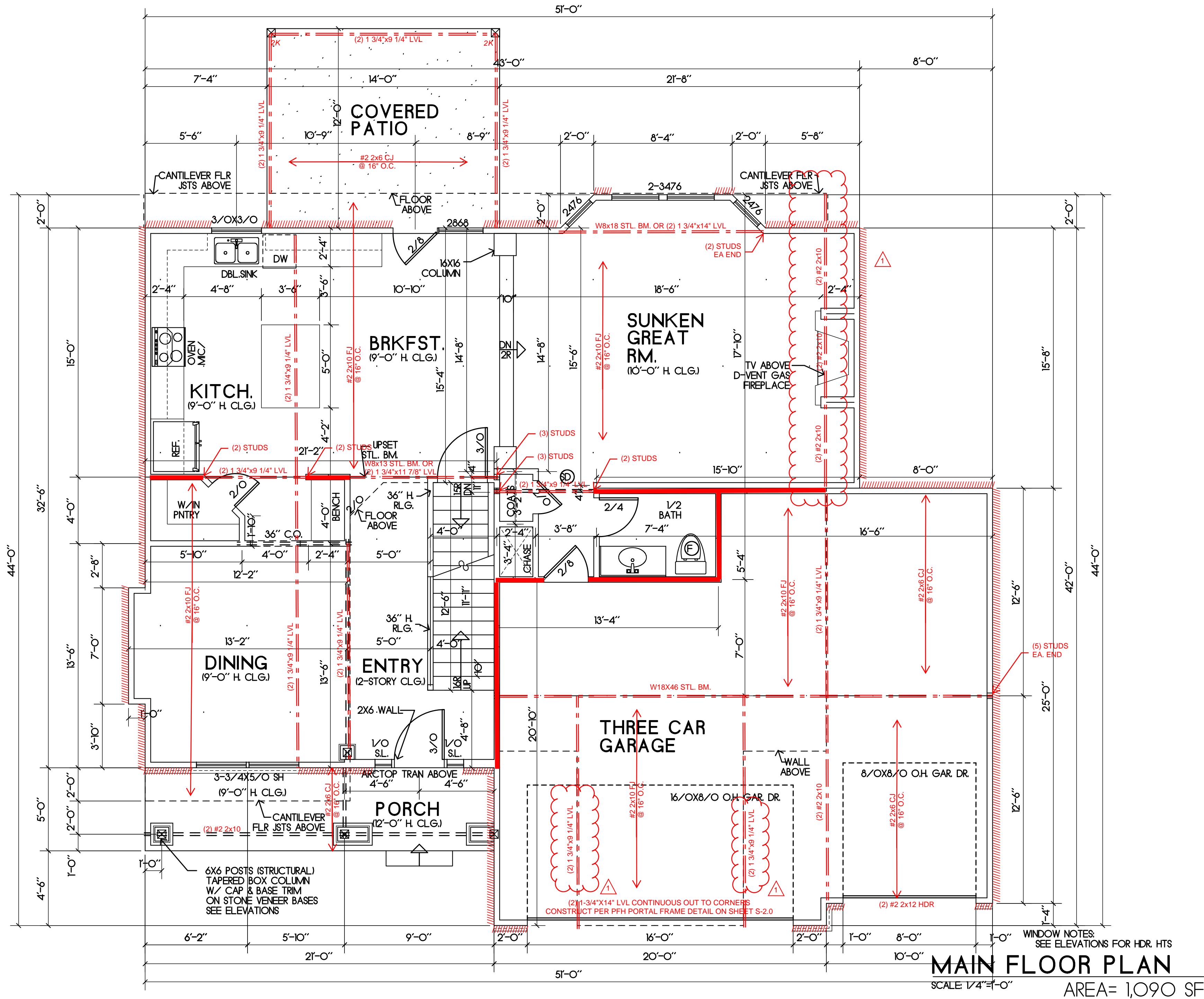
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LEE'S SUMMIT, MISSOURI
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2366 SW. OLD PORT RD. LEE'S SUMMIT, MO

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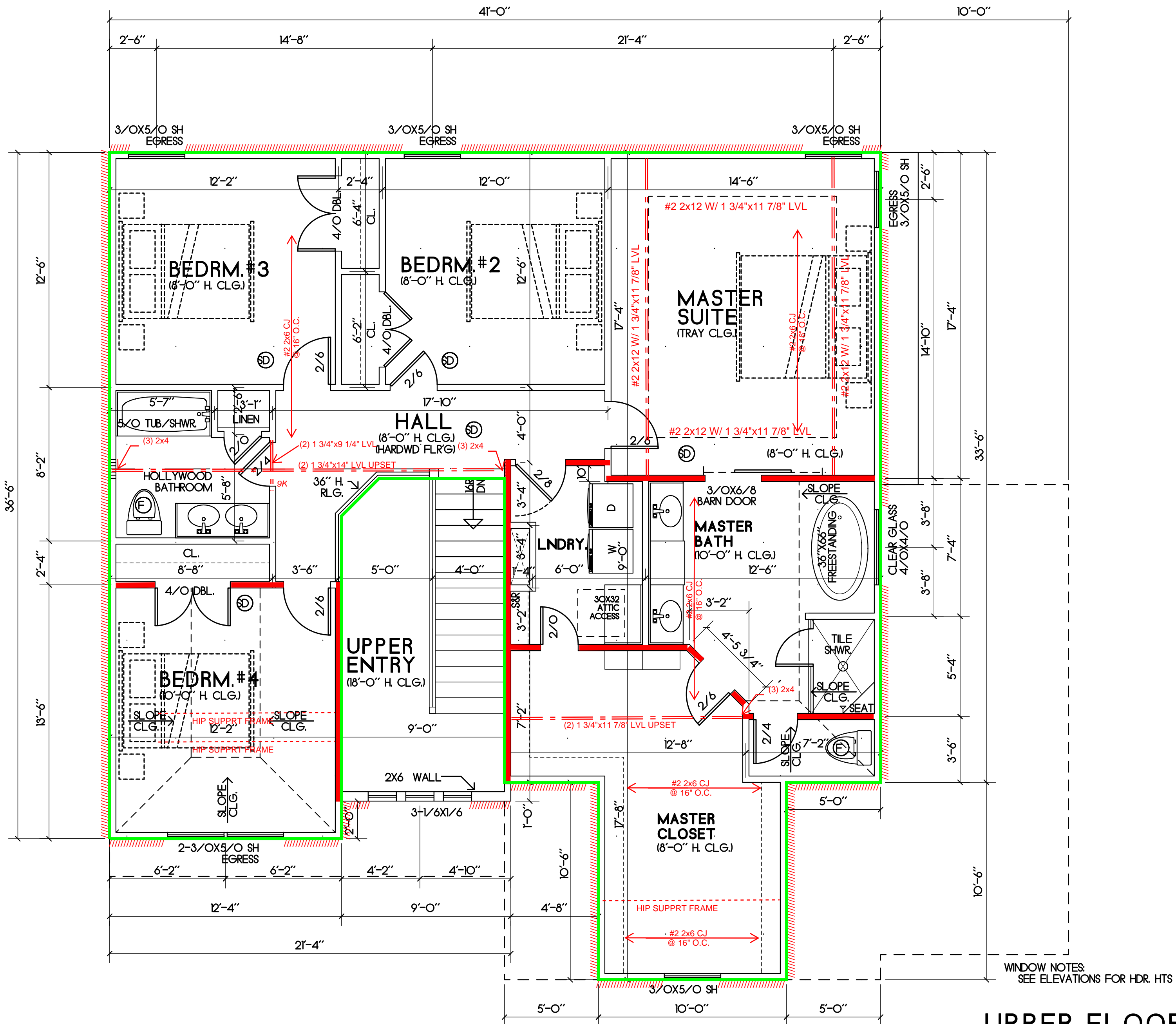
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WINDOW NOTES:
SEE ELEVATIONS FOR HDR. HTS

UPPER FLOOR PLAN
SCALE: 1/4"=1'-0" AREA= 1,385 SF

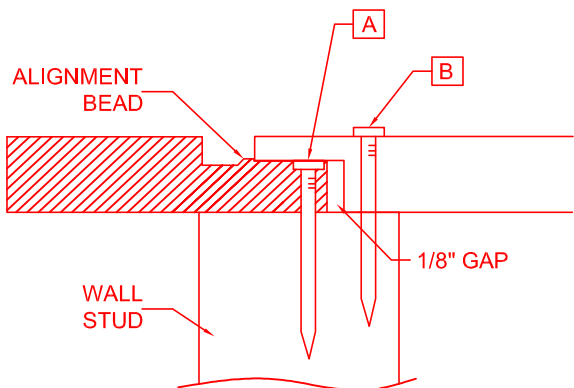
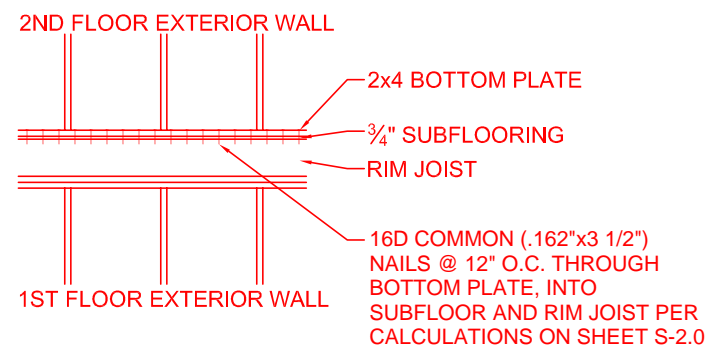
- LOAD BEARING WALL
- - - LOAD BEARING BEAM
- SD - SMOKE DETECTOR
- CO - CARBON MONOXIDE SENSOR

GENERAL NOTES:
-WINDOW SHALL HAVE FALL PROTECTION PER IRC 312.2.4
-HOUSE WILL BE PROVIDED WITH A "UFER" GROUND PER IRC SECTION 3608.1.5
-OVERHEAD GARAGE DOORS MUST MEET DASHA REQUIREMENTS SEE DETAIL SHEET S-1.0
-ALL HEADERS NOT LABELED SHALL BE MIN (2) #2-2X10 DFL
-DBL ALL JST UNDER ISLAND
-SOILS IN THIS AREA COMMONLY HAVE A VERY HIGH SHRINK SWELL CAPACITY, OUR FIRM RECOMMENDS ALL SITES BE EVALUATED BY A GEOTECHNICAL FIRM PRIOR TO PLACEMENT OF FOUNDATIONS
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-FOUNDATION SHALL BE CONSTRUCTED PER JOHNSON COUNTY RESIDENTIAL FOUNDATION GUIDELINE. SEE ATTACHED
-ICE AND WATER SHIELD AS REQUIRED PER IRC

BRACED WALLS:
SEE CALCULATIONS ON SHEET S-2.0, PER ASC7-10 REQUIREMENTS AS ALLOWED BY IRC 2018 R301.2.1

ALL EXTERIOR WALLS SHALL BE SHEATHED PER ANY ONE OF THE FOLLOWING OPTIONS:
-7/16" APA-RATED PLYWOOD/OSB WITH 8d NAILS @ 6" O.C. AT EDGES AND @ 12" O.C. IN THE FIELD
-7/16" SHIPLAP PANEL SHEATHING (I.E. LP SMARTSIDE OR EQUIVALENT) WITH 8d NAILS @ 6" O.C. AT EDGES AND @ 12" O.C. IN THE FIELD
-3/8" SHIPLAP PANEL SHEATHING (I.E. LP SMARTSIDE OR EQUIVALENT) WITH 8d NAILS @ 4" O.C. AT EDGES AND @ 12" O.C. IN THE FIELD

INTERIOR BRACED WALL LOCATIONS ONLY SHOWN WHEN REQUIRED BY ADDITIONAL BRACING SECTION OF CALCULATIONS ON SHEET S-2.0



NAILING WITH SPACING AS SPECIFIED PER PLAN. FOR EXAMPLE, IF REQUIRED SPACING IS 4" O.C., BOTTOM LAP SHALL FIRST BE NAILED AT 4" O.C. (NAIL "A"), THEN FULL DEPTH SECTION OF OVERLAP PANEL SHALL BE NAILED @ 4" O.C. (NAIL "B")

3/8" APA REQUIRED NAILING PATTERN FOR SHIPLAP PANEL SHEATHING

SECOND FLOOR PLAN NOTES

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SAB-E718-Redbud-Loss

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STRUCTURAL DETAILS & NOTES

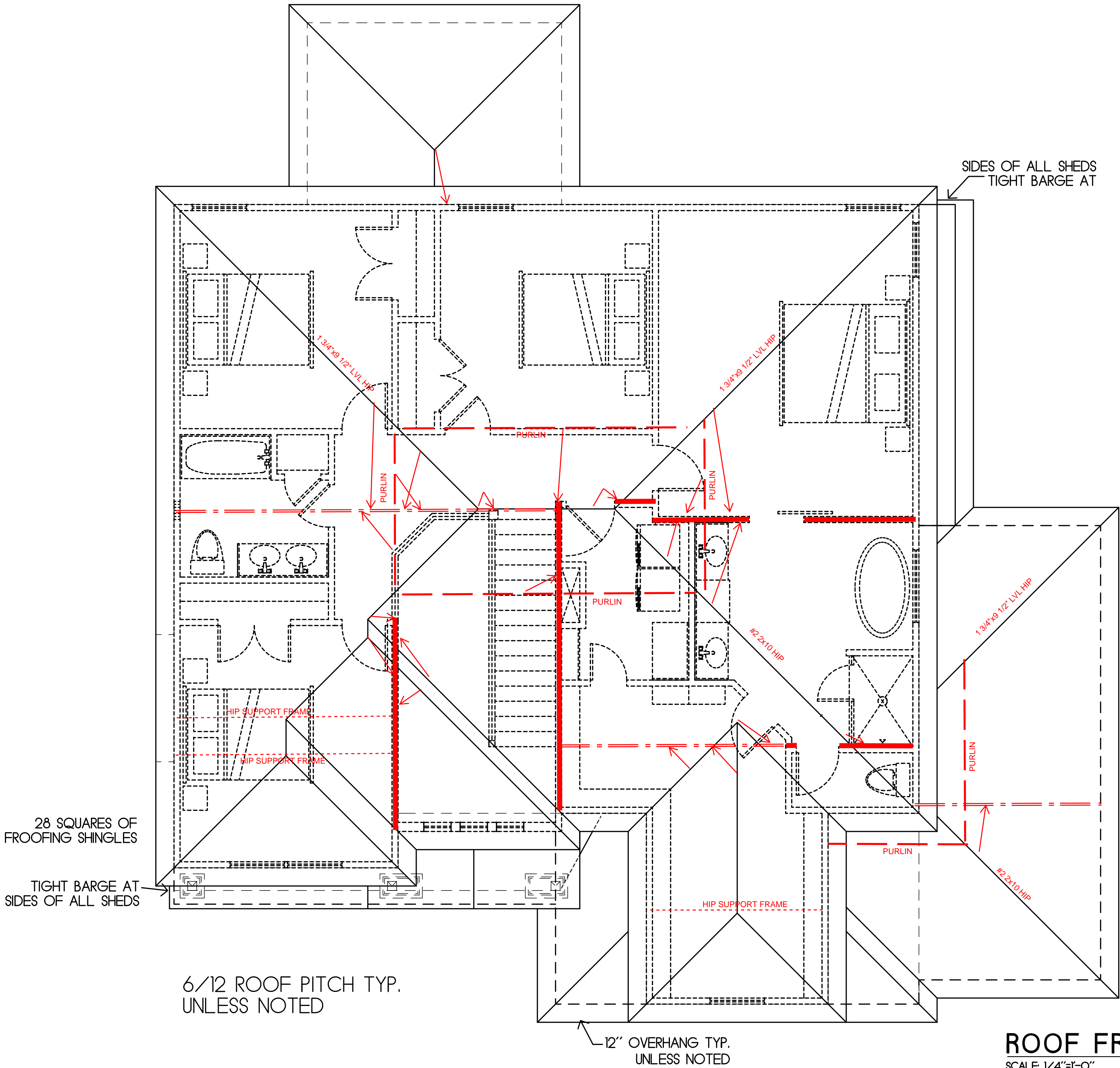
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PLANS DRAWN BY OTHERS

S-0.5



NOTES

ROOF DESIGNED FOR LIGHT ROOF COVERING 30PSF
TOTAL LOAD [10PSF DL, 20PSF LL (SL)]

RAFTERS (DOUG-FIR, OR EQUAL):
SEE SPAN CHARTS BELOW

| CODE MINIMUM | RAFTERS | SPACING | MAX HORIZONTAL CLEARSPAN |
|--------------|---------|-----------|--------------------------|
| | #2-2x6 | @24" O.C. | 11'-11" |
| | #2-2x6 | @16" O.C. | 14'-1" |
| | #2-2x8 | @24" O.C. | 15'-1" |
| | #2-2x8 | @16" O.C. | 18'-5" |
| | #2-2x10 | @24" O.C. | 18'-5" |
| | #2-2x10 | @16" O.C. | 22'-6" |

NOTE: CODE MINIMUM L/240 DEFLECTION

| GREATER THAN CODE | | |
|-------------------|-----------|--------------------------|
| RAFTERS | SPACING | MAX HORIZONTAL CLEARSPAN |
| #2-2x6 | @24" O.C. | 8'-6" |
| #2-2x6 | @16" O.C. | 9'-9" |
| #2-2x8 | @24" O.C. | 11'-3" |
| #2-2x8 | @16" O.C. | 12'-9" |
| #2-2x10 | @24" O.C. | 14'-3" |
| #2-2x10 | @16" O.C. | 16'-3" |

DEFLECTION = L/360 LIVE LOAD, L/240 TOTAL LOAD
VAULTS TO BE 2x10 DEPTH

ALL RIDGES, HIPs, AND VALLEYS NOT MARKED SHALL BE (1)
NOMINAL SIZE LARGER THAN THE INTERSECTING RAFTERS

PURLINS ARE 2x6 MIN.
PURLIN STRUTS ARE AT 4'-0" O.C.
PURLIN STRUTS SHALL BE INSTALLED AT NOT LESS
THAN A 45 DEGREE ANGLE WITH THE HORIZONTAL
ALL PURLINS STRUTS SHALL HAVE A MAXIMUM UNBRACED
LENGTH OF 8'-0"
PURLINS STRUTS SHALL BE CONSTRUCTED IN A "T"
CONFIGURATION AND PER THE FOLLOWING CHART

| PURLIN STRUT | MAX PURLIN STRUT LENGTH |
|--------------------|-------------------------|
| (2) 2x4 | 8'-0" |
| (1) 2x4 & (1) 2x6 | 12'-0" |
| (1) 2x6 & (1) 2x8 | 20'-0" |
| (2) 2x6 & (1) 2x8 | 30'-0" |
| CONSULT ARCH/ENGR. | >30'-0" |

SEE DETAILS 1, 5, 6, 7, 11, 12, 13, & 14 ON S-1.2
FOR ROOF FRAMING AND INSULATION OPTIONS

— — — — — PURLIN
— — — — — LOAD BEARING WALL
— = — = — LOAD BEARING BEAM/
GIRDER PER PLAN

ALLOWABLE LOADS FOR PNEUMATIC OR MECHANICALLY DRIVEN NAILS AND STAPLES

| FASTENER DESCRIPTION | NAIL GUN NAILS/ WIRE DIA. | WIRE GA. | PENETRATION REQUIRED INTO MAIN MEMBER FOR LATERAL STRENGTH (IN.) | ALLOWABLE LOADS (IN POUNDS) | | | |
|-----------------------|---------------------------|----------|--|-----------------------------|------|---------------------|------|
| | | | | LATERAL STRENGTH | | WITHDRAWAL STRENGTH | |
| | | | | SP | DF/L | SP | DF/L |
| 16 GA. STAPLE | .063 | 16 | 1 | 51 | | 36 | 32 |
| 15 GA. STAPLE | .072 | 15 | 1 | 64 | | 42 | 37 |
| 14 GA. STAPLE | .080 | 14 | 1 | 75 | | 46 | 41 |
| 6d COOLER NAIL | .092 | 13 | 1 | 46 | | 27 | 23 |
| 6d SINKER NAIL | | | | | | | |
| 6d BOX NAIL | .099 | 12-1/2 | 1-1/8 | 61 | 55 | 31 | 24 |
| 6d CASING NAIL | | | | | | | |
| 7d COOLER NAIL | | | | | | | |
| 6d COMMON NAIL | .113 | 11-1/2 | 1-1/4 | 79 | 72 | 35 | 28 |
| 8d COOLER NAIL | | | | | | | |
| 8d SINKER NAIL | | | | | | | |
| 8d BOX NAIL | | | | | | | |
| 8d CASING NAIL | .120 | 11 | 1-3/8 | 89 | 81 | 41 | 32 |
| 6d RING SHANK NAIL | | | | | | | |
| 6d SCREW SHANK NAIL | | | | | | | |
| 8d RING SHANK NAIL | | | | | | | |
| 8d SCREW SHANK NAIL | .128 | 10-1/2 | 1-1/2 | 89 | 81 | 36 | 31 |
| 10d Cooler Nail | | | | | | | |
| 10d Sinker Nail | | | | | | | |
| 12d Short | | | | | | | |
| 10d Box Nails | .128 | 10-1/2 | 1-1/2 | 101 | 93 | 40 | 31 |
| 12d Box Nails | | | | | | | |
| 10d Casing Nails | .131 | 10-1/4 | 1-1/2 | 106 | 97 | 41 | 32 |
| 8d Common Nails | | | | | | | |
| 16d Short | | | | | | | |
| 12d Sinkers | | | | | | | |
| 16d Box Nails | .135 | 10 | 1-1/2 | 113 | 103 | 42 | 33 |
| 10d Ring Shank Nails | .135 | 10 | 1-5/8 | 113 | 103 | 46 | 36 |
| 10d Screw Shank Nails | | | | | | | |
| 12d Ring Shank Nails | | | | | | | |
| 12d Screw Shank Nails | | | | | | | |
| 10d Common Nails | .148 | 9 | 1-5/8 | 128 | 118 | 46 | 36 |
| 12d Common Nails | | | | | | | |
| 16d Sinker Nails | | | | | | | |
| 20d Box Nails | | | | | | | |
| 30d Box Nails | .148 | 9 | 1-3/4 | 128 | 118 | 50 | 40 |
| 16d Ring Shank Nails | | | | | | | |
| 16d Screw Shank Nails | | | | | | | |
| 16d Common Nails | | | | | | | |
| 40d Box Nails | .162 | 8 | 1-3/4 | 154 | 141 | 50 | 40 |
| 20d Ring Shank Nails | .177 | 7 | 2-1/8 | 178 | 163 | 59 | 47 |
| 20d Screw Shank Nails | .177 | 7 | 2-1/8 | 178 | 163 | 54 | 43 |
| 20d Sinker Nails | | | | | | | |
| 20d Common Nails | | | | | | | |
| 30d Sinker Nails | | | | | | | |

SHEATHING SCHEDULE

ALL SHEATHING MATERIALS TO BE APPLIED PERPENDICULAR TO JOISTS AND ENDS STAGGERED

| BUILDING COMPONENT | MATERIAL | FASTENING |
|-------------------------|--|---|
| ROOF SHEATHING | 7/16" PLYWOOD | 16 GA X 1 3/4" STAPLES @ 6" OC EDGES & 12" OC IN FIELD |
| | 1x 4 #3 FURRING | 1/2" CROWN STAPLES |
| FLOOR SHEATHING | 3/4" T&G YELLOW PINE PLYWOOD | 14 GA X 1 3/4" STAPLES @ 6" OC EDGES & 12" OC IN FIELD |
| | | 12.5 GA X 1 1/2" RING OR SCREW SHANK NAILS @ 6" OC EDGES & 12" OC IN FIELD |
| WALL COVERING | 1/2" GYPSUM SHEATHING | 6D COMMON NAILS : 1 5/8" GALVANIZED STAPLES; 1 1/4" SCREWS, TYPE W OR S @ 4" OC EDGES & 8" OC IN FIELD |
| CEILING COVERING | 1/2" GYPSUM SHEATHING | 7" OC NAILED / 12" OC SCREWED W/ 13GA, 1 3/8" LONG, 19/64" HEAD; 0.098 Ø, 1 1/4" LONG, ANG-RINGED; 5D COOLER NAIL, 0.086 Ø, 1 5/8" LONG, 15/64" HEAD; OR GYP BD NAIL, 0.086 Ø, 1 5/8" LONG, 19/64" HEAD |
| EXTERIOR WALL SHEATHING | 7/16" APA RATED SHEATHING | 8D COMMON NAILS @ 6" OC EDGES & 12" OC IN THE FIELD |
| | RATED PANEL SIDING, RATED 16" O.C. 7/16" THICK | 8D BOX OR SINKER NAILS @ 6" OC EDGES & 12" OC IN THE FIELD |

FRAME FASTENING SCHEDULE

| BUILDING COMPONENT | FASTEN TO | FASTEN WITH |
|--------------------|---|---|
| | | |
| RAFTERS | RIDGE / VALLEY / HIP PLATE | TOENAIL W/ (4) 16D, FACENAIL W/ (3) 16D TOENAIL W/ (3) 10D |
| | LEDGER STRIPS SUPPORTING JOISTS OR RAFTERS | FACENAIL W/ (3) 16D |
| | COLLAR TIE TO RAFTERS | FACENAIL W/ (3) 10D |
| CEILING JOISTS | TOP PLATE | TOENAIL W/ (3) 8D @ EACH END |
| | WHERE CLG JST RUN PARALLEL TO RAFTERS | FACENAIL TO RAFTERS W/ (3) 10D MINIMUM |
| | LAPS OVER PARTITIONS | FACENAIL W/ (3) 10D |
| | BLOCKING BETWEEN JOISTS OR RAFTERS TO TOP PLATE | TOENAIL W/ (3) 8D |
| BEAMS | BUILT-UP BEAMS, 2" LUMBER LAYERS, FACENAIL OPPOSITE SIDES, (2) @ EACH END PLUS | 10D @ 32" OC STAGGERED, TOP & BOTTOM, OPPOSITE SIDES |
| | BUILT-UP BEAMS OF ENGINEERED LUMBER, FACE NAIL OPPOSITE SIDES | (2) ROWS @ 12" OC |
| | BUILT-UP HEADER, TWO PIECES W/ 1/2" SPACER | 16D @ 16" OC ALONG EDGES |
| | BUILT-UP HEADER, TWO PIECES, NO 1/2" SPACER | 3" x 0.131" NAILS @ 12" OC ALONG EDGES |
| FLOOR JOISTS | BEARING | TOENAIL W/ (2) 18D @ EACH END |
| | RIM JOIST TO SILL OR TOP PLATE | TOENAIL W/ 8D COMMON OR 10D BOX NAILS @ 6" OC |
| | JOIST TO SILL OR GIRDER | TOENAIL W/ (3) 8D |
| | JOIST TO RIM JOIST | FACENAIL W/ (3) 16D |
| | BRIDGING TO JOIST | TOENAIL W/ (2) 8D |
| | I-JOIST TO BEARING PLATE | TOENAIL W/ (2) 8D - ONE INTO EACH SIDE AT LEAST 1 1/2" FROM THE END |
| | RIM JOIST TO I-JOIST | FACENAIL W/ (2) 10D BOX NAILS - ONE INTO EACH FLANGE |
| | SOLE PLATE TO LSL RIM BOARD | 16D BOX NAILS @ 12" OC |
| | SINGLE JOIST HANGERS * | 10D FACENAILS AND TOENAILS |
| | DOUBLE JOIST HANGERS * | 16D FACENAILS AND TOENAILS |
| WALLS | TOP & SOLE PLATE TO STUD | END NAIL W/ (2) 16D |
| | STUD TO SOLE AND TOP PLATE | TOENAIL W/ (4) 8D |
| | DOUBLE TOP PLATES | FACENAIL W/ 16D @ 16" OC |
| | DOUBLE TOP PLATE LAP SPLICE | FACENAIL W/ (8) 16D |
| | TOP PLATE LAPS & INTERSECTIONS | FACENAIL W/ (2) 16D |
| | DOUBLE STUDS | FACENAIL W/ 16D @ 24" OC |
| | BUILT-UP CORNER STUDS | FACENAIL W/ 16D - 2 ROWS @ 24" OC |
| | STEEL "X" BRACING | FACENAIL W/ (2) 16D IN EACH TOP & BOTTOM PLATE & (1) 8D PER STUD |
| | SOLE PLATE TO JOIST OR BLOCKING | FACENAIL W/ 16D @ 16" OC |
| | SOLE PLATES TO JOIST OR BLOCKING AT BRACED WALL LINES, PERPENDICULAR TO FRAMING | FACENAIL W/ (3) 16D @ 16" OC ALONG BRACED WALL PANEL |
| | TOP PLATE TO JOIST OR BLOCKING AT BW LINES, PERPENDICULAR TO FRAMING | TOENAIL W/ 8D @ 6" OC ALONG BRACED WALL PANEL |
| | SOLE PLATES TO JOIST OR BLOCKING AT BW LINES PARALLEL TO FRAMING, BLOCKING @ 16" OC | FACENAIL W/ (3) 16D @ 16" OC ALONG BW PANEL & AT EACH BLOCK |
| | TOP PLATE TO JOIST OR BLOCKING AT BW LINES, PARALLEL TO FRAMING, BLOCKING @ 16" OC | TOENAIL W/ 8D @ 6" OC ALONG BW PANEL & AT EACH BLOCK |
| | NON-STRUCT. SIDING OVER STRUCT. SHEATHING | (1) 6D BOX NAIL IN EACH STUD |
| | FIBER CEMENT PLANK SIDING | (1) 6D GALVANIZED NAIL IN EACH STUD |
| | WINDOW INSTALLATION NAILING | 1 3/4" - 2" ROOFING NAILS @ 12" OC MAX. |

* JOIST HANGER NOTES: 1) NO JOIST HANGER NAILS ALLOWED FOR TOENAILS, 2) NO GUN NAILS OR SCREWS ALLOWED IN CONNECTORS, 3) TOENAILS SHALL ALWAYS BE A FULL 3" OR 3.5" NAIL

COLUMN CONNECTION TO STEEL BEAMS SHALL BE WITH A CLIP POST CAP WITH ALL FOUR TAB ENDS BENT AROUND THE BOTTOM FLANGE OF THE BEAM. FOR A BEARING PLATE, FOUR HOLES SHALL BE DRILLED IN THE BOTTOM FLANGE OF THE STEEL BEAM TO MATCH THE HOLE PATTERN OF THE PLATE. 1/2"x2" BOLTS SHOULD THEN BE INSTALLED WITH A FLAT WASHER, LOCK WASHER, AND A NUT IN EACH OF THE HOLES. THE POST CAP MAY BE WELDED TO THE STEEL BEAM IN ACCORDANCE WITH AWS D1.1-92 AS AN ALTERNATIVE, AND WOULD NEED TO BE INSPECTED BY AN AWS-CERTIFIED INSPECTOR.

DUCT SEALING METHOD, PER IRC2018 W1103.3.2

N1103.2.2 (R403.2.2) SEALING (MANDATORY) DUCTS, AIR HANDLERS, AND FILTER BOXES SHALL BE SEALED. JOINTS AND SEAMS SHALL COMPLY WITH SECTION M1601.4.1 OF THIS CODE.

