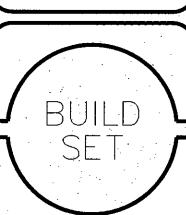


BACK ELEVATION

SCALE: 1/4" = 1'-0"

/23/21

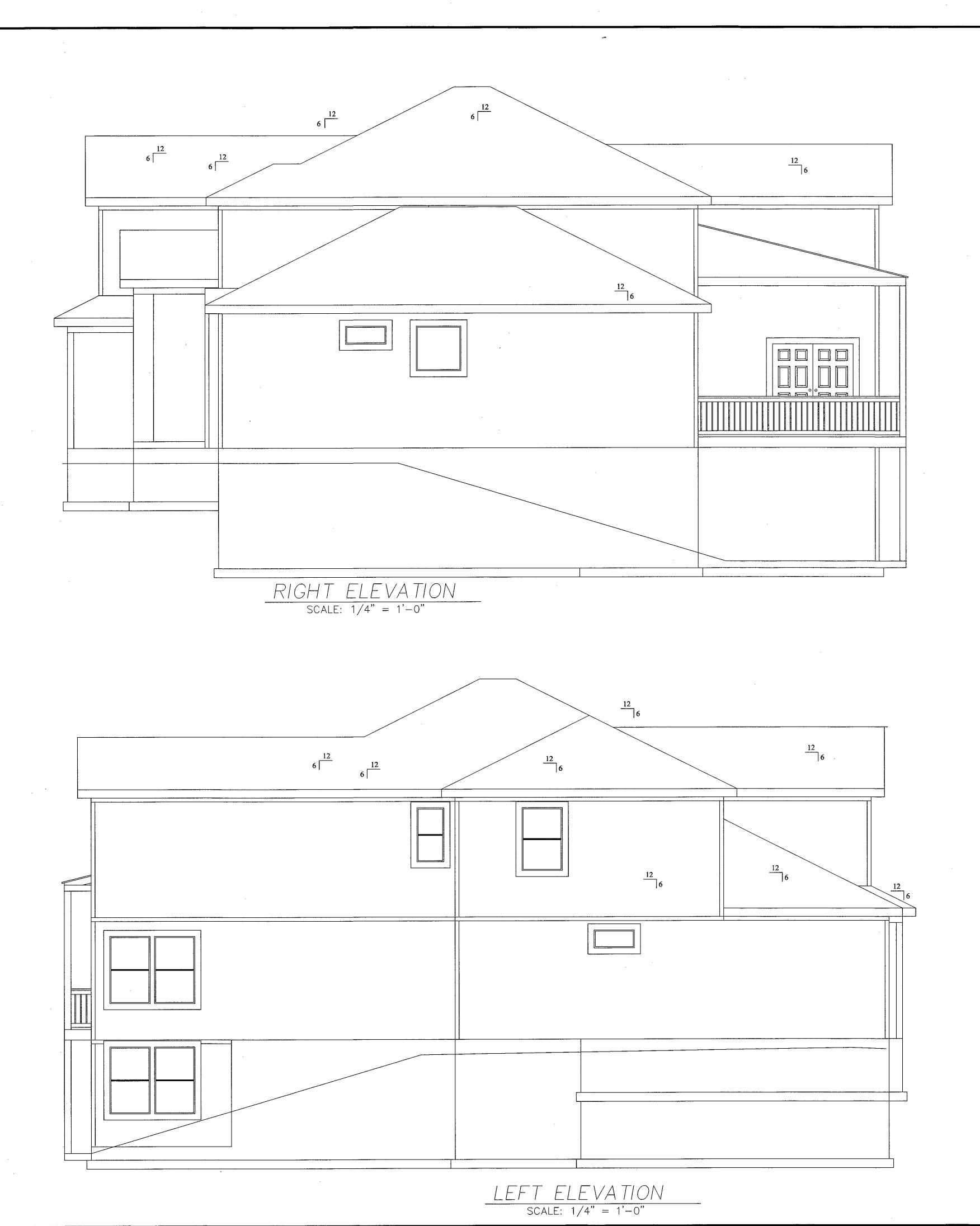
ARCHITECT IS NOT RESPONSIBLE FOR THE STRUCTURAL ELEMENTS OF THESE PLANS. A STRUCTURAL ENGINEER MAY NEED TO VERIFY ALL STRUCTURAL ASPECTS OF THESE PRINTS BEFORE CONSTRUCTION BEGINS. FIELD CONDITIONS MAY BE DIFFERENT FROM PLAN. ALL STATE AND



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8/16/21

RELEASE FOR CONSTRUCTION AS NOTED ON PLANS REVIEW DEVELOPMENT SERVICES LEE'S SUMMIT, MISSOURI 08/26/2021 4:50:43



EVSCRIPTION: RIGHT MODEL:

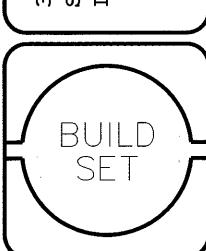
ATIONS

BIRCHWOOD

DATE: 4/23/21

ARCHITECT IS NOT RESPONSIBLE FOR THE STRUCTURAL ELEMENTS OF THESE PLANS. A STRUCTURAL ENGINEER MAY NEED TO VERIFY ALL STRUCTURAL ASPECTS OF THESE PRINTS BEFORE CONSTRUCTION BEGINS. FIELD CONDITIONS MAY BE DIFFERENT FROM PLAN. ALL STATE AND LOCAL CODES TAKE PRECIDENCE OVER THESE PLANS. CONTRACTOR WILL BE RESPONSIBLE FOR PLAN INTEGRITY AND CODE COMPLIANCE

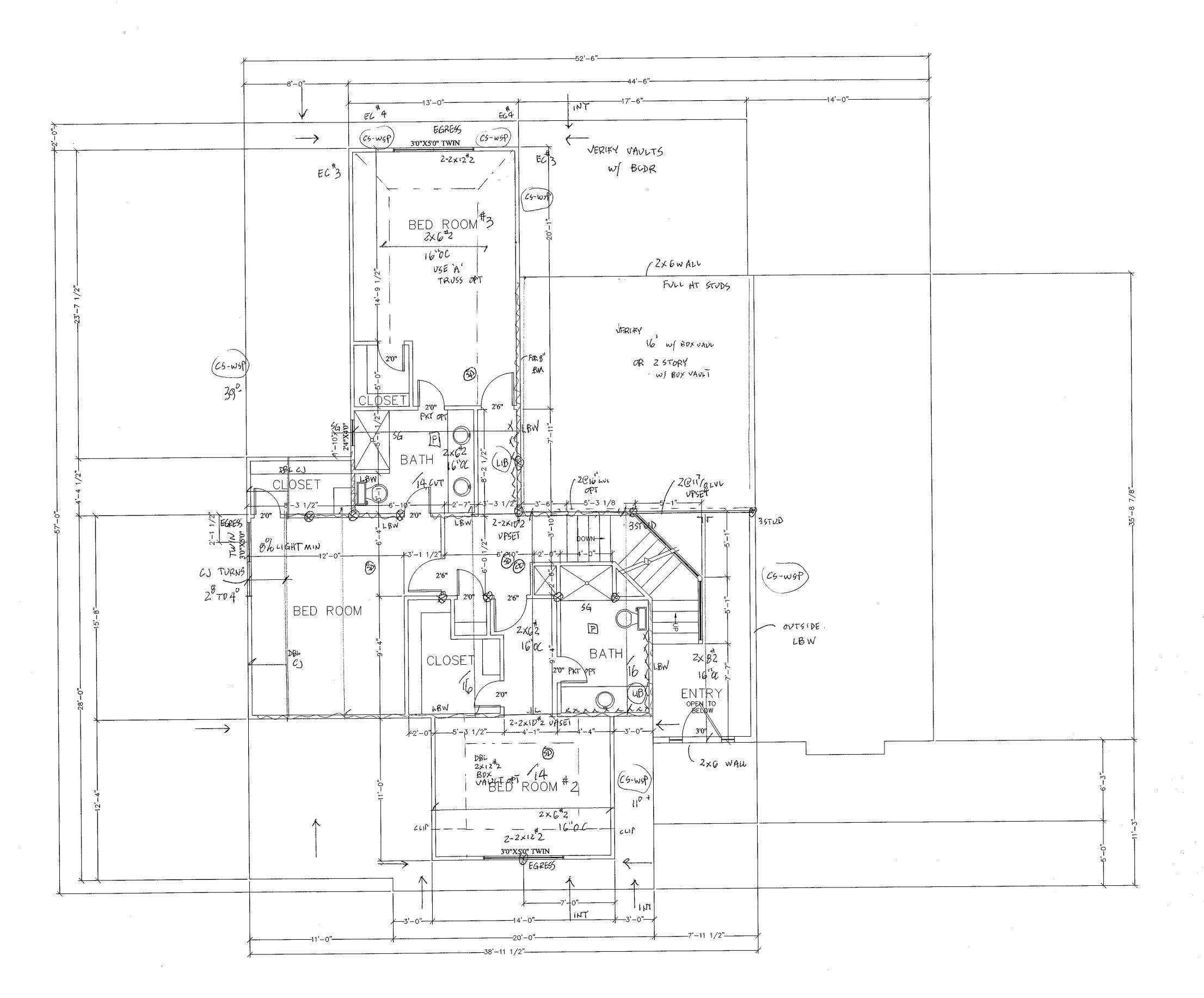
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KENNETH SIDOROWICZ

