1. General Information

- A. The contractor shall verify dimensions and conditions before construction and notify the engineer of any discrepancies, inconsistencies, or difficulties affecting the work
- B. The contractor shall coordinate all disciplines, verifying size and location of all openings, whether shown on structural drawings or not, as called for on architectural, mechanical, or electrical drawings. In the case of work in an existing building the contractor shall scan existing structure to locate all rebar in the area of the new core/opening using ground penetrating radar and notify the engineer of record for review prior to coring/cutting. Conflicts, inconsistencies, or other difficulties affecting
- structural work shall be called to the architect or engineer's attention for direction C. All design and construction work for this project shall conform to the requirements of the following governing design codes: 1.) International Building Code (IBC 2018) as amended by the city of
- Lee's Summit, Missouri 2.) Minimum Design Loads for Buildings and Other Structures (ASCE7-16) 3.) Specification for Structural Steel Buildings (AISC 360-16) Member Design Basis is Allowable Stress Design (ASD) Connection Design Basis is Allowable Stress Design (ASD)
- 4.) Structural Welding Code (AWS D1.3-98) 5.) Building Code Requirements for Structural Concrete (ACI 318-14) 6.) Building Code Requirements for Masonry Structures (TMS 402-14)

7.) North American Specification for the Design of Cold-Formed Steel Structural

- Members (AISI S100-16/S1-1) 8.) National Design Specification (NDS) for Wood Constriction with 2015 Supplements (ANSI/AWC NDS-2015)
- 9.) Special Design Provisions for Wind and Seismic (AWC SDPWS-2015) D. These drawings are for this specific project and no other use is authorized.

2. Structural Load Design Criteria

A.	Dead Load:	
	Deck Floors	= 35 psf
	Apartment Floors	= 35 psf
	Roofs	= 20 psf
	Stairs	= 40 psf
B.	Live Load:	·
	Public Rooms	= 100 psf
	Stairs	= 100 psf
	Apartment Floors (Private Rooms)	= 40 psf
	Corridors	= 100 psf
	Storage Areas	= 125 psf
	Decks/Balconies (Private)	= 60 psf
	Decks/Balconies (Public)	= 100 psf
	Roofs	= 20 psf
		- 1

- Pg = 20 psf, Ce = 1.0Pf = 14 psf, Pm = 20 psfls = 1.0, Cs = 1.0, Ct = 1.0Drift & unbalanced snow loads per ASCE/SEI 7-10
- D. Lateral Loads: 1.) Wind V(ult) = 109 mph, Exposure B, GCpi = +/-0.18Design wind pressures to be used for the deisgn of exterior components and cladding material son the designated zones of walls and roof structures shall be per section 30.7 and table 30.7-2 of ASCE/SEI 7-16. Tabulated pressures shall be multiplied by effective are reduction factors, exposure adjustment factors, and topographic factors where applicable.
- 2.) Seismic: Ss = 0.099, S1 = 0.068, le = 1.0 Sds=0.086; Sd1=0.068; Site Classification C Seismic Design Category B Basic Seismic Force-Resisting System A.17- Light-Framed Walls with Shear Panels of All
- R=2. Omega = 2 1/2. Cd = 2. V=0.043W E. This project is designed to resist the most critical effects esulting from the load combinations of section 1605.3 of the 2018 International Building Code.

Other Materials

3. Concrete

- A. All concrete for foundations (walls, grade beams, footings and piers) shall develop minimum ultimate compressive design strength of 3500 psi in 28 days, but not less than 500 pounds of cement shall be used per cubic yard of concrete regardless of strengths obtained, not over 6 gallons of water per 100 pounds of cement and not over 4 inches of slump.
- B. All concrete for garage slab on grade shall develop minimum ultimate compressive design strength of 4000 psi in 28 days, but not less than 525 pounds of cement shall be used per cubic yard of concrete regardless of strengths obtained, not over 5.75 gallons of water per 100 pounds of cement and not over 4 inches of slump. Concrete mix shop drawing shall contain testing data proving concrete design mix shrinkage is less than 0.034% at 28 days when tested according to ASTM C157 (air drying method only).
- C. All concrete for interior flatwork (except garage slab on grade) shall develop minimum ultimate compressive design strength of 4000 psi in 28 days, but not less than 540 pounds of cement shall be used per cubic yard of concrete regardless of strengths obtained, not over 5.40 gallons of water per 100 pounds of cement and not over 4 inches of slump. Concrete mix shop drawing shall contain testing data proving concrete design mix shrinkage is less than 0.034% at 28 days when tested according to ASTM C157 (air drying method only).
- All concrete for exterior flatwork shall have a minimum design compressive strength of 4500 psi in 28 days, with not less than 560 pounds of cement per cubic yard of concrete, not over 5 gallons of water per 100 pounds of cement, with 6% +/- 1% air entrainment, and a maximum of 4 inches of slump.
- E. The preceding minimum mix requirements may have water-reducing admixtures conforming to ASTM C494 added to the mix at manufacturer's dosage rates for improved workability.
- F. The preceding minimum mix requirements may have up to 15% maximum of the cement content replaced with an approved ASTM C618 Class C fly ash, provided the total minimum cementitious content is not reduced. 6. All interior concrete slabs on grade shall be placed over 15 mil, Class A Vapor
- Barrier per ASTM E1745 with less than 0.01 perms, tested after mandatory conditioning. All joints shall be lapped and sealed per manufacturer's recommendations. All penetrations, as well as damaged vapor barrier material shall also be sealed per manufacturer's recommendation prior to concrete placement. Install barrier per manufacturer recommended details at all discontinuous edges (at interior columns, exterior edge of slab, etc.) to ensure terms of warranty are followed. The vapor barrier shall be placed over free-
- draining granular material as prescribed by the project soils report. H. All concrete is reinforced concrete unless specifically called out as unreinforced. Reinforce all concrete not otherwise shown with same steel as in similar sections or areas. Any details not shown shall be detailed per ACI 315 and meet
- requirements of ACI 318, current editions. Control joints in dirt formed slab to be as shown on plans. Where not shown, limit controlled areas to not more than 144 square feet, or 12 feet on any side. Slab panel side ratio shall not exceed 1 1/2 to 1.
- J. Contractor shall verify that all concrete inserts, reinforcing and embedded items are correctly located and rigidly secured prior to concrete placement.
- K. Construction joints in beams, slabs, and grade beams shall occur at midspan (middle third) unless noted otherwise. Provide 2 x 4 horizontal keys at construction joints for shear transfer.
- L. No aluminum items shall be embedded in any concrete.

4. Reinforcing Steel

otherwise.

- A. All reinforcing steel shall conform to the requirements of ASTM A615 or A706 grade 60 steel. Welded plain wire fabric shall be supplied in sheets and conform
- to the requirements of ASTM A185. B. Clear minimum coverage of concrete over reinforcing steel shall be as follows: 1.) Concrete placed against earth: 3" 2.) Formed concrete against earth: 2"
- 3.) Slabs: 4.) Beams or Columns: 5.) Other
- All coverage shall be nominal bar diameter minimum. C. All dowels shall be the same size and spacing as adjoining main bars (splice lap 48 bar diameters or 24" minimum unless noted otherwise). D. At corners of all walls, beams, and grade beams supply corner bars (minimum 2'-0" in each direction or 48 bar diameters) in outside face of wall, matching size
- and spacing of horizontal bars. Where there are no vertical bars in outside face of wall, supply 3 - #4 vertical support bars for corner bars. Bars marked continuous and all vertical steel shall be lapped 48 bar diameters (2'-0" minimum) at splices and embedments, unless shown otherwise. Splice top bars near midspan and splice bottom bars over supports, unless noted
- F. At all holes in concrete walls and slabs, add 2 #5 bars (opening dimension plus 96 diameters long) at each of four sides and add 2 - #5 x 5'-0" diagonally at each of four corners of hole. Openings in 8" thick walls are reinforced similar, but with 1 - #5 instead of 2 - #5, respectively. G. Unless otherwise covered on architectural plans or specifications, vertical control
- coordinated with the architect. Every other horizontal wall reinforcing bar shall be discontinuous at control joints except heavy top and bottom bars unless noted otherwise. Provide base seal waterstop style number 772 (by Greenstreak Inc. or approved equal) on dirt face side of wall at all walls below grade. H. Accessories shall be as specified in latest edition of the ACI Detailing Handbook

joints in concrete wall shall be spaced at a maximum of 20'-0" on center and

- and the concrete Reinforcing Steel Institute Design Handbook. Maximum accessory spacing shall be 4'-0" on center, and all accessories on exposed surfaces are to have plastic coated feet.
- All slabs and stairs not shown otherwise shall be 6" thick with #4 bars at 12" on center each way. All exterior porches and stoops not otherwise detailed may be constructed in any standard manner, solid or hollow, but must be reinforced with #4 bars at 12" on center each way minimum. Porches shall be doweled to adjacent walls or grade beams with #4 bars at 12" on center, hooked or embedded 48 diameters into both members. Slope porches 1/8" per foot for
- drainage unless noted otherwise. Allow 1 ton of reinforcing bars #4 or larger to be used as directed in the field for special conditions by the engineer of record (labor for placing same to

5. Structural Steel

- A. All structural steel beams and columns shall be ASTM A992, grade 50 steel and all miscellaneous steel shall be ASTM A36 grade steel (except at moment connections where plates shall be ASTM A572, grade 50). Hollow Structural Sections (HSS) shall be ASTM A500, grade C. Fabrication and erection shall be in accordance with AISC 303-16 "Code of Standard Practice for Steel Buildings and Bridges" in the
- 15th Edition of the AISC Steel Construction Manual. B. All welding shall conform to the recommendations of the AWS.
- All exterior steel and connections, and brick relief angles shall be hot-dip galvanized. D. All bolts not otherwise specified shall be 3/4" diameter high strength (ASTM A325-N). All bolts shall be fully pretensioned. All beam connections shall be designed per the AISC Manual of Steel Construction "Framed Beam Connections" for the indicated reactions or at least 0.3 x beam total shear capacity, Vn/Omega, shown in the maximum total uniform load tables, whichever is greater; and, shall account for eccentricity when the bolt line is more than 2" from the center of the support.
- All connections must be two bolt minimum. E. All anchor bolts shall be 3/4" diameter, ASTM F1554, Grade 36 unless noted otherwise. Washers of minimum size and thickness for the given anchor diameter in Table 14-2 of the AISC Steel Construction Manual shall be provided at every column anchor bolt. Washers shall have a standard size hole for the anchor bolt.

6. Post Installed Anchors

- A. Post-installed anchors shall be used only where specified on the drawings unless approved in writing by the engineer of record. See drawings for anchor diameter, spacing and embedment. Performance values of the anchors shall be obtained for specified products using appropriate design procedures and/or standards as required by the governing building code. Anchors installed in concrete shall have an ICC-ES Evaluation Service Report. Special inspection is required for all post installed anchors. The contractor shall coordinate an on-site meeting with the post installed anchor manufacturer field representative to educate the construction
- team on the anchor installation guidelines and requirements. B. Mechanical anchors used in cracked and uncracked concrete shall have been tested and qualified for use in accordance with ACI 355.2 and ICC-ES AC193. All anchors shall be installed per the anchor manufacturer's written instructions.
- Adhesive anchors used in cracked and uncracked concrete shall have been tested. and qualified for use in accordance with ICC-ES AC308. All anchors shall be installed per the anchor manufacturer's written instructions. D. Mechanical anchors used in solid grouted masonry shall have been tested and
- qualified for use in accordance with ICC-ES AC01. All anchors shall be installed per the anchor manufacturer's written instructions. . Adhesive anchors used in solid grouted masonry shall have been tested and qualified for use in accordance with ICC-ES AC58. All anchors shall be installed per the anchor manufacturer's written instructions.
- F. Anchors used in hollow concrete masonry shall have been tested and qualified in accordance with ICC-ES AC106 or ICC-ES AC58 as appropriate. All anchors shall be installed per the anchor manufacturer's written instructions with appropriate screen tubes used for adhesives.

7. Foundations

- A. The soil investigation was prepared by CFS Engineers, the report number is 20-5555 and their telephone number is 913-627-9040.
- B. Spread footings, grade beams, and retaining walls are designed to bear on insitu clay or engineered clay fill capable of safely sustaining 2,500 psf. C. Retaining walls are designed for an active lateral load of 55 pcf equivalent
- fluid pressure. D. Basement walls are designed for an at rest lateral load of 80 pcf equivalent fluid
- pressure. See General Note 3H for wall bracing requirements. E. Contractor shall provide for dewatering at excavations from either surface water or
- F. All foundation excavations shall be inspected by a qualified soil engineer, approved by the architect and/or structural engineer, prior to placement of steel or concrete. This inspection shall be at the owner's expense. G. All concrete in the structural portion retaining the backfill shall have attained its
- design strength prior to being backfilled. H. Moisture content in soils beneath building locations should not be allowed to change after footing excavations and after grading for slabs on grade are completed. If subgrade materials become desiccated or softened by water or other conditions. ecompact materials to the density and water content specified for engineered fill. Do not place concrete on frozen ground.

8. Concrete Masonry Units

- A. Concrete block used in exterior walls or load bearing walls shall meet the requirements of ASTM C90 and have a minimum net compressive strength of 1900 psi and laid up using type N mortar such that f'm equals 1500 psi. Mortar shall be volume proportion based cement lime mortar. Proportioning shall be completed by box measure. Any block in contact with earth shall be normal weight units, laid using type "S" mortar and grouted solid. B. The contractor shall provide adequate temporary bracing for all masonry walls
- during construction. C. All concrete block shall have 9 gage (or larger) horizontal joint reinforcing (ladder or truss) per architectural drawings and specifications (16" maximum vertical spacing). D. Cavity wall construction shall be reinforced as designed for specific concrete block used. The horizontal joint reinforcing shall be of the ladder or truss style per
- specification and continuous between brick and block, as prescribed by the architectural drawings. E. Concrete block shall be reinforced as follows in 6", 8", 10", and 12" walls: 1.) Vertical reinforcing shall be a minimum of 1 - #5 bar in 6" and 8" walls and 2 - #5 bars in 10" and 12" walls at 24" OC, at each corner, at each door and window jamb, each side of control joints and in the end void of each length of wall. Lap splices for masonry vertical reinforcing
 - shall be 48 bar diameters or 24" minimum. 2.) Horizontal reinforcing: A.) Horizontal joint reinforcing as noted above. B.) Continuous horizontal bars shall be included per section or detail in bond beam or optional running bond beam where noted. Where bond beams are continuous at corners of walls, supply

corner bars matching size of horizontal bars (minimum 2'-0" or

- 40 bar diameters in each direction). F. Grout, where noted above, shall have a minimum design ultimate compressive strength of 2500 psi at 28 day test and 3/8" maximum aggregate size.
- G. Non-load bearing concrete block walls shall be isolated from adjacent structural elements with vertical 3/8" control joints and at the top of the wall with 1" air space or compressible material and support per architectural detail. H. Unless otherwise covered on architectural plans or specifications, vertical control
- joints in masonry construction shall be 3/8" wide, full height of wall. Joints shall be spaced at a maximum of 24'-0" on center and coordinated with the architect. All horizontal joint reinforcing shall be discontinuous at control joints in masonry.
- All bond beam horizontal reinforcing shall be continuous through control joints. Lintels over all openings up to 8'-0" wide in new and existing masonry walls not otherwise covered shall be one 6x3 1/2x5/16 angle for each 4" width of masonry. All exterior lintels to be galvanized.
- J. Walls shall be anchored top and bottom by dowels matching wall vertical reinforcing(unless noted otherwise) from floor slab bottom and bracing angles at the top, per details on the drawings.

9. Timber and Wood Framing

- A. Quality and construction of wood framing members and their fasteners for load supporting purposes not otherwise indicated on the drawings shall be in accordance with the International Building Code. B. All studs and top and bottom plates shall be Douglas Fir No. 2 grade visually graded
- lumber, with an allowable fiber stress in bending of 900 psi minimum and an elastic modulus of 1,600,000psi unless noted otherwise. All joist, truss members, and headers to be No. grade 2 (min.) unless noted otherwise. All lumber for exterior decks and balconies shall be treated Southern Yellow Pine No. 2 grade. C. Blocking of stud bearing walls and shear walls shall be solid, matching sheathing joints.
- D. Joist blocking and bridging shall be solid wood or cross bridging of either wood or metal straps. Spacing, in any case, shall not exceed 8'-0".
- E. Wood members and sheathing shall be fastened with number and size of fasteners not less than that set forth in Table 2304.9.1 of the International Building Code. Floor sheathing shall be APA rated tongue and groove Sturd-I-Floor, exposure 1, glued and nailed with 8d ring shank nails or # 10 screws at 12" on center to all supports. Sheathing of shear walls or roof diaphragms shall be edge nailed with 8d common nails at 6" on center and nailed to intermediate framing and/or blocking members with 8d common nails at 12" on center unless otherwise noted on the drawings. All floor sheathing shall be installed with 1/8 inch
- gaps between panel edges and end joints. F. Sill plates shall be bolted to concrete walls or steel beams with 1/2" diameter bolts at 32" on center. Sill plates in direct contact with concrete or masonry shall be treated lumber. G. Joist hangers shall have Uniform Building Code approval and shall be equal to

Simpson Strong Tie "LUS" for wood application and "LB" for steel weld-on application.

J. Parallel Strand Lumber (PSL) shall have an allowable flexural stress (Fb) of 2,900 psi (reduced by size factor) and an elastic modulus (E) of 2,000,000 psi. K. Pre-engineered wood trusses shall be designed in accordance with the Truss Plate Institute's national design standard for metal-plate connected wood truss construction (ANSI/TPI-1 latest edition). Trusses shall be designed and manufactured by an authorized

2.600 psi (reduced by size factor) and an elastic modulus (E) of 1.900.000 psi.

H. Service condition - dry with moisture content at or below 19% in service.

I. Laminated veneer lumber (LVL) shall have an allowable flexural stress (Fb) of

- member of the Wood Truss Council of America (WTCA). Truss design shall conform to specified codes, allowable stress increases, deflection limitations and other applicable criteria of the governing code. L. Shop drawings showing complete erection and fabrication details and calculations (including connections) shall be submitted to the project architect/engineer for review prior to fabrication and/or erection. Such drawings shall bear the seal of a professional engineer,
- registered in the state of the project location. Shop drawings shall also be submitted to the local government controlling agency when requested by that agency. M. All trusses shall be securely braced both during erection and permanently, as indicated on the approved truss design drawings and in accordance with TPI's commentary and recommendations for handling, installing and bracing metal-plate connected wood trusses
- (HIB-91, booklet) and the latest edition of ANSI/TPI-1. N. The truss manufacturer shall supply all hardware and fasteners for joining truss members together and fastening truss members to their supports. Metal connector plates shall be manufactured by a member of the Wood Truss Council of America (WTCA) and shall be 20 gauge minimum. Connector plates shall meet or exceed ASTM A653, grade 33, with ASTM A924 galvanized coating designation G60.
- O. Shipment, handling, and erection of trusses shall be by experienced, qualified persons and shall be performed in a manner so as not to endanger life or property. Apparent truss damage shall be reported to the truss manufacturer for evaluation prior to erection. Cutting or alteration of trusses is not permitted.
- P. Contractor shall coordinate truss layout for openings and penetrations required by other trades including for plumbing, HVAC, electrical, roof access hatches, chases, etc. Q. Pre-engineered floor truss and I-Joist design load and deflection criteria are as
- Top Chord Dead Load Top Chord Live Load = Per General Note 2A Bottom Chord Dead Load = 5psf Allowable Total Load Deflection = L/360Allowable Live Load Deflection = L/480; ½" maximum
- R. Pre-engineered roof truss design load and deflection criteria are as follows: Top Chord Dead Load = 15psf = 20psf Top Chord Live Load Bottom Chord Dead Load = 10psf Allowable Total Load Deflection = L/300Allowable Live Load Deflection = L/360

Components & Cladding Wind Loads Diagram.

A. Bob D. Campbell and Company, Inc. will review the General Contractor's (GC) shop drawings and related submittals (as indicated below) with respect to the ability of the detailed work, when complete, to be a properly functioning integral element of the overall structural system designed by

Roof trusses shall be designed for wind uplift loads indicated in Building

- Bob D. Campbell and Company, Inc. B. Prior to submittal of a shop drawing or any related material to Bob D. Campbell and Company, Inc., the GC shall: 1.) Review each submission for conformance with the means, methods, techniques, sequences and operations of construction and safety precautions and programs incidental thereto, all of which are the sole
 - responsibility of the GC. 2.) Review and approve each submission. 3.) Stamp each submission as approved.
- C. Bob D. Campbell and Company. Inc. shall assume that no submission comprises a variation unless the GC advises Bob D. Campbell and Company, Inc. with written documentation. D. Bob D. Campbell and Company, Inc. shall review shop drawings and related materials with comments provided that each submission has met the above
- requirements. Bob D. Campbell and Company, Inc. shall return without comment unrequired material or submissions without GC approval stamp. E. Shop drawings and related material (if any) required are indicated below. Should Bob D. Campbell and Company, Inc. require more than ten (10) working days to perform the review, Bob D. Campbell and Company, Inc. shall so notify
- 1.) Concrete mix designs and material certificates including admixtures and compounds applied to the concrete after placement. 2.) Reinforcing steel shop drawings including erection drawings and
- bending details. Bar list will not be reviewed for correct quantities. 3.) Elevations of all reinforced concrete masonry walls at a scale no smaller than 3/8" = 1'-0" showing all required reinforcing.
- 4.) Grout mix designs (for CMU). 5.) Construction and control joint plans and/or elevations. 6.) Structural steel shop drawings including erection drawings and piece details. Include joist, decking and connector submittals. Include miscellaneous framing specified on the structural drawings, but do not submit framing specified on non-structural drawings for Bob D.
- Campbell and Company, Inc. review. 7.) Miscellaneous anchors shown on the structural drawings. 8.) Wood truss design calculations and detailed erection and fabrication drawings. Standard stick framing shop drawings need not be submitted.

11. Statement of Structural Special Inspections

- A. The structural design for this project is based on completion of special inspections during construction in accordance with section 1704 of the International Building Code. The owner shall employ one or more qualified
- special inspectors to provide the required special inspections. B. The special inspector shall furnish inspection reports to the building official, owner, architect and structural engineer, and any other designated person. C. All discrepancies shall be brought to the immediate attention of the contractor for correction, then, if uncorrected, to the proper design authority, building official and structural engineer.
- D. The special inspector shall submit a final signed report stating that the work requiring special inspection was, to the best of the inspector's knowledge, in conformance with the approved plans and specifications and the applicable workmanship provisions of the building code. E. The following inspections and tests are required with the frequency (continuous or
- periodic) as defined within the referenced section or standard listed below. The General Contractor shall provide notification to the inspector when items requiring inspection are ready to be inspected and provide access for those
- 1. Shop Fabrication structural steel per Section 1704.2.5 unless AISC certified shop 2. Shop Fabrication – pre-engineered wood trusses per Section 1704.2.5 unless TPI
- 3. Steel Construction per Section 1705.2 and the quality assurance requirements of AISC 341 Chapter J (as referenced by AISC 360) 5. Concrete Construction per Section 1705.3 and Table 1705.3 a. Reinforcing Steel Placement b. Reinforcing Steel Welding c. Cast in Place Anchors
 - d. Post Installed Anchors e. Design Mix Verification f. Concrete Sampling and Testing g. Concrete Placement
- h. Concrete Curing 6. Masonry Construction per Section 1705.4 and the quality assurance requirements of TMS 402/ACI530/ASCE5 and TMS602/A530.1/ASCE6 [Level B] 7. Verification of Soils per Table 1705.6
- a. Wood shearwalls (include sheathing, rim board and bottom plate b. Portal frames
- c. Shear wall and portal frame holdowns d. Shear wall tension rod system 9. Wood Gravity Framing and Placement (adjust frequency of random sampling where indicated as required) a. Heavy timber/SCL/glulam beams and supports (periodic)
- b. Headers and jambs (random sampling) c. Bearing walls (random sampling) d. Connector/hardware installation (random sampling) e. Floor and roof trusses (random sampling)

12. Copyright and Disclaimer

A. All drawings in the structural set (S-series drawings) are the copyrighted work of Bob D. Campbell and company, Inc. These drawings may not be photographed, traced, or copies in any manner without the written permission of Bob D. Campbell and Company, Inc. Exception: Original drawings may be printed for distribution to the owner, architect, and general contractor for coordination, bidding, and construction. Subcontractors may not reproduce these drawings for any purpose or in any manner.

I, Clark A. Basinger, P.E., registered engineer and a representative of Bob D. Campbell and Company, Inc., do hereby accept professional responsibility as required by the professional registration laws of this state for the structural design drawings consisting of S-series drawings. I hereby disclaim responsibility for all other drawings in the construction document package, they being the responsibility of other design professionals whose seals and signed statements may appear elsewhere in the construction document package.

ESTIMATED BUILDING MOVEMENT			ABLE
FLOOR	ACCUMULATIVE WOOD SHRINKAGE	HEIGHT OF BRICK	ACCUMULATIVE BRICK EXPANSION
ROOF	1"	30'	0.33"
3rd FLOOR	0.7"	20'	0.22"
2nd FLOOR	0.35"	10'	0.11"

Wood Shrinkage Notes:

Bob D. Campbell & Company takes no responsibility for the naturally-occurring shrinkage that will occur in a wood structure or the impact the movement will have on the architectural, mechanical, electrical and plumbing systems that are designed by others. The analysis provided below are estimated values in accordance with IBC Section 2304.3.3 and indicate the systems and/or routing of the systems shall be designed to accommodate the movement. Failure to follow the considerations below can result in a failure of the impacted components within the system.

Estimated values are based on the following moisture content in the framing At install (MC) = 19% b. At equilibrium (EMC) = 8%

Reference wall sections on this sheet for estimated cumilative values per floor. The following is a list of recommendations to minimize potential issues related to wood shrinkage and veneer expansion. Veneer expansion is seasonable and variable depending on sun exposure. The majority of wood shrinkage will occur in the first 24 months of occupancy with minor seasonal variations.

 MEP System Considerations a. Postpone MEP installation as long as possible to allow as much dead load to be applied--allowing construction gaps to close. b. Provide oversized and vertically slotted holes at pipe horizontal penetration and notches. Refer to typical notching and cutting of stud wall detail for additional considerations on size limitations. c. Plumbing pipe and electrical conduit joints and connections shall be flexible and allow for

expansion/contraction to prevent a rigid assembly.

- d. Hangers and necessary rigid connections shall be adjusted prior to completion of construction or closing of wall/ceiling assembly. e. Horizontal vent penetrations through exterior veneers shall be provided with double flashing. f. All sheet metal vertical down spouts shall have intermediate slip ioints.
- g. Roof drains shall utilize adjustable fittings that are adjusted back to the roof finish sheathing elevation at the completion of construction and then shall be adjusted as required to maintain proper drainage. 2. Architectural System Considerations a. At stucco, EIFS and thin set veneer systems provide
- horizontal expansion joints, slip joints with appropriate flashing, this includes transitions between changes in veneer material. b. At brick and stone veneers provide veneers ties designed to accommodate differential movement.
- c. Refer to architectural window and door head and sill: parapet: and horizontal material changes for specific horizontal gap requirements between materials. 3. Construction Tolerance Considerations a. All studs shall be cut level, square and tight to top and
- bottom plates to reduce any additional shortening of the building due to nesting. b. All wood structural panels on the walls shall have a 1/2" relief gap at each floor level to reduce the potential for bulging. c. All floor sheathing shall have 1/8" gaps around all four sides at time of install to allow for expannsion. d. All shearwall holdown shall be checked and retighten immediately prior to sheathing of the walls. If a continuous rod system is utilized for

at changes in materials. Caulked as needed as shrinkage occurs and

- holdowns or uniform uplift anchors, the take-up devise pins shall be verified to have been pulled prior to sheathing the walls. 4. Material Storage and Protection a. All stored material shall remain covered and elevated from the elements to reduce the potential for an increase in
- b. Do not allow water to pond on the floor sheathing. Provide drain holes in the floor sheathing as required to relieve any water that might temporary pond. 5. Post Occupancy Consideration a. Recommend a review of roof drains every 3 months for the first 24 months of occupancy and then annually and adjusted as needed. b. Recommend a review of vertical joints at exterior doors, windows and

moisture content.

original joint fails.

LEGEND:

LEVEL BEAM W14x22 — STEEL BEAM SIZE

DESIGNATION T 117'-6"—— TOP OF BEAM ELEVATION

SLOPING BEAM W14x22 — STEEL BEAM SIZE

DESIGNATION T 132'-5" TOP OF BEAM ELEVATION

SAW JOINT PER 1/S200

CONSTRUCTION JOINT PER 2/S200

SPAN DIRECTION OF DECK

HSS 8"x8"x5/16"COLUMN SIZE

FOOTING MARK - SEE SCHEDULE ON

BASE PLATE MARK - SEE SCHEDULE ON SHEET S002

EACH END

UPSET BEAM OR HEADER PER SCHEDULE ON S003

"WIDE CORRIDOR" WITH ADDITIONAL STUDS PER

SHEAR WALL PER SCHEDULE ON SHEET S004

WOOD STUD BEARING WALL SCHEDULE ON SHEET S003

BEAM OR HEADER PER SCHEDULE ON S003

	NAIL	ING SCHEDULE (REFER)	O NOTES #1 and #2)
No.	CONNECTION	ATTACHMENTS	(REF NOTE #3 and #4)
1	JOIST TO SILL OR GIRDER	3- 3" x 0.131" NAILS-TOENAIL	3-8d NAILS-TOENAIL
2	BRIDGING TO JOIST	2- 3" x 0.131" NAILS-TOENAIL EACH END	2-8d NAILS-TOENAIL EACH END
3	SOLE PLATE TO JOIST OR BLOCKING	3" x 0.131" NAILS AT 8"o.c TYPICAL FACE NAIL 4-3" x 0.131" NAILS AT 6"o.c. BRACED WALL PANELS	16d BOX NAILSZ AT 16"o.c. MAX. FACE NAILING 3-16d BOX NAILS AT 16"o.c. BRACED WALL PANEL
4	TOP PLATE TO STUD	3- 3" x 0.131" NAILS-END NAIL	2-16d NAILS-END NAIL
5	STUD TO SOLE PLATE	4- 3" x 0.131" NAILS-TOENAIL OR 3- 3" x 0.131" NAILS-END NAIL	4-8d NAILS-TOENAIL OR 2-16d NAILS-END NAIL
6	DOUBLE STUDS	3" x 0.131" NAILS AT 8"o.cFACE NAIL	16d BOX NAILS AT 24"o.c. MAX. FACE NAIL
7	DOUBLED TOP PLATES	3" x 0.131" NAILS AT 12"o.cFACE NAIL	16d BOX NAILS AT 16"o.c. MAX. FACE NAIL
8	DOUBLE TOP PLATE LAPS AND INTERSECTIONS	12-3" x 0.131" NAILS	8-16d NAILS
9	BLOCKING BETWEEN JOISTS OR RAFTERS TO TOP PLATE	3-3" x 0.131" NAILS -TOENAIL	3-8d NAILS-TOENAIL
10	RIM JOIST TO TOP PLATE	3" x 0.131" NAILS AT 6"o.cTOENAIL	8d NAILS AT 6"o.c. MAXTOENAIL
11	TOP PLATE LAPS AND INTERSECTIONS	3- 3" x 0.131" NAILS-FACE NAIL	2-16d NAILS-FACE NAIL
12	CONTINUOUS HEADER, TWO PIECES	3" x 0.131" NAILS AT 10"o.c. ALONG EACH EDGE	16d NAILS AT 16"o.c. MAX. ALONG EACH EDGE-TOENAIL
13	CEILING JOISTS TO PLATE	5- 3" x 0.131" NAILS-TOENAIL	3-8d NAILS-TOENAIL
14	CONTINUOUS HEADER TO STUD	4- 3" x 0.131" NAILS-TOENAIL	4-8d NAILS-TOENAIL
15	CEILING JOISTS, LAPS OVER PARTITIONS	4- 3" x 0.131" NAILS-FACE NAIL	3-16d NAILS-FACE NAIL
16	CEILING JOISTS TO PARALLEL RAFTERS	4- 3" x 0.131" NAILS-FACE NAIL	3-16d NAILS-FACE NAIL
17	RAFTER TO PLATE	3- 3" x 0.131" NAILS-TOENAIL	3-8d NAILS-TOENAIL
18	1" BRACE TO EACH STUD AND PLATE	2- 3" x 0.131" NAILS-FACE NAIL	2-8d NAILS-FACE NAIL
19	BUILT-UP CORNER AND MULTIPLE STUDS	3" x 0.131" NAILS AT 16"o.c.	16d NAILS AT 24"o.c. MAX.
20	BUILT-UP GIRDER AND BEAMS	3" x 0.131" NAILS AT 24"o.c. FACE NAILED TOP AND BOTTOM STAGGERED ON OPPOSITE SIDES 3- 3" x 0.131" NAILS AT ENDS AND EACH SPLICE	20d NAILS AT 32"o.c. MAX. TOP AND BOTTOM, STAGGERED ON 0PPSITE SIDES. 2-20d NAILS AT ENDS AND EACH SPLICE
21	BUILT-UP LAMINATED VENEER LUMBER BEAMS	3" x 0.131" NAILS AT 6"o.c. TOP AND BOTTOM ALONG EDGE	16d NAILS AT 12"o.c. TOP AND BOTTOM ALONG EDGE
22	2" PLANKING	4- 3" x 0.131" NAILS AT EACH SUPPORT	16d NAILS AT EACH SUPPORT
23	RIM BOARD TO TRUSS	2- 3" x 0.131" FACE NAILS (IT/IB @ EA. TRUSS)	2- 10d NAILS - FACE NAILS (IT/IB @ EA. TRUSS)
24	BUILD-UP STUD-PACK COLUMNS	REFER TO DETAIL 3/S003	REFER TO DETAIL 3/S003
	NOTES: 1.) ALL NAILS SHALL BE AS NO	TED UNLESS OTHERWISE SPECIFIED ON ST	RUCTURAL DRAWINGS OR

NAILING SCHEDLIF (REFER TO NOTES #1 and #2)

1.) ALL NAILS SHALL BE AS NOTED UNLESS OTHERWISE SPECIFIED ON STRUCTURAL DRAWINGS OR ALTERNATE PROVIDED BY ENGINEER IN WRITING. 2.) CONDITIONS NOT SPECIFIED SHALL BE IN ACCORDANCE WITH CURRENT INTERNATIONAL BUILDING CODE. 3.) NAILING DESIGNATION: 4 - 3" x 0.131" NAILS DIAMETER IN INCHES ———— NAIL LENGTH

— QUANITY 4.) ALL NAILS NOTED AS 8d, 10d, 16d, ETC. SHALL BE COMMON NAILS UNLESS NOTED BOX.

