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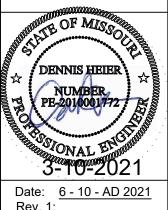
e into the creation and design of this plan. However, the design er and construction from these plans should not be undertake a construction professional, architect or engineer. Because of e consultation and supervision, Viewpoint Residential Design, responsibility for any damages, including structural failures, o sions or error in the design or blueprints. Also, site conditions of an this plan. Designer does not warrant the suitability of thes cific site. Consult your architect to determine the suitability of t

For God so loved the rorld, that he gave his nly begotten Son, that hosoever believeth in im should not perish, it have everlasting life" (John 3:16).

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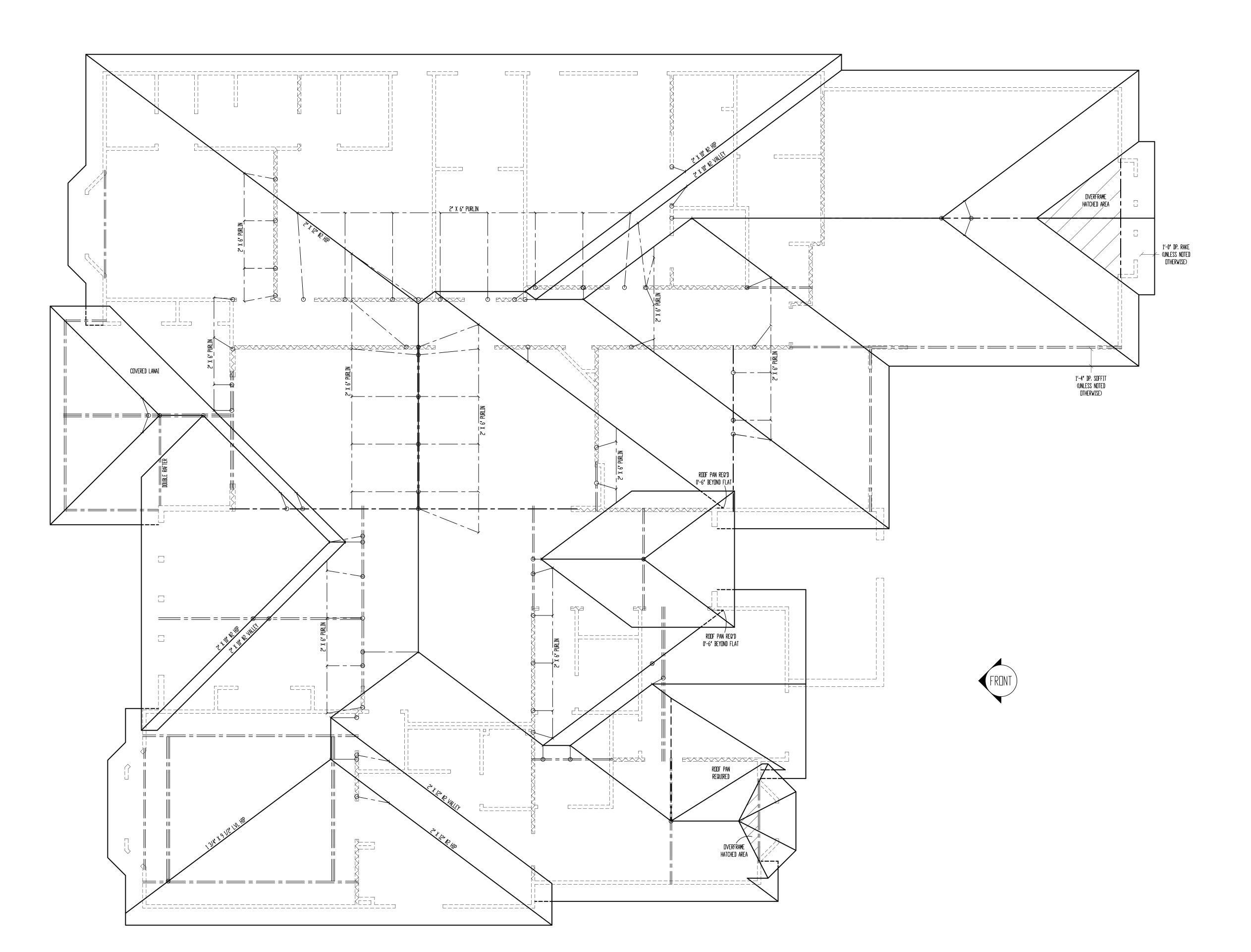
TOTTA
Seneral Contractor:



Date: 6 - 10 - AD 2021 Rev. 1: Rev. 2: Rev. 3:

Sheet Title: **ELEVATIONS**





ROOF AREA	4
PITCH	S
8/12	30
6/12	23

*ALL RAFTERS SHALL BE 2' X 6' #2 @ 16' D.C., UNLESS NOTED OTHERWISE.

FLASHING NOTE: DRIP EDGE, VALLEYS AND FLASHINGS TO BE METAL CLAD.

	OBSE TIERRETOTT				
	RAFTERS	SPACING	MAX HORIZONTAL CLEARSPAN		
	#2-2x6	@24″ □.C.	11'-7 '		
$\rangle\rangle\rangle$	#2-2x6	016 ′ □.C.	14'-2 "	((
	#2-2x8	@24″ □.C.	14'-8 "		
	#2-2x8	016 ′ □.C.	17'-11 '		
	#2-2x10	@24″ □.C.	17′-10 ″		
	#2-2x10	016 ′ □.C.	21′-11 ′		
	NOTE: COD	E MINIMUM ALL	OWS FOR A RAFTER DEFLECTION	OF	

OF L/180 TOTAL LOAD LITCHED DEDELIDMYNICE (DECLIMMENDED)

KALIEKS	2LHCTINO	MAY UNKITONIAL CEEAKOLAN
#2-2x6	@24″ □.C.	8'-6 "
#2-2x6	016 ′ □.C.	9'-9 '
#2-2x8	@24 ″ □.C.	11'-3 '
#2-2x8	016 ′ □.C.	12'-9 "
#2-2x10	@24 ″ □.C.	14′-3 ″
#2-2x10	016 ′ □.C.	16'-3 '
DEFLECTIO	N = L/360 LI ^v	VE LOAD, L/240 TOTAL LOAD

- #2- 2X8 UP TO 10/12 PITCH

- #2- 2X8 UP TO 10/12 PITCH

- #2- 2X10 OVER 10/12 PITCH

- Purlin Struts are at 4'-0" [].C. - PURLIN STRUTS SHALL BE INSTALLED AT NOT LESS THAN A

LOKTIN 21KO1	IMHY LOKTIN ZIKOI TENAI
(2) 2x4	8′-0 ′
(1) 2x4 & (1) 2x6	12'-0 '
(1) 2x6 & (1) 2x8	20'-0 '
(2) 2x6 & (1) 2x8	30'-0 "
CONSULT ARCH,/ENGR. >	30'-0 "

* RIDGE BRACES ARE SAME AS PURLIN BRACES-SPACING, SIZE, CONFIGURATION, & INSTALLATION (SEE PURLIN BRACE NOTES ABOVE) * HIP & VALLEY BRACES ARE SAME AS PURLIN SIZE, CONFIGURATION, & INSTALLATION

DOT IS BOTTOM OF BRACE (o).

* DENOTES BEARING WALL *---- DENOTES ROOF BRACE

Rev. 2:

Sheet Title: **ROOF PLAN**



SCALE: 1/4'' = 1'-0''

ROOF NOTES: ROOF DESIGNED FOR LIGHT ROOF COVERING 30psf TOTAL LOAD [10psf DL, 20psf LL (SL)]

* RAFTERS (HEM-FIR, DDUG-FIR, DR EQUAL): SEE SPAN CHARTS BELDW

	CODE MINIMUM			
	RAFTERS	SPACING	MAX HORIZONTAL CLEARSPAN	
	#2-2x6	@24″ □.C.	11'-7 '	
>>>	#2-2x6	016 ′ □.C.	14'-2 '	
	#2-2x8	@24″ □.C.	14'-8 "	
	#2-2x8	016 ′ □.C.	17'-11 '	
	#2-2x10	@24″ □.C.	17′-10 ″	
	#2-2x10	016 ′ □.C.	21′-11 ′	
	NOTE: CODE	E MINIMUM ALL	OWS FOR A RAFTER DEFLECTION	

HIGHER PERFURMANCE (RECUMMENDED)						
	RAFTERS	SPACING	MAX HORIZONTAL CLEARSPA			
	#2-2x6	@24″ □.C.	8'-6 '			
	#2-2x6	016 ′ □.C.	9'-9 '			
	#2-2x8	@24″ □.C.	11'-3 '			
	#2-2x8	016 ′ □.C.	12'-9 '			
	#2-2x10	@24″ □.C.	14'-3 '			
	#2-2x10	016 ′ □.C.	16'-3 "			
200 00000 1 1000 1000 1000 1000 1000						

* VAULTS TO BE 2x10 DEPTH * RIDGE BOARDS ARE: (UNLESS OTHERWISE NOTED) - #2- 2X10 OVER 10/12 PITCH

* ALL HIPS & VALLEYS ARE: (UNLESS OTHERWISE NOTED)

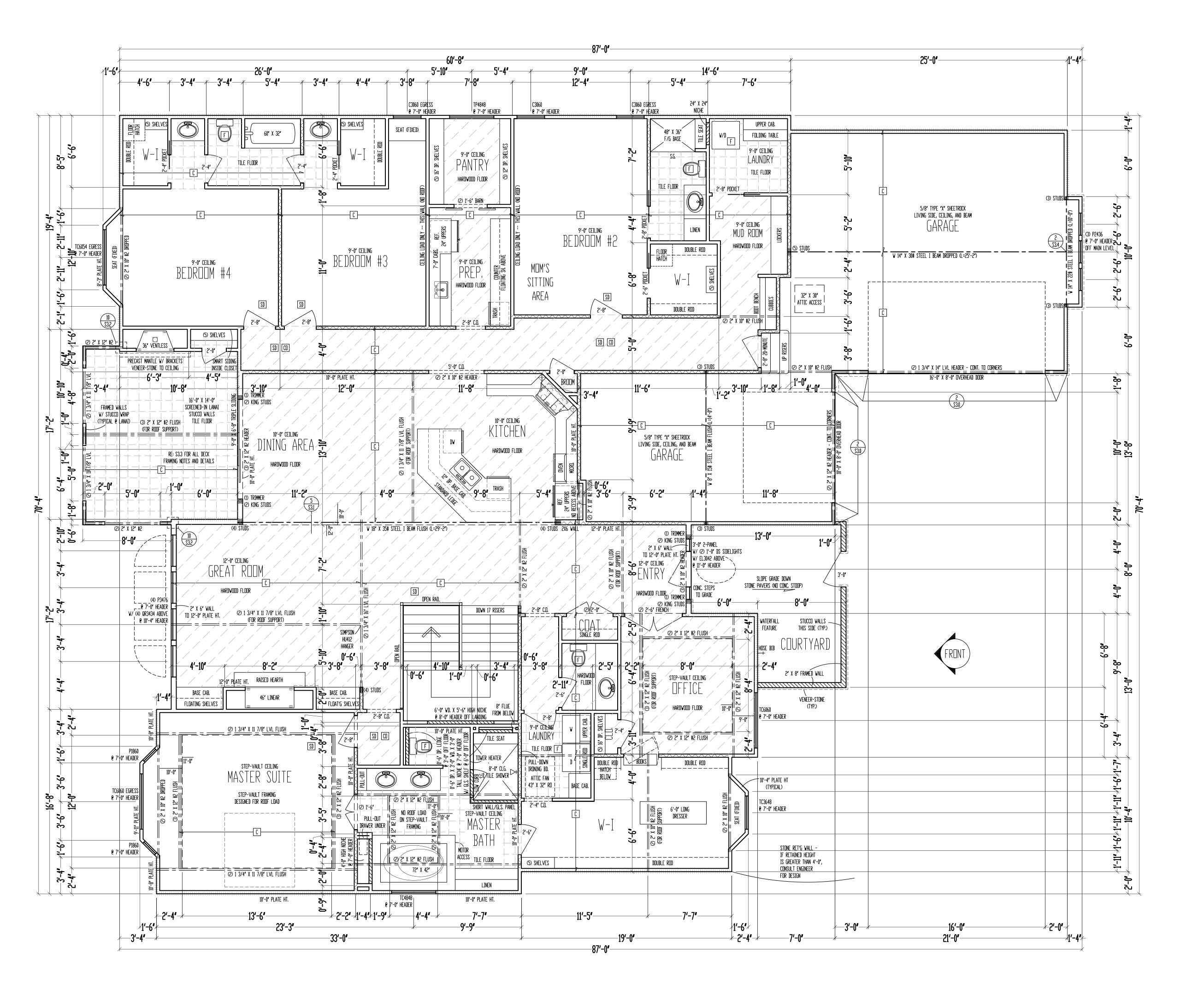
* PURLINS ARE 2X6 MIN.

45 Degree angle with the Horizontal - ALL PURLINS STRUTS SHALL HAVE A MAXIMUM UNBRACED LENGTH OF 8'-0' - PURLINS STRUTS SHALL BE CONSTRUCTED IN A 'T' CONFIGURATION AND PER THE FOLLOWING CHART:

PURI IN STRUT MAX PURI IN STRUT I FNGTH

*---- DENOTES PURLIN *—— DENOTES BEARING STRUCTURE

Rev. 3:



SCALE: 1/4'' = 1'-0''

GARAGE: 888 SQ. FT. COV. OUT/LIV: 240 SQ. FT. UNFIN. BASEMENT: 1499 SQ. FT.

JOIST SCHEDULE **€** 16″ □.C.

++++++ = WALL BRACING PER FRAMING NOTE #1 AND PER CALCULATIONS

FRAMING NOTES

1. MAIN LEVEL EXTERIOR WALLS SHALL BE SHEATHED W/ 7/16' D.S.B. A.P.A. PANELS W/ 8d COMMON NAILS @ 6' D.C. AT EDGES & @ 12' D.C. IN THE FIELD. SMART PANEL, DR EQUAL, INSTALLED PER MANUFACTURER'S SPECIFICATIONS. 2. \ \ \ \ \ \ \ \ = G.B.: 1/2" MIN. GYPSUM BOARD OVER STUDS SPACED 24" MAX

4. (2) 2' X 10' #2 HEADER AT ALL EXTERIOR AND LOAD BEARING WALLS, UNLESS NOTED

6. RUN STUDS THE FULL HEIGHT OF RAISED PLATE WALLS. 7. BLOCK JOISTS ABOVE BEAMS, CANTILEVERS AND LOAD BEARING WALLS WITH JOIST MATERIAL (NOT REQUIRED WITH I-JOISTS). 8. PROVIDE MULTIPLE STUDS FOR SOLID BEARING BELOW ALL BEAMS. 9. ALL DESIGNATED 2' X 6' WALLS SHALL HAVE DOUBLE KING STUDS AT DOOR AND WINDOW

12. EXTERIOR WALL BOTTOM PLATES SHALL BE NAILED TO FRAMING BELOW WITH 16d COMMON NAILS @ 8' D.C. MAX. (WHERE APPLICABLE.) 13. CONTRACTOR SHALL NOTIFY ENGINEER OF RECORD BEFORE CONSTRUCTION OF ANY DEFLECTION LIMITATIONS MORE STRINGENT THAN CODE MINIMUMS ABOVE ANY OPENINGS.

TILE FLOOR A	AREA	WOOD FLOOR A	AREA
LOCATION	SQ. FT.	LOCATION	SQ. FT
MASTER BATH	100	HARD₩OOD	1437
LAUNDRY	4 5	DECK	240
GAR. LAUNDRY	48		
BEDROOM #2 BATH	33		
JACK & JILL BATH	67		
LL POWDER ROOM	19		
LL WALKOUT	39		
II WET BAR	119		

LL 3/4 BATH 28



MAIN LEVEL: 3498 SQ. FT. LOWER LEVEL: 2027 SQ. FT. TOTAL: 5525 SQ. FT.

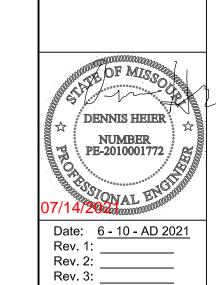


FASTENED W/ NO. 6 - 1 1/4" TYPE W OR S DRYWALL SCREWS @ 7" D.C. EDGES & FIELD. (MIN. 8'-0' SECTIONS ONE SIDE OF WALL (OR) MIN. 4'-0' SECTION FOR BOTH SIDES) 3. /\/\\/\/\/\\ = LOAD BEARING INTERIOR WALL.