DEVELOPMENT SERVICES
LEE'S SUMMIT, MISSOURI
08/26/2021 4:50:48 2 4 A35, 1 each Har.---3'0"X5'0" 3'0"X5'0" 3'0"X5'0" 2011/gw FLOO(W5P) (5-PF) 3'0"X5'0" 3'0"X5'0" 3'0"X5'0" (6-PF) 2'4"X6'2" 2'4"X6'2" 2'4"X6'2" 2'4"X6'2" 2×6 WALL 3-2×122 2-2×12+2 MODEL: BIRCHWOOBEDROOM DATE: /23/21 GREAT ROOM VERIFY VAULTS CEILING. W/ BLDR 12-2×122 ARCHITECT IS NOT RESPONSIBLE FOR THE (WSP) 7-2×12*2 STRUCTURAL ELEMENTS OF THESE PLANS. A STRUCTURAL 3'-6 1/2"-ENGINEER MAY NEED TO OF THESE PRINTS BEFORE CONSTRUCTION BEGINS. FIELD CONDITIONS MAY BE DIFFERENT FROM PLAN. ALL STATE AND 5' 8 1/2" 4' 3 1/2" LOCAL CODES TAKE PRECIDENCE OVER
THESE PLANS, CONTRACTOR WILL BE
RESPONSIBLE FOR PLAN INTEGRITY
AND CODE COMPLIANCE 25TUD 45TUT (WSP) 2 x102 S T S/8 TYPE X GARAGE LIVING SIDE OPEN TO ABOVE BMS CEILING 3STUD-W14x22 3@91/8LVL T WELD 2-2×10 2 3'0"X5'0" 2×642 (G5-WSF) ZX102 2×62 3110 Summiree's 9'X8' O.H.D. 16' 'OC -2-2×10"2 1600 FLOOR STOOP 16'X8' O.H.D. DBL W 3@9/gLVL 2-2×10 1/2 (WSP) BUILD DF/L MIN 个2:2%,2 42 HOUSE IS SHEATHED W/ 18" OSB
APA PANELS, SMART PANEL OR
EQUAL, INSTALLED PER MANU.
SPECS, SHIP LAPPED PANELS
REQUIRE NAILING OF OVER AND
UNDER PANELS SEPARATELY. SET CLIP JOISTS (LIB) INT SHALL BE SIMPSON STRAP CS-PF) HEADER LENGTHS ARE SHOWN FOR CS-PF THIS DRAWING IS THE PROPERTY OF BILLY SPELLERBERG AND IS NOT TO BE REPRODUCED, MODIFIED, OR USED FOR ANY OTHER PROJECT, OR EXTENTION OF THIS PROJECT, SIDING LAPS RIM 2x4, 9' PLATE, FULL HT. STUDS S.C. = SELF CLOSING $\frac{FIRST \ FLOOR \ PLAN}{SCALE: 1/4" = 1'-0"}$ D2 GN #25 FOR WINDOWS THE OF MISSO EXCEPT BY AGREEMENT WITH CS = CONTINUOUSLY SHEATHED KENNETH SIDOROWICZ THIS COMPANY. EC = END CONDITION SEE D2 FOR INSULATION VALUES EC#5, 16" LONG CS16 STRAP, CENTERED ON SUBFLOOR, FILL ALL NAIL HOLES. E-19986 ``8/16/21



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 $\begin{bmatrix} -4 - of -6 \end{bmatrix}$ SHEET NO:

FLOOR FRAMING FRAMING PLAN

SECOND ROOF

MODEL:

BIRCHWOOL

DATE:

ARCHITECT IS NOT RESPONSIBLE FOR THE STRUCTURAL ELEMENTS OF THESE PLANS. A STRUCTURAL ENGINEER MAY NEED TO VERIFY ALL STRUCTURAL ASPECTS OF THESE PRINTS BEFORE CONSTRUCTION BEGINS. FIELD CONDITIONS MAY BE DIFFERENT FROM PLAN. ALL STATE AND LOCAL CODES TAKE PRECIDENCE OVER THESE PLANS. CONTRACTOR WILL BE RESPONSIBLE FOR PLAN INTEGRITY AND CODE. COMPLIANCE

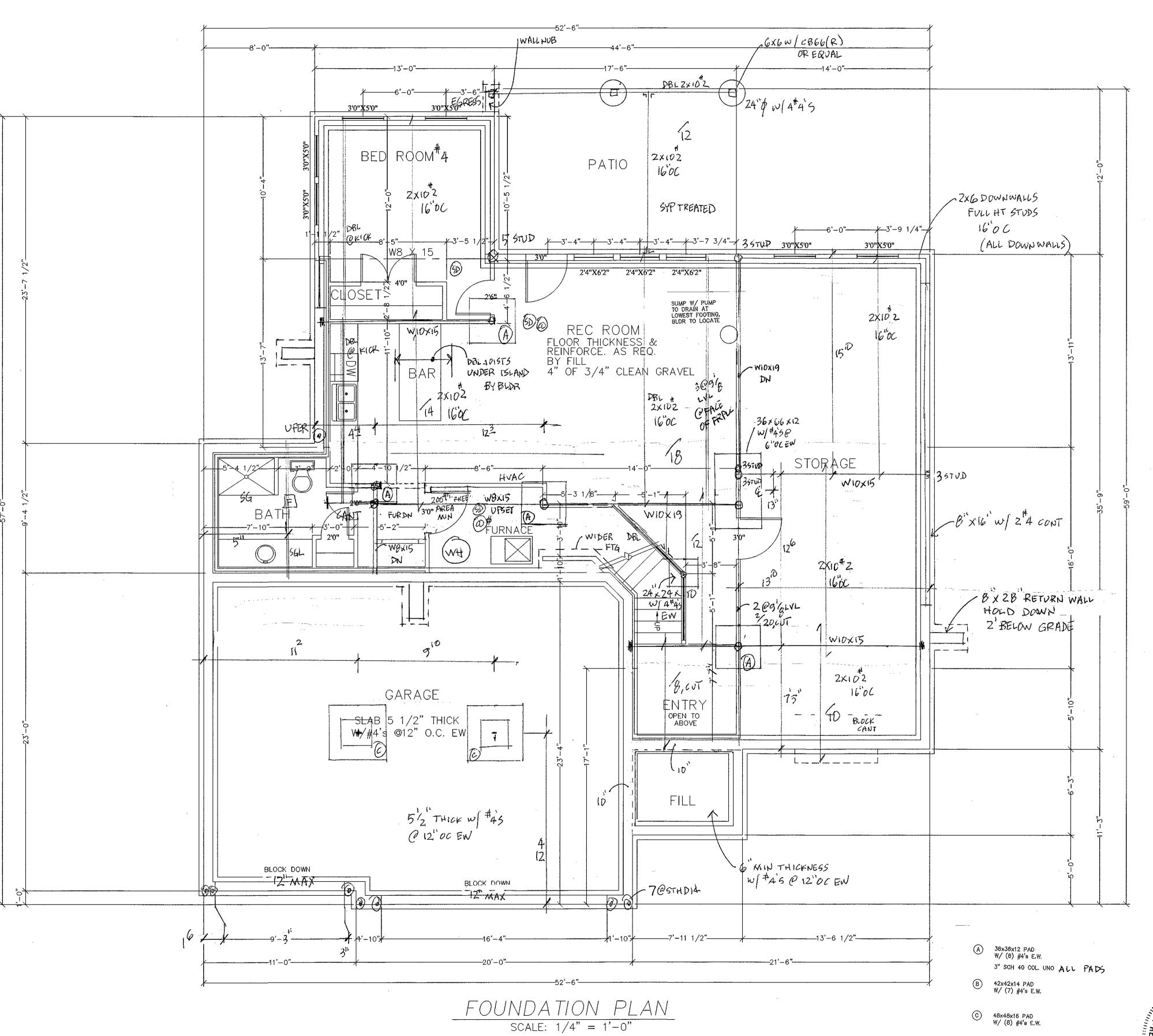
st

3110 Summi Lee's

/23/21

DESCRIPTION:

SECOND FLOOR PLAN SCALE: 1/4" = 1'-0" RELEASE FOR CONSTRUCTION
AS NOTED ON PLANS REVIEW
DEVELOPMENT SERVICES
LEE'S SUMMIT, MISSOURI
08/26/2021 4:50:43



DESCRIPTION: FOUNDATION

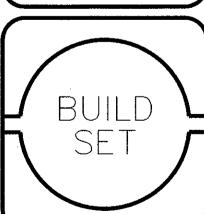
MODEL: BIRCHWOOD

> DATE: 4/23/21

ARCHITECT IS NOT RESPONSIBLE FOR THE

ARCHITECT IS NOT RESPONSIBLE FOR THE STRUCTURAL ELEMENTS OF THESE PLANS. A STRUCTURAL ENGINEER MAY NEED TO VERIFY ALL STRUCTURAL ASPECTS OF THESE PRINTS BEFORE CONSTRUCTION BEGINS. FIELD CONDITIONS MAY BE DIFFERENT FROM PLAN. ALL STATE AND LOCAL CODES TAKE PRECIDENCE OVER THESE PLANS. CONTRACTOR WILL BE RESPONSIBLE FOR PLAN INTEGRITY AND CODE COMPLIANCE