STRUCTURAL DECK & SLAB SCHEDULE DESCRIPTION COMPOSITE DECKING PER ARCHITECTURAL DRAWINGS/SPECIFICATIONS 3/4" GYPCRETE ATOP 23/32" T&G PLYWOOD SHEATHING, SHEATHING SHALL BE GLUED AND NAILED W/ 8d RING SHANK NAILS OR #10 SCREWS @ 6"o.c. @ EDGES & 12"o.c. AT FIELD. 19/32" PLYWOOD SHEATHING ATTACHED WITH 8d NAILS @ 6"o.c. AT EDGES & 12"o.c. AT FIELD. 4" CONCRETE SLAB REINFORCED W/ 6x6-W2.9xW2.9 WWF ATOP VAPOR BARRIER PER GENEARL NOTES ATOP 4" COMPACTED GRANULAR FILL ATOP 4" MINIMUM OPEN GRADED STONE ATOP PREVIOUSLY PREPARED PAD IN COMPLIANCE WITH SOILS REPORT

ΑT

EXTG. EXIST EXISTING

FDN

FTG

FLOOR DECK TYPE

FOUNDATION

FAR FACE

FINISH

FLOOR

FAR SIDE

FOOTING FIELD VERIFY

- 1. CD = COMPOSITE/CONCRETE DECK TYPE
- 2. FD = FLOOR DECK TYPE 3. NCD = NON-COMPOSITE DECK TYPE
- 4. RD = ROOF DECK TYPE 5. SOG = SLAB-ON-GRADE TYPE
- 6. REFER TO NOTE 10.T ON SHEET S0.01 FOR FIRE RETERDANT TREAD SHEATHING REQUIREMENTS. PROVIDE 1" DEEP TOOLED CONTROL JOINT (TRANSVERSE DIRECTION) @ MID-SPAN OF SINGLE BAY BALCONY OR @ THIRD POINTS OF DOUBLE BAY BALCONY. FILL JOINT w/ SEALANT

RAD

RADIUS

ROOF DECK TYPE

STRUCTURAL ABBREVIATIONS

GAGE

<u></u>	AND	GALV	GALVANIZE(D)	RD-#	ROOF DECK TYPE
Ø	ROUND, DIAMETER	GEN	GENERAL	REF	REFERENCE
ADTL	ADDITIONAL	GR	GRADE	REINF	REINFORCEMENT
AFF	ABOVE FINISHED FLOOR	HORIZ	HORIZONTAL	REQD	REQUIRED
ALT	ALTERNATE	HSS	HOLLOW STRUCTURAL SECTION	REV	REVISION
ARCH	ARCHITECTURAL	IF	INSIDE FACE	RLL	ROOF LIVE LOAD
BLDG	BUILDING	INFO	INFORMATION	RTU	ROOF TOP UNIT
3/	BOTTOM OF	INT	INTERIOR	SC	SLIP CRITICAL
ВМ	BEAM	JST	JOIST	SCHED	SCHEDULE(D)
BOTT	BOTTOM	JT	JOINT	SECT	SECTION
BRG	BEARING	K	KIPS (1000 LBS)	SHT	SHEET
2	CAMBER	KSF	KIPS PER SQUARE FOOT	SIM	SIMILAR
CD-#	CONCRETE DECK TYPE	KSI	KIPS PER SQUARE INCH	SJ	SAW JOINT
CJ	CONSTRUCTION/CONTROL JOIN	LBS, #	POUNDS	SL	SNOW LOAD
CJP	COMPLETE JOINT PENETRATION	Ld	DEVELOPMENT LENGTH	SOG	SLAB-ON-GRADE
CL	CENTERLINE	LL	LIVE LOAD	SOG-#	SLAB-ON-GRADE TYPE
CMU	CONCRETE MASONRY UNIT	LLH	LONG LEG HORIZONTAL	SPCG	SPACING
COL	COLUMN	LLV	LONG LEG VERTICAL	SPEC	SPECIFICATION
CONC	CONCRETE	LONG	LONGITUDINAL	SPRT	SUPPORT
CONN	CONNECTION	LSLT	LONG-SLOTTED HOLE TRANSVERSE	SQ	SQUARE
CONT	CONTINUOUS	LTWT	LIGHTWEIGHT	SS	STAINLESS STEEL
COORD	COORDINATE	M	MOMENT FORCE	SSLT	SHORT-SLOTTED HOLE TRANSVERSE
COV, CVR	COVER	MAX	MAXIMUM MECHANICAL MANUFACTURER MINIMUM MISCELLANEOUS	STD	STANDARD
OBL .	DOUBLE	MECH	MECHANICAL	STIFF	STIFFENER
DET	DETAIL	MFGR	MANUFACTURER	STIR	STIRRUP
OIA	DIAMETER	MIN	MINIMUM	STL	STEEL
OIM	DIMENSION	MISC	MISCELLANEOUS	STRUCT	STRUCTURE, STRUCTURAL
DL	DEAD LOAD	MSRY	MASONRY	T/	TOP OF
DWG	DRAWING	MTL	METAL	THRU	THROUGH
ĒΑ	EACH	NF	NEAR FACE	TOS	TOP OF STEEL, TOP OF SLAB
ĒF	EACH FACE	NS	NEAR SIDE	TRANS	TRANSVERSE
 ≣J	EXPANSION JOINT	NTS	NOT TO SCALE	TYP	TYPICAL
EL, ELEV	ELEVATION	NW	NORMAL WEIGHT	UNO	UNLESS NOTED OTHERWISE
EMBED	EMBEDMENT, EMBEDDED	OC	ON CENTER	V	SHEAR FORCE
ENGR	ENGINEER	OF	OUTSIDE FACE	VERT	VERTICAL
EOD	EDGE OF DECK	OPNG	OPENING	W/	WITH
EOR	ENGINEER OF RECORD	OPP	OPPOSITE	W/0	WITHOUT
EOS	EDGE OF SLAB	OVS	OVERSIZED HOLE	WF	WIDE FLANGE
ΞQ	EQUAL	Р	AXIAL FORCE	WL	WIND LOAD
EQUIP	EQUIPMENT	PAF	POWDER ACTUATED FASTENER	WP	WORK POINT
EW	EACH WAY	PC	PRECAST	WWF	WELDED WIRE FABRIC
EXP	EXPANSION	PCF	POUNDS PER CUBIC FOOT		
EXT	EXTERIOR	PEMB	PRE-ENGINEERED METAL BUILDING		
-//-	E. ((OTING)		DEDDENDIQUIA D		

PERPENDICULAR

POUNDS PER LINEAR FOOT

PARTIAL JOINT PENETRATION

POUNDS PER SQUARE FOOT

POUNDS PER SQUARE INCH

PLATE

QUANTITY

PLF

PJP

PSF

PSI

QTY

at

00 AND 2150 NW LOWENSTEIN LEE'S SUMMIT, MISSOURI 6408 ature

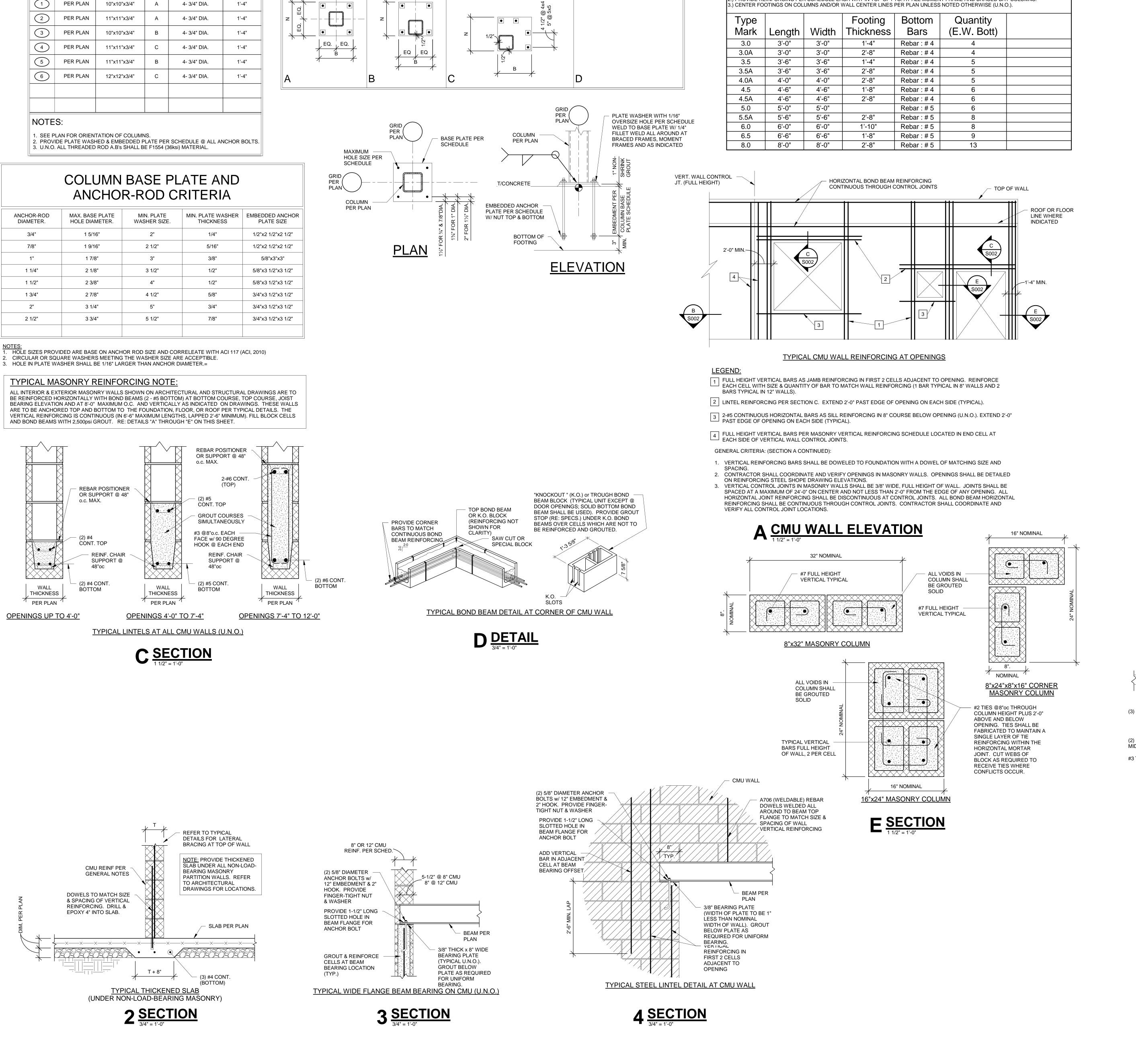
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BASE PLATE SHAPE (NOT TO SCALE)

COLUMN BASE PLATE SCHEDULE

ANCHOR RODS

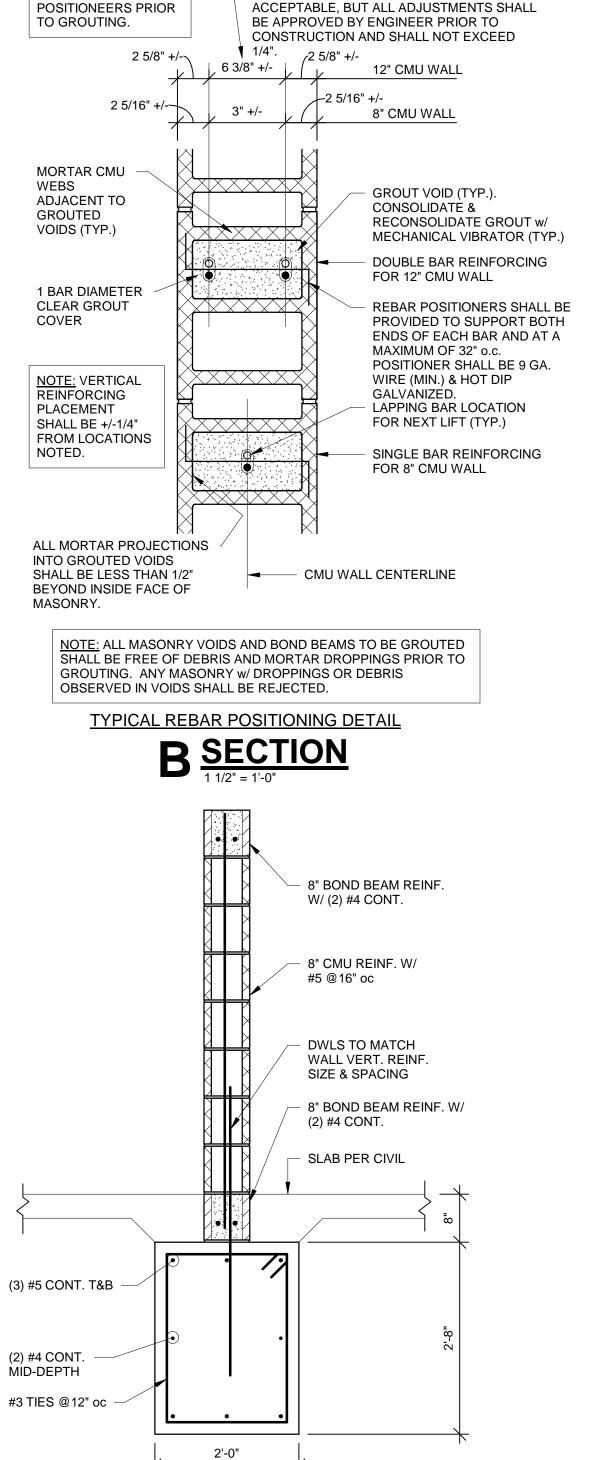
Structural Foundation Schedule

1) EXTERIOR FOOTINGS OR FOOTING AT GRADE BEAM SHALL MATCH GRADE BEAM DEPTH AND BE PLACED WITH GRADE BEAM. PROVIDE

.) PROVIDE REINFORCING PER SCHEDULE EACH WAY IN TOP OF FTG. AT ALL MOMENT FRAME AND BRACED BAY COLUMNS

SPECIFIED REBAR TOP AND BOTTOM WITH 4 STANDEES TO SUPPORT MATS.

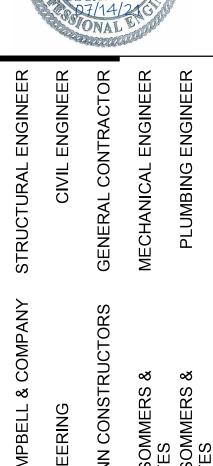
TYPICAL CMU TRASH ENCLOSURE



ADJUSTMENTS TO DIMENSIONS TO PROVIDE

DEFINED CLEAR GROUT COVER ARE

NOTE: REINFORCING SHALL BE PLACED IN



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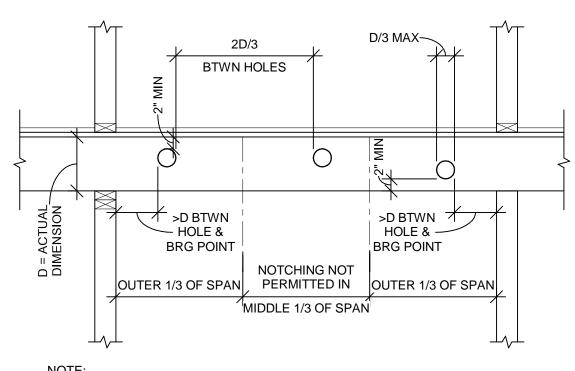
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TYPICAL NOTES FOR BEARING WALLS
 HOLES SHALL NOT BE LOCATED IN THE SAME STUD AS A CUT OR NOTCH
 CONTACT ARCHITECT PRIOR TO CUTTING OR NOTCHING TO VERIFY SIZE AND LOCATION IF HOLE IS GREATER THAN 20% STUD WIDTH OR NOTCHES GREATER THAN 10% STUD WIDTH ARE REQUIRED IN TWO OR MORE CONSECUTIVE STUDS
 NOTCHES OR HOLES NOT PERMITTED IN JAMBS, STUD PACKS AND AT ENDS OF SHEARWALLS
 STUD SHOES ARE NOTE AN ACCEPTABLE REMEDIATION OF OVER-NOTCHED OR

OVER-CUT STUDS WITHOUT PRIOR APPROVAL BY EOR
5. SLOT HOLES VERTICALLY FOR SHRINKAGE ALLOWANCE.

ALLOWABLE HOLES/NOTCHES IN WALL STUDS

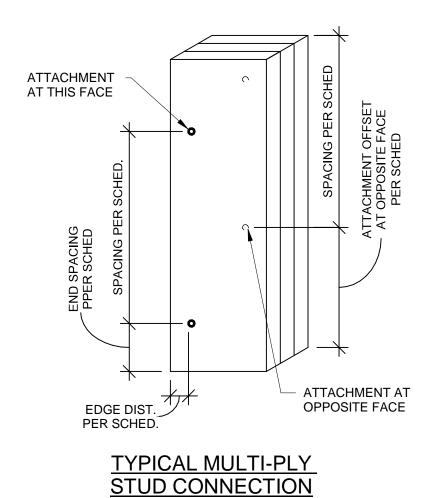
DETAIL



NOTE:
1. CONTACT ARCHITECT PRIOR TO CUTTING JOISTS TO VERIFY SIZE & LOCATION.
2. DETAIL APPLIES TO 2x FRAMING ONLY. REFER TO ENGINEERED OR COMPOSITE LUMBER MANUFACTURER'S RECOMMENDATIONS AT PSLs, LVLS, LSLs & GLULAM.

2 **DETAIL**

1 DETAIL



3	DETAIL
	1 1/2" = 1'-0"

_			
	BUI	LT-UP STUD PACK COLUM	IN ATTACHMENT SCHEDULE
NUMBER OF PLIES		ATTACHMENT AT COLUMN STUD PACKS SUPPORTING BEAMS	ATTACHMENT AT COLUMN STUD PACKS SUPPORTING TRUSSES
2-PLY MEMBERS 8d NAILS AT 12"oc, 1" FROM EDGE, w/ OPPOSITE EDGE NAILED FROM OPPOSITE SIDE OFFSET 6", @ 12"oc w/ FIRST NAIL 2" FROM EA. END		OPPOSITE EDGE NAILED FROM OPPOSITE SIDE OFFSET 6", @ 12"oc w/	8d NAILS AT 12"oc, 1" FROM EDGE, w/ OPPOSITE EDGE NAILED FROM OPPOSITE SIDE OFFSET 6", @ 12"oc w/ FIRST NAIL 2" FROM EA. END
		w/ OPPOSITE EDGE NAILED FROM OPPOSITE SIDE OFFSET 8", @ 16"oc w/	8d NAILS AT 12"oc, 1" FROM EDGE, w/ OPPOSITE EDGE NAILED FROM OPPOSITE SIDE OFFSET 6", @ 12"oc w/ FIRST NAIL 2" FROM EA. END
	1/4"Øx5" SIMPSON SDS SCREWS AT 16"oc, 1 1/2" FROM EDGE w/ OPPOSITE EDGE SCREWED FROM OPPOSITE SIDE OFFSET 8", @ 16"oc w/ FIRST SCREW 4" FROM EA. END		3 PLIES ATTACHED PER 3-PLY ATTACHMENT w/ 4th PLY ATTACHED w/ 8d NAILS AT 12"oc IN 2 ROWS, 1 1/2" FROM EDGE, OFFSET ROW 6"
	5-PLY MEMBERS	1/4"Øx6" SIMPSON SDS SCREWS AT 12"oc, 1 1/2" FROM EDGE w/ OPPOSITE EDGE SCREWED FROM OPPOSITE SIDE OFFSET 6", @ 12"oc w/ FIRST SCREW 4" FROM EA. END	3 PLIES ATTACHED PER 3-PLY ATTACHMENT w/ 4th & 5th PLY ATTACHED w/ 8d NAILS AT 12"oc IN 2 ROWS, 1 1/2" FROM EDGE, OFFSET ROW 6"
	1/4"Øx8" SIMPSON SDS SCREWS AT 12"oc, 1 1/2" FROM EDGE w/ OPPOSITE 6-PLY MEMBERS EDGE SCREWED FROM OPPOSITE		3 PLIES ATTACHED PER 3-PLY ATTACHMENT w/ 4th PLY ATTACHED w/ 8d NAILS AT 12"oc IN 2 ROWS, 1 1/2" FROM EDGE, OFFSET ROW 6" AND 5th AND 6th PLIES ATTACHED w/ 1/4"Øx5" SIMPSON SDS SCREWS @ 12"oc IN 2 ROWS, 1 1/2" FROM EDGE, OFFSET ROSS 6"oc w/ FIRST SCREW 4" FROM EA. END

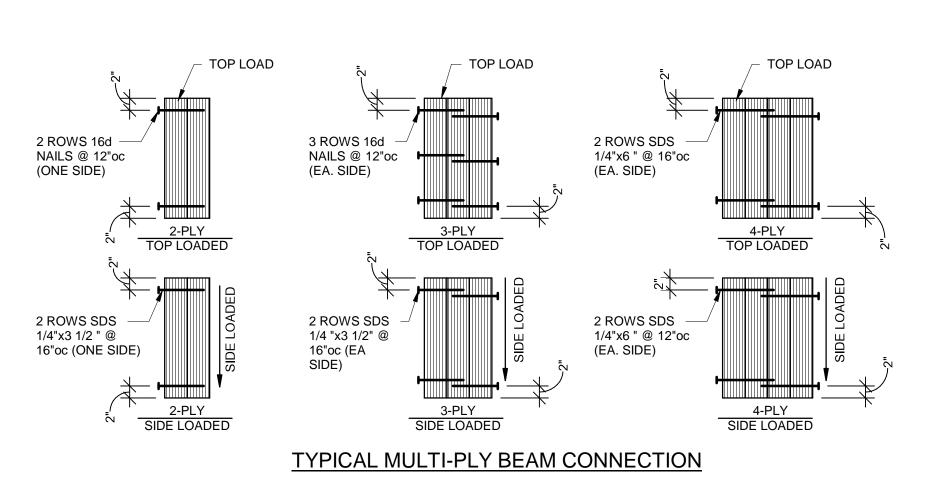
ALL BUILT-UP STUD PACKS MUST ALIGN FLOOR-TO-FLOOR WITH SOLID BLOCKING (SQUASH BLOCKS) AT FLOOR CAVATIES.
 EXTEND ALL STUD PACKS TO COLUMNS UNLESS NOTED OTHERWISE.
 ALL NAILS ARE COMMON NAILS UNLESS NOTED OTHERWISE.

KING STUD(S). FASTEN MULTIPLE KING STUDS TOGETHER PER BUILT-UP STUD PACK ATTACHMENT SCHEDULE INDEPENDENT OF JACK STUDS JACK STUD 0.131"x3" OR 10d COMMON NAILS. FACE NAIL TO KING STUDS @ 12"oc ADDITIONAL JACK STUD(S) STAGGERED. (SEE SCHEDULE OR PLAN NOTES FOR EXACT QUANTITIES OF JACK STUDS) FASTEN SUCCESSIVE JACK STUDS w/ 0.131"x3" OR 10d COMMON NAILS. FACE NAILS @ 12"oc STAGGERED AND MIRRORED FROM PREVIOUS PLY

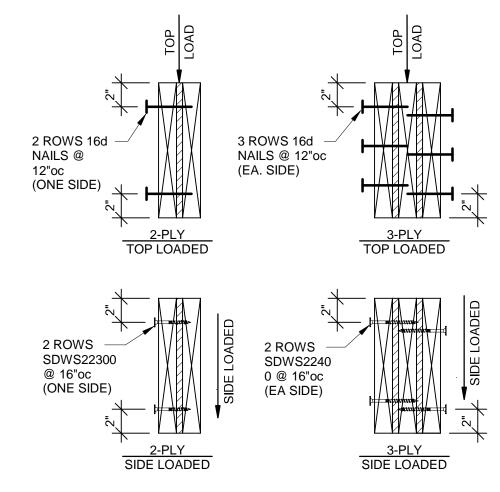
TYPICAL JACK STUD ATTACHMENT

1" EDGE DIST.-

4 DETAIL 1 1/2" = 1'-0"



5A DETAIL 3/4" = 1'-0"



TYPICAL MULTI-PLY HEADER CONNECTION

5B DETAIL

1 1/2" = 1'-0"

W	WOOD FLOOR/ROOF FRAMING HEADERS/BEAMS & JAMBS SCHEDULE				
		JAMB TYPE # (U.N.O. W/ COLUMN SCHEDULE)			NOTES
MARK	HEADER	" 3 " (2nd FLOOR FRAMING, TYP U.N.O.)	" 2 " (3rd FLOOR FRAMING, TYP U.N.O.)	" 1 " (ROOF FRAMING, TYP U.N.O.)	NOTES
(A1-#)	(2) 2x8 AT 2x4 WALL (3) 2x8 AT 2x6 WALL	1 JACK / 2 KING	1 JACK / 1 KING	1 JACK / 1 KING	
B1-#)	(3) 2x8	1 JACK / 1 KING	1 JACK / 1 KING	1 JACK / 1 KING	
(C1-#)	(3) 2x10	1 JACK / 2 KING	1 JACK / 1 KING	1 JACK / 1 KING	
(D1-#)	(3) 2x10	3 JACK/ 1 KING EACH SIDE	3 JACK/ 1 KING EACH SIDE	3 JACK/ 1 KING EACH SIDE	
E1-#)	3 1/2x14 PLUS PSL	2 JACK/ 1 KING EACH SIDE	2 JACK/ 1 KING EACH SIDE	2 JACK/ 1 KING EACH SIDE	
F1-#	(3) 1 3/4x18 LVL	2 JACK / 2 KING	2 JACK / 2 KING	1 JACK / 1 KING	
(G1-#)	(4) 2x10	1 JACK/ 2 KING	1 JACK/ 2 KING	1 JACK/ 1 KING	
(H1-#)	(3) 1 3/4x11 1/4 LVL	2 JACK/ 1 KING	2 JACK/ 1 KING	1 JACK/ 1 KING	

JAMB STUDS SHALL MATCH SIZE & GRADE OF WALL STUDS U.N.O.
 WHERE BEAM IS NOTED "UPSET", ALL JAMB STUDS NOTED WILL EXTEND TO DOUBLE TOP PLATE.
 ALL EXTERIOR LUMBER TO BE TREATED AGAINST MOISTURE. REFER TO NOTE 11B ON SHEET S001 FOR FIRE RETARDANT TREATED HEADER AND STUD REQUIREMENTS.
 PROVIDE SQUASH BLOCKS AT TRUSSES & BLOCKING FRAMING WHERE JAMBS OR STUD PACKS ARE DISCONT AND IN TRUSS CAVITY. QUANTITY TO MATCH JAMB OR STUD PACK ABOVE.
 PROVIDE ½" PLYWOOD SPACER PLATES AT INTERIOR HEADERS CONSTRUCTED WITH 2x LUMBER.
 AT CONTRACTOR'S OPTION, PROVIDE GLULAM IN LIEU OF PSL OF EQUAL OR GREATER STRENGTH.

REFER TO DETAIL 3/S003 FOR MULTI-PLY MEMBER CONNECTION REQUIREMENTS.
 ATTACH JAMB & KING STUDS TOGETHER PER CONNECTION TYPE 24 ON NAILING SCHEDULE ON S001.

	WOOD	STUD BEARI	NG WALL SC	HEDULE
TYPE	1st FLOOR WALLS (2nd FLOOR FRAMING)	2nd FLOOR WALLS (3rd FLOOR FRAMING)	3rd FLOOR WALLS (ROOF FRAMING)	NOTES
EXTERIOR	2x6 @ 16"oc	2x6 @ 16"oc	2x6 @ 16"oc	
DEMISING	(2) 2x4 @ 16"oc	2x4 @ 16"oc w/ ADDT'L 2x4 @ 32"oc	2x4 @ 16"oc	PROVIDE 2x BLOCKING AT MID HEIGHT BTWN EA. STUD
UNIT INTERIOR	(2) 2x4 @ 16"oc w/ ADDT'L 2x4 @ 32"oc	(2) 2x4 @ 16"oc	2x4 @ 16"oc	
TYPICAL CORRIDOR	2x6 @ 16"oc	2x6 @ 16"oc	2x6 @ 16"oc	
WIDE CORRIDOR	2x6 @ 16"oc w/ ADDT'L 2x6 @ 32"oc	2x6 @ 16"oc	2x6 @ 16"oc	HATCHED THUS ON PLAN:

NOTES: 1. PROVIDE 2x BLOCKING AT MID HEIGHT (5'-0" MAX) AT ALL LOAD BEARING WALLS NOT SHEATHED ON BOTH SIDES. 2. ALL STUDS TO BE No. 2 GRADE U.N.O.

RE: 3/S003 FOR NAILING OF MULTIPLE STUDS.
 REFER TO ARCH/MEP DRAWING FOR LOCATIONS OF FURRED OUT.

REFER TO ARCH/MEP DRAWING FOR LOCATIONS OF FURRED OUT WALLS TO ACCOMODATE PLUMBING OR MEP ITEMS.
 REFER TO FRAMING PLANS AND ARCH PLANS FOR LEVEL(S) AT WHICH WALLS OCCUR.
 STACK/ALIGN WALL STUDS FROM FLOOR TO FLOOR AT ALL EXTERIOR WALLS.

MSRY VENEER ARCH		STUD WALL PER SCHED.
LOOSE LINTEL PER SCHEDULE W/ 8" MIN BEARING AT EACH E (ALL EXTERIOR LINT TO BE GALV.)	END ELS	- GALV. 2 1/2x2 1/2x3/16" CLOSURE ANGLE SHOP WELDED TO LOOSE LINTEL (CLOSURE ANGLE LENGTH TO MATCH OPENING WIDTH) HEADER PER STUD PLAN
<u>T</u>	YPICAL LOOSE LINTEL D	<u>ETAIL</u>

6 TYPICAL LOOSE LINTEL DETAIL

LOOSE LINTEL SCHEDULE		
FOR OPENINGS:	GALV. ANGLE	
OPENING < 9'-0"	6"x4"x5/16" (LLV)	

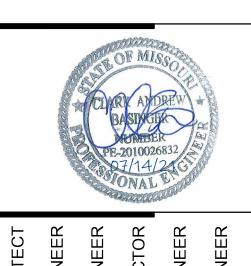
JOIST/BEAM/	TRUSS HANGE	R SCHEDULE
JOIST/BEAM/TRUSS SIZE	HANGER SIZE	NOTES
2x10	LUS28 HU210	AT CORRIDOR & BALCONY * AT SKEWED CONDITIONS
2x12	LUS210	AT CORRIDOR
(2) 2x12	LUS210-2	AT CORRIDOR
18" TRUSS FACE MOUNT TO UPSET BEAM	LUS410 *HHUS419	* AT SKEWED CONDITIONS

 HANGERS APPLY TO ALL LOCATIONS WHERE NOT OTHERWISE SPECIFIED IN DETAIL OR PLAN NOTE

	SIMPSON STRONG TIE
TRUSS TYPE	CONNECTION
COMMON/HIP/JACK AND 1-PLY TRUSSES	H2.5A
1-PLY TRUSS PER TRUSS SHOP DWGS GIRDER TRUSS	(2) H2.5A
AS NOTED PER PLAN	(ONE EACH SIDE)
2-PLY TRUSS PER TRUSS	
SHOP DWGS GIRDER TRUSS AS NOTED PER PLAN	LGT2
3-PLY TRUSS PER TRUSS	
SHOP DWGS OR GIRDER TRUSS AS NOTED PER PLAN	LGT3

NOTES:

1. GC COORDINATE HOLDOWN CONNECTION TYPE, QUANTITY & LOCATION WITH FINAL ROOF TRUSS SHOP DRAWINGS.



SM ENGINEERII
SM ENGINEERII
BRINKMANN CC
TEIN DR.
LATIMER SOMMASSOCIATES
LATIMER SOMMASSOCIATES

The Signature at W

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DATE:
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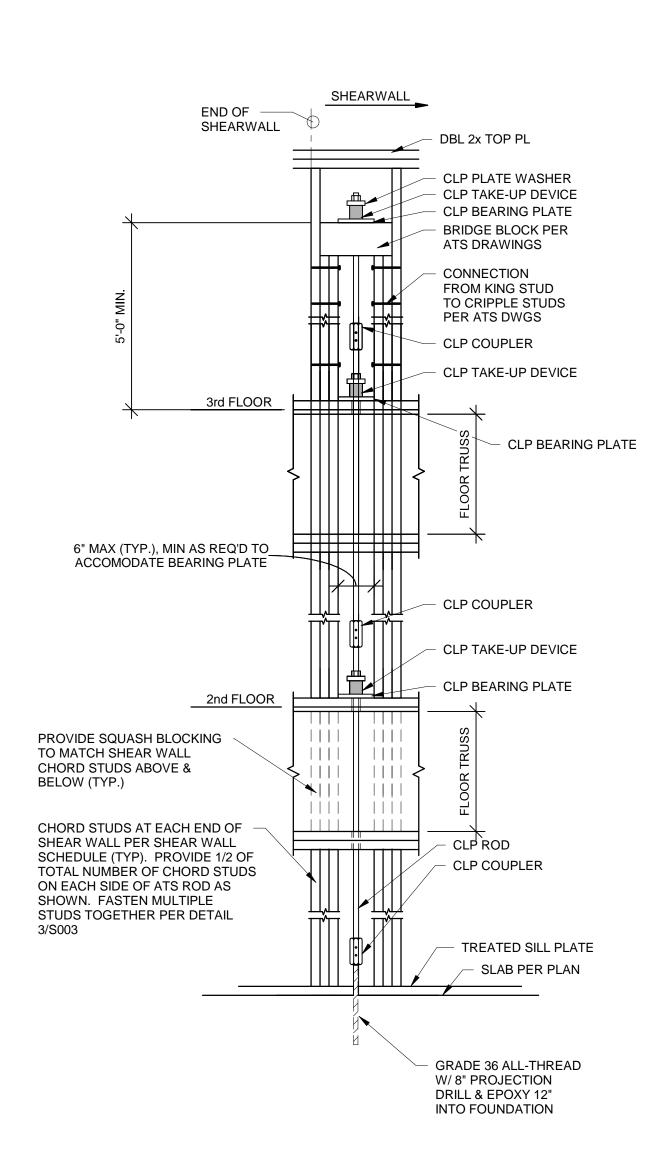
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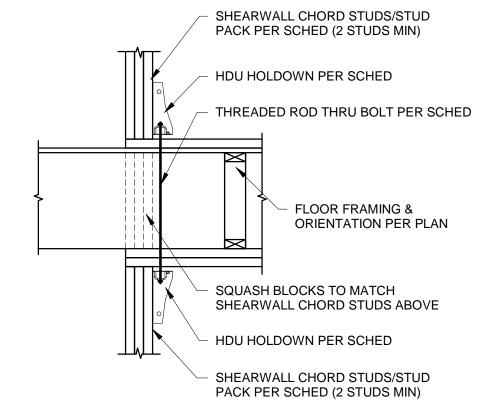
S003

TYPICAL WOOD GRAVITY
SCHEDULES & DETAILS



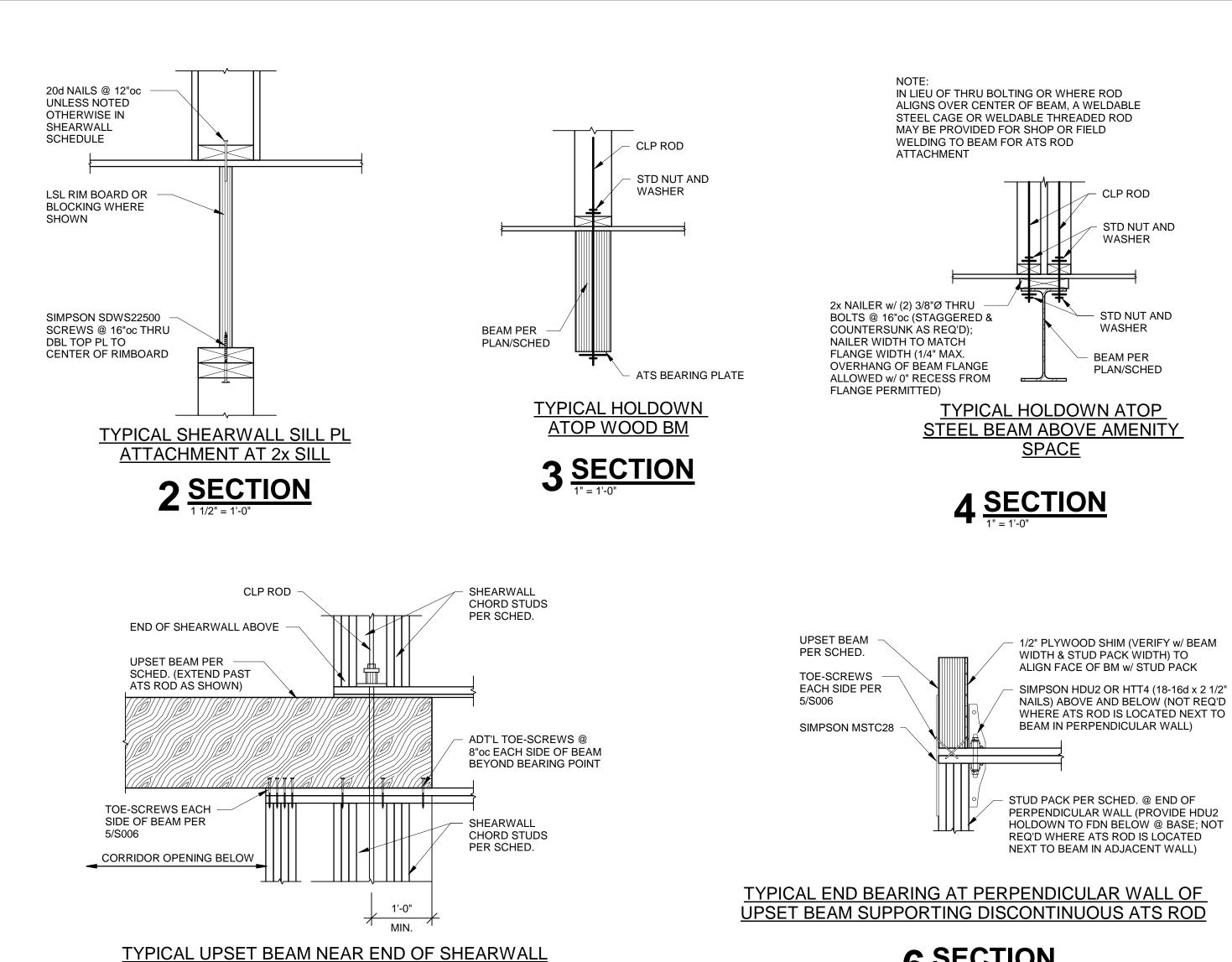
TYPICAL ANCHOR TIEDOWN SYSTEM DETAIL AT EACH END OF SHEAR WALLS REFER TO PLANS FOR SHEAR WALL LOCATIONS AND TO SHEAR WALL SCHEDULE ON THIS SHEET FOR ADDITIONAL INFORMATION.

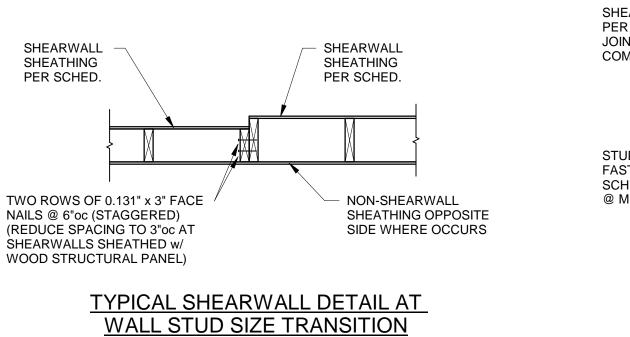
1 DETAIL 3/4" = 1'-0"



TYPICAL HDU FLOOR TO FLOOR HOLDOWN

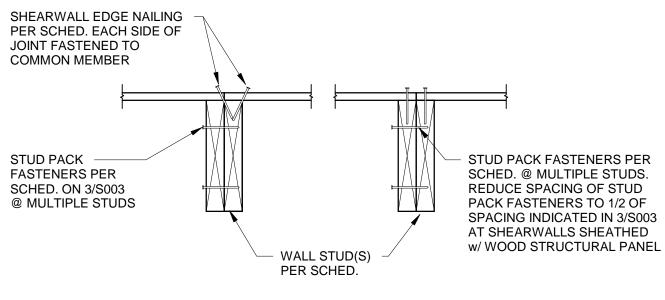
$7 \frac{\text{SECTION}}{3/4" = 1'-0"}$







5 SECTION 3/4" = 1'-0"



TYPICAL SHEARWALL SHEATHING JOINT

9 <u>SECTION</u>

SHEARWALL TYPE		FLOOR			PLATE CONNECTION (SILL TO RIM BOARD & RIM
		1st FLOOR WALLS	2nd FLOOR WALLS	3rd FLOOR WALLS	BOARD TO TOP PLATE) (RE: NOTES 6 & 7)
SW1	SHEATHING	5/8" GYPSUM SHEATHING FINISH SIDE, w/ EDGES BLOCKED 6d NAILS @ 7/7	5/8" GYPSUM SHEATHING FINISH SIDE, w/ EDGES BLOCKED 6d NAILS @ 7/7	5/8" GYPSUM SHEATHING FINISH SIDE, w/ EDGES BLOCKED 6d NAILS @ 7/7	
	& FASTENING	7/16" OSB SHEATHING AIR SIDE, w/ EDGES BLOCKED 6d NAILS @ 6/12	7/16" OSB SHEATHING AIR SIDE, w/ EDGES BLOCKED 6d NAILS @ 6/12	7/16" OSB SHEATHING AIR SIDE, w/ EDGES BLOCKED 6d NAILS @ 6/12	
	HOLDOWN	3/8"Ø STANDARD THREADED ROD w/ (2)2x4 OR (2)2x6	3/8"Ø STANDARD THREADED ROD w/ (2)2x4 OR (2)2x6	3/8"Ø STANDARD THREADED ROD w/ (2)2x4 OR (2)2x6	
SW2		5/8" GYPSUM SHEATHING FINISH SIDE, w/ EDGES BLOCKED 6d NAILS @ 7/7	5/8" GYPSUM SHEATHING FINISH SIDE, w/ EDGES BLOCKED 6d NAILS @ 7/7	5/8" GYPSUM SHEATHING FINISH SIDE, w/ EDGES BLOCKED 6d NAILS @ 7/7	2nd FLR - 20d NAILS @ 8"oc
		7/16" OSB SHEATHING AIR SIDE, w/ EDGES BLOCKED 6d NAILS @ 6/12	7/16" OSB SHEATHING AIR SIDE, w/ EDGES BLOCKED 6d NAILS @ 6/12	7/16" OSB SHEATHING AIR SIDE, w/ EDGES BLOCKED 6d NAILS @ 6/12	
	HOLDOWN	5/8"Ø STANDARD THREADED ROD w/ (6)2x4 OR (4)2x6	3/8"Ø STANDARD THREADED ROD w/ (2)2x4 OR (2)2x6	3/8"Ø STANDARD THREADED ROD w/ (2)2x4 OR (2)2x6	
SW3		5/8" GYPSUM SHEATHING FINISH SIDE, w/ EDGES BLOCKED 6d NAILS @ 7/7	5/8" GYPSUM SHEATHING FINISH SIDE, w/ EDGES BLOCKED 6d NAILS @ 7/7	5/8" GYPSUM SHEATHING FINISH SIDE, w/ EDGES BLOCKED 6d NAILS @ 7/7	3rd FLR - 20d NAILS @ 8"oc 2nd FLR - 20d NAILS @ 6"oc
		7/16" OSB SHEATHING AIR SIDE, w/ EDGES BLOCKED 6d NAILS @ 6/12	7/16" OSB SHEATHING AIR SIDE, w/ EDGES BLOCKED 6d NAILS @ 6/12	7/16" OSB SHEATHING AIR SIDE, w/ EDGES BLOCKED 6d NAILS @ 6/12	
	HOLDOWN	5/8"Ø STANDARD THREADED ROD w/ (6)2x4 OR (4)2x6	3/8"Ø STANDARD THREADED ROD w/ (2)2x4 OR (2)2x6	3/8"Ø STANDARD THREADED ROD w/ (2)2x4 OR (2)2x6	

- 1. NAILING SHALL BE TO ALL STUDS, TOP & BOTTOM PLATES, AND BLOCKING WHERE INDICATED. NAILS FOR GYPSUM SHEATHING ARE COOLER NAILS AND NAILS FOR OSB SHEATHING ARE COMMON NAILS. GYPSUM CAN BE ATTACHED WITH DRYWALL SCREWS AT SAME SPACING INDICATED FOR NAILS.
- . HOLDOWNS PER PLAN & SCHEDULE. 3. WHERE THE ENDS OF PERPENDICULAR SHEAR WALLS INTERSECT AND ONLY ON HOLDOWN SHOWN ON PLAN, FASTEN ALL STUDS TOGETHER PER SCHEDULE AND USE LARGER OF THE TWO HOLDOWNS SHOWN IN THE SHEARWALL SCHEDULE.
- 4. REFER TO HOLDOWN SCHEDULE FOR NUMBER OF STUDS REQ'D AT EA END OF THE SHEARWALL 5. NAIL AND STAPLE SPACING SHOWN AS (#/#) INDICATES FASTENERS SPACING IN INCHES AT THE EDGES/FIELD WHERE FIELD IS THE INTERMEDIATE MEMBERS. TYPICAL SILL PLATE TO WOOD (RIM BOARD) SHALL BE 20d NAILS AT 12"oc UNLESS NOTED OTHERWISE IN SCHEDULE. TYPICAL WOOD (RIM BOARD) TO TOP
- PLATES SHALL BE SDWS22500 SCREWS @ 16"oc. 7. TYPICAL SILL PLATE TO CONCRETE SHALL BE 1/2"Øx6" Lg SIMPSON TITEN HD ANCHOR: AT 2x4 WALLS SPACE AT 24"oc MAX WITH 1/4"x2 1/2"x2 1/2" PLATE WASHER OR
 - SIMPSON BPS1/2-3 @ CONTRACTORS OPTION AT 2x6 WALLS SPACE AT 24"oc MAX WITH 1/4"x2 1/2"x4 1/2" PLATE WASHER OR
 - SIMPSON BPS1/2-6 @ CONTRACTORS OPTION AT 2x8 WALLS STAGGER AT 18"oc MAX WITH 1/4"x2 1/4"x2 1/2" PLATE WASHER OR
- SIMPSON BPS1/2-3 @ CONTRACTORS OPTION 8. PLATE WASHERS TO MAINTAIN MAX OF 1/2" BETWEEN EDGE OF SILL PLATE AND EDGE OF PLATE WASHER.
- 9. OSB @ INTERIOR WALL SHALL BE IN ADDITION TO 5/8" GYP SHEATHING. 10. SHEARWALL SHEATHING CALLED OUT AT CORRIDOR WALLS SHALL BE LOCATED AT UNIT SIDE OF WALL.
- 11. HOLDOWN TYPES ARE BASED UPON CONTINUOUS THREADED RODS UTILIZING CLP SYSTEMS. 12. RODS SHALL HAVE THE FOLLOWING MINIMUM TENSILE CAPACITY:
- A. 3/8"Ø = 2,900 LBS B. 1/2"Ø = 4,270 LBS
- C. 5/8"Ø = 6,675 LBS D. 3/4"Ø = 9,610 LBS 13. REFER TO SECTION DETAILS ON S004 FOR TYPICAL HOLDOWN DETAILS.
- 14. ALL RODS TO HAVE HALF OF THE LISTED REQ'D STUDS EA SIDE OF THREADED ROD TO MATCH STUD SIZE & GRADE NOTED IN WALL SCHEDULE. CHORD STUDS ARE IN ADDITION TO BEARING WALL OR HEADER JAMB STUDS - PROVIDE ADDITIONAL STUDS AS REQ'D TO MEET QUANTITY NOTED IN SCHED. OFFSET STUD PACK 3" TYPICAL FROM CENTERLINE OF THREADED ROD. PROVIDE SQUASH BLOCKS WITHING FLOOR PLATE DEPTH (TRUSS DEPTH) ALIGNED WITH STUD PACKS. QUANITY OF
- SQUASH BLOCK TO MATCH QUANITY OF STUDS BELOW. 15. PROVIDE PLATE WASHER AND NUT CAPABLE OF DEVELOPING CAPACITY OF ROD AT EACH FLOOR.
- 16. PROVIDE TAKE-UP DEVICE AT EACH FLOOR CAPABLE OF ACCOMODATING THE SHRINKAGE INDICATED IN DETAIL 1/S004. 17. PROVIDE COUPLING TAKE-UP DEVICE AS REQUIRED. 18. PROVIDE SHOP DRAWINGS SHOWING LOCATIONS OF ALL HOLD-DOWNS AND HARDWARE FOR REVIEW BY THE EOR PRIOR
- TO INSTALLATION. 19. THE HOLE THRU THE TOP AND SILL PLATES SHALL BE EQUAL TO THE ROD DIAMETER PLUS 1/4".