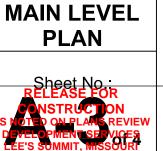
5. LOW TIES @ 4'-0" D.C. (TYPICAL)

10. ALL UNSQUARE WALLS SHALL BE 45°, UNLESS NOTED OTHERWISE. 11. ALL WALLS TO BE FRAMED W/ MIN. STUD GRADE 2" X 4"S @ 16" D.C., UNLESS NOTED

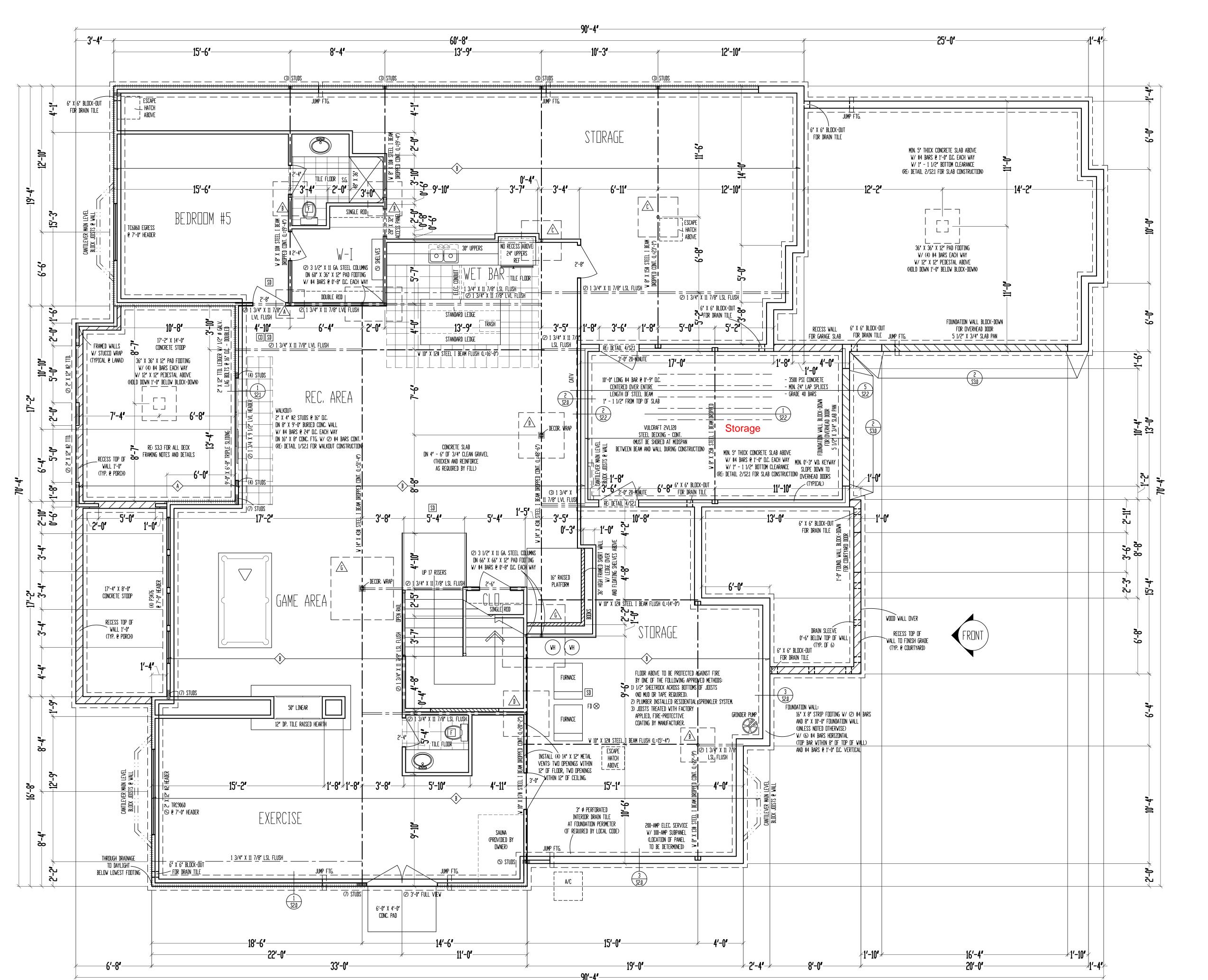
ILE FLOOR AREA		WOOD FLOOR A	AREA
LOCATION	SQ. FT.	LOCATION	SQ. FT.
ASTER BATH	100	HARDWOOD	1437
LAUNDRY	4 5	DECK	240
r. Laundry	48		
ROOM #2 BATH	33		
& JILL BATH	67		
POWDER ROOM	19		
L WALKOUT	39		
L WET BAR	119		



Care and effort have gon not an architect or engin without the assistance o impossibility of any on s and Designer assume n to any deficiencies, omit vary from those illustrate plans for use on your sp



Sheet Title:



(STEP WHERE GRADE REQUIRES)

11 7/8" I-JOIST FLOOR SYSTEM ABOVE FOUNDATION SCALE: 1/4" = 1'-0"

STEEL COLUMN & PAD FOOTING SCHEDULE 3" X 11 GA. STEEL COLUMN IN 30' X 30' X 10' PAD FOOTING W/ (4) #4 BARS EACH WAY (12.5k) B 3 1/2' X 11 GA. STEEL COLUMN ON 36' X 36' X 10' PAD FOOTING W/ (4) #4 BARS EACH WAY (18.0k) 3" SCH. 40 STEEL COLUMN C ON 42' X 42' X 12' PAD FOOTING W/ (5) #4 BARS EACH WAY (24.5k) 3 1/2" SCH. 40 STEEL COLUMN D\ DN 48" X 48" X 12" PAD FOOTING W/ (6) #4 BARS EACH WAY (32.0k) 3 1/2" SCH. 40 STEEL COLUMN E ON 54" X 54" X 14" PAD FOOTING W/ (7) #4 BARS EACH WAY (40.5k) 3 1/2" SCH. 40 STEEL COLUMN F\ DN 60" X 60" X 14" PAD FOOTING W/ (8) #4 BARS EACH WAY (50.0k) 4" SCH. 40 STEEL COLUMN G DN 66' X 66' X 16' PAD FOOTING

W/ (9) #4 BARS EACH WAY

PIER	R FOOTING SCHEDULE
₹	16" Ø PIER FTG.
\searrow	18" Ø PIER FTG.
K	24" Ø PIER FTG.
\triangle	28' Ø PIER FTG.

	JOIST SCHEDULE
\bigcirc A $>$	2" X 12" #2 TTD. FLOOR JOIS" @ 12" O.C.
\bigcirc B $>$	11 7/8' LPI-20 FLOOR JOIST @ 16' D.C. CONT.

FLATWORK AREA LOCATION SQ. FT. BASEMENT 3599 i GARAGE 843 REAR STOOP 385

CONC. PAD 24

1. BASEMENT LEVEL EXTERIOR WOOD-FRAMED WALLS SHALL BE SHEATHED W/ 7/16' D.S.B. A.P.A. PANELS W/ 8d COMMON NAILS @ 6' D.C. AT EDGES & @ 12' D.C. IN THE FIELD. SMART PANEL, DR EQUAL, INSTALLED PER MANUFACTURER'S SPECIFICATIONS. 2. \ \ \ \ \ \ \ \ = G.B.; 1/2' MIN. GYPSUM BDARD OVER STUDS SPACED 24' MAX FASTENED W/

ND. 6 - 1 1/4" TYPE W DR S DRYWALL SCREWS @ 7" D.C. EDGES & FIELD. (MIN. 8'-0" SECTIONS DNE SIDE OF WALL (OR) MIN. 4'-0" SECTION FOR BOTH SIDES) 3. /\/\/\/\/\/\\ = LOAD BEARING INTERIOR WALL.

4. (2) 2" X 10" #2 HEADER AT ALL EXTERIOR AND LOAD BEARING WALLS, UNLESS NOTED OTHERWISE.

5. LOW TIES @ 4'-0" D.C. (TYPICAL) 6. RUN STUDS THE FULL HEIGHT OF RAISED PLATE WALLS. 7. BLOCK JOISTS ABOVE BEAMS, CANTILEVERS AND LOAD BEARING WALLS WITH JOIST MATERIAL (NOT

REQUIRED WITH I-JOISTS). 8. PROVIDE MULTIPLE STUDS FOR SOLID BEARING BELOW ALL BEAMS.

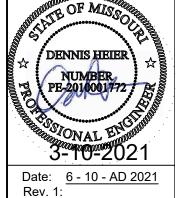
9. ALL DESIGNATED 2' X 6' WALLS SHALL HAVE DOUBLE KING STUDS AT DOOR AND WINDOW OPENINGS. 10. ALL UNSQUARE WALLS SHALL BE 45°, UNLESS NOTED OTHERWISE. 11. ALL WALLS TO BE FRAMED V/ MIN. STUD GRADE 2' X 4'S @ 16' O.C., UNLESS NOTED OTHERWISE. 12. 1/2' Ø ANCHOR BOLTS W/ MIN. 7' EMBEDMENT @ 48' D.C. MAX. & VITHIN 6' - 12' OF END OF EACH

13. NEW FOUNDATION SHALL BEAR ON ORIGINAL SOIL WITH MINIMUM BEARING CAPACITY OF 1500 PSF. A GEDTECHNICAL ENGINEER IS RECOMMENDED FOR VERIFICATION OF THESE CONDITIONS DURING THE EXCAVATION PHASE. ENGINEER OF RECORD ASSUMES NO RESPONSIBILITY FOR CONSTRUCTION NOT VERIFIED TO BE FOUNDED ON ANYTHING SHORT OF THE AFOREMENTIONED REQUIREMENTS. 14. CONTRACTOR SHALL NOTIFY ENGINEER OF RECORD BEFORE CONSTRUCTION OF ANY DEFLECTION

LIMITATIONS MORE STRINGENT THAN CODE MINIMUMS ABOVE ANY OPENINGS.

PLAN Sheet No.:

07/19/2021



Rev. 2: Rev. 3:

Sheet Title: **FOUNDATION**

	FASTENER SCHEDULE FOR STRUCTURAL MEMBERS	
DESCRIPTION OF BUILDING ELEMENTS	NUMBER AND TYPE OF FASTENER ROOF ¹	SPACING AND LOCATION
BLOCKING BETWEEN JOISTS OR RAFTERS TO TOP PLATE, TOE NAIL	4-8d (2½" x 0.113")	TOENAIL
CEILING JOISTS TO PLATE, TOE NAIL	4-8d (2½" x 0.113")	PER JOIST, TOENAIL
CEILING JOISTS NOT ATTACHED TO PARALLEL RAFTER, LAPS OVER PARTITIONS, FACE NAIL	4-10d (3" x 0.128")	FACE NAIL
CEILING JOIST TO PARALLEL RAFTER (HEEL JOINT)	TBLE R802.5.2	FACE NAIL
COLLAR TIE TO RAFTER, FACE NAIL OR 1 $\frac{1}{4}$ " x 20 GA. RIDGE STRAP TO RAFTER	4-10d (3" x 0.128")	FACE NAIL, EACH RAFTER
RAFTER OR ROOF TRUSS TO PLATE	3-16d BOX NAILS (3½" x 0.135") OR 3-10d COMMON NAILS (3" x 0.148")	2 TOE NAILS ON ONE SIDE AND 1 TOE NAIL ON OPPOSITE SIDE OF EACH RAFTER OR TRUSS
ROOF RAFTERS TO RIDGE, VALLEY, OR HIP RAFTERS OR ROOF RAFTER TO MINIMUM 2" RIDGE BEAM	4-16d (3 ½" x 0.135") - TOENAIL; 3-16d BOX (3 ½" x 0.135") - END NAIL	TOENAIL, END NAIL
	WALL	
STUD TO STUD (NOT AT BRACED WALL PANELS)	10d (3" x 0.128")	16" O.C. FACE NAIL
STUD TO STUD AND ABUTTING STUDS AT INTERSECTING WALL CORNERS (AT BRACED WALL PANELS)	16d (3½" x 0.135")	12" O.C. FACE NAIL
BUILT-UP HEADER, TWO PIECES WITH ½" SPACER	16d (3½" x 0.135")	12" O.C. EACH EDGE FACE NAIL
CONTINUOUS HEADER TO STUD	4-8d (2½" x 0.131")	TOENAIL
TOP PLATE TO TOP PLATE	10d (3" x 0.128")	12" O.C. FACE NAIL
DOUBLE TOP PLATE SPLICE	8-16d COMMON (3 ½" x 0.162")	FACE NAIL ON EACH SIDE OF END JOINT (MIN. 24 LAP SPLICE LENGTH EACH SIDE OF END JOINT)
BOTTOM PLATE TO JOIST, RIM JOIST, BAND JOIST, OR BLOCKING (NOT AT BRACED WALL PANELS)	16d COMMON (3 ½" x 0.162")	16" O.C. FACE NAIL
BOTTOM PLATE TO JOIST, RIM JOIST, BAND JOIST, OR BLOCKING (AT BRACED WALL PANEL)	3-16d BOX (3 ½" x 0.135")	3 EACH 16" O.C. FACE NAIL
TOP OR SOLE PLATE TO STUD, END NAIL	4-8d BOX (2 $\frac{1}{2}$ " x 0.113") - TOENAIL; 3-16d BOX (3 $\frac{1}{2}$ " x 0.135") - END NAIL	TOENAIL, END NAIL (SEE LEFT)
TOP PLATES, LAPS AT CORNERS AND INTERSECTIONS	3-10d BOX (3" x 0.128")	FACE NAIL
1" BRACE TO EACH STUD AND PLATE	3-8d BOX (2 ½" x 0.113")	FACE NAIL
1"x6" SHEATHING TO EACH BEARING	3-8d BOX (2 ½" x 0.113")	FACE NAIL
1"x8" SHEATHING TO EACH BEARING	3-8d BOX (2 $\frac{1}{2}$ " x 0.113") - FACE NAIL; WIDER THAN 1"x8" - 4-8d BOX (2 $\frac{1}{2}$ " x 0.113")	FACE NAIL
	FLOOR	
JOIST TO SILL, TOP PLATE, OR GIRDER	4-8d BOX (2 ½" x 0.113")	TOE NAIL
RIM JOIST, BAND JOIST, OR BLOCKING TO SILL OR TOP PLATE (ROOF APPLICATIONS ALSO)	8d BOX (2 ½" x 0.113")	4" O.C. TOE NAIL
1" x 6" SUBFLOOR OR LESS TO EACH JOIST	3-8d BOX (2 ½" x 0.113")	FACE NAIL
2" SUBFLOOR TO JOIST OR GIRDER	3-16d BOX (3 ½" x 0.135")	BLIND AND FACE NAIL
2" PLANKS (PLAN & BEAM - FLOOR AND ROOF)	3-16d BOX (3 ½" x 0.135")	AT EACH BEARING, FACE NAIL
BAND OR RIM JOIST TO JOIST	3-16d COMMON (3 ½" x 0.162")	END NAIL
BUILT-UP GIRDERS AND BEAMS, 2-INCH LUMBER LAYERS	10d BOX (3" x 0.128")	24" O.C. FACE NAIL AT TOP AND BOTTOM STAGGERED ON OPPOSITE SIDES
LEDGER STRIP SUPPORTING JOISTS OR RAFTERS	4-16d BOX (3 ½" x 0.135")	AT EACH JOIST OR RAFTER, FACE NAIL

DESCRIPTION OF BUILDING MATERIAL	FASTNER SCHEDULE FOR S DESCRIPTION OF FASTENER	R STRUCTURAL MEMBERS EDGE SPACING (INCHES)	INTERMEDIATE SUPPORTS (INCHES)
WOOD STRUCTURAL PANELS, SU	BFLOOR, ROOF AND INTERIOR WALL SHE	ATHING TO FRAMING AND PARTICLEBOA	RD WALL SHEATHING TO FRAMING ¹
3/8" - 1/2"	6d COMMON (2" x 0.113") NAIL (SUBFLOOR, WALL) 8d COMMON NAIL (ROOF)	6	12
¹⁹ / ₃₂ " - 1"	8d COMMON NAIL (2½" x 0.131")	6	12
1½" - 1½"	10d COMMON (3" x 0.148") NAIL OR 8d (2½" x 0.131") DEFORMED NAIL	6	12
	OTHER WALL	SHEATHING 1	
½" STRUCTURAL CELLULOSIC FIBERBOARD SHEATHING	1 $\frac{1}{2}$ " GALVANIZED ROOFING NAIL, $\frac{7}{16}$ " HEAD DIAMETER, OR 1 $\frac{1}{4}$ " LONG 16 GA. STAPLE WITH $\frac{7}{16}$ " OR 1" CROWN	3	6
25" STRUCTURAL CELLULOSIC FIBERBOARD SHEATHING	$1\frac{3}{4}$ " GALVANIZED ROOFING NAIL, $\frac{7}{16}$ " HEAD DIAMETER, OR $1\frac{1}{2}$ " LONG 16 GA. STAPLE WITH $\frac{7}{16}$ " OR 1" CROWN	3	6
½" GYPSUM SHEATHING	1½" GALVANIZED ROOFING NAIL; STAPLE GALVANIZED, 1½" LONG; 1¼" SCREWS, TYPE W OR S	7	7
%" GYPSUM SHEATHING	1¾" GALVANIZED ROOFING NAIL; STAPLE GALVANIZED, 1%" LONG; 1%" SCREWS, TYPE W OR S	7	7
V	OOD STRUCTURAL PANELS, COMBINATIO	N SUBFLOOR UNDERLAYMENT TO FRAM	ING
¾" AND LESS	6d DEFORMED (2" x 0.120") NAIL OR 8d COMMON (2½" x 0.131") NAIL	6	12
7∕8" - 1"	8d COMMON (2½" x 0.131") NAIL OR 8d DEFORMED (2½" x 0.120") NAIL	6	12
11/8" - 11/4"	10d COMMON (3" x 0.148") NAIL OR 8d DEFORMED (2½" x 0.120") NAIL	6	12

1. IF INFORMATION LISTED ON PLAN SHEETS CONTRADICTS INFORMATION IN THIS TABLE, INFORMATION ON PLANS TAKES PRECEDENCE OVER INFORMATION

FOUNDATION NOTES

- CONCRETE SHALL BE AIR-ENTRAINED BETWEEN 5%-7% WITH A MINIMUM 28-DAY COMPRESSIVE STRENGTH OF 2500 PSI FOR BASEMENT AND INTERIOR FLOOR SLABS-ON-GRADE, 3000 PSI FOR FOUNDATION WALLS, AND 3500 PSI FOR
- PORCHES AND GARAGE FLOOR SLABS

 2. THE FOUNDATION DESIGN SHALL COMPLY WITH THE ENFORCING JURISDICTION'S RESIDENTIAL FOUNDATION STANDARDS
- 3. PROVIDE A MINIMUM 4"-DIAMETER PERFORATED DRAIN PIPE ALONG PERIMETER OF USABLE SPACE AT FOOTING LEVEL OR OTHER EQUIVALENT MATERIALS PER IRC SECTION R405.1. THE PIPE SHALL BE COVERED WITH A MINIMUM OF 6" OF GRAVEL OR CRUSHED ROCK. THE DRAIN SHALL DAYLIGHT BELOW FOOTING LEVEL OR TERMINATE IN A
- MINIMUM 20 GALLON SUMP PIT.

 4. FOUNDATION SHALL BE DESIGNED FOR A BEARING CAPACITY OF 1500 PSF AND FOUNDED ON COMPETENT ORIGINAL SOIL AS DETERMINED AND CONFIRMED BY A LICENSED GEOTECHNICAL ENGINEER OR ENGINEERING GEOLOGIST. ENGINEER OF RECORD ASSUMES NO RESPONSIBILITY FOR CONSTRUCTION NOT VERIFIED TO BE FOUNDED ON ANY
- ENGINEER OF RECORD ASSUMES NO RESPONSIBILITY FOR CONSTRUCTION NOT VERIFIED TO BE FOUNDED ON ANY SOIL WITH THE AFOREMENTIONED MINIMUM PROPERTIES.