3110 Sw Blue Ribbon St. Summit View Farms Lot 64 Lee's Summit, MO



PADS

OF MISSO

KENNETH
SIDOROWICZ

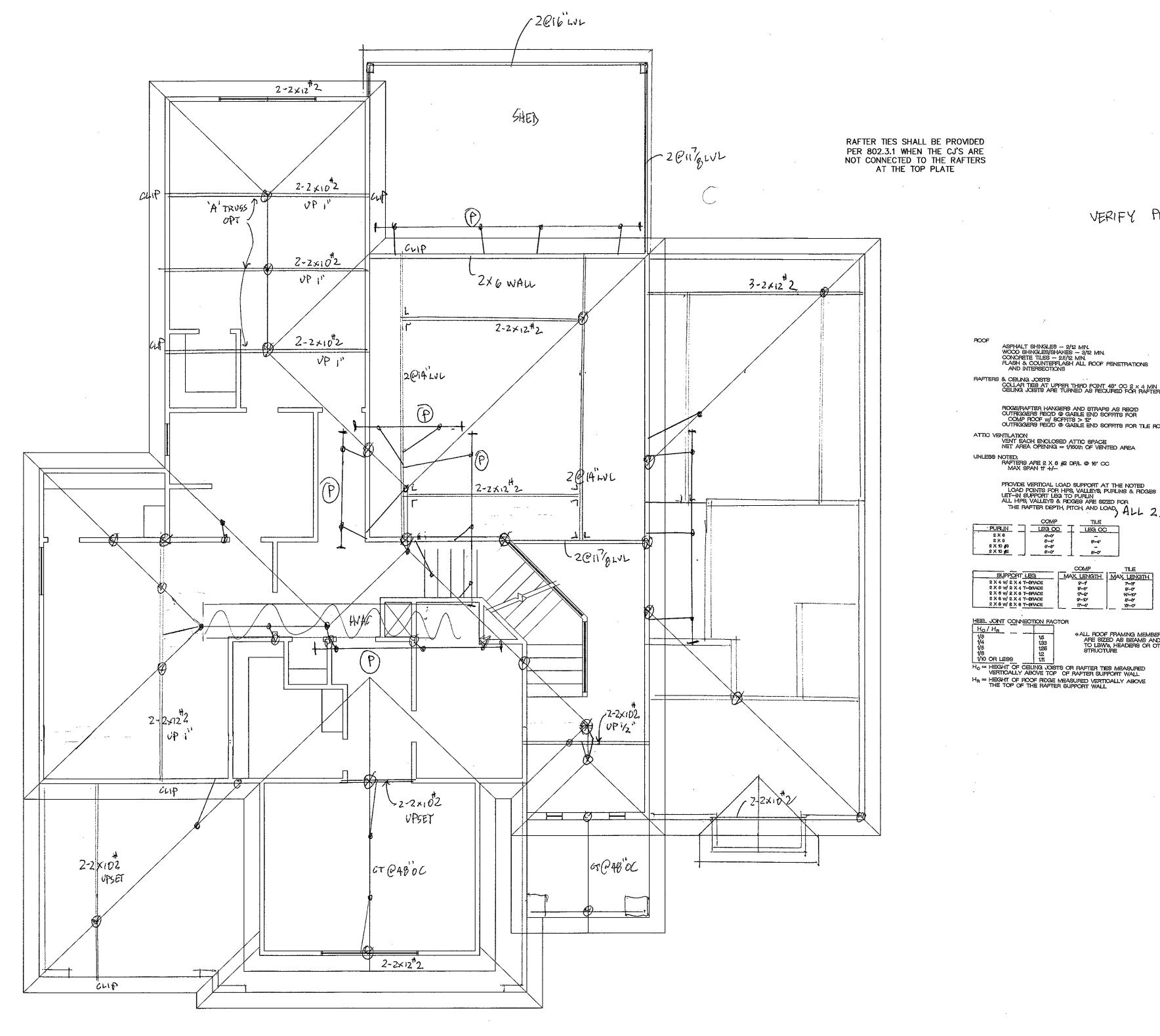
NUMBER E-19986

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5_of_6_

SHEET N



VERIFY PLATES & SPRING LINES W/ BLDR

ASPHALT SHIVOLES — 2/12 MIN.
WOOD SHINOLES/SHAVES — 3/12 MIN.
CONCRETE TILES — 25/12 MIN.
FLASH & COUNTERFLASH ALL FOOF PENETRATIONS
AND INTERSECTIONS RAFTERS & CEILING JOSTS COLLAR TES AT UPPER THIRD POINT 48° OC 2 x 4 MIN CEILING JOSTS ARE TURNED AS RECURED FOR RAFTER TIES

PROCE/RAFTER HANGERS AND STRAPS AS RECTO CUTFICACERS RECTO @ GABLE END SOFFITS FOR COMP ROOF W/ SOFFITS > 12' CUTFICACERS RECTO @ GABLE END SOFFITS FOR TILE ROOF

UNLESS NOTED; RAFTERS ARE 2 X 6 #2 DF/L @ 16' OC MAX SPAN 11' +/-

PROVIDE VERTICAL LOAD SUPPORT AT THE NOTED

LOAD POINTS FOR HPS, VALLEYS PURLINS & RIDGES
LET-IN SUPPORT LEG TO PURLIN
ALL HPS, VALLEYS & RIDGES ARE SIZED FOR
THE RAFTER DEPTH, RTICH, AND LOAD, ALL 2×8 UND

COMP TLE

SUPPORT LEG MAX LENGTH MAX LENGTH

2 X 4 W 2 X 4 T-BRACE 9-6' 8-6'
2 X 6 W 2 X 6 T-BRACE 17-2' 14-0'
2 X 6 W 2 X 4 T-BRACE 9-6' 8-6'
2 X 6 W 2 X 6 T-BRACE 17-2' 14-0'
2 X 6 W 2 X 6 T-BRACE 9-6' 8-6'
2 X 8 W 2 X 8 T-BRACE 17-4' 15-0'

*ALL ROOF FRAMING MEMBERS ARE SIZED AS BEAMS AND BRACED TO LBW'S, HEADERS OR OTHER STRUCTURE

BIRCHWOOD $\sqrt{23/21}$

MODEL:

FRAMING

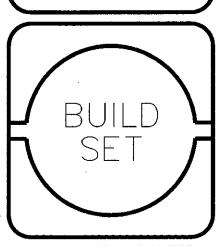
FLOOR

FIRS

SCRIPTION

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> S T SW Blue nit View F s Summit, 3110 Summi



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KEMMETH SIDOROVICZ NUMBER E-19986 8/16/21

ROOF FRAMING PLAN
SCALE: 1/4" = 1'-0"

<u>DIVISION 1</u> — GENERAL REQUIREMENTS

1. DESIGN AND CONSTRUCTION WORK FOR THIS PROJECT SHALL CONFORM TO THE REQUIREMENTS OF THE 2018 IRC.

2. FURNISH ALL LABOR, MATERIALS, AND EQUIPMENT NECESSARY TO COMPLETE THE WORK AS SHOWN OR INFERRED BY THE DRAWINGS.

3. DESIGN FACTORS: A) GROUND SNOW LOAD (INCLUDING DRIFTING SNOW)__20 PSF B) WIND SPEED (EXPOSURE B)_

C) SEISMIC CATEGORY (A), GROUND ACCELERATION = NA

4. DESIGN LOADS (PSF, UNLESS NOTED OTHERWISE): A) ROOF (LL/DL)_ SEE TABLE B) FLOOR (LL/DL) _SEE TABLE _SEE TABLE, (0/10 TRUSSES) C) CEILING (LL/DL)_

5. DO NOT SCALE DRAWINGS. IF DIMENSIONS ARE IN QUESTION, OBTAIN CLARIFICATION FROM A / E BEFORE CONTINUING

6. THE CONTRACTOR SHALL EXAMINE ACTUAL JOB CONDITIONS AND BE RESPONSIBLE FOR VERIFYING ALL DIMENSIONS AND ELEVATIONS SHOWN ON THE PLANS. IF ERRORS, OMISSIONS, OR DISCREPANCIES ARE FOUND THEY SHALL BE REPORTED TO THE DESIGN PROFESSIONAL BEFORE PROCEEDING WITH THE WORK.

DIMENSIONS FOR NEW CONSTRUCTION ARE TO FACE OF FINISH OR COLUMNS AND FACE OF CONCRETE, WOOD, OR MASONRY WALLS UNLESS OTHERWISE INDICATED. DIMENSIONS INDICATE NOMINAL DIMENSIONS RATHER THAN ACTUAL DIMENSIONS.

8. CONTRACTOR IS RESPONSIBLE FOR COORDINATING ALL TRADES EVEN IF THE TRADE IS UNDER A SEPARATE CONTRACT

9. PROVIDE SUFFICIENT STUDS AND BLOCKING WHERE REQUIRED TO SUPPORT EQUIPMENT AND/OR MISCELLANEOUS ITEMS, I.E., LOAD POINTS, TYPICAL CASEWORK, CABINETS, GRAB BARS ETC.

10. PRETREAT FOUNDATION FOR TERMITES AS REQUIRED.

11. GARAGE DOORS AND FRAMES SHALL BE DESIGNED AND INSTALLED TO MEET THE 115 MPH WIND LOAD RESISTANCE REQUIREMENTS OF DASMA 108 AND ASTM E 330.

12. ALL EXTERIOR DOORS, INCLUDING THE DOOR LEADING FROM THE GARAGE TO THE DWELLING UNIT, SHALL INCORPORATE THE PHYSICAL SECURITY PROVISIONS OF THE JURISDICTION IN WHICH THE CONSTRUCTION TAKES PLACE.

<u>DIVISION 2</u> — EARTHWORK

1. ALL PROPERTY MARKERS SHALL BE EXPOSED.

2. ALL FOOTINGS ARE DESIGNED TO BEAR ON NATURAL UNDISTURBED SOIL CAPABLE OF ADEQUATELY SUSTAINING A MINIMUM BEARING PRESSURE OF 1,500 PSF. IF SUITABLE UNDISTURBED BEARING CAPACITY IS NOT ENCOUNTERED AT THE ELEVATION INDICATED ON THE DRAWINGS, CONTRACTOR SHALL NOTIFY THE ENGINEER OF RECORD IMMEDIATELY

3. ALL TOPSOIL, ORGANIC MATERIAL, AND EXISTING STRUCTURES SHALL BE REMOVED FROM BUILDING AREA AND FROM AREAS TO BE PAVED. STOCKPILE ALL TOPSOIL FOR REUSE.