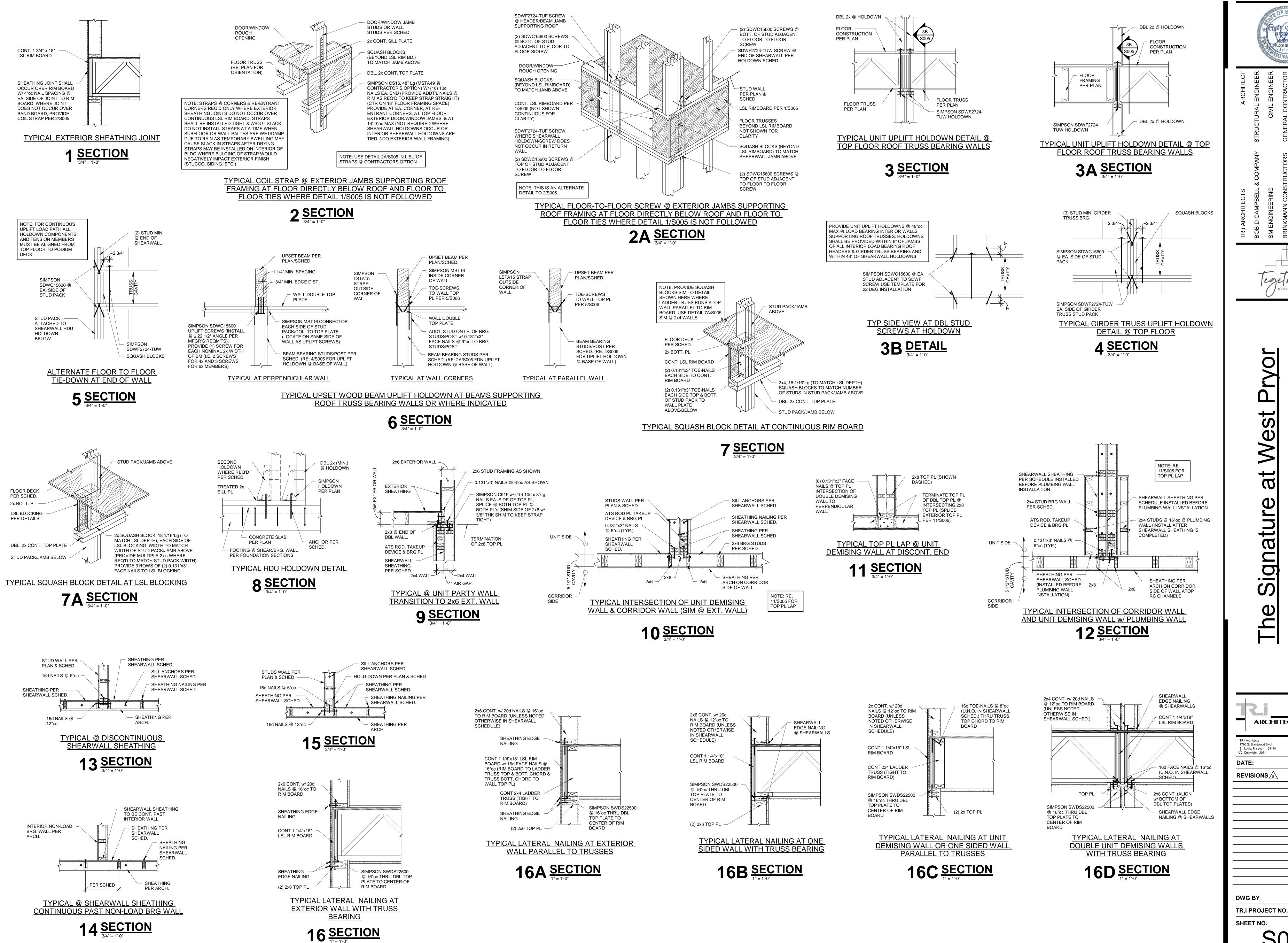
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100 AND 2150 NW LOWENSTEIN LEE'S SUMMIT, MISSOURI 6408 West at Signature The ARCHITECTS www.triarchitects.com 7.15.2021

20-001 SHEET NO. S005

TYPICAL WOOD DETAILS

13 <u>SECTION</u>

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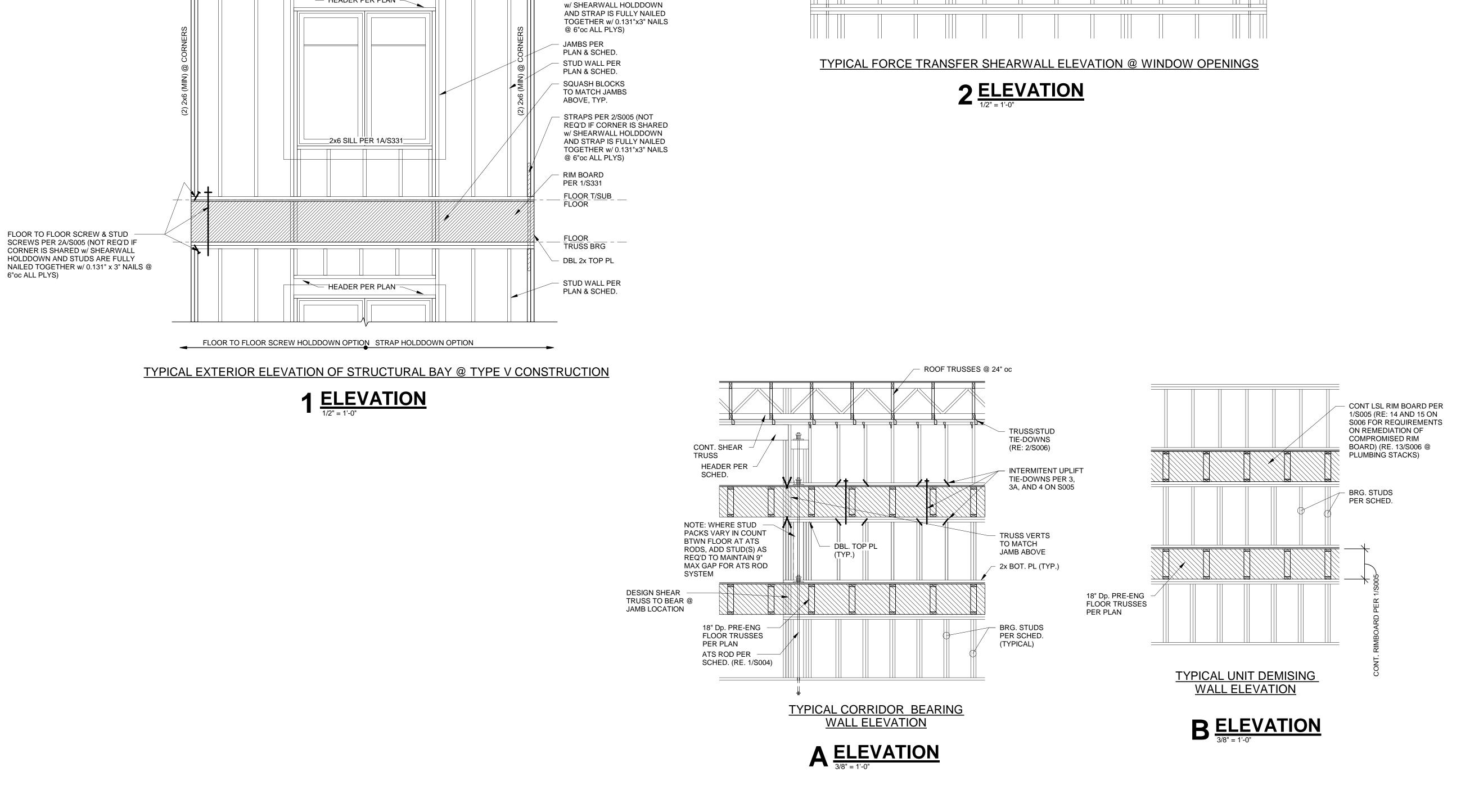
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TYPICAL WOOD DETAILS



ATS TAKE-UP DEVICE TYP - ATS BEARING PLATE TYP

- ATS ROD

- ATS COUPLER

TYP

HEADER PER SCHED.

FLAT 2x4 BLOCKING ALIGNED

w/ WINDOW SILL 2 BAYS MIN.

- CS16 COIL STRAP INSTALLED

OVER SHEATHING w/ 13" MIN

BLOCKING AND SILL/HEADER

(EXTEND 2 BAYS MIN. PAST

ALIGNED w/ WINDOW SILL 2 BAYS MIN. PAST R.O. EA.

SILL PER DETAILS

END LENGTHS, & 8d x 2 1/2"

NAILS AT ALL HOLES TO

PAST R.O. EA. SIDE

R.O. EA. SIDE)

FLAT 2x4 BLOCKING

HOLDDOWN @ EA TRUSS

& STUD PER 2/S006

FLOOR TO FLOOR SCREW AND STUD SCREWS PER 2A/S005 (TYP. @ END

JAMBS OF ROOF BEARING

HEADERS)

FLOOR TO FLOOR SCREW & STUD SCREWS PER 2A/S005 (NOT REQ'D IF

CORNER IS SHARED w/ SHEARWALL HOLDDOWN AND STUDS ARE FULLY

NAILED TOGETHER w/ 0.131" x 3"

NAILS @ 6"oc ALL PLYS)

6"oc ALL PLYS)

ROOF DECK

—HEADER PER PLAN——

-2x6 SILL PER 1A/S331-

HEADER PER PLAN -

PER PLAN

ROOF TRUSS

_ <u>ROOF</u> _ ___ _ _ _ TRUSS BRG

DBL 2x TOP PL HOLDOWN @ EA.

STUD PER 2/S006

STRAP PER 2/S006

STUD WALL PER

PLAN & SCHED.

JAMB PER PLAN & SCHED

STRAPS PER 2/S005 (TYP @

TRUSS BEARING HEADERS)

END JAMBS OF ROOF

SQUASH BLOCKS

TO MATCH JAMBS

ABOVE, TYP.

RIM BOARD

PER 1/S005

CONT. 2x

_FL<u>OOR_T/SUB</u>_ _ FLOOR

FLOOR_ ___ TRUSS BRG

DBL 2x TOP PL

STRAPS PER 2/S005 (NOT REQ'D IF CORNER IS SHARED

PER PLAN



TYPICAL INTERIOR BEARING WALL ELEVATION

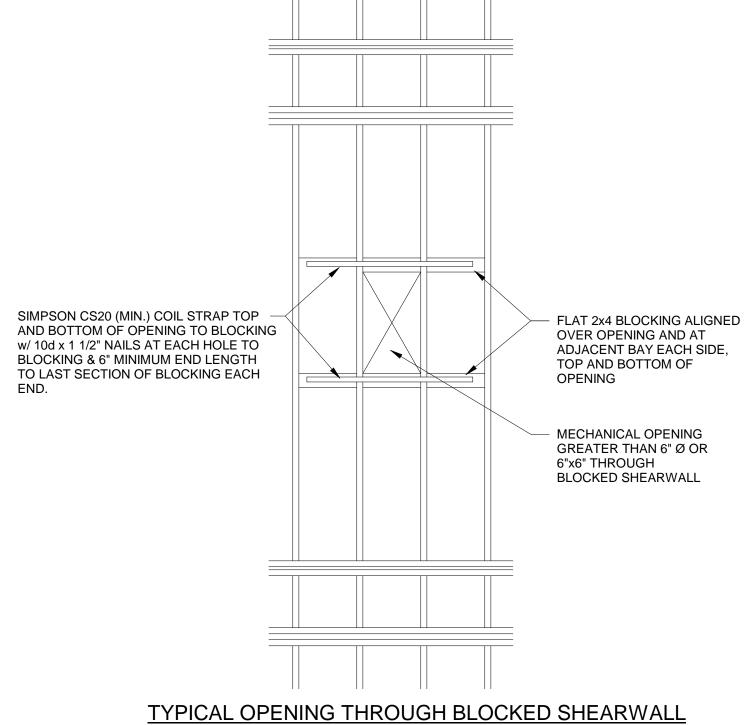
 NON-LOAD BRG STUD
 WALL NEED NOT ALIGN
 w/ FLOOR TRUSSES JAMB PER PLAN BELOW GIRDER TRUSS BEARING NOTE: LSL BLOCKING NOT — REQ'D @ TOP FLOOR BELOW NON-LOAD BEARING WALLS BRG. STUDS -PER SCHED. - HEADER & JAMB PER PLAN/SCHED. 1 1/4"x18" LSL BLOCKING IN — EACH BAY (RE. 14 AND 15 ON S006 FOR REQUIREMENTS (TYPICAL) OF REMEDIATION OF COMPROMISED BLOCKING AT JAMBS OF DISCONT. OPNGS ABOVE, PROVIDE STUD PACK TO MATCH 2x BOT. PL (TYP.) JAMB STUDS ABOVE 18" Dp. FLOOR TRUSSES PER PLUS 50% (MIN.) OF WALL STUDS INDICATED IN WALL STUD SCHEDULE DBL. TOP PL (TYP.) - SQUASH BLOCKS TO ALIGN w/ JAMBS/STUD PACKS ABOVE (TYPICAL) STUD PACK TO MATCH GIRDER TRUSS SUPPORT, TYPICAL AT EACH FLOOR AND ALIGNED DOWN TO FOUNDATION

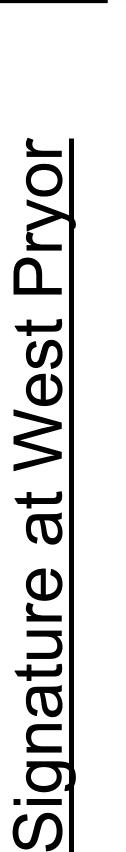
3 **ELEVATION**1/2" = 1'-0"

ROOF GIRDER TRUSS WHERE OCCURS

LGT TYPE HOLDOWN PER

GENERAL NOTE 11-G





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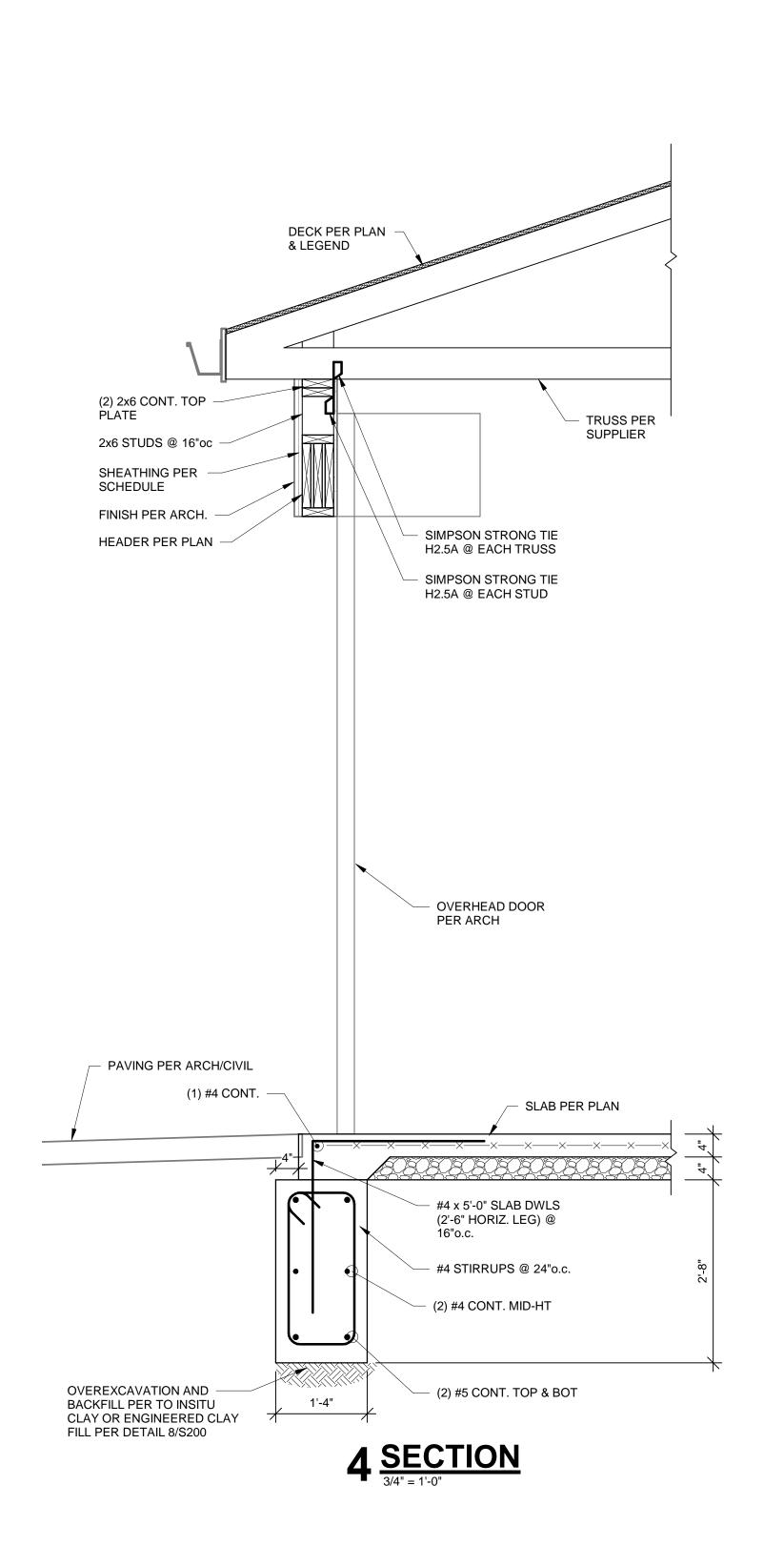
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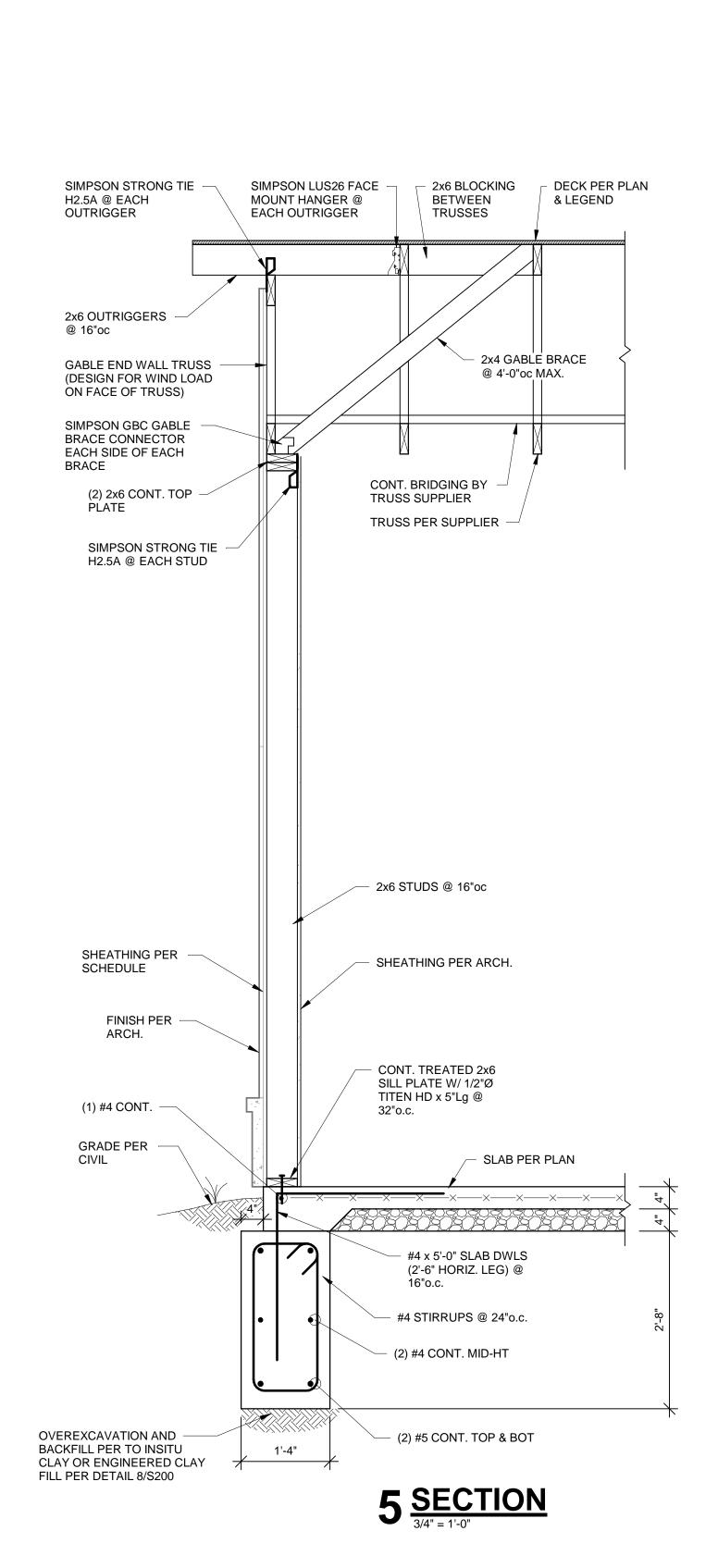
DECK PER PLAN -& LEGEND SIMPSON STRONG TIE H2.5A @ EACH TRUSS (2) 2x6 CONT. TOP -PLATE SIMPSON STRONG TIE H2.5A @ EACH STUD TRUSS PER SUPPLIER -- 2x6 STUDS @ 16"oc SHEATHING PER SCHEDULE - SHEATHING PER ARCH. FINISH PER -ARCH. CONT. TREATED 2x6 SILL PLATE W/ 1/2"Ø TITPN HD x 5"Lg @ (1) #4 CONT. GRADE PER – CIVIL SLAB PER PLAN #4 x 5'-0" SLAB DWLS(2'-6" HORIZ. LEG) @ - #4 STIRRUPS @ 24"o.c. (2) #4 CONT. MID-HT OVEREXCAVATION AND — BACKFILL PER TO INSITU - (2) #5 CONT. TOP & BOT CLAY OR ENGINEERED CLAY FILL PER DETAIL 8/S200

3 <u>SECTION</u>

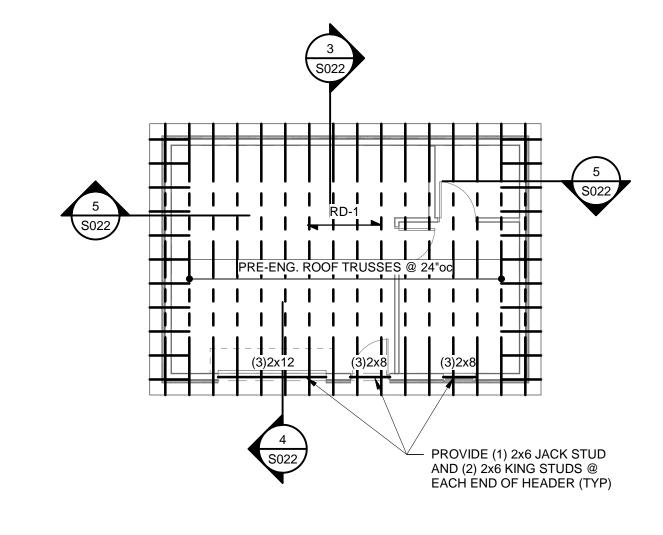
1 MAINTENANCE SHED FOUNDATION PLAN

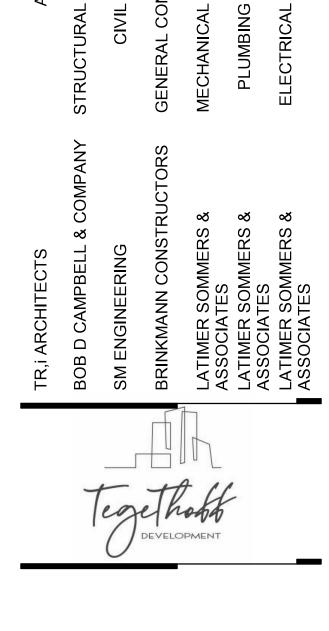
1/8" = 1'-0"





2 MAINTENANCE SHED ROOF FRAMING PLAN 1/8" = 1'-0"



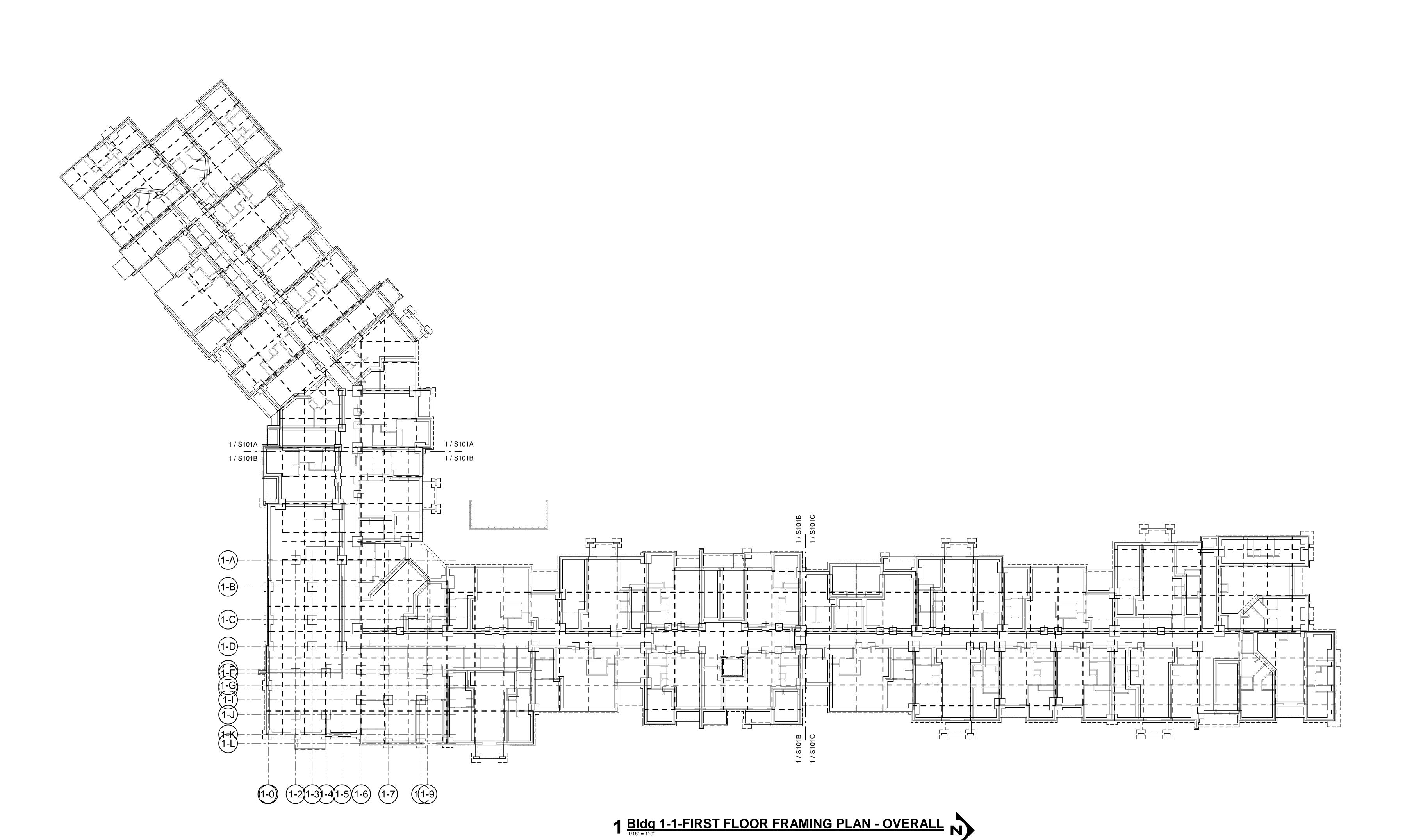


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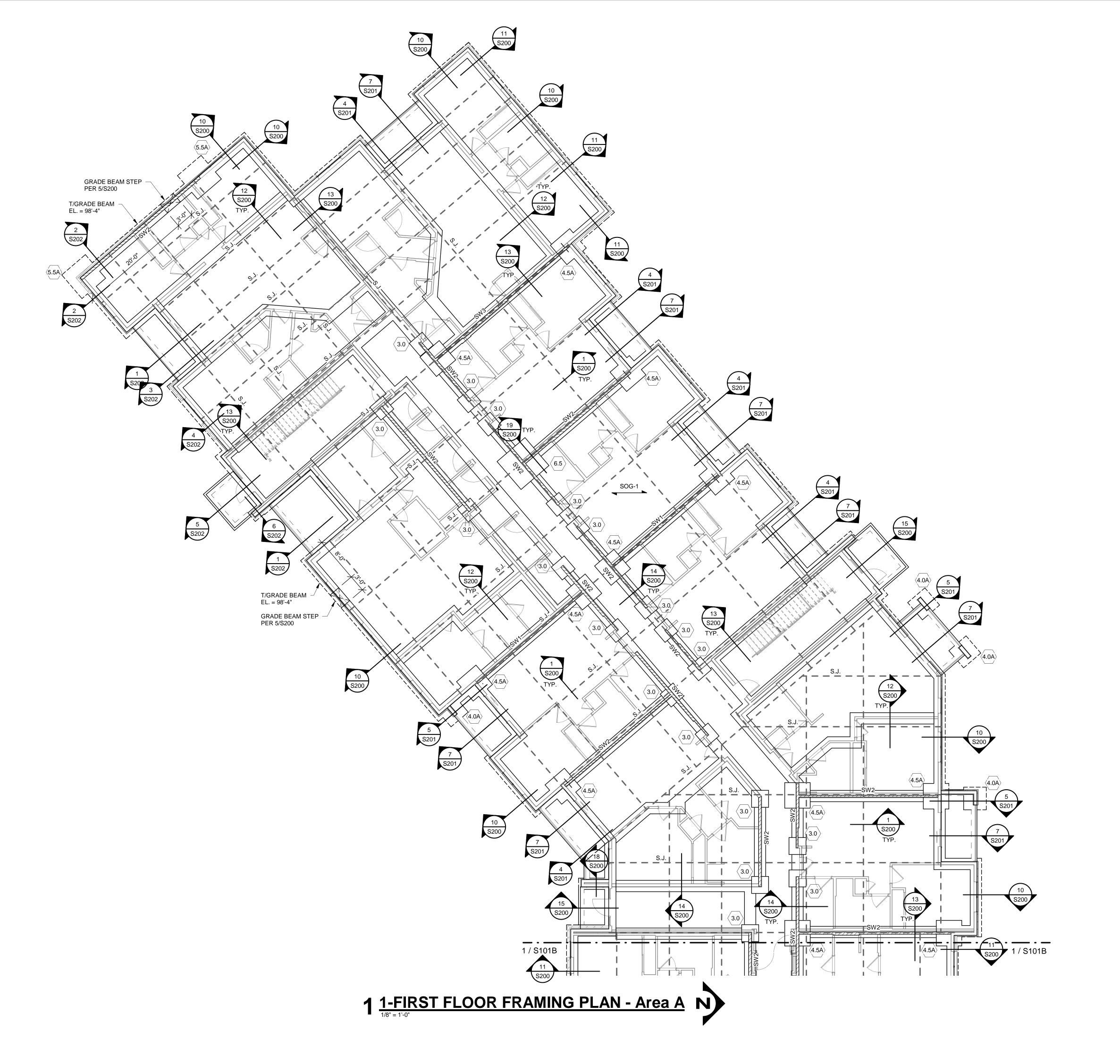


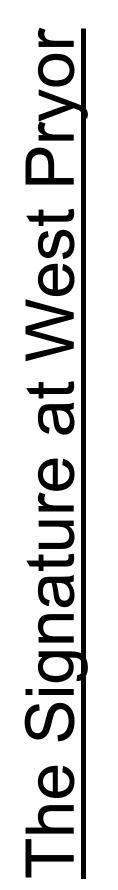


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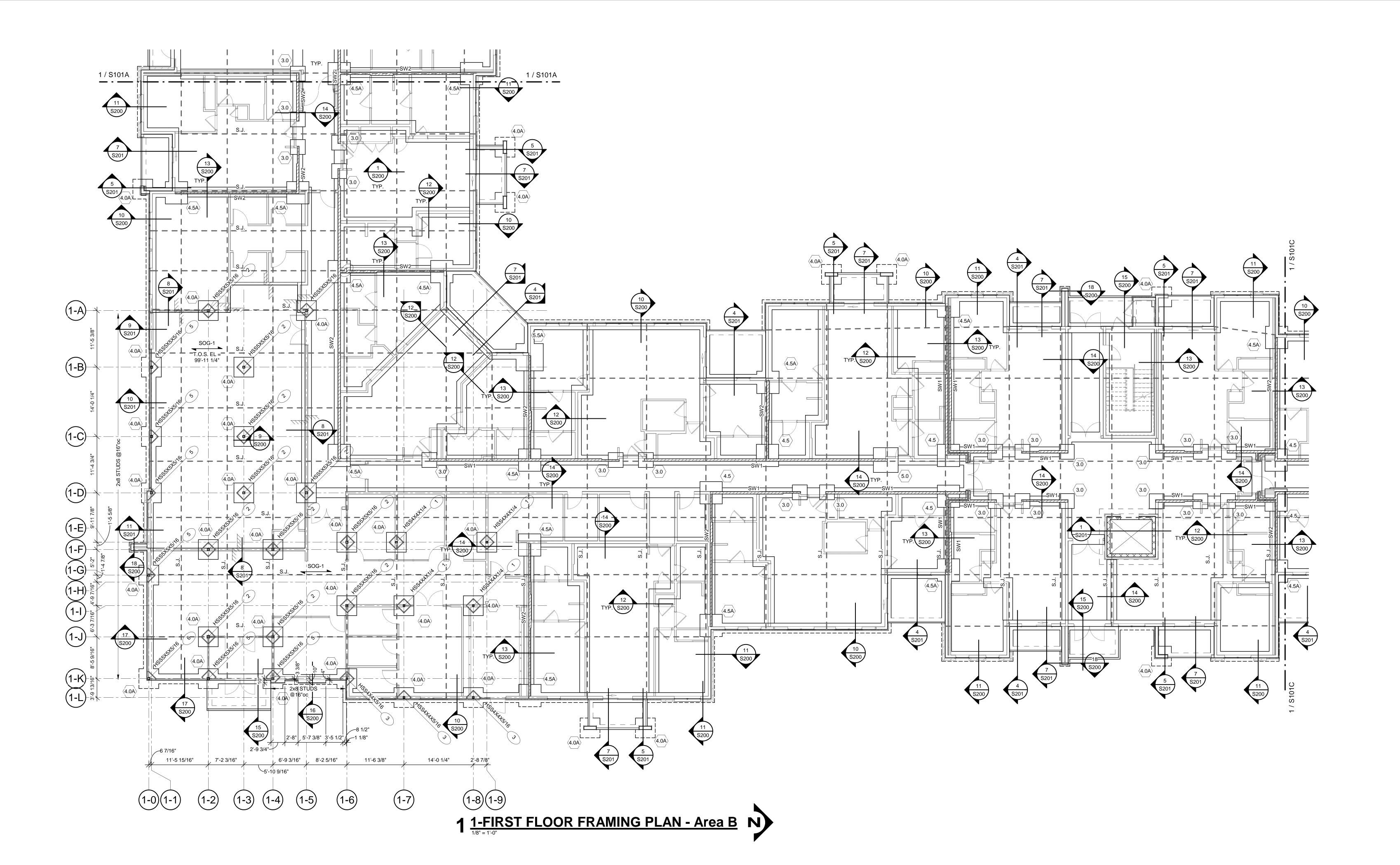
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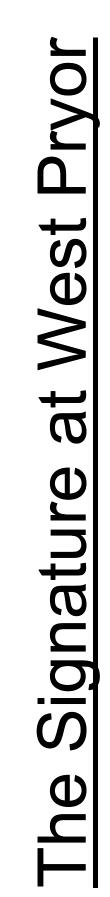