EXCEPTIONS:

- AIR-IMPERMEABLE SPRAY FOAM PRODUCTS SHALL BE PERMITTED TO BE APPLIED WITHOUT ADDITIONAL JOINT SEALS.
- WHERE A DUCT CONNECTION IS MADE THAT IS PARTIALLY INACCESSIBLE, THREE SCREWS OR RIVETS SHALL BE EQUALLY SPACED ON THE EXPOSED PORTION OF THE JOINT SO AS TO PREVENT A HINGE EFFECT.
- CONTINUOUSLY WELDED AND LOCKING-TYPE LONGITUDINAL JOINTS AND SEAMS IN DUCTS OPERATING AT STATIC PRESSURE LESS THAN 2 INCHES OF WATER COLUMN (500 Pa) PRESSURE CLASSIFICATION SHALL NOT REQUIRE ADDITIONAL CLOSURE SYSTEMS.
- DUCT TIGHTNESS SHALL BE VERIFIED BY EITHER OF THE FOLLOWING:
 - POST CONSTRUCTION TEST: TOTAL LEAKAGE SHALL NOT BE LESS THAN OR EQUAL TO 4 CFM (113.3 L/MIN) PER 100FT² (9.29m²) OF CONDITIONED FLOOR AREA WHEN TESTED AT A PRESSURE DIFFERENTIAL OF 0.1 INCHES W.G. (25 Pa) ACROSS THE ENTIRE SYSTEM, INCLUDING THE MANUFACTURER'S AIR HANDLER ENCLOSURE. ALL REGISTER BOOTS SHALL BE TAPED OR OTHERWISE SEALED DURING THE TEST.
 - ROUGH-IN TEST: TOTAL AIR LEAKAGE SHALL BE LESS THAN OR EQUAL TO 4 CFM (113.3 L/MIN) PER 100FT² (9.29m²) OF CONDITIONED FLOOR AREA WHEN TESTED AT A PRESSURE DIFFERENTIAL OF 0.1 INCHES W.G. (25 Pa) ACROSS THE ENTIRE SYSTEM, INCLUDING THE MANUFACTURER'S AIR HANDLER ENCLOSURE. ALL REGISTERS SHALL BE TAPED OR OTHERWISE SEALED DURING THE TEST. IF THE AIR HANDLER IS NOT INSTALLED AT THE TIME OF THE TEST, TOTAL AIR LEAKAGE SHALL BE LESS THAN OR EQUAL TO 3 CFM (85 L/MIN) PER 100FT² (9.29m²) OF CONDITIONED FLOOR AREA.
- EXCEPTION: THE TOTAL LEAKAGE IS NOT REQUIRED FOR DUCTS AND AIR HANDLERS LOCATED ENTIRELY WITHIN THE BUILDING THERMAL ENVELOPE.

GENERAL NOTES:

- PLANS SHALL COMPLY WITH THE 2018 INTERNATIONAL RESIDENTIAL CODE, IECC AS ADOPTED BY AHJ, AND ALL AMENDMENTS AS ADOPTED BY THE AHJ. IF ANY CHANGES OR DEVIATIONS ARE MADE FROM THESE PLANS THE CONTRACTOR SHALL NOTIFY THE APPROPRIATE AUTHORITY AND THE ENGINEER TO EVALUATE THE CHANGES AND MAKE ANY APPROPRIATE MODIFICATIONS TO THE PLANS.
- WHERE DISCREPANCIES EXIST BETWEEN THE STANDARD COMMENTS, NOTES FOR THE DESIGN PROFESSIONAL OR THE CODE, THE MOST RESTRICTIVE SHALL APPLY.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING THE OWNER, ARCHITECT AND THE AHJ WITH A SET OF PLANS THAT MEET AHJ AND CODE REQUIREMENTS FOR A SINGLE SITE CONSTRUCTION PROJECT. UNLESS REQUESTED BY OUR CLIENT, CODE/AHJ MINIMUM DESIGNS WILL BE UTILIZED. ALSO, UNLESS REQUESTED BY THE OWNER, OUR FIRM CAN NOT AND WILL NOT BE AUTHORIZED TO VISIT THE SITE TO EVALUATE THE SITE OR ANY CONSTRUCTION FOR THIS PROJECT. IMPLEMENTATION OF ALTERNATES TO THE DESIGNS INCLUDING BUT NOT LIMITED TO PIER DESIGNS, FOUNDATION ALTERATIONS, OR ANY STRUCTURAL CHANGES NOT PROVIDED BY HD ENGINEERING OR A PROFESSIONAL REFERRED BY HD ENGINEERING SHALL RELEASE HD ENGINEERING FROM ALL LIABILITY ASSOCIATED WITH THIS DESIGN.
- OUR FIRM HIGHLY RECOMMENDS THAT ANY SITE WITH GREATER THAN A 15% GRADE, ANY SITE WHERE A PREVIOUS STRUCTURE WAS LOCATED, OR ANY SITE WITH POTENTIAL FILL MATERIAL OR A POTENTIAL SOIL BEARING CAPACITY BELOW 1500 PSF SHOULD BE EVALUATED BY OUR FIRM OR AN HD ENGINEERING REFERRED GEOTECHNICAL FIRM PRIOR TO PLACING FOOTINGS. THE ATTACHED PLANS HAVE BEEN DESIGNED WITH THE UNDERSTANDING THAT OUR FIRM HAS NOT AND CAN NOT VISIT OR INSPECT THE SITE WITHOUT WRITTEN CONSENT/REQUEST OF THE OWNER/BUILDER. DUE TO THIS FACT OUR FIRM CAN ONLY DESIGN THE ATTACHED PLANS TO CERTAIN CODE REQUIREMENTS WHICH ARE DETAILED THROUGHOUT THE PLAN AND ATTACHED DETAIL SHEETS, IF THE OWNER DESIRES GREATER THAN CODE DESIGNS THAT REQUEST MUST BE MADE CLEARLY AND IN WRITING PRIOR TO ENGINEERING OF THE PLAN.
- DUE TO THE WIDE VARIETY OF SOIL CONDITIONS IN OUR AREA AND THE WIDE VARIETY OF PLASTICITY INDEX AND SOIL BEARING CAPACITIES OUR FIRM RECOMMENDS ALL SITES BE EVALUATED BY HD ENGINEERING OR AN HD ENGINEERING REFERRED GEOTECHNICAL FIRM PRIOR TO PLACEMENT OF ANY "STANDARD" FOUNDATIONS.

FOUNDATION NOTES:

- THE FOUNDATION DESIGN SHALL COMPLY WITH THE ENFORCING JURISDICTION RESIDENTIAL FOUNDATION STANDARD IN LIEU OF ENGINEERING REPORT REQUIREMENTS BASED ON ACTUAL SITE CONDITIONS.
- FOUNDATION WALLS SHALL BE DAMP-PROOFED PER IRC SECTION R406.
- PROVIDE A MINIMUM 4" PERFORATED DRAIN AROUND USABLE SPACE BELOW GRADE OR OTHER EQUIVALENT MATERIALS PER IRC SECTION 405.1. THE PIPE SHALL BE COVERED WITH NOT LESS THAN 6" OF WASHED GRAVEL OR CRUSHED ROCK. THE DRAIN SHALL DAYLIGHT TO THE EXTERIOR BELOW THE FLOOR LEVEL OR TERMINATE IN A MINIMUM 20 GALLON SUMP PIT.
- FOUNDATION DESIGN SHALL BE BASED ON A MINIMUM SOIL BEARING CAPACITY OF 1500 PSF.
- FOOTINGS SHALL BE A MIN. OF 16" WIDE AND 8" DEEP W/ (2) #4 BARS CONTINUOUS, LOCATED A MIN. OF 3" CLEAR FROM BOTTOM. FOOTINGS SHALL BE A MINIMUM OF 36" BELOW GRADE FOR FROST PROTECTION.
- COLUMN PADS SHALL BE A MINIMUM OF 24"x24"x8" WITH (3) #4 BARS EACH WAY.
- FOUNDATION WALLS SHALL BE A MINIMUM 8" THICK W/ MINIMUM #4 BARS @ 24" O.C. HORIZONTAL AND VERTICAL W/ THE TOP BAR WITHIN 8" OF THE TOP OF THE WALL UNLESS NOTED OTHERWISE ON PLAN.
- REINFORCEMENT SHALL LAP A MINIMUM OF 24"
- INTERIOR BEARING WALLS AND COLUMNS SHALL BE ISOLATED FROM THE BASEMENT FLOOR SLAB.
- INTERIOR NON-BEARING WALLS, OTHER THAN THOSE RESTING DIRECTLY ON THE FOOTING, SHALL BE ISOLATED FROM THE FLOOR FRAMING ABOVE BY A SEPARATION OF 1/2".
- CONCRETE FLOOR SLABS ON GRADE, SHALL BE A MINIMUM 4" THICK OVER A MINIMUM 4" BASE OF SAND, GRAVEL, OR CRUSHED STONE. BASEMENT SLABS SHALL HAVE A MIN. 6 MIL POLYETHYLENE OR APPROVED VAPOR RETARDER WITH JOINTS LAPPED NOT LESS THAN 6" SHALL BE PLACED BETWEEN THE FLOOR SLAB AND THE BASE COURSE.
- FLOOR SLABS SUPPORTED BY FILL CONSISTING OF MORE THAN 24" OF GRANULAR FILL OR 8" OF EARTH SHALL BE REINFORCED PER A SEPARATE ENGINEERING DESIGN.
- BASEMENT FOUNDATION SILL PLATES SHALL BE BOLTED TO THE FOUNDATION W/ A MINIMUM OF 1/2" ANCHOR BOLTS EMBEDDED AT LEAST 7" INTO THE CONCRETE AND SPACED NOT MORE THAN 3' ON CENTER AND WITHIN 12" OF EACH END PIECE PER IRC SECTION R403.1.6.
- FOUNDATION WINDOW WELLS FOR SECONDARY MEANS OF EGRESS SHALL PROVIDE A MINIMUM 3'X3' HORIZONTAL AREA.
- THE BASE OF ALL FOOTING EXCAVATIONS SHOULD BE FREE OF ALL WATER AND LOOSE MATERIAL PRIOR TO PLACING CONCRETE. CONCRETE SHOULD BE PLACED AS SOON AS POSSIBLE AFTER EXCAVATING SO THAT EXCESSIVE DRYING OR DISTURBANCE OF BEARING MATERIALS DOES NOT OCCUR. SHOULD THE MATERIALS AT BEARING LEVEL BECOME EXCESSIVELY DRY OR SANDY, WE RECOMMEND THAT THE AFFECTED MATERIAL BE REMOVED PRIOR TO PLACING CONCRETE.
- IT IS RECOMMENDED THAT ALL FOOTING EXCAVATIONS BE EVALUATED AND TESTED BY A GEOTECHNICAL ENGINEER IMMEDIATELY PRIOR TO PLACEMENT OF FOUNDATION CONCRETE. UNSUITABLE AREAS IDENTIFIED AT THIS TIME SHOULD BE CORRECTED. CORRECTIVE PROCEDURES WOULD BE DEPENDENT UPON CONDITIONS ENCOUNTERED AND MAY INCLUDE DEEPENING OF FOUNDATION ELEMENTS, OR UNDERCUTTING OF UNSUITABLE MATERIALS AND REPLACEMENT WITH ENGINEERED FILL.

STAIRWAY NOTES:

- STAIRWAYS SHALL PROVIDE A MAXIMUM 7 3/4" RISE AND MIN. 10" RUN.
- PROVIDE MINIMUM 36" GUARDRAILS ON THE OPEN SIDES OF RAISED FLOORS, PORCHES AND BALCONIES. MINIMUM 34" GUARDRAILS ON THE OPEN SIDES OF STAIRWAYS LOCATED MORE THAN 30" ABOVE THE FLOOR OR GRADE BELOW. GUARDRAIL ENCLOSURES SHALL HAVE INTERMEDIATE RAILS OR ORNAMENTAL PATTERNS THAT DO NOT ALLOW PASSAGE OF A SPHERE 4" IN DIAMETER.
- EACH STAIRWAY OF 3 OR MORE RISERS SHALL PROVIDE A CONTINUOUS HANDRAIL ON AT LEAST ONE SIDE BETWEEN 34" AND 38" ABOVE THE NOSING OF THE TREADS. HANDRAILS SHALL HAVE A CIRCULAR CROSS-SECTION OF 1 1/4" MINIMUM TO 2" MAXIMUM OR OTHER APPROVED GRASPABLE SHAPE PER IRC SECTION R311.7.8.5
- PROVIDE A MINIMUM 6'-8" OF HEADROOM CLEARANCE IN STAIRWAYS.
- ENCLOSED ACCESSIBLE SPACE UNDER STAIRWAYS SHALL HAVE WALLS AND THE UNDERSIDE OF THE STAIR AND LANDING PROTECTED WITH 1/2" GYPSUM BOARD ON ENCLOSURE SIDE.
- WINDERS SHALL PROVIDE A MINIMUM TREAD OF AT LEAST 6" AT ANY POINT WITHIN CLEAR WIDTH OF STAIRS. WINDER TREAD PROPORTION TO COMPLY WITH IRCR311.7.5.2.1.

GLAZING NOTES:

- GLAZING IN HAZARDOUS LOCATIONS AS IDENTIFIED IN IRC SECTION R308.4 SHALL BE OF APPROVED SAFETY GLAZING MATERIALS. GLASS IN STORM DOORS, INDIVIDUAL FIXED OR OPERABLE PANELS ADJACENT TO A DOOR WHERE THE NEAREST VERTICAL EDGE IS WITHIN A 24" ARCH OF THE DOOR IN A CLOSED POSITION AND WHOSE BOTTOM EDGE IS WITHIN 60" OF THE FLOOR, WALLS ENCLOSING STAIRWAYS AND LANDINGS WHERE THE GLAZING IS WITHIN 60" OF THE TOP OR BOTTOM OF THE STAIR, ENCLOSURES FOR SPAS, TUBS, SHOWERS AND WHIRLPOOLS, GLAZING IN FIXED OR OPERABLE PANELS EXCEEDING 9 S.F. AND WHOSE BOTTOM EDGE IS LESS THAN 18" ABOVE THE FLOOR OR WALKING SURFACE WITHIN 36"
- IN DWELLING UNITS, WHERE THE OPENING OF AN OPERABLE WINDOW IS LOCATED MORE THAN 72 INCHES ABOVE THE FINISHED GRADE OR SURFACE BELOW, THE LOWEST PART OF THE CLEAR OPENING OF THE WINDOW SHALL BE A MINIMUM OF 24 INCHES ABOVE THE FINISHED FLOOR OF THE ROOM IN WHICH THE WINDOW IS LOCATED. OPERABLE SECTIONS OF WINDOWS SHALL NOT PERMIT OPENINGS THAT ALLOW PASSAGE OF A 4 INCH DIAMETER SPHERE WHERE SUCH OPENINGS ARE LOCATED WITHIN 24 INCHES OF THE FINISHED FLOOR.

FRAMING NOTES:

- ALL LUMBER SIZES ARE FOR DOUGLAS FIR-LARCH UNLESS OTHERWISE NOTED.
- ALL HEADERS TO BE A MINIMUM OF (2) #2-2X10'S UNLESS OTHERWISE NOTED.
- BLOCK CANTILEVERS, DOOR JAMBS, AND OVER BEAMS.
- ALL HEADERS/BEAMS TO BEAR ON A MINIMUM OF (1) 2X4 POSTS UNLESS NOTED OTHERWISE.
- INTERIOR NON-BEARING WALLS, OTHER THAN THOSE RESTING DIRECTLY ON THE FOOTING SHALL BE ISOLATED FROM THE FLOOR FRAMING ABOVE.
- WHERE JOISTS RUN PARALLEL TO FOUNDATION WALLS, SOLID BLOCKING FOR A MINIMUM OF (2) JOIST SPACES SHALL BE PROVIDED AT A MAXIMUM OF 4' CENTERS TO TRANSFER LATERAL LOADS ON THE WALL TO THE FLOOR DIAPHRAGM. THE BLOCKING SHALL BE SECURELY NAILED TO THE JOISTS AND FLOORING. NAIL JOISTS AND BLOCKING TO SILL PLATE WITH (4) 10D NAILS.
- IF DUCTS ARE INSTALLED IN THE FIRST JOIST SPACE(S), NAIL 2X4'S FLAT AT 4' CENTERS WITHIN THE JOIST SPACE(S) AND THEN PROVIDE SOLID BLOCKING, INSTALLED UPRIGHT, IN THE NEXT TWO JOIST SPACES. SECURE THE 2X4'S TO THE SILL PLATE WITH (4) 10D NAILS.
- ALL SILLS AND SLEEPERS SUPPORTED ON CONCRETE OR MASONRY AND FURRING ATTACHED TO CONCRETE OR MASONRY SHALL BE OF DECAY RESISTANT MATERIALS.
- JOISTS UNDER BEARING PARTITIONS SHALL BE SIZED TO CARRY THE DESIGN LOAD IN ACCORDANCE WITH IRC SECTION R502.4.
- JOISTS FRAMING FROM OPPOSITE SIDES OVER BEARING SUPPORTS SHALL LAP A MINIMUM OF 3" AND SHALL BE NAILED TOGETHER WITH A MINIMUM 10D FACE NAILS.
- JOISTS FRAMING INTO A WOOD GIRDER OR BEAM SHALL BE SUPPORTED BY APPROVED FRAMING ANCHORS OR ON MINIMUM 2"x2" LEDGER STRIPS.
- HEADER AND TRIMMERS SHALL BE OF SUFFICIENT CROSS SECTION TO SUPPORT THE FLOOR FRAMING. TRIMMER JOISTS SHALL BE DOUBLED WHEN THE HEADER IS SUPPORTED MORE THAN 3' FROM THE TRIMMER JOIST BEARING. WHEN THE HEADER SPAN EXCEEDS 4', THE HEADER AND TRIMMER SHALL BE DOUBLED.
- JOISTS AT SUPPORTS SHALL BE SUPPORTED Laterally AT THE ENDS BY FULL-DEPTH SOLID BLOCKING NOT LESS THAN 2" NOMINAL THICKNESS OR BY ATTACHMENT TO A HEADER, BAND OR RIM JOIST OR TO AN ADJOINING STUD OR OTHERWISE PROVIDED WITH LATERAL SUPPORT TO PREVENT ROTATION.
- ALL WALL COVERINGS TO COMPLY WITH IRC SECTION 702 AND 703
- ALL RAFTER / COLLAR TIES TO COMPLY WITH IRC SECTIONS 804
- ALL RAFTERS TO HAVE 2x4 COLLAR TIES @ 48" OC IN UPPER 1/3 OF DISTANCE BETWEEN CEILING AND ROOF
- BLOCKING BETWEEN JOISTS UNDER A PERPENDICULAR LOAD-BEARING WALL IS NOT REQUIRED.
- BOTTOM OF ALL FLOOR ASSEMBLIES SHALL BE PROVIDED WITH A 1/2" GYPSUM WALLBOARD MEMBRANE (IF REQUIRED BY LOCAL CODE)
- I-JOIST AND FLOOR TRUSS SYSTEMS SHALL BE FIRE PROTECTED PER IRC AS ADOPTED BY AHJ
- STUDS SHALL BE CONTINUOUS FROM THE FLOOR TO THE ROOF/ CEILING DIAPHRAGM PER IRC 602.3

CONCRETE NOTES:

- CONCRETE SHALL BE AIR-ENTRAINED (5%-7%) WITH A MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS OF 2500 PSI FOR BASEMENT AND INTERIOR FLOOR SLABS, 3000 PSI FOR BASEMENT AND FOUNDATION WALLS AND 3500 PSI FOR PORCHES, CARPORTS AND GARAGE FLOOR SLABS.

EMERGENCY EGRESS AND RESCUE NOTES:

- PROVIDE ONE WINDOW FOR EACH BEDROOM THAT HAS A MINIMUM OPENABLE AREA OF 5.7 S.F. WITH A MINIMUM OPENABLE HEIGHT OF 24" AND WIDTH OF 21". IN ADDITION, THE OPENABLE PORTION OF EGRESS WINDOWS SHALL NOT EXCEED 44" ABOVE THE ADJOINING FLOOR OR PERMANENT STEP.
- PROVIDE SMOKE ALARMS IN EACH SLEEPING ROOM, OUTSIDE OF EACH SLEEPING AREA AND ON EACH FLOOR INCLUDING BASEMENTS. ALARMS SHALL BE INTERCONNECTED IN SUCH A MANNER THAT THE ACTIVATION OF ONE ALARM WILL ACTIVATE ALL OF THE ALARMS IN THE DWELLING.
- PROVIDE CARBON MONOXIDE ALARMS AS REQUIRED PER IRC. CARBON MONOXIDE ALARMS SHALL BE INSTALLED OUTSIDE OF EACH SEPARATE SLEEPING AREA, WHERE FIRE-BURNING APPLIANCES ARE LOCATED WITHIN A BEDROOM OR ITS ATTACHED BATHROOM, A CARBON MONOXIDE ALARM SHALL BE INSTALLED IN THE BEDROOM.

GARAGE NOTES:

- THE GARAGE FLOOR SHALL SLOPE TOWARDS THE GARAGE DOORWAYS OR SLOPE TO A TRENCH OR UNTR

TABLE R602.3(1)
FASTENER SCHEDULE FOR STRUCTURAL MEMBERS

| ITEM | DESCRIPTION OF BUILDING ELEMENTS | NUMBER AND TYPE OF FASTENER ^{a,b,c} | SPACING OF FASTENERS |
|-------|--|---|---|
| ROOF | | | |
| 1 | BLOCKING BETWEEN JOISTS OR RAFTERS TO TOP PLATE, TOE NAIL | 4-8D BOX (2 1/2" X 0.113") | TOE NAIL |
| 2 | CEILING JOISTS TO PLATE, TOE NAIL | 3-8D (2 1/2" X 0.113") 3-10D (3"X0.128") 3-3"X 0.131" NAILS | PER JOIST, TOE NAIL |
| 3 | CEILING JOISTS NOT ATTACHED TO PARALLEL RAFTER, LAPS OVER PARTITIONS (SEE SECTION R802.5.2 AND TABLE R802.52 | 4-10D BOX (3"X 0.128") 3-16D COMMON (3 1/2"X 0.162") 4-3"X 0.131"NAILS | FACE NAIL |
| 4 | CEILING JOIST ATTACHED TO PARALLEL RAFTER (HEEL JOINT) SEE SECTION R802.5.2 AND TABLE R802.5.2) | TABLE R802.5.2 | FACE NAIL |
| 5 | COLLAR TIE TO RAFTER, FACE NAIL OR 1 1/4" X 20GA. RIDGE STRAP TO RAFTER | 4-10D BOX (3" X 0.128") 3-10D COMMON (3" X 0.148") 4-3" X 0.131" NAILS | FACE NAILS EACH RAFTER |
| 6 | RAFTER OR ROOF TRUSS TO PLATE | 3-16D BOX NAILS (3 1/2" X0.135") 3-10D COMMON NAILS (3" X 0.148") 4-10D BOX (3" X 0.128") 4-3" X0.131" NAILS | 2 TOE NAILS ON ONE SIDE AND 1 TOE NAIL ON OPPOSITE SIDE OF EACH RAFTER OR TRUSS ¹ |
| 7 | ROOF RAFTERS TO RIDGE, VALLEY OR HIP RAFTERS OR ROOF RAFTER TO MINIMUM 2" RIDGE BEAM | 4-16D(3 1/2" X 0.135"); OR 3-10D COMMON (3" X 0.148") 4-10D BOX (3" X 0.128"); OR 4-3" X 0.131" NAILS | TOE NAIL |
| | | 3-16D(3 1/2" X0.135"); OR 2-16D COMMON (3 1/2" X0.162") 3-10D BOX (3" X 0.128"); OR 3-3" X 0.131" NAILS | |
| WALL | | | |
| 8 | STUD TO STUD (NOT BRACED WALL PANELS) | 16D (3 1/2" X 0.162") | 24" OC FACE NAIL |
| | | 10D BOX (3" X 0.128"); OR 3" X 0.131" NAILS | 16" OC FACE NAIL |
| 9 | STUD TO STUD AND ABUTTING STUDS AT INTERSECTING WALL CORNERS (AT BRACED WALL PANELS) | 16D BOX (3 1/2" X 0.135"); OR 3" X 0.131" NAILS | 12" OC FACE NAIL |
| | | 16D COMMON (3 1/2" X 0.162") | 16" OC FACE NAIL |
| 10 | BUILT-UP HEADER (2" TO 2" HEADER WITH 1/2" SPACER) | 16D COMMON (3 1/2" X 0.162") | 16" OC EACH EDGE FACE NAIL |
| | | 16D BOX (3 1/2" X 0.135") | 12" OC EACH EDGE FACE NAIL |
| 11 | CONTINUOUS HEADER TO STUD | 5-8D BOX (2 1/2" X 0.113") or 4-8D COMMON (2 1/2" X 0.131") 4-10D BOX (3" X 0.128") | TOE NAIL |
| 12 | TOP PLATE TO TOP PLATE | 16D COMMON (3 1/2" X 0.162") | 16" OC FACE NAIL |
| | | 10D BOX (3" X 0.128") OR 3" X 0.131" NAILS | 12" OC FACE NAIL |
| 13 | DOUBLE TOP PLATE SPLICE | 8-16D COMMON (3 1/2" X 0.162"); or 12-16D BOX (3 1/2" X 0.135"); or 12-10D BOX (3" X 0.128"); or 12-3" X 0.131" NAILS | FACE NAIL ON EACH SIDE OF END JOINT (MINIMUM 24" LAP SPLICE LENGTH EACH SIDE OF END JOINT) |
| 14 | BOTTOM PLATE TO JOIST, RIM JOIST, BAND JOIST OR BLOCKING (NOT AT BRACED WALL PANELS) | 16D COMMON (3 1/2" X 0.162") | 16" OC FACE NAIL |
| | | 16D BOX (3 1/2" X 0.135"); OR 3" X 0.131" NAILS | 12" OC FACE NAIL |
| 15 | BOTTOM PLATE TO JOIST, RIM JOIST, BAND JOIST OR BLOCKING (NOT AT BRACED WALL PANELS) | 3-16D BOX (3 1/2" X 0.135"); or 2-16D COMMON (3 1/2" X0.162"); or 4-3" X 0.131" NAILS | 3, 2, OR 4 EACH 16" OC FACE NAIL |
| 16 | TOP OR BOTTOM PLATE TO STUD | 4-8D BOX (2 1/2" X 0.113"); or 3-16D BOX (3 1/2" X0.135"); or 4-8D COMMON (2 1/2" X0.131");or 4-10D BOX (3" X0.128"); or 3-3" X 0.131" NAILS | TOE NAIL |
| | | 3-16D BOX (3 1/2" X 0.135"); or 2-16D COMMON (3 1/2" X0.162"); or 3-10D BOX (3" X0.128");or 3-3" X 0.131" NAILS | END NAIL |
| 17 | TOP PLATES, LAPS AT CORNERS AND INTERSECTIONS | 3-10D BOX (3" X 0.128"); or 2-16D COMMON (3 1/2" X0.162"); or 3-3" X 0.131" NAILS | FACE NAIL |
| 18 | 1" BRAVE TO EACH STUD AND PLATE | 3-8D BOX (2 1/2" X 0.113"); or 2-8D COMMON (2 1/2" X0.131") or 2-10D BOX (3" X 0.128"); or 2 STAPLES 1 3/4" | FACE NAIL |
| 19 | 1" X 6" SHEATHING TO EACH BEARING | 3-8D BOX (2 1/2" X 0.113"); or 2-8D COMMON (2 1/2" X0.131") or 2-10D BOX (3" X 0.128"); or 2 STAPLES 1" CROWN, 16GA., 1 3/4" LONG | FACE NAIL |
| 20 | 1" X 8" AND WIDER SHEATHING TO EACH BEARING | 3-8D BOX (2 1/2" X 0.113"); or 3-8D COMMON (2 1/2" X0.131") or 3-10D BOX (3" X 0.128"); or 3 STAPLES, 1" CROWN, 16GA., 1 3/4" LONG | FACE NAIL |
| | | WIDER THAN 1" X 8" 4-8D BOX (2 1/2" X 0.113"); or 3-8D COMMON (2 1/2" X0.131") or 3-10D BOX (3" X 0.128"); or 4 STAPLES, 1" CROWN, 16GA., 1 3/4" LONG | |
| FLOOR | | | |
| 21 | JOIST TO SILL, TOP PLATE OR GIRDER | 4-8D BOX (2 1/2" X 0.113"); or 3-8D COMMON (2 1/2" X0.131") or 3-10D BOX (3" X 0.128"); or 3-3" X 0.131" NAILS | TOE NAIL |
| 22 | RIM JOIST, BAND JOIST OR BLOCKING TO SILL OR TOP PLATE (ROOF APPLICATIONS ALSO) | 8D BOX (2 1/2" X 0.113") | 4" OC TOE NAIL |
| | | 8D COMMON (2 1/2" X 0.131"); or 10D BOX(3" X0.128") or 3-3" X 0.131" NAILS | 6" OC TOE NAIL |
| 23 | 1" X 6" SUBFLOOR OR LESS TO EACH JOIST | 3-8D BOX (2 1/2" X 0.113"); or 2-8D COMMON (2 1/2" X0.131") or 3-10D BOX (3" X 0.128"); or 2 STAPLES, 1" CROWN, 16GA., 1 3/4" LONG | FACE NAIL |
| 24 | 2" SUBFLOOR TO JOIST OR GIRDER | 3-16D BOX (3 1/2" X 0.135"); or 2-16D COMMON (3 1/2" X0.162") | BLIND AND FACE NAIL |
| 25 | 2" PLANKS (PLANK & BEAM-FLOOR AND ROOF) | 3-16D BOX (3 1/2" X 0.135"); or 2-16D COMMON (3 1/2" X0.162") | AT EACH BEARING, FACE NAIL |
| 26 | BAND OR RIM JOIST TO JOIST | 3-16D COMMON (3 1/2" X 0.162"); or 4-10D BOX (3" X0.128") or 4-3" X 0.131" NAILS; or 4-3" X 14GA. STAPLES, 7/16" CROWN | END NAIL |
| 27 | BUILT-UP GIRDERS AND BEAMS, 2-INCH LUMBER LAYERS | 20D COMMON (4" X 0.192"); or | NAIL EACH LAYER AS FOLLOWS: 32" OC AT TIP AND BOTTOM AND STAGGERED 24" OC FACE NAIL AT TOP AND BOTTOM STAGGERED ON OPPOSITE SIDES |
| | | 10D BOX (3" X 0.128"); or 3" X 0.131" NAILS | |
| | | AND: 2-20D COMMON (4" X 0.192"); or 3-10D BOX (3" X 0.128; or 3-3" X 0.131" NAILS | |
| 28 | LEDGER STRIP SUPPORTING JOISTS OR RAFTERS | 4-16D BOX (3 1/2" X 0.135"); or 3-26D COMMON (3 1/2" X 0.162"); or 4-10D BOX (3" X 0.128"); or 4-3" X 0.131" NAILS | AT EACH JOIST OR RAFTER, FACE NAIL |
| 29 | BRIDGING OR BLOCKING TO JOIST | 2-10D BOX (3" X 0.128"); or 2-8D COMMON (2 1/2" X 0.131" or 2-3" X 0.131") NAILS | EACH END, TOE NAIL |

a. ALL NAILS ARE SMOOTH COMMON, BOX OR DEFORMED SHANKS EXCEPT WHERE OTHERWISE STATED. NAILS USED FOR FRAMING AND SHEATHING CONNECTIONS SHALL HAVE MINIMUM AVERAGE BENDING YIELD STRENGTHS AS SHOWN: 80 KSI FOR SHANK DIAMETER OF 0.192 INCH (20D COMMON); NAILS FOR SHANK DIAMETERS LARGER THAN 0.42 INCH BUT NOT LARGER THAN 0.171 INCH; AND 100 KSI FOR SHANK DIAMETER OF 0.142 INCH OR LESS.
b. STAPLES ARE 16 GAGE WIRE AND HAVE A MINIMUM 7/16" INCH ON DIAMETER CROWN WIDTH.
c. NAILS SHALL BE SPACED AT NOT MORE THAN 6 INCHES ON CENTER AT ALL SUPPORTS WHERE SPANS ARE 48 INCHES OR GREATER.
d. FOURFOOT BY 6FOOT OR 4FOOT BY 8FOOT PANELS SHALL BE APPLIED VERTICALLY.
e. SPACING OF FASTENERS NOT INCLUDED IN THIS TABLE SHALL BE BASED ON TABLE R602.3(2).
f. FOR REGIONS HAVING BASIC WIND SPEED OF 110 MPH OR GREATER, 8D DEFORMED (2 1/2" X 0.120) NAILS SHALL BE USED FOR ATTACHING PLYWOOD AND WOOD STRUCTURAL PANEL ROOF SHEATHING TO FRAMING WITHIN MINIMUM 48-INCHES DISTANCE FROM GABLE END WALLS; IF MEAN ROOF HEIGHT IS MORE THAN 25 FEET, UP TO 35 FEET MAXIMUM.
g. FOR REGIONS HAVING BASIC WIND SPEED OF 110 MPH OR LESS, NAILS FOR ATTACHING WOOD STRUCTURAL PANEL ROOF SHEATHING TO GABLE END WALL FRAMING SHALL BE SPACED 6 INCHES ON CENTER, WHEN BASIC WIND SPEED IS GREATER THAN 100 MPH, NAILS FOR ATTACHING PANEL ROOF SHEATHING TO INTERMEDIATE SUPPORTS SHALL BE SPACED 6 INCHES ON CENTER FOR MINIMUM 48-INCH DISTANCE FROM RIDGES, EAVES AND GABLE END WALLS, AND 4 INCHES ON CENTER TO GABLE END WALL FRAMING.
h. GYPSUM SHEATHING SHALL CONFORM TO ASTM C 1396 AND SHALL BE INSTALLED IN ACCORDANCE WITH GA 283 FIBERBOARD SHEATHING SHALL CONFORM TO ASTM C 208.
i. SPACING OF FASTENERS ON FLOOR SHEATHING PANEL EDGES SUPPORTED BY FRAMING MEMBERS AND REQUIRE BLOCKING AND AT ALL FLOOR PERIMETERS ONLY; SPACING OF FASTENERS ON ROOF SHEATHING PANEL EDGES APPLIES TO PANEL EDGES SUPPORTED BY FRAMING MEMBERS AND REQUIRE BLOCKING, BLOCKING OF ROOF OR FLOOR SHEATHING PANEL EDGES PERPENDICULAR TO THE FRAMING MEMBERS NEED NOT BE PROVIDED EXCEPT AS REQUIRED BY OTHER PROVISIONS OF THIS CODE. FLOOR PERIMETER SHALL BE SUPPORTED BY FRAMING MEMBERS OR SOLID BLOCKING.
j. WHERE A RAFTER IS FASTENED TO AN ADJACENT PARALLEL CEILING JOIST IN ACCORDANCE WITH THIS SCHEDULE, PROVIDE TWO TOE NAILS ON ONE SIDE OF THE RAFTER AND TWO NAILS FROM CEILING JOIST TO TOP PLATE IN ACCORDANCE WITH THIS SCHEDULE. THE TOE NAIL ON THE OPPOSITE SIDE OF THE RAFTER SHALL NOT BE REQUIRED.