4. REFERENCE THE SOILS REPORT FOR ALL FILL CONDITIONS.

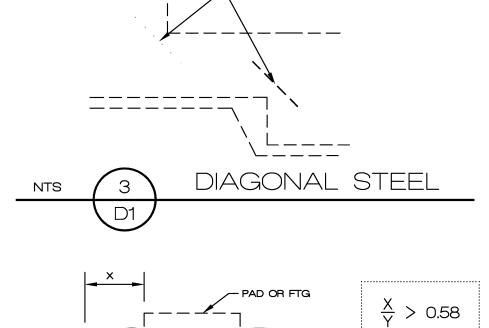
5. OVEREXCAVATE BUILDING AREA BELOW SLAB SUBGRADE ELEVATION AND REPLACE WITH MATERIAL PER SOILS REPORT,

6. SITE EROSION CONTROL SHALL COMPLY WITH ALL STATE AND LOCAL ORDINANCES. 7. IN-SITU SOIL CONDITIONS, SEE SOILS REPORT OR 1,500 PSF BEARING

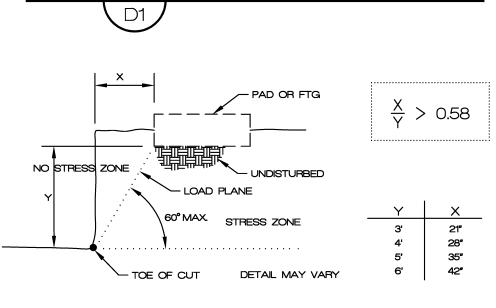
& 60 PCF EQUIVALENT FLUID WEIGHT. 8. SOIL CONDITIONS AT THE DEPTH OF EXCAVATION FOR THE FOOTING SHALL BE UNIFORM AND CONSISTENT. NOTIFY THE

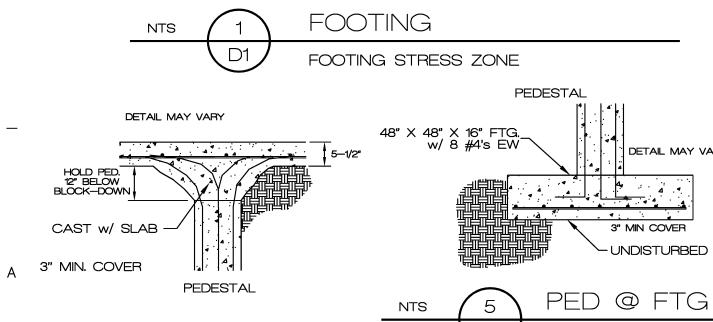
9. CONTRACTOR SHALL BE RESPONSIBLE FOR REMOVING AND DISPOSING OF ANY EXCESS EXCAVATION MATERIALS AND FOR OBTAINING AND SUPPLYING ADDITIONAL FILL MATERIAL AS

ENGINEER OF RECORD OF ANY INCONSISTENCIES.



48" LONG MIN.





SLAB @ PEI

SLAB ON FILL

<u>DIVISION 3</u> — CONCRETE

1. ALL CONCRETE WORK SHALL CONFORM TO THE REQUIREMENTS OF ACI 318 "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE" AND ACI 332 "REQUIREMENTS FOR RESIDENTIAL CONCRETE CONSTRUCTION."

CONCRETE MATERIALS SHALL COMPLY WITH: BLOCK STRENGTH A) CEMENT - ASTM C 150 TYPE 1 MORTAR STRENGTH B) AGGREGATE - ASTM C 33, MAXIMUM AGGREGATE SIZE 3/4" GROUT STRENGTH_ C) WATER - POTABLE, WATER/CEMENT RATIO .5 (MAX.)

SUPERPLASTICIZERS. F) FLY ASH - ASTM C 618, CLASS C 3. CONCRETE SHALL DEVELOP THE FOLLOWING MINIMUM 28 DAY DESIGN COMPRESSIVE STRENGTH (f'c): TYPE OF CONSTRUCTION COMP. STRENGTH (f'c)

SFF TABLE

SEE TABLE

A) FOOTINGS, WALLS, AND SLABS B) EXTERIOR SLABS AND CURBS (AIR-ENTRAINED CONCRETE)

D) AIR-ENTRAINING ADMIXTURE - ASTM C 260

E) WATER-REDUCING ADMIXTURE - ASTM C 494, INCLUDING

CONCRETE PROPORTIONS SHALL BE ESTABLISHED ON THE BASIS OF FIELD EXPERIENCE AND/OR TRIAL MIXTURES IN ACCORDANCE WITH ACI 318-89 SECTIONS 5.2 AND 5.3. WHEN FLY ASH IS UTILIZED IN THE MIX, MIX SHALL CONTAIN A WATER-REDUCER. FLY ASH SHALL BE ADDED AT THE RATE OF NOT MORE THAN 100 POUNDS PER CUBIC YARD AND CEMENT SHALL BE REDUCED BY NOT MORE THAN 15 PERCENT BY WEIGHT.

4. PROPORTION AND DESIGN MIXES TO RESULT IN CONCRETE SLUMP AT A POINT OF PLACEMENT OF NOT MORE THAN 4" TO 5".

5. USE AIR-ENTRAINING ADMIXTURES IN EXTERIOR EXPOSED CONCRETE TO RESULT IN CONCRETE AT POINT OF PLACEMENT HAVING AIR CONTENT OF 5 TO 7 PERCENT ENTRAINED AIR.

6. ALL PLUMBING AND ELECTRICAL ROUGH-INS MUST BE COMPLETE, INSPECTED AND APPROVED BEFORE REQUESTING THE SLAB

CONCRETE WORK EXECUTION:

A) MINIMUM CONCRETE COVER FOR REINFORCING SHALL BE, UNLESS NOTED OTHERWISE ON DRAWINGS: CAST AGAINST AND EXPOSED TO EARTH__ EXPOSED TO EARTH OR WEATHER_

NOT EXPOSED TO EARTH OR WEATHER_ B) IN CORNERS OF GRADE BEAMS PROVIDE CORNER REINFORCEMENT LAP TWO FEET EACH DIRECTION IN OUTSIDE FACE, MATCHING SIZE AND SPACING OF HORIZONTAL REINFORCEMENT.

C) PROVIDE CONTROL JOINTS IN SLABS-ON-GRADE AT NOT GREATER THAN 20 FEET ON CENTER IN EACH DIRECTION. SAW CUT CONTROL JOINTS MINIMUM 1/4 OF THE SLAB DEPTH, AS SOON AFTER SLAB FINISHING AS POSSIBLE WITHOUT DISLODGING AGGREGATE. (DO NOT SAW CUT STRUCTURAL SLABS w/o

BATCH TICKETS SHALL BE SUBMITTED TO A CONTRACTORS REPRESENTATIVE PRIOR TO OFF LOADING. ANY CONCRETE MORE THAN 45 MINUTES OUT PRIOR TO STARTING PLACEMENT SHALL BE

THE MAXIMUM ADDITION OF WATER SHALL BE LIMITED TO 1 GALLON PER YARD; NOTE THAT THIS ADDITION SHALL BE USED TO CONTROL HEAT ONLY (NOT SLUMP).

10. PUMPS SHALL NOT BE PRIMED IN FORMS.

TURN DOWN SLAB @

DETAIL MAY VARY

HVAC BLOCK DOWN

11. REINFORCEMENT:

A) ALL REINFORCING BARS SHALL BE A615, GR40 MIN. LAP SPLICES 18" MIN FOR #4 BAR. SEE TABLE

B) WELDED WIRE FABRIC SHALL BE ASTM A185, LAP AT LEAST ONE FULL MESH AND LACE SPLICES WITH WIRE.

C) REBAR SHALL BE CLEAN, AND FREE FROM RUST AND OIL PRIOR TO THE PLACEMENT OF CONCRETE. REBAR SHALL BE TIED AND SECURED AS REQUIRED TO PREVENT DISPLACEMENT IN THE FORMS. D) TIE STEEL TO PREVENT DISPLACEMENT. HOOK AND TIE STEEL AS POSSIBLE, TIES, CHAIRS, OR OTHER PRODUCTS SHALL BE

PROTECTED WHEN LOCATED NEAR EXPOSED SURFACES E) STEEL SHALL BE STORED ON SITE ABOVE GRADE, AND COVERED AS REQUIRED FOR PROTECTION FROM RAIN AND OTHER POSSIBLE

12. ADJUST FOUNDATION FOR SITE AND SOIL CONDITIONS AND VERIFY

HVAC TRUNK

12" ADDITIONAL

SLAB @ HVAC

DETAIL MAY VARY

JOIST HNGR -

FLOOR

JOIST

6" MIN. STEM WALL-

1-1/2" LEDGE

BLOCK DOWN @ HVAC

DBL PLATE FOR GYP CRETE

- CONCRETE SLAB

/__ 1--1/2" COVER MIN.

2.5" INSULATION,

- 1—1/2" LEDGE MIN.

ICF WALL

FLUSH FRAMING @ FDN

<u>DIVISION 4</u> - MASONRY

COMPRESSIVE STRENGTH OF CONCRETE MASONRY CONSTRUCTION (CMU) SHALL BE AS FOLLOWS (PSI). MASONRY STRENGTH NOT SPECIFICALLY NOTED ON PLAN SHALL BE (f'm) 1500 PSI.

MASONRY STRENGTH (F'm DESIGN)_ 1800

2. CONCRETE BLOCK SHALL BE HOLLOW LOAD-BEARING CONCRETE MASONRY UNITS CONFORMING TO ASTM C 90, TYPE N-II. ALL BLOCKS SHALL BE PLACED IN RUNNING BOND CONSTRUCTION (UNLESS OTHERWISE NOTED) WITH ALL VERTICAL CELLS IN ALIGNMENT.

3. MORTAR MIX SHALL CONFORM TO THE REQUIREMENTS OF ASTM C 270, TYPE M OR S. TYPE M MORTAR SHALL BE USED WHERE MASONRY IS IN CONTACT WITH SOIL.

4. GROUT SHALL CONFORM TO THE REQUIREMENTS OF ASTM C 476. USE SUFFICIENT WATER FOR GROUT TO FLOW INTO ALL JOINTS OF THE MASONRY WITHOUT SEGREGATION. ALL CELLS IN CONCRETE BLOCKS CONTAINING REINFORCING SHALL BE FILLED SOLID WITH GROUT. ALL MASONRY BELOW FINISHED FLOOR OR GRADE SHALL BE GROUTED SOLID. HOLD GROUT DOWN 1-3" BELOW TOP OF BLOCK AT GROUT LIFT JOINTS AND AT CONCRETE PLACED OVER

5. MINIMUM LINTEL, WHERE NOT ON PLANS, SHALL HAVE A MINIMUM OF 2 — #5's CONTINOUS HORIZONTAL BARS IN BOTTOM OF BOND BEAM OR LINTEL BLOCK AND SHALL BE GROUTED SOLID TO A MIN. DEPTH OF 24." ALL LINTEL REINFORCING AND GROUT SHALL EXTEND 2' MINIMUM PAST JAMBS UNLESS NOTED OTHERWISE ON PLANS OR DETAILS.