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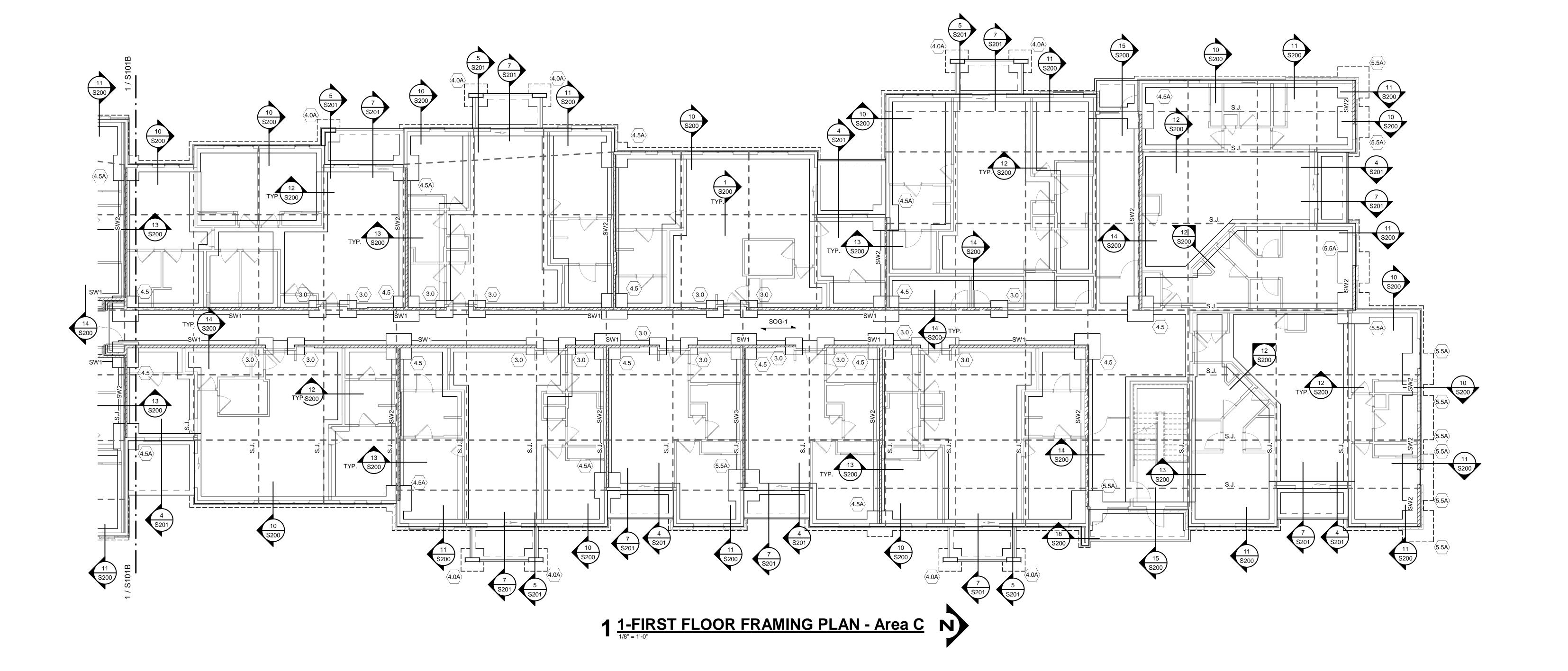
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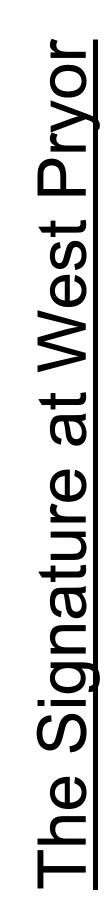
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FOUNDATION PLAN - AREA B

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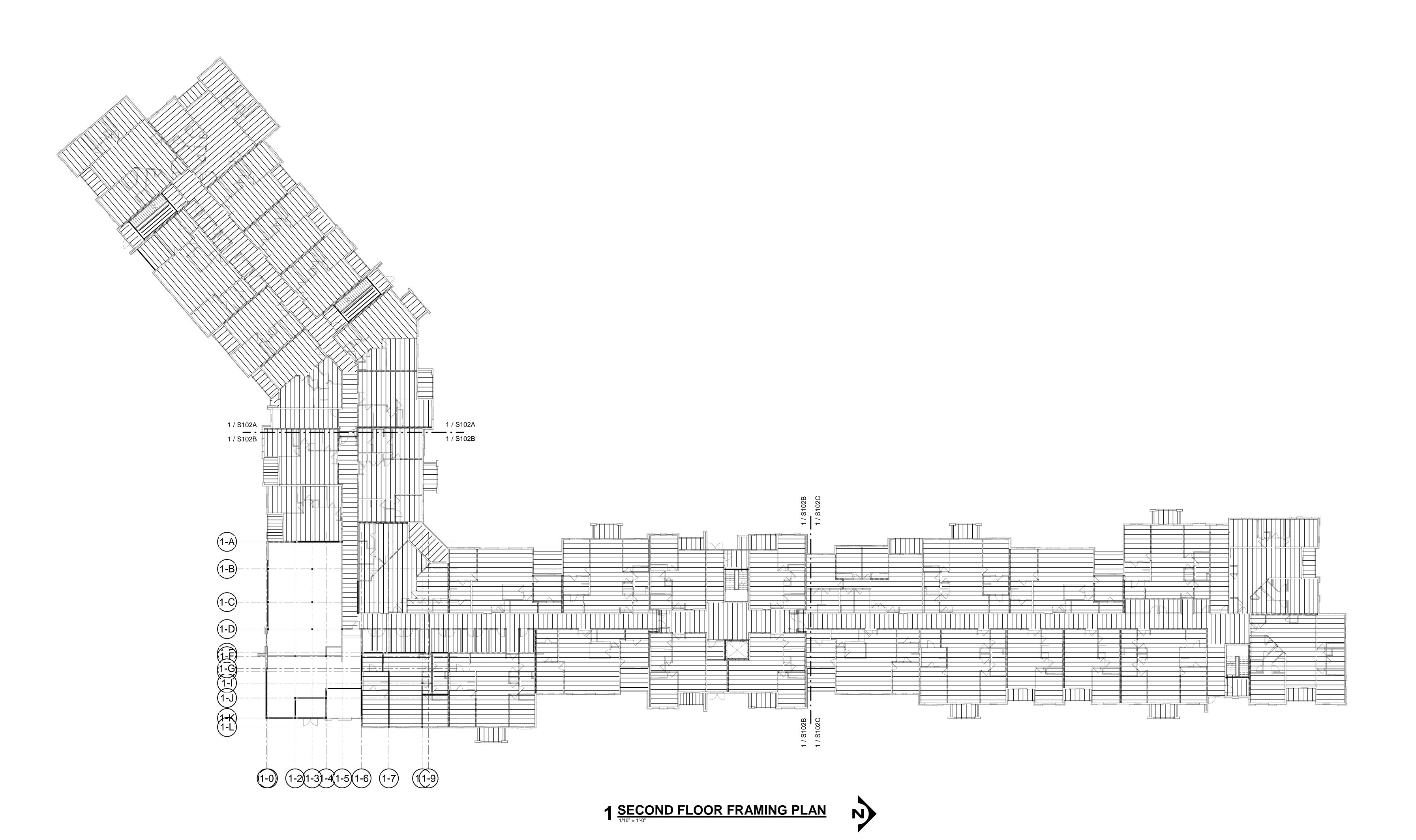
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FOUNDATION PLAN - AREA C



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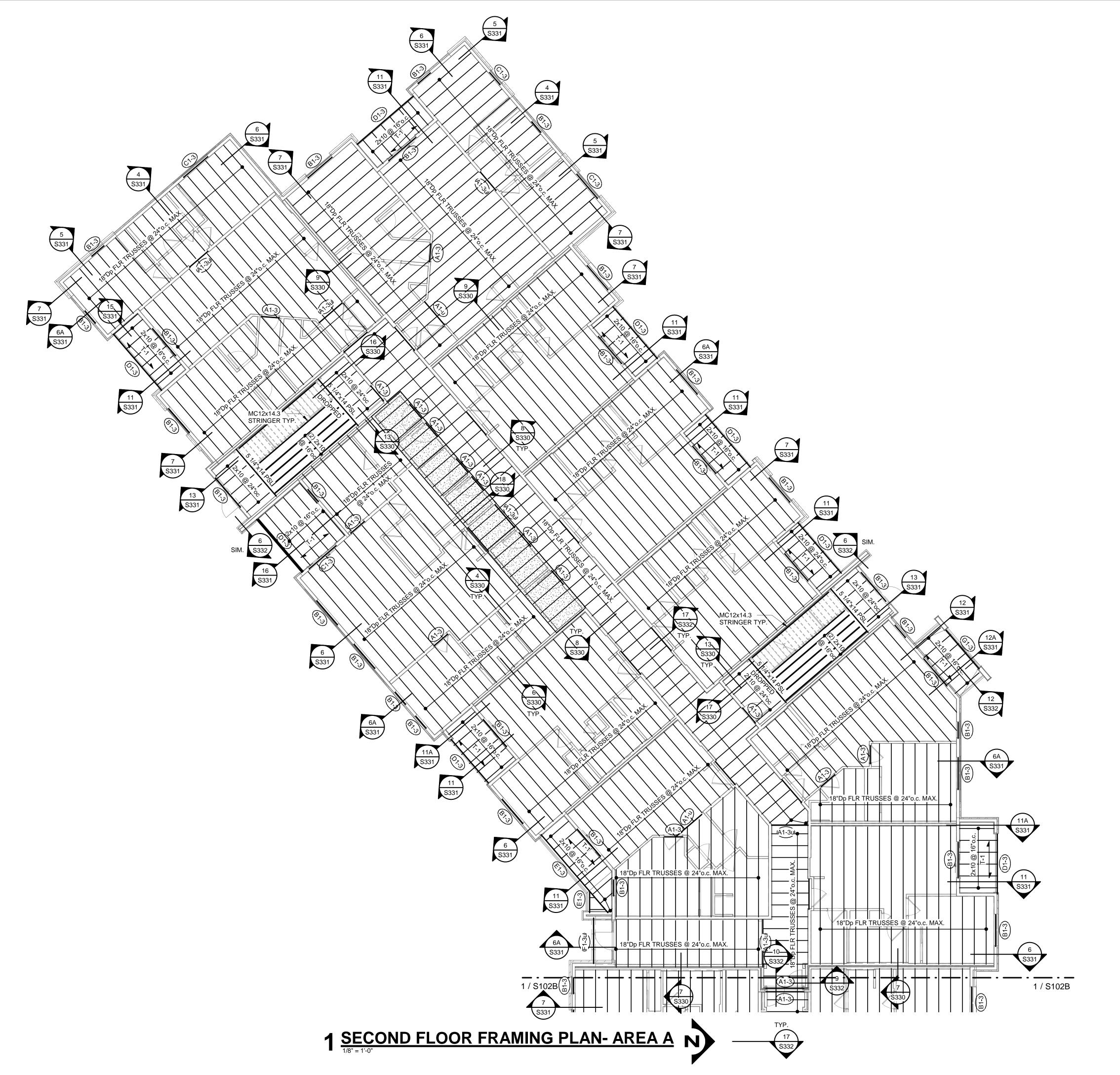
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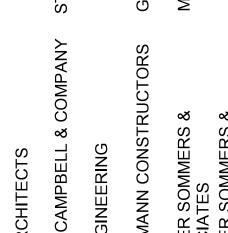


WOOD FLOOR FRAMING

- NOTES:
 REFER TO GENERAL NOTES ON SHEET S001.
 REFER TO STUD BEARING WALL SCHEDULE ON SHEET S003.
 REFER TO HEADER/BEAM/JAMB SCHEDULE ON SHEET S003.
 REFER TO SHEARWALL AND HOLDOWN SCHEDULE ON S004.
 REFER TO S330 SERIES OF SHEETS FOR ADDITIONAL FLOOR FRAMING DETAILS NOT INDICATED HERE
 PROVIDE TRUSS SPACE DIRECTLY ABOVE AND CENTERED OVER HVAC CLOSET; REFER TO ARCH AND MEP DRAWINGS FOR LOCATIONS.
 REFER TO S001 FOR SLAB AND DECK SCHEDULE.

 WOOD FLOOR DECK TO BE DECK TYPE "FD-1" U.N.O.
 BALCONY FLOOR DECK TO BE DECK TYPE "T-1" U.N.O.

 STORAGE AREA: DESIGN FOR LIVE LOAD PER GENERAL NOTE 2.B ON S001





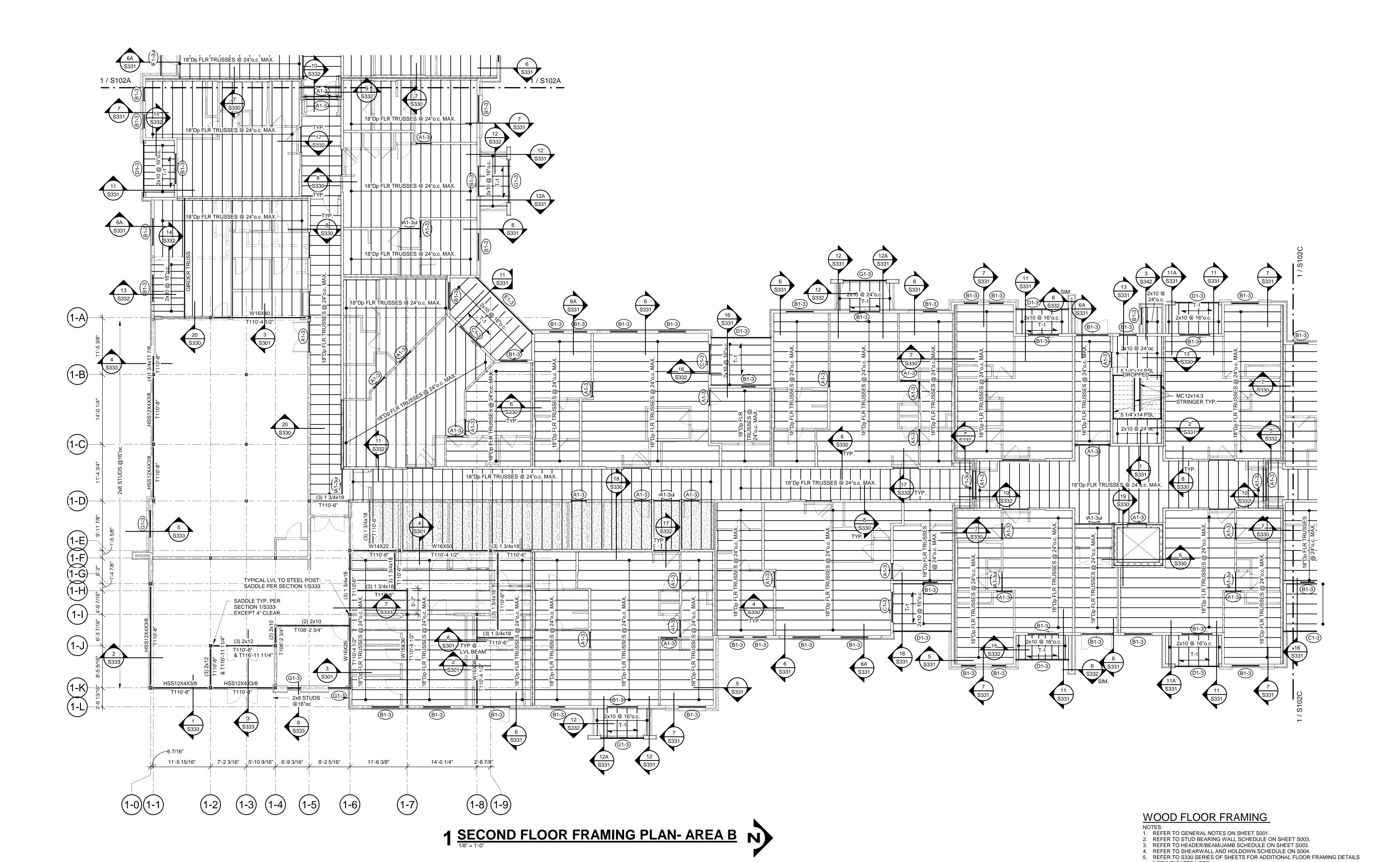
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NOT INDICATED HERE

6. PROVIDE TRUSS SPACE DIRECTLY ABOVE AND CENTERED OVER HVAC CLOSET;

8. PUBLIC AREA: DESIGN FOR LIVE LOAD PER GENERAL NOTE 2.B ON S001

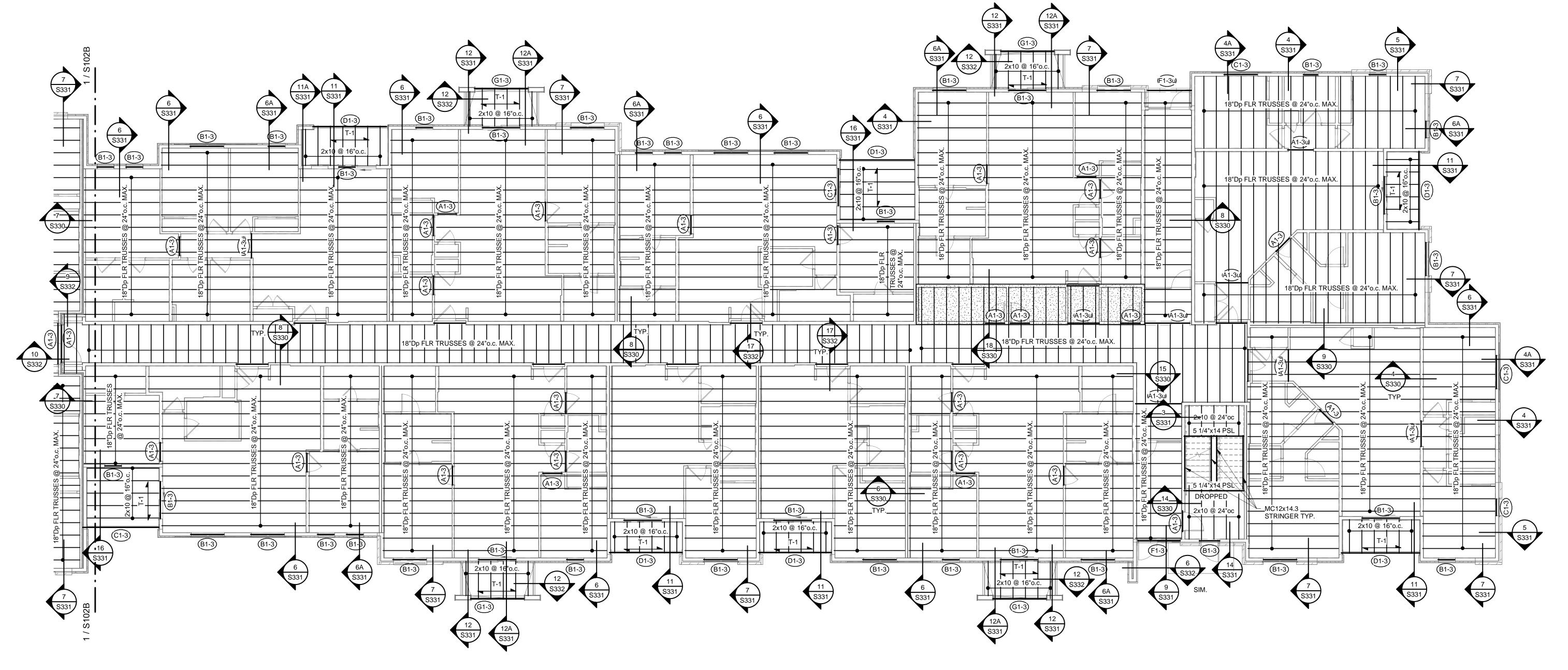
REFER TO ARCH AND MEP DRAWINGS FOR LOCATIONS.

A. WOOD FLOOR DECK TO BE DECK TYPE "FD-1" U.N.O. B. BALCONY FLOOR DECK TO BE DECK TYPE "T-1" U.N.O.

7. REFER TO S001 FOR SLAB AND DECK SCHEDULE.

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SHEET NO.

S102B



1 SECOND FLOOR FRAMING PLAN- AREA C

WOOD FLOOR FRAMING

1. REFER TO GENERAL NOTES ON SHEET S001.

- REFER TO STUD BEARING WALL SCHEDULE ON SHEET S003.
 REFER TO HEADER/BEAM/JAMB SCHEDULE ON SHEET S003.
 REFER TO SHEARWALL AND HOLDOWN SCHEDULE ON S004.
- REFER TO SHEARWALL AND HOLDOWN SCHEDULE ON S004.
 REFER TO S330 SERIES OF SHEETS FOR ADDITIONAL FLOOR FRAMING DETAILS
- NOT INDICATED HERE
 6. PROVIDE TRUSS SPACE DIRECTLY ABOVE AND CENTERED OVER HVAC CLOSET;
 REFER TO ARCH AND MEP DRAWINGS FOR LOCATIONS.
- REFER TO S001 FOR SLAB AND DECK SCHEDULE.
 A. WOOD FLOOR DECK TO BE DECK TYPE "FD-1" U.N.O.
- B. BALCONY FLOOR DECK TO BE DECK TYPE "T-1" U.N.O.

 8. PUBLIC AREA: DESIGN FOR LIVE LOAD PER GENERAL NOTE 2.B ON S001 STORAGE AREA: DESIGN FOR LIVE LOAD PER GENERAL NOTE 2.B ON S001



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S102C
SECOND FLOOR FRAMING PLAN-



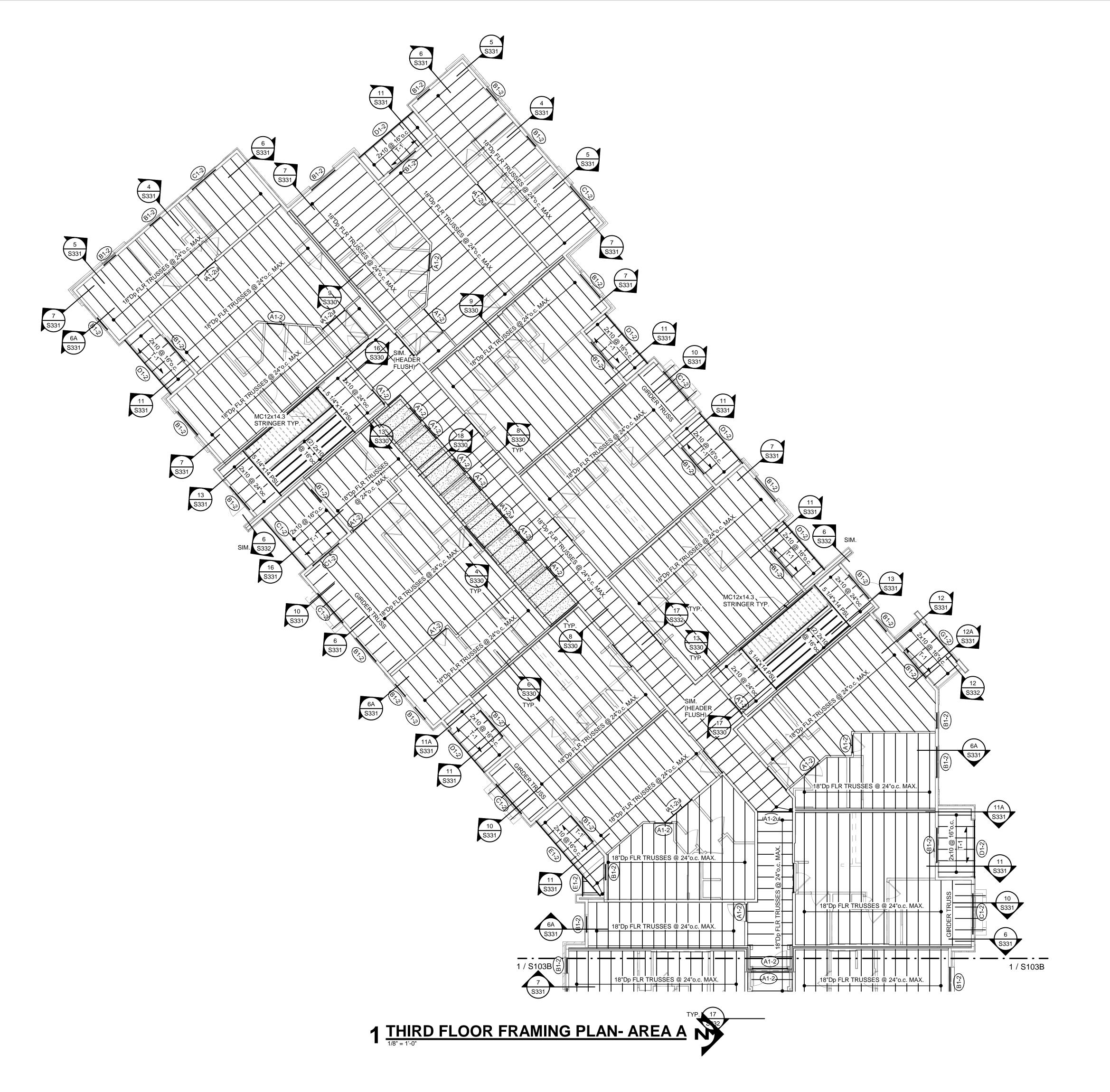
1 THIRD FLOOR FRAMING PLAN

1/16" = 1'-0"

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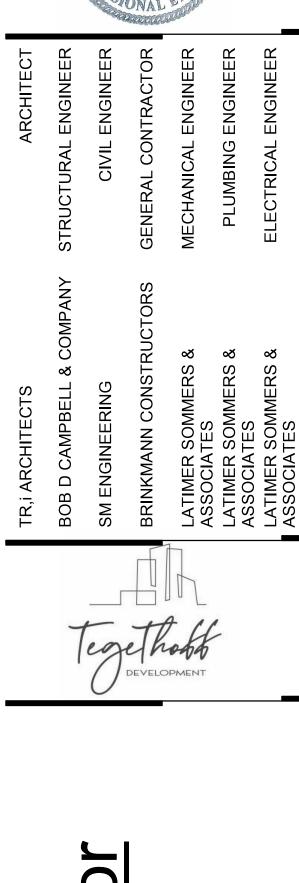
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WOOD FLOOR FRAMING

- REFER TO GENERAL NOTES ON SHEET S001.
 REFER TO STUD BEARING WALL SCHEDULE ON SHEET S003.
 REFER TO HEADER/BEAM/JAMB SCHEDULE ON SHEET S003.
- REFER TO SHEARWALL AND HOLDOWN SCHEDULE ON S004.
 REFER TO S330 SERIES OF SHEETS FOR ADDITIONAL FLOOR FRAMING DETAILS
- NOT INDICATED HERE
- PROVIDE TRUSS SPACE DIRECTLY ABOVE AND CENTERED OVER HVAC CLOSET; REFER TO ARCH AND MEP DRAWINGS FOR LOCATIONS. 7. REFER TO S001 FOR SLAB AND DECK SCHEDULE. A. WOOD FLOOR DECK TO BE DECK TYPE "FD-1" U.N.O. B. BALCONY FLOOR DECK TO BE DECK TYPE "T-1" U.N.O.

8. PUBLIC AREA: DESIGN FOR LIVE LOAD PER GENERAL NOTE 2.B ON S001 STORAGE AREA: DESIGN FOR LIVE LOAD PER GENERAL NOTE 2.B ON S001

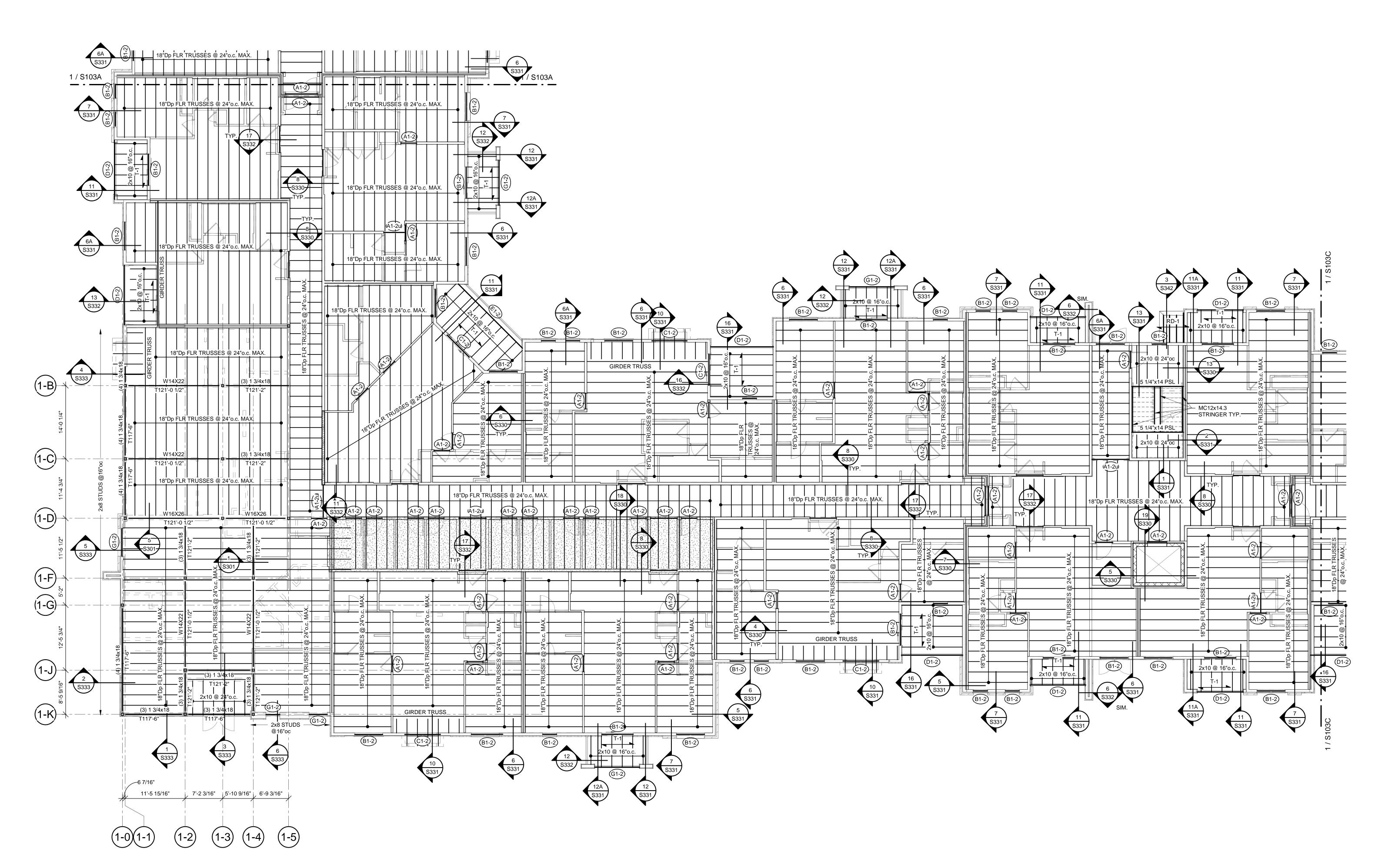


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SHEET NO.	
S103	BA

THIRD FLOOR FRAMING PLAN- AREA



1 THIRD FLOOR FRAMING PLAN- AREA B



WOOD FLOOR FRAMING

- NOTES:

 1. REFER TO GENERAL NOTES ON SHEET S001.

 2. REFER TO STUD BEARING WALL SCHEDULE ON SHEET S003.
- REFER TO HEADER/BEAM/JAMB SCHEDULE ON SHEET S003.
 REFER TO SHEARWALL AND HOLDOWN SCHEDULE ON S004.
 REFER TO S330 SERIES OF SHEETS FOR ADDITIONAL FLOOR FRAMING DETAILS
- REFER TO S330 SERIES OF SHEETS FOR ADDITIONAL FLOOR FRAMING DETAILS NOT INDICATED HERE
 PROVIDE TRUSS SPACE DIRECTLY ABOVE AND CENTERED OVER HVAC CLOSET;
- REFER TO ARCH AND MEP DRAWINGS FOR LOCATIONS.

 7. REFER TO S001 FOR SLAB AND DECK SCHEDULE.
 A. WOOD FLOOR DECK TO BE DECK TYPE "FD-1" U.N.O.
- B. BALCONY FLOOR DECK TO BE DECK TYPE "T-1" U.N.O.

 8. PUBLIC AREA: DESIGN FOR LIVE LOAD PER GENERAL NOTE 2.B ON S001

 STORAGE AREA: DESIGN FOR LIVE LOAD PER GENERAL NOTE 2.B ON S001



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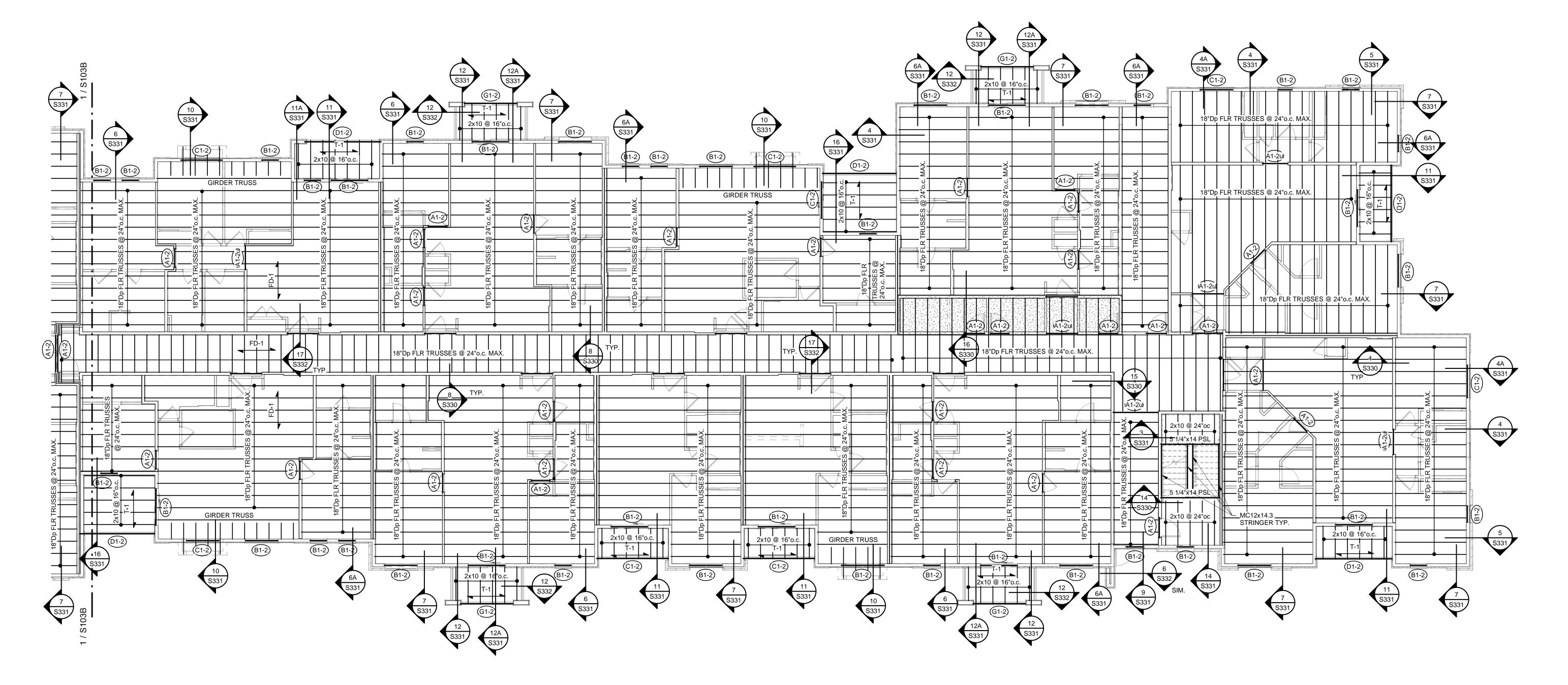
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TR,i PROJECT NO.

7.15.2021

20-001

2100 AND 2150 NW LOWENSTEIN DIEE'S SUMMIT, MISSOURI 64081



1 THIRD FLOOR FRAMING PLAN- AREA C

WOOD FLOOR FRAMING

NOTES:

REFER TO GENERAL NOTES ON SHEET S001.
 REFER TO STUD BEARING WALL SCHEDULE ON SHEET S003.
 REFER TO HEADER/BEAM/ JAMPA SCHEDULE ON SHEET S003.

7. REFER TO S001 FOR SLAB AND DECK SCHEDULE.

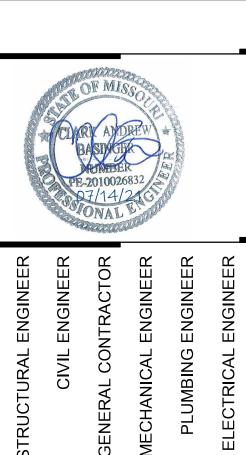
- REFER TO HEADER/BEAM/JAMB SCHEDULE ON SHEET S003.
 REFER TO SHEARWALL AND HOLDOWN SCHEDULE ON S004.
- 5. REFER TO S330 SERIES OF SHEETS FOR ADDITIONAL FLOOR FRAMING DETAILS NOT INDICATED HERE
- NOT INDICATED HERE

 6. PROVIDE TRUSS SPACE DIRECTLY ABOVE AND CENTERED OVER HVAC CLOSET;
 REFER TO ARCH AND MEP DRAWINGS FOR LOCATIONS.
- A. WOOD FLOOR DECK TO BE DECK TYPE "FD-1" U.N.O.