CONTINUED TABLE R602.3(1)
FASTENER SCHEDULE FOR STRUCTURAL MEMBERS

| ITEM | DESCRIPTION OF BUILDING ELEMENTS | NUMBER AND TYPE OF FASTENER | SPACING OF FASTENERS | |
|--|--|--|-----------------------------|---|
| | | | EDGES (INCHES) _b | INTERMEDIATE _c SUPPORTS (INCHES) |
| WOOD STRUCTURAL PANELS, SUBFLOOR, ROOF AND INTERIOR WALL SHEATHING TO FRAMING AND PARTICLEBOARD WALL SHEATHING TO FRAMING [SEE TABLE R602.3(3) FOR WOOD STRUCTURAL PANEL EXTERIOR WALL SHEATHING TO WALL FRAMING] | | | | |
| 30 | 3/8" - 1/2" | 6D COMMON (2"x 0.113" NAIL (SUBFLOOR, WALL) ; 8D COMMON (2 1/2" X 0.131 NAIL (ROOF); or RRSR-01 (2 3/8" X 0.113" NAIL (ROOF)) | 6 | 12 " |
| 31 | 19/32" - 1" | 8D COMMON NAIL (2 1/2" X 0.131; or RRSR-01; 2 3/8" X 0.113) NAIL ROOF) | 6 | 12 " |
| 32 | 1 1/8" - 1 1/4" | 10D COMMON NAIL (3" X 0.148) NAIL; or 8D (2 1/2" X 0.131") DEFORMED NAIL | 6 | 12 |
| OTHER WALL SHEATHING ^g | | | | |
| 33 | 1/2" STRUCTURAL CELLULOSE FIBERBOARD SHEATHING | 1 1/2" GALVANIZED ROOF NAIL, 7/16" HEAD DIAMETER, OR 1 1/4" LONG 16GA. STAPLE WITH 7/16" OR 1" CROWN | 3 | 6 |
| 34 | 25/32" STRUCTURAL CELLULOSE FIBERBOARD SHEATHING | 1 3/4" GALVANIZED ROOF NAIL, 7/16" HEAD DIAMETER, OR 1 1/2" LONG 16GA. STAPLE WITH 7/16" OR 1" CROWN | 3 | 6 |
| 35 | 1/2" GYPSUM SHEATHING ^d | 1 1/2" GALVANIZED ROOF NAIL, STAPLE GALVANIZED, 11/2" LONG; 1 1/4" SCREWS, TYPE W or S | 7 | 7 |
| 36 | 5/8" GYPSUM SHEATHING ^d | 1 3/4" GALVANIZED ROOF NAIL; STAPLE GALVANIZED, 1 5/8" LONG; 1 5/8" SCREWS, TYPE W or S | 7 | 7 |
| WOOD STRUCTURAL PANELS, COMBINATION SUBFLOOR UNDERLAYMENT TO FRAMING | | | | |
| 37 | 3/4" AND LESS | 6D DEFORMED (2" X 0.120") NAIL OR 8D COMMON (2 1/2" X 0.131") NAIL | 6 | 12 |
| 38 | 7/8" - 1" | 8D COMMON (2 1/2" X 0.131") NAIL OR 8D DEFORMED (2 1/2" X 0.120") NAIL | 6 | 12 |
| 39 | 1 1/8" - 1 1/4" | 10D COMMON (3" X 0.148") NAIL OR 8D DEFORMED (2 1/2" X 0.120") NAIL | 6 | 12 |

For St: 1 inch = 25.4mm, 1 foot = 304.8 mm, 1 mile per hour = 0.447 m/s; 1 ksi = 6.895 MPa.

TABLE R 602.3(5) SIZE, HEIGHT, AND SPACING OF WOOD STUDS

| STUD SIZE (IN) | BEARING WALLS | | | | | NON-BEARING WALLS | |
|------------------|---|---|---|--|---|---|--|
| | LATERALLY UNSUPPORTED STUD HEIGHT ^a (feet) | MAXIMUM SPACING WHERE SUPPORTING A ROOF-CEILING ASSEMBLY OR A HABITABLE ATTIC ASSEMBLY, ONLY (inches) | MAXIMUM SPACING WHERE SUPPORTING ONE FLOOR, PLUS A ROOF-CEILING ASSEMBLY OR A HABITABLE ATTIC ASSEMBLY (inches) | MAXIMUM SPACING WHERE SUPPORTING TWO FLOORS, PLUS A ROOF-CEILING ASSEMBLY OR A HABITABLE ATTIC ASSEMBLY (inches) | MAXIMUM SPACING WHERE SUPPORTING ONE FLOOR HEIGHT ^a (inches) | LATERALLY UNSUPPORTED STUD HEIGHT ^a (feet) | LATERALLY UNSUPPORTED STUD HEIGHT (feet) |
| | | | | | | | |
| 2x3 ^b | --- | --- | --- | --- | --- | 10 | 16 |
| 2x4 | 10 | 24 _c | 16 _c | --- | 24 | 14 | 24 |
| 3x4 | 10 | 24 | 24 | 16 | 24 | 14 | 24 |
| 2x5 | 10 | 24 | 24 | --- | 24 | 16 | 24 |
| 2x6 | 10 | 24 | 24 | 16 | 24 | 20 | 24 |

FOR ST: 1 INCH = 25.4mm, 1 FOOT = 304.8mm
a. LISTED HEIGHTS ARE DISTANCES BETWEEN POINTS OF LATERAL SUPPORT PLACED PERPENDICULAR TO THE PLANE OF THE WALL. BEARING WALL SHALL BE SHEATHED ON NOT LESS THAN ONE SIDE OR BRIDGING SHALL BE INSTALLED NOT GREATER THAN 4 FEET APART MEASURED VERTICALLY FROM EITHER END OF THE STUD. INCREASES IN UNSUPPORTED HEIGHT ARE PERMITTED WHERE IN COMPLIANCE WITH EXCEPTION 2 OF SECTION R602.3.1 OR DESIGNED IN ACCORDANCE WITH ACCEPTED ENGINEERING PRACTICES.
b. SHALL NOT BE USED IN EXTERIOR WALLS.
c. A HABITABLE ATTIC ASSEMBLY SUPPORTED BY 2X4 STUDS IS LIMITED TO A ROOF SPAN OF 32 FEET. WHERE THE ROOF SPAN EXCEEDS 32 FEET, THE WALL STUDS SHALL BE INCREASED TO 2X6 OR THE STUDS SHALL BE DESIGNED IN ACCORDANCE WITH ACCEPTED ENGINEERING PRACTICE.

MINIMUM MECHANICAL EQUIPMENT EFFICIENCY
VALUES BY COMPONENT, PER IRC2018 N1103.6.1

| FAN LOCATION | AIR FLOW RATE MINIMUM (CFM) | MINIMUM EFFICACY CFM/WATT | AIR FLOW RATE MAXIMUM (CFM) |
|----------------------|-----------------------------|---------------------------|-----------------------------|
| HRV OR ERV | ANY | 1.2 CFM/WATT | ANY |
| RANGE HOOD | ANY | 2.8 CFM/WATT | ANY |
| IN-LINE FAN | ANY | 2.8 CFM/WATT | ANY |
| BATHROOM UTILITY FAN | 10 | 1.4 CFM/WATT | <90 |
| BATHROOM UTILITY FAN | 90 | 2.8 CFM/WATT | ANY |

MINIMUM INSULATION & FENSTRATION VALUES BY COMPONENT, PER IRC2018 N1102.1.2

VALUES BELOW ARE PER 2018 IECC. ACTUAL VALUES MAY VARY BASED ON ALTERNATE ENERGY COMPLIANCE PATH CHOSEN (IN JURISDICTIONS WHERE ALTERNATIVE PATHS ARE AVAILABLE).

| CLIMATE ZONE | FENSTRATION U-FACTOR | SKYLIGHT U-FACTOR | GLAZED SHGC FENSTRATION | INSULATED METAL DOOR U-VALUE | INSULATED WOOD DOOR U-VALUE | CEILING R-VALUE | WOOD FRAMED WALL R-VALUE | FLOOR R-VALUE | BASEMENT WALL R-VALUE | SLAB R-VALUE & DEPTH | CRAWL SPACE WALL R-VALUE | DUCTWORK OVER OUTSIDE R-VALUE | DUCTWORK (ALL OTHER) R-VALUE |
|-----------------|----------------------|-------------------|-------------------------|------------------------------|-----------------------------|-----------------|--------------------------|---------------|----------------------------|----------------------|----------------------------|-------------------------------|------------------------------|
| 4 EXCEPT MARINE | 0.32 | 0.55 | 0.40 | 0.60 | 0.50 | 49 | 20 OR 13 CAV. +5 | 19 | 10 CONTINUOUS OR 13 CAVITY | R-10, 2 FT. | 10 CONTINUOUS OR 13 CAVITY | 8 | 6 |

NOTES: 1) BUILDING THERMAL ENVELOPE IS REQUIRED TO BE SEALED WITH AN AIR BARRIER AS PER N1102.4.1 OF THE 2018 IRC
2) RECESSED LIGHTING SHALL BE SEALED TO PREVENT LEAKAGE BETWEEN THE CONDITIONED SPACE AND UNCONDITIONED SPACE
3) ALL DUCTS, AIR HANDLERS, FILTER BOXES, AND BUILDING CAVITIES USED AS DUCTS SHALL BE SEALED AS PER N1103.2 OF THE 2018 IRC

BUILDER'S PLANS: THE TERM "BUILDER'S PLANS" REFERS TO A CERTAIN LEVEL OF DEVELOPMENT OF THE DRAWINGS. AS THE NAME IMPLIES, THESE PLANS REQUIRE THAT THE CONTRACTOR POSSESSES COMPETENCE IN RESIDENTIAL CONSTRUCTION AND A THOROUGH UNDERSTANDING OF THE INTERNATIONAL RESIDENTIAL CODE (IRC). THE CONTRACTOR WARRANTS TO HD ENGINEERING & DESIGN THAT HE POSSESSES THE PARTICULAR COMPETENCE AND SKILL IN CONSTRUCTION NECESSARY TO BUILD THIS PROJECT WITHOUT FULL ENGINEERING AND DESIGN SERVICES, AND FOR THAT REASON THE CONTRACTOR OR HOME OWNER HAS RESTRICTED THE SCOPE OF PROFESSIONAL SERVICES. THE CONSTRUCTION DOCUMENTS PROVIDED BY THE LIMITED SERVICES SHALL BE TERMED "BUILDER'S PLANS" IN RECOGNITION OF THE CONTRACTOR'S SOPHISTICATION. ALTHOUGH HD ENGINEERING & DESIGN HAVE PERFORMED THEIR SERVICES WITH DUE CARE AND DILIGENCE, WE CANNOT GUARANTEE PERFECTION. ANY AMBIGUITY OR DISCREPANCY DISCOVERED BY THE USE OF THESE PLANS SHALL BE REPORTED IMMEDIATELY TO HD ENGINEERING. CONSTRUCTION MAY REQUIRE THAT THE CONTRACTOR ADAPT THE "BUILDER'S PLANS" TO THE FIELD CONDITIONS ENCOUNTERED AND MAKE LOGICAL ADJUSTMENTS IN FIT, FORM, DIMENSION AND QUANTITY. CHANGES MADE FROM THE PLANS WITHOUT THE CONSENT OF HD ENGINEERING & DESIGN ARE UNAUTHORIZED. IT IS ALSO UNDERSTOOD THAT THE CONTRACTOR WILL BE RESPONSIBLE FOR MEETING ALL APPLICABLE BUILDING CODES INCLUDING BUT NOT LIMITED TO MECHANICAL, ELECTRICAL, AND PLUMBING CODE REQUIREMENTS (WHICH IS EXCLUDED FROM THESE PLANS). IN THE EVENT ADDITIONAL DETAIL OR GUIDANCE IS NEEDED BY THE CONTRACTOR OR HOMEOWNER FOR CONSTRUCTION OF ANY ASPECT OF THE PROJECT, HD ENGINEERING & DESIGN OR A QUALIFIED ENGINEER SHALL IMMEDIATELY BE RETAINED. FAILURE TO NOTIFY US OF THESE NEEDS OR OF CHANGES TO THE PLANS SHALL RELIEVE HD ENGINEERING & DESIGN OF ALL RESPONSIBILITIES OF THE CONSEQUENCES.

DESIGN LOADS (PSF)

THE DWELLING SHALL COMPLY WITH THE FOLLOWING LOAD CONDITIONS

| AREA | MIN DEAD LOAD | MIN LIVE LOAD |
|--|----------------|---------------|
| EXTERIOR BALCONIES | 10 | 60 |
| DECKS, STAIRS | 10 | 40 |
| CEILING JOISTS / ATTICS NO STORAGE - SCUTTLE ACCESS ONLY ROOF SLOPE 3:12 OR LESS | 10 | 10 |
| CEILING JOISTS / ATTICS NO STORAGE - SCUTTLE ACCESS ONLY ROOF SLOPE OVER 3:12 | 10 | 10 |
| CEILING JOISTS / ATTICS WITH STORAGE - DOOR PULL DOWN LADDER ACCESS | 10 | 20 |
| ROOMS: NON-SLEEPING | 10 | 40 |
| ROOMS: SLEEPING | 10 | 30 |
| ROOF: LIGHT ROOF COVERING | 10 | 20 |
| ROOF: HEAVY ROOF COVERING / CONCRETE / TILE / SLATE | 20 | 20 |
| GUARDRAILS, HANDRAILS | 200# LL NORMAL | |

HEAVY ROOF COVERING MATERIAL (TILE, CONCRETE, SLATE, ETC.) SHALL NOT BE USED UNLESS 20 PSF DEAD LOAD AND HEAVY ROOF IS NOTED ON THE ROOF PLAN. IF HEAVY ROOFING IS TO BE USED AND NOT NOTED ON THE ROOF PLAN NOTIFY ENGINEER PRIOR TO ANY CONSTRUCTION, INCLUDING FOUNDATION AND SITE WORK. IF THE PLAN HAS BEEN DESIGNED FOR HEAVY ROOF LOADS IT WILL BE NOTED IN THE ROOF NOTES ON THE ROOF PLAN.

COLUMN SCHEDULE

BASED ON FOOTING SIZE (ASSUME 1500 PSF SOIL)

| PAD SIZE | REINFORCEMENT | COL. MIN. | COL. TYPE | MAX. LOAD |
|----------|------------------|-----------|-----------|-----------|
| 24x24x12 | (4) #4 BARS E/W | 3" | SCH40 | 6K |
| 30x30x12 | (5) #4 BARS E/W | 3" | SCH40 | 9.4K |
| 36x36x12 | (6) #4 BARS E/W | 3" | SCH40 | 13.5K |
| 42x42x14 | (7) #4 BARS E/W | 3 1/2" | SCH40 | 18.4K |
| 48x48x16 | (8) #4 BARS E/W | 3 1/2" | SCH40 | 24.0K |
| 54x54x16 | (9) #4 BARS E/W | 3 1/2" | SCH40 | 30.4K |
| 60x60x18 | (10) #4 BARS E/W | 3 1/2" | SCH40 | 37.5K |

COLUMN CONNECTION TO STEEL BEAMS SHALL BE WITH A CLIP POST CAP WITH ALL FOUR TAB EARS BENT AROUND THE BOTTOM FLANGE OF THE BEAM. FOR A BEARING PLATE, FOUR HOLES SHALL BE DRILLED IN THE BOTTOM FLANGE OF THE STEEL BEAM TO MATCH THE HOLE PATTERN OF THE PLATE. 1/2" X 2" BOLTS SHOULD THEN BE INSTALLED WITH A FLAT WASHER, LOCK WASHER, AND A NUT IN EACH OF THE HOLES. THE POST CAP MAY BE WELDED TO THE STEEL BEAM IN ACCORDANCE WITH AWS D1.1-92 AS AN ALTERNATIVE, AND WOULD NEED TO BE INSPECTED BY AN AWS-CERTIFIED INSPECTOR.

ENGINEERED LUMBER

MIN. DESIGN REQUIREMENTS

| | F _b (psi) | E (psi) | F _v (psi) |
|---------|----------------------|---------|----------------------|
| LVL | 2600 | 1.8x10 | 285 |
| GLULAM | 2400 | 1.8x10 | 190 |
| PARALAM | 2600 | 2.0x10 | 290 |

CATHEDRAL / VAULTED CEILING
FRAMING AND INSULATION

MINIMUM R-38 INSULATION REQUIRED, SEE DETAIL 14/S-1.2

WHERE THE CEILING IS APPLIED DIRECTLY TO THE BOTTOM OF THE RAFTERS, A MINIMUM 1" AIR SPACE SHALL BE PROVIDED BETWEEN THE TOP OF THE INSULATION AND THE SHEATHING FOR VENTILATION (R806.3)
NOTE: RAFTER SIZES SPECIFIED ON PLANS ARE THE MINIMUM REQUIRED FOR STRUCTURAL PURPOSES ONLY.
BUILDER TO VERIFY:
IF FULL RAFTER DEPTH IS NOT ADEQUATE FOR MINIMUM INSULATION VALUE, RAFTER SIZES WILL NEED TO BE INCREASED, OR ADEQUATE FURRING SHALL BE USED TO OBTAIN THE MINIMUM JOIST DEPTH FOR THE REQUIRED INSULATION. IN ADDITION, IF THE RAFTER SIZE IS INCREASED IT SHALL BE VERIFIED THAT THE RIDGE BE A MINIMUM OF ONE NOMINAL SIZE LARGER THAN THE RAFTERS BEING RECEIVED. (SEE CHART BELOW)

| MAXIMUM INSULATION VALUE 1" AIR SPACE (FIBERGLASS) | 2x6 | 2x8 | 2x12 |
|---|------------------------|--------------|------------------------|
| | R-13, 3 1/2" | R-19, 6 1/4" | CONDENSED R-38, 8 1/4" |
| | CONDENSED R-38, 8 1/4" | | |
| | R-38, 10 1/4" | | |

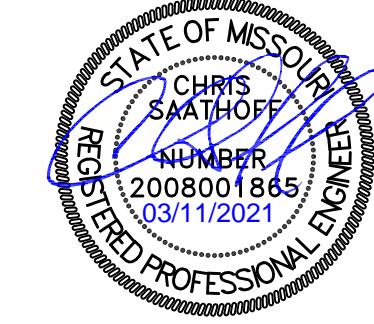
HD ENGINEERING & DESIGN, INC

11655 W. 75TH STREET
SHAWNEE, KS 66214

WWW.HDENGINEERS.COM

913.631.2222

SERVICE@HDENGINEERS.COM



SAB HOMES, INC.

REDBUD E718

2366 SW. OLD PORT RD. LEE'S SUMMIT, MO

STRUCTURAL DETAILS & NOTES

HD#: 41286

DATE: 03/11/2021
CHECKED BY: CLS

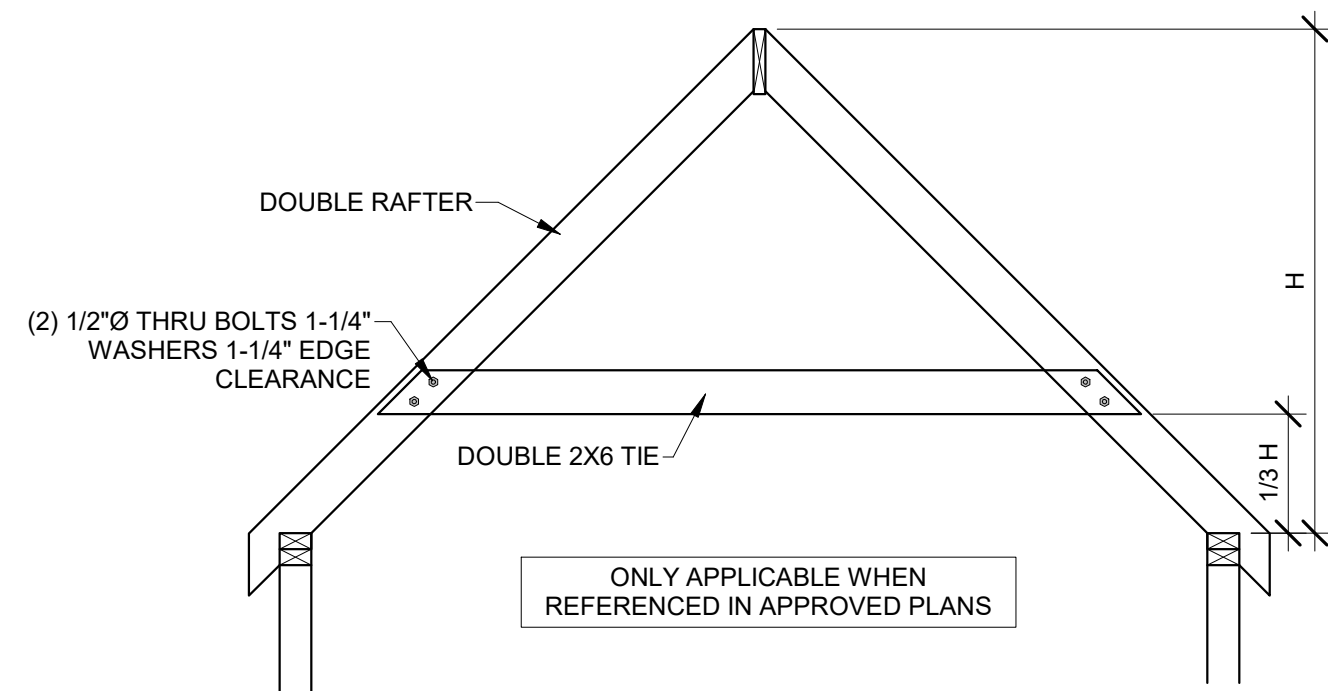
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GENERAL NOTES

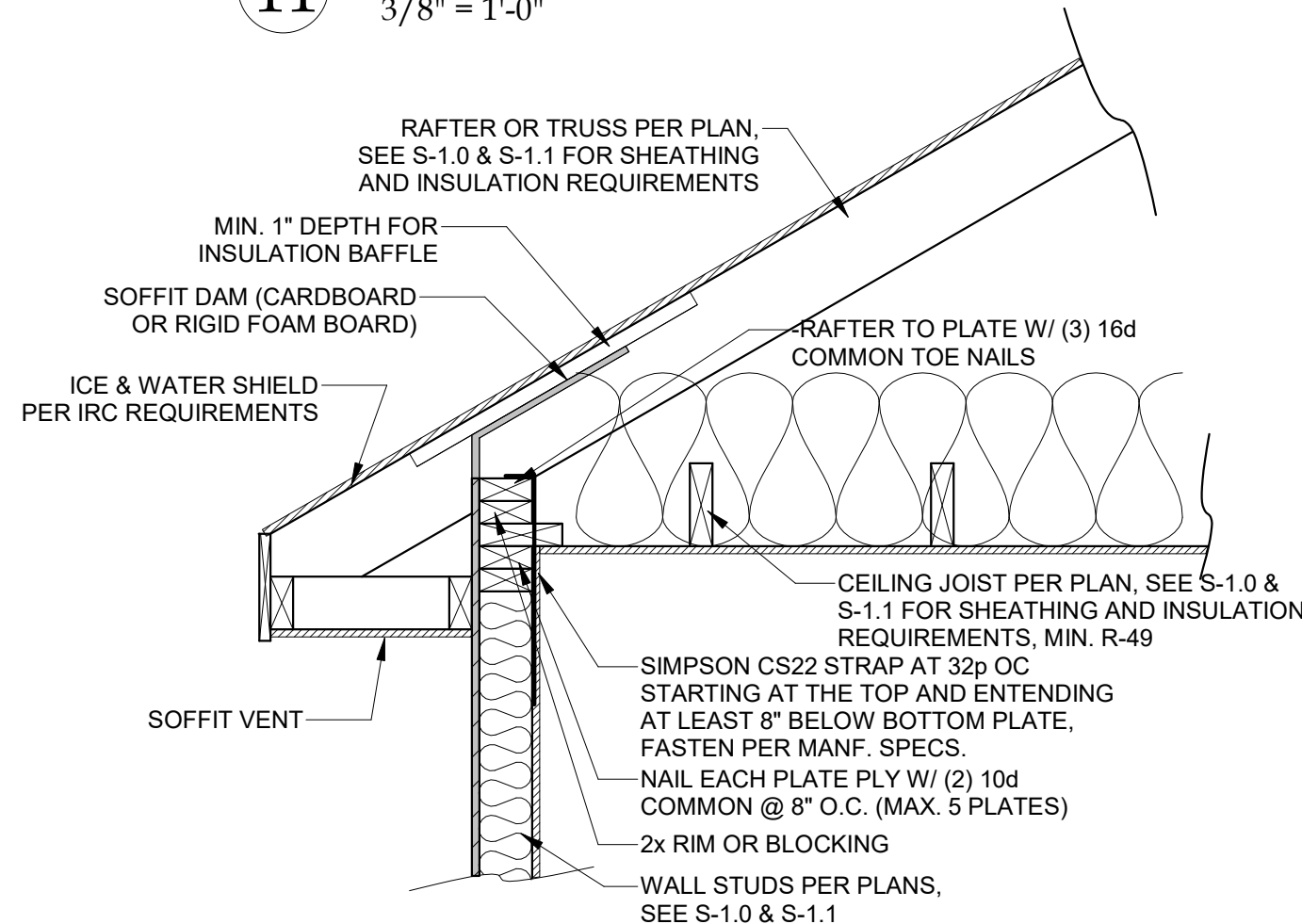
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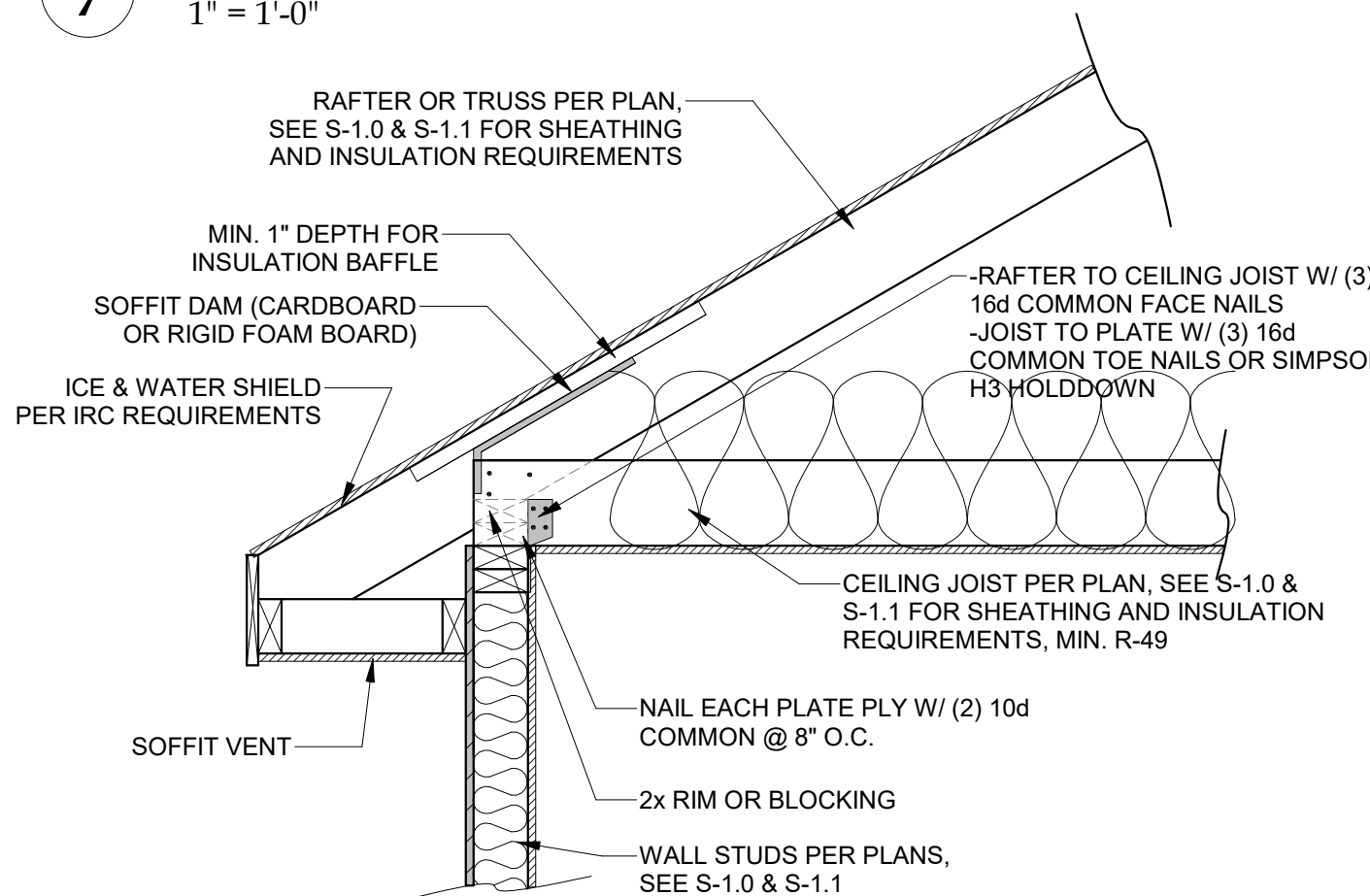
RELEASE FOR CONSTRUCTION
AS NOTED FOR PLAN REVIEW
DEVELOPMENT SERVICES
LEE'S SUMMIT, MISSOURI
09/28/2021