6. LAP REINFORCING 48 BAR DIAMETERS. STAGGER LAP SPLICES A MINIMUM OF ONE LAP LENGTH.

7. MASONRY VENEER SHALL BE ATTACHED TO SUPPORT WALL FRAMING WITH %" DIAMETER WALL TIES OR DOVETAIL-TYPE METAL TIES OF EQUIVALENT STIFFNESS EMBEDDED INTO HORIZONTAL MORTAR JOINTS. MAXIMUM VERTICAL SPACING OF TIES SHALL BE 16." MAXIMUM HORIZONTAL SPACING SHALL BE 24." TIES IN ALTERNATE COURSES SHALL BE STAGGERED. PROVIDE #9 WIRE REINFORCING IN HORIZONTAL MORTAR JOINTS AT 16" OC. ENGAGE #9 WIRE WITH WALL ANCHOR TIES. CONSTRUCTION JOINTS IN MASONRY VENEER WALLS SHALL BE LOCATED PER THE DRAWINGS.

8. WATERPROOFING, DRAINAGE PLANE, AND INSTALLATION PER ADOPTED BUILDING CODE.

<u>DIVISION 5.5</u> — MISC. STRUCTURAL STEEL

1. ALL MISCELLANEOUS STRUCTURAL STEEL WORK SHALL CONFORM TO THE REQUIREMENTS OF AISC "SPECIFICATIONS FOR DESIGN, FABRICATION, AND ERECTION OF STRUCTURAL STEEL FOR

MISCELLANEOUS STRUCTURAL STEEL MATERIAL SHALL COMPLY A) STRUCTURAL STEEL - ASTM A992

B) STEEL PIPE COLUMNS - ASTM A53 GRADE B(Sch 40 TYP) C) ANCHOR BOLTS - ASTM A307 GRADE A, NON-HEADED TYPE UNLESS OTHERWISE NOTED.

3. FLITCH PLATES SHALL HAVE 1" DIA. BOLTS @ 16" OC, STAGGERED TOP AND BOTTOM BETWEEN JOIST LAYOUT.

RETURN WALLS RETURN SPACING ABOVE FLOOR (HOLD DOWN 24" BELOW GRADE) LESS THAN 4' RETURN WALLS NOT REQ'D

16'-4" ON CENTER (MAX.), AND WITHIN

8' OF STEP DOWN OR AS SHOWN

LSIMPSON MAS

ALTERNATIVE

OPT. MUDSILL ANCHORAGE

ALTERNATIVE TO J—BOLTS

INSTALLATION

-2 X LEDGE

SLAB @ WALI

SLAB ON FILL CONCRETE OR CMU

REQ'D STRENGTH

3,000 psi

3,500 psi

3,500 psi

7 SACK MIX

CONC STRENGTH

- DRILL & SEAL AS REQ'D

- DOWELS @ 12" OC

* RETURN WALLS ALLOW FOR BACKFILL W/O FLOOR DECK IN PLACE FOR 60 PCF EQUIVALENT FLUID WEIGHT SOIL. NO HEAVY EQUIPMENT OR SURCHARE LOADING.

DETAIL MAY VARY

5-1/2° MIN.

SUS-SLAB

24" LAP, MIN

1-1/2" COVER

>4' TO 9'

2 X 4 OR 2 X 6

<u>DIVISION 6</u> - ROUGH CARPENTRY

1. ALL ROUGH CARPENTRY WORK SHALL CONFORM TO THE REQUIREMENTS OF NFPA "NATIONAL DESIGN SPECIFICATION OF WOOD CONSTRUCTION", TPI "DESIGN SPECIFICATIONS FOR LIGHT METAL PLATE CONNECTED WOOD TRUSSES", APA "PLYWOOD DESIGN SPECIFICATIONS", DOC PS 1 "PRODUCT STANDARD FOR CONSTRUCTION AND INDUSTRIAL PLYWOOD", DOC PS 56 "STRUCTURAL GLUED" LAMINATED TIMBER", AND APPLICABLE SECTIONS OF THE INTERNATIONAL BUILDING CODE.

2. ROUGH CARPENTRY MATERIALS SHALL COMPLY WITH: A) LUMBER - S4S, S-DRY, KD, OR S-GRN GRADE MARKED, COMPLYING WITH PS 20, GRADED UNDER WWPA OR SPIB RULES: STUDS: STUD GRADE HEADER: #2 DOUGLAS FIR MIN TYPICAL #2 DOUGLAS FIR RAFTER: PLATES: #2 DOUGLAS FIR BLOCKING: #2 DOUGLAS FIR

B) METAL FRAMING FASTENERS - ASTM A 153, HOT-DIP GALVANIZED FASTENERS; EQUAL TO SIMPSON STRONG-TIE CONNECTORS COMPLYING WITH APPLICABLE ICC-ES REPORTS C) PLYWOOD - APA RATED SHEATHING, COMPLYING TO PS 1. D) LVL - LAMINATED VENEER LUMBER SHALL BE GRADE 2800

F-2.0E AND SHALL MEET THE REQUIREMENTS OF APPLICABLE ICC-ES REPORTS. E) GLULAM BEAMS - COMBINATION 24F-V3 IN ACCORDANCE WITH AITC A190.1

3. EXTERIOR WALL AND ROOF SHEATHING SHALL BE 话 APA RATED SHEATHING 24/0 EXTERIOR GLUED (MIN) FOR 16" OC STUD SPACING. NAIL SHEATHING TO SUPPORT MEMBERS WITH 8D COMMON NAILS AT 6" ON CENTER ALONG EDGE SUPPORTS AND 12" ON CENTER ALONG FIELD SUPPORTS UNLESS NOTED OTHERWIDE. PROVIDE SOLID BLOCKING AT ALL UNSUPPORTED PANEL EDGES; 4/8 GUN NAILS.

NOTE: ROOF SHEATHING SHALL BE \{ \}" APA RATED SHEATHING FOR TILE ROOF, OR AS REQUIRED BY MANUFACTURER.

4. INTERIOR SHEAR WALL SHEATHING WHERE NOTED SHALL BE 表" APA RATED SHEATHING 24/0 EXTERIOR GLUED (MIN) FOR 16" OC STUD SPACING. NAIL SHEATHING TO SUPPORT MEMBERS WITH 8D COMMON NAILS AT 4" ON CENTER ALONG EDGE SUPPORTS AND 6" ON CENTER ALONG FIELD SUPPORTS UNLESS NOTED OTHERWISE, PROVIDE SOLID BLOCKING AT ALL UNSUPPORTED PANEL EDGES.

5. ATTACH METAL FRAMING FASTENERS TO FRAMING MEMBERS WITH MINIMUM NUMBER AND SIZE OF NAILS LISTED IN THE APPLICABLE ICC-ES REPORTS.

6. WOOD TRUSS SYSTEM; TRUSS JOIST SYSTEM AND GLULAM SYSTEM FOR ROOFS: A) DESIGN, FABRICATE, AND ERECT IN ACCORDANCE WITH BCSI

STANDARDS AND NDS SPECIFICATIONS. B) DESIGN LOADS: 25 PSF SNOW LIVE LOAD 10 PSF DEAD LOAD TOP CHORD (20 TILE) 10 PSF DEAD LOAD BOTTOM CHORD

C) SUBMIT SHOP DRAWINGS, INCLUDING DESIGN CALCULATIONS,

MATERIAL STRESSES, GRADE AND SPECIES OF WOOD, AND

PLACEMENT DRAWING. 7. DEFAULT HEADER SIZE NOT SPECIFIED SPANNING 8'-0" MAX SHALL BE $2-2 \times 10$ #2, WITH 2 STUD SUPPORT.

8. ALL HEADERS OVER 4'-0" SHALL HAVE DOUBLE TRIMMER @ EACH SUPPORT, OR AS SPECIFIED, UNO.

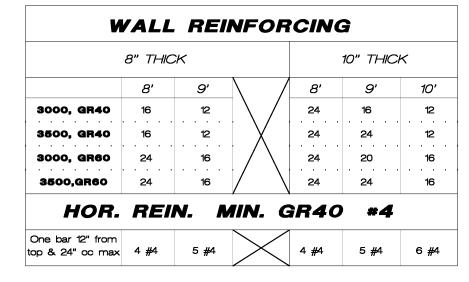
9. SOLID BLOCKING BETWEEN JOISTS @ 36" OC FOR JOISTS PARALLEL TO THE EXTERIOR FOUNDATION WALL, MIN. 48" OR 3 JOIST SPACES.

10. ALL FLUSH FRAMING @ HEADERS OR GIRDERS SHALL BE HANGERED.

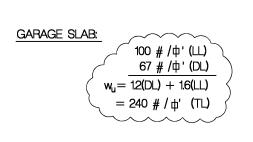
11. BLOCK BETWEEN JOISTS @ SUPPORTS OR OVER BEAMS. 12. RATED CONSTRUCTION FOR PROJECTIONS INTO SETBACKS AS REQ'D.

13. DOUBLE JOIST BELOW PARALLEL NONBEARING WALLS ON LAYOUT, SINGLE JOIST OFF LAYOUT. STRUCTURE BELOW LOAD-BEARING WALLS AS NOTED ON PLANS.