 B. BALCONY FLOOR DECK TO BE DECK TYPE "T-1" U.N.O.

 8. PUBLIC AREA: DESIGN FOR LIVE LOAD PER GENERAL NOTE 2.B ON S001

 STORAGE AREA: DESIGN FOR LIVE LOAD PER GENERAL NOTE 2.B ON S001



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St. Louis, Missouri 63144
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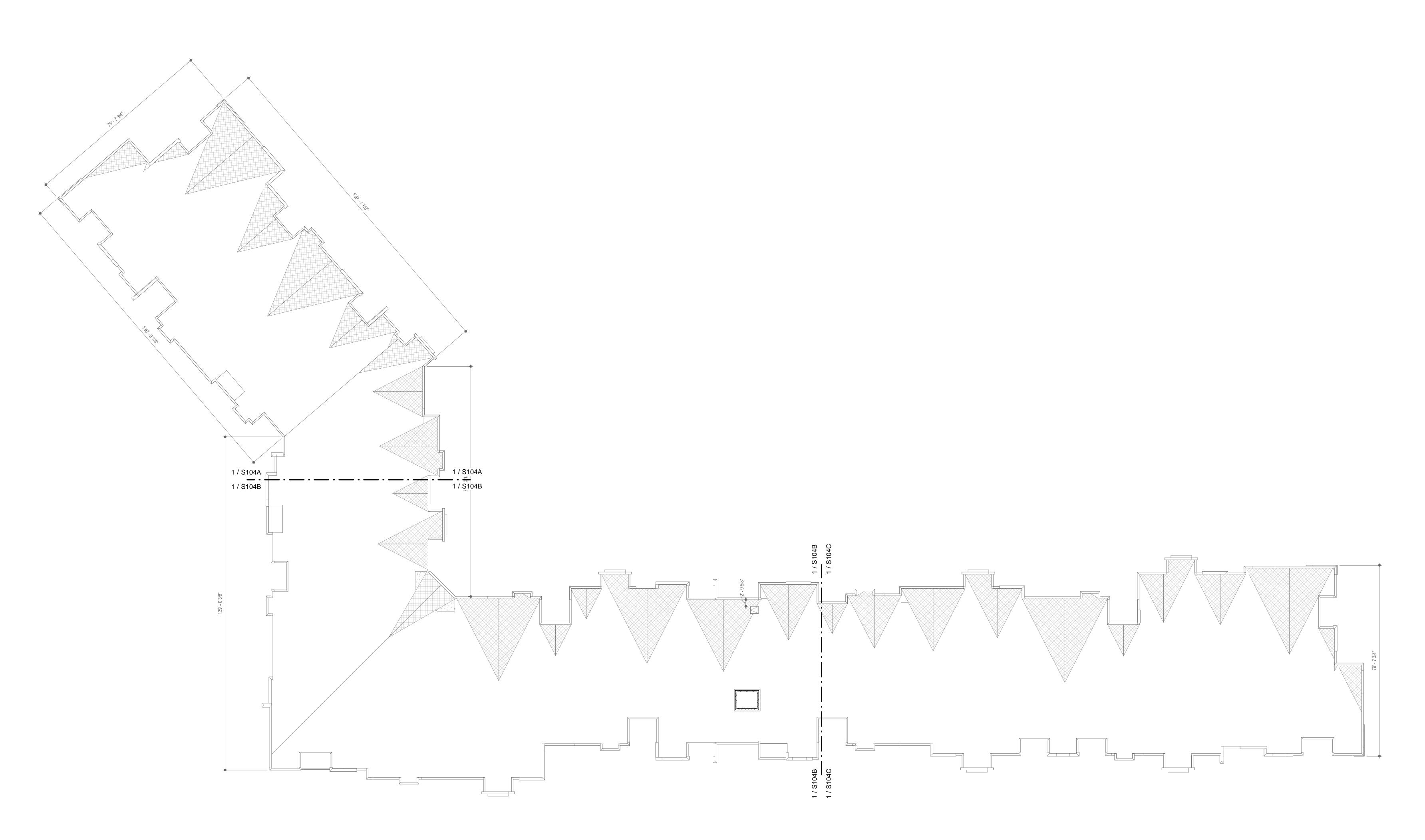
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www.triarchitects.com

T. 7.15.2021

REVISIONS #

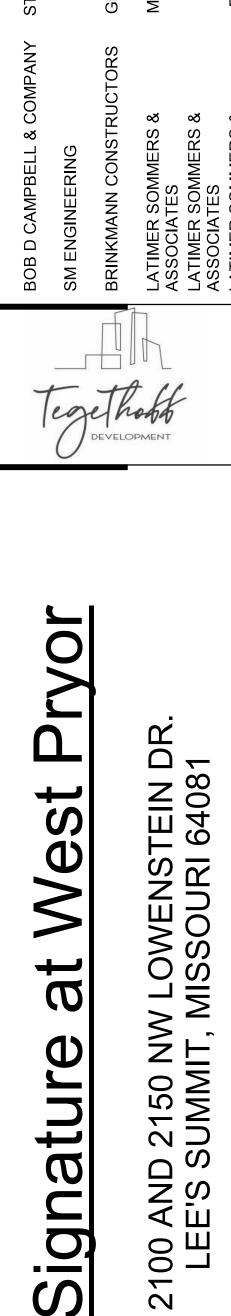
TR,i PROJECT NO. 20-0
SHEET NO.

S103C



1 ROOF FRAMING PLAN

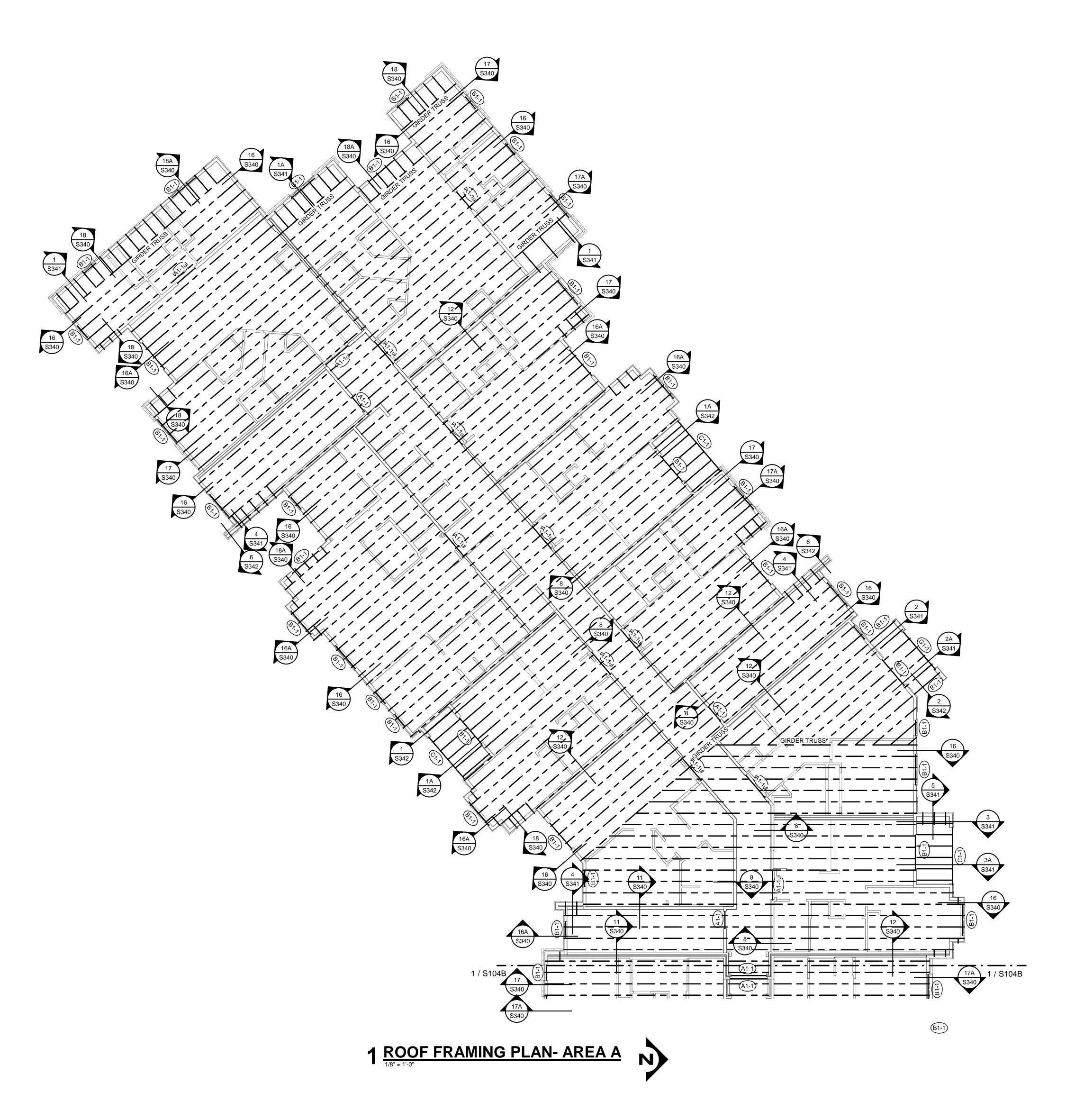
1/16" = 1'-0"



at West The Signature

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TR,i PROJECT NO. SHEET NO.



WOOD ROOF FRAMING

- 1. REFER TO GENERAL NOTES ON SHEET S001.
 2. REFER TO STUD BEARING WALL SCHEDULE ON SHEET S003.
 3. REFER TO HEADER/BEAM/JAMB SCHEDULE ON SHEET S003.
- 4. REFER TO SHEARWALL AND HOLDOWN SCHEDULE ON S004. 5. REFER TO S340 SERIES OF SHEETS FOR ADDITIONAL FLOOR FRAMING DETAILS NOT INDICATED HERE
- 6. PROVIDE (3) STUDS MINIMUM ALIGNED UNDER EACH END OF EACH GIRDER TRUSS. PROVIDE SQUASH BLOCKS AND JAMBS AT EA. FLOOR BELOW DOWN TO FOUNDATION. (TOTAL STUDS TO MATCH NUMBER OF GIRDER TRUSS PLIES (3 MIN.) REFER TO DETAIL 3/S340.
 7. PROVIDE UNIFORM UPLIFT SCREWS @ UPPER FLOOR PER DETAILS 3, 3A, 3B, 4
- AND 5 ON S005. PRE-ENG TRUSSES TO HAVE A MINIMUM DEPTH OF 24". SLOPE TOP CHORD PER ARCHITECTUAL DRAWINGS.
 REFER TO S001 FOR SLAB AND DECK SCHEDULE.

 WOOD ROOF DECK TO BE DECK TYPE "RD-1" U.N.O.

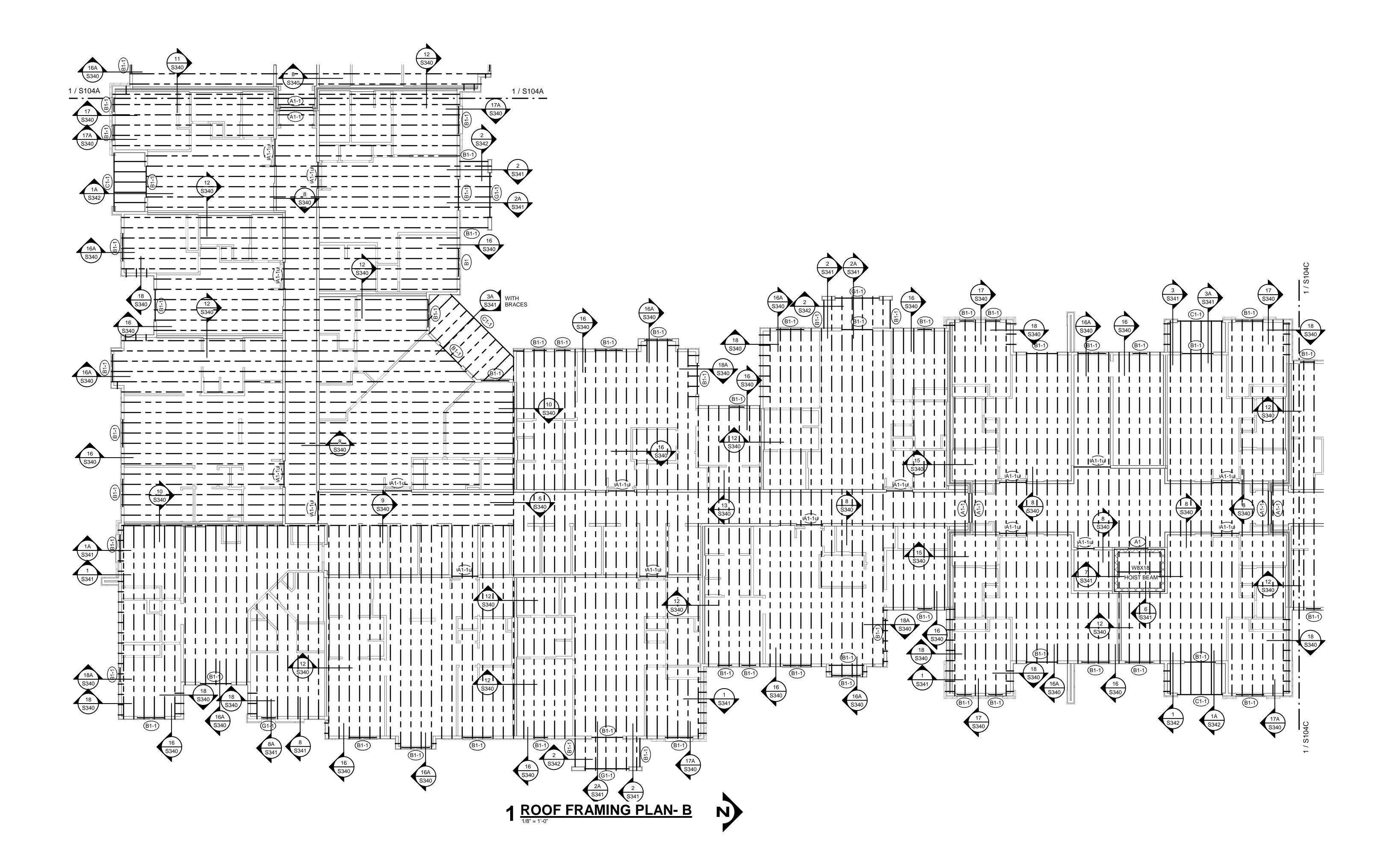


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SHEET NO.



WOOD ROOF FRAMING

- REFER TO GENERAL NOTES ON SHEET S001.
 REFER TO STUD BEARING WALL SCHEDULE ON SHEET S003. REFER TO HEADER/BEAM/JAMB SCHEDULE ON SHEET S003.
 REFER TO SHEARWALL AND HOLDOWN SCHEDULE ON S004.
- 5. REFER TO S340 SERIES OF SHEETS FOR ADDITIONAL FLOOR FRAMING DETAILS
- NOT INDICATED HERE 6. PROVIDE (3) STUDS MINIMUM ALIGNED UNDER EACH END OF EACH GIRDER TRUSS. PRÓVIDE SQUASH BLOCKS AND JAMBS AT EA. FLOOR BELOW DOWN TO FOUNDATION. (TOTAL STUDS TO MATCH NUMBER OF GIRDER TRUSS PLIES
- (3 MIN.) REFER TO DETAIL 3/S340. 7. PROVIDE UNIFORM UPLIFT SCREWS @ UPPER FLOOR PER DETAILS 3, 3A, 3B, 4
- 8. PRE-ENG TRUSSES TO HAVE A MINIMUM DEPTH OF 24". SLOPE TOP CHORD PER ARCHITECTUAL DRAWINGS.

 9. REFER TO S001 FOR SLAB AND DECK SCHEDULE.
- A. WOOD ROOF DECK TO BE DECK TYPE "RD-1" U.N.O.

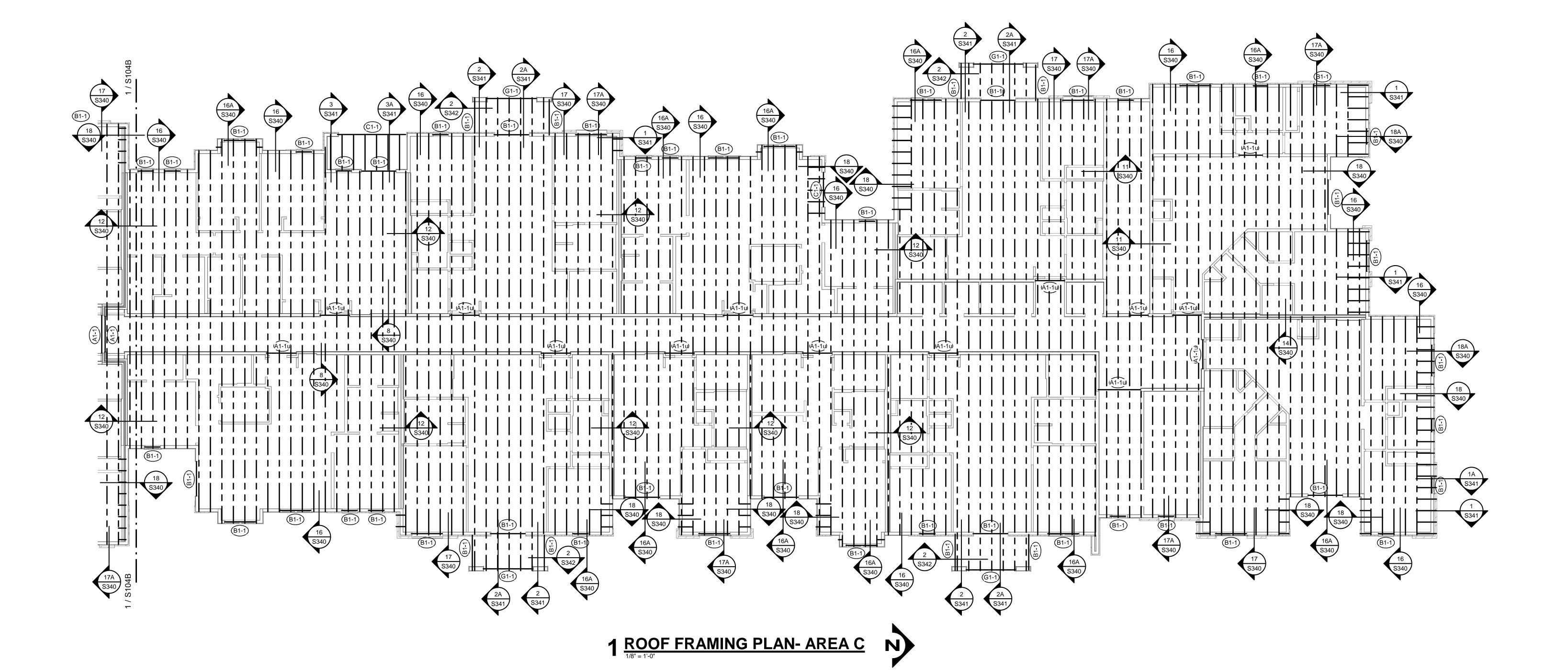


Signature The

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TR,i PROJECT NO.	20-001
SHEET NO	

ROOF FRAMING PLAN- AREA B



WOOD ROOF FRAMING

NOTES:

1. REFER TO GENERAL NOTES ON SHEET S001.

2. REFER TO STUD BEARING WALL SCHEDULE ON SHEET S003. 3. REFER TO HEADER/BEAM/JAMB SCHEDULE ON SHEET S003.

4. REFER TO SHEARWALL AND HOLDOWN SCHEDULE ON S004. 5. REFER TO S340 SERIES OF SHEETS FOR ADDITIONAL FLOOR FRAMING DETAILS

NOT INDICATED HERE

6. PROVIDE (3) STUDS MINIMUM ALIGNED UNDER EACH END OF EACH GIRDER TRUSS. PROVIDE SQUASH BLOCKS AND JAMBS AT EA. FLOOR BELOW DOWN

TO FOUNDATION. (TOTAL STUDS TO MATCH NUMBER OF GIRDER TRUSS PLIES (3 MIN.) REFER TO DETAIL 3/S340.

7. PROVIDE UNIFORM UPLIFT SCREWS @ UPPER FLOOR PER DETAILS 3, 3A, 3B, 4 AND 5 ON S005.

8. PRE-ENG TRUSSES TO HAVE A MINIMUM DEPTH OF 24". SLOPE TOP CHORD PER ARCHITECTUAL DRAWINGS.

9. REFER TO S001 FOR SLAB AND DECK SCHEDULE. A. WOOD ROOF DECK TO BE DECK TYPE "RD-1" U.N.O.



2100 AND 2150 NW LOWENSTEIN DIEE'S SUMMIT, MISSOURI 64081

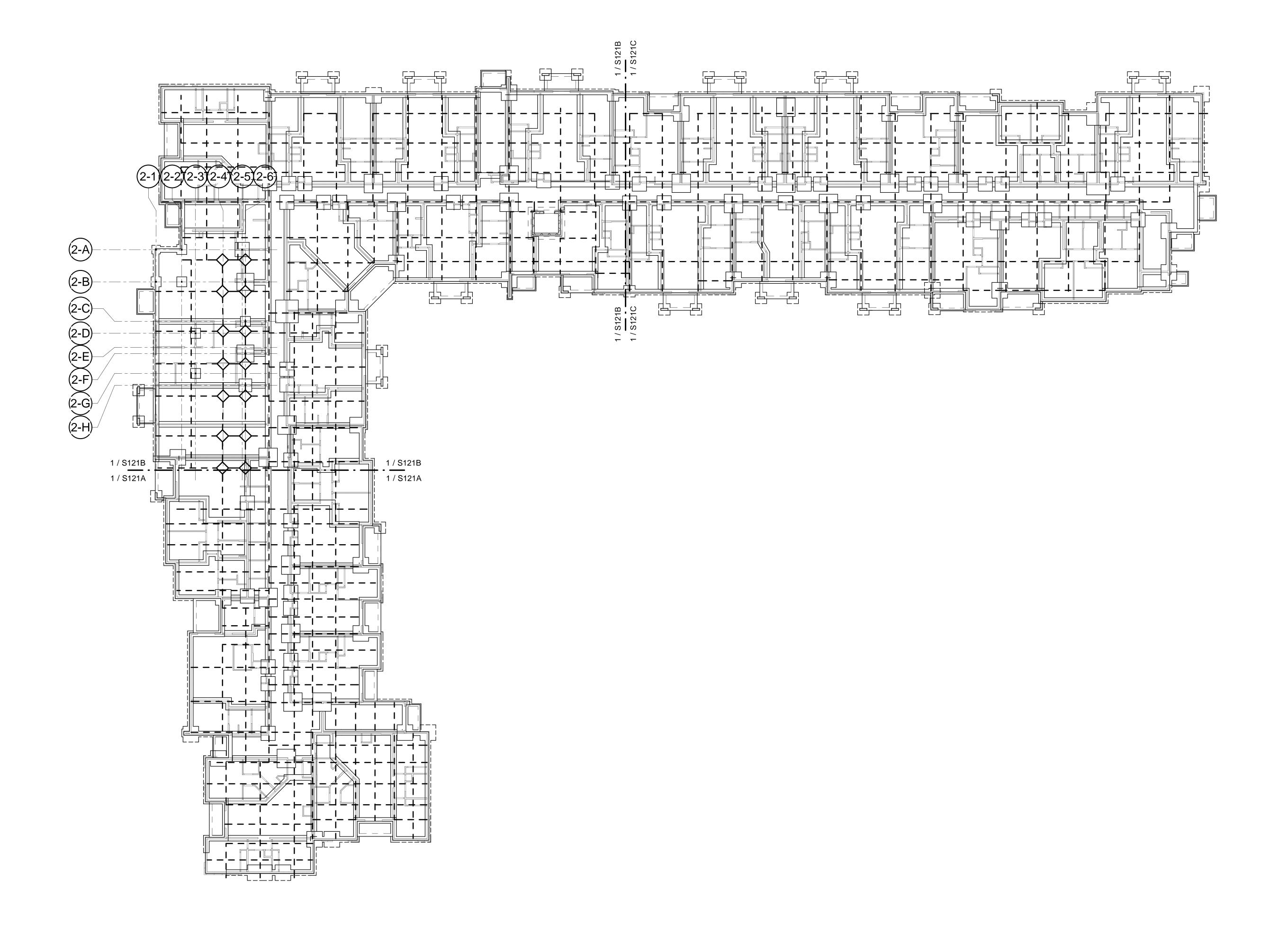
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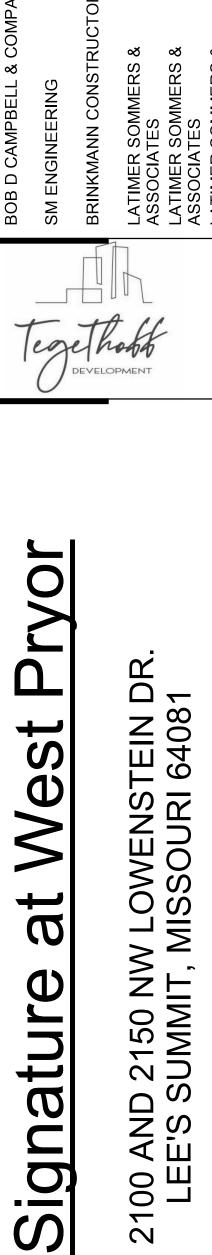
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ROOF FRAMING PLAN- AREA C



1 Bldg 2- FOUNDATION PLAN

1/16" = 1'-0"



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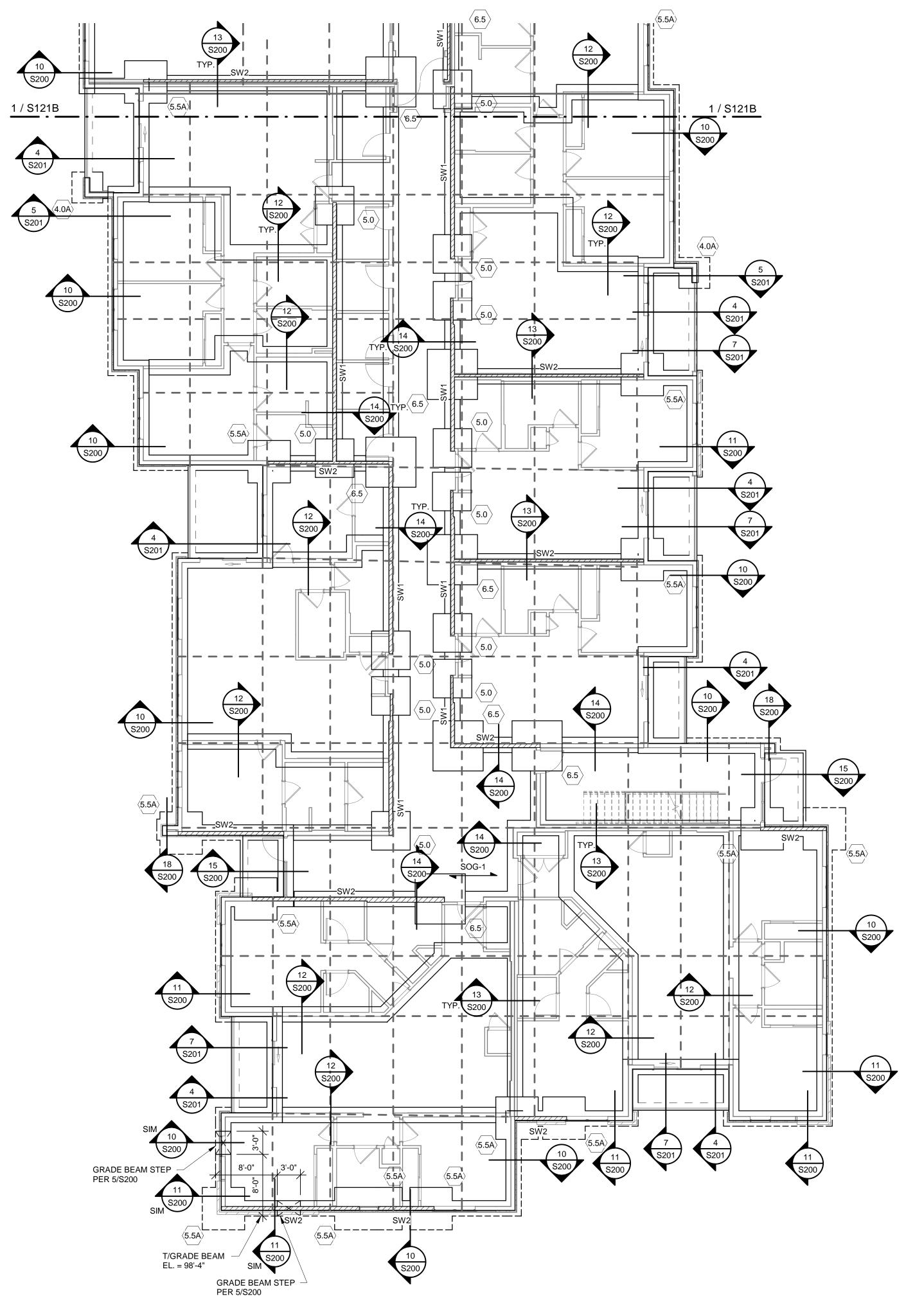
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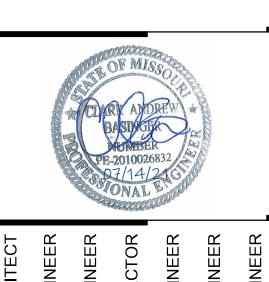
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1 BLDG 2- FOUNDATION PLAN - AREA A



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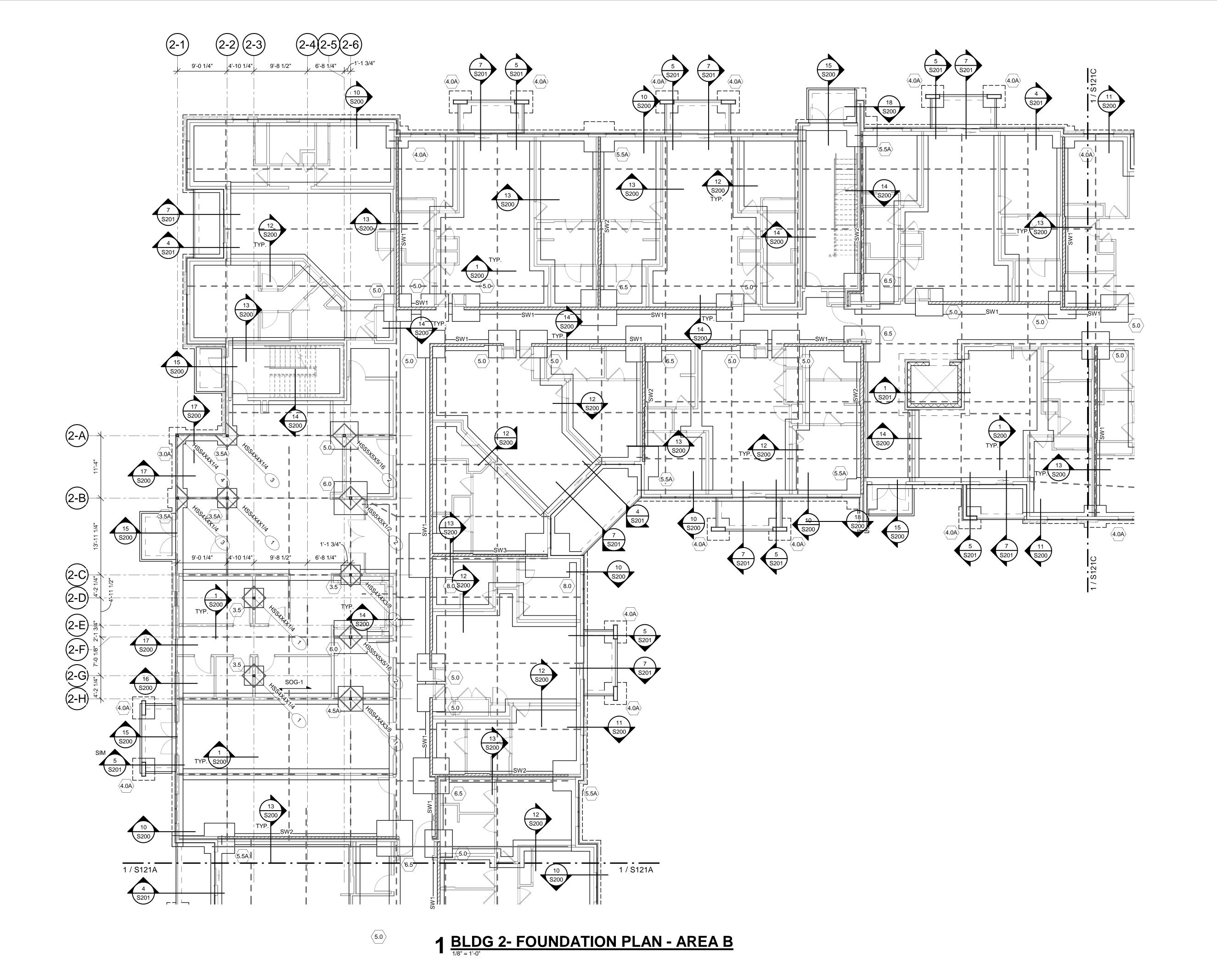
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7.15.2021

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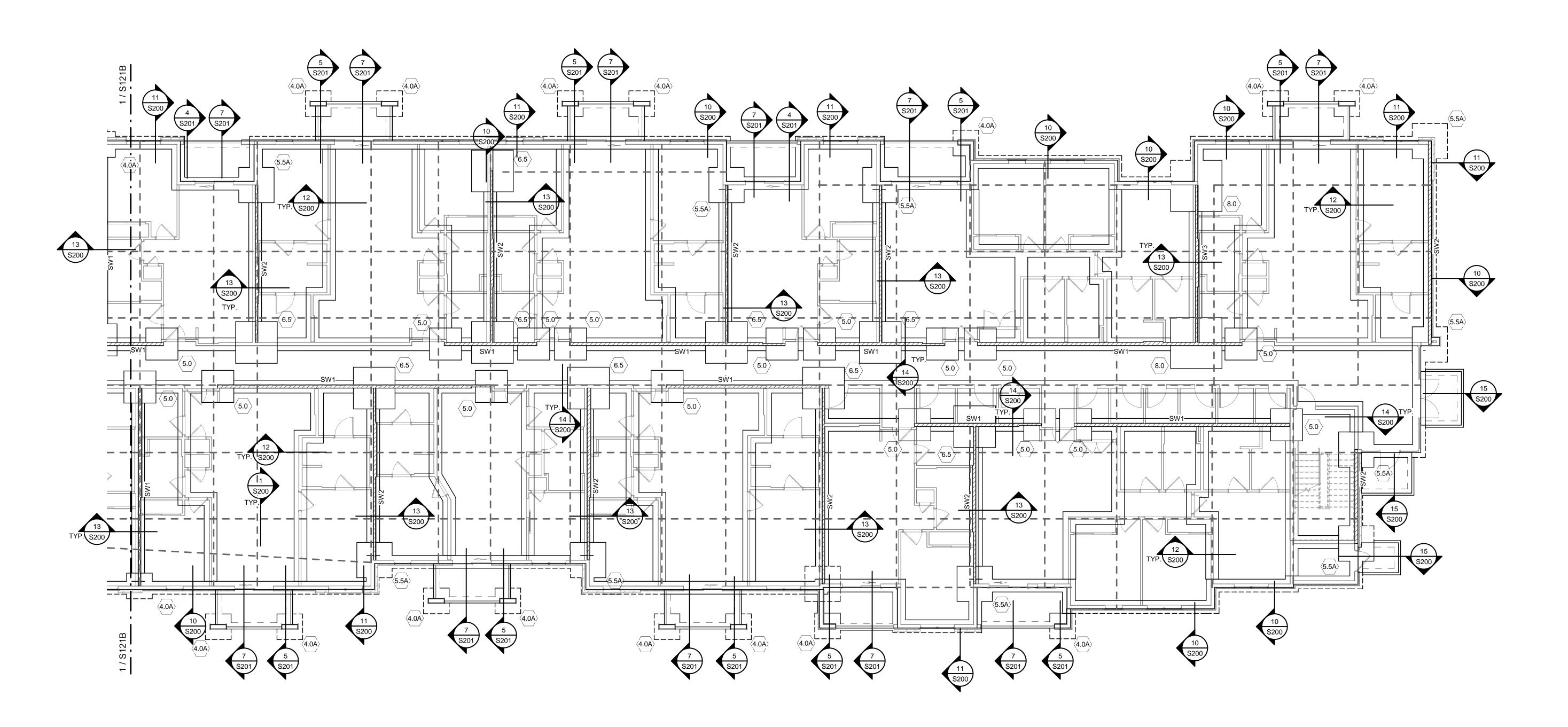
BLDG 2- FOUNDATION PLAN - AREA A



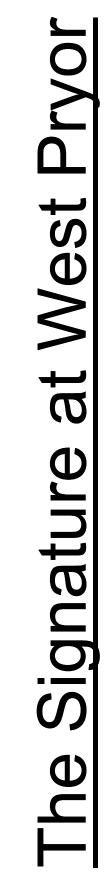
he Signature at West Pryor

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1 BLDG 2- FOUNDATION PLAN - AREA C



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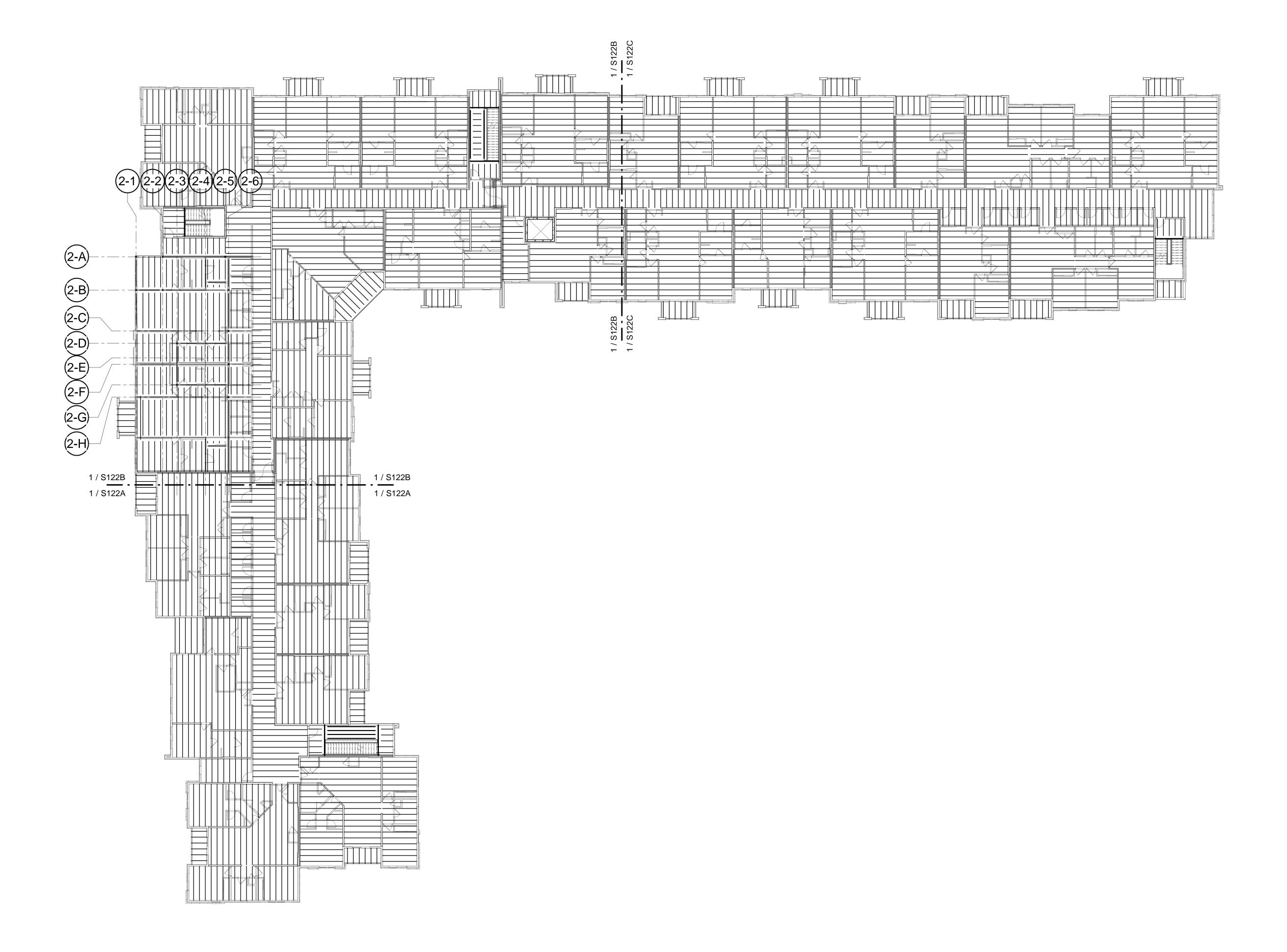
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BLDG 2- FOUNDATION PLAN - AREA C