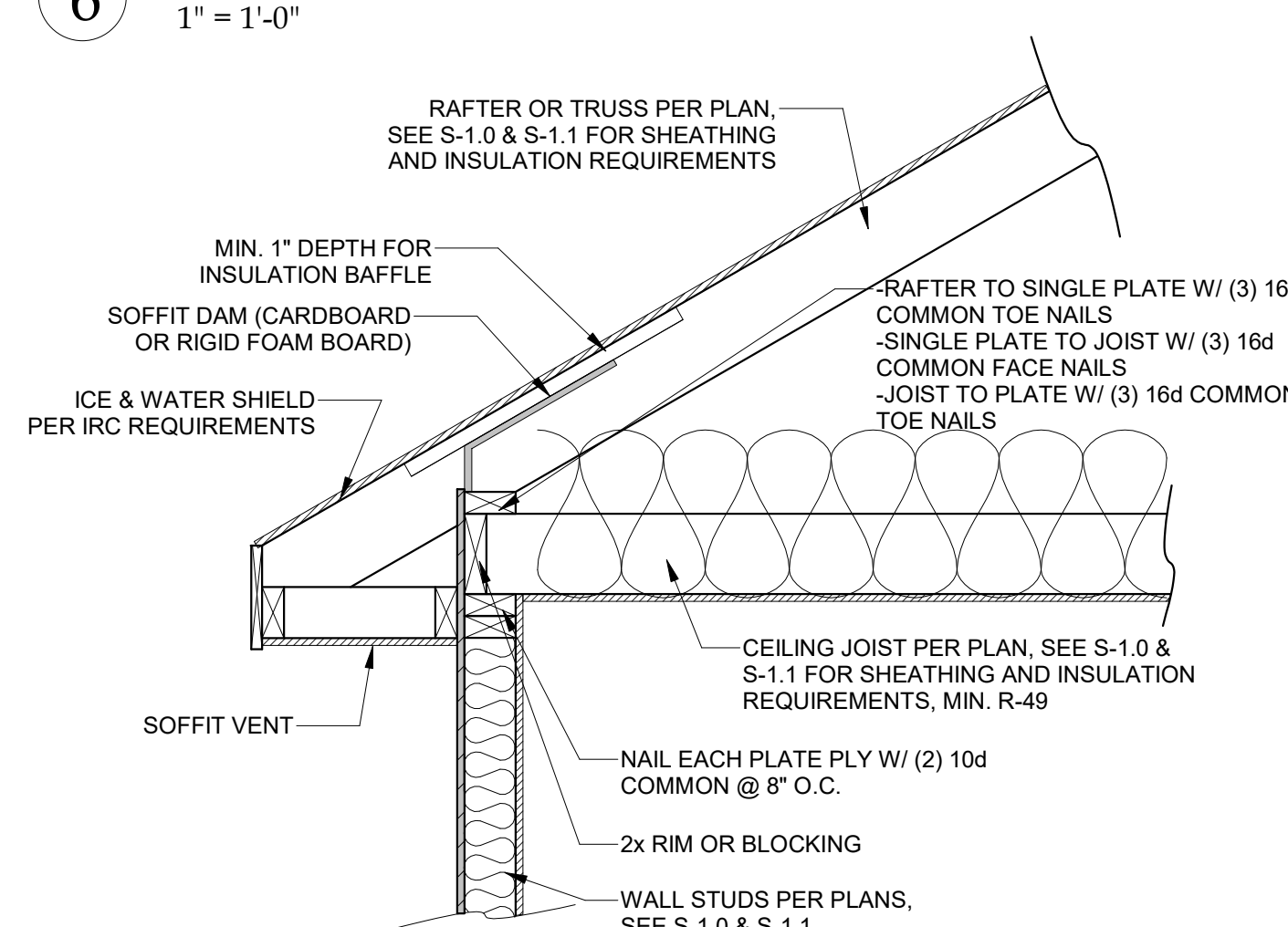
11 HIP SUPPORT FRAME
3/8" = 1'-0"



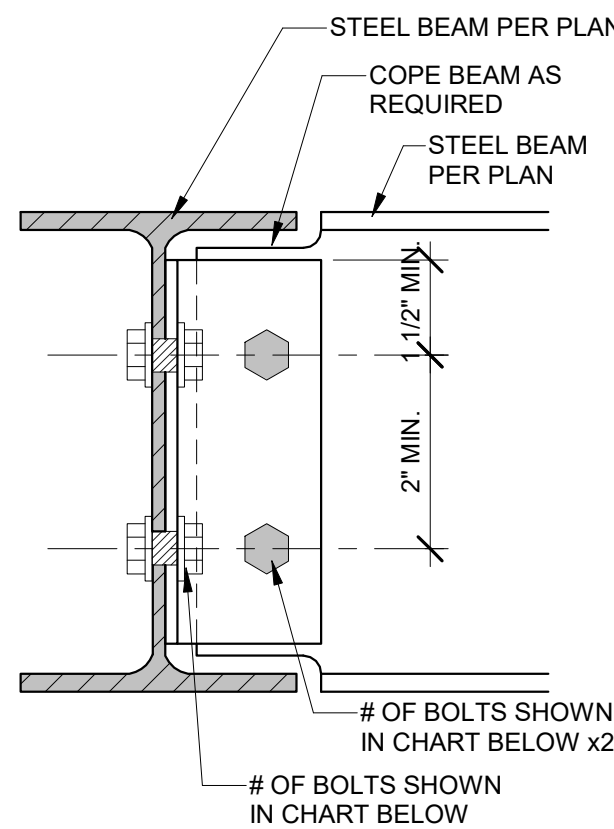
7 OPTION 4 RAFTER BEARING
1" = 1'-0"



6 OPTION 3 RAFTER BEARING
1" = 1'-0"



5 OPTION 2 RAFTER BEARING
1" = 1'-0"
THIS OPTION NOT AVAILABLE IN KC, MO

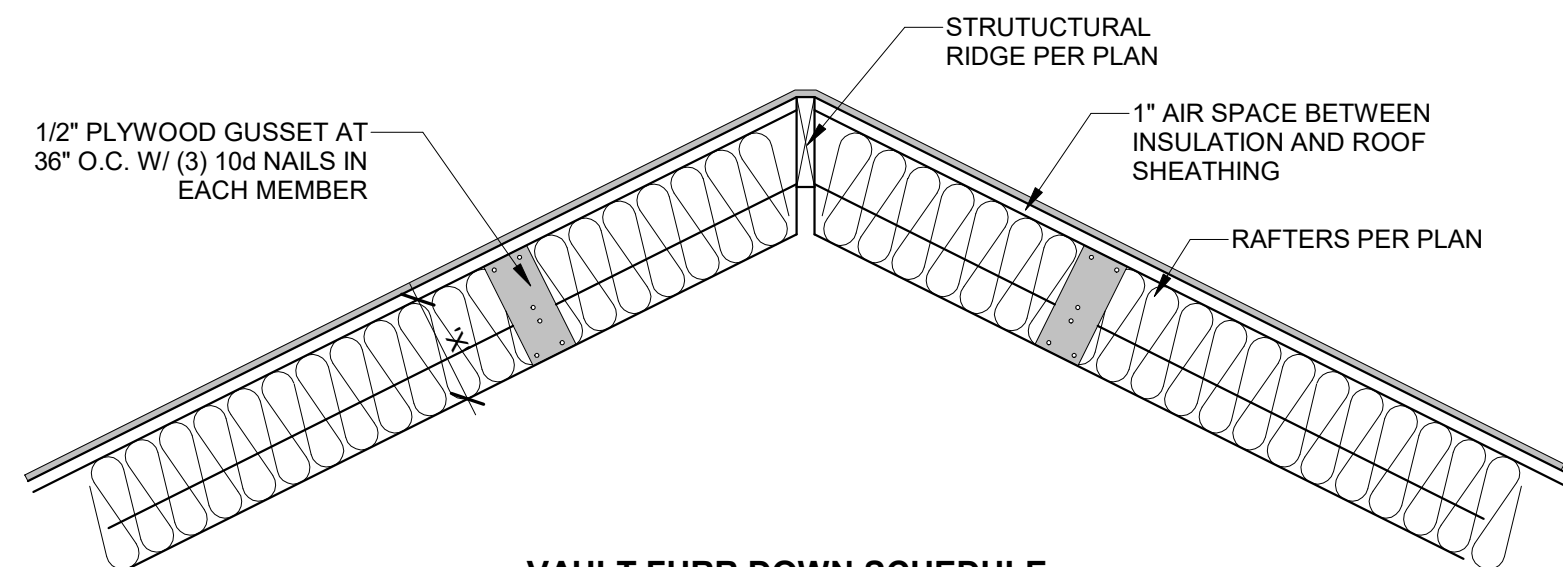


10 BEAM TO GIRDER CONNECTION
3" = 1'-0"

| BEAM CONNECTION SCHEDULE | |
|--------------------------|-------------------------|
| BEAM SIZE | # OF BOLT IN CONNECTION |
| W8, W10 | 2 |
| W12, W14 | 3 |
| W16, W18 | 4 |

NOTES:
1. NUMBER OF BOLTS DETERMINED BY SMALLER OF TWO BEAMS BEING CONNECTED
2. ALL BOLTS, 3/4" DIAMETER A325-N, UNO
3. FULL PERIMETER 1/4" FILLET WELD MAY BE SUBSTITUTED FOR EITHER OR BOTH BOLTED CONNECTIONS

| TYPE | MAX. UNSUPPORTED SPAN | | | | |
|---------------|-----------------------|--------|--------|-------------------|--------------------|
| | 2x8 | 2x10 | 2x12 | 1 3/4"x9 1/2" LVL | 1 3/4"x11 7/8" LVL |
| HIP RAFTER | 11'-3" | 13'-3" | 15'-2" | 15'-8" | 18'-2" |
| VALLEY RAFTER | 8'-11" | 10'-6" | 12'-0" | 13'-2" | 15'-3" |



| <u>VAULT FURR DOWN SCHEDULE</u> | | |
|---------------------------------|--------------------------------------|---------------------------------------|
| <u>RAFTER SIZE</u> | <u>R-30C INSULATION (X = 9 1/4")</u> | <u>R-38C INSULATION (X = 11 1/4")</u> |
| 2x6 | 2x6 | 2x8 |
| 2x8 | 2x4 | 2x6 |
| 2x10 | NOT REQUIRED | 2x4 |
| 2x12 | NOT REQUIRED | 2x2 |

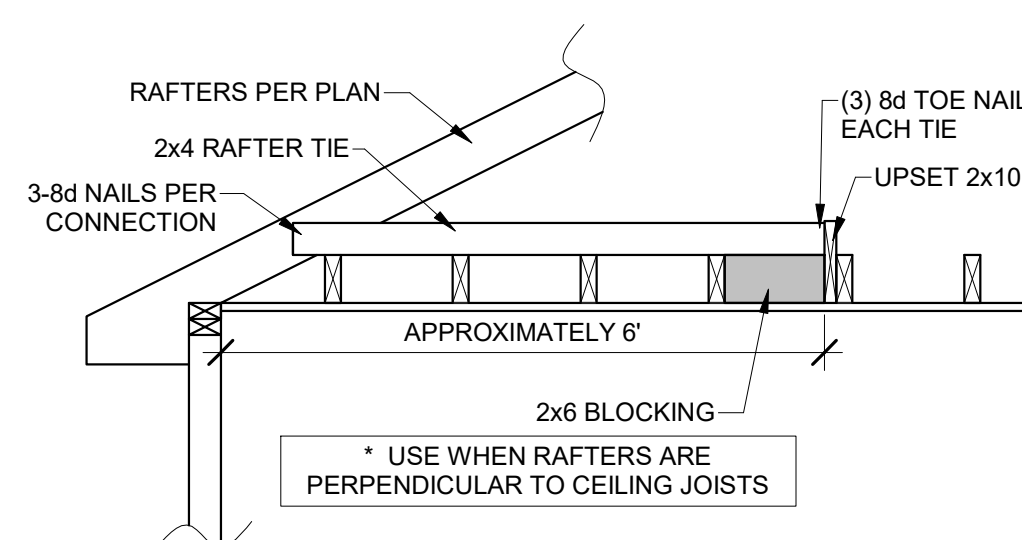
NOTES:

1. ALL VAULTS SHALL BE FURRED DOWN WITH 2x FRAMING TO TEH
REQUIRED DEPTH OF INSULATION, PLUS 1" AIR SPACE.

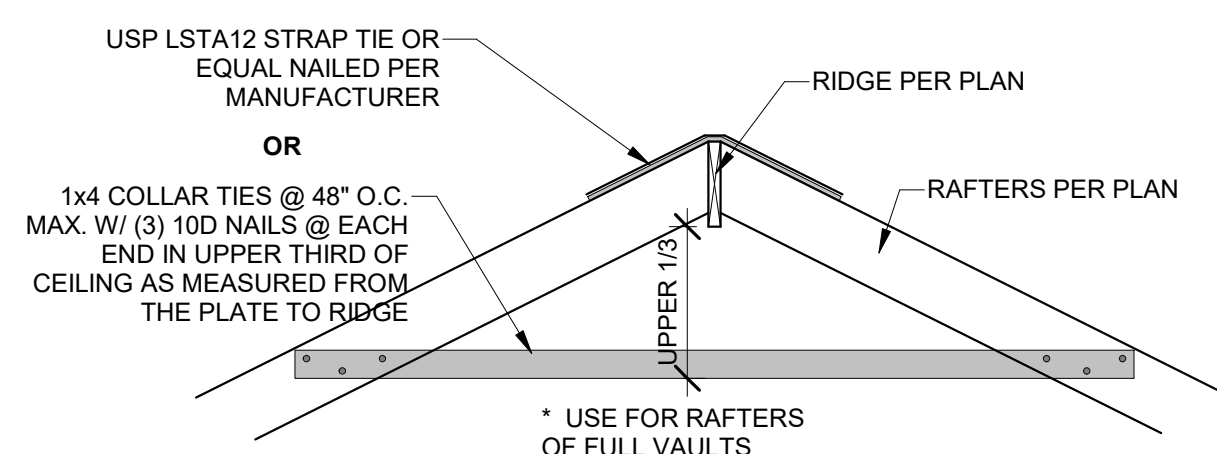
2. R-38C REQUIRED = 11" WITH AIR SPACE.

3. ALL VAULTED RAFTERS SHALL BE MIN. #2 2x6 DFL @ 16" O.C. OR PER
ROOF PLAN.

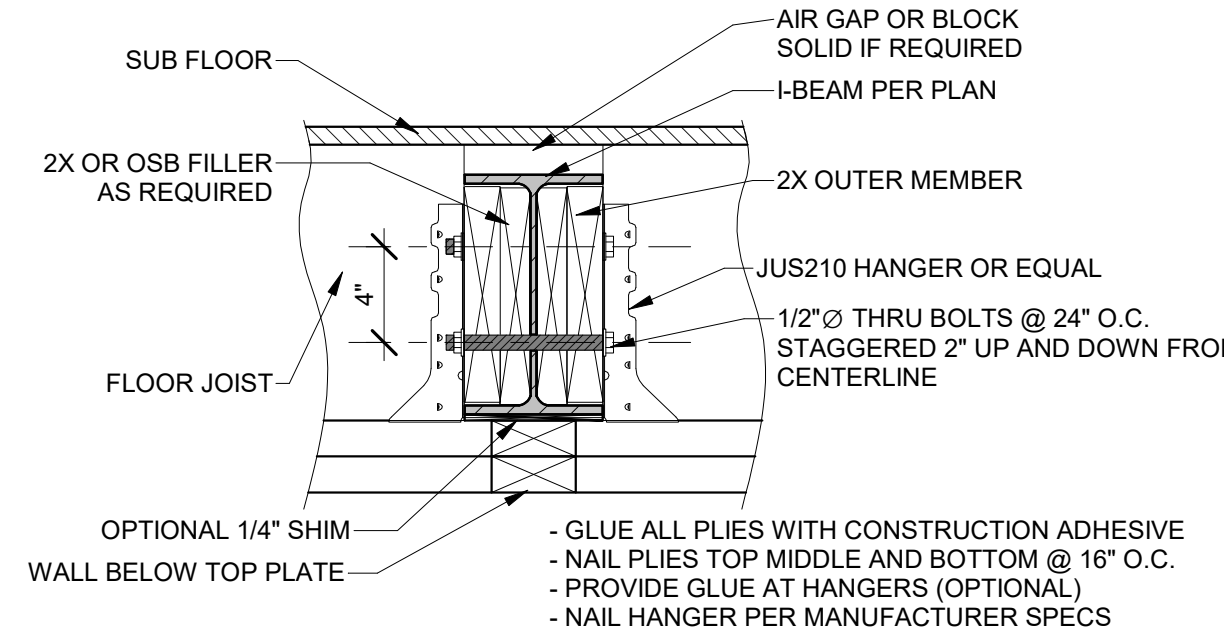
14 VAULTED RAFTER INSULATION
3/4" = 1'-0"



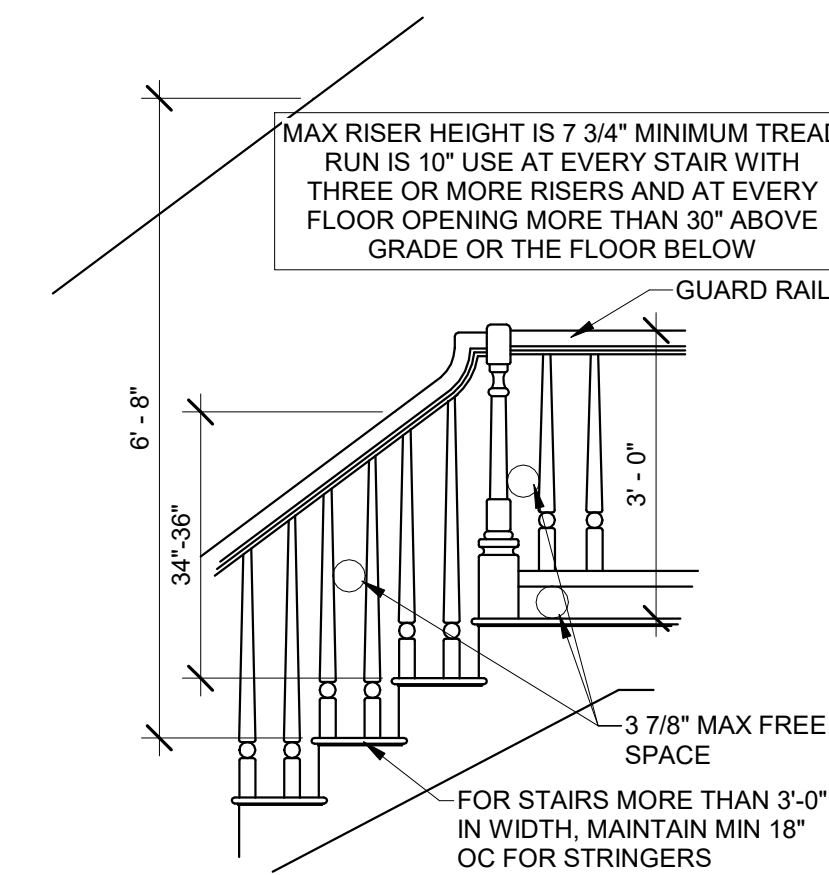
12 RAFTER TIE CONNECTION
1/2" = 1'-0"



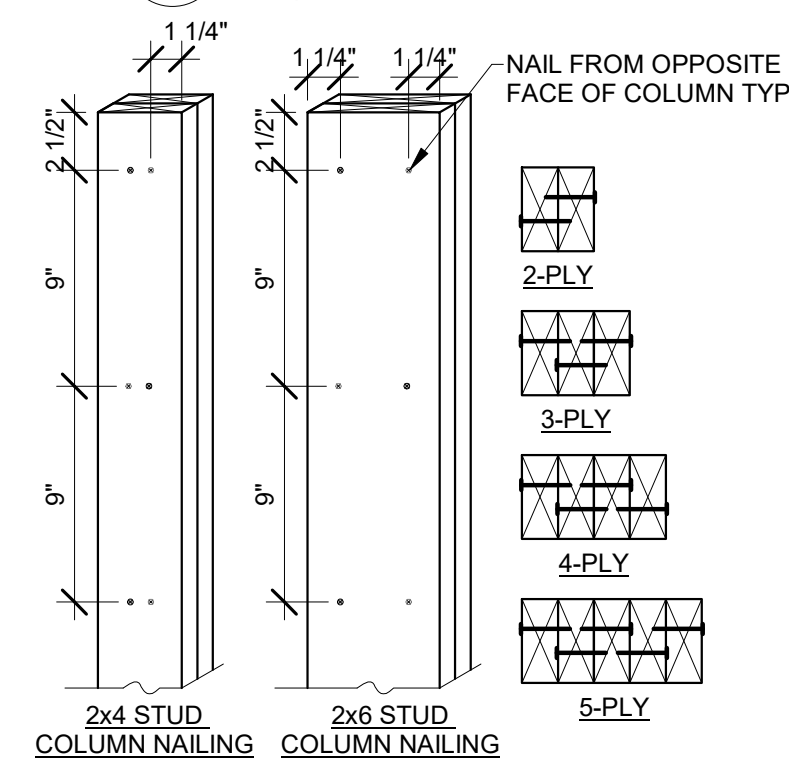
13 RIDGE SUPPORT
1/2" = 1'-0"



8 UPSET STEEL BEAM DETAIL
1 1/2" = 1'-0"

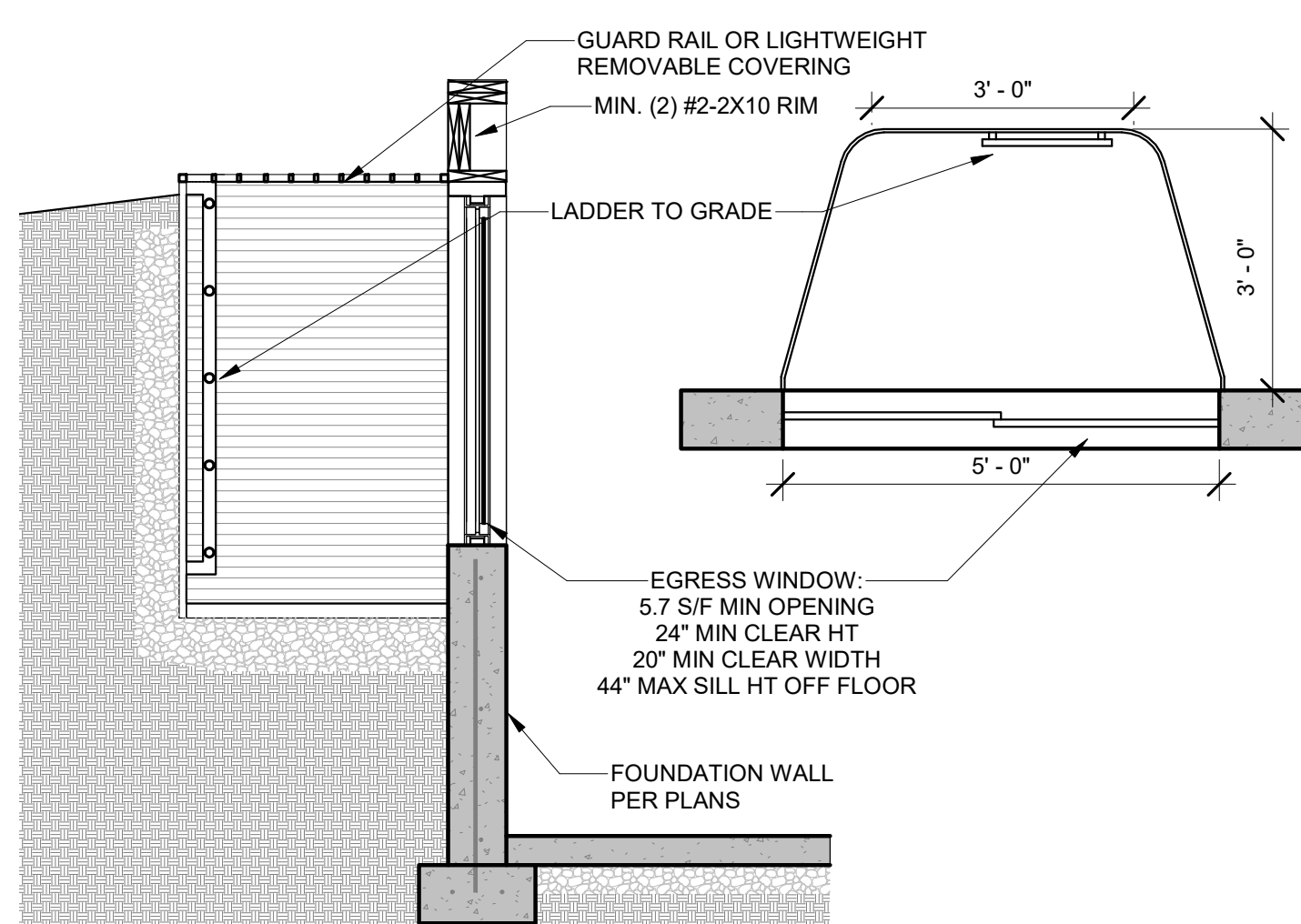


4 STAIR/ RAIL DETAIL
1/2" = 1'-0"

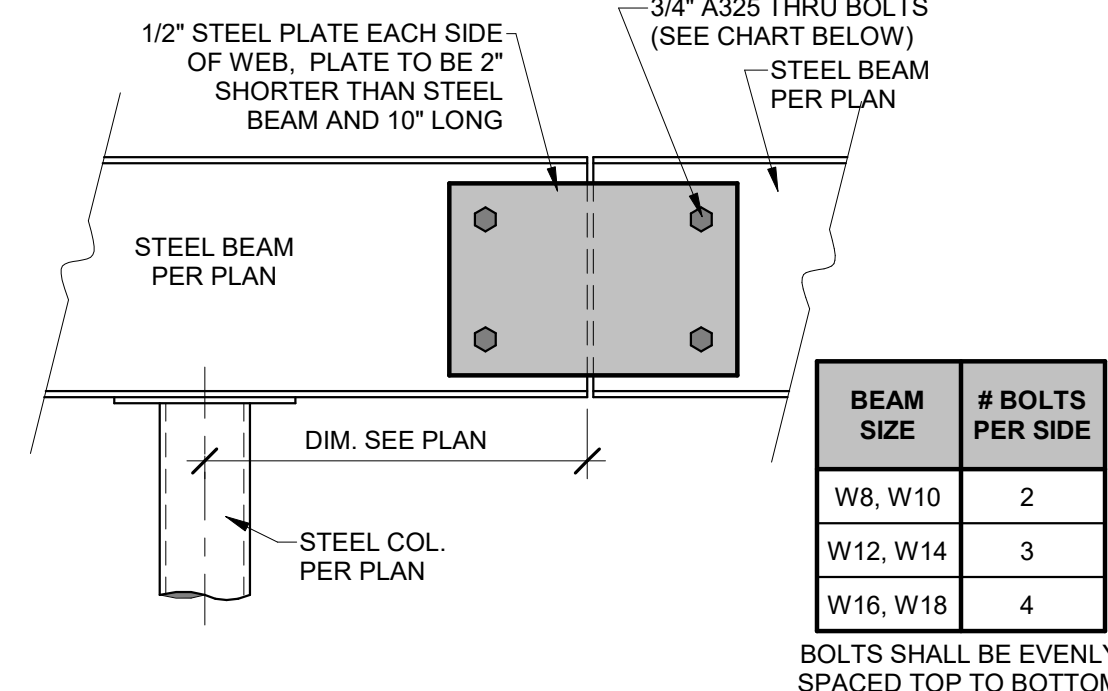


NOTES:
1. EACH 2x PLY SHALL BE FASTENED WITH (1) ROW OF 10d NAILS AT 9" O.C. ALTERNATING SIDE TO SIDE
2. 1 1/4" MIN. EDGE DISTANCE, AND STARTING 2 1/2" FROM EACH END.
3. EXTEND FULL AREA OF COLUMN AS SOLID BLOCKING THROUGH JOIST BAYS AND WALLS TO LOAD-BEARING BEAM/WALL BELOW.