 5. FOOTINGS SHALL BE A MINIMUM OF 16" WIDE x 8" DEEP AND SHALL HAVE A MINIMUM OF (2) CONTINUOUS GRADE 40
- #4 BARS WITH 3" BOTTOM CLERANCE. BOTTOM OF FOOTING SHALL BE LOCATED A MINIMUM OF 3'-0" BELOW GRADE FOR FROST PROTECTION.
- CONCRETE PADS SUP0PORTING COLUMN LOADS SHALL BE NO SMALLER THAN 2'-0" x 2'-0" x 1'-0" DEEP WITH A MINIMUM OF (4) GRADE 40 #4 BARS EACH WAY WITH 3" BOTTOM CLEARANCE
- 7. FOUNDATION WALLS SHALL BE A MINIMUM OF 8" NOMINAL WIDTH AND SHALL HAVE HOIZONTAL GRADE 40 #4 BARS AT 2'-0" O.C. MAX. WITH VERTICAL #4 BARS AS REQUIRED ON FOUNDATION CROSS SECTION ON SHEET S2.0

 8. REINFORCEMENT SHALL LAP A MINIMUM OF 2'-0" (CLASS B SPLICE)
- 9. INTERIOR BEARING WALLS AND COLUMNS SHALL BE ISOLATED FROM THE BASEMENT FLOOR SLAB
- 10. BASEMENT FLOOR SLAB SHALL BE A MINIMUM OF 4" THICK ON A MINIMUM BASE COURSE OF 4" TO 6" OF SAND, GRAVEL OR CRUSHED ROCK. BETWEEN THE BASE COURSE AND FLOOR SLAB SHALL BE PLACED A 6-MIL POLY VAPOR RETARDER WITH MINIMUM OVERLAP OF 6" AT DISCONTINUITIES
- 11. IF A FLOOR IS TO BE SUPPORTED BY A MINIMUM OF 2'-0" OF GRANULAR FILL OR 8" OF EARTH, BASEMENT SLAB SHALL BE DESIGNED BY A LICENSED ENGINEER
- 12. SILL PLATES SHALL BE ANCHORED TO THE FOUNDATION WALL WITH ½" Ø ANCHOR BOLTS EMBEDDED A MINIMUM OF 7" INTO CENTER OF WALL STEM AND SHALL BE INSTALLED AT A MAXIMUM OF 6'-0" O.C. (OR AS NOTED ON PLANS)
- AND SHALL BE INSTALLED WITHIN 6" TO 12" OF EACH END OF EACH SILL PLATE LENGTH, PER IRC SECTION R403.1.6

 13. FOUNDATION WINDOW WELLS SHALL BE PROVIDED WITH MINIMUM DIMENSIONS AS SHOWN IN DETAIL ON SHEET
- 14. THE GARAGE FLOOR SHALL SLOPE TOWARD THE VEHICLE DOORS OR TO A TRENCH OR UNTRAPPED DRAIN THAT DISCHARGES TO THE EXTERIOR, ABOVE GRADE

FRAMING NOTES

- 5. ALL DIMENSIONAL LUMBER SHALL BE DOUGLAS-FIR-LARCH GRADE #2, UNLESS NOTED OTHERWISE ON PLANS

 ALL INTERIOR LOAD REARING AND EXTERIOR WALL HEADERS SHALL BE (2) #2 2×10's LINI ESS NOTED OTHERWISE
- ALL INTERIOR LOAD-BEARING AND EXTERIOR WALL HEADERS SHALL BE (2) #2 2x10's, UNLESS NOTED OTHERWISE ON PLANS
- 17. BLOCK OVER BEAMS AND AT CANTILEVERS AND DOOR JAMBS
- 18. INTERIOR NON-BEARING WALLS RESTING ON BASEMENT SLAB SHALL BE ISOLATED FROM ABOVE FRAMING BY A
- 19. ALL HEADERS/BEAMS SHALL BEAR ON A MINIMUM OF (2) 2x4 POSTS (KING AND JACK STUDS), UNLESS NOTED OTHERWISE
- 20. WHERE JOISTS SPAN PARALLEL TO FOUNDATION, BLOCKING SHALL BE PROVIDED IN THE TWO SPACES MOST ADJACENT TO THE FOUNDATION WALL AT 4'-0" O.C. FOR THE PURPOSE OF TRANSFERRING LATERAL FOUNDATION WALL LOAD TO THE FLOOR DIAPHRAGM. FASTEN JOISTS AND BLOCKING TO SILL PLATE WITH (4) 10d NAILS. IF MECHANICAL DUCTWORK IS INSTALLED IN ONE OF THESE FIRST TWO BAYS, FASTEN 2x4's FLAT AT 4'-0" O.C. BETWEEN JOIST(S) AND/OR SILL AND PROVIDE BLOCKING AS PRESCRIBED ABOVE IN THE NEXT TWO JOIST BAYS. SECURE 2x4's TO JOIST(S)/SILL PLATE WITH (4) 10d NAILS.
- 21. ALL WOOD MATERIAL SUPPORTED ON CONCRETE OR MASONRY SHALL BE TREATED OR OF DECAY-RESISTANT
- 22. JOISTS UNDER BEARING PARTITIONS ON PLANS HAVE BEEN SIZED TO SUPPORT THE DESIGN LOAD.
- 23. JOISTS FRAMING INTO THE FACE OF A STEEL OR WOOD BEAM SHALL BE SUPPORTED WITH APPROPRIATE COLD-FORMED STEEL JOIST HANGERS
- 24. JOISTS FRAMED ON TOP OF STRUCTURAL MEMBER SHALL BE SUPPORTED AT EN DS BY FULL-DEPTH SOLID
- BLOCKING MIN. 1%" IN THICKNESS OR BY FASTENING RIM TO JOISTS PER FASTENING TABLE TO LEFT 25. ALL WALL COVERINGS SHALL COMPLY WITH IRC SECTION R702.3
- 26. ALL RAFTERS AND COLLAR TIES SHALL COMPLY WITH IRC SECTION R802.3.
- 20. ALL RAFTERS AND COLLAR TIES SHALL COMPLY WITH IRC SECTION R002.3.
- 27. ALL RAFTERS SHALL HAVE 2x4 COLLAR TIES @ 4'-0" O.C. IN UPPER 1/3 OF VERTICAL DISTANCE BETWEEN CEILING AND ROOF
- 28. BLOCKING BETWEEN JOISTS UNDER A LOAD-BEARING WALL IS NOT REQUIRED
 29. PER IRC SECTION 501.3, BOTTOM OF ALL FLOOR ASSEMBLIES ABOVE UNFINISHED AREAS SHALL BE PROVIDED WITH
- A ½" GYPSUM BOARD MEMBRANE OR RESIDENTIAL FIRE SPRINKLER SYSTEM WHEN FLOOR SYSTEM IS CONSTRUCTED OF OTHER THAN DIMENSION LUMBER OR STRUCTURAL COMPOSITE LUMBER EQUAL TO OR GREATER THAN 2x10 NOMINAL DIMENSION(WHERE REQUIRED BY ENFORCING JURISDICTION)
- 30. ENGINEERED LVL's SHALL HAVE MINIMUM PROPERTIES OF Fb = 2600 psi, E=1900 ksi, AND Fv=285 psi
- 31. ENGINEERED PARALLAMS SHALL HAVE MINIMUM PROPERTIES OF Fb = 2600 psi, E = 2000 ksi, AND Fv = 290 psi
 32. COLUMN CONNECTION TO STEEL BEAMS SHALL BE WITH A CLIP POST CAP WITH ALL FOUR TAB EARS BENT AROUND THE BOTTOM FLANGE OF THE BEAM. FOR A BEARING PLATE, FOUR HOLES SHALL BE DRILLED IN THE BOTTOM FLANGE OF THE STEEL BEAM TO MATCH THE HOLE PATTERN OF THE PLATE. ½" x 2" BOLTS SHALL THEN BE INSTALLED WITH A FLAT WASHER, LOCK WASHER, AND A NUT IN EACH OF THE HOLES. THE POST CAP MAY BE WELDED TO THE STEEL BEAM IN ACCORDANCE WITH AWS D1.1-92 AS AN ALTERNATIVE, AND WOULD NEED TO BE INSPECTED BY AN AWS-CERTIFIED INSPECTOR.
- 33. WHEN MECHANICAL EQUIPMENT IS LOCATED IN AN ENCLOSED ROOM, THERE SHALL BE (2) 14"x12" VENTS LOCATED IN A WALL COMMON WITH ADDITIONAL LIVING AREA. ONE VENT SHALL BE LOCATED SUCH THAT THE BOTTOM OF THE VENT BEGINS 12" FROM THE FLOOR AND THE OTHER VENT SHALL BE LOCATED SUCH THAT THE TOP OF THE VENT BEGINS 12" FROM THE CEILING.
- 34. ALL ROOF SHEATHING SHALL BE $\frac{7}{16}$ " OSB WITH 8d COMMON NAILS @ 6" O.C. AT PANEL EDGES AND @ 12" O.C. IN FIELD

GLAZING NOTES

- 35. GLAZING IN HAZARDOUS LOCATIONS AS IDENTIFIED IN IRC SECTION R308.4 SHALL BE OF APPROVED SAFETY GLAZING MATERIALS. GLASS IN STORM DOORS, INDIVIDUAL FIXED OR OPENABLE PANELS ADJACENT TO A DOOR WHERE THE NEAREST VERTICAL EDGE IS WITHIN A 2'-0" ARC OF THE DOOR IN A CLOSED POSITION AND FOR WHICH THE BOTTOM EDGE IS WITHIN 5'-0" OF THE FLOOR, WALLS ENCLOSING STAIRWAYS AND LANDINGS WHERE THE GLAZING IS WITHIN 5'-0" OF THE TOP OR BOTTOM OF THE STAIR, ENCLOSURES FOR SPAS, TUBS, SHOWERS, AND WHIRLPOOLS, GLAZING IN FIXED OR OPENABLE PANELS EXCEEDING NINE SQUARE FEET AND FOR WHICH THE BOTTOM EDGE IS LESS THAN 1'-6" ABOVE THE FLOOR OR WALKING SURFACE WITHIN 3'-0"
- 36. ALL OPERABLE WINDOWS SHALL HAVE FALL PROTECTION PER IRC SECTION R612.2

ATTIC VENTILATION

ENCLOSED ATTICS SHALL HAVE CROSS VENTILATION FOR EACH SEPARATE SPACE BY VENTILATING OPENINGS PROTECTED AGAINST THE ENTRANCE OF RAIN OR SNOW. VENTILATING OPENINGS SHALL BE PROVIDED WITH CORROSION-RESISTANT WIRE MESH, WITH $\frac{1}{8}$ " TO $\frac{1}{4}$ " OPENINGS. THE TOTAL FREE VENTILATING AREA SHALL NOT BE LESS THAN $\frac{1}{150}$ OF THE AREA OF SPACE VENTILATED, EXCEPT WHERE THE VENTILATORS ARE LOCATED IN THE UPPER PORTION OF THE SPACE TO BE VENTILATED - THE REQUIRED AREA MAY BE REDUCED TO 1/300.

EMERGENCY EGRESS

- 8. PROVIDE A MINIMUM OF ONE WINDOW FOR EACH BEDROOM THAT HAS A MINIMUM OPENABLE AREA OF 5.7 SQUARE FEET WITH A MINIMUM OPENABLE HEIGHT OF 2'-0" AND A MINIMUM WIDTH OF 1'-9". IN ADDITION, THE OPENABLE PORTION OF EGRESS WINDOWS SHALL NOT EXCEED 3'-8" ABOVE THE ADJOINING FLOOR OR PERMANENT STEP.
- 9. PROVIDE SMOKE ALARMS IN EACH SLEEPING ROOM, OUTSIDE OF EACH SLEEPING AREA AND ON EACH FLOOR, INCLUDING BASEMENT (IF APPLICABLE). ALARMS SHALL BE HARDWIRED TOGETHER SO THAT THE ACTIVATION OF ONE SMOKE ALARM WILL ACTIVATE ALL SMOKE ALARMS IN THE DWELLING. PROVIDE CARBON MONOXIDE DETECTORS OUTSIDE EACH SLEEPING AREA.

MASONRY VENEER

- 40. MASONRY VENEER SHALL BE ANCHORED TO THE SUPPORTING WALL STUDS WITH CORROSION-RESISTANT METAL TIES EMBEDDED IN MORTAR OR GROUT AND EXTENDING INTO THE VENEER A MINIMUM OF 1½", WITH NOT LESS
- THAN %" MORTAR OR GROUT COVER TO OUTSIDE FACE.

 41. VENEER TIES, IF STRAND WIRE, SHALL NOT BE LESS IN THICKNESS THAN NO. 9 U.S. GAGE WIRE AND SHALL HAVE A HOOK EMBEDDED IN THE MORTAR JOINT, OR IF SHEET METAL, SHALL BE NOT LESS THAN NO. 22 U.S. GAGE BY %" CORRUGATED.

OPENINGS GREATER THAN 16 INCHES IN EITHER DIMENSION. METAL TIES AROUND THE PERIMETER OF OPENINGS

42. EACH TIE SHALL SUPPORT NOT MORE THAN 2.67 SQUARE FEET OF WALL AREA AND SHALL BE SPACED NOT MORE THAN 32 INCHES ON CENTER HORIZONTALLY AND 24 INCHES ON CENTER VERTICALLY.
 43. VENEER TIES AROUND WALL OPENINGS: ADDITIONAL METAL TIES SHALL BE PROVIDED AROUND ALL WALL

SHALL BE SPACED NOT MORE THAN 3 FEET ON CENTER AND PLACED WITHIN 12 INCHES OF THE WALL OPENING.

- 14. DOOR(S) BETWEEN THE GARAGE AND DWELLING SHALL BE MINIMUM 1% SOLID CORE OR HONEY-COMBED STEEL DOOR WITH 20-MINUTE FIRE RATING EQUIPPED WITH A SELF-CLOSING DEVICE
- 45. VEHICLE DOORS AND FRAMES SHALL BE DESIGNED AND INSTALLED TO MEET THE 115-MPH 3-SECOND GUST LOADING PER DASMA 108 AND ASTM E 330-96 PER IRC 2018

GARAGE NOTES (CONTINUED)

- 44. THE GARAGE SHALL BE SEPARATED FROM THE DWELLING AND ITS ATTIC AREAS BY MINIMUM 5/8" GYP. BOARD APPLIED TO THE GARAGE SIDE OF FRAMING. WHERE HABITABLE SPACE OCCURS ABOVE THE GARAGE, THE GARAGE CEILING ASSEMBLY SHALL BE PROTECTED WITH A MINIMUM 5/8" TYPE X GYP. BOARD. WHERE A FLOOR/CEILING SPACE IS PROVIDED ABOVE THE GARAGE COLUMNS AND BEAMS SUPPORTING THE SEPARATION SHALL ALSO BE PROTECTED WITH 5/8" GYP. BOARD.

 45. GARAGE DOOR H-FRAME FOR THE ATTACHMENT OF THE TRACK AND COUNTER
- 45. GARAGE DOOR H-FRAME FOR THE ATTACHMENT OF THE TRACK AND COUNTER BALANCE SHALL CONSIST OF THE FOLLOWING: 2x6 VERTICAL JAMBS RUNNING FROM FLOOR TO CEILING AND SHALL BE FASTENED WITH 2½"" x 0.120" NAILS AT 7" O.C. STAGGERED WITH (7) 3½" x 0.120" NAILS THROUGH THE JAMB INTO THE HEADER. MINIMUM 2x8 HEADER FOR ATTACHMENT OF COUNTER BALANCE SYSTEM.

DESIGN LOADING (PER TABLE R301.5)

MINIMUM UNIFORMLY DISTRIB		ADS (PSF)
USE	LIVE LOAD	ĎEAD LOAD
UNINHABITABLE ATTICS WITHOUT STORAGE	10	10
UNINHABITABLE ATTICS WITH LIMITED STORAGE	20	10
HABITABLE ATTICS AND ATTICS SERVED WITH FIXED STAIRS	30	10
BALCONIES (EXTERIOR) AND DECKS	40	10 ^d
FIRE ESCAPES	40	10
GUARDRAILS AND HANDRAILS ^a	200 ^c	-
GUARDRAIL IN-FILL COMPONENTS ^b	50 ^c	ı
PASSENGER VEHICLE GARAGES	50	DEPENDENT UPON SLAB CONSTRUCTION
ROOMS OTHER THAN SLEEPING ROOM	40	10 ^d
SLEEPING ROOM	30	10 ^d
STAIRS	40	10 ^d

a. A single concentrated load applied in any direction at any point along the top.

withstand a horizontally applied normal load of 50 pounds on an area equal to one square foot. This load need not be assumed to act concurrently with any other live load requirement.

c. Glazing used in handrail assemblies and guards shall be designed with a safety factor of 4. The safety factor shall be applied to each of the concentrated loads applied to the top of the rail, and to the load on the infill components. These loads shall be determined independently of one another, and loads are assumed not to occur with any other live load.

b. Guard in-fill components (all those except the handrail), ballusters and panel fillers shall be designed to

d. An additional dead loading of 10 psf shall be applied where thinset tile floor is to be installed. An additional dead loading of 50 psf shall be applied where mudset tile floor is to be installed.

INSULATION/EFFICIENCY

- 1. BUILDING ENVELOPE INSULATION SHALL COMPLY WITH IRC TABLE N1102.1.1 OR THE 2012 IECC (SEE SHEET S3.1 FOR FRAMING DETAILS AND TABLES ON THIS SHEET FOR MORE INFORMATION)
- 2. CATHEDRAL -VAULTED CEILING FRAMING SHALL BE FRAMED WITH A MINIMUM INSULATION VALUE OF R-38. IF VAULTED RAFTERS DO NOT PROVIDE REQUIRED DEPTH TO ACHIEVE R-38 INSULATION BUILDER SHALL FUR DOWN RAFTERS PER DETAILS PROVIDED ON SHEET S3.1.

INSULATION AND FENESTRATION REQUIRE	MENTS BY COMPONENT (TABLE N1102.1.1)
CLIMATE ZONE	4-A
FENESTRATION U-FACTOR	0.35
SKYLIGHT U-FACTOR	0.55
GLAZED FENSTRATION SHGC	0.40
CEILING R-VALUE	49
WOOD FRAME WALL R-VALUE	15
MASS WALL R-VALUE	8 / 13
FLOOR R-VALUE	19
BASEMENT WALL R-VALUE	10-CONTINUOUS OR 13-CAVITY
SLAB R-VALUE AND DEPTH	10 AT 2'-0"
CRAWL SPACE WALL R-VALUE	10-CONTINUOUS OR 13-CAVITY
DUCTWORK EXPOSED TO OUTSIDE AIR R-VALUE	8
DUCTWORK NOT EXPOSED TO OUTSIDE AIR R-VALUE	6
CATHEDRAL VAULTED CEILING R-VALUE	38

DUCT SEALING

N1103.2.2 (R403.2.2) SEALING (MANDATORY). DUCTS, AIR HANDLERS, AND FILTER BOXES SHALL BE SEALED. JOINTS AND SEAMS SHALL COMPLY WITH SECTION M1601.4.1 OF 2018 IRC.

- 1. AIR-IMPERMEABLE SPRAY FOAM PRODUCTS SHALL BE PERMITTED TO BE APPLIED
- WITHOUT ADDITIONAL JOINT SEALS.
 WHERE A DUCT CONNECTION IS MADE THAT IS PARTIALLY INACCESSIBLE, THREE SCREWS OR RIVETS SHALL BE EQUALLY SPACED ON THE EXPOSED PORTION OF THE
- JOINT SO AS TO PREVENT A HINGE EFFECT.

 CONTINUOUSLY WELDED AND LOCKING-TYPE LONGITUDINAL JOINTS AND SEAMS IN DUCTS OPERATING AT STATIC PRESSURES LESS THAN 2 INCHES OF WATER COLUMN PRESSURE CLASSIFICATION SHALL NOT REQUIRE ADDITIONAL CLOSURE SYSTEMS.
- DUCT TIGHTNESS SHALL BE VERIFIED BY EITHER OF THE FOLLOWING:
- 1. POST-CONSTRUCTION TEST: TOTAL LEAKAGE SHALL BE LESS THAN OR EQUAL TO 4 CFM PER 100 SQUARE FEET OF CONDITIONED FLOOR AREA WHEN TESTED AT A PRESSURE DIFFERENTIAL OF 0.1 INCHES W.G. ACROSS THE ENTIRE SYSTEM, INCLUDING THE MANUFACTURER'S AIR HANDLER ENCLOSURE. ALL REGISTER BOOTS SHALL BE TAPED OR OTHERWISE SEALED DURING THE TEST.
- ROUGH-IN TEST: TOTAL LEAKAGE SHALL BE LESS THAN OR EQUAL TO 4 CFM PER 100 SQUARE FEET OF CONDITIONED FLOOR AREA WHEN TESTED AT A PRESSURE DIFFERENTIAL OF 0.1 INCHES W.G. ACROSS THE SYSTEM, INCLUDING THE MANUFACTURER'S AIR HANDLER ENCLOSURE. ALL REGISTERS SHALL BE TAPED OR OTHERWISE SEALED DURING THE TEST. IF THE AIR HANDLER IS NOT INSTALLED AT THE TIME OF THE TEST, TOTAL LEAKAGE SHALL BE LESS THAN OR EQUAL TO 3 CFM PER 100 SQUARE FEET OF CONDITIONED FLOOR AREA.