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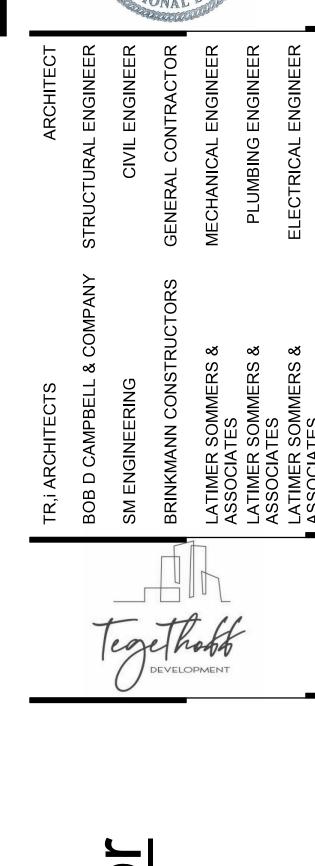
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2100 AND 2150 NW LOWENSTEIN DR. LEE'S SUMMIT, MISSOURI 64081



1 BLDG 2- SECOND FLOOR FRAMING PLAN

1/16" = 1'-0"



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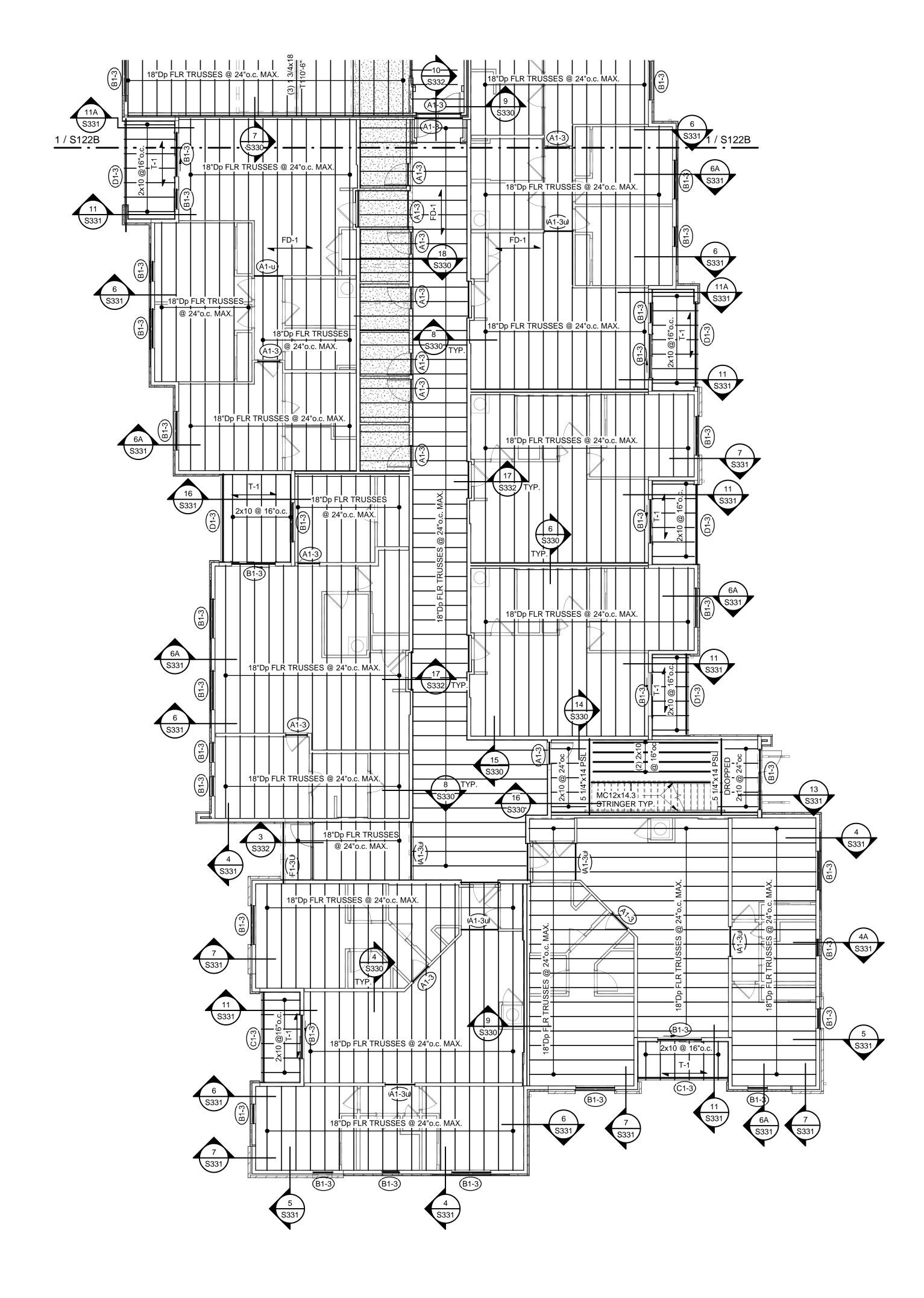
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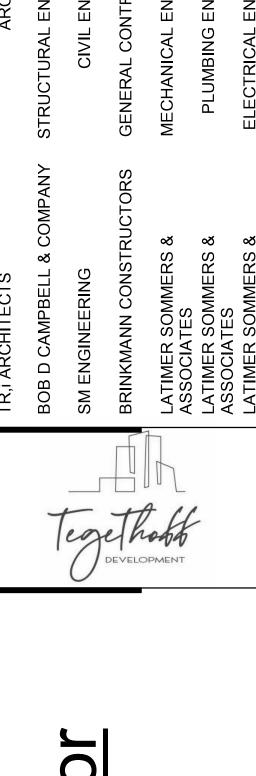
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1 BLDG 2- SECOND FLOOR FRAMING - AREA A 1/8" = 1'-0"

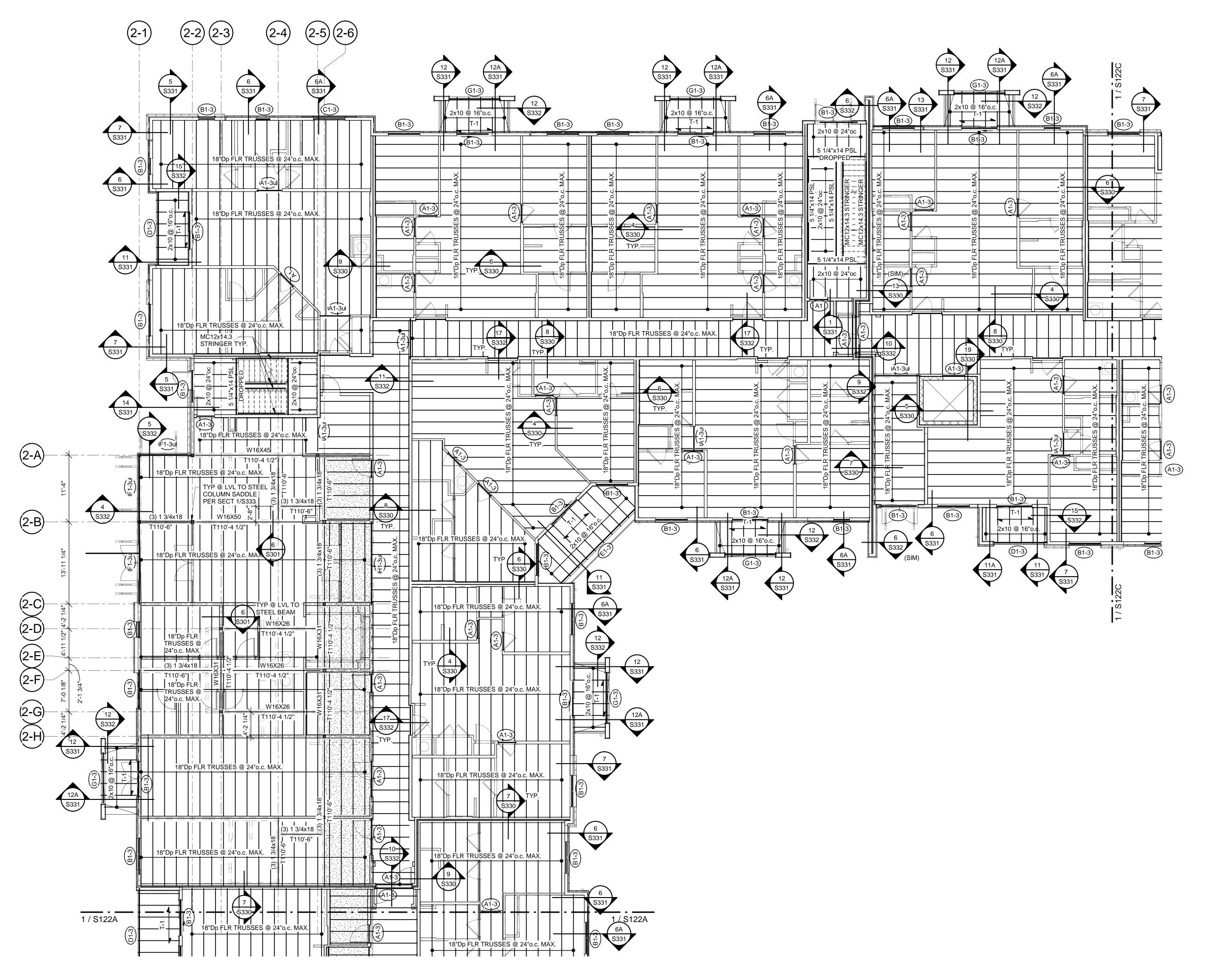
WOOD FLOOR FRAMING

- REFER TO GENERAL NOTES ON SHEET S001.
 REFER TO STUD BEARING WALL SCHEDULE ON SHEET S003.
- 3. REFER TO HEADER/BEAM/JAMB SCHEDULE ON SHEET S003. 4. REFER TO SHEARWALL AND HOLDOWN SCHEDULE ON S004.
- 5. REFER TO \$330 SERIES OF SHEETS FOR ADDITIONAL FLOOR FRAMING DETAILS NOT INDICATED HERE
- 6. PROVIDE TRUSS SPACE DIRECTLY ABOVE AND CENTERED OVER HVAC CLOSET; REFER TO ARCH AND MEP DRAWINGS FOR LOCATIONS.
- 7. REFER TO S001 FOR SLAB AND DECK SCHEDULE. A. WOOD FLOOR DECK TO BE DECK TYPE "FD-1" U.N.O.
- B. BALCONY FLOOR DECK TO BE DECK TYPE "T-1" U.N.O. 8. PUBLIC AREA: DESIGN FOR LIVE LOAD PER GENERAL NOTE 2.B ON S001
 STORAGE AREA: DESIGN FOR LIVE LOAD PER GENERAL NOTE 2.B ON S001



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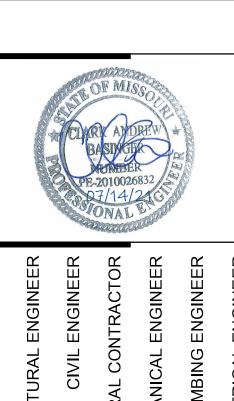
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1 BLDG 2- SECOND FLOOR FRAMING PLAN - AREA B

WOOD FLOOR FRAMING

- 1. REFER TO GENERAL NOTES ON SHEET S001. REFER TO STUD BEARING WALL SCHEDULE ON SHEET S003.
- REFER TO HEADER/BEAM/JAMB SCHEDULE ON SHEET S003. REFER TO SHEARWALL AND HOLDOWN SCHEDULE ON S004. REFER TO S330 SERIES OF SHEETS FOR ADDITIONAL FLOOR FRAMING DETAILS
- NOT INDICATED HERE
- PROVIDE TRUSS SPACE DIRECTLY ABOVE AND CENTERED OVER HVAC CLOSET; REFER TO ARCH AND MEP DRAWINGS FOR LOCATIONS.
- 7. REFER TO S001 FOR SLAB AND DECK SCHEDULE. A. WOOD FLOOR DECK TO BE DECK TYPE "FD-1" U.N.O. B. BALCONY FLOOR DECK TO BE DECK TYPE "T-1" U.N.O.
- 8. [+++++] PUBLIC AREA: DESIGN FOR LIVE LOAD PER GENERAL NOTE 2.B ON S001 STORAGE AREA: DESIGN FOR LIVE LOAD PER GENERAL NOTE 2.8 ON S001



at

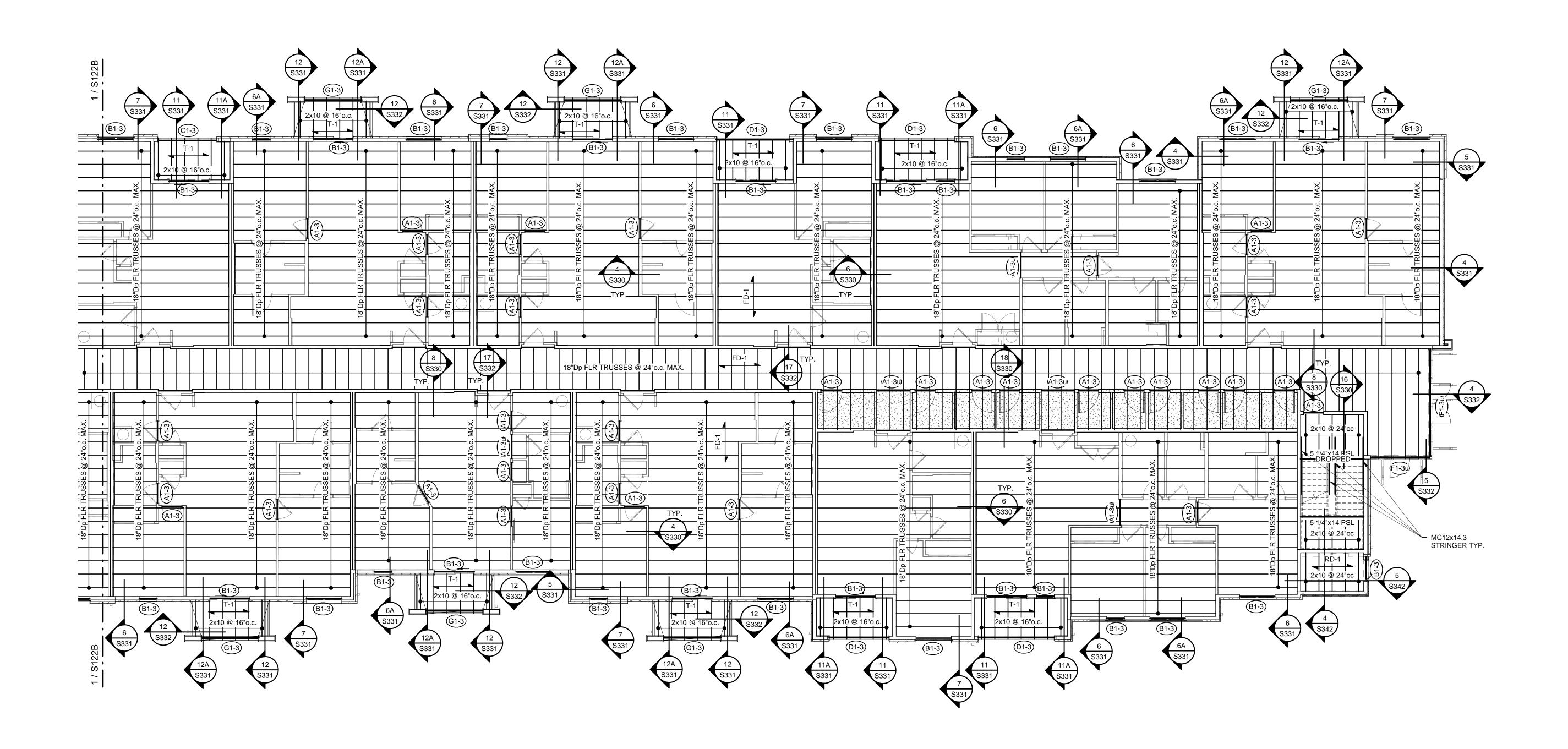
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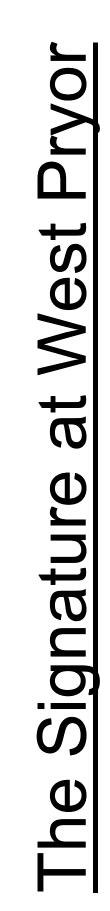
1 BLDG 2- SECOND FLOOR FRAMING PLAN - AREA C

1.) TOP OF STEEL ELEVATION=103'-3 3/4" UNLESS NOTED OTHERWISE. 2.) REFER TO A401 FOR COURTYARD CONCRETE. SHEM WALL LOCATIONS AND WIDTHS REINFORCE W/ #4 @ 12"oc EACH WAY, DRILL & EPOXY VERTICAL REINF. 3" INTO SLAB BELOW

WOOD FLOOR FRAMING

1. REFER TO GENERAL NOTES ON SHEET S001.

- REFER TO STUD BEARING WALL SCHEDULE ON SHEET S003. 3. REFER TO HEADER/BEAM/JAMB SCHEDULE ON SHEET S003.
- 4. REFER TO SHEARWALL AND HOLDOWN SCHEDULE ON S004. 5. REFER TO S330 SERIES OF SHEETS FOR ADDITIONAL FLOOR FRAMING DETAILS
- NOT INDICATED HERE 6. PROVIDE TRUSS SPACE DIRECTLY ABOVE AND CENTERED OVER HVAC CLOSET;
- REFER TO ARCH AND MEP DRAWINGS FOR LOCATIONS. 7. REFER TO S001 FOR SLAB AND DECK SCHEDULE.
- A. WOOD FLOOR DECK TO BE DECK TYPE "FD-1" U.N.O.
- B. BALCONY FLOOR DECK TO BE DECK TYPE "T-1" U.N.O. 8. PUBLIC AREA: DESIGN FOR LIVE LOAD PER GENERAL NOTE 2.B ON S001
 STORAGE AREA: DESIGN FOR LIVE LOAD PER GENERAL NOTE 2.B ON S001



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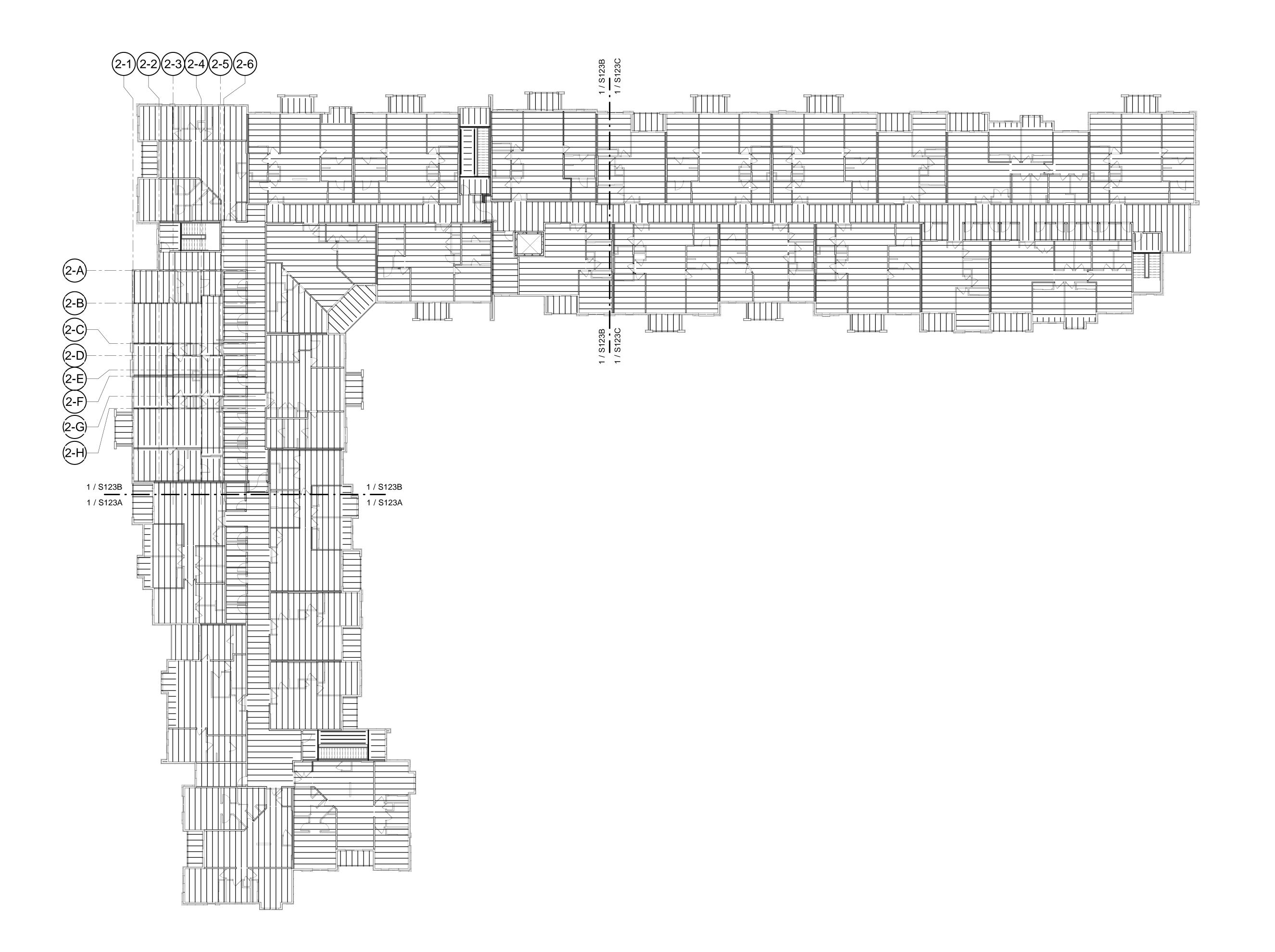
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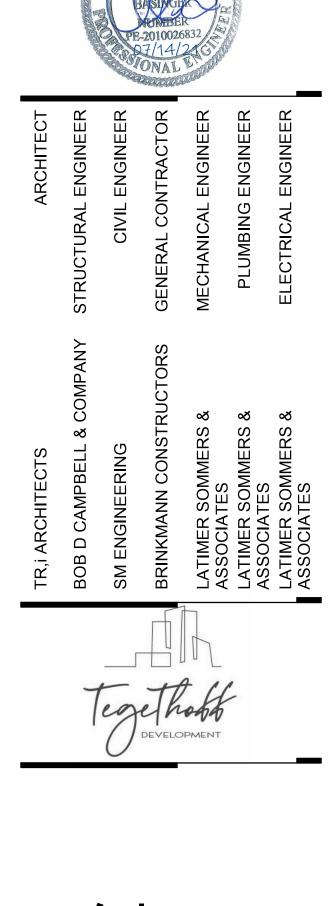
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1 BLDG 2 - THIRD FLOOR FRAMING PLAN



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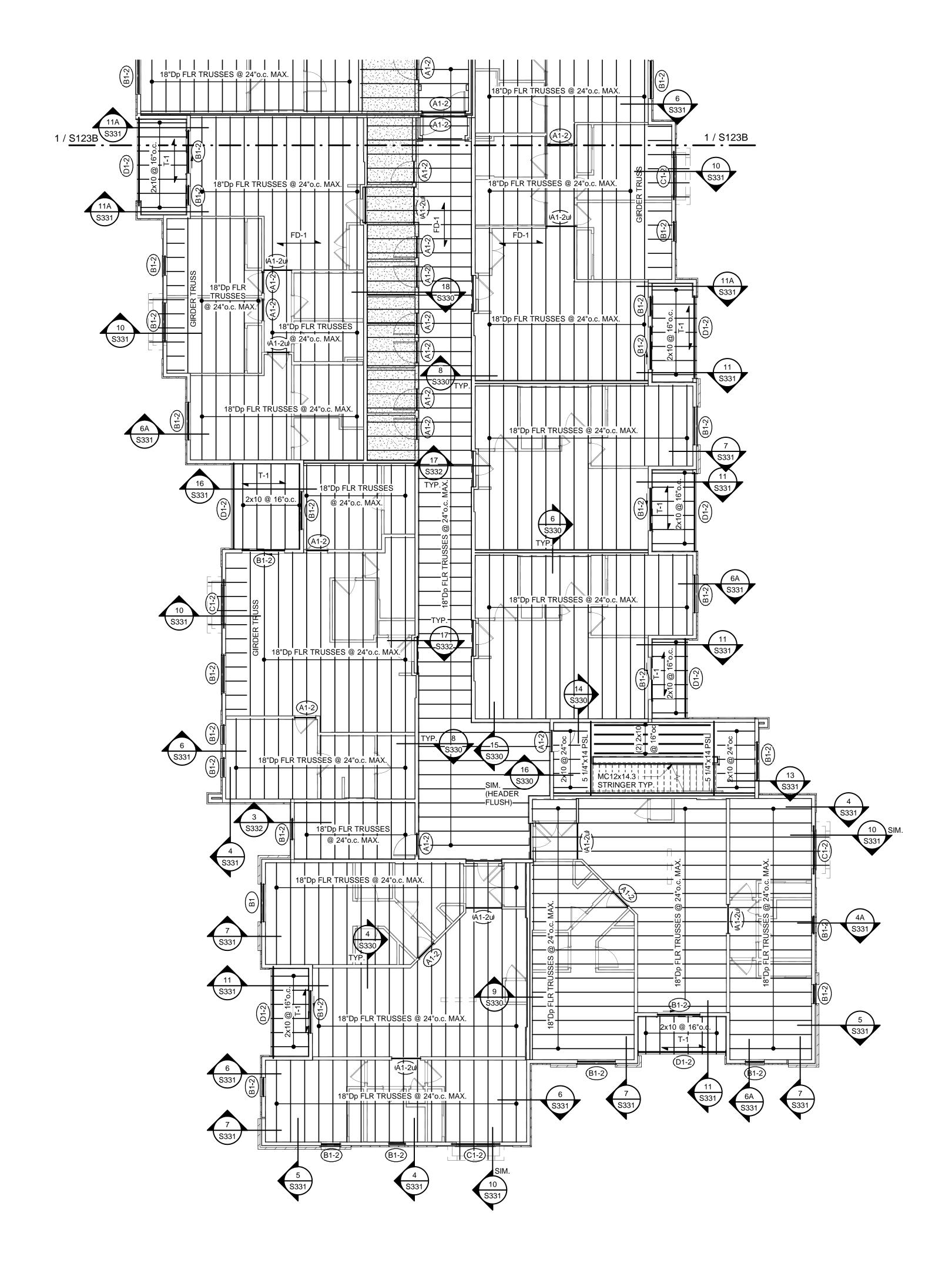
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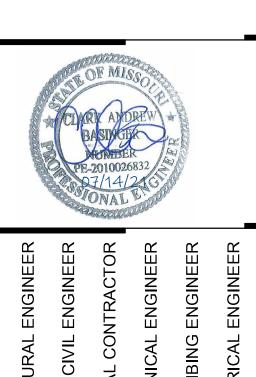
SHEET NO.



1 BLDG 2- THIRD FLOOR FRAMING PLAN - AREA A

WOOD FLOOR FRAMING

- REFER TO GENERAL NOTES ON SHEET S001.
 REFER TO STUD BEARING WALL SCHEDULE ON SHEET S003.
- 3. REFER TO HEADER/BEAM/JAMB SCHEDULE ON SHEET S003. 4. REFER TO SHEARWALL AND HOLDOWN SCHEDULE ON S004.
- 5. REFER TO S330 SERIES OF SHEETS FOR ADDITIONAL FLOOR FRAMING DETAILS NOT INDICATED HERE
- 6. PROVIDE TRUSS SPACE DIRECTLY ABOVE AND CENTERED OVER HVAC CLOSET; REFER TO ARCH AND MEP DRAWINGS FOR LOCATIONS.
- 7. REFER TO S001 FOR SLAB AND DECK SCHEDULE. A. WOOD FLOOR DECK TO BE DECK TYPE "FD-1" U.N.O.
- B. BALCONY FLOOR DECK TO BE DECK TYPE "T-1" U.N.O. 8. PUBLIC AREA: DESIGN FOR LIVE LOAD PER GENERAL NOTE 2.B ON S001
 STORAGE AREA: DESIGN FOR LIVE LOAD PER GENERAL NOTE 2.B ON S001



Signature

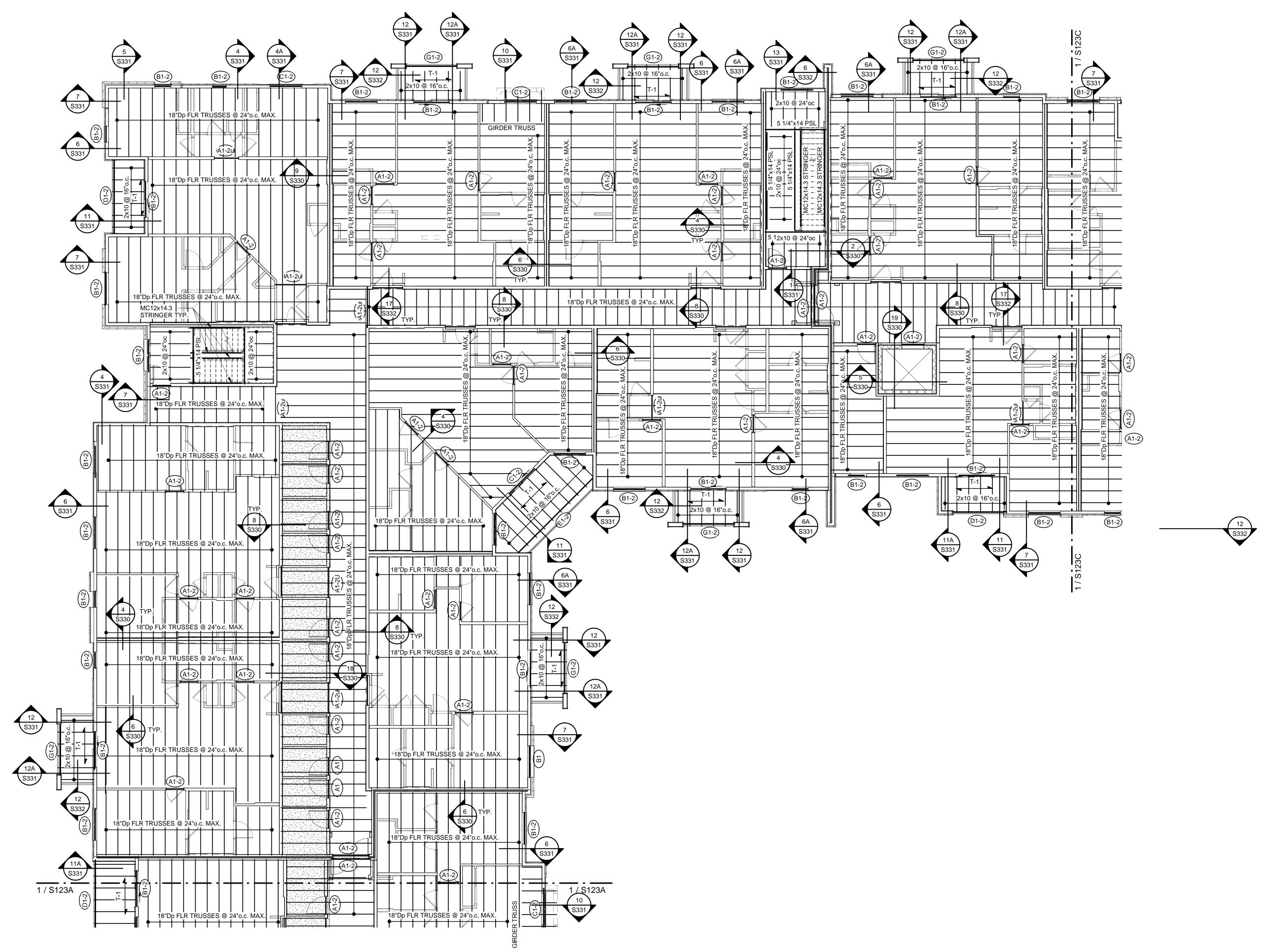
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1 BLDG 2- THIRD FLOOR FRAMING PLAN - AREA B

WOOD FLOOR FRAMING

- NOTES:

 1. REFER TO GENERAL NOTES ON SHEET S001.

 2. REFER TO STUD BEARING WALL SCHEDULE ON SHEET S003.
- REFER TO STOD BEAKING WALL SCHEDULE ON SHEET 5003.
 REFER TO HEADER/BEAM/JAMB SCHEDULE ON SHEET S003.
 REFER TO SHEARWALL AND HOLDOWN SCHEDULE ON S004.
- 5. REFER TO \$330 SERIES OF SHEETS FOR ADDITIONAL FLOOR FRAMING DETAILS NOT INDICATED HERE
- 6. PROVIDE TRUSS SPACE DIRECTLY ABOVE AND CENTERED OVER HVAC CLOSET; REFER TO ARCH AND MEP DRAWINGS FOR LOCATIONS.
- 7. REFER TO S001 FOR SLAB AND DECK SCHEDULE.
 A. WOOD FLOOR DECK TO BE DECK TYPE "FD-1" U.N.O.
- B. BALCONY FLOOR DECK TO BE DECK TYPE "T-1" U.N.O.

 8. PUBLIC AREA: DESIGN FOR LIVE LOAD PER GENERAL NOTE 2.B ON S001

 STORAGE AREA: DESIGN FOR LIVE LOAD PER GENERAL NOTE 2.B ON S001



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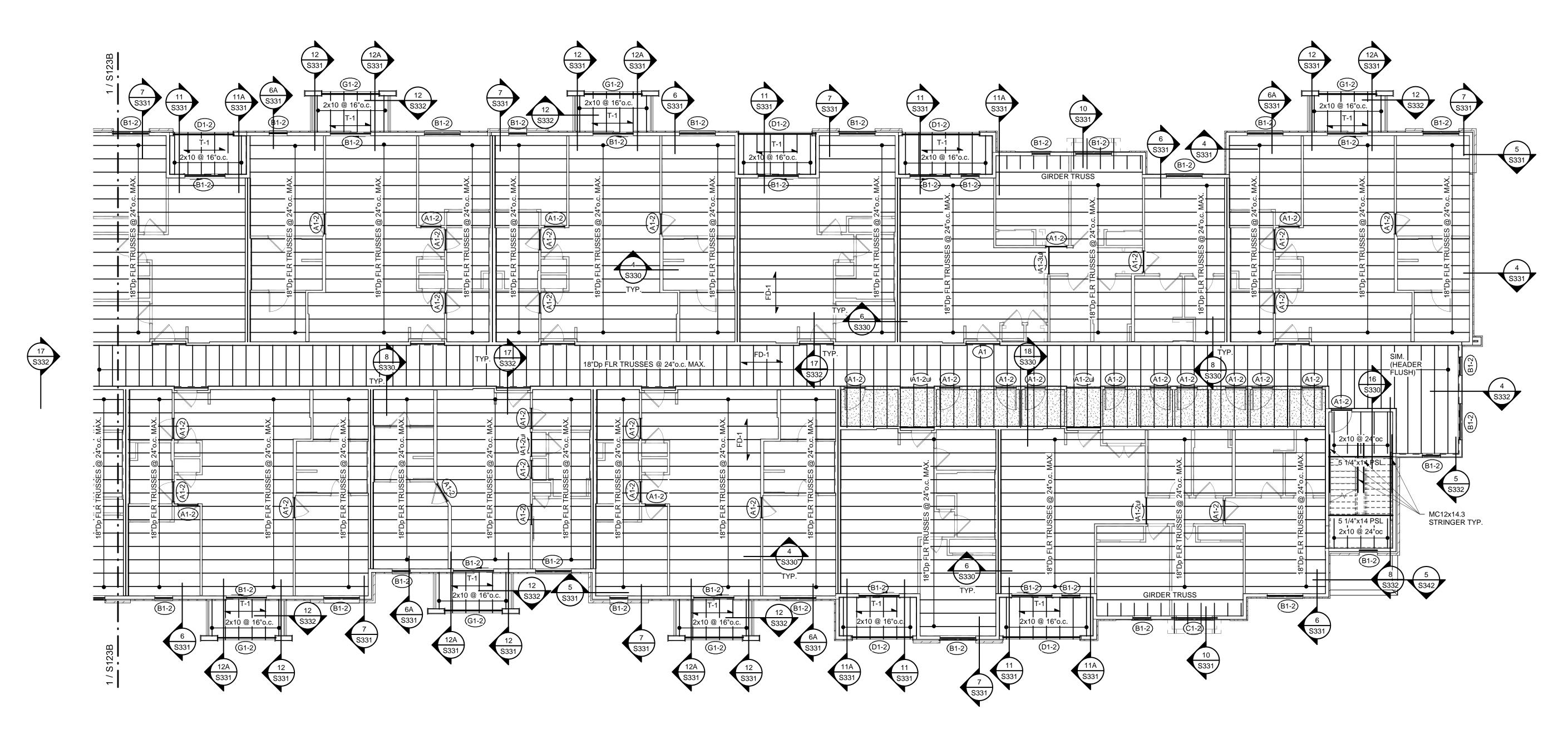
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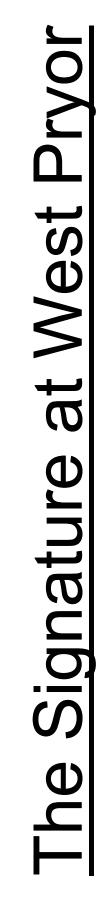
BLDG 2- THIRD FLOOR FRAMIN



1 BLDG 2- THIRD FLOOR FRAMING PLAN - AREA C

WOOD FLOOR FRAMING

- 1. REFER TO GENERAL NOTES ON SHEET S001.
- 2. REFER TO STUD BEARING WALL SCHEDULE ON SHEET S003. 3. REFER TO HEADER/BEAM/JAMB SCHEDULE ON SHEET S003.
- 4. REFER TO SHEARWALL AND HOLDOWN SCHEDULE ON S004. 5. REFER TO \$330 SERIES OF SHEETS FOR ADDITIONAL FLOOR FRAMING DETAILS
- NOT INDICATED HERE 6. PROVIDE TRUSS SPACE DIRECTLY ABOVE AND CENTERED OVER HVAC CLOSET;
- REFER TO ARCH AND MEP DRAWINGS FOR LOCATIONS.
- 7. REFER TO S001 FOR SLAB AND DECK SCHEDULE. A. WOOD FLOOR DECK TO BE DECK TYPE "FD-1" U.N.O.
- B. BALCONY FLOOR DECK TO BE DECK TYPE "T-1" U.N.O.
- 8. PUBLIC AREA: DESIGN FOR LIVE LOAD PER GENERAL NOTE 2.B ON S001
 STORAGE AREA: DESIGN FOR LIVE LOAD PER GENERAL NOTE 2.B ON S001



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SHEET NO.