FOUNDATION PER JOCOBO RESIDENTIAL **FOUNDATION GUIDELINE**



BASEMENT SLAB:



— 27,206 #—in φM_N= *φA * f_s(d − ,a/2)

∴ Use #4 @ 12" OC EW

12'-6" (+/-) MODULE

 $\phi M_N = *\phi A * f(d - a/2)$ = 0.9(0.2)(40000)(4-0.22/2)= 0.9(0.2)(40000)(4-0.22/2)= 28,008 # in > 25,951 (OKAY)= 28,008 # in > 27,206 (OKAY)

> ∴ Use #4 @ 12" OC EW 15'-6" (+/-) MODULE

40 # /中' (LL)

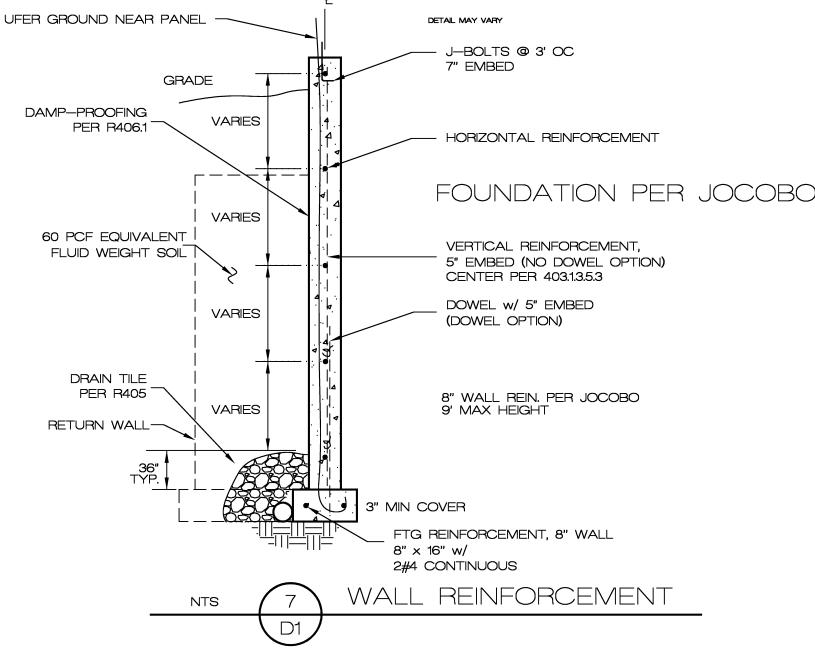
67 # /中' (DL)

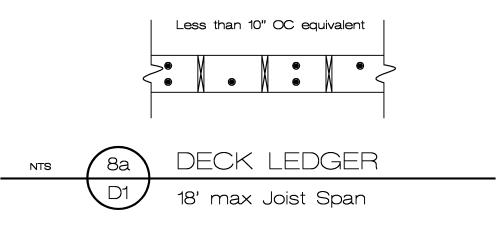
40,000 * 0.2

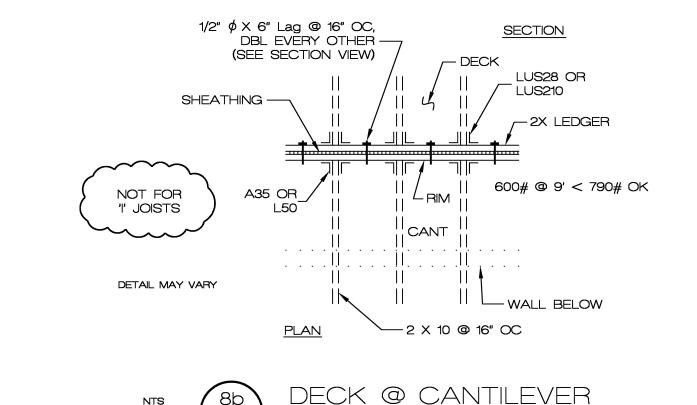
w = 1.2(DL) + 1.6(LL)

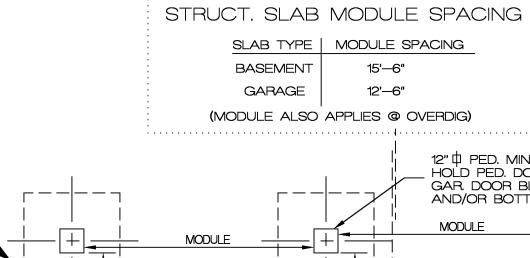
= 144 # / d (TL)