EXCEPTION: THE TOTAL LEAKAGE TEST IS NOT REQUIRED FOR DUCTS AND AIR HANDLERS LOCATED ENTIRELY WITHIN THE BUILDING THERMAL ENVELOPE.

MF	CHANICAL VENTILATIO	N SYSTEM FAN EFFICA	CY	
FAN LOCATION	AIR FLOW RATE MINIMUM (CFM)	MINIMUM EFFICACY (CFM/WATT)	AIR FLOW RATE MAXIMUM (CFM)	
RANGE HOODS	ANY	2.8	ANY	
IN-LINE FAN	ANY	2.8	ANY	
BATHROOM, UTILITY ROOM	10	1.4	90	
BATHROOM, UTILITY ROOM	90	2.8	ANY	



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SHEET NUMBER

RELEASE FOR
CONSTRUCTION
AS NOTED ON PLANS RE
DEVELOPMENT SERVICE
LEE'S SUMMIT, MISSO

DEVELOPMENT SERV LEE'S SUMMIT, MISSO 07/19/2021

RESIDENTIAL SEISMIC & WIND ANALYSIS

				INPUT
DETERMINE WEIGHT OF HOUSE:				CALCULATED VALUE
LOCATION		DEAD LOAD (psf)	AREA (ft ²)	WEIGHT (lbs.)
ROOF		10	4626	46260
CEILING		10	4626	46260
FIRST FLOOR		10	4626	46260
	WALL LENGTH (ft)	WALL HEIGHT (ft)	WALL UNIT WT. (psf)	WEIGHT (lbs)
FIRST FLOOR EXT. WALL DL	314.66	10	10	31466
		DEAD LOAD (psf)	AREA (ft2)	WEIGHT (lbs)
FIRST FLOOR INT. PARTITION WALL DL		6	4626	27756
,				

PROJECTED AREAS (WIND DESIGN PER 115 MPH 3-SECOND GUST, EXPOSURE C AND MEAN ROOF HEIGHT <= 30 FT ASSUMED)								
	FRONT-TO-BACK				SIDE-TO-SIDE			
	AREA	LOAD			AREA	LOAD		
SLOPED ROOF	613	5215		SLOPED ROOF	777	6496		
VERT. ROOF	112	1392	CUMULATIVE	VERT. ROOF	0	0	CUMULATIVE	
1ST	773.63	9618	16318	1ST	957	11686	18275	
BSMT ^a	0	0	0	BSMT ^a	212	3002	12139	
			PRESSURE (PSF) - PER ASCE CH. 6					
	SLOPED ROOF	ZONE B	9.7		ZONE C	11.3	2a (FIG. 28.6-1, ASCE7)	
	WALL/VERT. ROOF	ZONE A	14.2		ZONE D	7.7	14.066	
	MEAN ROOF HT., h		23					

a) If there is a walkout wall to be sheathed, determine tributary wind area and enter here. If no walkout, enter 0 for area.

 q_{z10_ASD} =0.6 q_{z10} (Design Velocity Pressure for ASD analysis under ASCE7-10 and IRC/IBC 2012)

1ST FLOOR TRIBUTARY WEIGHT BASEMENT TRIBUTARY WEIGHT S_S (SITE GROUND MOTION - %g - FROM ASCE7 SEISMIC MAP) F_a (from ASCE7 Table 11.4-1) S_{DS} (= 2/3 * S_{S} * F_{a})

 q_{z10} =0.00256 $K_z K_{zt} K_d V^2$ (ASCE7-10 Velocity Pressure)

R (from ASCE7 Table 12.2-1)

108253 12.0% 1.6 0.128

		SEISMIC SHEAR		
LOCATION		From	ASCE7 (Eq. 12.8-1):	V (= 1.2 * S _{DS} * W / R) (lbs.)
1ST FLOOR				2558
BASEMENT				2558
01	Mire Observations Ostas deals	Factoria a Oak a data	Allers Isla Olean /////	\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \

Sheathing Location	Min. Sheathing Schedule	Fastening Schedule	Allowable Shear (#/LF)	Code Reference
Exterior (Option #1)	7/16" APA Rated Plywood/OSB	1-1/2" 16ga. Staples w/ 1" penetration@ 6" OC Edges, 6" OC Field For 24" stud spacing, 12" OC Field For 16" stud spacing	155	per IBC, Table 2306.3(1)
Exterior (Option #2)	7/16" APA Rated Plywood/OSB	1-1/2" 16gs. Staples w/ 1" penetration@ 4" OC Edges, 6" OC Field For 24" stud spacing, 12" OC Field For 16" stud spacing	230	per IBC, Table 2306.3(1)
Exterior (Option #3)	7/16" APA Rated Plywood/OSB	1-1/2" 16ga. Staples w/ 1" penetration@ 3" OC Edges, 6" OC Field For 24" stud spacing, 12" OC Field For 16" stud spacing	310	per IBC, Table 2306.3(1)
Exterior (<i>Option #4</i>)	7/16" APA Rated Plywood/OSB or shiplap panel sheathing, or 3/8" shiplap panel sheathing with tighter nail spacing 8d Common Nails w/ 1-3/8" penetration @ 6" O.C. Edges, 12" O.C. Field for 7/16" APA-rated plywood/OSB or shiplap panel sheathing OR @ 4" O.C. Edges, 12" O.C. Field for 3/8" shiplap panel sheathing		220	AF&PA SDPW: Table 4.3A
Exterior (<i>Option #5</i>)	7/16" APA Rated Plywood/OSB or shiplap panel sheathing, or 3/8" shiplap panel sheathing with tighter nail spacing	8d Common Nails w/ 1-3/8" penetration @ 4" O.C. Edges, 12" O.C. Field for 7/16" APA-rated plywood/OSB or shiplap panel sheathing OR @ 3" O.C. Edges, 12" O.C. Field for 3/8" shiplap panel sheathing	320	AF&PA SDPW Table 4.3A
Exterior (Option #6)	7/16" APA Rated Plywood/OSB or shiplap panel sheathing, or 3/8" shiplap panel sheathing with tighter nail spacing and double studs at each panel edge	8d Common Nails w/ 1-3/8" penetration @ 3" O.C. Edges, 12" O.C. Field	410	AF&PA SDPWS Table 4.3A
Interior	1/2" Gypsum Board	No. 6- 1 ¹ / ₄ " Type W or S Screws @ 8" O.C. Edges, 12" O.C. Field	60	per IBC, Table 2306.4.4
Interior	16 Ga. Simpson/USP Type WB Steel X-Brace (or equal)	(3) 16d @ end studs & (1) 8d @ intermediate studs (per manufacturer specifications - see detail on sheet S3)	325	

EXTERIOR SHEATHING OPTION FOR FIRST FLOOR	4	WIDTH OF 1ST STORY (FT.)	70.33
EXTERIOR SHEATHING OPTION FOR BASEMENT WALLS	4	DEPTH OF 1ST STORY (FT.)	87
		BACK WALL OF GARAGE (FT.)	0
		GAR. WALL: 1=F-B, 2=S-S	2

			EXIER	HOR STRUCTURAL WALL	LENGTHS (II.) & RESISTANCES			
		SE	ISMIC		WIND			
	FRONT-TO-BACK	RESISTANCE (lbs.)	SIDE-TO-SIDE	RESISTANCE (lbs.)	FRONT-TO-BACK	RESISTANCE (lbs.)	SIDE-TO-SIDE	RESISTANCE (lbs.)
1ST FLOOR	92	25760	49	13720	92	36064	49	19208
BASEMENT	0	0	33	9240	0	0	33	12936
		ADDITIONAL RESIS	TANCE REQUIRED		Anchor Bolt Spacing (in.)		16d Nail Spacing req'd at bottom plate (in.)	
		SEISMIC	WIND		diameter (in.)	0.5	1st Floor F-B	29
1ST FLOOR FRONT-T	O-BACK	0	0		Shear value (per NDS)	944	1st Floor S-S	21
1ST FLOOR SIDE-TO-	SIDE	0	0		Spacing F-B (inches)	193.3	-	

spacing S-S (inches)

F									
	RESISTANCE REQUIRED IN ADDITION TO RESISTANCE PROVIDED BY EXTERIOR WALLS**								
	ADDITIONAL RESISTANCE REQUIRED (POUNDS)	PORTAL FRAMES OR PERF. SHEAR WALL RESISTANCE	INTERIOR X-BRACES (325#/BRACE)	INTERIOR WALL LENGTH W/ 1/2" GYPSUM BOARD PER TABLE (FT.)	INT. WALL LENGTH SHEATHED W/ OSB (TOTAL LENGTH, ONE SIDE, FT.)	RESISTANCE PROVIDED BY ADDITIONAL METHODS (POUNDS)	OK?		
1ST FLOOR FRONT-TO-BACK	0					0	YES		
1ST FLOOR SIDE-TO-SIDE	0					0	YES		
BASEMENT FRONT-TO-BACK	0					0	YES		
BASEMENT SIDE-TO-SIDE	0					0	YES		

**NOTES: 1) SEE ATTACHED CALCULATIONS FOR PORTAL FRAME OR PERFORATED SHEAR WALL RESISTANCE CAPACITIES (IF APPLICABLE),

2) SEE SHEET S1 FOR INTERIOR STEEL X-BRACE INSTALLATION, 3) INTERIOR WALLS SHEATHED WITH OSB SHALL BE ATTACHED WITH SAME STAPLE/NAILING PATTERN AS EXTERIOR OSB ON SAME FLOOR (SEE TABLE ABOVE) AND ARE ONLY APPLICABLE FOR FULL-HEIGHT SECTIONS OF 2'-8" OR LONGER ALL LATERAL BRACING ACHIEVED AT EXTERIOR WALLS AND WALLS DIRECTLY ON FOUNDATIONS; THEREFORE, NO INTERIOR BRACING PER 2012 IRC SECTION R502.2.1 IS REQUIRED

WIND UPLIFT ANALYSIS									
	X/12	DEGREES			_				
ROOF PITCH (MAX)	8	33.7	PITCH OF 6 OR LESS:	EOH -13.3, E -7.2, G -5.2					
		ASCE 7			_				
	LENGTH (FT.)	PRESSURE (PSF)	LINEAL FT. OF OH	UPLIFT PER FT* (LBS)					
OVERHANG	1	-1.08	316.66	-1.08					
	TOTAL AREA (FT ²)	ZONE E AREA (FT ²)	ZONE G AREA (FT ²)	PRESSURE ZN. E (PSF)	PRESSURE ZN. G (PSF)	TOTAL FORCE (LBS)	FORCE PER LINEAL FT @ PERIMETER (LBS)		
MAIN ROOF**	6118.71	-735.145424	6853.855424	-1.08	-0.36	-1673	-5.3		
*ALONG PERIMETER		TOTAL UPLIFT PER LINEAL	FOOT ALONG EXTERIOR (PO	DUNDS)	-6.4	UPLIFT OK			
**INSIDE EXTERIOR WALLS RESISTANCE DUE TO DEAD WEIGHT & (3) 10d TOENAILS					251.6				

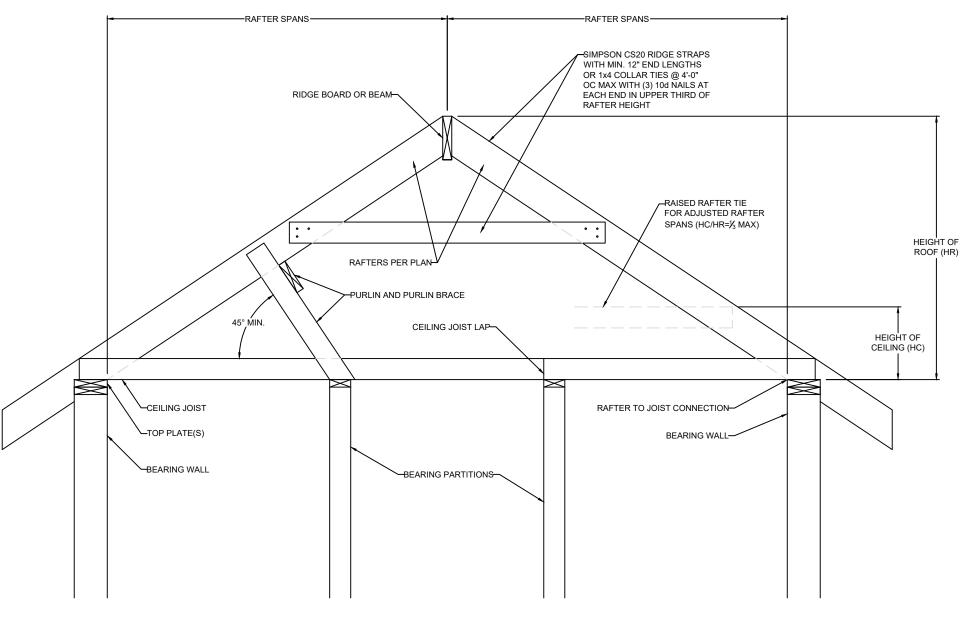
BASEMENT FRONT-TO-BACK

BASEMENT SIDE-TO-SIDE

THE CONTINUOUS STRUCTURAL PANEL SHEATHING BRACING METHOD REQUIRES USE OF THE ABOVE TABLE FOR SHEATHING OF THE ENTIRE STRUCTURE. IN ADDITION, FRAMING MEMBERS SHALL BE @ 16" O.C. MAX., UNBLOCKED, AND W/ SHEATHING APPLIED DIRECTLY TO FRAMING MEMBERS

ALL WALLS USED IN THE CALCULATION OF THE RESISTANCE FOR THIS STRUCTURE SHALL HAVE A MINIMUM UNINTERRUPTED HEIGHT OF 8'-0" AND LENGTH OF 2'-8". ALLOWABLE RESISTANCES HAVE BEEN #/FT AND INCREASED BY 40% FOR WIND LOADS, PER VALUES IN 2012 IBC SECTION 2306 AND AF&PA SDPWS TABLE 4.3A. FOR EXAMPLE, 7/16" APA-RATED SHEATHING WITH 8d @ 6" & 12" HAS A SEISMIC SHEAR VALUE OF 240 A WIND SHEAR VALUE OF 335#/FT - 40% GREATER THAN THAT OF SEISMIC)

NOTE: SOIL SITE CLASS ASSUMED TO BE CLASS D. IF SITE CONDITIONS ARE DETERMINED TO BE CLASS E OR F, CONSULT ENGINEER BEFORE PROCEEDING



Combustion Air Calculation Per 2012 IRC Section G2407.5

100000 BTU/h Appliance #1 Furnace 100000 BTU/h Appliance #2 Furnace 50000 BTU/h Appliance #3 Water Heater 50000 BTU/h Appliance #4 Water Heater Total BTU/hr 300000 BTU/h

Area of Combined Space (floor where appliances are located) 2939 ft² Ceiling Height in Usable Space

Note: Per 2012 IRC Section G2407.5.3.2, The volumes of spaces in different stories shall be considered as communicating spaces where such spaces are connected by one or more openings in doors or floors having a total minimum free area of 2 square inches per 1,000 BTU/h of total input rating of all appliances

Is floor where appliances are located open to adjacent level? If Yes, what is the area of open space adjacent to appliance area?

Per 2012 IRC Section G2407.5.1 (Standard Method), the minimum required volume shall be 50 cubic feet per 1,000 BTU/hr (Total BTU/hr / 1,000 BTU/hr x 50 ft³)

Area of Combined Space > Required combined area?

15000 ft³ Required air space in combined areas:

1765 ft² Required combined area:

Per Section G2407.5.3.1, each opening shall have a minimum free area of 1 square inch per 1,000 BTU/hr of the total input rating of all

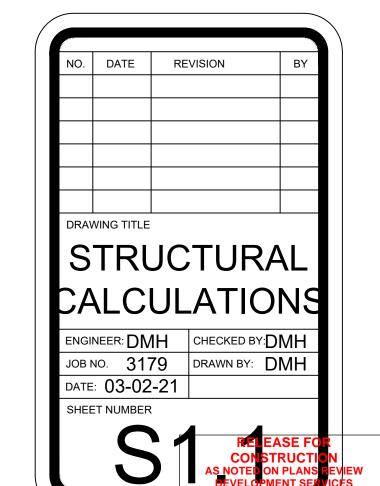
appliances in the space, but not less than 100 square inches. One opening shall commence within 12 inches of the top and one opening shall commence within 12 inches of the bottom of the enclosure. The minimum dimension of air openings shall be not less than 3 inches.