3 BUILT-UP STUD COLUMN
1 1/2" = 1'-0"



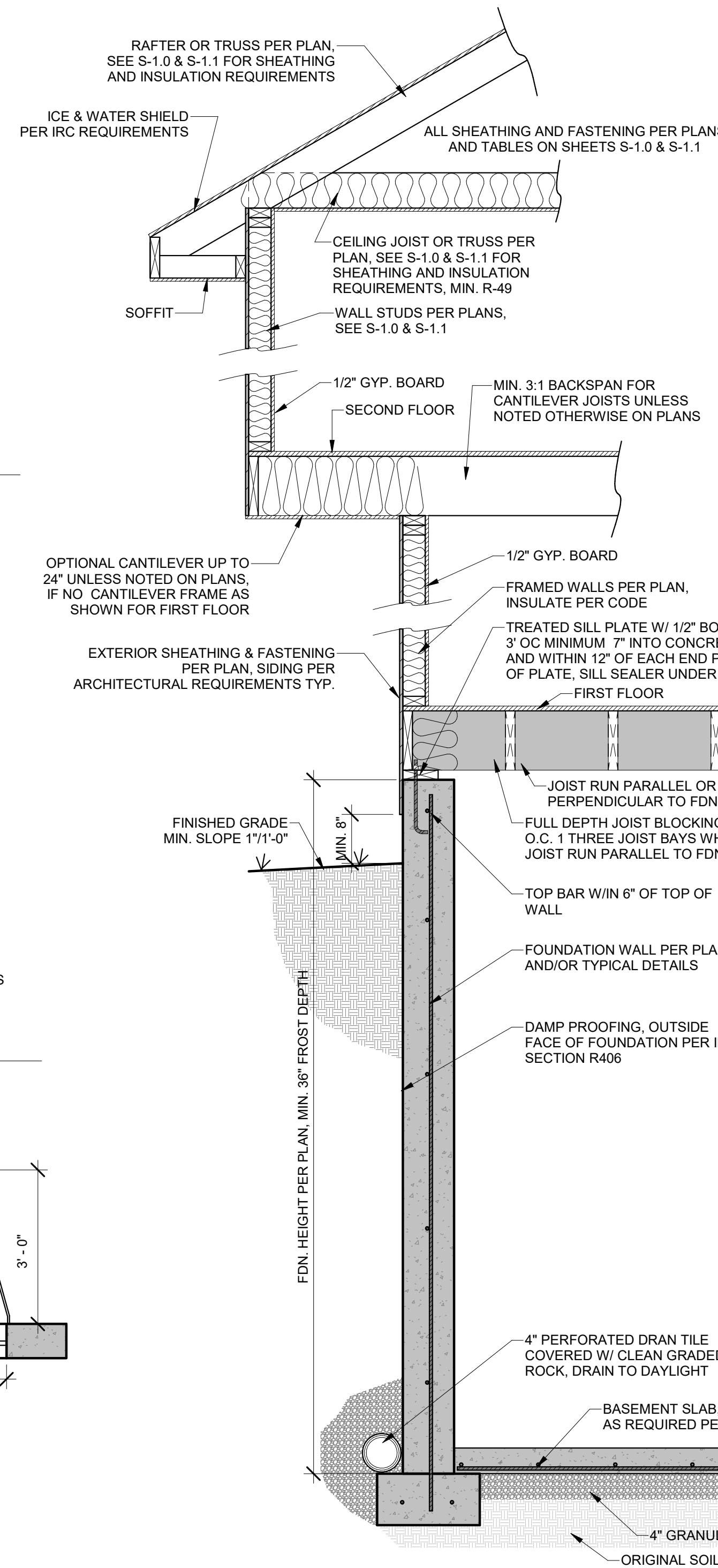
2 EGRESS WINDOW SECTION
1/2" = 1'-0"



9 STEEL BEAM SPLICE DETAIL
1 1/2" = 1'-0"

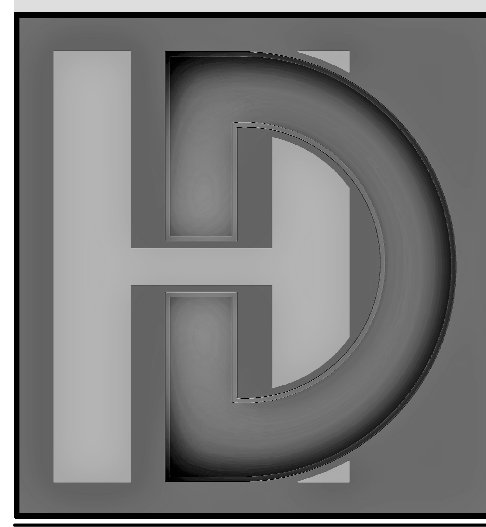
| BEAM SIZE | # BOLTS PER SIDE |
|-----------|------------------|
| W8, W10 | 2 |
| W12, W14 | 3 |
| W16, W18 | 4 |

BOLTS SHALL BE EVENLY SPACED TOP TO BOTTOM

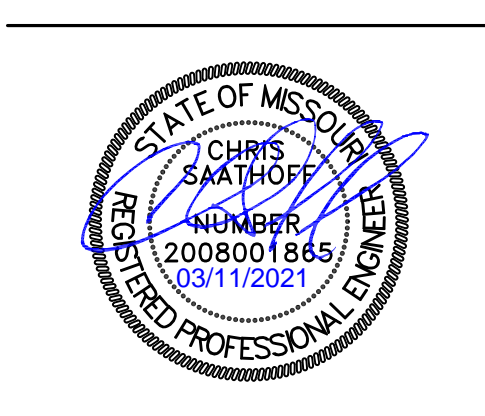


DUE TO THE WIDE VARIETY OF SOIL CONDITIONS IN OUR AREA AND THE WIDE VARIETY OF PLASTICITY INDEX AND SOIL BEARING CAPACITIES OUR FIRM RECOMMENDS ALL SITES BE EVALUATED BY HD ENGINEERING OR AN HD ENGINEERING REFERRED GEOTECHNICAL FIRM PRIOR TO PLACEMENT OF ANY "STANDARD" FOUNDATIONS.

1 TYPICAL WALL SECTION
3/4" = 1'-0"



| HD#: | 41286 | |
|-------------|----------------|---------------|
| DATE: | | 03/11/2021 |
| CHECKED BY: | | CLS |
| NO. | ISSUE/REVISION | Revision Date |
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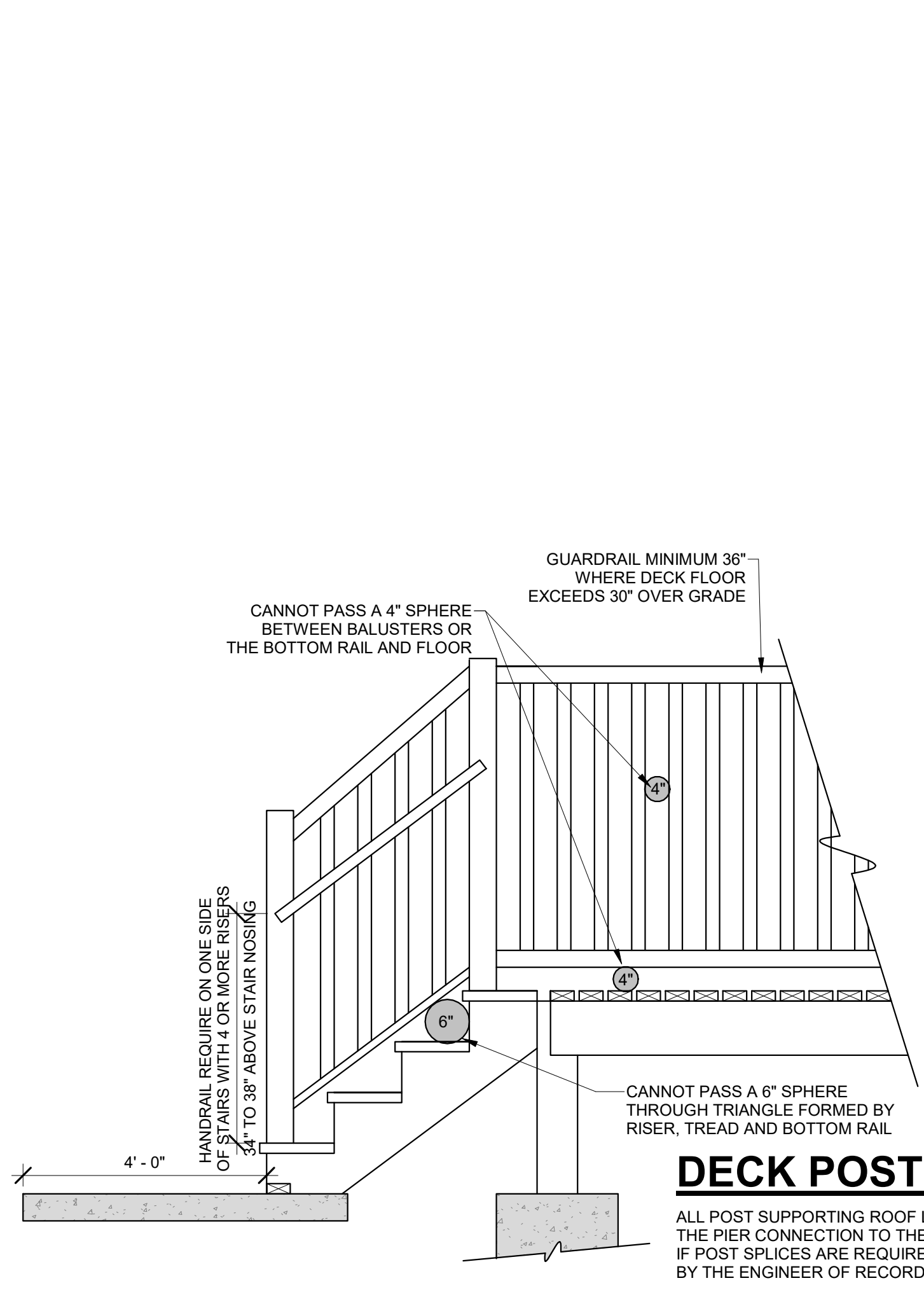


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STRUCTURAL DETAILS & NOTES

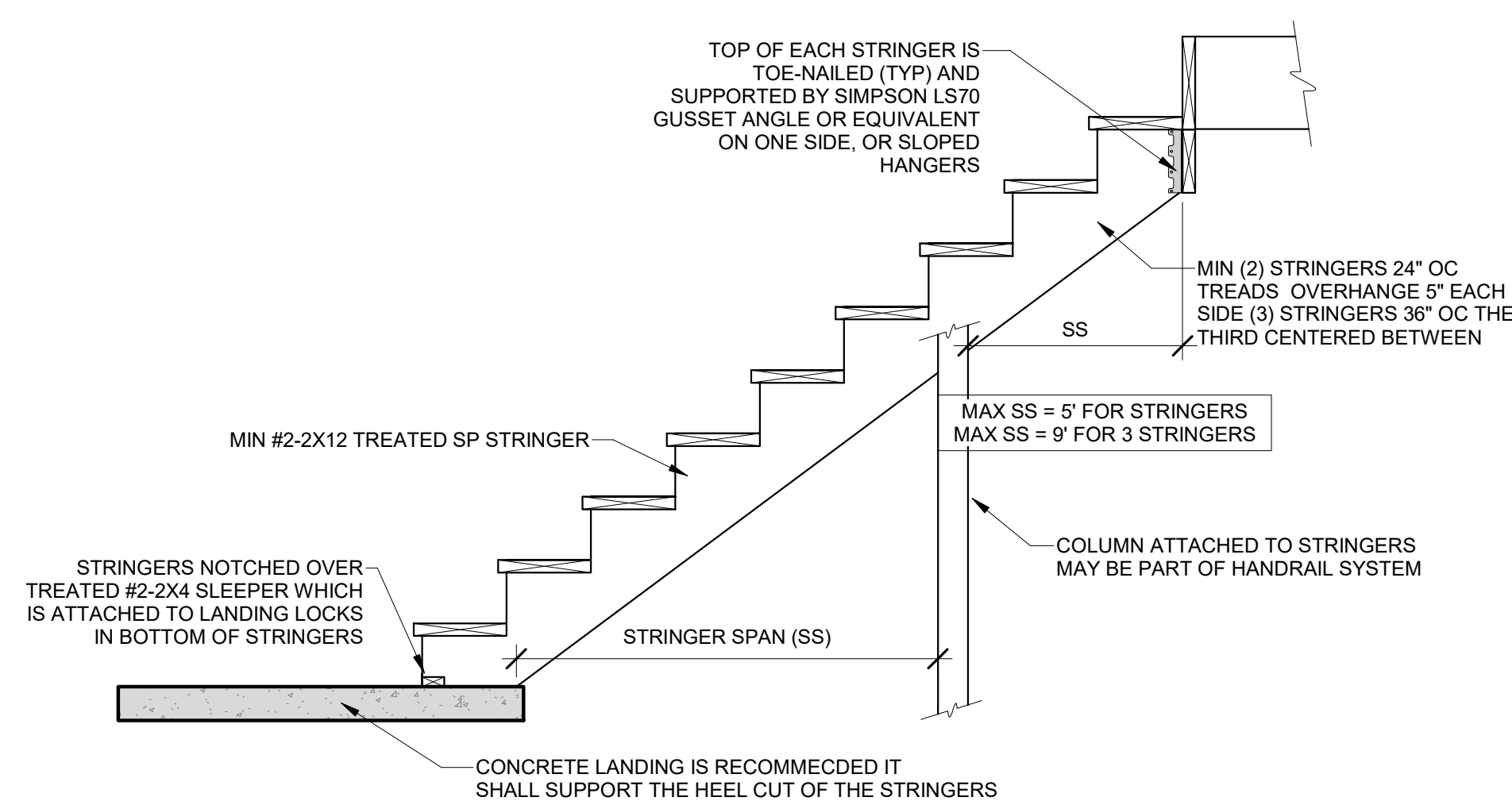
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DECK DETAILS



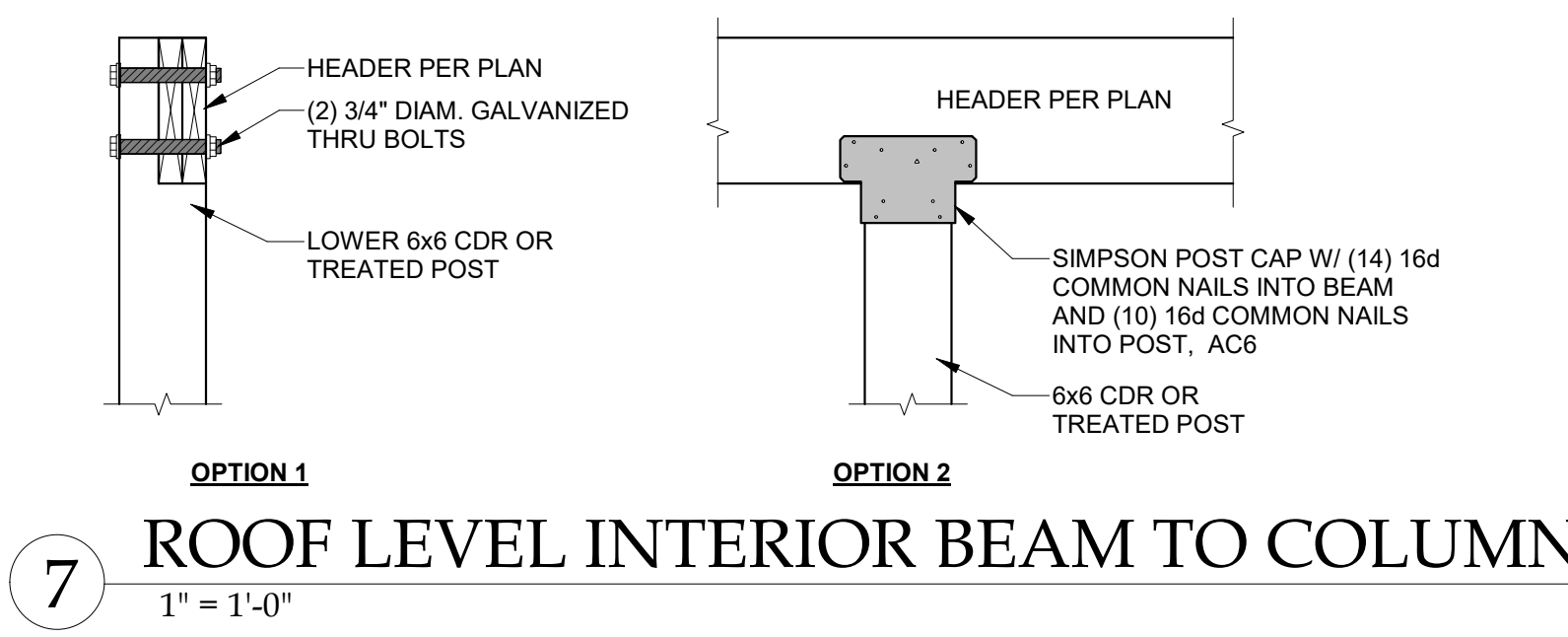
8 GUARD RAIL

1/2" = 1'-0"



9 STAIR STRINGER DETAIL

1/2" = 1'-0"



7 ROOF LEVEL INTERIOR BEAM TO COLUMN

1" = 1'-0"

TABLE IRC2018 R507.9.1.3(1) DECK LEDGER CONNECTION TO BAND JOIST^{a,b} (DECK LIVE LOAD = 40 PSF, DECK HEAD LOAD = 10 PSF, SNOW LOAD ≤ 40 PSF)

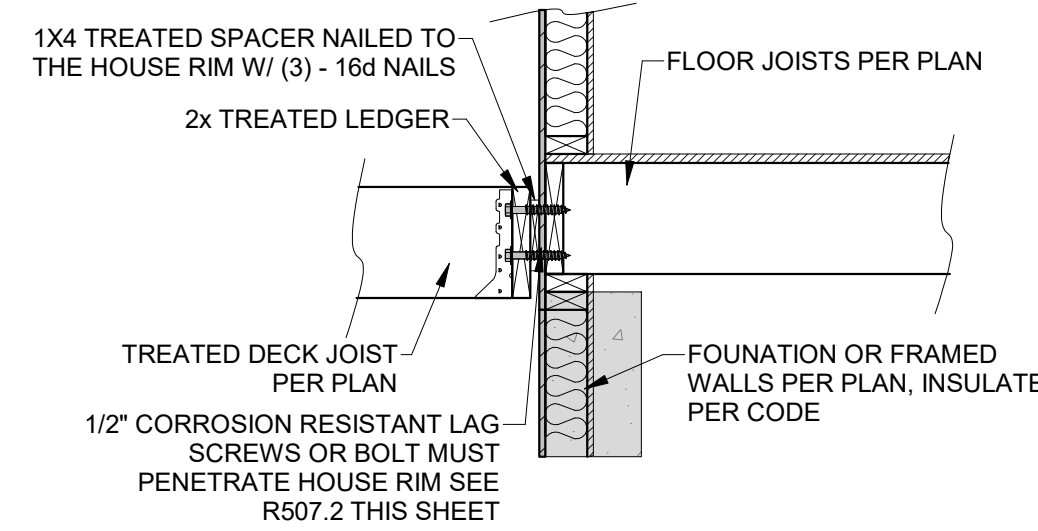
| JOIST SPAN | 6' AND LESS | 6'-1" TO 8' | 8'-1" TO 10' | 10'-1" TO 12' | 12'-1" TO 14' | 14'-1" TO 16' | 16'-1" TO 18' |
|--|---|-------------|--------------|---------------|---------------|---------------|---------------|
| CONNECTION DETAILS | ON-CENTER SPACING OF FASTENERS ^{c,e} | | | | | | |
| 1/2" LAG SCREW WITH 15/32" MAX. SHEATHING ^{c,d} | 30 | 23 | 18 | 15 | 13 | 11 | 10 |
| 1/2" DIAM. BOLT WITH 15/32" MAX. SHEATHING ^d | 36 | 36 | 34 | 29 | 24 | 21 | 19 |
| 1/2" DIAM. BOLT WITH 15/32" MAX. SHEATHING & 1/2" STACKED WASHERS ^e | 36 | 36 | 29 | 24 | 21 | 18 | 16 |

For SI: 1 inch = 25.4mm, 1 foot = 304.8mm, 1 pound per square foot = 0.0479 kPa
a. Ledges shall be flashed in accordance with Section R703.4 to prevent water from contacting the house band joist.
b. Snow load shall not be assumed to act concurrently with live load.
c. The tip of the lag screw shall fully extend beyond the inside face of the band joist.
d. Sheathing shall be wood structural panel or solid sawn lumber.
e. Sheathing shall be permitted to be wood structural panel, gypsum board, fiberboard lumber or foam sheathing. Up to 1/2" thickness of stacked washers shall be permitted to substitute for you to 1/2" of allowable sheathing thickness where combined with wood structural panel or lumbers sheathing.

TABLE IRC2018 R507.9.1.3(2) PLACEMENT OF LAG SCEWS AND BOLT IN DECK LEDGERS AND BAND JOISTS

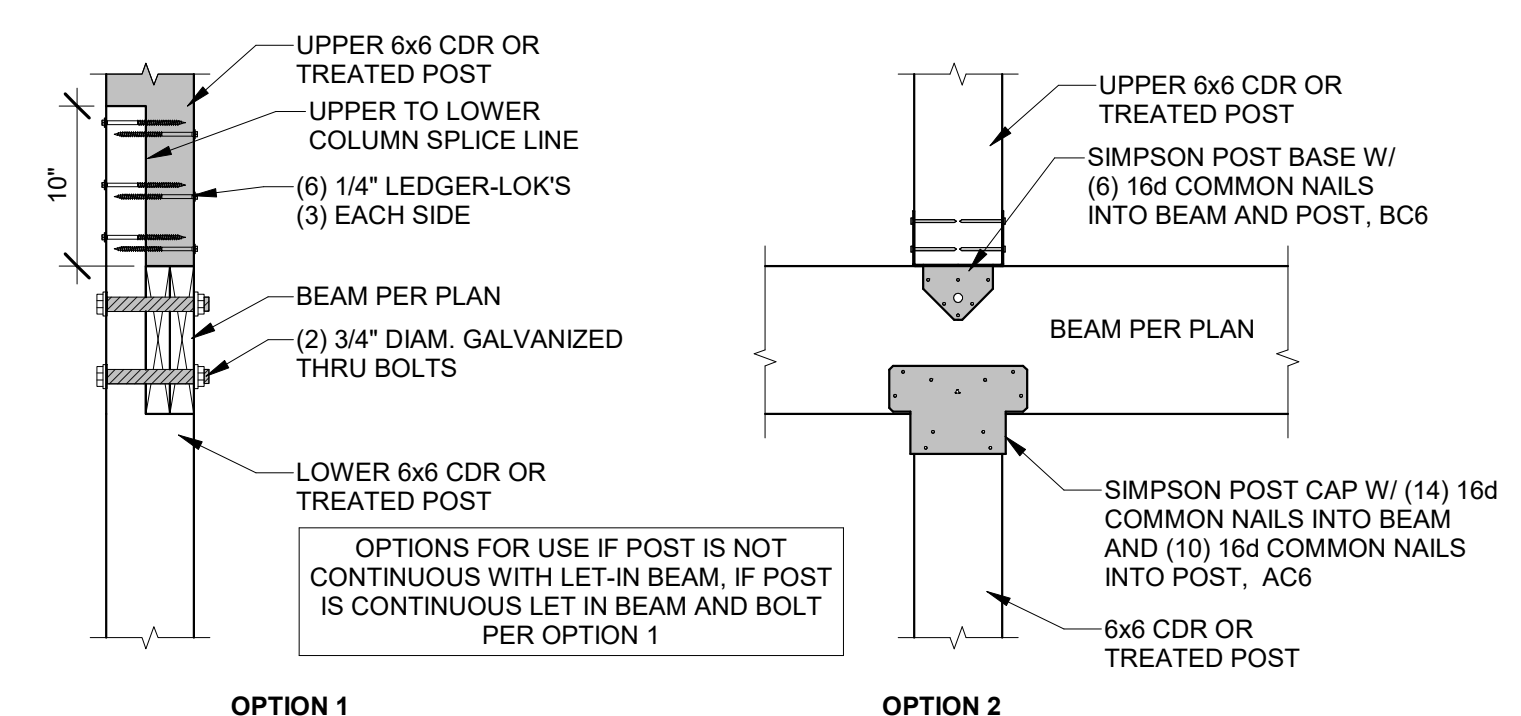
| MINIMUM END AND EDGE DISTANCES AND SPACING BETWEEN ROWS | | | | |
|---|-----------------------|-------------|-----------------------|---------------------------|
| | TOP EDGE | BOTTOM EDGE | ENDS | ROW SPACING |
| LEDGER ^a | 2 inches ^d | 3/4 inches | 2 inches ^b | 1 5/8 inches ^b |
| BAND JOIST ^c | 3/4 inches | 2 inches | 2 inches | 1 5/8 inches ^b |

For SI: 1 inch = 25.4mm.
a. Lag screws or bolts shall be staggered from the top to the bottom along the horizontal run of the deck ledger in accordance with Figure R507.9.1.3(1)
b. Maximum 5 inches
c. For engineered rim joists, the manufacturer's recommendations shall govern.
d. The minimum distances from bottom row of lag screws or bolts to the top of the ledger shall be in accordance with Figure R507.9.1.3(1)



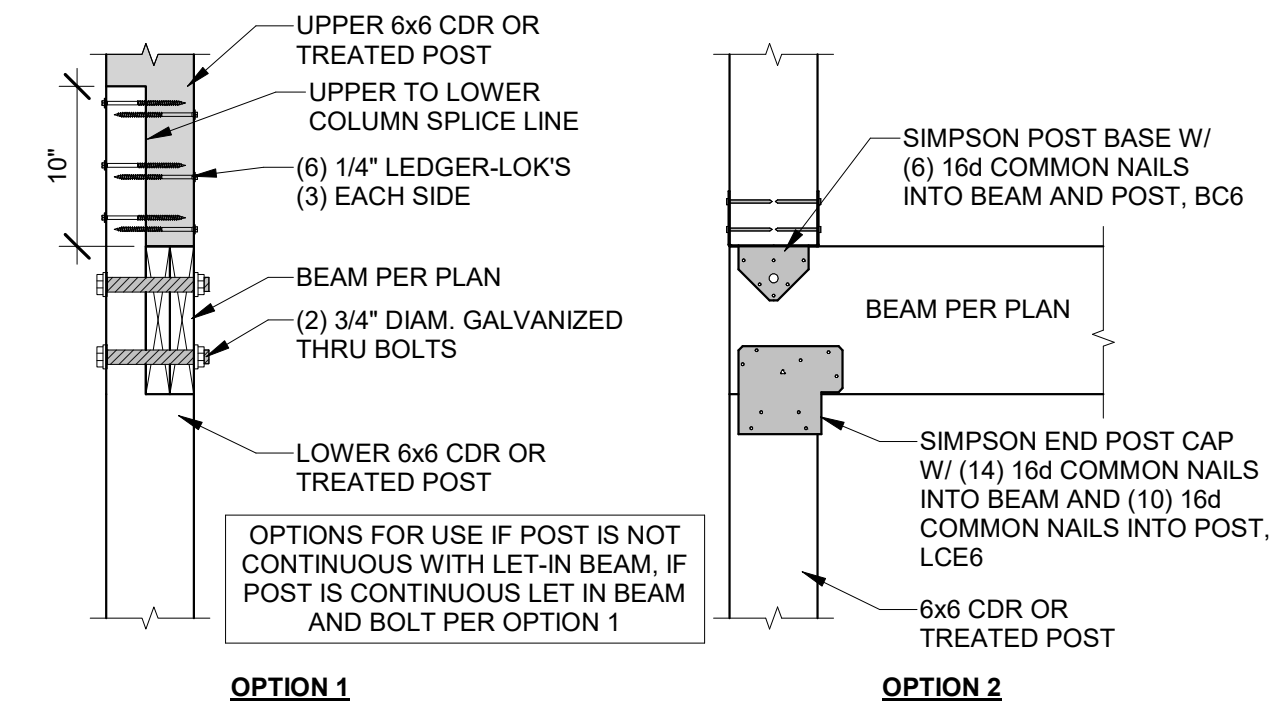
2 DECK LEDGER ATTACHMENT

3/4" = 1'-0"



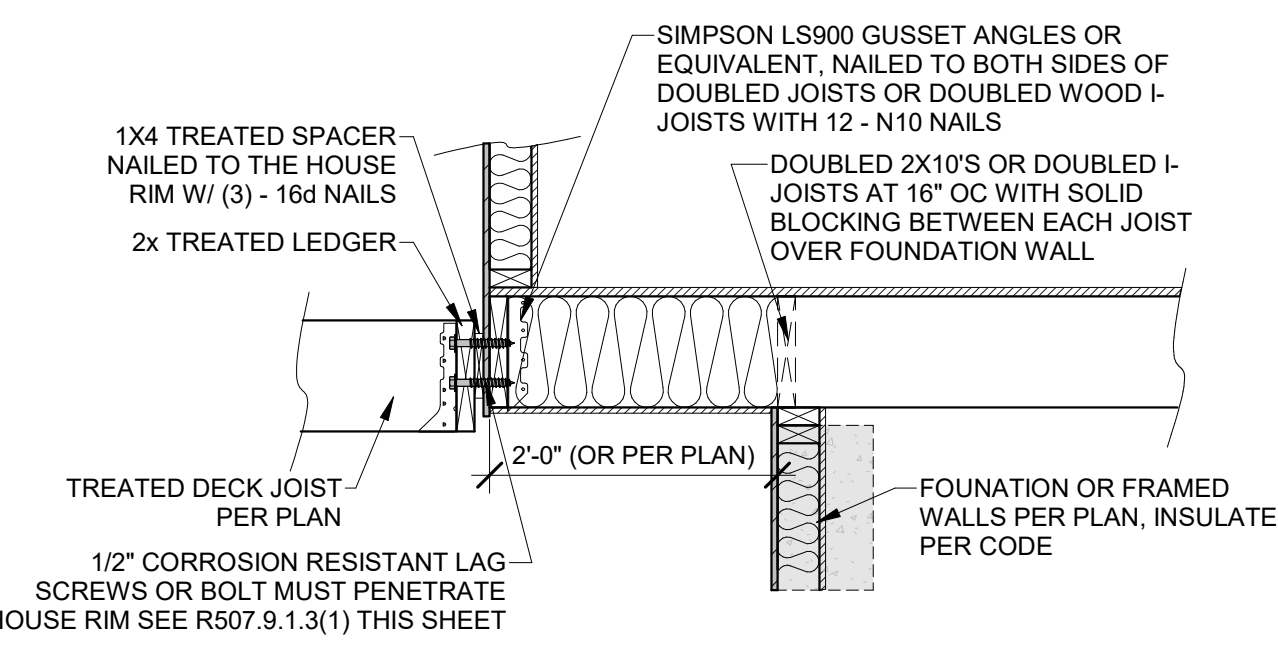
6 DECK LEVEL INTERIOR BEAM TO COLUMN

1" = 1'-0"