DATE:

1 BLDG 2 - ROOF FRAMING PLAN

1/16" = 1'-0"

WOOD FLOOR FRAMING

- NOTES:

 1. REFER TO GENERAL NOTES ON SHEET S001.

 2. REFER TO STUD BEARING WALL SCHEDULE ON SHEET S003.

 3. REFER TO HEADER/BEAM/JAMB SCHEDULE ON SHEET S003.

 4. REFER TO SHEARWALL AND HOLDOWN SCHEDULE ON S004.

 5. REFER TO S330 SERIES OF SHEETS FOR ADDITIONAL FLOOR FRAMING DETAILS NOT INDICATED HERE

 6. PROVIDE TRUSS SPACE DIRECTLY ABOVE AND CENTERED OVER HVAC CLOSET; REFER TO ARCH AND MEP DRAWINGS FOR LOCATIONS.

 7. REFER TO S001 FOR SLAB AND DECK SCHEDULE.

 A. WOOD FLOOR DECK TO BE DECK TYPE "FD-1" U.N.O.

 B. BALCONY FLOOR DECK TO BE DECK TYPE "T-1" U.N.O.

 8. STORAGE AREA: DESIGN FOR LIVE LOAD PER GENERAL NOTE 2.B ON S001

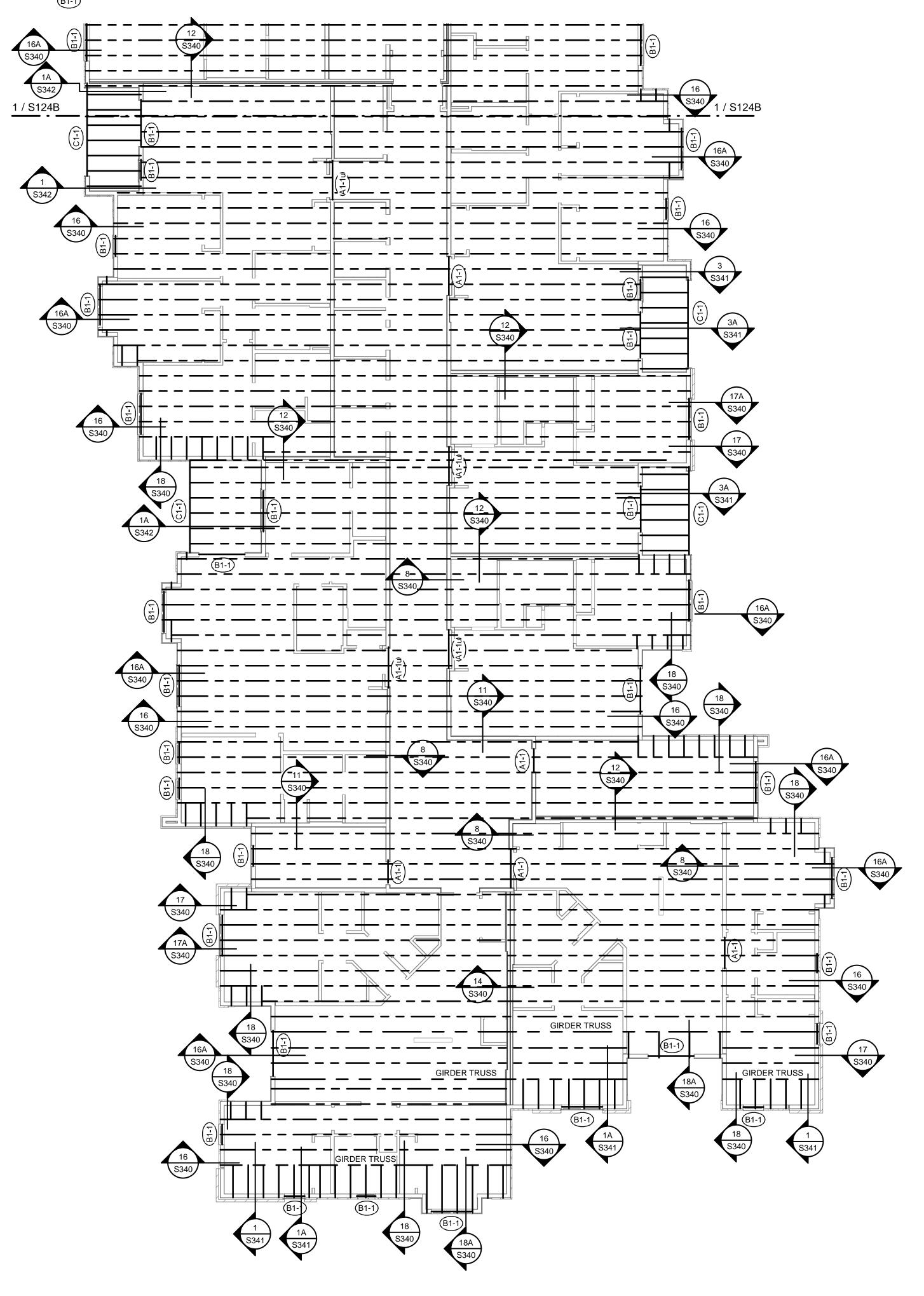
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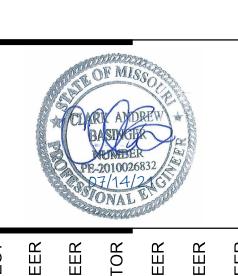
BLDG 2 - ROOF FRAMING PLAN



1 BLDG 2- ROOF FRAMING PLAN - AREA A

WOOD FLOOR FRAMING

- REFER TO GENERAL NOTES ON SHEET S001.
 REFER TO STUD BEARING WALL SCHEDULE ON SHEET S003.
 REFER TO HEADER/BEAM/JAMB SCHEDULE ON SHEET S003.
- 4. REFER TO SHEARWALL AND HOLDOWN SCHEDULE ON S004.
- REFER TO SHEARWALL AND HOLDOWN SCHEDULE ON 5004.
 REFER TO S330 SERIES OF SHEETS FOR ADDITIONAL FLOOR FRAMING DETAILS NOT INDICATED HERE
 PROVIDE TRUSS SPACE DIRECTLY ABOVE AND CENTERED OVER HVAC CLOSET; REFER TO ARCH AND MEP DRAWINGS FOR LOCATIONS.
- 7. REFER TO S001 FOR SLAB AND DECK SCHEDULE.
- A. WOOD FLOOR DECK TO BE DECK TYPE "FD-1" U.N.O.
 B. BALCONY FLOOR DECK TO BE DECK TYPE "T-1" U.N.O. 8. PUBLIC AREA: DESIGN FOR LIVE LOAD PER GENERAL NOTE 2.B ON S001
 STORAGE AREA: DESIGN FOR LIVE LOAD PER GENERAL NOTE 2.B ON S001





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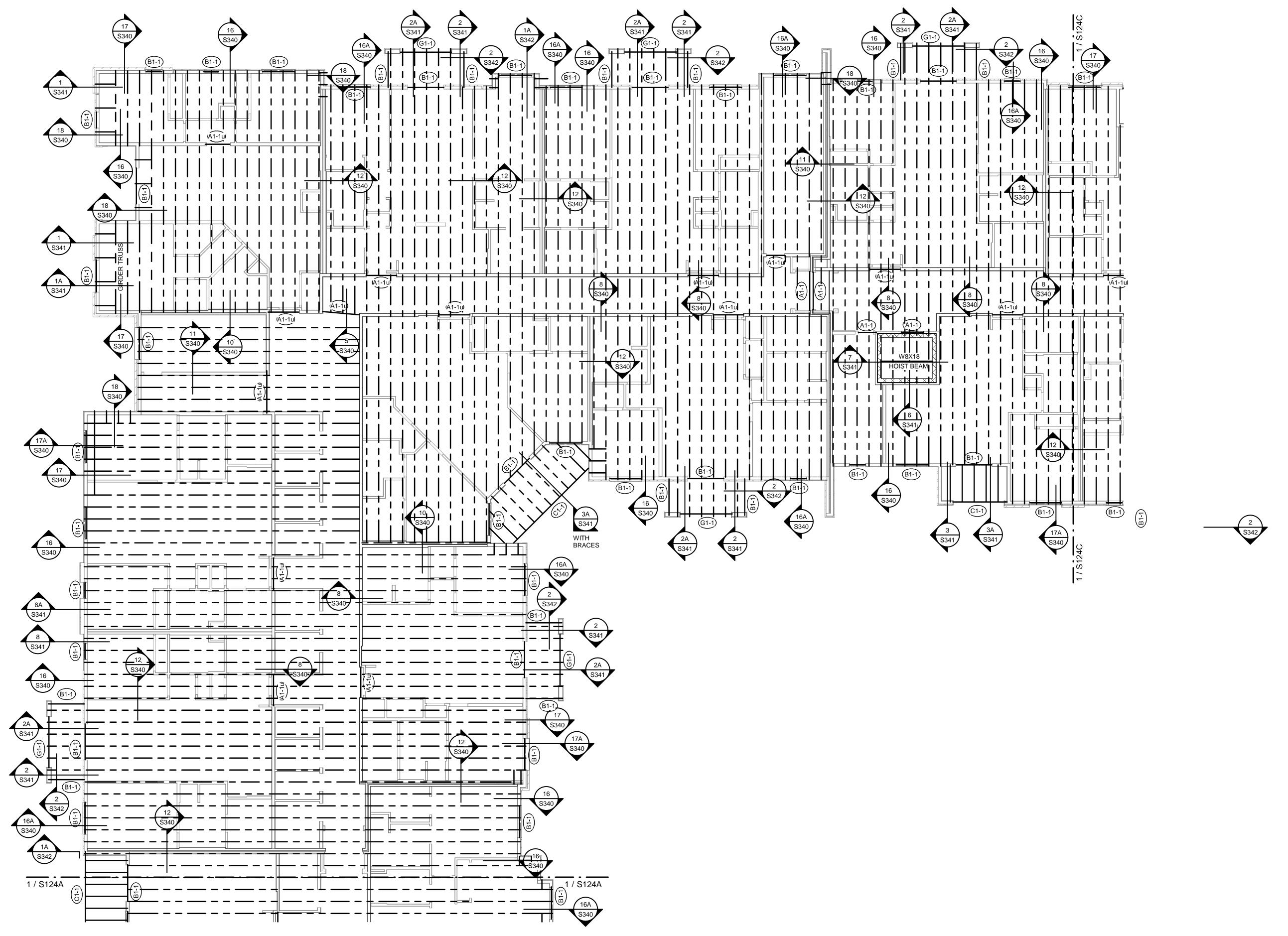
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DWG BY

SHEET NO.

TR,i PROJECT NO.

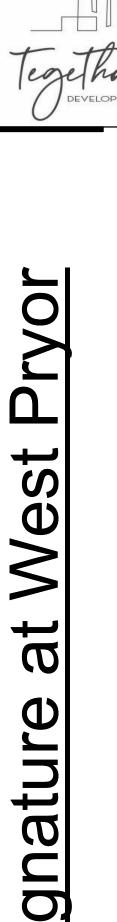
BLDG 2- ROOF FRAMING PLAN - AREA



1 BLDG 2- ROOF FRAMING PLAN - AREA B

WOOD FLOOR FRAMING

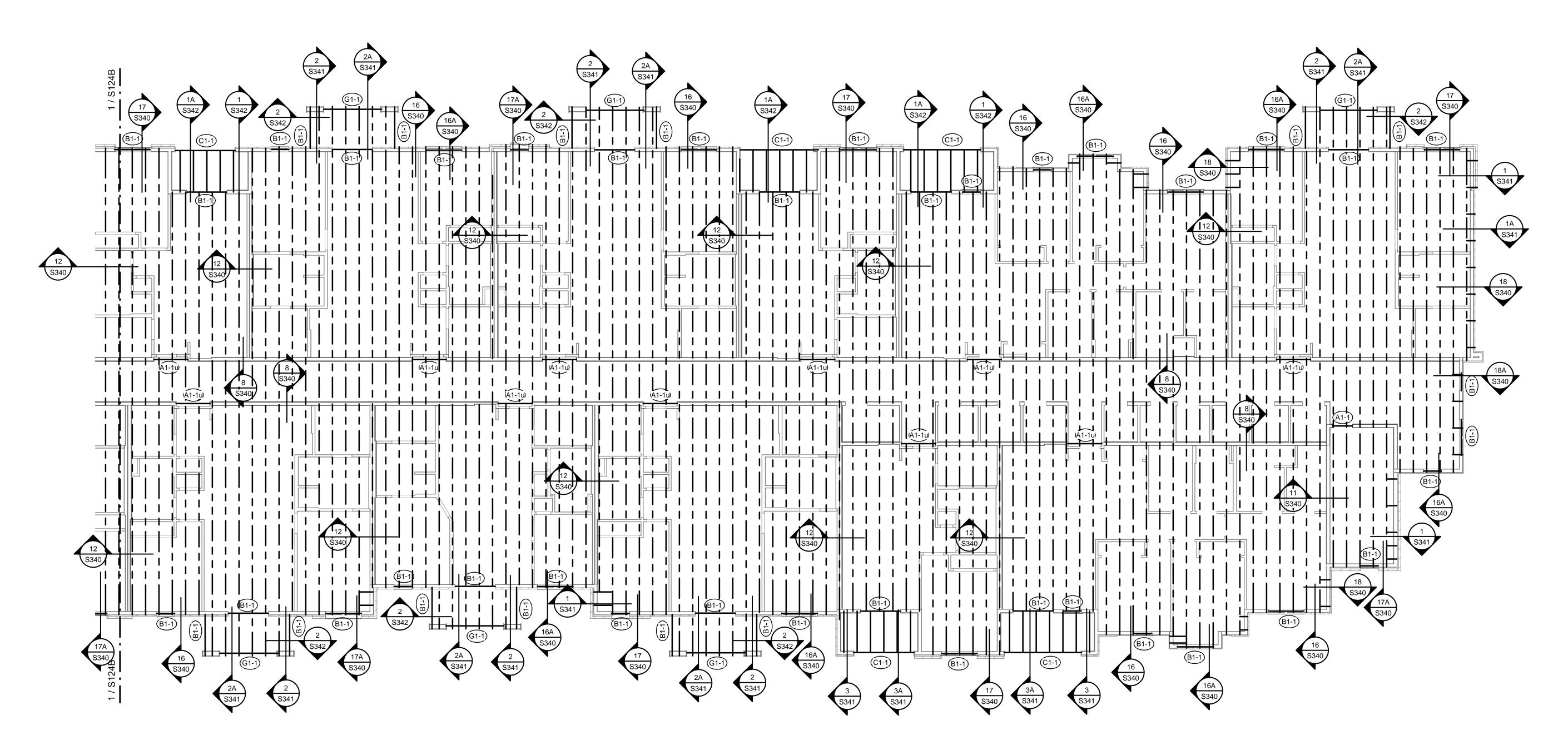
- 1. REFER TO GENERAL NOTES ON SHEET S001.
 2. REFER TO STUD BEARING WALL SCHEDULE ON SHEET S003.
 3. REFER TO HEADER/BEAM/JAMB SCHEDULE ON SHEET S003.
 4. REFER TO SHEARWALL AND HOLDOWN SCHEDULE ON S004.
 5. REFER TO S330 SERIES OF SHEETS FOR ADDITIONAL FLOOR FRAMING DETAILS
- REFER TO ARCH AND MEP DRAWINGS FOR LOCATIONS. 7. REFER TO S001 FOR SLAB AND DECK SCHEDULE. A. WOOD FLOOR DECK TO BE DECK TYPE "FD-1" U.N.O.
- B. BALCONY FLOOR DECK TO BE DECK TYPE "T-1" U.N.O. 8. PUBLIC AREA: DESIGN FOR LIVE LOAD PER GENERAL NOTE 2.B ON S001
 STORAGE AREA: DESIGN FOR LIVE LOAD PER GENERAL NOTE 2.B ON S001



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BLDG 2- ROOF FRAMING PLAN - AREA



1 BLDG 2- ROOF FRAMING PLAN - AREA C

WOOD FLOOR FRAMING

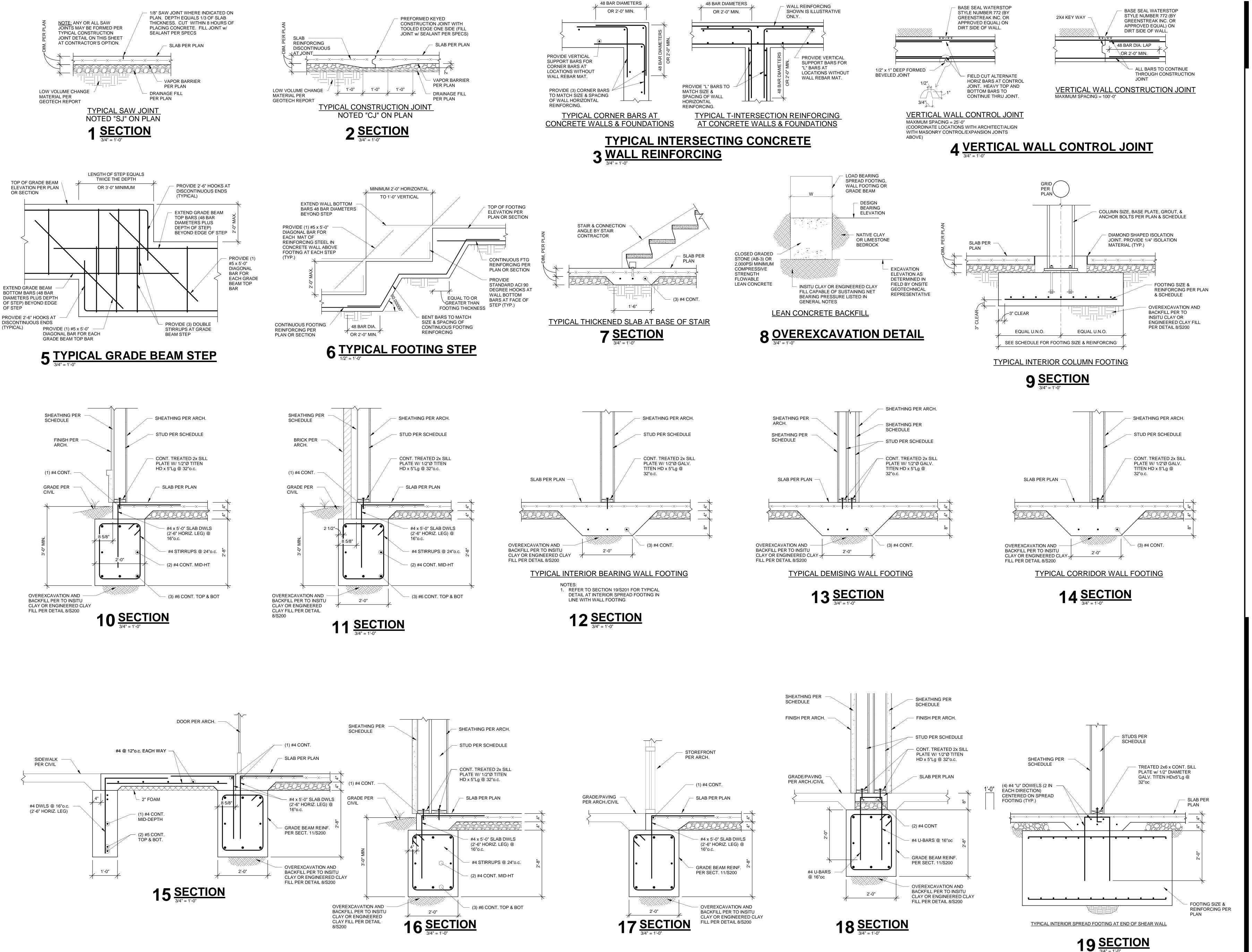
- REFER TO GENERAL NOTES ON SHEET S001.
 REFER TO STUD BEARING WALL SCHEDULE ON SHEET S003.
- 3. REFER TO HEADER/BEAM/JAMB SCHEDULE ON SHEET S003.
- REFER TO SHEARWALL AND HOLDOWN SCHEDULE ON S004.
 REFER TO S330 SERIES OF SHEETS FOR ADDITIONAL FLOOR FRAMING DETAILS NOT INDICATED HERE
- 6. PROVIDE TRUSS SPACE DIRECTLY ABOVE AND CENTERED OVER HVAC CLOSET; REFER TO ARCH AND MEP DRAWINGS FOR LOCATIONS.
- REFER TO S001 FOR SLAB AND DECK SCHEDULE.
 A. WOOD FLOOR DECK TO BE DECK TYPE "FD-1" U.N.O.
- B. BALCONY FLOOR DECK TO BE DECK TYPE "T-1" U.N.O. 8. PUBLIC AREA: DESIGN FOR LIVE LOAD PER GENERAL NOTE 2.B ON S001 STORAGE AREA: DESIGN FOR LIVE LOAD PER GENERAL NOTE 2.B ON S001

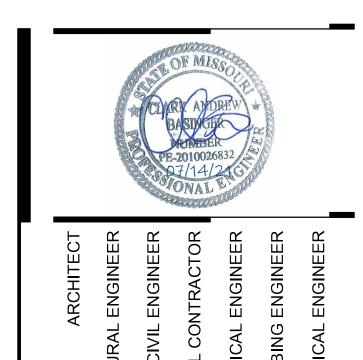


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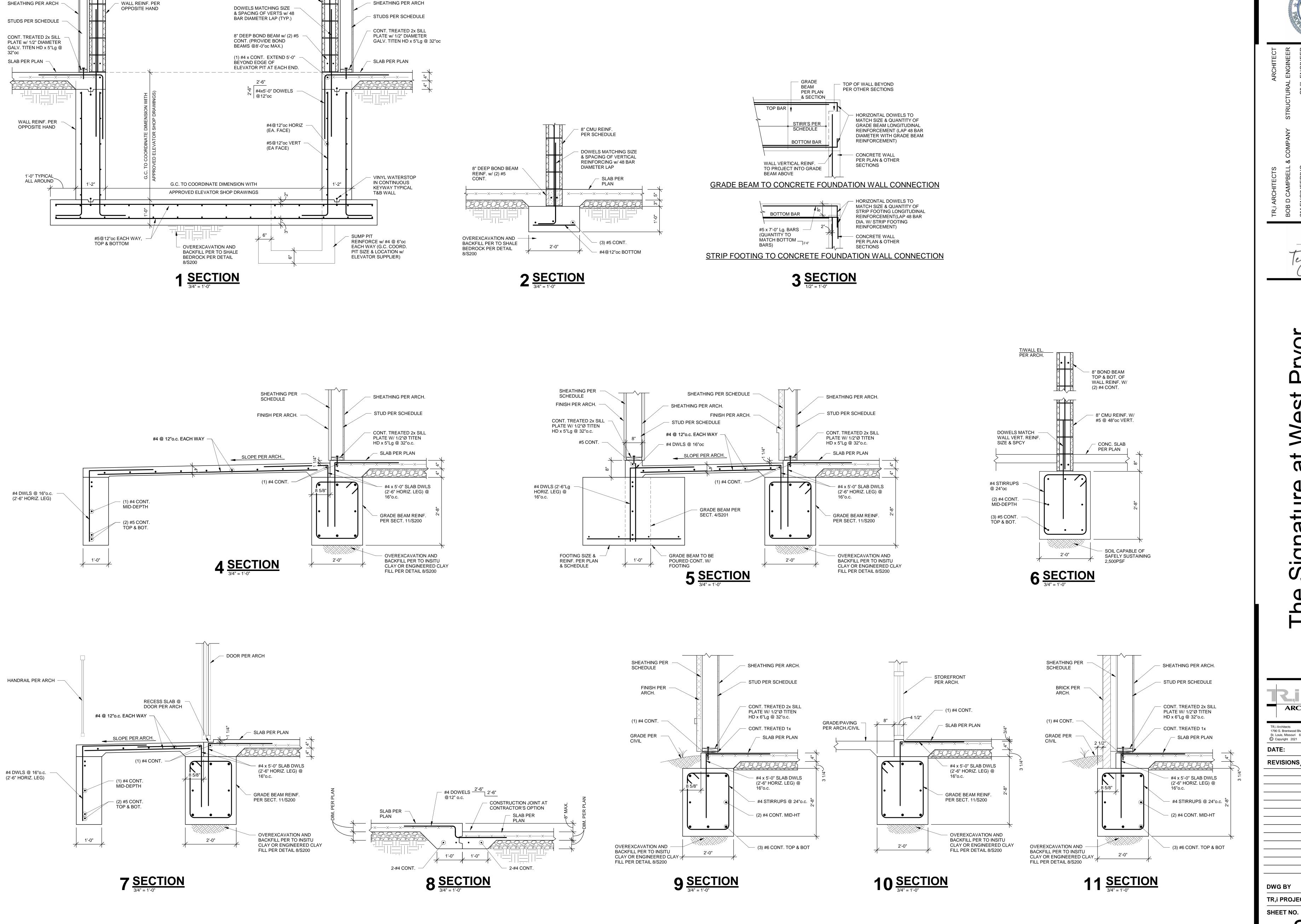
100 AND 2150 NW LOWENSTEIN LEE'S SUMMIT, MISSOURI 6408 West at Signature The

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SHEET NO.



8" CMU WALL GROUTED & REINF. PER SCHEDULE

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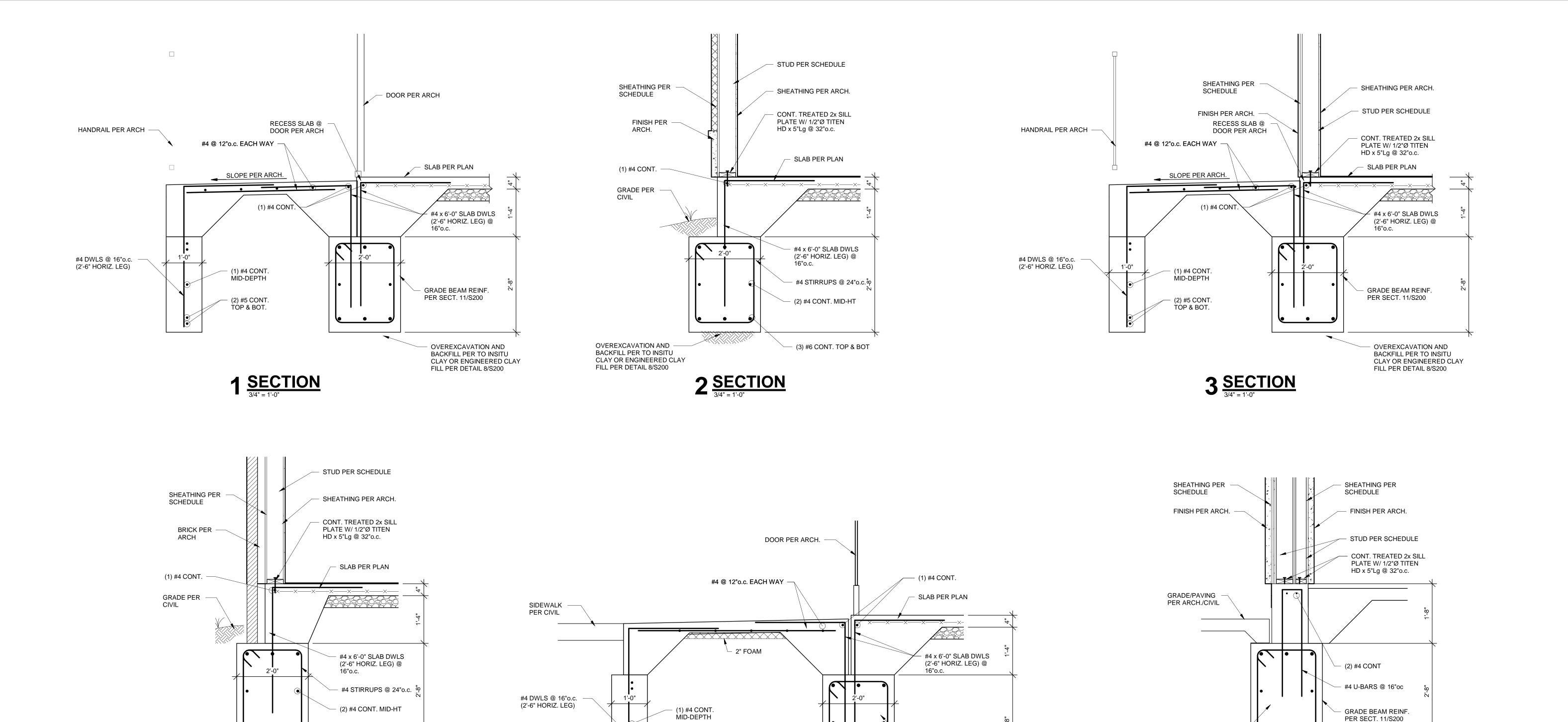
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FOUNDATION SECTIONS



(2) #5 CONT.TOP & BOT.

5 **SECTION**3/4" = 1'-0"

OVEREXCAVATION AND -

BACKFILL PER TO INSITU CLAY OR ENGINEERED CLAY FILL PER DETAIL 8/S200

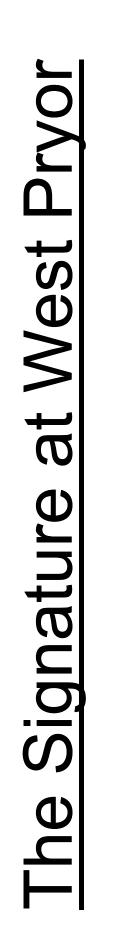
4 **SECTION**3/4" = 1'-0"

— (3) #6 CONT. TOP & BOT

GRADE BEAM REINF.

- OVEREXCAVATION AND BACKFILL PER TO INSITU CLAY OR ENGINEERED CLAY FILL PER DETAIL 8/S200

PER SECT. 11/S200



PER SECT. 11/S200

OVEREXCAVATION AND
 BACKFILL PER TO INSITU
 CLAY OR ENGINEERED CLAY
 FILL PER DETAIL 8/S200

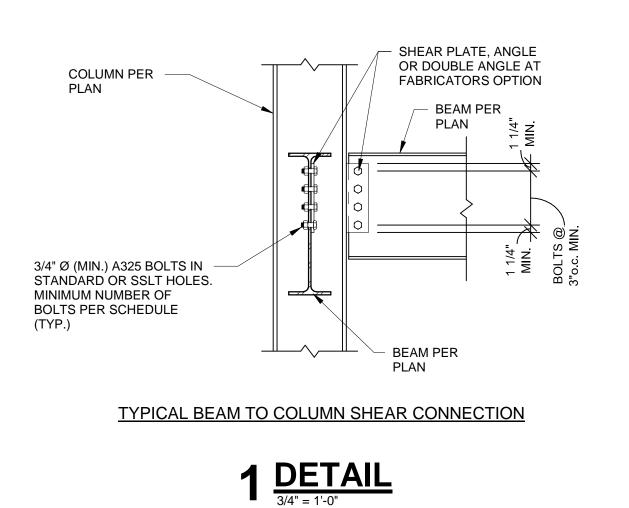
#4 U-BARS @ 16"oc

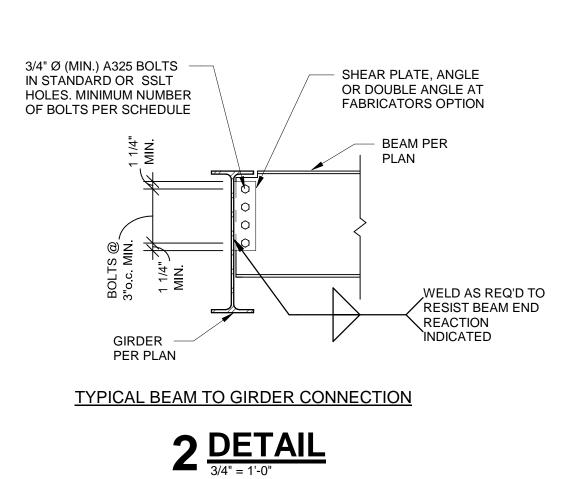
2'-0"

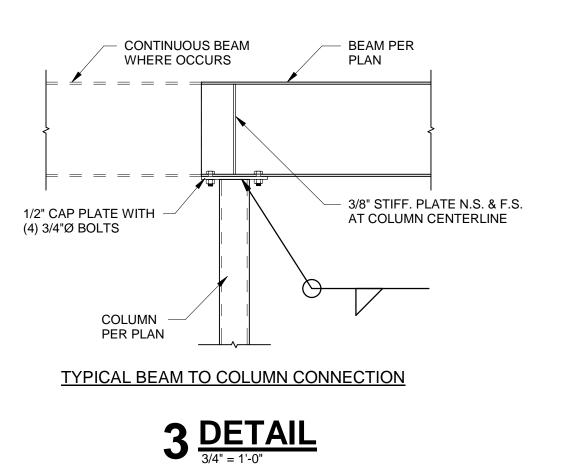
6 **SECTION**3/4" = 1'-0"

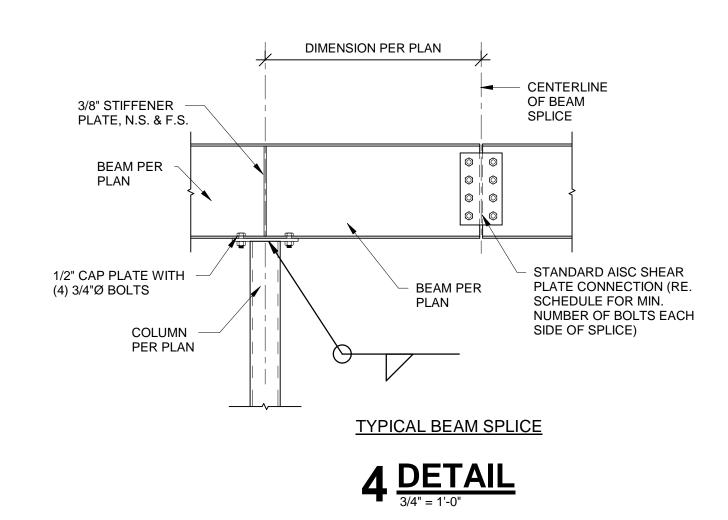
2100 AND 2150 NW LOWENSTEIN D LEE'S SUMMIT, MISSOURI 64081

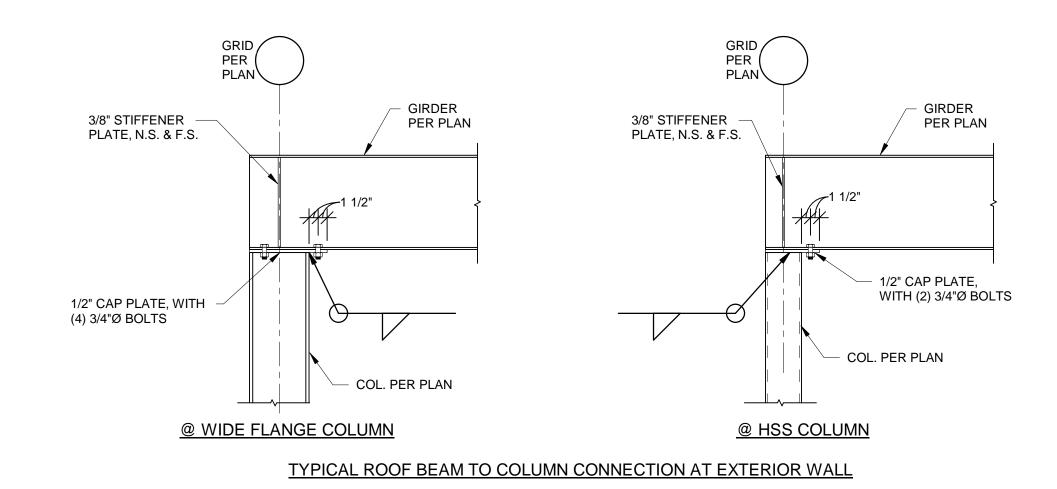
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5 **SECTION**3/4" = 1'-0"

SHEAR CONNECTION MINIMUM ROWS OF BOLTS SCHEDULE

BEAM SIZE	MINIMUM ROWS OF BOLTS
W8,C8	2
W10,C10	2
W12,C12	2
W14	3
W16, C15	3
W18	4
W21	5
W24	5
W27	6
W30	7
W33	8
W36	8

- SCHEDULE NOTES: 1. ALL BOLTS SHALL BE 3/4" Ø ASTM A325
- 2. ALL BOLTS SHALL BE SPACED AT 3"o.c. MINIMUM.
- 3. ALL BOLTS SHALL HAVE HEAVY HEX NUTS. 4. ALL BOLTS SHALL BE FULLY PRE-TENSIONED.
- 5. CLIP ANGLES MAY BE SHOP WELDED TO
- BEAM WEB PER AISC. 6. FOR BEAMS WITH AXIAL LOADS PER DRAWINGS, BOLTS AND CONNECTIONS SHALL BE SLIP-CRITICAL PER AISC GUIDELINES. INCREASE NUMBER OF BOLTS AND/OR PROVIDE EXTENDED SHEAR PLATE CONNECTION w/ AN ADDITIONAL COLUMN OF
- FORCES. 7. PROVIDE ASTM A490 BOLTS IF REQUIRED TO MEET END REACTION LOAD REQUIREMENTS

BOLTS TO ACCOMODATE COMBINED

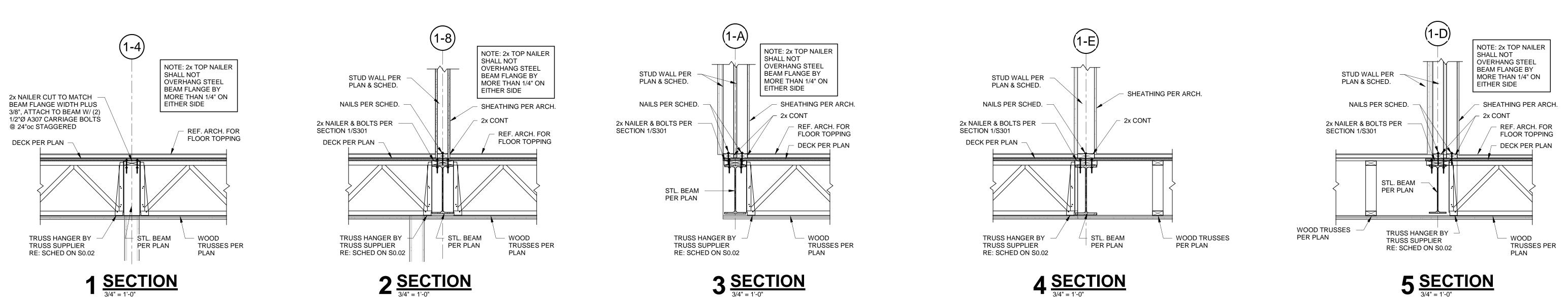
- STEEL CONNECTION NOTES: 1. CONNECTIONS SHOWN IN THESE DETAILS ARE MINIMUM REQUIREMENTS.
- 2. FABRICATOR SHALL BE RESPONSIBLE FOR THE DESIGNING AND DETAILING OF EACH CONNECTION FOR LOADS SHOWN ON THE DRAWINGS IN ACCORDANCE WITH THE SPECIFICATIONS AND THE STRUCTURAL GENERAL NOTES. SUGGESTED CONNECTION DETAILS ARE SHOWN.
- 4. FABRICATOR MAY OPT TO USE OTHER AISC APPROVED
- CONNECTIONS IN LIEU OF THESE SHOWN HEREIN. 5. CONNECTION DETAILING SHALL COMPLY WITH THE STANDARD
- 6. BOLT SPACING AND EDGE DISTANCES SHALL BE ADJUSTED PER AISC MANUAL FOR BOLTS LARGER THAN 3/4" DIAMETER.