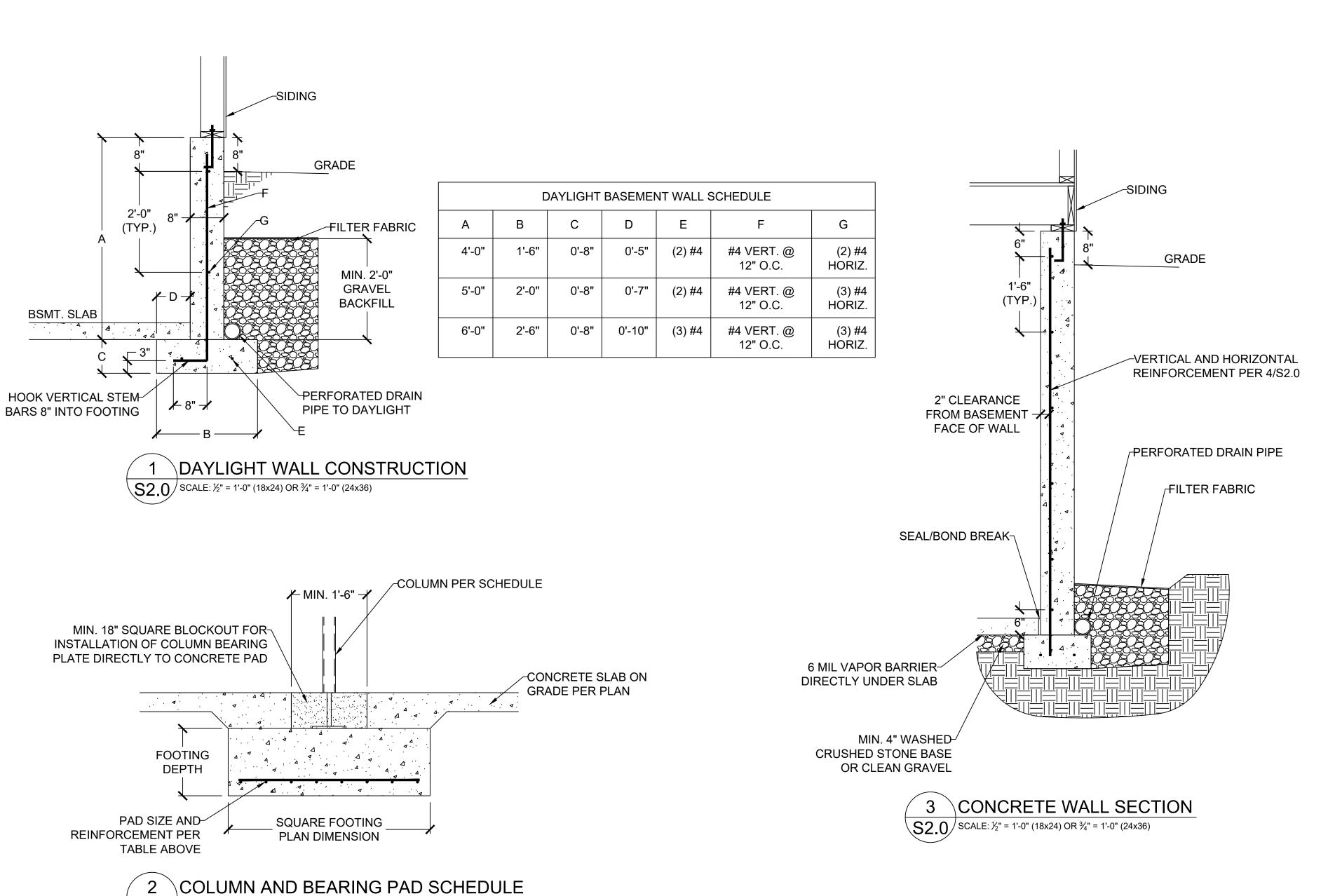
300 in² Minmum required opening area: Minimum grill size: 14 x 22 (inches)

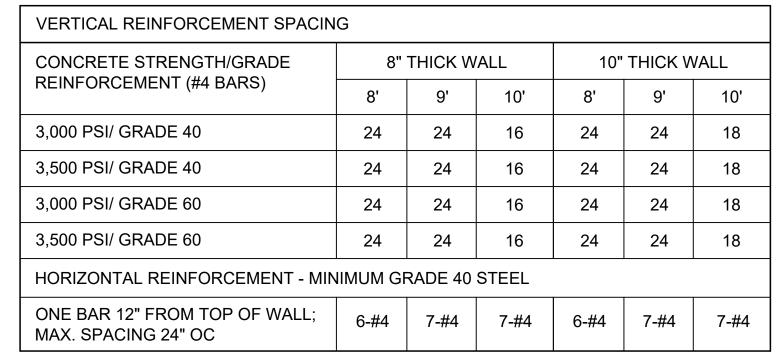
Note: two grills required - one within 12" of floor, one within 12" of clg. Alternate: install two 14"x12" grills in TWO locations









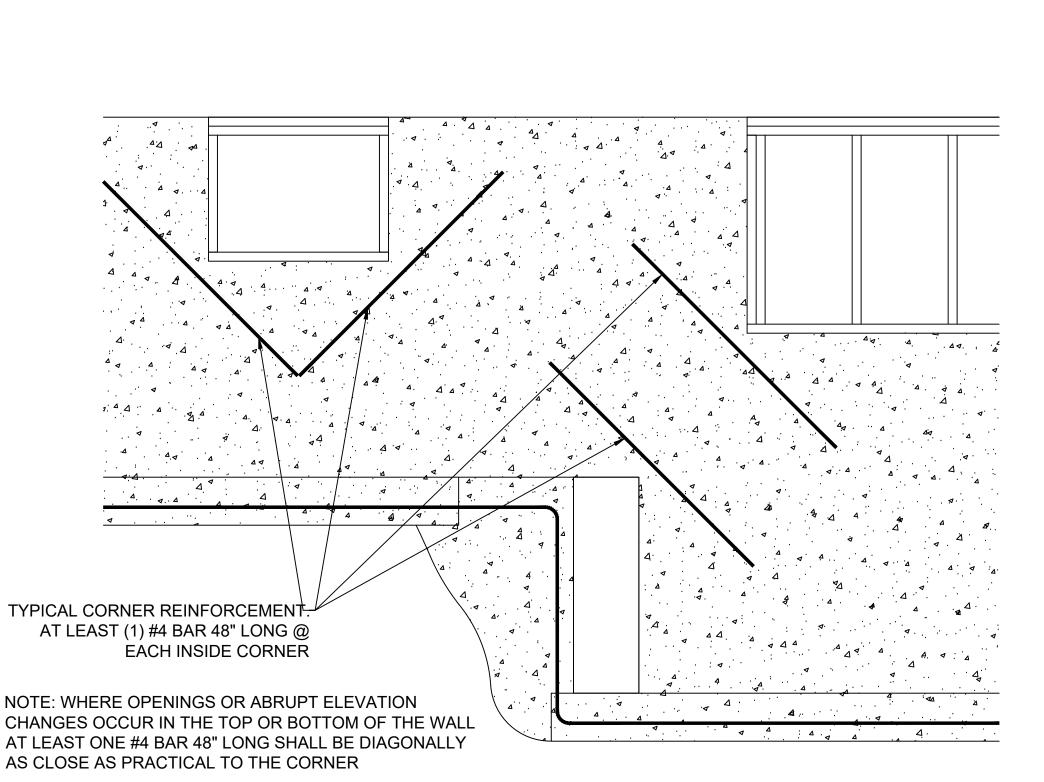


FOOTNOTES:

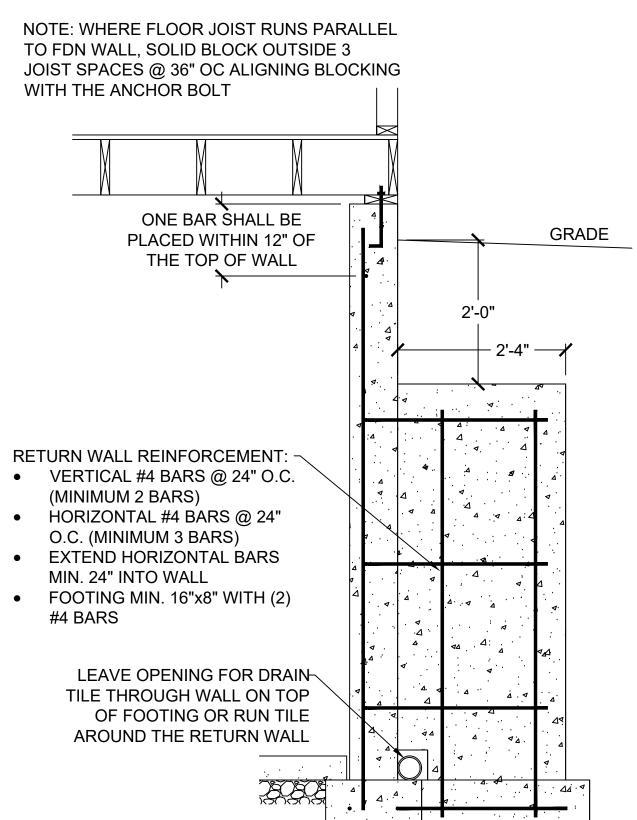
1) WALL HEIGHT IS MEASURED FROM THE TOP OF THE WALL TO THE TOP OF THE FLOOR SLAB 2) VERTICAL REINFORCEMENT FOR CONCRETE WALLS THAT ARE NOT FULL HEIGHT, AND FOR REINFORCEMENT SPACING 24" OC, REINFORCEMENT MAY BE PLACED IN THE MIDDLE OF THE WALL. OTHER WALLS SHALL HAVE VERTICAL REINFORCEMENT AS FOLLOWS:

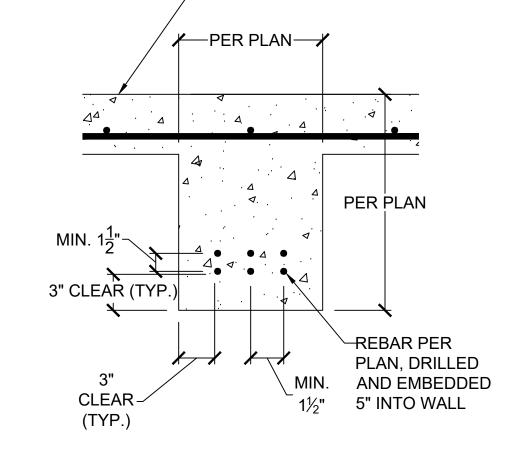
- A) 8" WALL MINIMUM 5" FROM THE OUTSIDE FACE
- B) 10" WALL MINIMUM 63/4" FROM THE OUTSIDE FACE C) EXTEND BARS TO WITHIN 8" OF THE TOP OF THE WALL
- 3) REINFORCEMENT CLEARANCES:
 - A) CONCRETE EXPOSED TO EARTH MINIMUM 11/2"
 - B) NOT EXPOSED TO WEATHER (INTERIOR SIDE OF WALLS) -3/4" C) CONCRETE EXPOSED TO WEATHER (TOP CLEARANCE IN GARAGE AND DRIVEWAY
- SLABS) 1½"
- 4) HORIZONTAL REINFORCEMENT:
- A) ONE BAR SHALL BE PLACED WITHIN 12" OF THE TOP OF THE WALL
- B) OTHER BARS SHALL BE EQUALLY SPACED WITH SPACING NOT TO EXCEED 24" OC C) HORIZONTAL BARS SHOULD BE AS CLOSE TO THE TENSION FACE AS POSSIBLE (INTERIOR) AND BEHIND THE VERTICAL REINFORCEMENT (I.E. 2" TOWARD THE
- D) SUPPLEMENTAL REINFORCEMENT AT CORNERS PLACE (1) #4 BAR 48" LONG AT 45 DEGREE ANGLE AT CORNERS OF OPENINGS. PLACE REINFORCEMENT WITHIN 6" OF THE EDGE OF INSIDE CORNERS.
- 5) REINFORCEMENT SHALL BE LAPPED A MINIMUM 24" AT ENDS, SPLICES, AND AROUND CORNERS.
- 6) AT MASONRY LEDGES THE MINIMUM WALL THICKNESS SHALL BE 3½". LEDGES SHALL NOT EXCEED A DEPTH OF MORE THAN 24" BELOW THE TOP OF THE WALL. FOR WALL THICKNESSES LESS THAN 4" PROVIDE #4 BARS AT MAX. 24" OC TO WITHIN 8" OF THE TOP
- 7) STRAIGHT WALLS MORE THAN 5' TALL AND MORE THAN 16 FEET LONG SHALL BE PROVIDED WITH EXTERIOR BRACED RETURN WALLS. WALL LENGTH SHALL BE MEASURED USING INSIDE THE SHORTEST DIMENSION BETWEEN INTERSECTING WALLS
- WALL SHALL NOT BE BACKFILLED UNTIL FLOOR SYSTEM AND DIAPHRAGM ARE IN PLACE

4 \FOUNDATION WALL REINFORCEMENT TABLE S2.0 NO SCALE



6 REINFORCEMENT AT OPENING CORNERS S2.0/AND STEP CORNERS @ INSIDE CORNERS SCALE: ½" = 1'-0" (18x24) OR ¾" = 1'-0" (24x36)





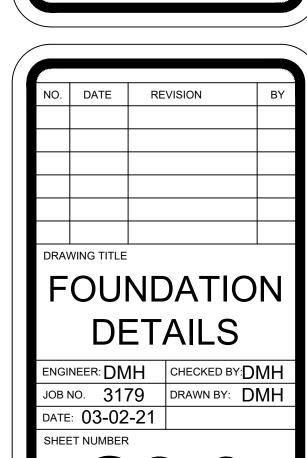
-SLAB PER PLAN,

IF APPLICABLE

CONCRETE GRADE BEAM SCALE: 1" = 1'-0" (18x24) OR 1½" = 1'-0" (24x36)

RETURN WALL DETAIL S2.0 SCALE: $\frac{1}{2}$ " = 1'-0" (18x24) OR $\frac{3}{4}$ " = 1'-0" (24x36)





5 SOLID JUMP S2.0 SCALE: $\frac{1}{2}$ " = 1'-0" (18x24) OR $\frac{3}{4}$ " = 1'-0" (24x36)

-CONTINUOUS FOOTING

AND REBAR THROUGH

6'-0" MAX.

SOLID JUMP

MAX. 12" BLOCKOUT FOR

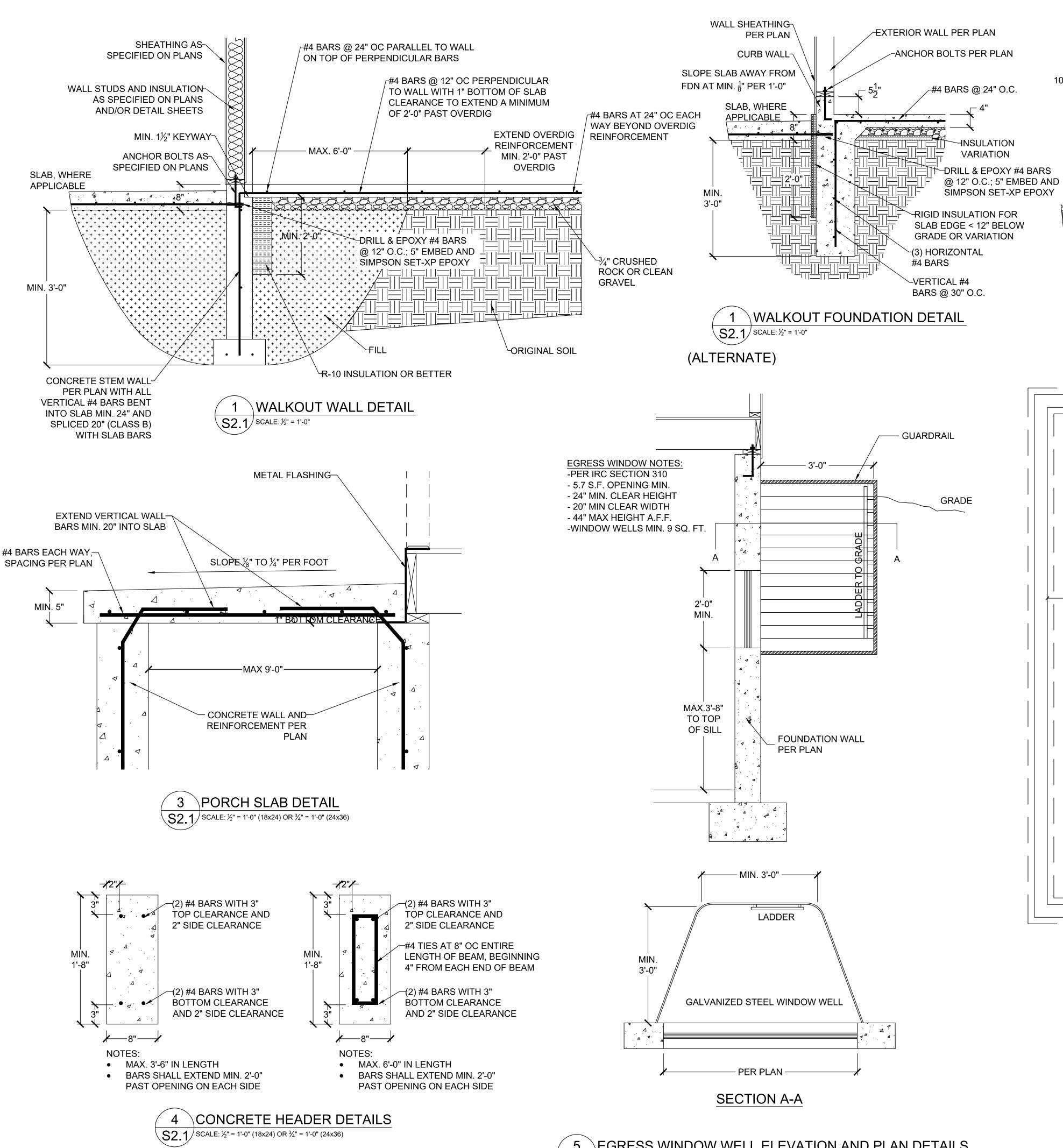
FORM PLACEMENT AND

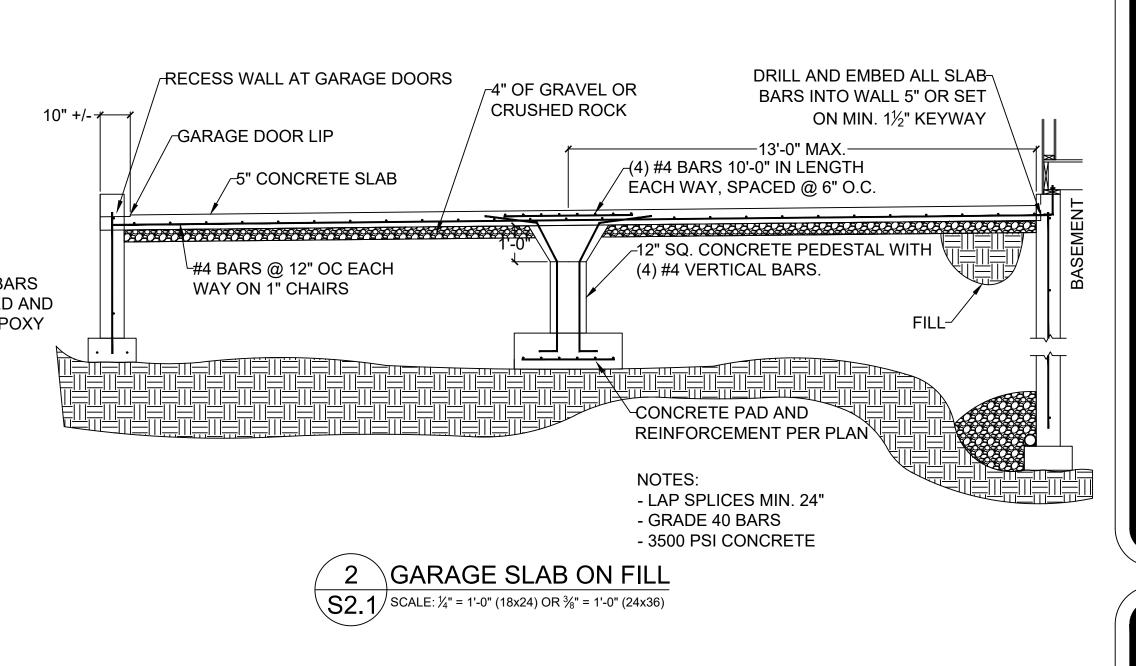
TO EXTEND DRAIN TILE

SCALE: $\frac{1}{2}$ " = 1'-0" (18x24) OR $\frac{3}{4}$ " = 1'-0" (24x36)

/MIN. (2) #4 BARS EXTENDING 24"

PAST OVER-EXCAVATION AND INTO INTERSECTING WALL





• 3500 PSI CONCRETE • 24" CLASS B LAP SPLICES SLAB REINFORCEMENT SHALL BE PLACED ON CHAIRS 5" BOTTOM REINFORCEMENT SHALL BE EMBEDDED 4" INTO WALLS GRADE 40 REINFORCEMENT PER PLAN -PER PLAN--PER PLAN- \neg (4) #4 BARS EACH WAY WITH $\frac{3}{4}$ " TOP CLEARANCE SPACED AT 6" O.C. AND CENTERED DIRECTLY ABOVE CONCRETE PAD/PEDESTAL PER PLAN ~5" CONCRETE SLAB WITH #4 BARS AT 12" O.C. EACH WAY WITH 3/4" BOTTOM CLEARANCE



WR132 TOTTA
OT 132, RESERVE AT WOODSIDE RIDGE
F'S SUMMIT MISSOURI

JOB TITLE:

COCATION:

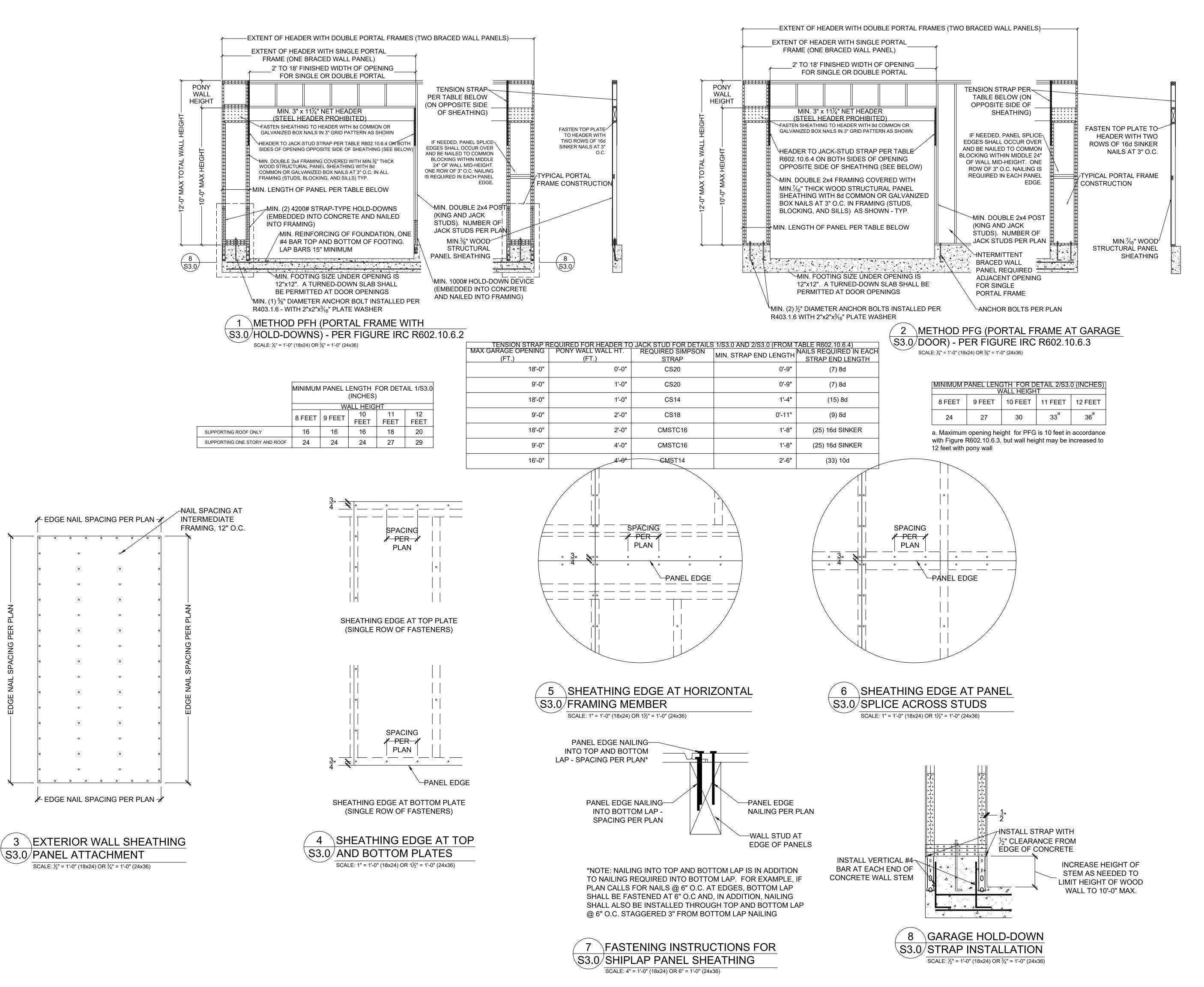
COCATION:



DRAWING TITLE
FOUNDATION
DETAILS

ENGINEER: DMH
JOB NO. 3179
DATE: 03-02-21

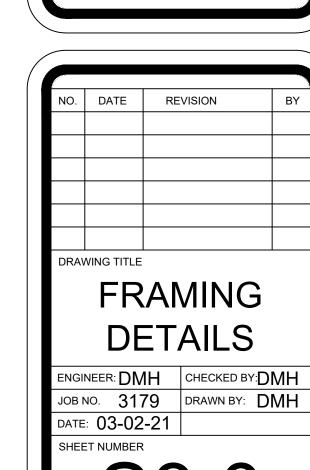
5 EGRESS WINDOW WELL ELEVATION AND PLAN DETAILS SCALE: ½" = 1'-0" (18x24) OR ¾" = 1'-0" (24x36)

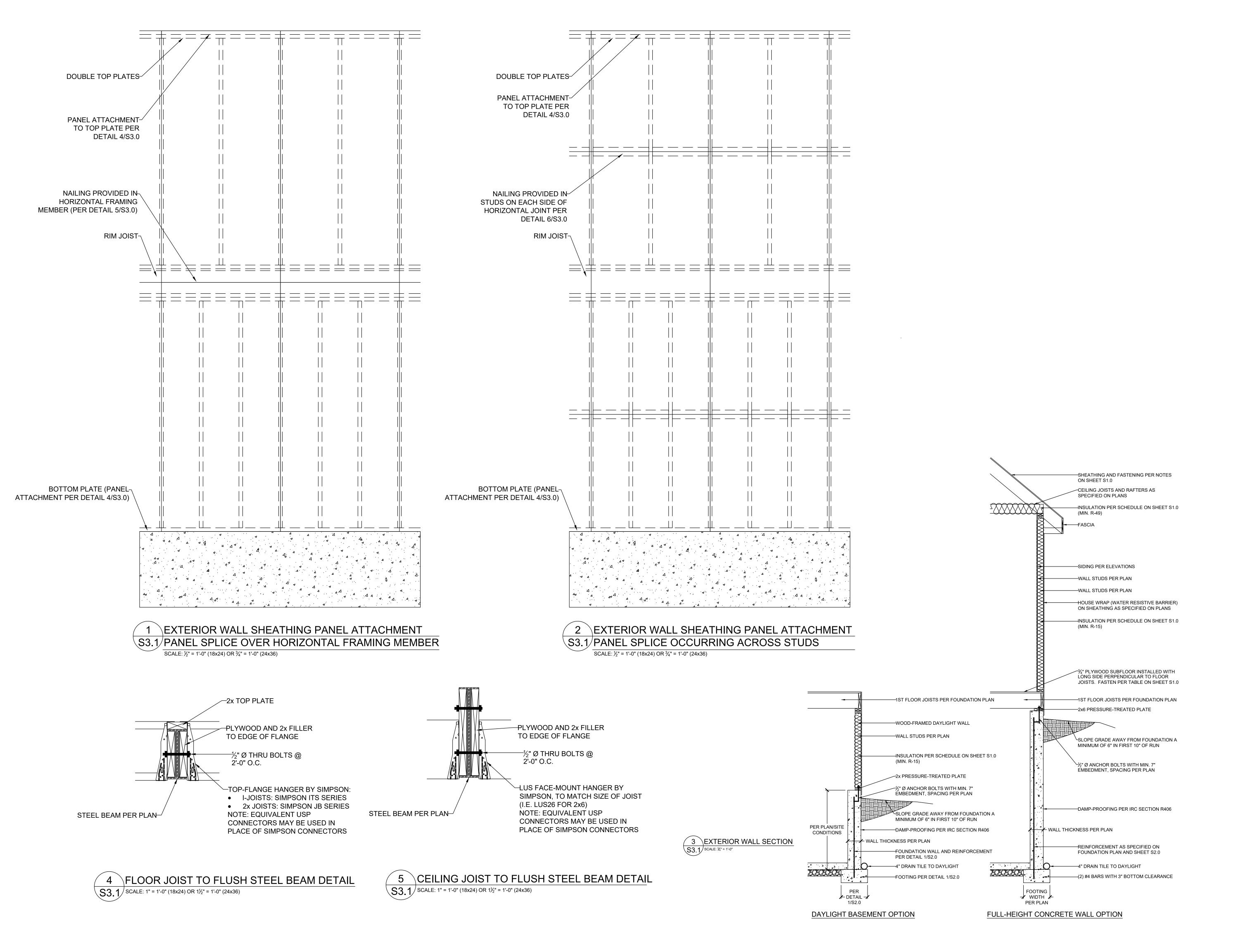




WR132 TOTTA OT 132, RESERVE AT WOODSIDE RIDGE









E: RWR132 TOTTA LOT 132, RESERVE AT WOODSID

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NUMBER

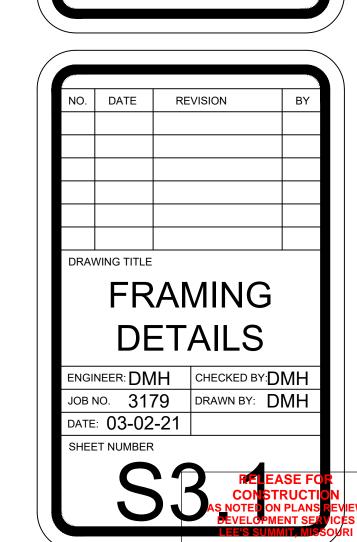
PE-2010001772

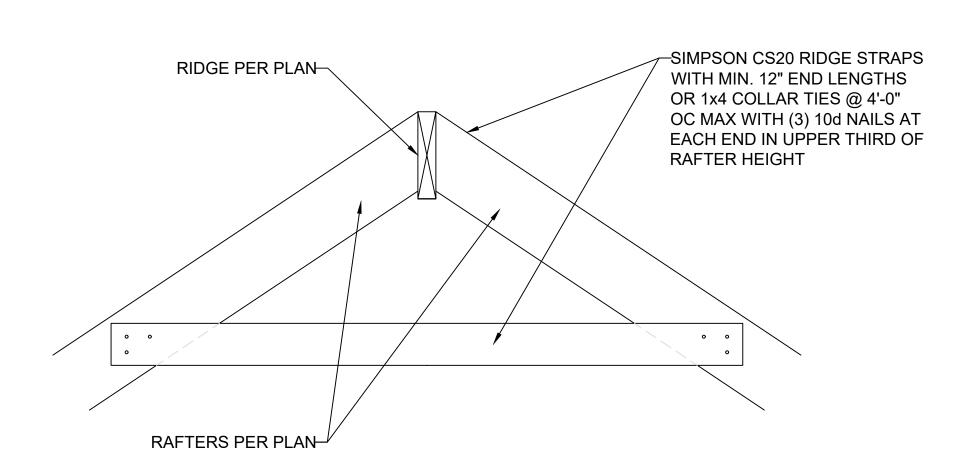
PE-2010001772

PE-2010001772

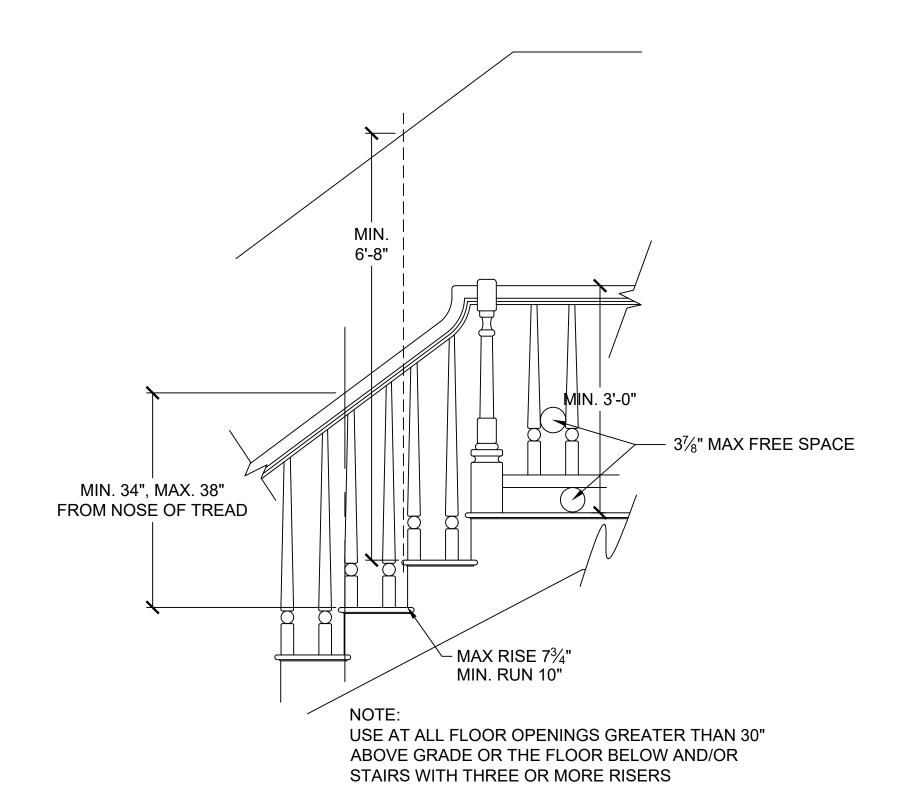
PE-2010001772

PE-2010001772

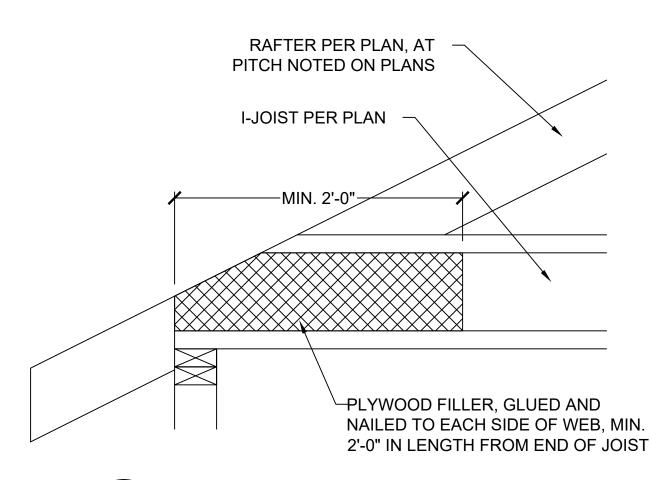




RIDGE FRAMING DETAIL S3.2 SCALE: 1" = 1'-0" (18x24) OR $1\frac{1}{2}$ " = 1'-0" (24x36)

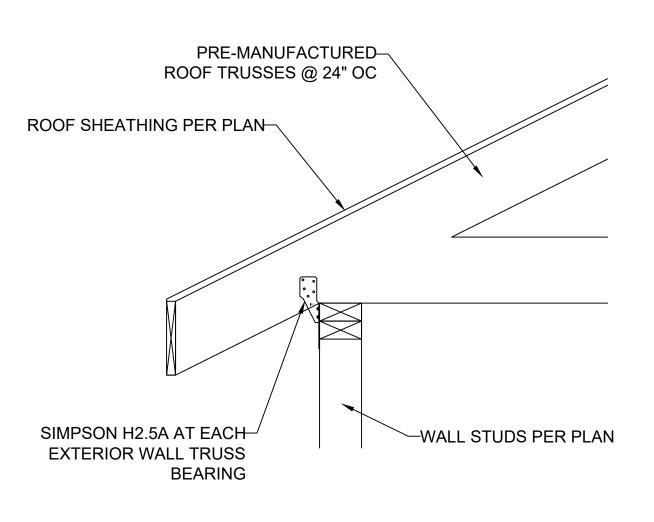


4 \STAIR AND HANDRAIL/GUARDRAIL DETAIL SCALE: ½" = 1'-0" (18x24) OR ¾" = 1'-0" (24x36)

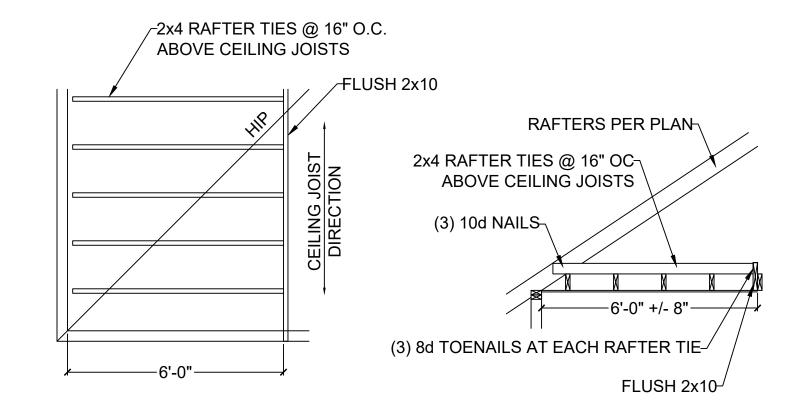


9 COPED I-JOIST REINFORCEMENT

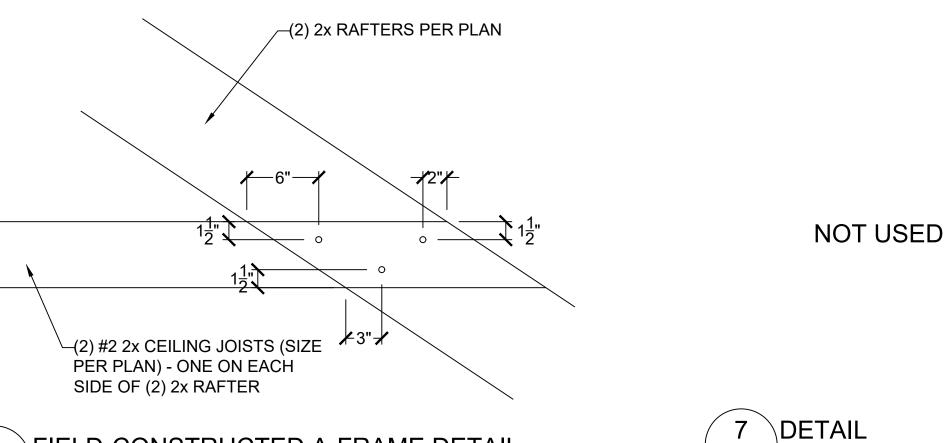
S3.2 SCALE: 1" = 1'-0" (18x24) OR $1\frac{1}{2}$ " = 1'-0" (24x36)



TRUSS CONNECTION TO EXT. WALL BEARING SCALE: 1" = 1'-0" (18x24) OR 1½" = 1'-0" (24x36)



5 RAFTER TIES AT CEILING JOISTS PERP. TO RAFTERS SCALE: $\frac{1}{4}$ " = 1'-0" (18x24) OR $\frac{3}{6}$ " = 1'-0" (24x36)