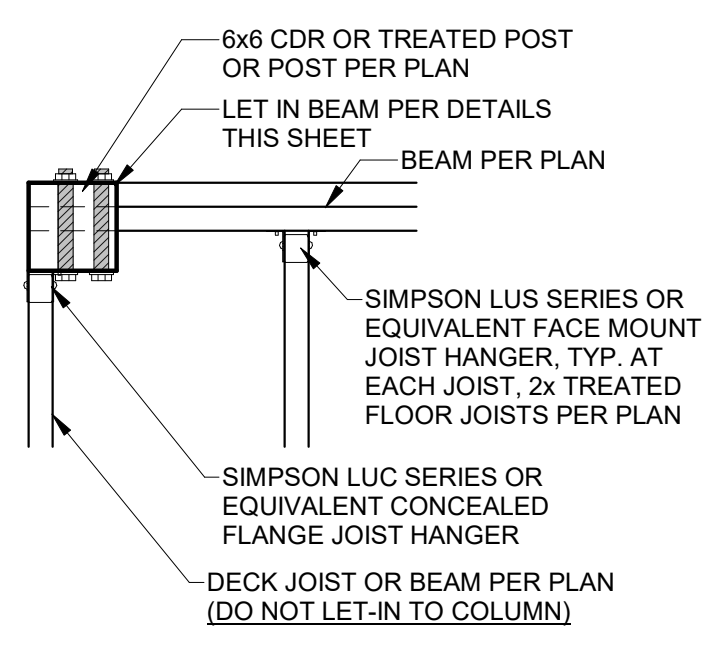
5 DECK LEVEL EXTERIOR BEAM TO COLUMN

1" = 1'-0"



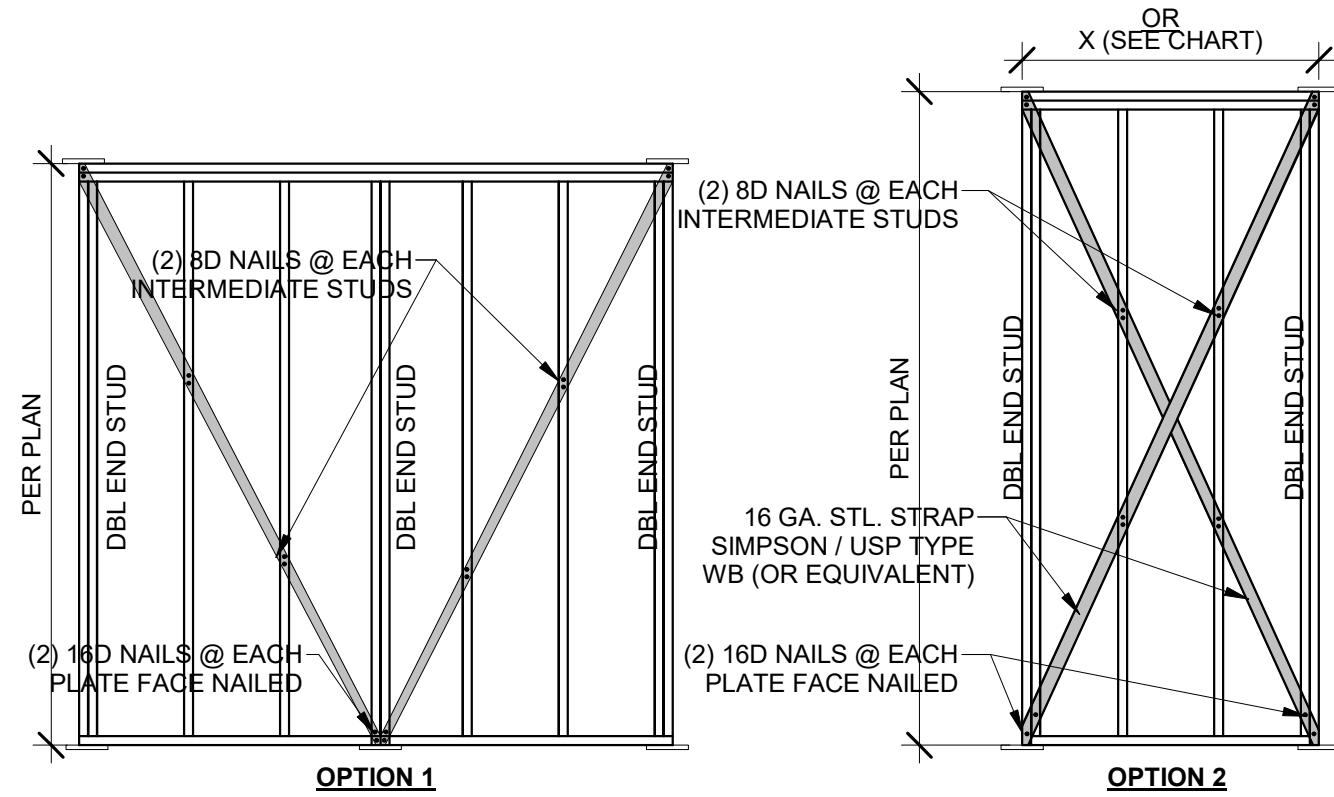
4 DECK LEDGER TO CANTILEVER

3/4" = 1'-0"



1 DECK CORNER COLUMN

1" = 1'-0"



| BRACED WALL PANEL LENGTH BASED ON WALL HEIGHT FOR IRC LIB | | |
|---|----------------------|---------------------|
| WALL HEIGHT | MIN. WALL LENGTH (X) | MAX WALL LENGTH (X) |
| 8'-0" | 4'-7" | 8'-0" |
| 9'-0" | 5'-2" | 9'-0" |
| 10'-0" | 5'-9" | 10'-0" |
| 11'-0" | NP | --- |
| 12'-0" | NP | --- |

6 LIB BRACING

3/8" = 1'-0"

FOR IRC CODE PRESCRIPTIVE METHOD
TABLE R602.10.5 MINIMUM LENGTH OF BRACED WALL PANELS

| METHOD (SEE TABLE R602.10.4) | | MINIMUM LENGTH (INCHES) ^a | | | | | CONTRIBUTING LENGTH (INCHES) |
|---------------------------------|--|--------------------------------------|--------|---------|---------|---------|---|
| | | 8 FEET | 9 FEET | 10 FEET | 11 FEET | 12 FEET | |
| DWB,WSP,SFB,PBS,PCP,HPS,BV-WSP | | 48 | 48 | 48 | 53 | 58 | ACTUAL ^b |
| GB | | 48 | 48 | 48 | 53 | 58 | DOUBLE SIDED = ACTUAL SINGLE SIDED=.5xACTUAL |
| LIB | | 55 | 62 | 69 | NP | NP | ACTUAL ^b |
| ABW | SDC A, B, AND C ULTIMATE DESIGN WIND SPEED<140 | 28 | 32 | 34 | 38 | 42 | 48 |
| | SDC D, D, D ULTIMATE DESIGN WIND SPEED<140 | 32 | 32 | 34 | NP | NP | |
| PFH | SUPPORTING ROOF ONLY | 16 | 16 | 16 | NOTE C | NOTE C | 48 |
| | SPTNG. ONE STORY & ROOF | 24 | 24 | 24 | NOTE C | NOTE C | |
| PFG | | 24 | 27 | 30 | NOTE D | NOTE D | 1.5 x ACTUAL ^b |
| CS-G | | 24 | 27 | 30 | 33 | 36 | ACTUAL ^b |
| CS-PF | | 16 | 18 | 20 | NOTE E | NOTE E | ACTUAL ^b |
| CS-WSP, CS-SFB | ADJACENT CLEAR OPENING HEIGHT (INCHES) | | | | | | ACTUAL ^b |
| | ≤64 | 24 | 27 | 30 | 33 | 36 | |
| | 68 | 26 | 27 | 30 | 33 | 36 | |
| | 72 | 27 | 27 | 30 | 33 | 36 | |
| | 76 | 30 | 29 | 30 | 33 | 36 | |
| | 80 | 32 | 30 | 30 | 33 | 36 | |
| | 84 | 35 | 32 | 32 | 33 | 36 | |
| | 88 | 38 | 35 | 33 | 33 | 36 | |
| | 92 | 43 | 37 | 35 | 35 | 36 | |
| | 96 | 48 | 41 | 38 | 36 | 36 | |
| | 100 | - | 44 | 40 | 38 | 38 | |
| | 104 | - | 49 | 43 | 40 | 39 | |
| | 108 | - | 54 | 46 | 43 | 41 | |
| | 112 | - | - | 50 | 45 | 43 | |
| | 116 | - | - | 55 | 48 | 45 | |
| | 120 | - | - | 60 | 52 | 48 | |
| | 124 | - | - | - | 56 | 51 | |
| | 128 | - | - | - | 61 | 54 | |
| | 132 | - | - | - | 66 | 58 | |
| | 136 | - | - | - | - | 62 | |
| | 140 | - | - | - | - | 66 | |
| | 144 | - | - | - | - | 72 | |

- a. LINEAR INTERPOLATION SHALL BE PERMITTED
b. USE THE ACTUAL LENGTH WHEN IT IS GREATER THAN OR EQUAL TO THE MINIMUM LENGTH
c. MAX. HEADER HEIGHT FOR PFH IS 10' IN ACCORDANCE WITH R602.10.6.2, WALL HEIGHT MAY BE INCREASED TO 12' WITH PONY WALL
d. MAX. OPENING HEIGHT FOR PFG IS 10' IN ACCORDANCE WITH R602.10.6.3, WALL HEIGHT MAY BE INCREASED TO 12' WITH PONY WALL
e. MAX. OPENING HEIGHT FOR CS-PF IS 10' IN ACCORDANCE WITH R602.10.6.4, WALL HEIGHT MAY BE INCREASED TO 12' WITH PONY WALL

BRACED WALL PRESCRIPTIVE METHOD:
CONTINUOUS EXTERIOR SHEATHING (CS-WSP) PER WSP METHOD (BELOW) UNLESS OTHERWISE NOTED ON THE PLAN

EXTERIOR BRACED WALL METHOD: (SEE ON THIS SHEET)

WOOD STRUCTURAL PANEL SHEATHING WITH A THICKNESS NOT LESS THAN 3/8" WITH MINIMUM SPAN RATING OF 24/0 FOR 16" O.C. STUD SPACING WITH 8d NAILS COMMON NAILS @ 6" O.C. EDGES AND 12" O.C. FIELD OR SHEATHING THICKNESS NOT LESS THAN 7/16" WITH MINIMUM SPAN RATING OF 24/16 FOR 24" O.C. SPACING WITH 8d COMMON NAILS @ 6" O.C. EDGES AND 12" O.C. IN FIELD
(NOTE: FRAMING MEMBERS 16" O.C. MAX, UNBLOCKED, AND W/ SHEATHING APPLIED DIRECTLY TO FRAMING MEMBERS).

INTERIOR BRACED WALLS (SEE ON THIS SHEET)

GB METHOD:
1/2" MINIMUM GYPSUM BOARD OVER STUDS SPACED @ 24" MAXIMUM FASTENED W/ #6- 1 1/4" TYPE "W" OR "S" DRYWALL SCREWS @ 7" O.C. EDGES AND FIELD (MIN. 4'-0" SECTION FOR BOTH SIDES)
OR
LIB METHOD:
1X4 WOOD FASTENED W/ (3) 8d COMMON NAILS OR SIMPSON / USP 16 GA. TYPE WB (OR EQUIVALENT) STL. X-BRACE(S) @ 45° TO 60° ANGLES, MAXIMUM 16" O.C. STUDS FASTENED PER MANUF. SPECS.

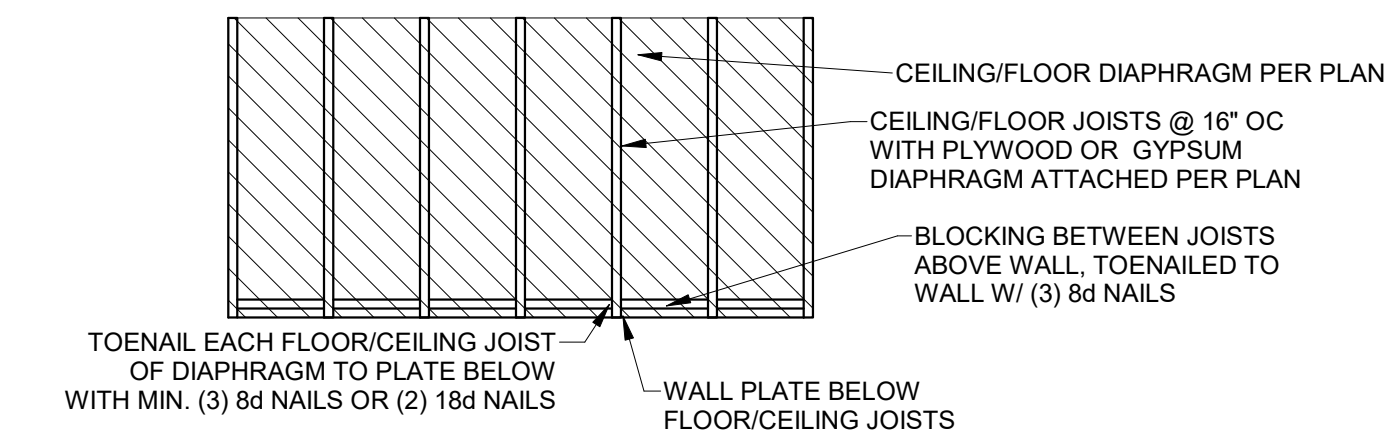
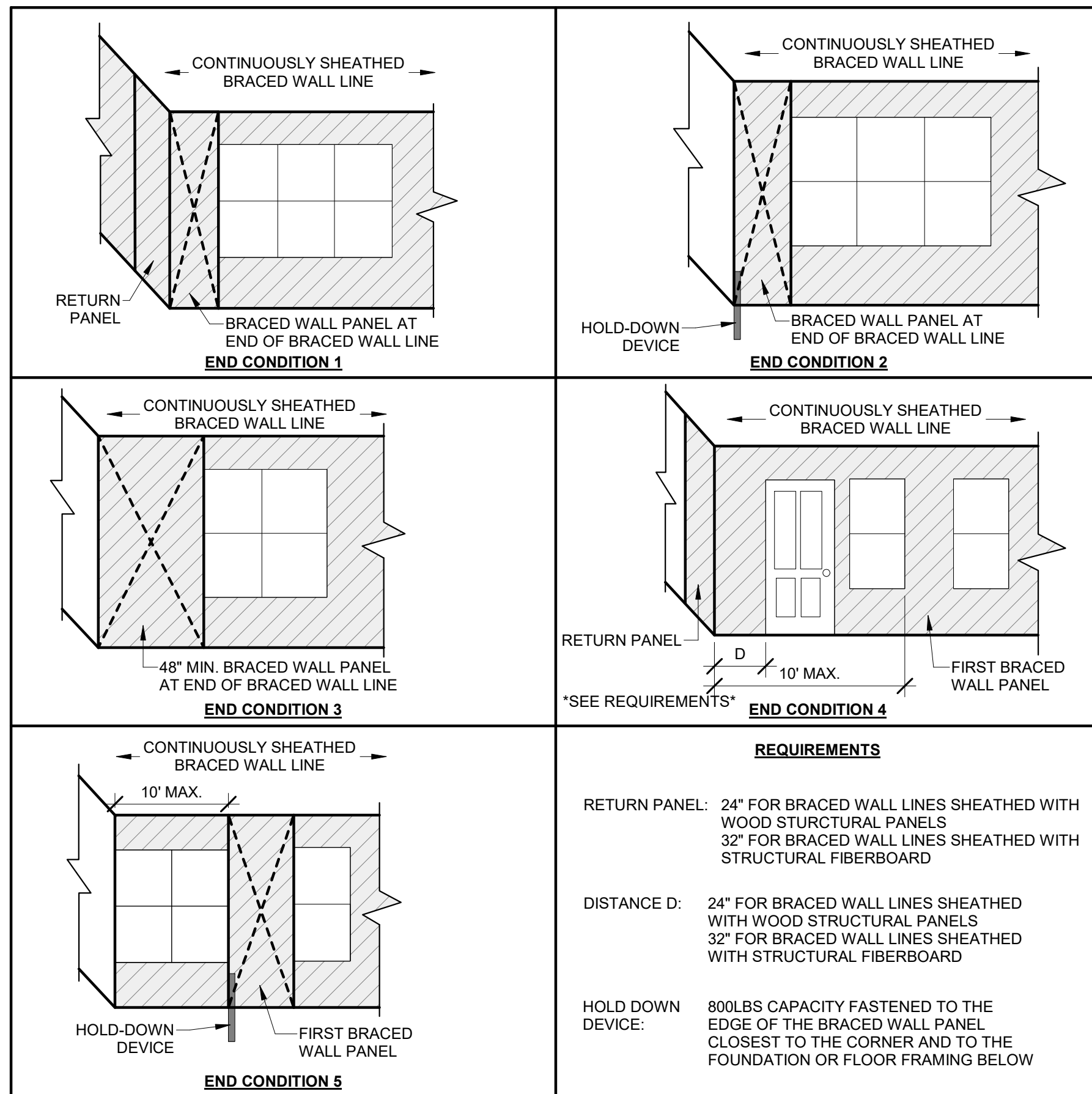
TENSION STRAP CAPACITY REQUIRED FOR RESISTING WIND PRESSURES PERPENDICULAR TO METHOD PFH, PFG AND CS-PF BRACED WALL PANELS IRC2018 TABLE R602.10.6.4

| MINIMUM WALL STUD FRAMING NOMINAL SIZE & GRADE | MAX. PONY WALL HEIGHT (FEET) | MAX. TOTAL WALL HEIGHT (FEET) | MAX. OPENING WIDTH (FEET) | TENSION STRAP CAPACITY REQUIRED (POUNDS) ^a | |
|---|------------------------------------|-------------------------------------|------------------------------|---|-------|
| | | | | ULTIMATE DESIGN WIND SPEED V (MPH) | |
| | | | | 115 | 115 |
| 2X4 NO. 2 GRADE | 0 | 10 | 18 | 1,000 | 1,000 |
| | | | 9 | 1,000 | 1,000 |
| | | | 16 | 1,025 | 2,500 |
| | 1 | 10 | 18 | 1,275 | 2,850 |
| | | | 9 | 1,000 | 1,875 |
| | | | 16 | 2,175 | 4,125 |
| | 2 | 10 | 18 | 2,500 | DR |
| | | | 9 | 1,500 | 3,175 |
| | | | 16 | 3,375 | DR |
| | 2 | 12 | 18 | 3,975 | DR |
| | | | 9 | 2,750 | DR |
| | | | 12 | 3,775 | DR |
| 2X6 STUD GRADE | 2 | 12 | 9 | 1,000 | 2,025 |
| | | | 16 | 2,150 | 3,675 |
| | | | 18 | 2,550 | DR |
| | 4 | 12 | 9 | 1,750 | 3,125 |
| | | | 16 | 2,400 | DR |
| | | | 18 | 3,800 | DR |

- a. DR = DESIGN REQUIRED
b. STRAP SHALL BE INSTALLED IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS.

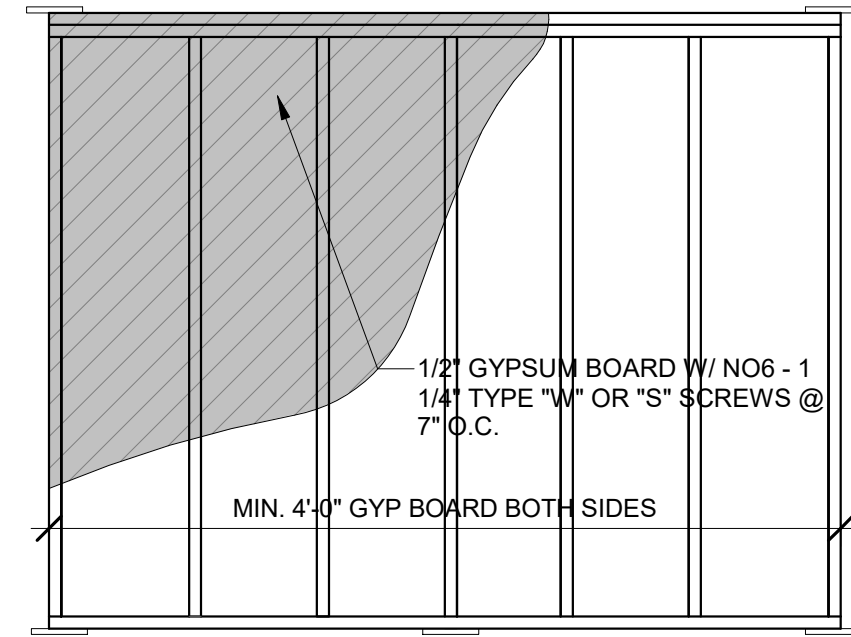
END WALL CONDITIONS

FOR CONTINUOUSLY SHEATHED BRACED WALL LINES



1 DIAPHRAGM CONNECTION TO INTERIOR WALL

3/8" = 1'-0"

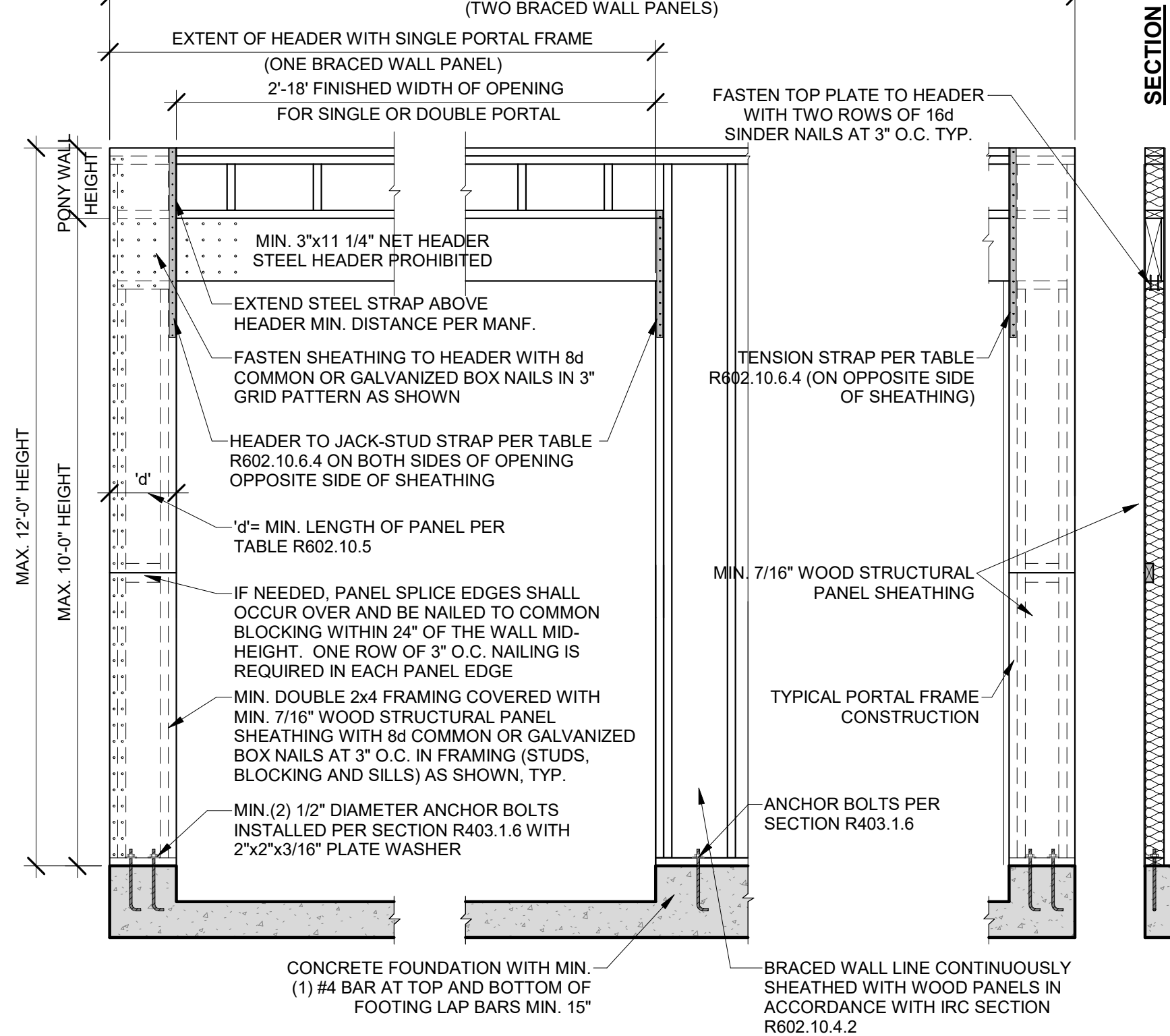


5 GB BRACING

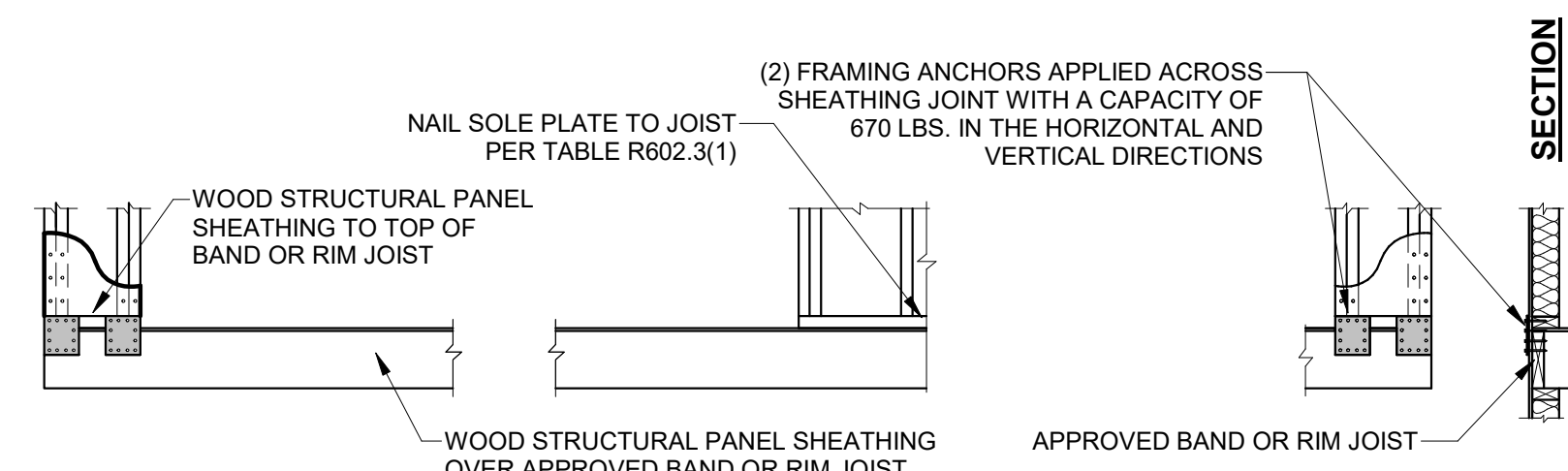
1/2" = 1'-0"

FRONT ELEVATION

EXTENT OF HEADER WITH DOUBLE PORTAL FRAMES
(TWO BRACED WALL PANELS)

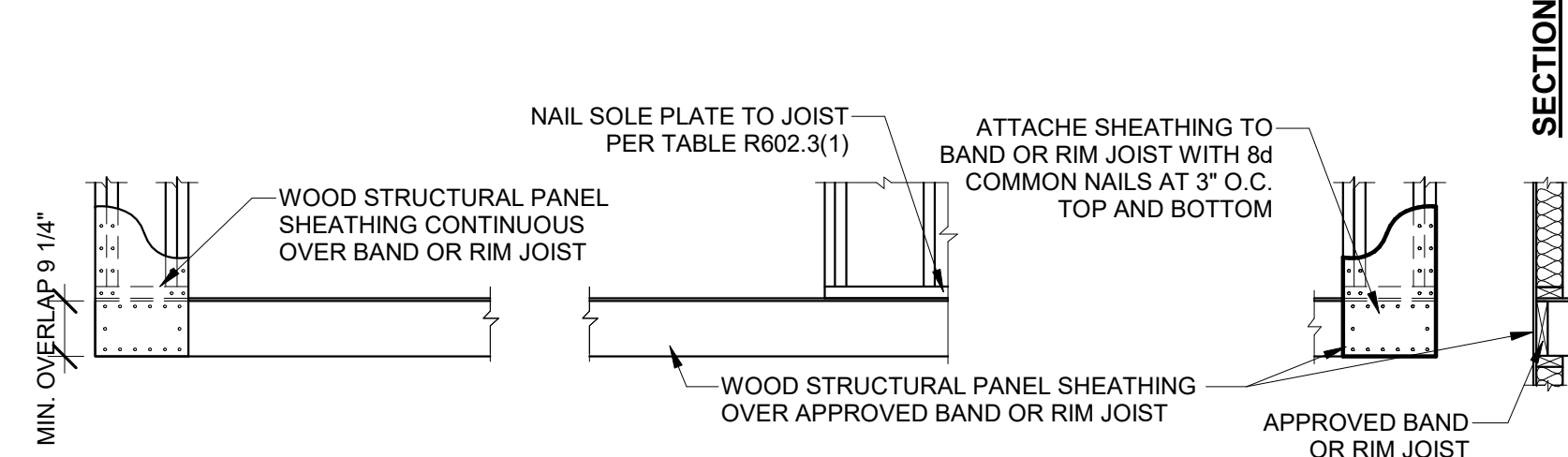


OVER CONCRETE OR MASONRY BLOCK FOUNDATION



OVER RAISED WOOD FLOOR - FRAMING ANCHOR OPTION

(WHEN PORTAL SHEATHING DOES NOT LAP OVER BAND OR RIM JOIST)



OVER RAISED WOOD FLOOR - OVERLAP OPTION

(WHEN PORTAL SHEATHING LAPS OVER BAND OR RIM JOIST)



4 CS-PF

1/2" = 1'-0"

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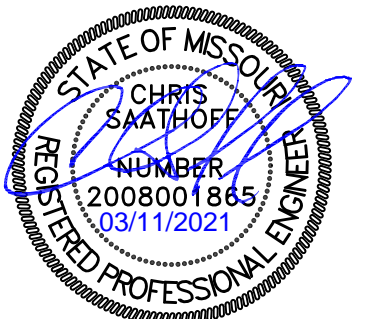
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SERVICE@HDENGINEERS.COM



SAB HOMES, INC.