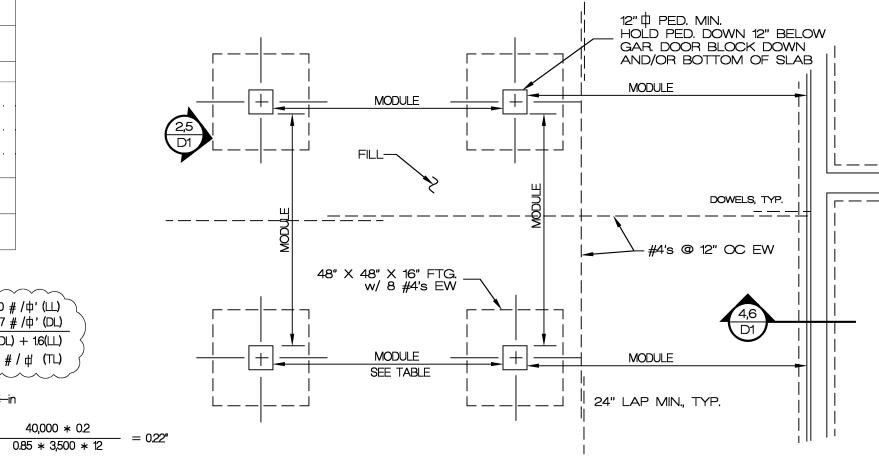
– – – 25,951 #-in











TRUCTURAL SLAB ON FILL DO NOT SAW CUT STRUCTURAL SLABS W/O APPROVAL

VERIFY ALL STRUCTURAL SLAB DETAILS W/ ENGINEER

DO NOT ISOLATE COLUMNS FROM STRUCTURAL SLABS

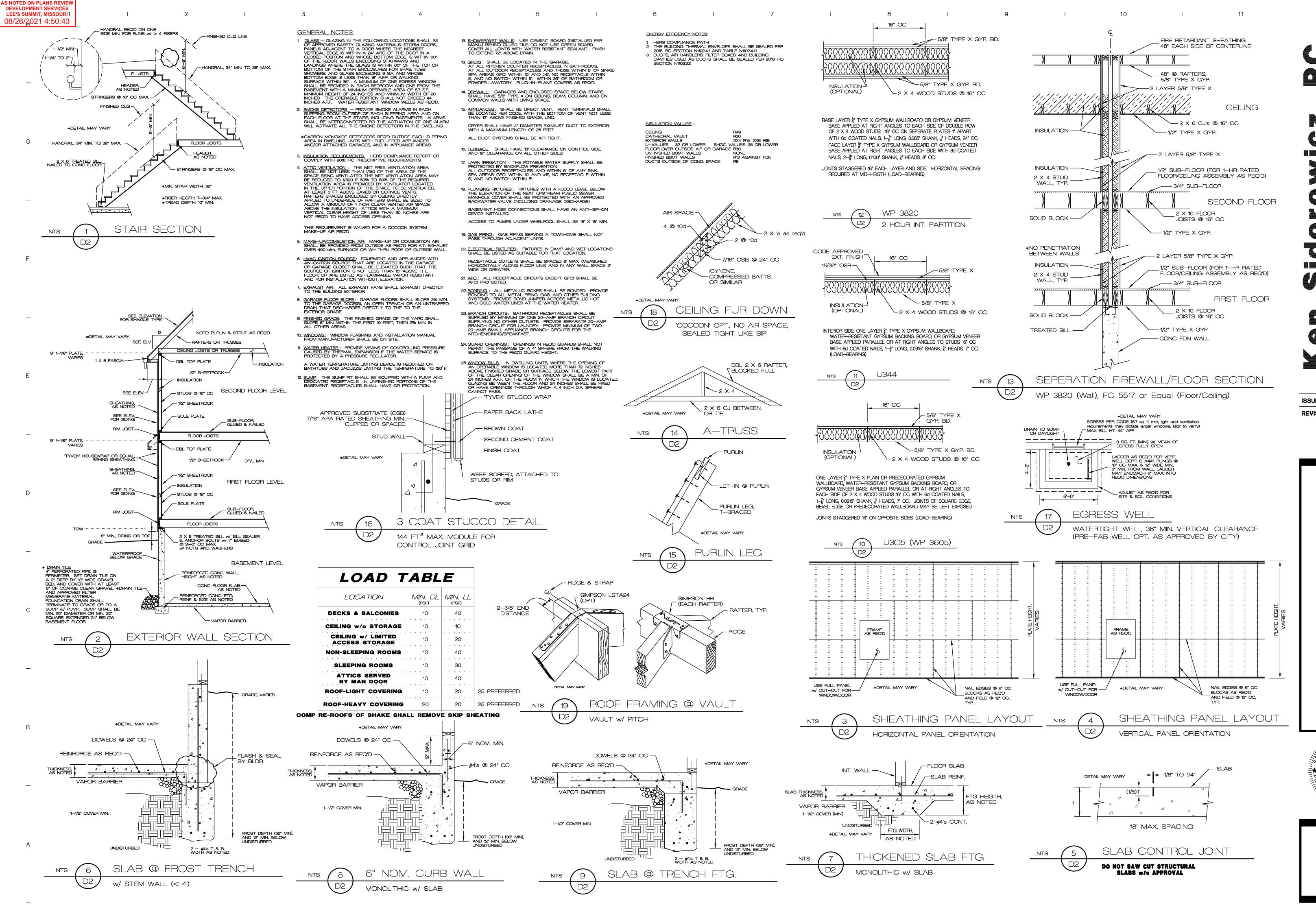
KENNETH 2 SIDOROWICZ NUMBER E-19986

ISSUE DATE

REVISIONS

11/2/15

`8/16/21



RELEASE FOR CONSTRUCTION

ISSUE DATE
REVISIONS

KENNETH SIDOROWICZ NUMBER E-19986

8/16/21

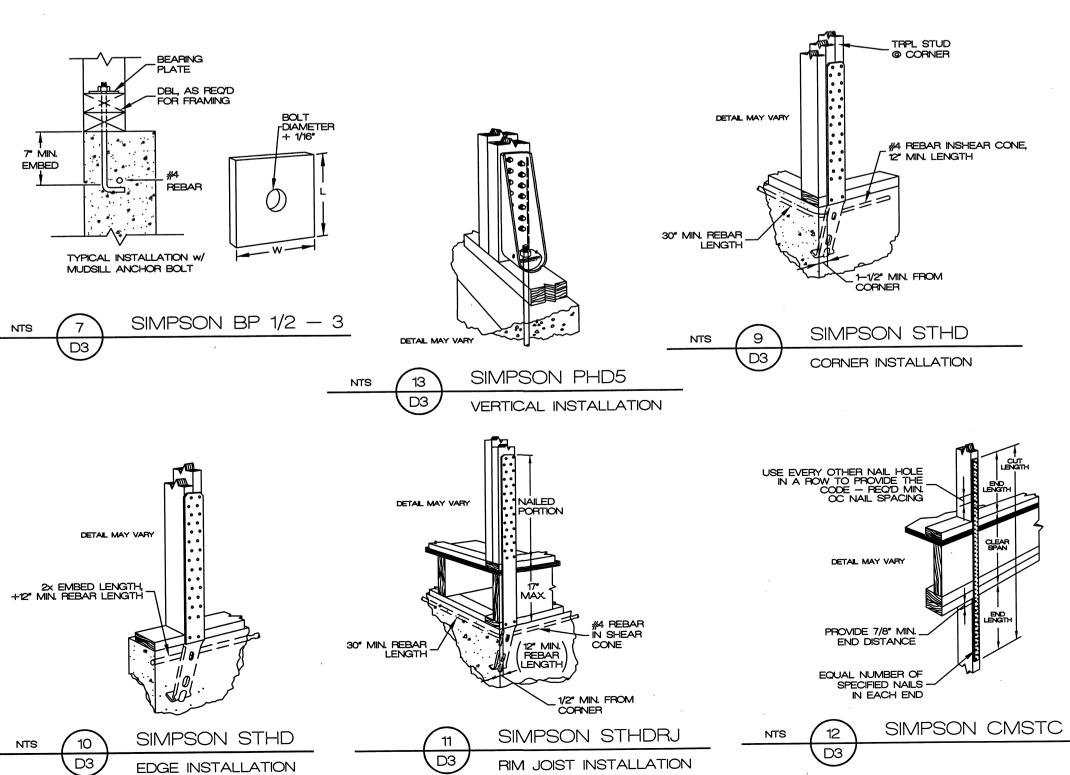
D2

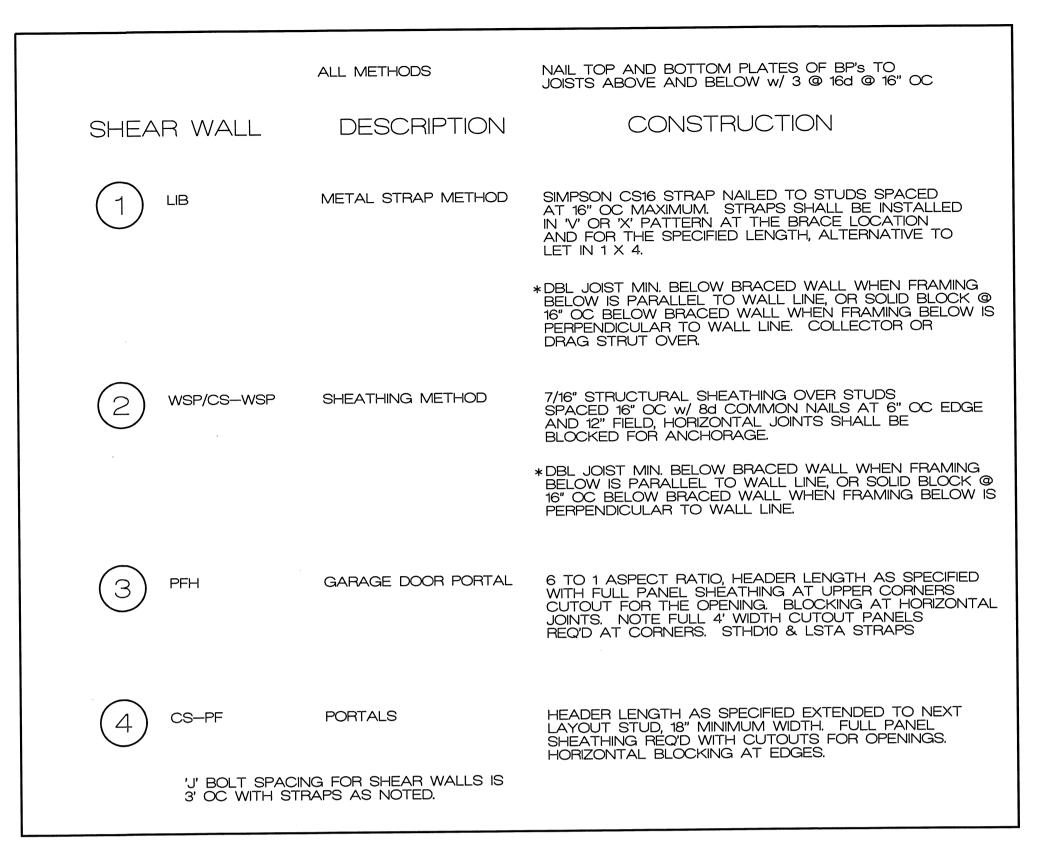
STAPLES NOT PERMITTED IN KCMO

	, , , , , ,	ENER SCHEDULE FO		
Item	Description of building ele		Number & type of fastener	Spacing of fasteners
oof				
1	Blocking between joists or rafters to to	op plate, toe nail	3-8d (2-1/2" × 0.113")	.
[2]	Ceiling joists to plate, toe nail		3-8d (2-1/2" × 0.113")	,
3	Ceiling joists not attached to parallel r. Collar tie rafter, face nail or 1-1/4" x 2		3-10d 3-10d (3" × 0.128")	; · · · · · · · · · · · · · - · · · · · · · · · · · · · · · · · · ·
⁴ 5	Rafter to plate, toe nail, note trusses u	use STC clips at NLB walls and spec'd holdowns	3-16d or 3-10d (3-1/2" × 0.135", 0.148")	2 toe nails side 1, 1 toe nail side 2 (note i)
6	Roof rafters to ridge, valley or hip raft	ers:	4 46d (0 4/0° × 0405°)	i I
	Toe nail: Face nail:		4-16d (3-1/2" x 0.135") 3-16d (3-1/2" x 0.135")	_
/all	race Hall.			
7	Built-up studs-face nail		10d (3" × 0.128")	24° o.c.
<mark>8</mark>	Abutting studs at intersecting wall con	ners, face nail	16d (3-1/2" × 0.135") 16d (3-1/2" × 0.135")	12" o.c. 16" o.c. along each edge
9	Built-up header, two pieces w/ 1/2" spacer Continued header, two pieces		16d (3-1/2" × 0.135")	16" o.c. along each edge
10 11	Continuous header to stud, toe nail		4-8d (2-1/2" × 0.113")	(
12	Double studs, face nail		10d (3" x 0.128") 10d (3" x 0.128")	24° o.c
13 14	Double top plates, face nail			
14 15	Double top plates, min. 48" offset of e Sole plate to joist or blocking, face na		8-16d (3-1/2" × 0.135") 16d (3-1/2" × 0.135")	[
	Sole plate to joist or blocking at brace	ed wall panels	3-16d (3-1/2" × 0.135")	16° a.c.
16 17	Stud to sole plate, toe nail		3-8d (2-1/2" x 0.113") or	_
	Top or sole plate to stud, end nail		2-16d (3-1/2" × 0.135") 2-16d (3-1/2" × 0.135")	· · · · · · · · · · · · · <u>-</u> · · · · · · · · · · · · · ·
. 18 . 19	Top plates, laps at corners and inters	sections, face nail	2-10d (3" x 0.128")	[
20	1" brace to each stud and plate, face		2-8d (2-1/2" × 0.113")	
			2 staples 1-3/4" 2-8d (2-1/2" x 0.113")	
21	1" x 6" sheathing to each bearing, fac		2 staples 1-3/4*	<u> </u>
. 22	1" x 8" sheathing to each bearing, fac	e nail	2-8d (2-1/2" × 0.113")	l .
			3 staples 1-3/4" 3-\$cs(2046€2"1-x3(0%)"3")	
23	Wider than 1" x 8" sheathing to each	bearing, face nail	3-803 (4 0) (40 2 (7×3/4/10)	
Floor 24	Joist to sill or girder, toe nail		3-8d (2-1/2" × 0.113")	
24 25	Rim joist to top plate, toe nail (roof a	oplications also)	8d (2-1/2" × 0.113")	6° o.c. 6° o.c.
26	Rim joist or blocking to sill plate, toe		8d (2-1/2" x 0.113") 2-8d (2-1/2" x 0.113")	
27	1" x 6" subfloor or less to each joist,	face nail	2 staples 1-3/4*	
28	2º subfloor to joist of girder, blind and	d face nail	2-16d (3-1/2" × 0.135")	
28 29 30	2" planks (plank & beam — floor and roof) Built—up girders and beams, 2" lumber layers		2-16d (3-1/2" × 0.135") 10d (3" × 0.128")	O each bearing Nail each layer as follows: 32" o.c. at top and bottom and staggered. Two nails at ends
	· li altri a l'aria i a l'aria de l'aria		3—16d (3—1/2" × 0.135")	and at each splice © each joist or rafter
31	Ledger strip supporting joists or rafte	I		f Fasteners
			,	Intermediate supports (inches)
	Description of building	Description of fastener	Edges (inches)	(notes: c, e)
	materials	\mid (notes: b, c, e) all sheathing to framing and particleboard wall sheath		
Wood struc	3/8" to 1/2"	6d common (2" x 0.113") nail (subfloor, wall) (note j)	6	12 (note: g)
		8d common (2-1/2" x 0.131") nail (roof)		La companya da com
33	19/32" to 1"	8d common nail (2-1/2" x 0.131")	6 6.	12 (note: g)
34	1-1/8" to 1-1/4"	10d common (3" x 0.148") nail or 8d deformed (2-1/2" x 0.131") nail		
Other wall	sheathing (note h)	an execution for the transfer of the transfer		
35	1/2" structural cellulosic fiberboard	1-1/2" galv. roofing nail, 7/16" crown or	3	6
	sheathing	1° crown staple 16 ga., 1-1/4° long		
36	25/32" structural cellulosic	1-3/4" galv. roofing nail, 7/16" crown or 1" crown staple 16 ga., 1-1/2" long	3	
	fiberboard sheathing 1/2" gypsum sheathing (note d)	1-1/2" galvanized roofing nail; staple galv.	7	7
	3,500	1-1/2" long; 1-1/4" screws, Type W or S	<u>.</u>	
38	5/8" gypsum sheathing (note d)	1-3/4" galvanized roofing nail; staple galv.	7	/
\\\\		1-5/8" long; 1-5/8" screws, Type W or S		
Wood struc	ctural panels, combination subfloor under 3/4" and less	6d deformed (2" x 0.120") nail or	6	12
,	3/7 & M 1633	8d common (2-1/2" × 0.131") nail	1	<u></u>
40	7/8" to 1"	8d common (2-1/2" x 0.131") nail or	6	12
		8d deformed (2-1/2" x 0.120") nail 10d common (3" x 0.148") nail or		12
41	1-1/8" to 1-1/4"			