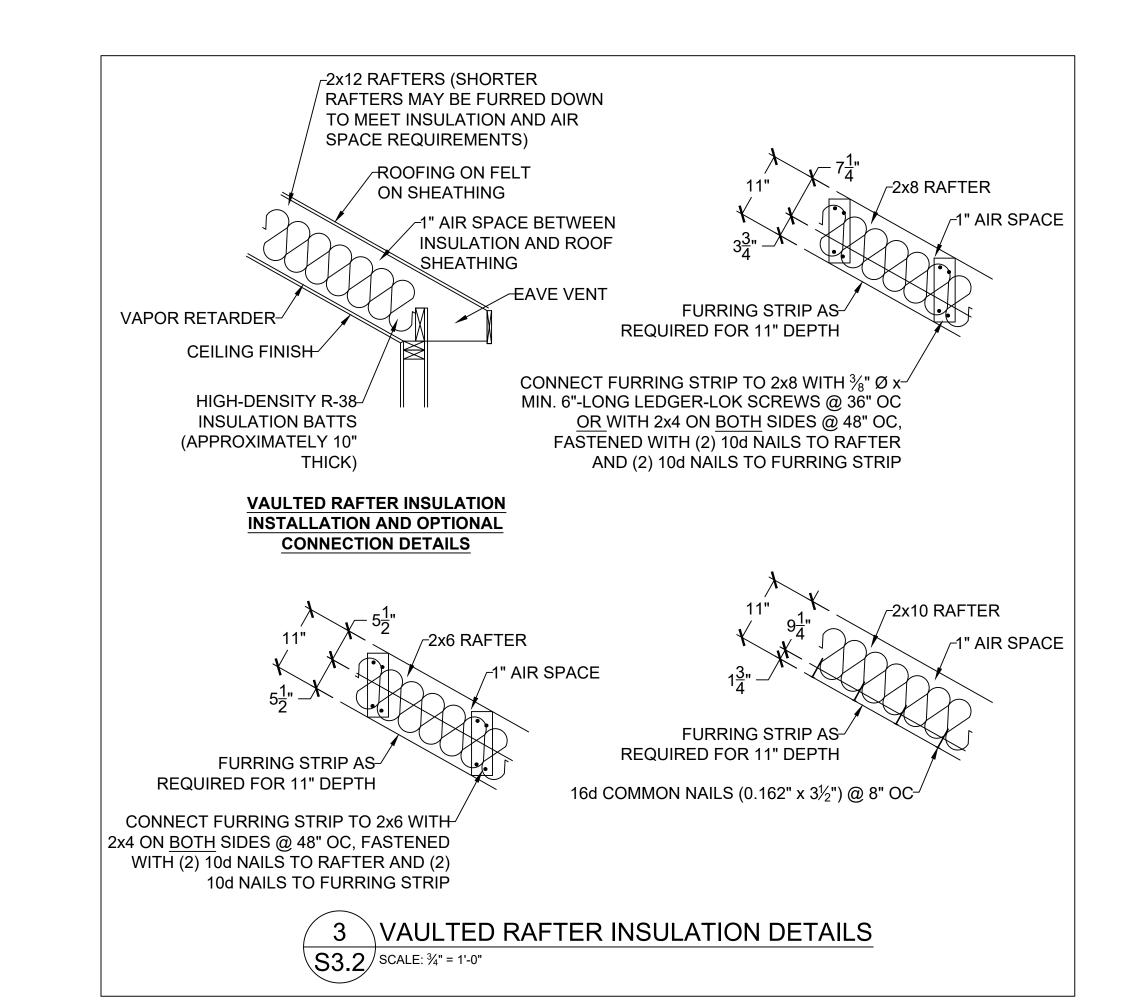
6 \FIELD-CONSTRUCTED A-FRAME DETAIL S3.2 | SCALE: 1" = 1'-0" (18x24) OR 1½" = 1'-0" (24x36)

> ∕─SIMPSON CS16 STRAP SIMPSON-CS16 STRAP -(11) 8d NAILS IN EACH END LENGTH HEADER/BEAM PER PLAN

END OF FLOOR OR **CEILING JOIST** HEADER/BEAM PER PLAN END LENGTH END LENGTH (11) 8d NAILS IN **EACH END LENGTH** BEARING WALL

S3.2 SCALE: 1" = 1'-0" (18x24) OR $1\frac{1}{2}$ " = 1'-0" (24x36)

END LENGTH END LENGTH └─(11) 8d NAILS IN EACH END LENGTH BEARING WALL BEARING WALL



	SPACING (INCHES O.C.)			
HEIGHT (FT.)	24	16	12	8
	SUPPORT	ING A ROOF	ONLY	
10 OR LESS	2x4	2x4	2x4	2x4
12	2x6	2x4	2x4	2x4
14	2x6	2x6	2x6	2x4
16	2x6	2x6	2x6	2x4
18	DR	2x6	2x6	2x6
20	DR	DR	2x6	2x6
SUPPORTING ONE FLOOR AND A ROOF				
10 OR LESS	2x6	2x4	2x4	2x4
12	2x6	2x6	2x6	2x4
14	2x6	2x6	2x6	2x6
16	DR	2x6	2x6	2x6
18	DR	2x6	2x6	2x6
20	DR	DR	2x6	2x6
SUPPO	ORTING TV	VO FLOORS	AND A ROC)F
10 OR LESS	2x6	2x6	2x4	2x4
12	2x6	2x6	2x6	2x6
14	2x6	2x6	2x6	2x6
16	DR	2x6	2x6	2x6
18	DR	DR	2x6	2x6
20	DR	DR	DR	2x6

NOTES:
1) DR = DESIGN REQUIRED
2) UTILITY, STANDARD, STUD AND #3 GRADE LUMBER OF
ANY SPECIES ARE NOT PERMITTED
3) THIS TABLE DOES NOT APPLY FOR STUDS
SUPPORTING MEMBERS WITH A TRIB. LENGTH GREATER
THAN 6'-0"

8 MAXIMUM ALLOWABLE LENGTH OF S3.2/WOOD WALL STUDS (IRC TABLE 602.3.1)



RIDGE



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JOB I	NO. 31	79	DRAWN BY:	DMF	1
DATE	: 03-02	2-21			

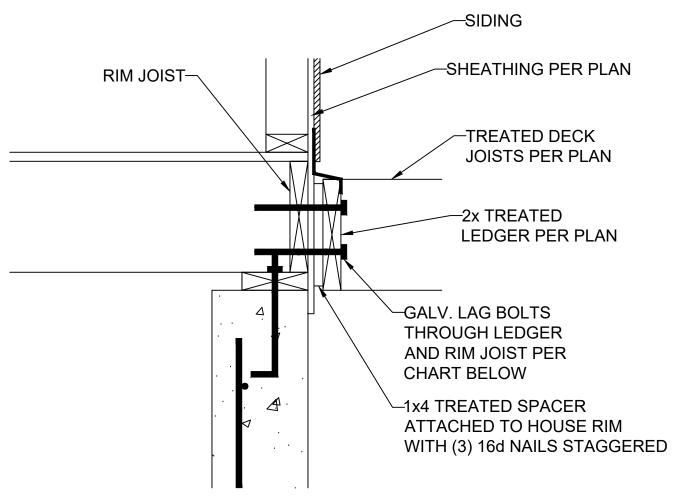
10 \HEADER/BEAM CONNECTION OPTIONS AT OUTDOOR/OPEN SPACE

SCALE: 1" = 1'-0" (18x24) OR 1½" = 1'-0" (24x36)

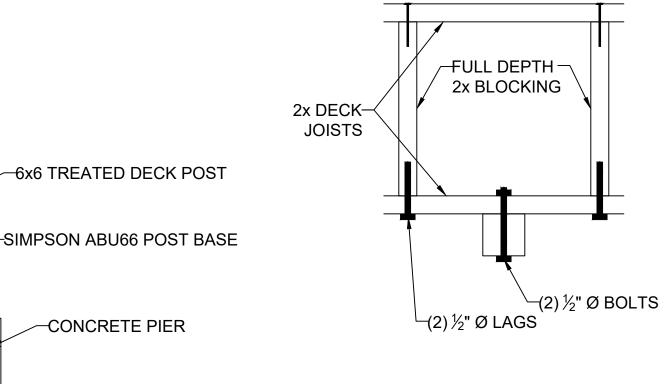
HEADER/BEAM

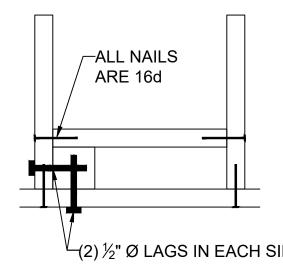
PER PLAN

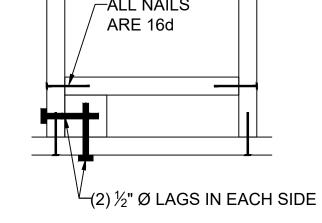
SIMPSON-CS16 STRAP



RIM JOIST-SIMPSON LS90 GUSSET ANGLES, NAILED TO BOTH-SIDES OF DOUBLED JOISTS W/ (12) N10 NAILS -SIDING FLOOR JOISTS PER PLAN. IF JOISTS ARE-NOT DOUBLED, INSTALL ADDITIONAL PLY SHEATHING PER PLAN TO EACH JOIST, MIN. 6'-0" IN LENGTH TREATED DECK JOISTS PER PLAN (MAX. SPAN = 16'-0") —2x TREATED LEDGER PER PLAN FASTEN DECK JOISTS TO LEDGER WITH SIMPSON LUS210 HANGERS GALV. LAG BOLTS _CANTILEVER LENGTH_ THROUGH LEDGER AND RIM PER PLAN JOIST PER CHART BELOW SIMPSON H2.5A AT EACH-**CANTILEVERED JOIST** -1x4 TREATED SPACER WHICH SUPPORTS DECK ATTACHED TO HOUSE RIM



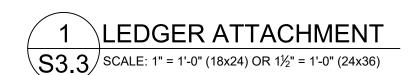


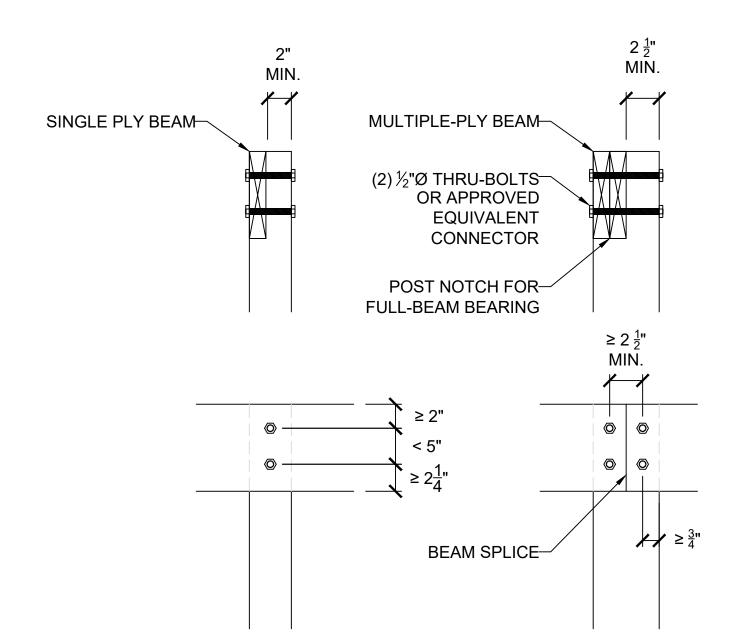




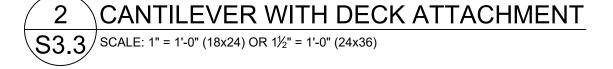
DECK LEDGER ATTACHMENT GUIDE

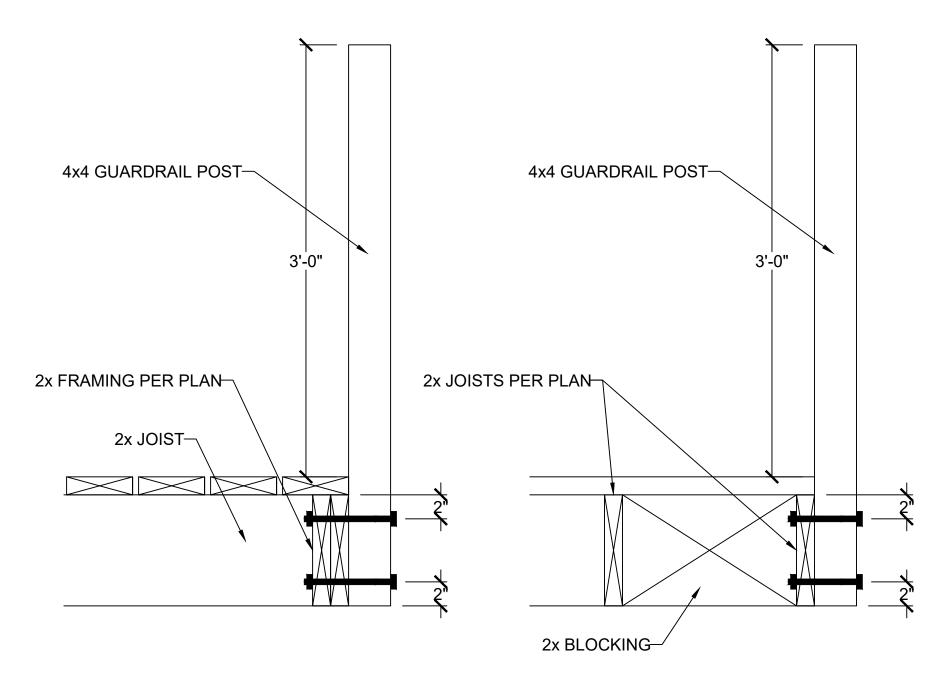
DECK JOIST SPAN	$\frac{1}{2}$ " Ø GALV. LAG OR $\frac{3}{8}$ " Ø LEDGER-LOK SPACING
10'-0" OR LESS	16" OC
10'-0" - 13'-11"	12" OC OR @ 16" OC DOUBLED EVERY OTHER
14'-0" - 18'-0"	8" OC OR @ 16" OC DOUBLED



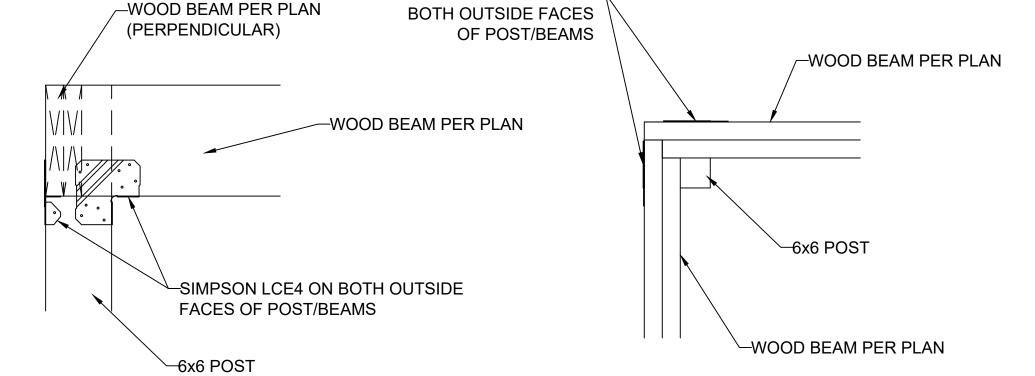


S3.3 SCALE: 1" = 1'-0" (18x24) OR 1½" = 1'-0" (24x36)





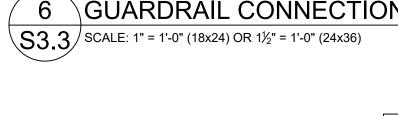
WITH (3) 16d NAILS STAGGERED

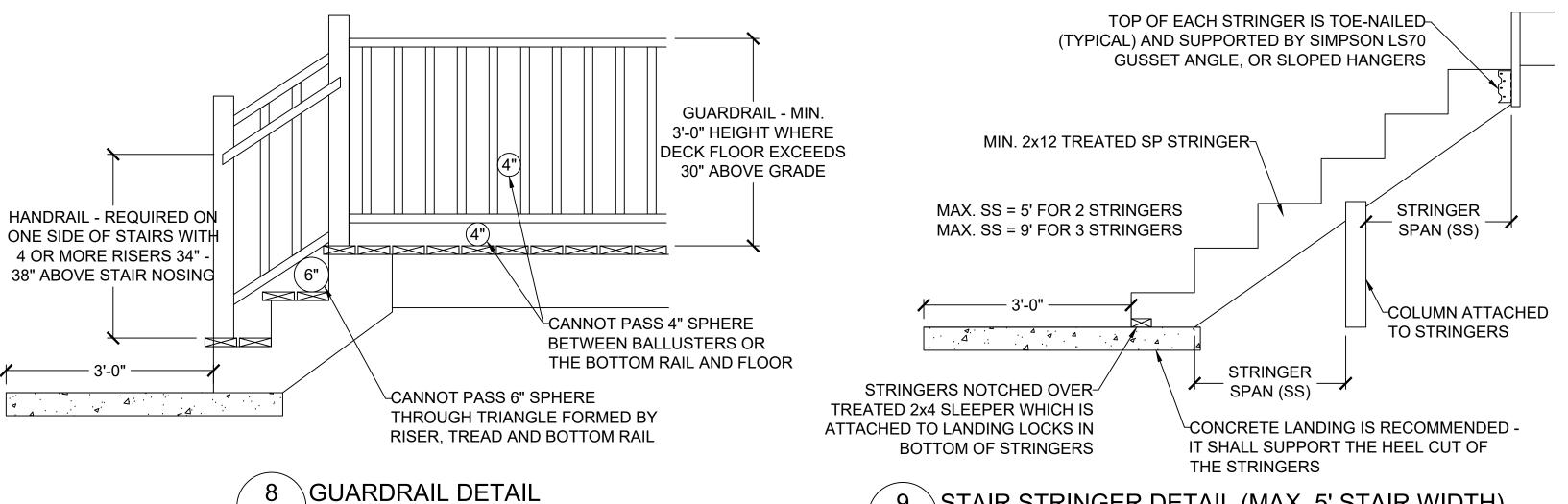


SIMPSON LCE4 ON-

ALTERNATE COVERED DECK/PORCH INTERSECTION S3.3/CORNER BEAM CONNECTION SCALE: 1" = 1'-0" (18x24) OR 1½" = 1'-0" (24x36)

6 \GUARDRAIL CONNECTION

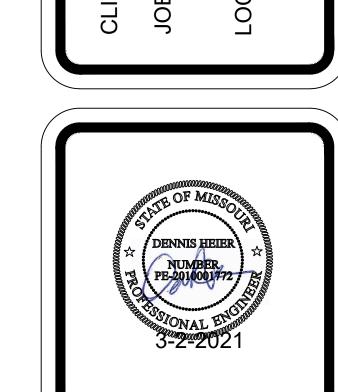




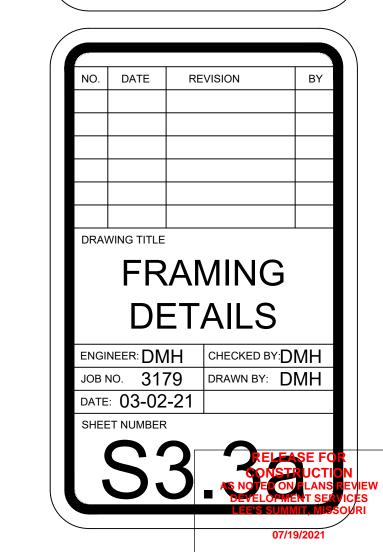
3 DECK POST BASE

S3.3 SCALE: 1" = 1'-0" (18x24) OR $1\frac{1}{2}$ " = 1'-0" (24x36)

9 \STAIR STRINGER DETAIL (MAX. 5' STAIR WIDTH) 53.3 SCALE: $\frac{1}{2}$ " = 1'-0" (18x24) OR $\frac{3}{4}$ " = 1'-0" (24x36) SCALE: $\frac{1}{2}$ " = 1'-0" (18x24) OR $\frac{3}{4}$ " = 1'-0" (24x36)