REDBUD E718

2366 SW. OLD PORT RD. LEE'S SUMMIT, MO

STRUCTURAL DETAILS & NOTES

HD#: 41286

DATE: 03/11/2021

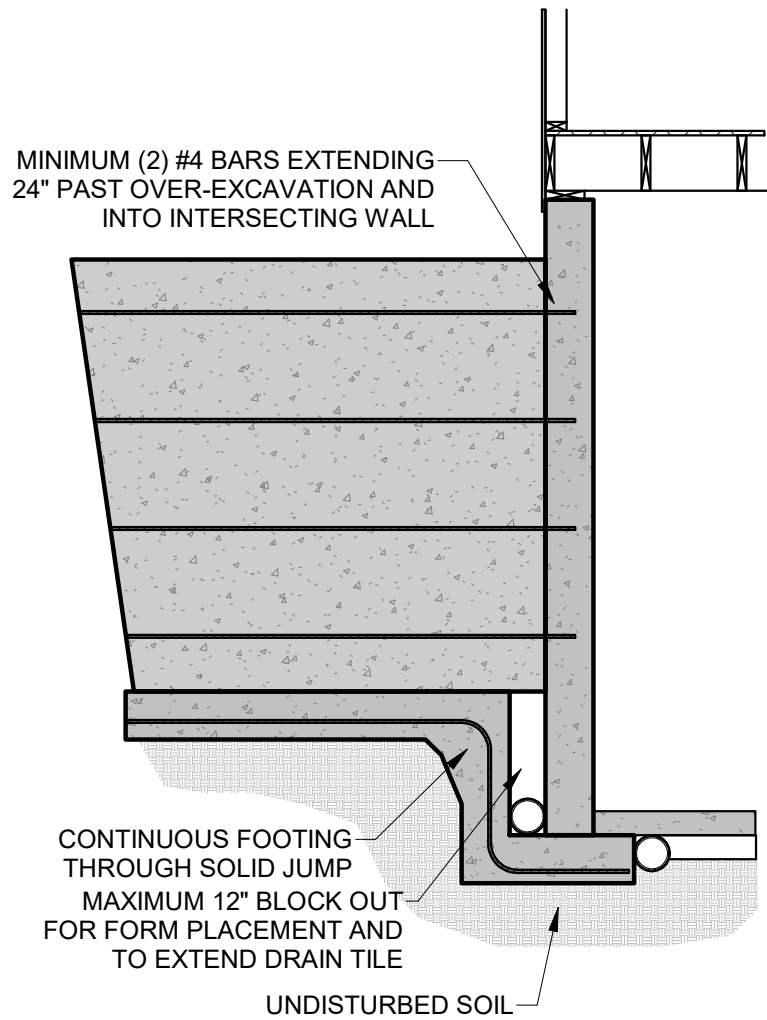
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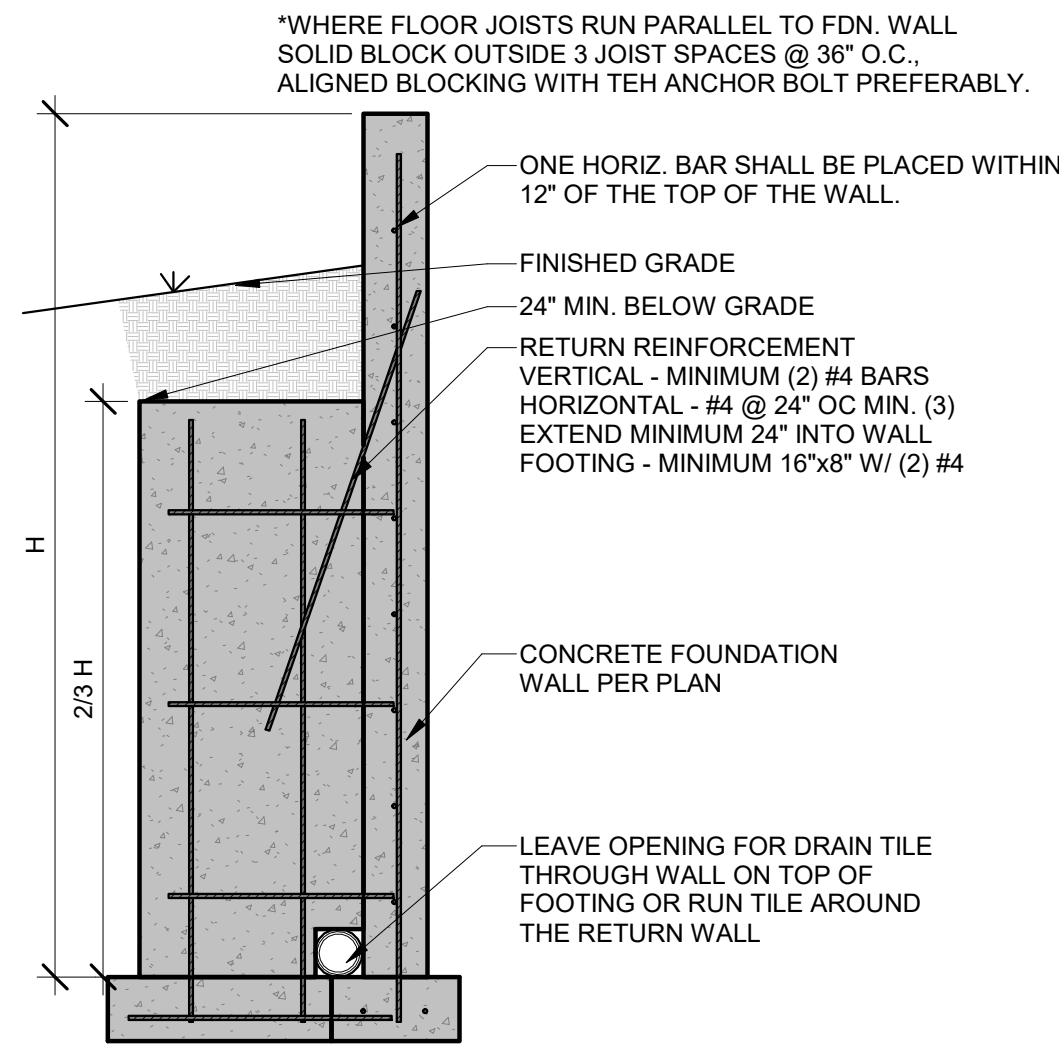
BRACED WALLS NOTES & DETAILS

S-2.1

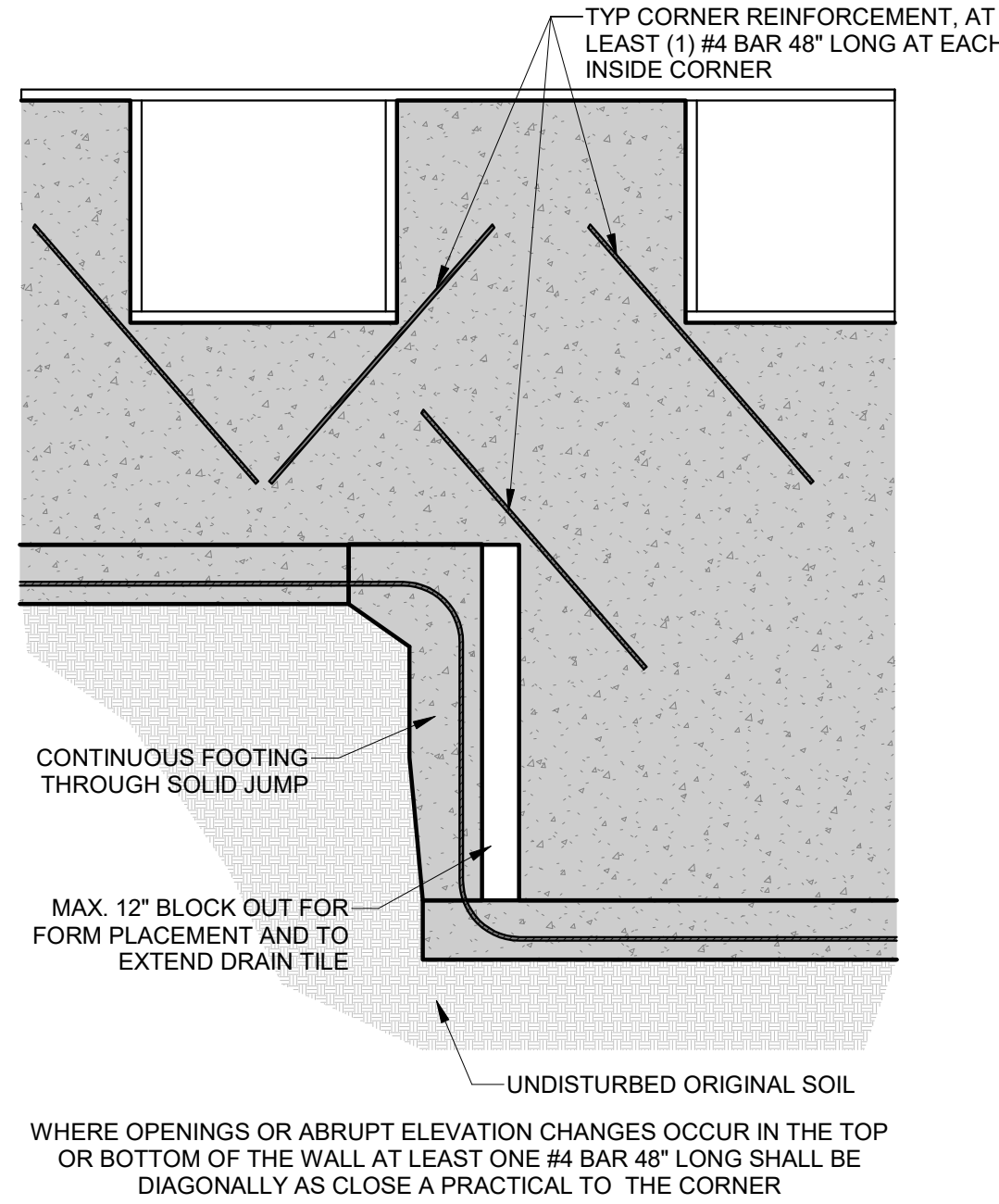
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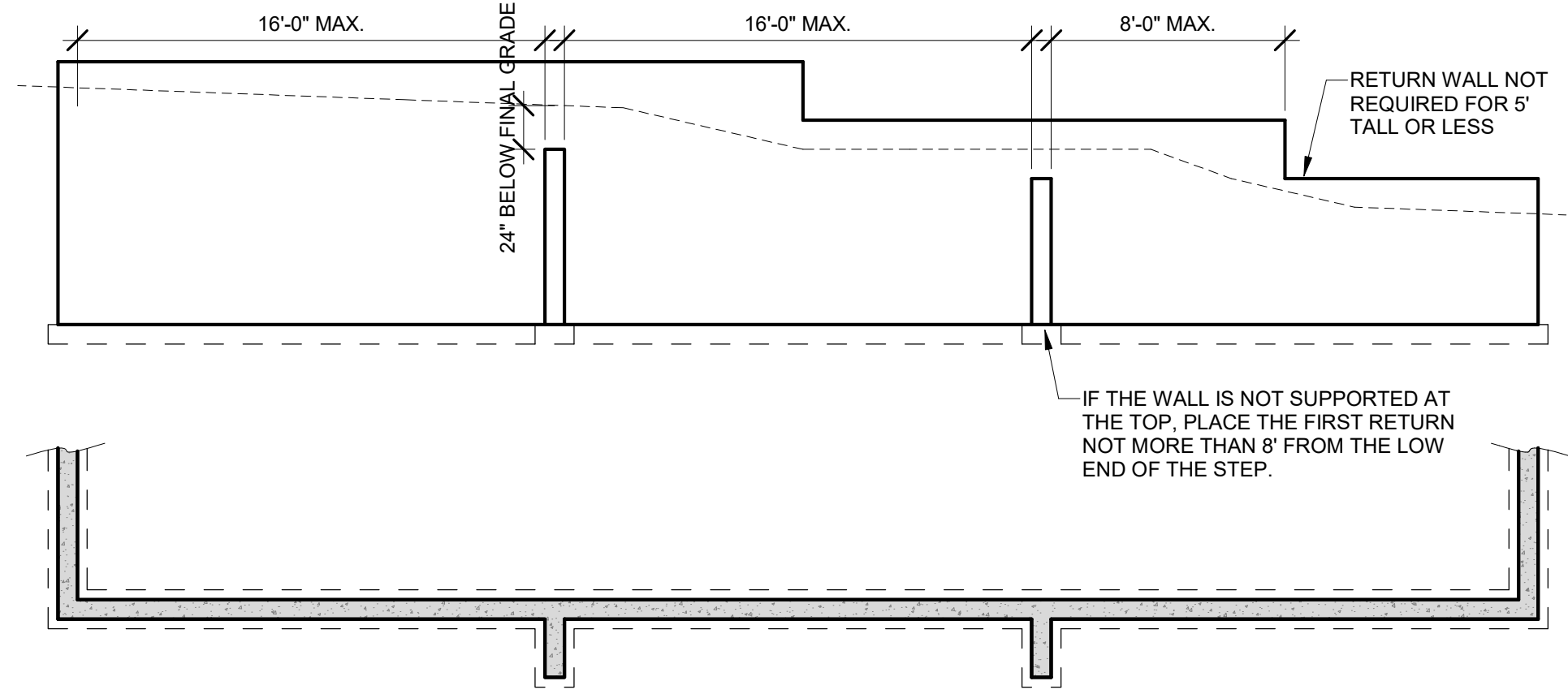
1 SOLID FOOTING JUMP DETAIL
3/8" = 1'-0"



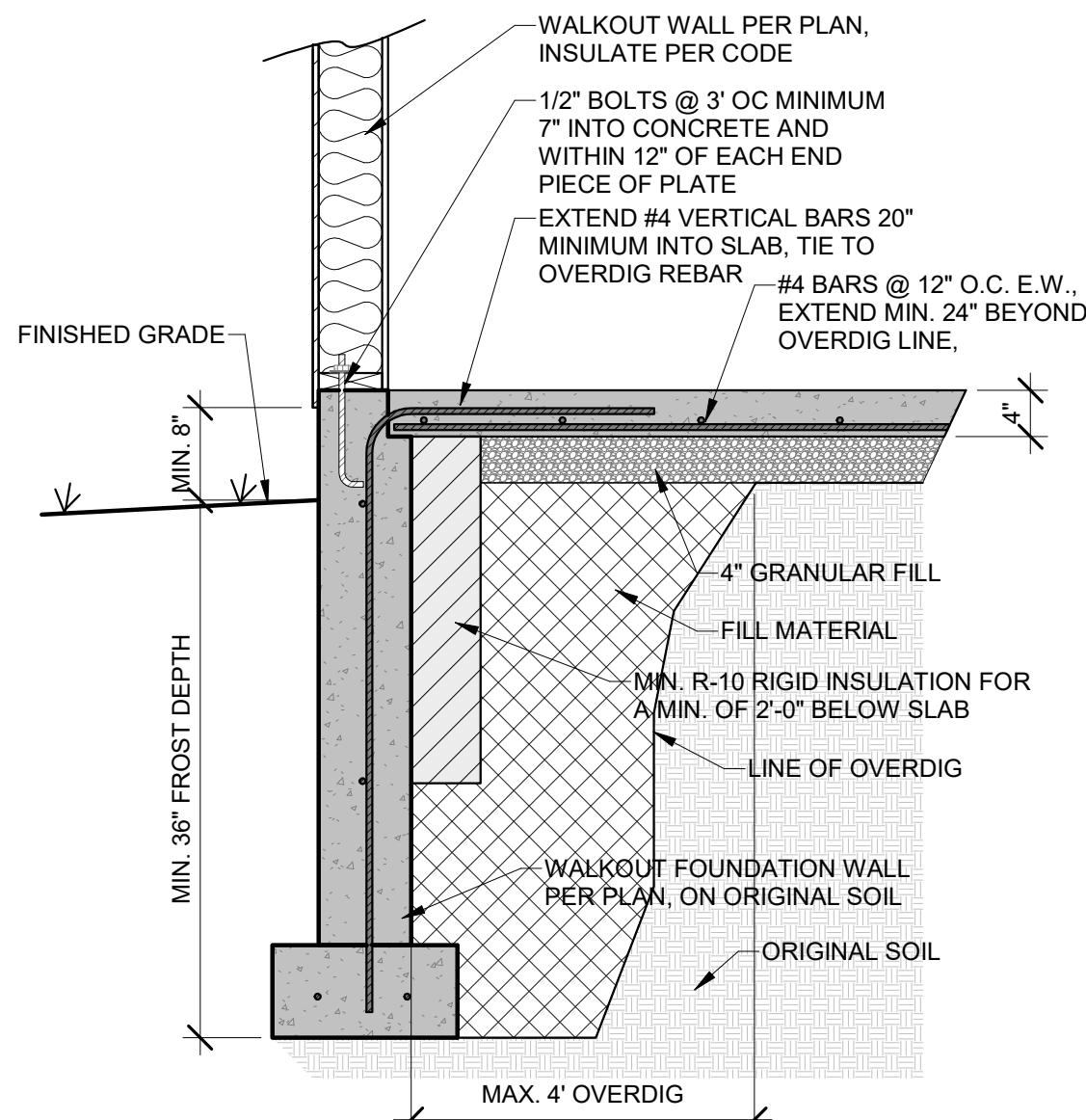
2 RETURN WALL DETAIL
1/2" = 1'-0"



3 REINFORCEMENT AT CORNERS AND STEPS
1/2" = 1'-0"



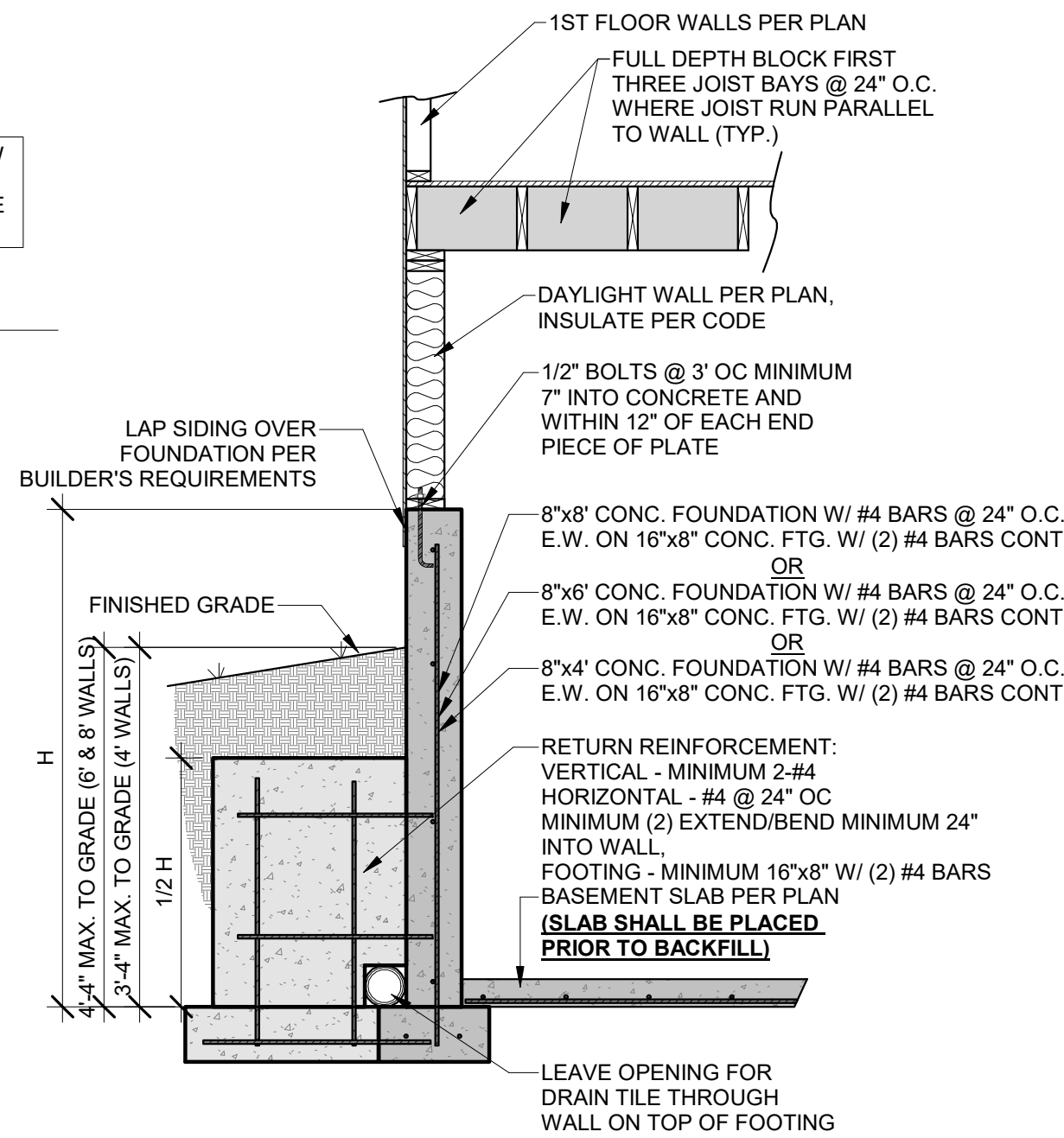
4 RETURN WALL PLACEMENT
3/16" = 1'-0"



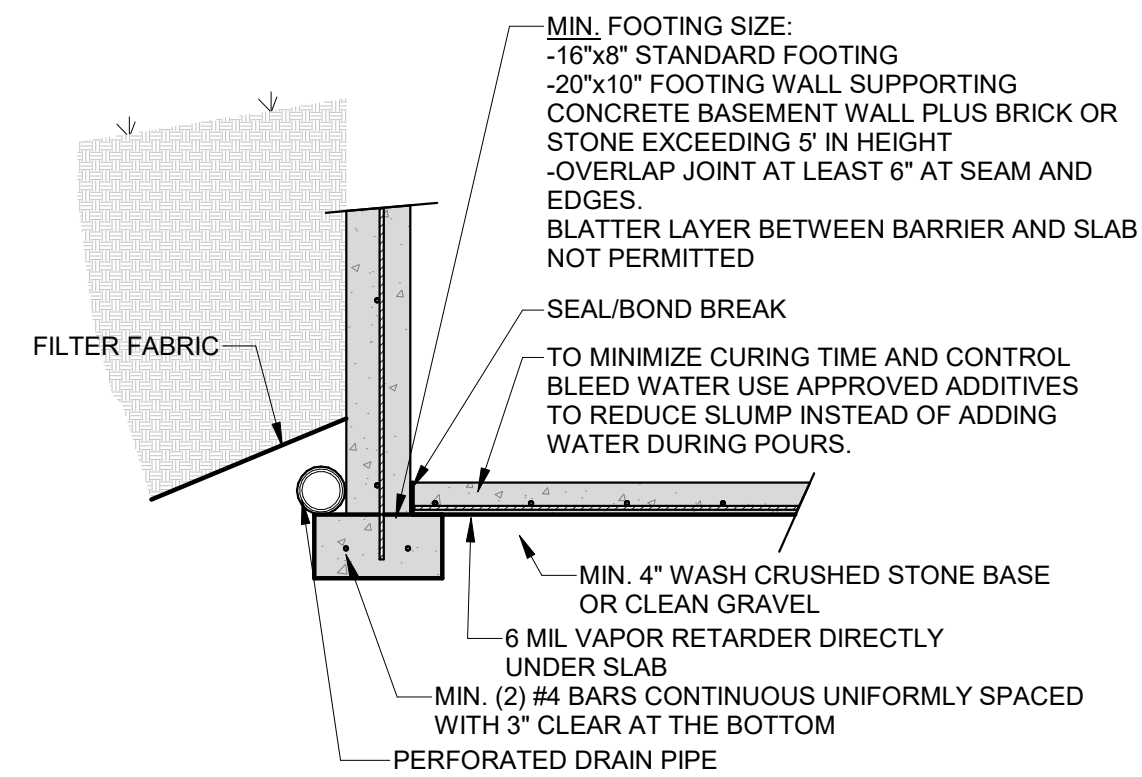
IF OVER 4' OVERDIG SEE HD ENGINEERING FOR STRUCTURAL BASEMENT SLAB DESIGN

IMPORTANT NOTE:
ANY SLAB WITH GREATER THAN 2' OF GRADED ROCK OR 8" OF FILL SOIL BELOW SHALL BE DESIGNED AS STRUCTURAL PER PLAN. OUR FIRM SHOULD BE CONTACTED IMMEDIATELY FOR DESIGN RECOMMENDATIONS. DESIGN MUST BE COMPLETED PRIOR TO PLACEMENT OF PIERS OR FOOTINGS.

6 WALKOUT DETAIL
3/4" = 1'-0"



7 FOUNDATION FOOTINGS
1/2" = 1'-0"



8 UNRESTRAINED FOUNDATION WALL
1/2" = 1'-0"

| VERTICAL REINFORCEMENT SPACING* 60 PSF SOIL; 40 & 60 KSI STEEL | | | | | |
|---|---------------|-------|----------------|-------|-------|
| CONCRETE STRENGTH | 8" THICK WALL | | 10" THICK WALL | | |
| | 8' | 9' | 8' | 9' | 10' |
| 3000 PSI/ 40 KSI | 16 | 12 | 24 | 16 | 12 |
| 3500 PSI/ 40 KSI | 16 | 12 | 24 | 24 | 12 |
| 3000 PSI/ 60 KSI | 24 | 16 | 24 | 20 | 16 |
| 3500 PSI/ 60 KSI | 24 | 16 | 24 | 24 | 16 |
| HORIZONTAL REINFORCEMENT** | | | | | |
| ONE BAR 12" FROM TOP OF WALL; MAX. SPACING 24" O.C. | 4- #4 | 5- #4 | 4- #4 | 5- #4 | 6- #4 |

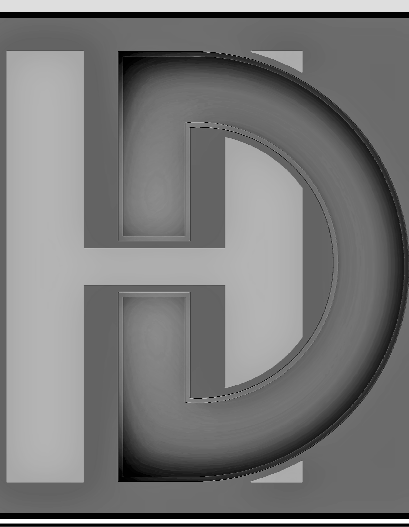
* CONCRETE SHALL HAVE AIR ENTRAINMENT OF 5-7%.
* MINIMUM REQUIREMENT FOR VERTICAL REBAR IN PLAIN CONCRETE WALLS IS #4 @ 36" ON CENTER (ACI 332).
* VERTICAL BARS SHALL BE CONTINUED UP TO WITHIN 8" OF THE TOP OF THE WALL.
* REBAR SHALL BE POSITIONED AT THE TENSION FACE OF THE WALL (2" FROM THE INSIDE FACE).
* REINFORCEMENT SHALL LAP A MINIMUM OF 24 INCHES AT ENDS, SPLICES, AND AROUND CORNERS.

** #4 BARS @ 24" ON CENTER.
** #4 BAR WITHIN 12 OF TOP AND BOTTOM OF WALL.
** MINIMUM GRADE 40 (40ksi) STEEL (PER ACI 332).
** HORIZONTAL REINFORCEMENT SHALL BE INSTALLED ON THE COMPRESSION SIDE (SOIL SIDE) OF THE VERTICAL REINFORCEMENT

DETAILS PROVIDED ARE DERIVED FROM JOHNSON COUNTY RESIDENTIAL FOUNDATION GUIDELINE

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REDBUD E718
2366 SW. OLD PORT RD. LEE'S SUMMIT, MO

STRUCTURAL DETAILS & NOTES

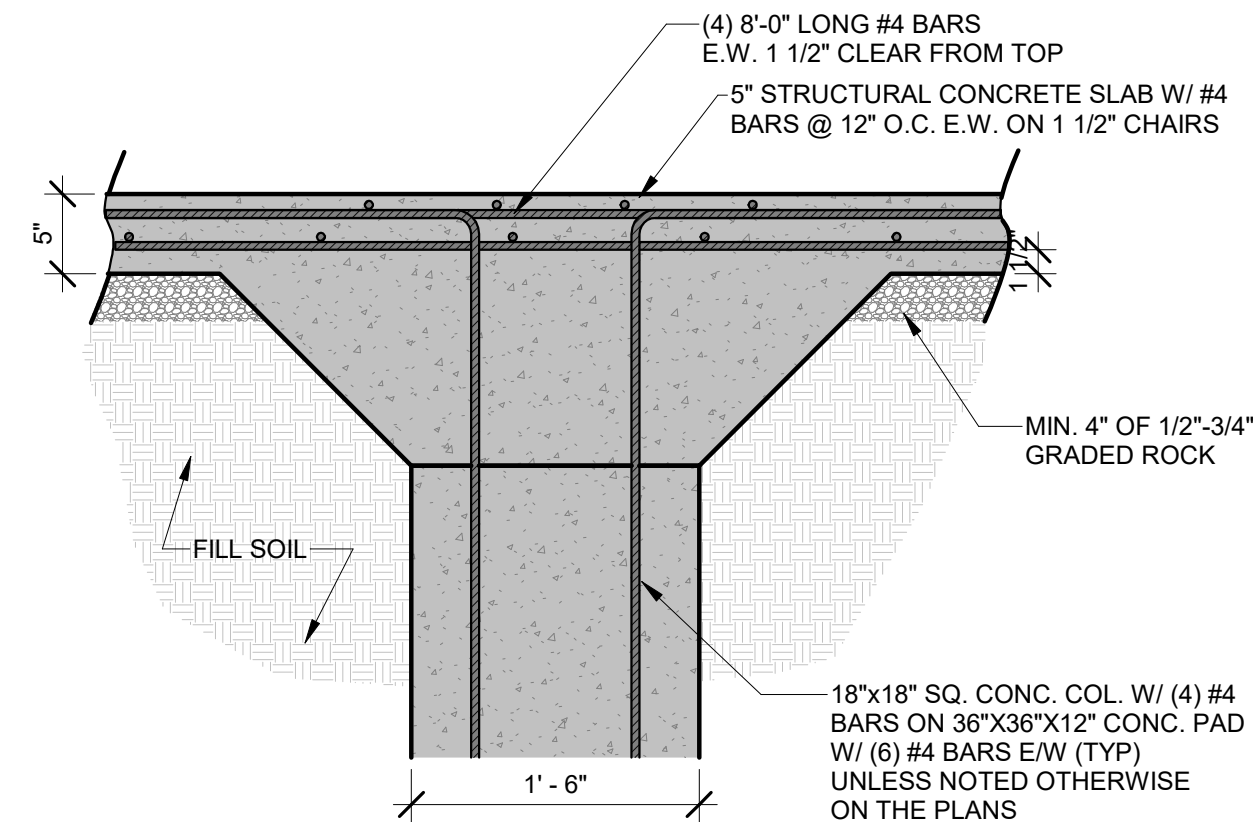
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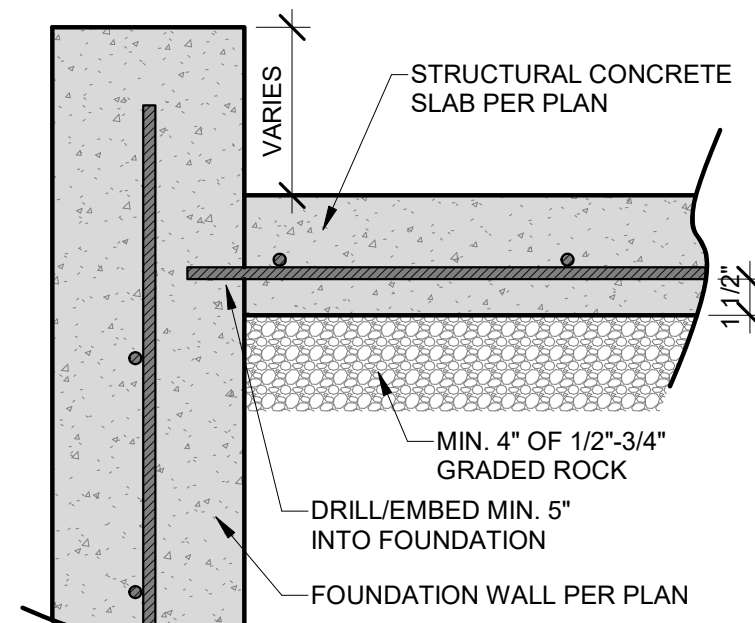
CONCRETE DETAILS