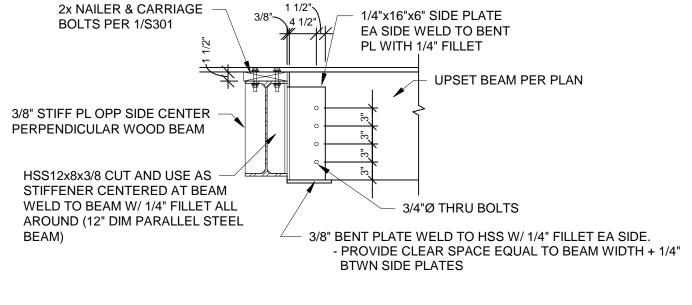
DETAILS SHOWN IN THE LATEST EDITION OF THE AISC MANUAL OF STEEL CONSTRUCTION.

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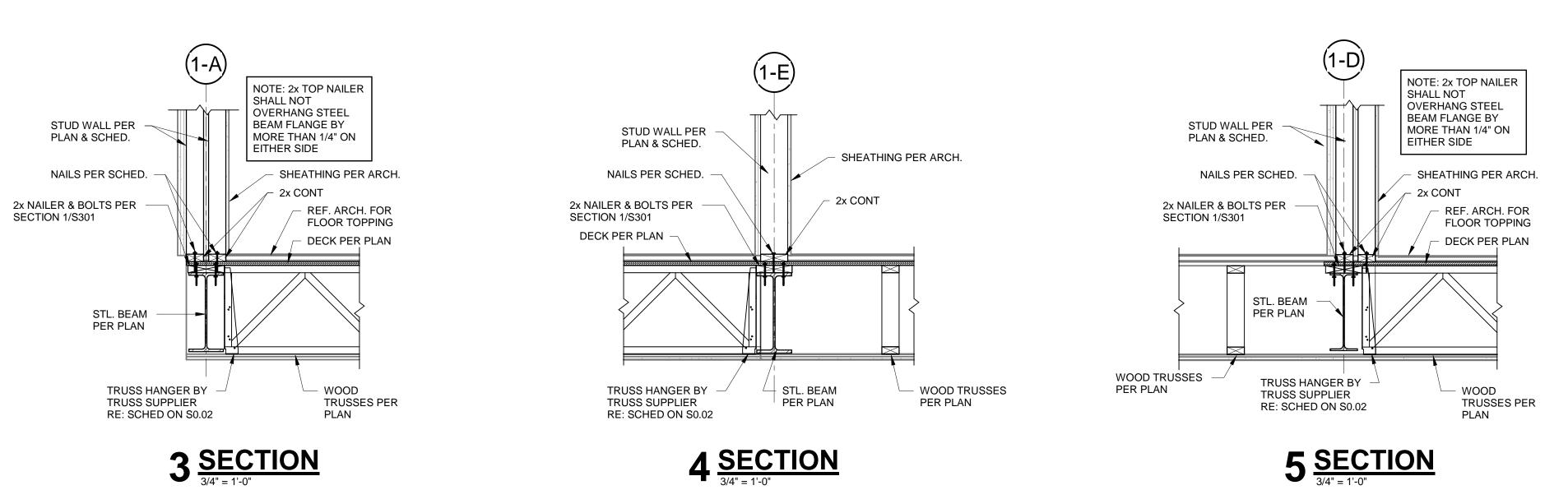
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at Signature

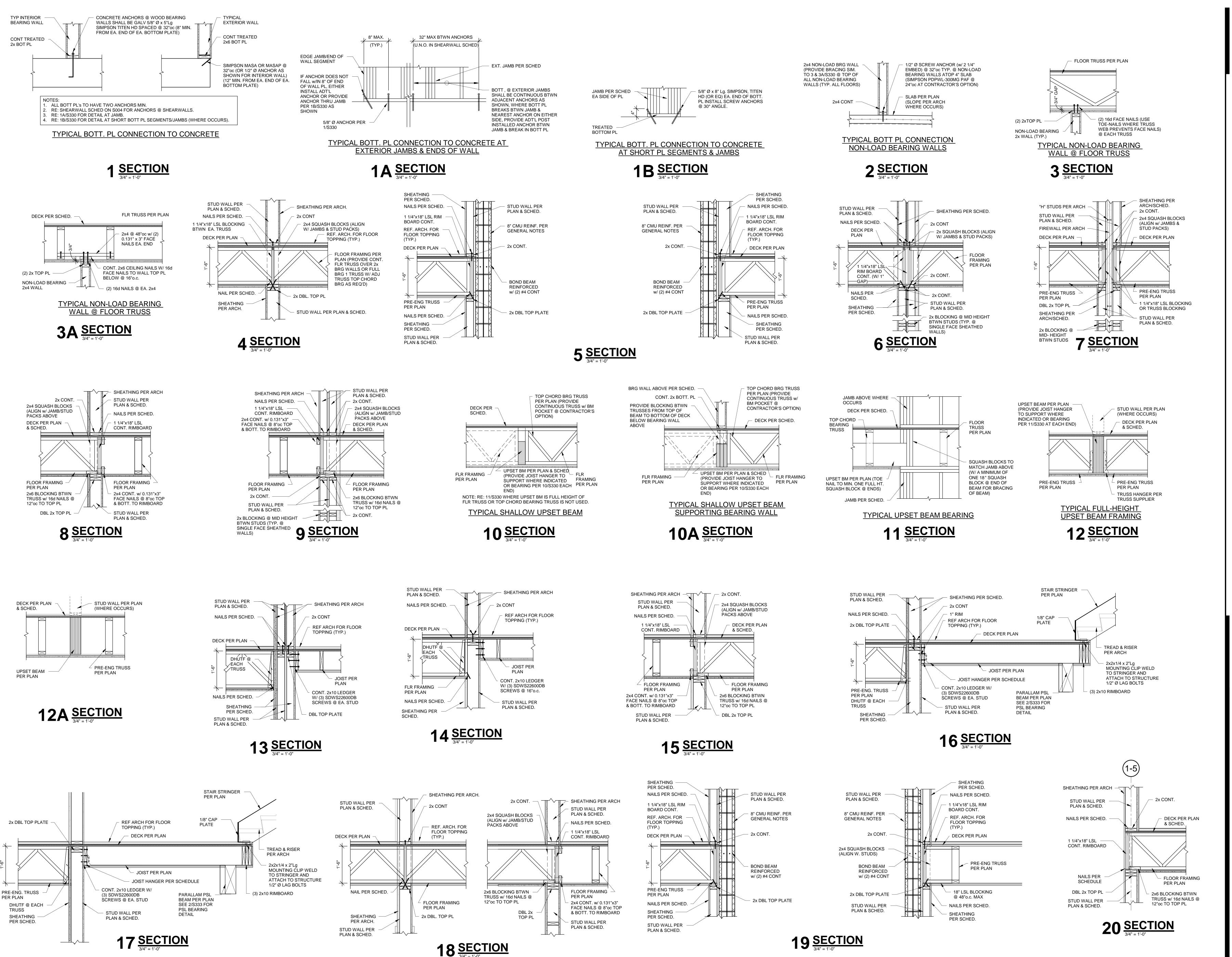


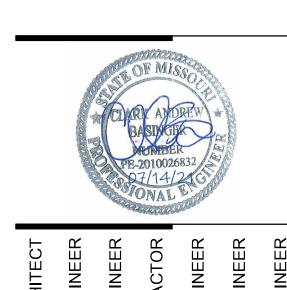






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AMPBELL & COMPANY STRUCTURAL ENGINEER

NEERING
ANN CONSTRUCTORS GENERAL CONTRACTOR
SOMMERS & MECHANICAL ENGINEER
TES
SOMMERS & PLUMBING ENGINEER
TES
SOMMERS & FLUMBING ENGINEER
TES
SOMMERS & FLUMBING ENGINEER
TES

BOB D CAMPBELL &

SM ENGINEERING

BRINKMANN CONST

LATIMER SOMMERS

ASSOCIATES

LATIMER SOMMERS

ASSOCIATES

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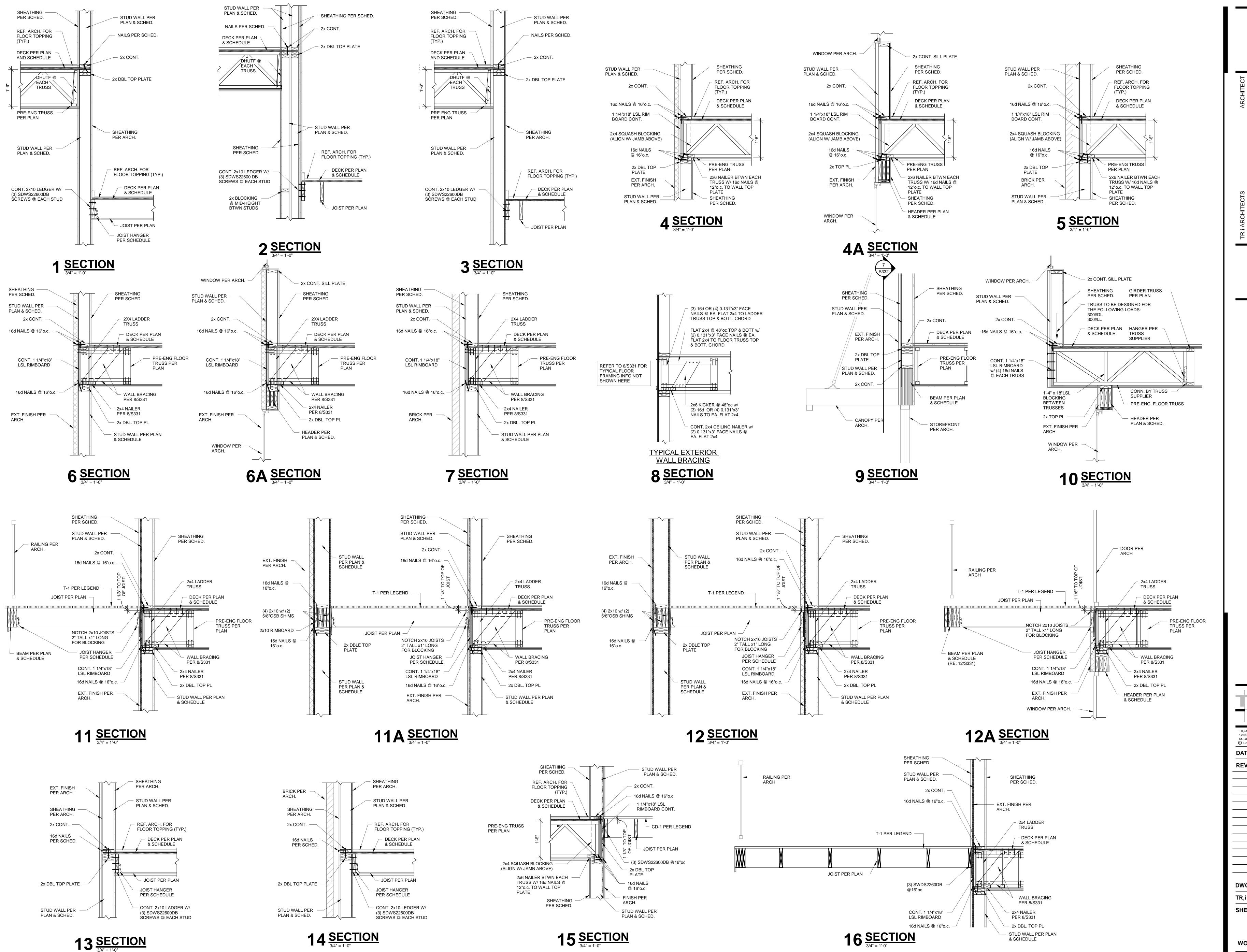
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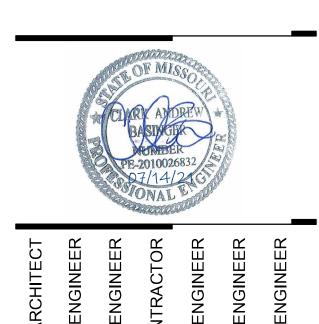
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S330

WOOD FLOOR FRAMING SECTIONS





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ASSOCIATES
LATIMER SOMMERS & PLUMBING ENGINE

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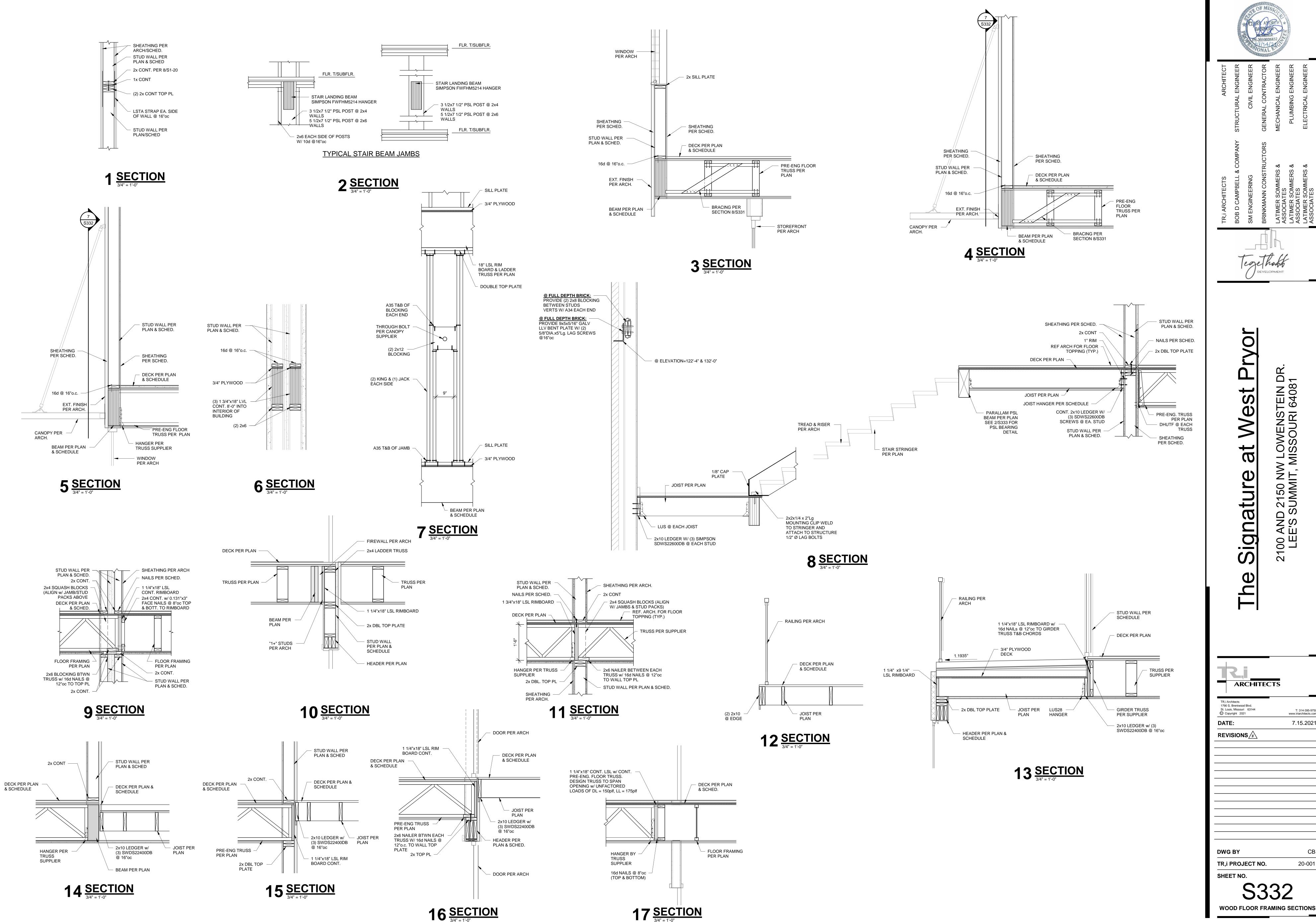
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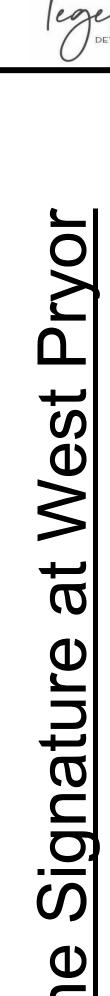
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S331
WOOD FLOOR FRAMING SECTIONS

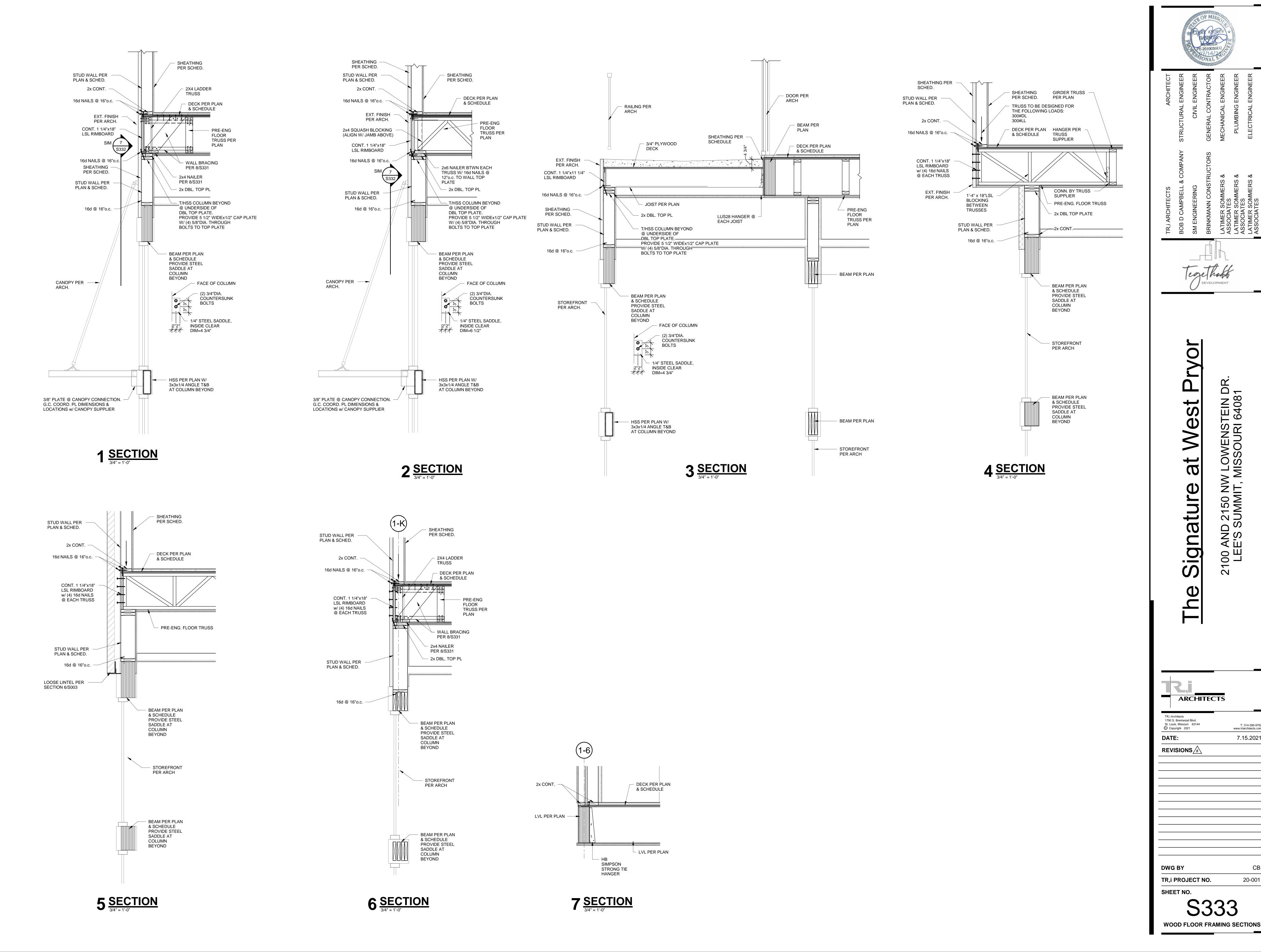




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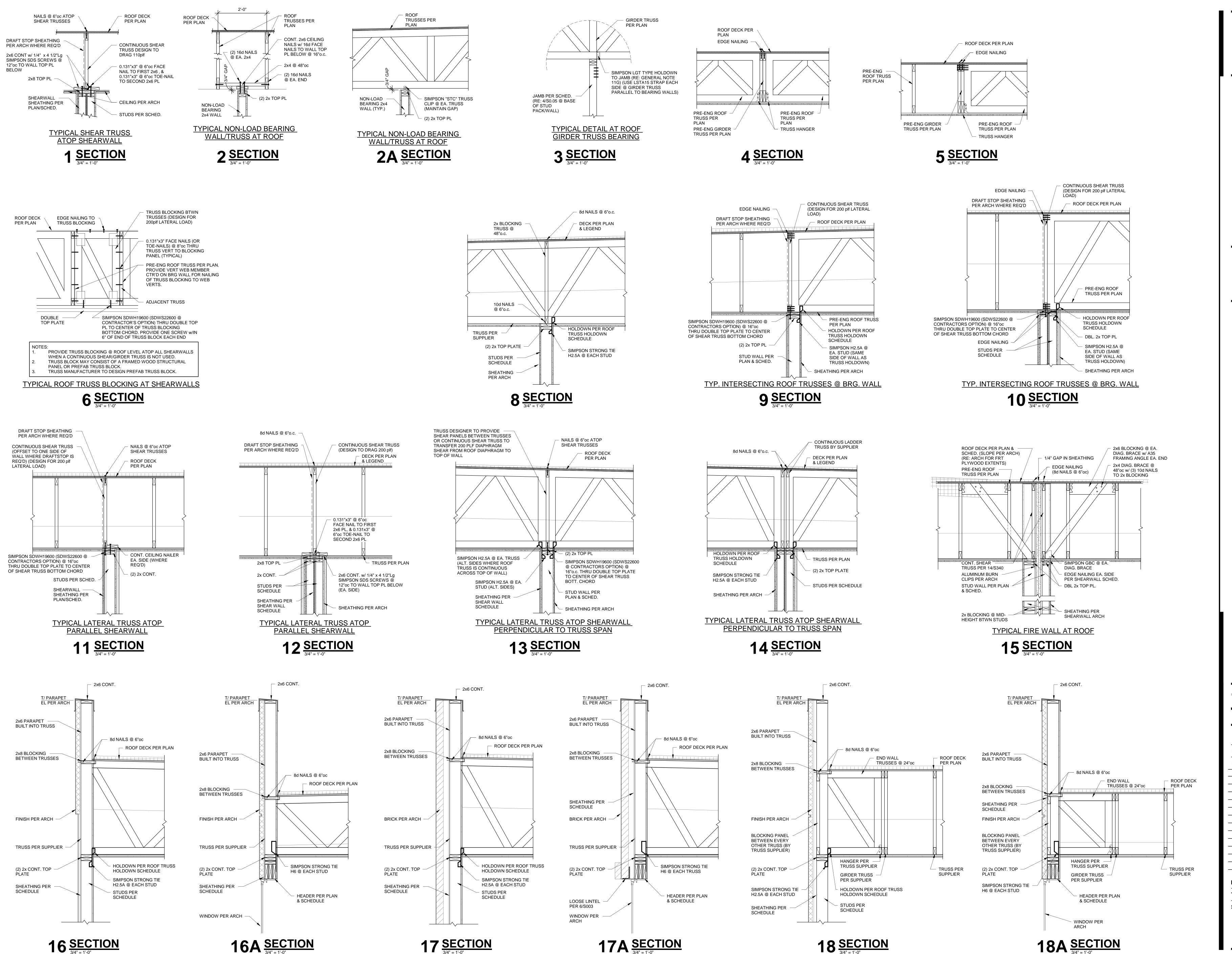
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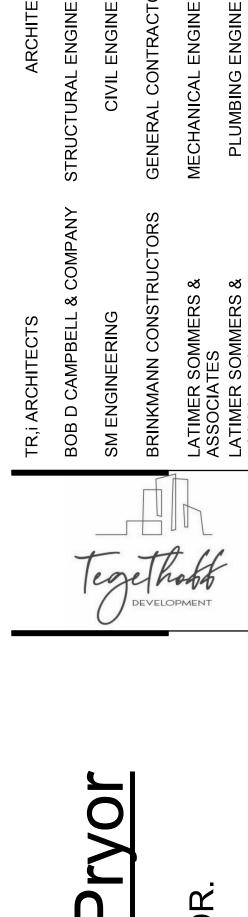




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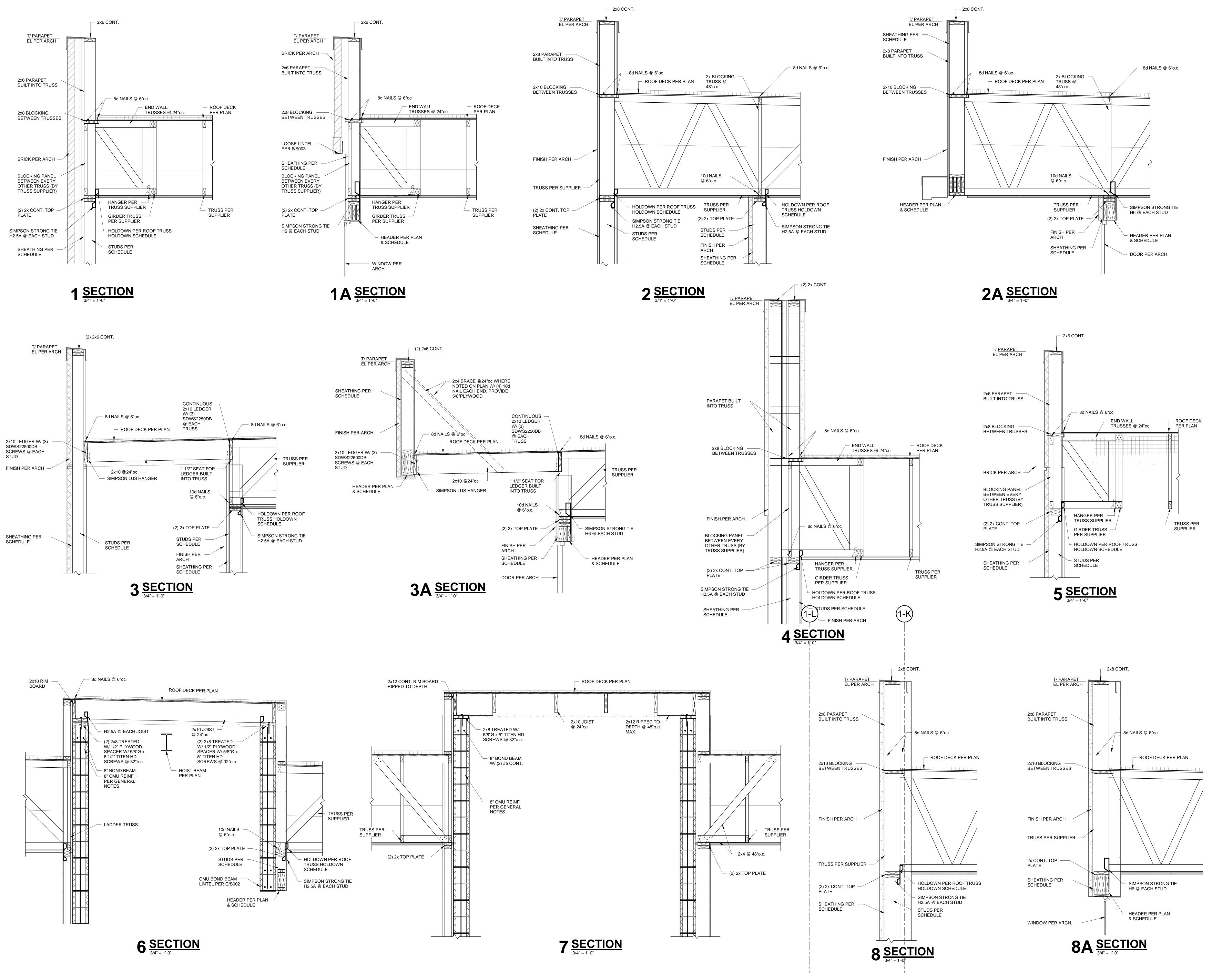
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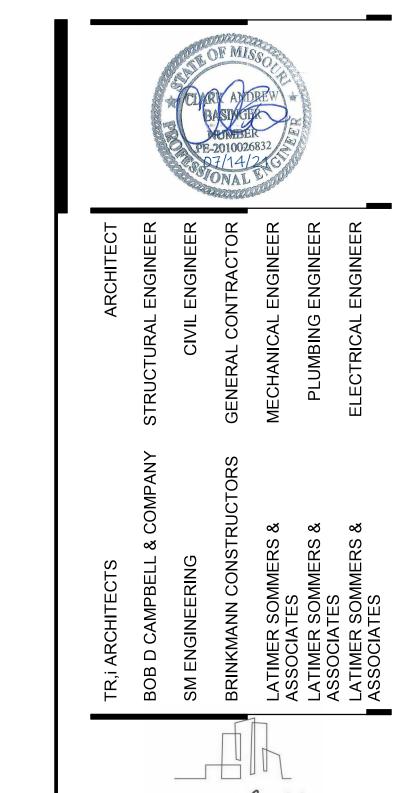




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WOOD ROOF FRAMING SECTIONS

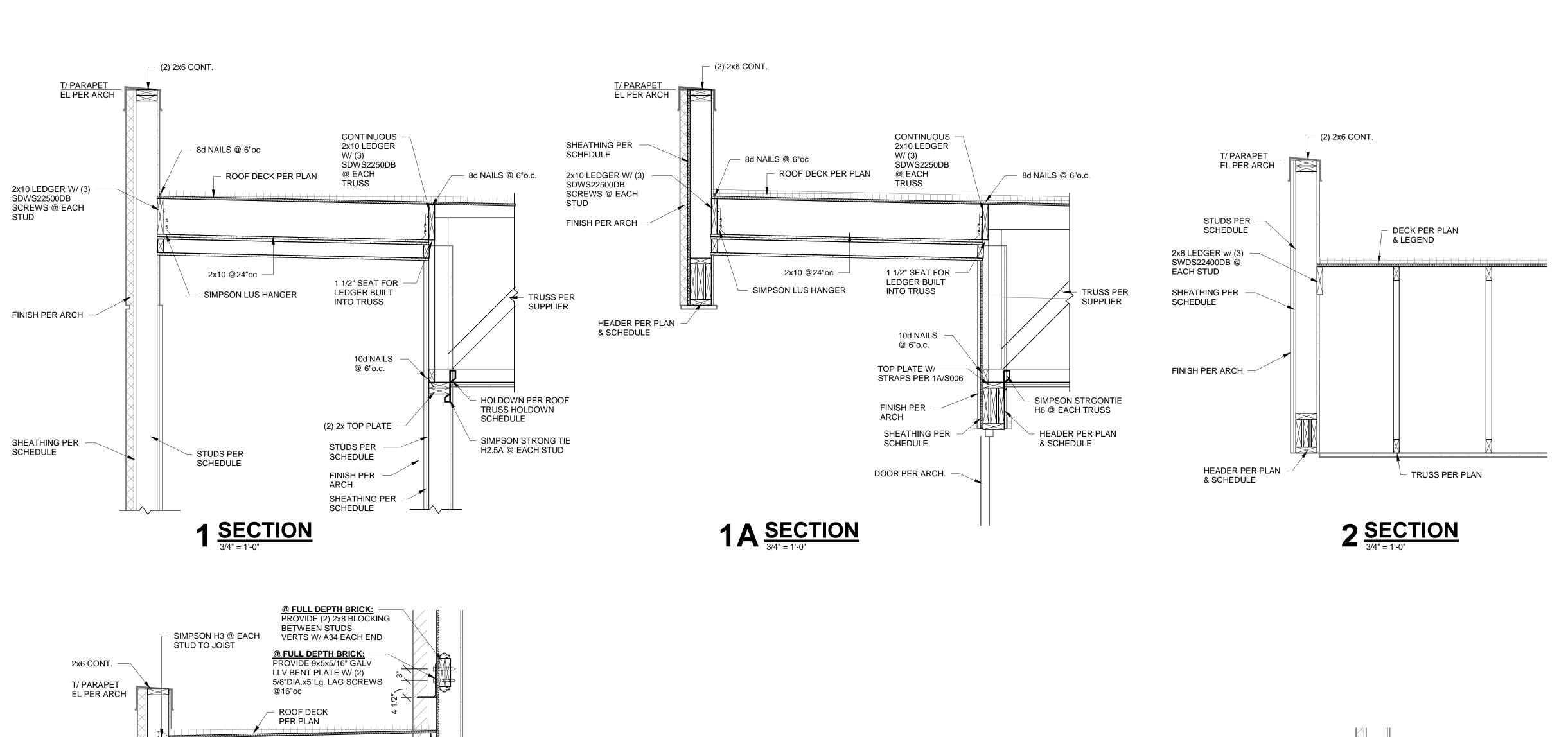




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SHEET NO. S34	.1
WOOD ROOF FRAMIN	IG SECTIONS



2x6 CONT.

FINISH PER ARCH.

T/ PARAPET EL PER ARCH

ROOF DECK PER PLAN

2x10 LEDGER W/ (3) SIMPSON SDWS22600DB @ EACH STUD

(2) 2x6 @ 16"oc

JOIST PER PLAN TYP.

5 **SECTION**3/4" = 1'-0"

__ 2x8 @ 16"oc

4 **SECTION** 3/4" = 1'-0"

2x10 LEDGER W/ (3) SIMPSON -SDWS22600DB @ EACH STUD

STUD WALL PER
 PLAN & SCHED.

- JOIST PER PLAN

- LUS @ EACH JOIST

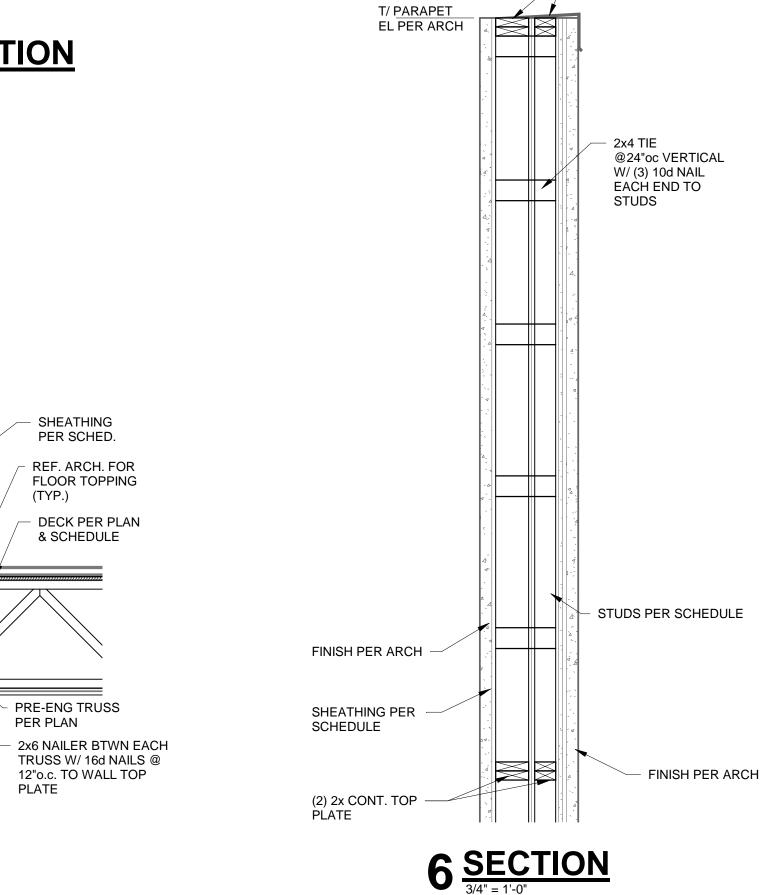
2x10 LEDGER W/ (3) SIMPSON SDWS22600DB @ EACH STUD

LUS @ EACH JOIST

- (2) 2x6 @ 16"oc

FINISH PER ARCH.

- 2x10 LEDGER W/ (3) SIMPSON SDWS22600DB @ EACH STUD



SHEATHING PER SCHED.

REF. ARCH. FOR FLOOR TOPPING

DECK PER PLAN & SCHEDULE

PRE-ENG TRUSS

PER PLAN

STUD WALL PER -PLAN & SCHED.

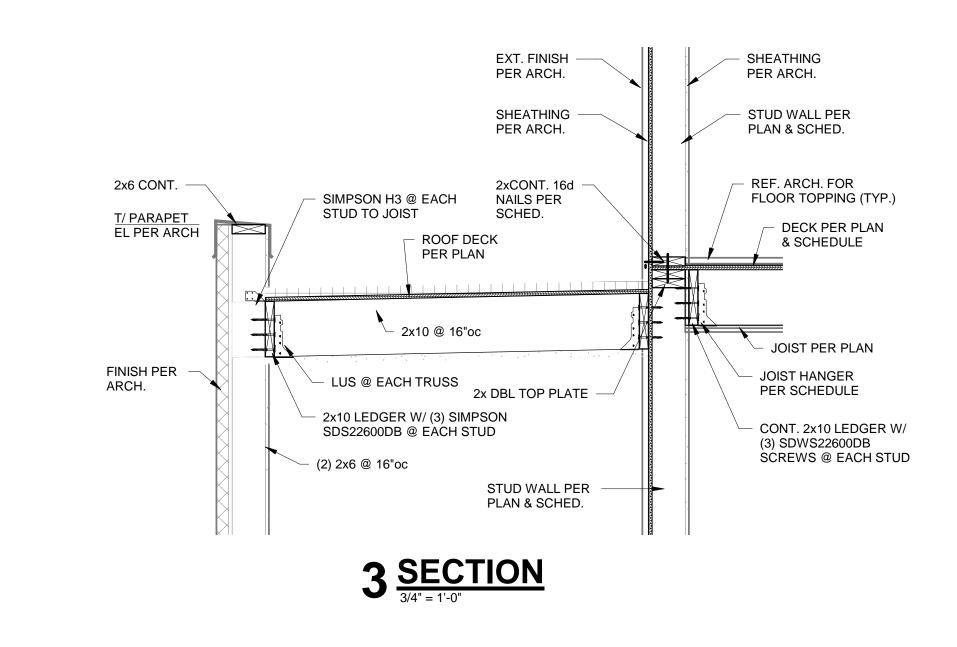
16d NAILS @ 16"o.c.

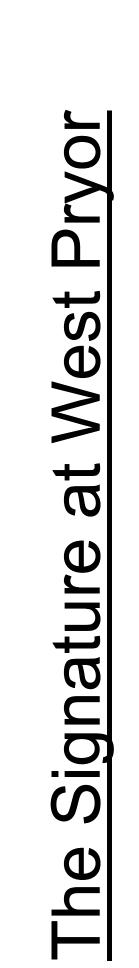
1 1/4"x18" LSL RIM -BOARD CONT.

2x10 LEDGER w/ (3) SDWS22600DB @ EACH TRUSS

16d NAILS @ 16"o.c.

2x CONT.





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