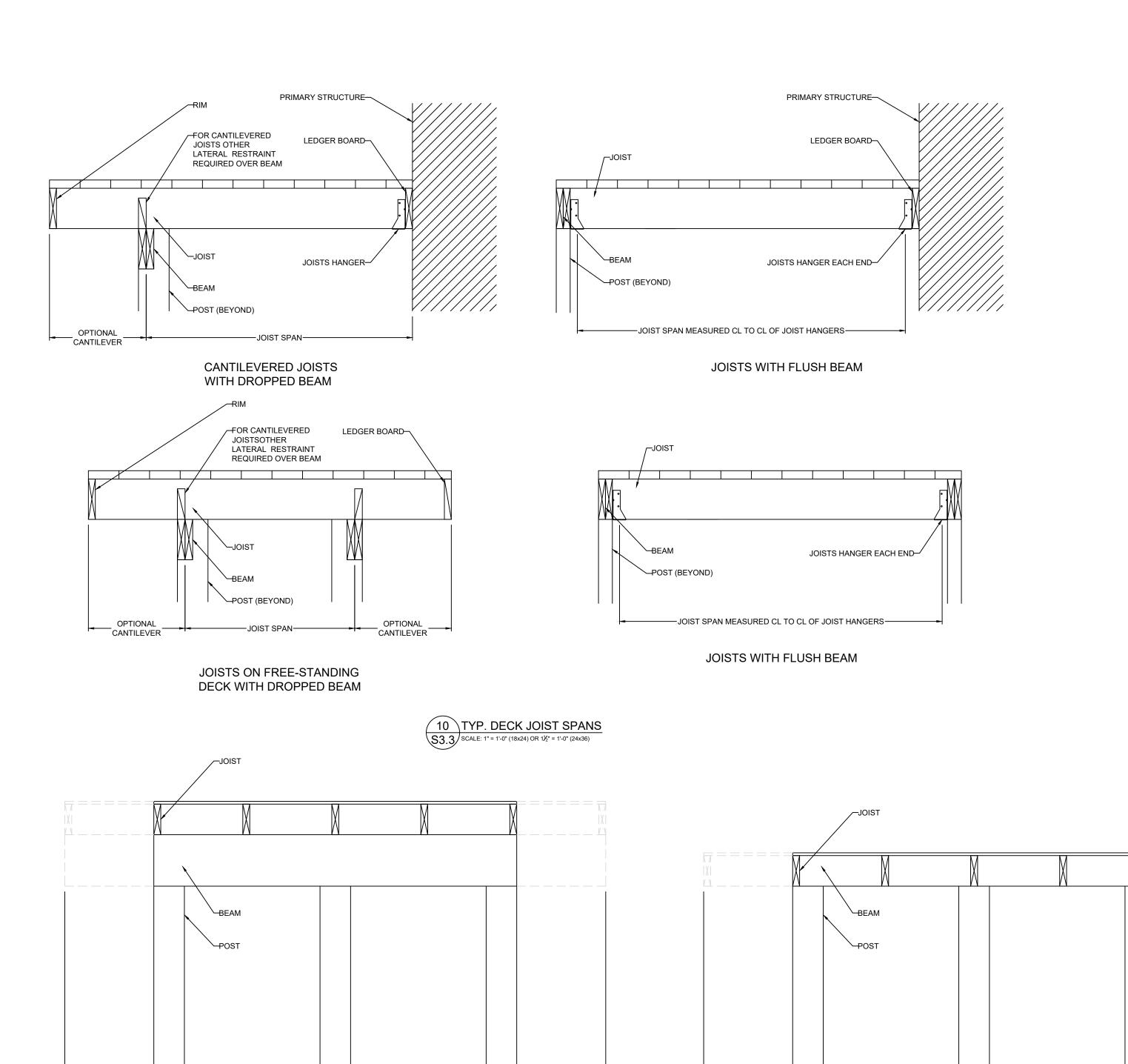


RIDGE



5 \LET-IN (COVERED) DECK BEAM CONNECTION





DROPPED BEAM FLUSH BEAM

___OPTIONAL __ CANTILEVER OPTIONAL CANTILEVER



__OPTIONAL ___ CANTILEVER

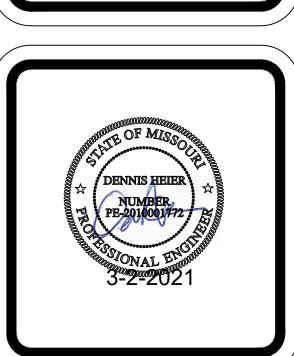
—BEAM SPAN—

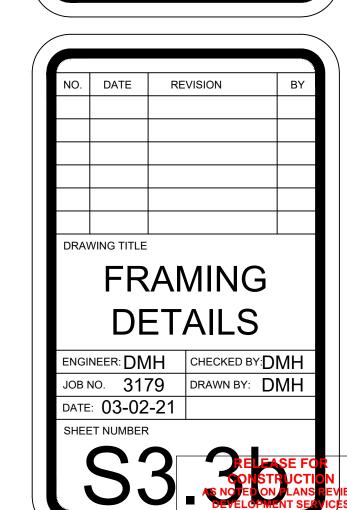
OPTIONAL ____CANTILEVER



WOODSIDE RIDGE

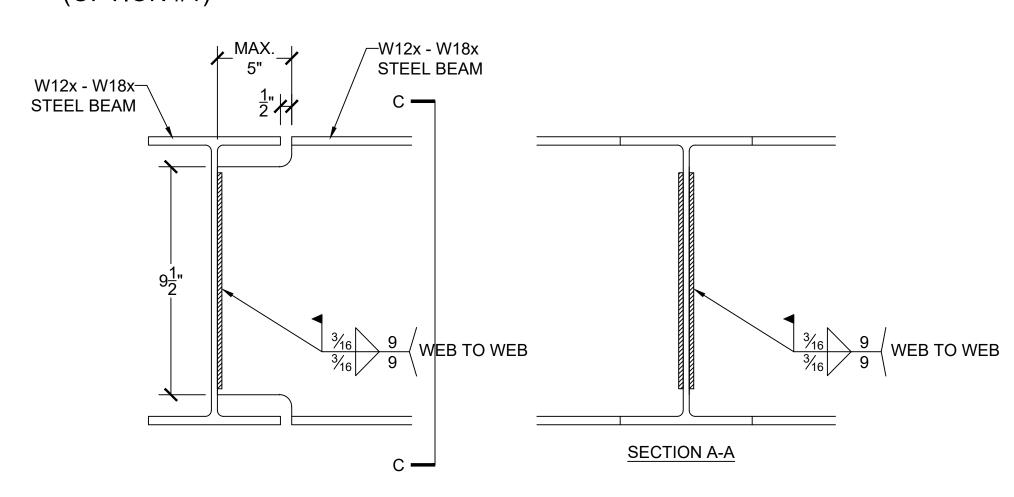
E: RWR132 TOTTA LOT 132, RESERVE AT





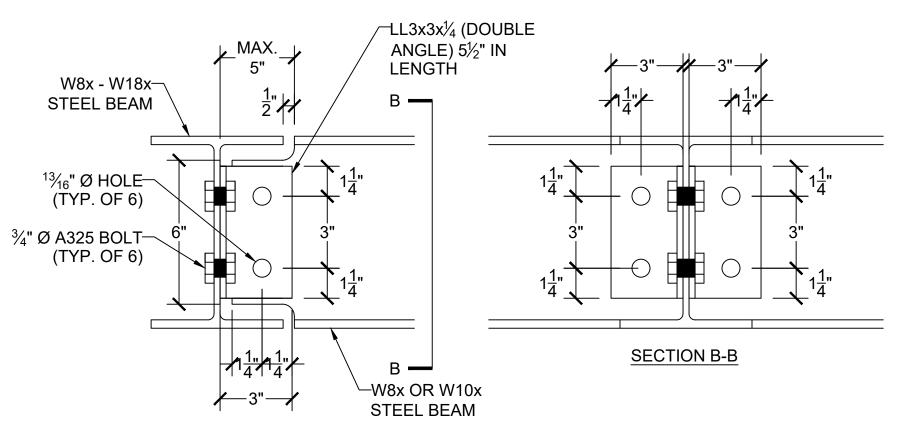
1 WELDED T-BEAM CONNECTION FOR W8x AND W10x BEAMS SCALE: 2" = 1'-0" (18x24) OR 3" = 1'-0" (24x36)

(OPTION #1)



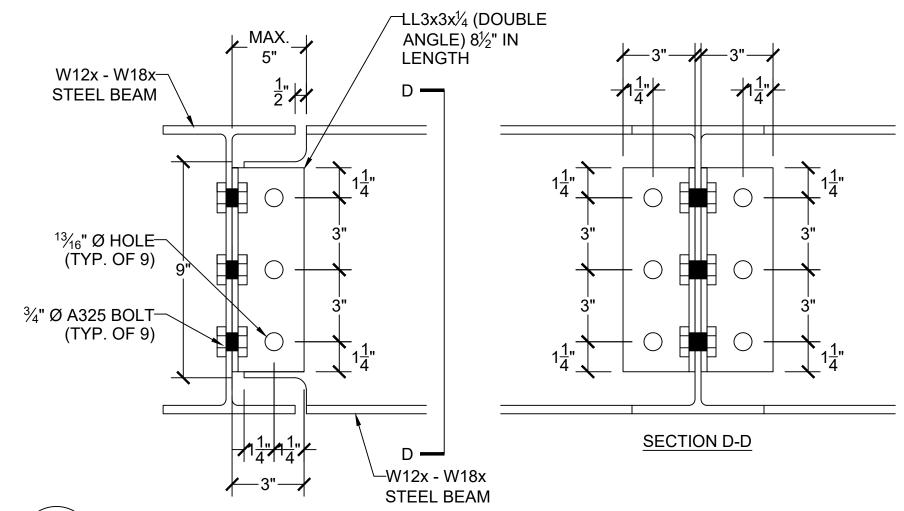
2 WELDED T-BEAM CONNECTION FOR W12x, W14x, W16x & W18x BEAMS SCALE: 2" = 1'-0" (18x24) OR 3" = 1'-0" (24x36)

(OPTION #1)



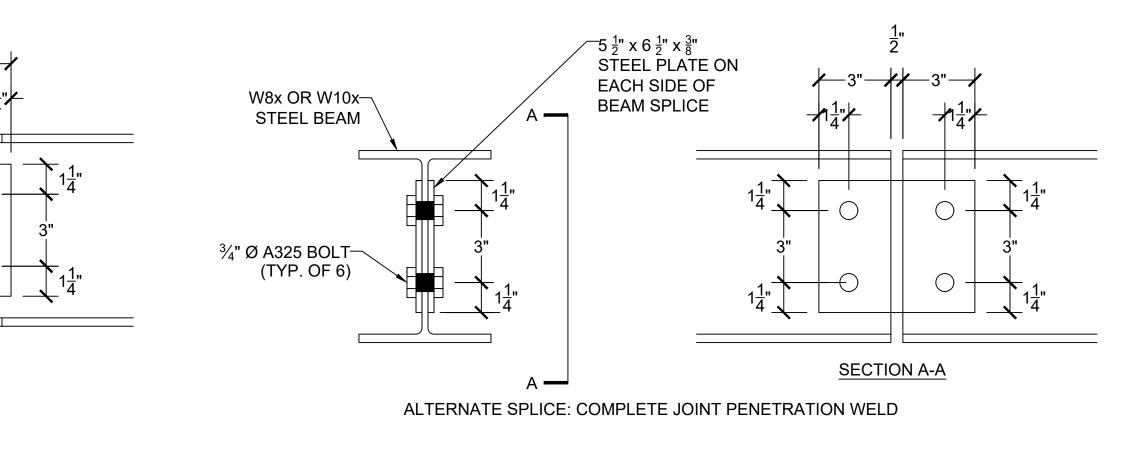
1 BOLTED T-BEAM CONNECTION FOR W8x AND W10x BEAMS
SCALE: 2" = 1'-0" (18x24) OR 3" = 1'-0" (24x36)

(OPTION #2)

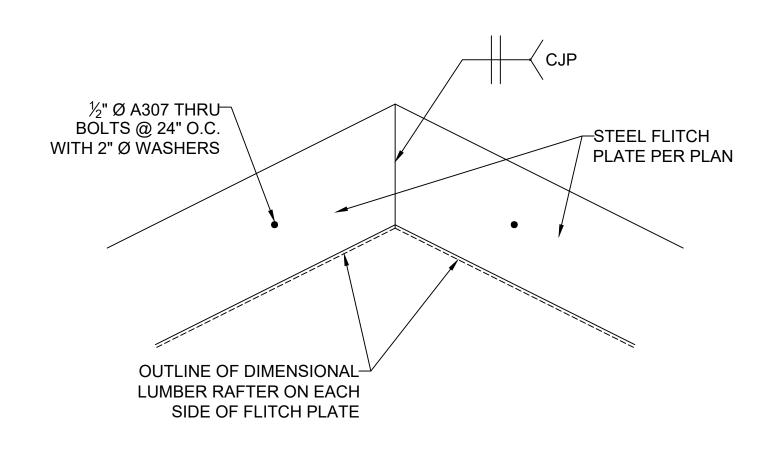


2 BOLTED T-BEAM CONNECTION FOR W12x, W14x, W16x & W18x BEAMS S3.4 SCALE: 2" = 1'-0" (18x24) OR 3" = 1'-0" (24x36)

(OPTION #2)

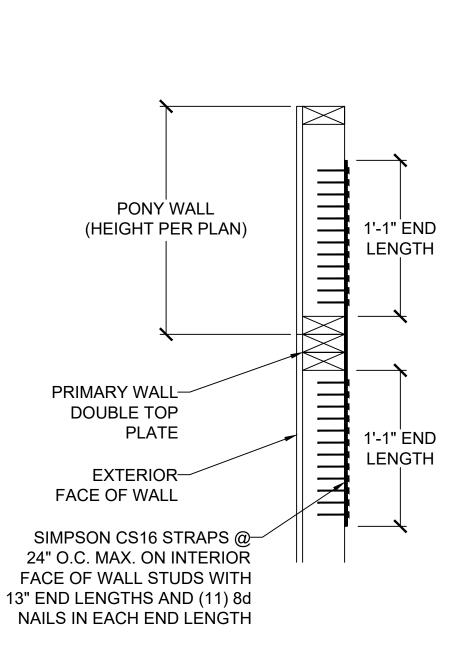


3 BEAM SPLICE CONNECTION FOR W8x AND W10x BEAMS S3.4 SCALE: 2" = 1'-0" (18x24) OR 3" = 1'-0" (24x36)

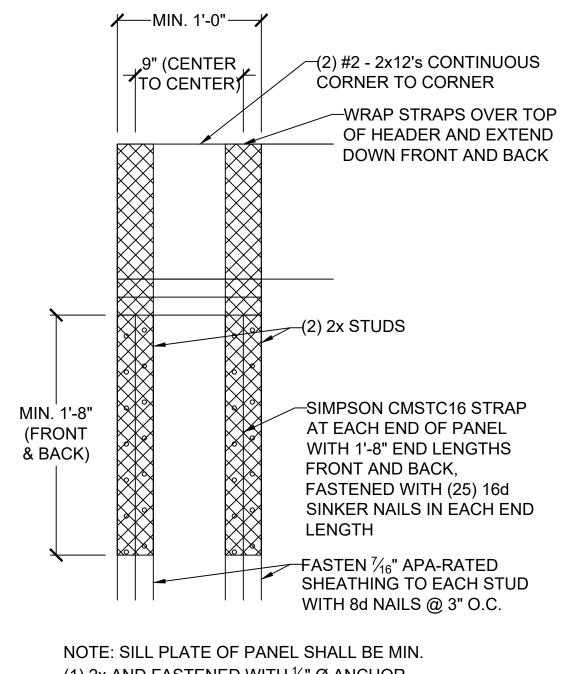


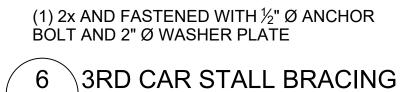
4 RAFTER FLITCH PLATE DETAIL

\$3.4 SCALE: 1" = 1'-0" (18x24) OR 1½" = 1'-0" (24x36)

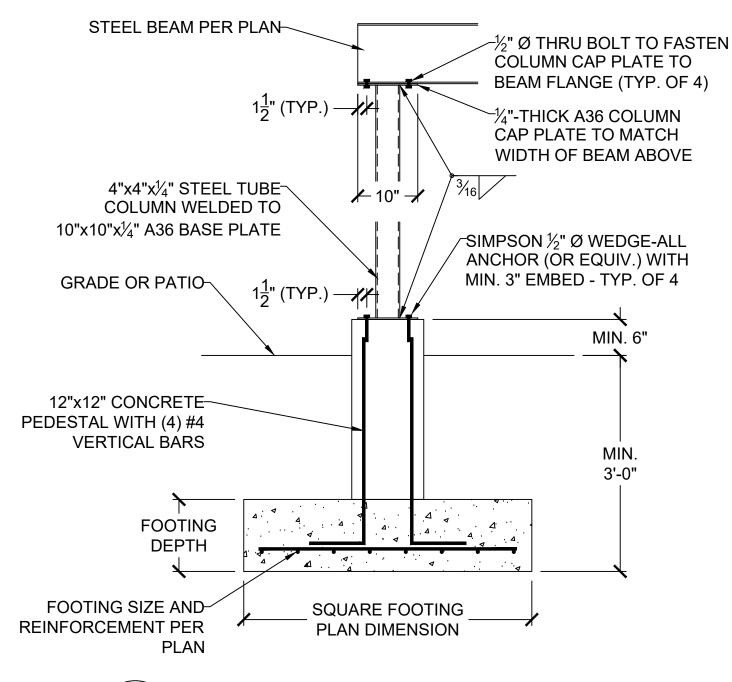




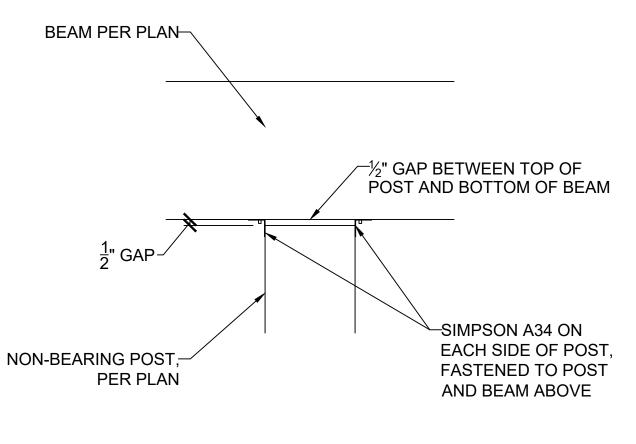




S3.4 SCALE: 1" = 1'-0" (18x24) OR $1\frac{1}{2}$ " = 1'-0" (24x36)



7 EXTERIOR STEEL COLUMN CONNECTIONS S3.4 SCALE: ½" = 1'-0" (18x24) OR ¾" = 1'-0" (24x36)



8 NON-BEARING POST CONNECTION S3.4 SCALE: 1" = 1'-0" (18x24) OR 1½" = 1'-0" (24x36)



CLIENT: PFEIFER HOMES, INC

JOB TITLE: RWR132 TOTTA

LOT 132, RESERVE AT WOODSIDE RIDGE

LOCATION: LEE'S SUMMIT, MISSOURI