S-3.0

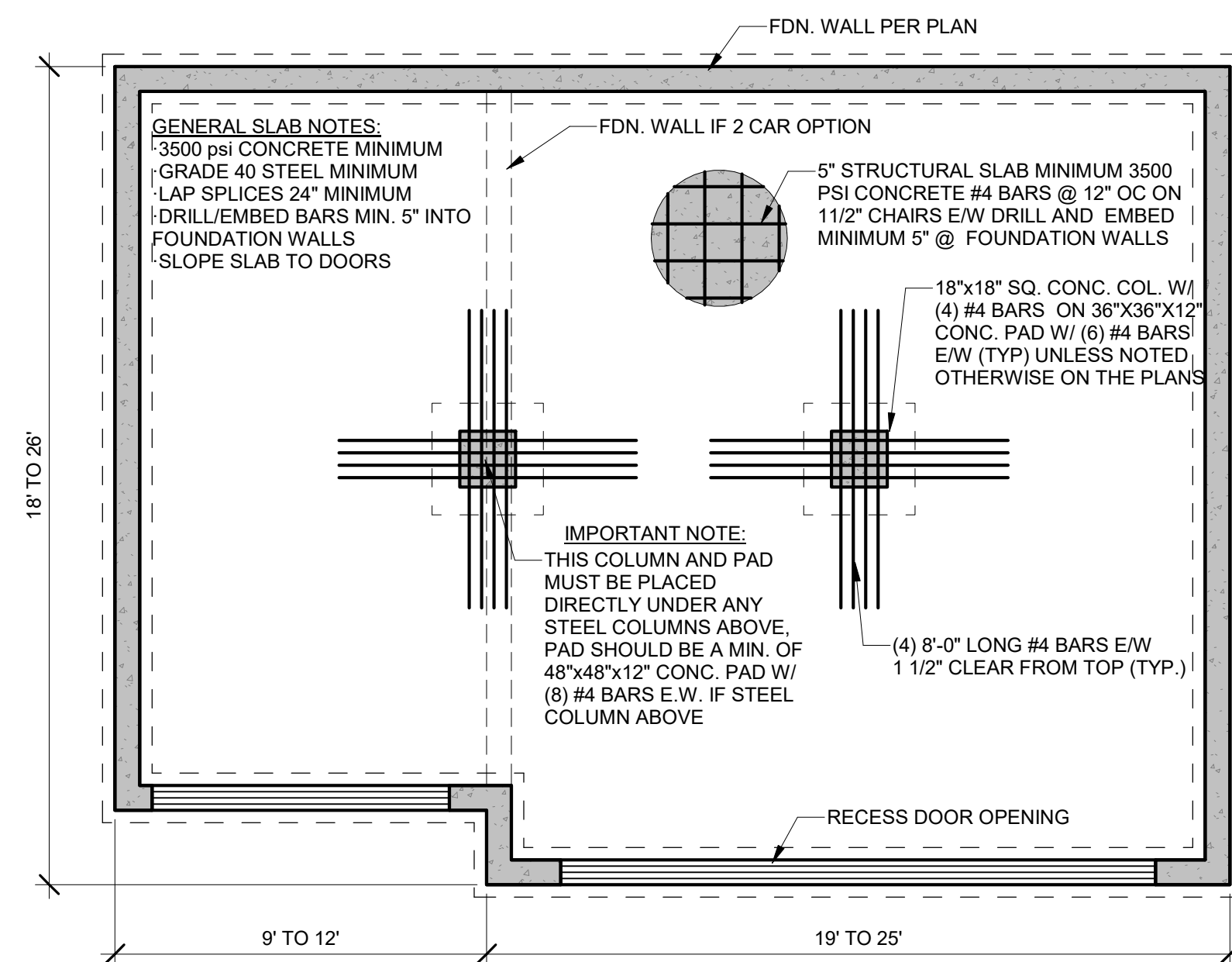
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7 GARAGE SLAB COLUMN DETAIL
1" = 1'-0"

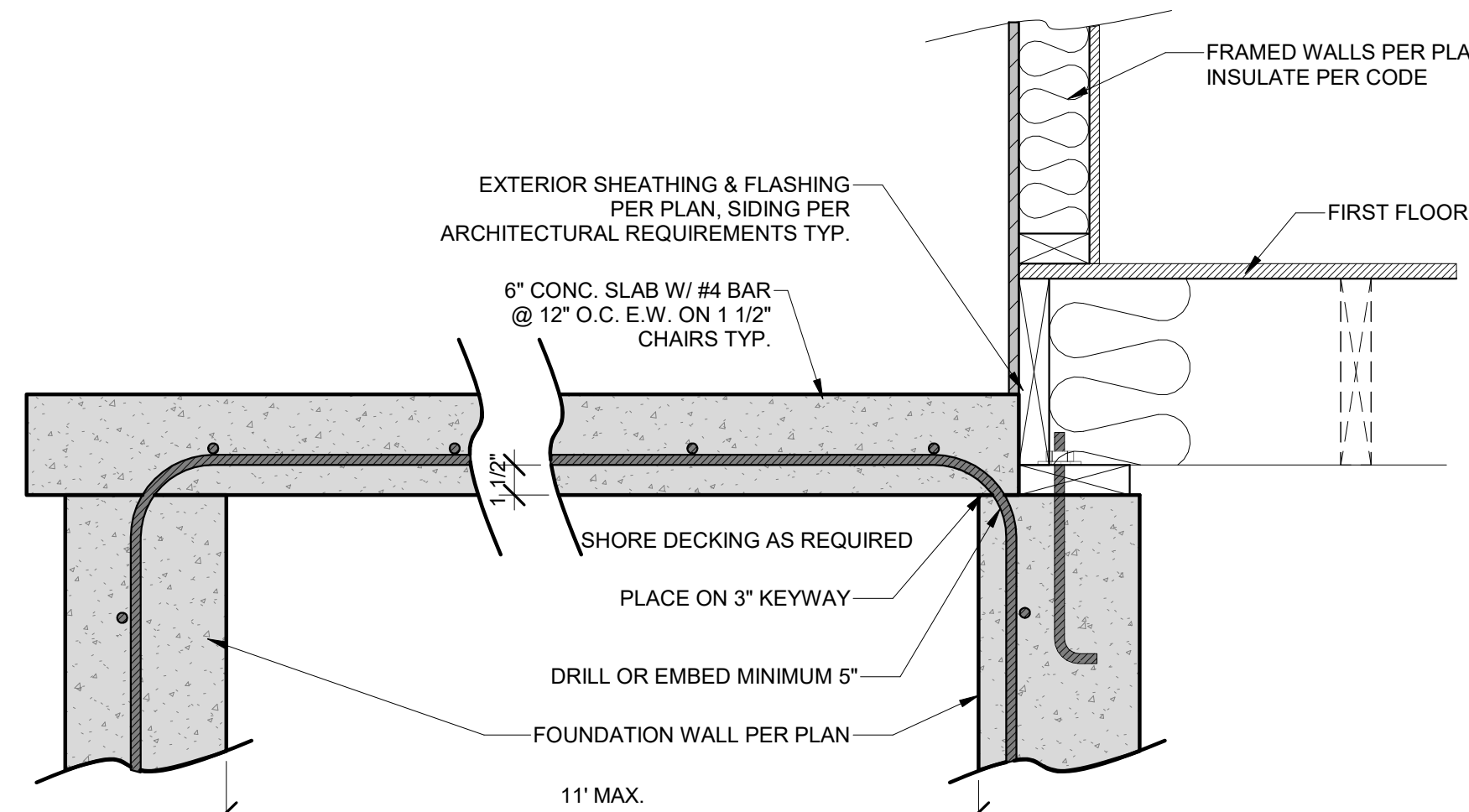


8 STRUCTURAL SLAB/ WALL
1 1/2" = 1'-0"



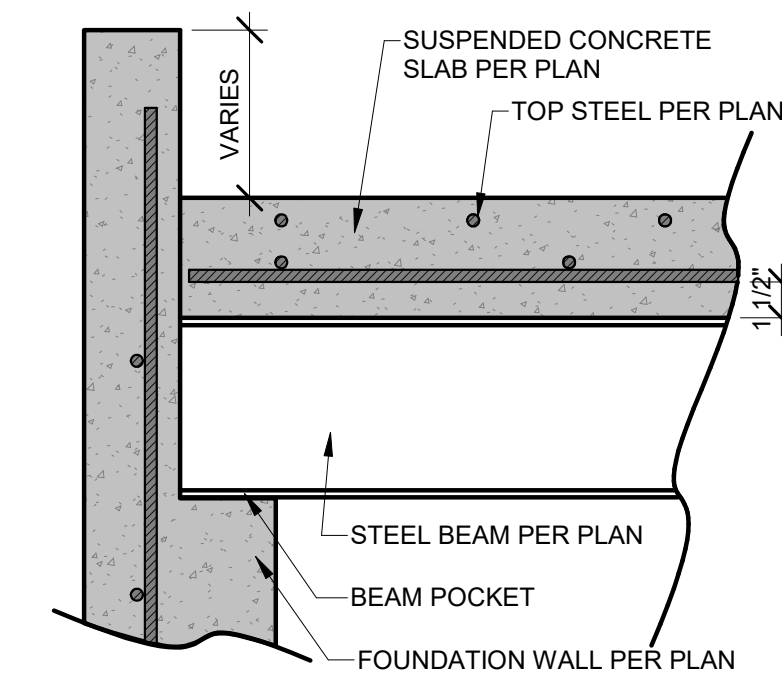
9 TYPICAL GARAGE SLAB
1/4" = 1'-0"

HD ENGINEERING STRUCTURAL GARAGE SLAB DETAILS

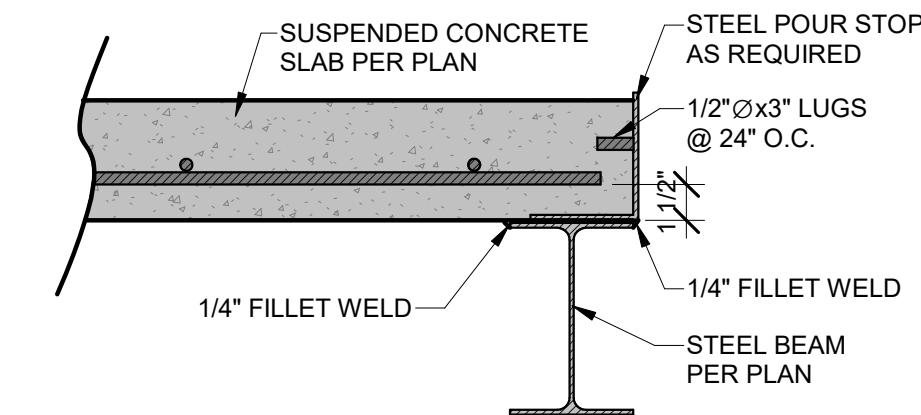


FOR SUSPENDED SLABS A MAXIMUM OF 10' ABOVE FLOOR BELOW: TEMPORARY SHORING WALLS SHALL BE PLACED AT A MAXIMUM OF 4' O.C. / #2-2X4 STUDS AT 16" O.C. W/ TOP AND BOTTOM PLATE. WALL TO HAVE CONTINUOUS DIAGONAL BRACING. LATERAL BRACING TO BE RUN FROM WALL TO WALL AT MID HEIGHT 4' ON CENTER. SHORING TO REMAIN IN PLACE FOR AT LEAST 21 DAYS.

6 SUSPENDED PORCH STOOP SLAB
1 1/2" = 1'-0"

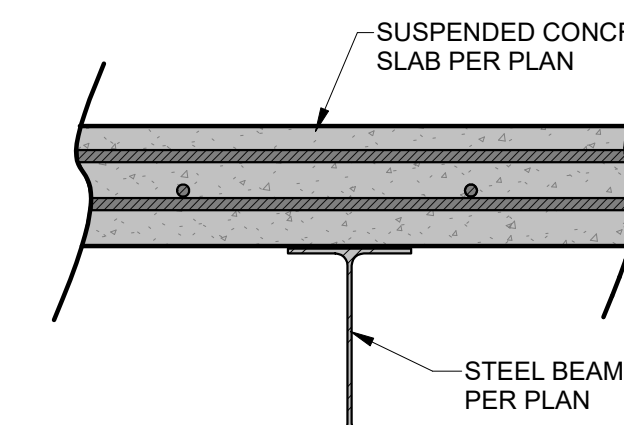


1 SUSPENDED SLAB BEAM/WALL CONNECTION
1 1/2" = 1'-0"

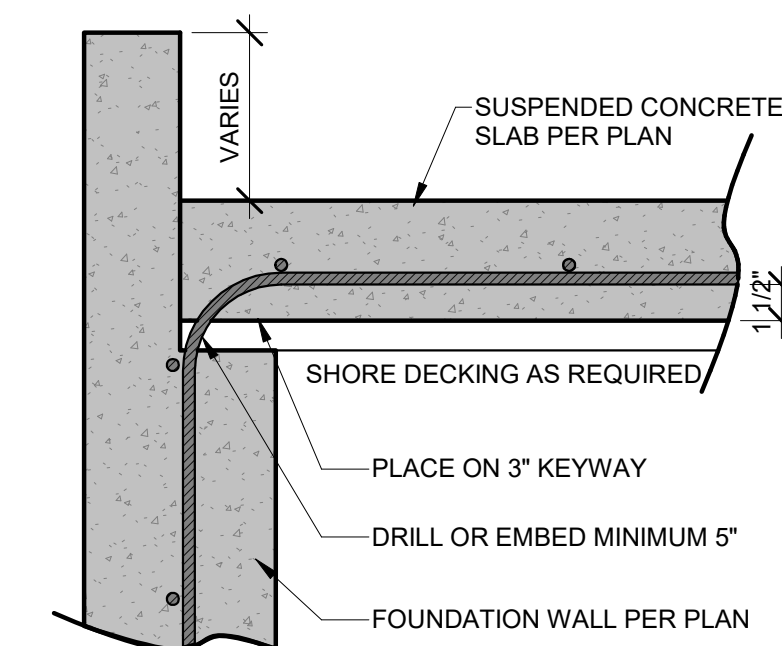


*FASTEN STEEL ANGLE TO BEAM W/ TEK SCREWS OR 2"x1/4" FILLET WELD @ 12" O.C.

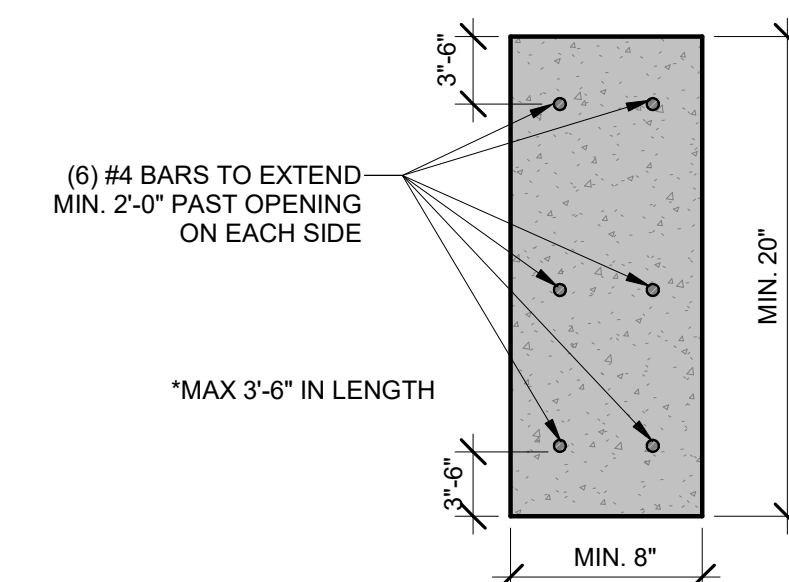
2 SUSPENDED SLAB POUR STOP
1 1/2" = 1'-0"



3 SUSPENDED SLAB/STEEL BEAM CROSS SECTION
1 1/2" = 1'-0"



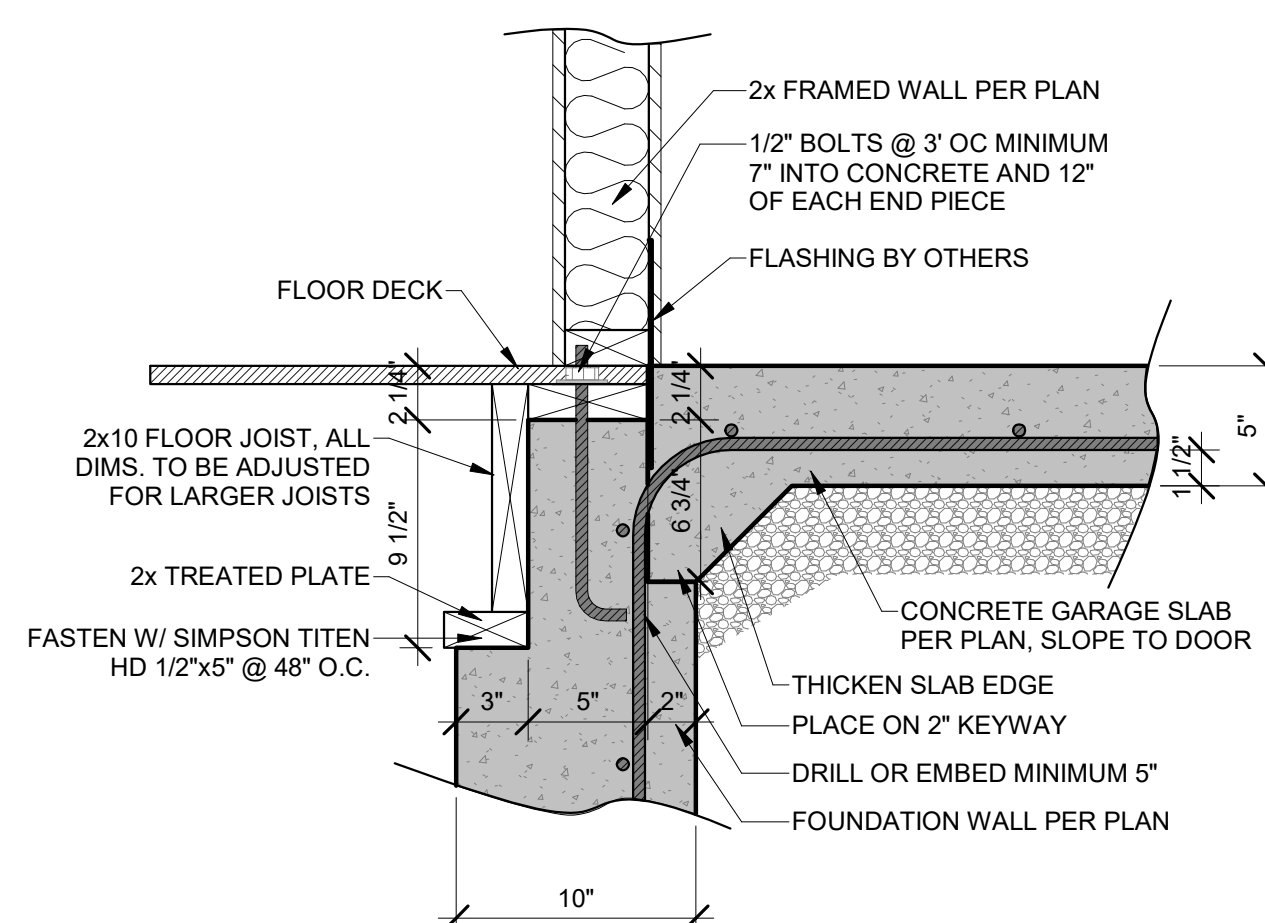
4 SUSPENDED SLAB/WALL CONNECTION
1 1/2" = 1'-0"



5 CONCRETE HEADER DETAIL
1 1/2" = 1'-0"

IMPORTANT NOTE:

FOR SUSPENDED SLABS A MAXIMUM OF 10' ABOVE FLOOR BELOW: TEMPORARY SHORING WALLS SHALL BE PLACED AT A MAXIMUM OF 4' O.C. / #2-2X4 STUDS AT 16" O.C. W/ TOP AND BOTTOM PLATE. WALL TO HAVE CONTINUOUS DIAGONAL BRACING. LATERAL BRACING TO BE RUN FROM WALL TO WALL AT MID HEIGHT 4' ON CENTER. SHORING TO REMAIN IN PLACE FOR AT LEAST 21 DAYS.
ANY CAST IN PLACE SLABS FORMED MORE THAN 10' ABOVE THE FLOOR BELOW SHALL HAVE A SITE SPECIFIC SHORING DESIGN DONE. OUR FIRM SHOULD BE CONSULTED FOR THIS DESIGN ONCE FOUNDATION WALLS ARE IN PLACE TO EVALUATE ALL FIELD CONDITIONS. IT SHOULD BE NOTED THAT FAILURE TO HAVE AN ADEQUATE SHORING DESIGN CAN RESULT IN FORM COLAPSE AND/OR CATASTROPHIC FAILURE.



10 ZERO ENTRY GARAGE DETAIL
1 1/2" = 1'-0"

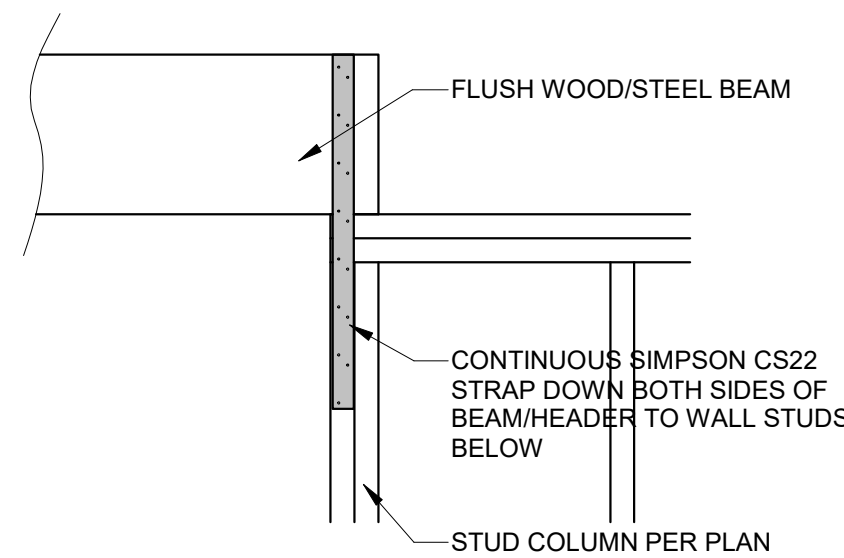
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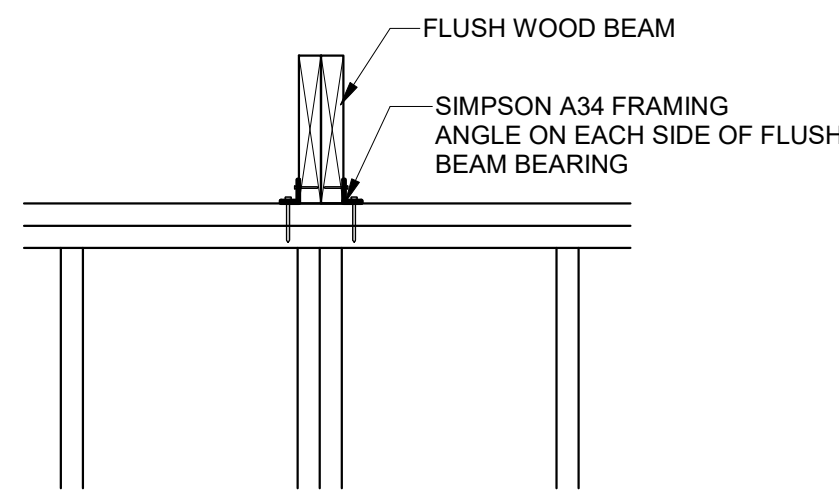
10 ZERO ENTRY SHOWER DETAIL
1/4" = 1'-0"



9 WOOD TO WOOD STACKED CONNECTION
1" = 1'-0"



8 UPSET WOOD/STEEL PARALLEL TO WALL
1" = 1'-0"



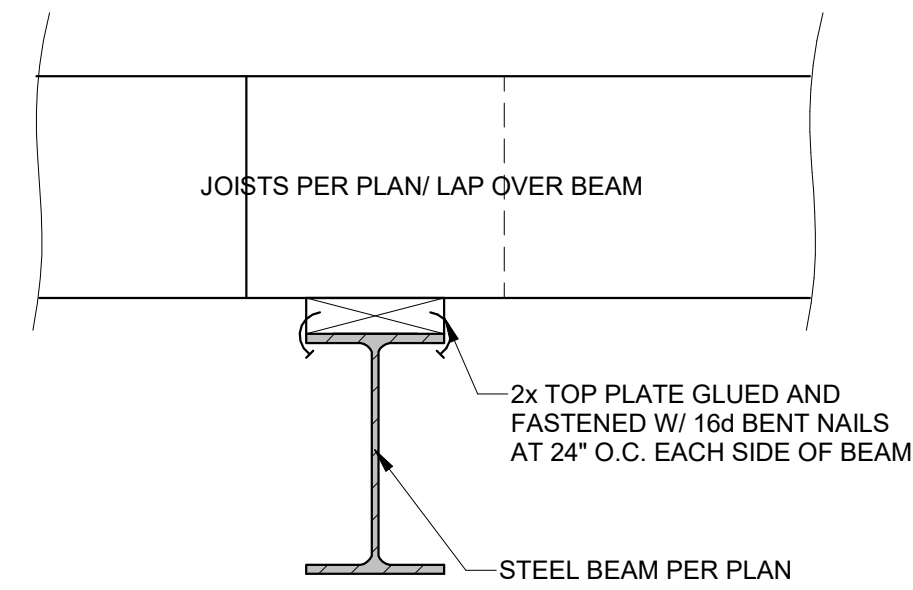
7 UPSET WOOD PERPENDICULAR TO WALL
1" = 1'-0"



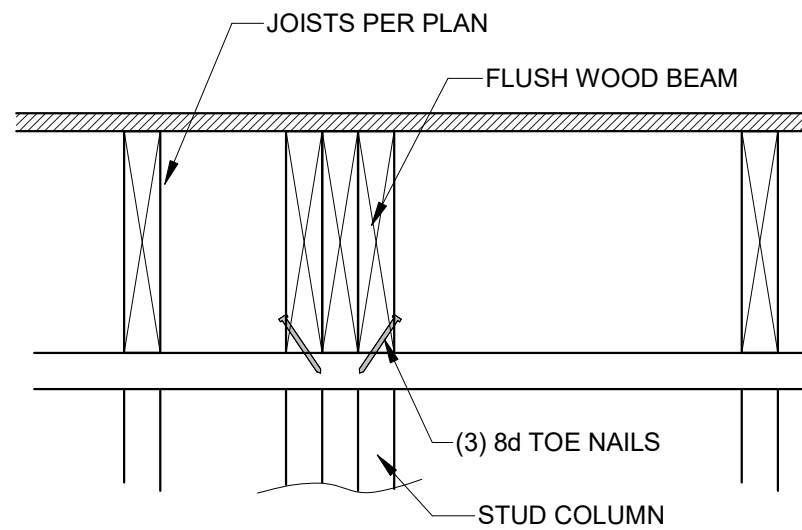
11 SHEATHING JOINT LOCATION
1" = 1'-0"



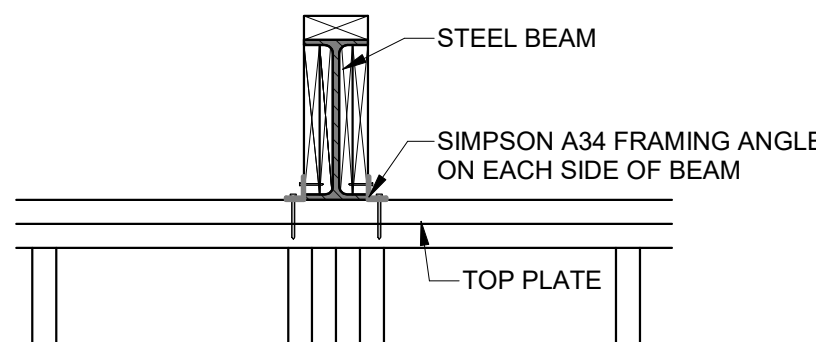
6 STEEL COLUMN TO WOOD FLOOR
1 1/2" = 1'-0"



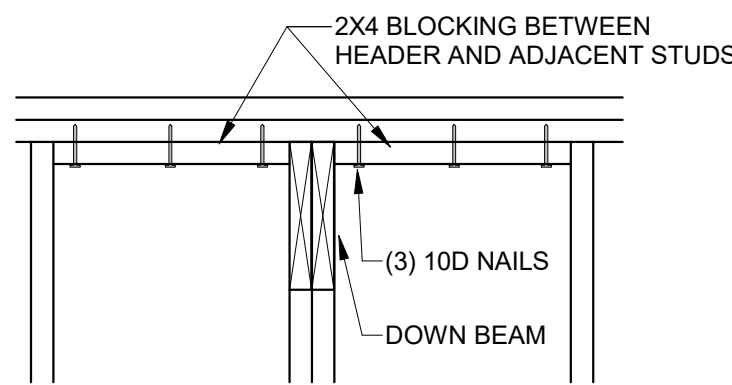
5 STEEL BEAM TO WOOD PLATE
1 1/2" = 1'-0"



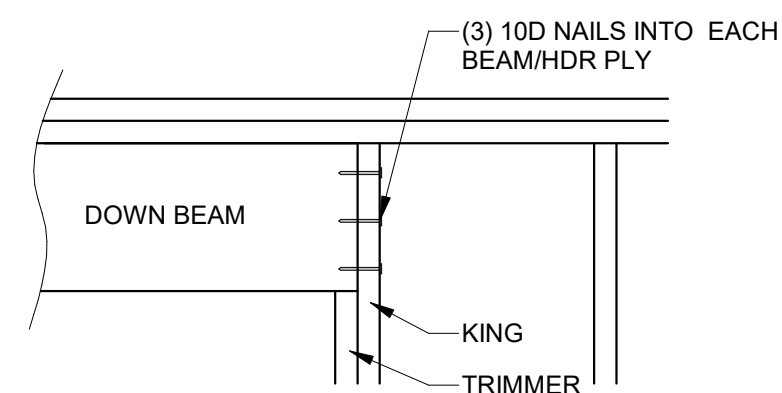
4 FLUSH WOOD BEAM CONNECTION
1 1/2" = 1'-0"



3 EXTERIOR WALL STEEL BEAM BEARING
1" = 1'-0"



2 DOWN WOOD BEAM PERPENDICULAR
1" = 1'-0"



1 DOWN WOOD BEAM PARALLEL
1" = 1'-0"

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