For St 1 inch = 25.4 mm, 1 foot = 304.8 mm, 1 mile per hour = 0.447 m/s; 1 ksi = 6.895 MPa.

- a. All nails are smooth-common, box or deformed shanks except where otherwise stated. Nails used for framing and sheathing connections shall have minimum average bending yield strengths as shown: 80 ksi (551 MPa) for shank diameter of 0.192 inch (20d common nail), 90 ksi (620 MPa) for shank diameters larger than 0.142 inch but not larger than 0.177 inch, and 100 ksi (689 MPa) for shank diameters of 0.142 inch or less.
- Staples are 16 gage wire and have a minimum 7/16-inch on diameter crown width. c. Nails shall be spaced at not more than 6 inches on center at all supports where spans are 48 inches or greater.
- Four-foot-by-8-foot or 4-foot-by-9-foot panels shall be applied vertically. e. Spacing of fasteners not included in this table shall be verified w/ EOR.
- f. For regions having basic wind speed of 110 mph or greater, 8d deformed nails shall be used for attaching plywood and wood structural panel roof sheathing to framing within minimum 48-inch distance from gable end walls, if mean roof height is more than 25 feet, up to 35 feet maximum.
- g. For regions having basic wind speed of 100 mph or less, nails for attaching wood structural panel roof sheathing to gable endwall framing shall be spaced 6 inches on center. When basic wind speed is greater than 100 mph, nails for attaching panel roof sheathing to intermediate supports shall be spaced 6 inches on center for minimum 48-inch distance from ridges, eaves and gable end walls; and 4 inches on center to gable end wall framing.
- Gypsum sheathing shall conform to ASTMC 1396 and shall be installed in accordance with GA 253. Fiberboard sheathing shall conform to ASTM C 208. Spacing of fasteners on floor sheathing panel edges applies to panel edges supported by framing members and at all floor perimeters only. Spacing of fasteners on roof sheathing panel edges applies to panel edges supported by framing
- members and at all roof plane perimeters. Blocking of roof or floor sheathing panel edges perpendicular to the framing members shall not be required except at intersection of adjacent roof planes. Floor and roof perimeter shall be supported by
- Where a rafter is fastened to an adjacent parallel ceiling joist in accordance with this schedule, provide two toe nails on one side of the rafter and toe nails from the ceiling joist to top plate in accordance with this schedule. The toe nail on the opposite side of the rafter shall not be required.





SHEAR WALL SCHEDULE

CHAPTER 6 WALL CONSTRUCTION

FIGURE R602.10.6.2

METHOD PFH—PORTAL FRAME WITH HOLD-DOWNS

TYPICAL PORTAL FRAME CONSTRUCTION -

SECTION

EXTENT OF HEADER WITH SINGLE PORTAL FRAME
(ONE BRACED WALL PANEL)

MIN. 3"x11%" NET HEADER STEEL HEADER PROHIBITED IF %" SPACER IS USED, PLACE ON BACK-SIDE OF HEADEL

FASTEN SHEATHING TO HEADER WITH 8D COMMON OR GALVANIZED BOX NAILS IN 3" GRED PATTERN AS SHOWN

HEADER TO JACK-STUD STRAP PER TABLE —— R602, 10,64 ON BOTH SIDES OF OPENING OPPOSITE SIDE OF SHEATHING

MIN. DOUBLE 2X4 FRAMING COVERED WITH MIN.
NOT THICK WOOD STRUCTURAL PANEL SHEATHING
WITH 8D COMMON OR GALVANIZED BOX NAILS AT
3" O.C. IN ALL FRAMING (STUDS, BLOCKING, AND

IIN. LENGTH OF PANEL PER TABLE RE02.10.5

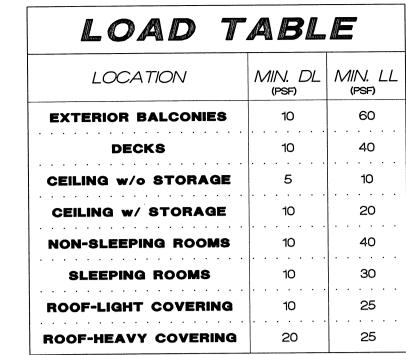
TOP AND BOTTOM OF FOOTING, LAP BARS 15"

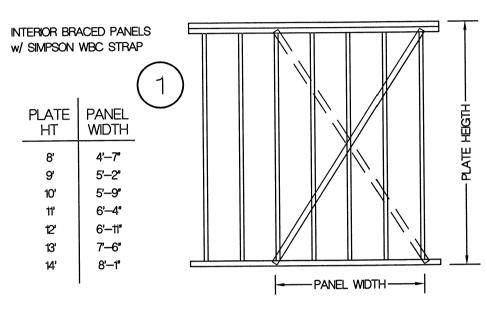
Alcon has hopen has has has has has A service of the serv — MIN. FOOTING SIZE UNDER OPENING IS 12"x12". A TURNED DOWN SLAB SHALL BE PERMITTED AT DOOR OPENINGS.

FRONT ELEVATION

For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm.

--- MIN. (1) % DIAMETER ANCHOR BOLT INSTALLED PER SECTION R403.1.5 - WITH 2"X 2" X*16" PLATE WASHER





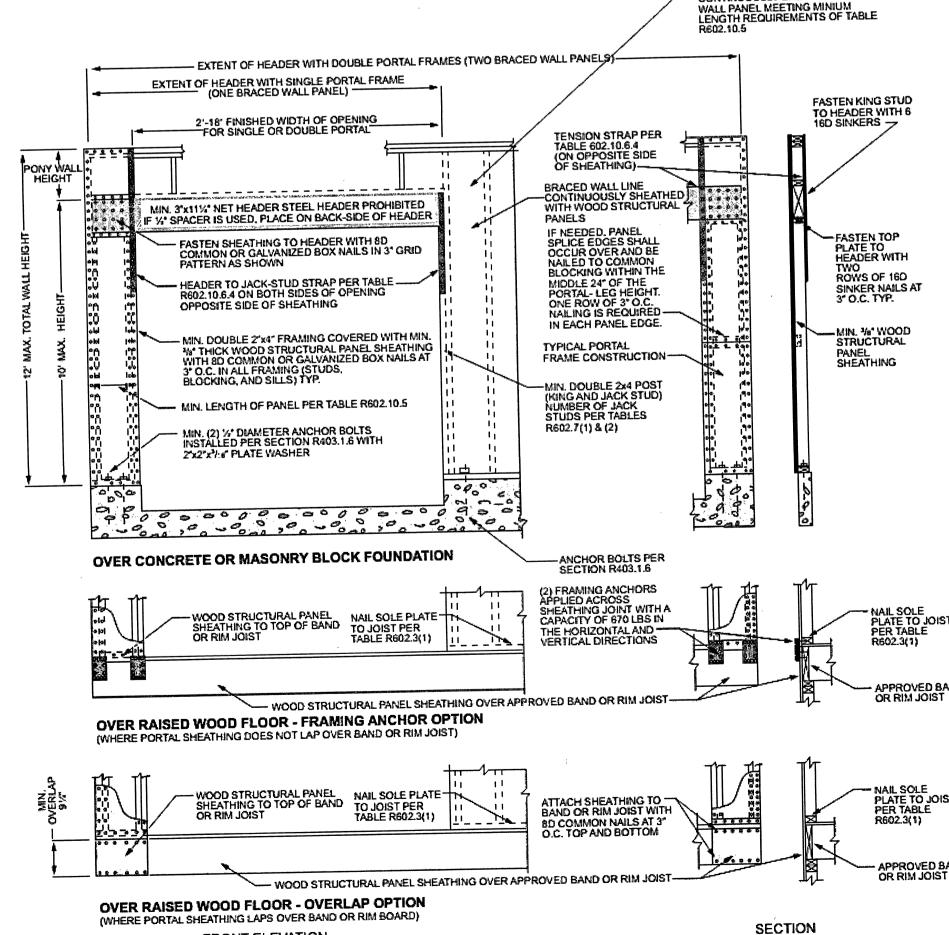
INT. BRACED WALL PANEL LIB, METAL STRAP ALT. TO LET IN 1 X 4

> 2018 International Residential Code Third Printing: Sep 2019

> > CONTINUOUSLY SHEATHED BRACED

11

CHAPTER 6 WALL CONSTRUCTION



For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm.

FRONT ELEVATION

DIGITAL CODES

FIGURE R602.10.6.4 METHOD CS-PF—CONTINUOUSLY SHEATHED PORTAL FRAME PANEL CONSTRUCTION

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