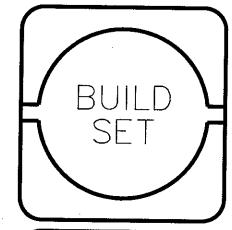


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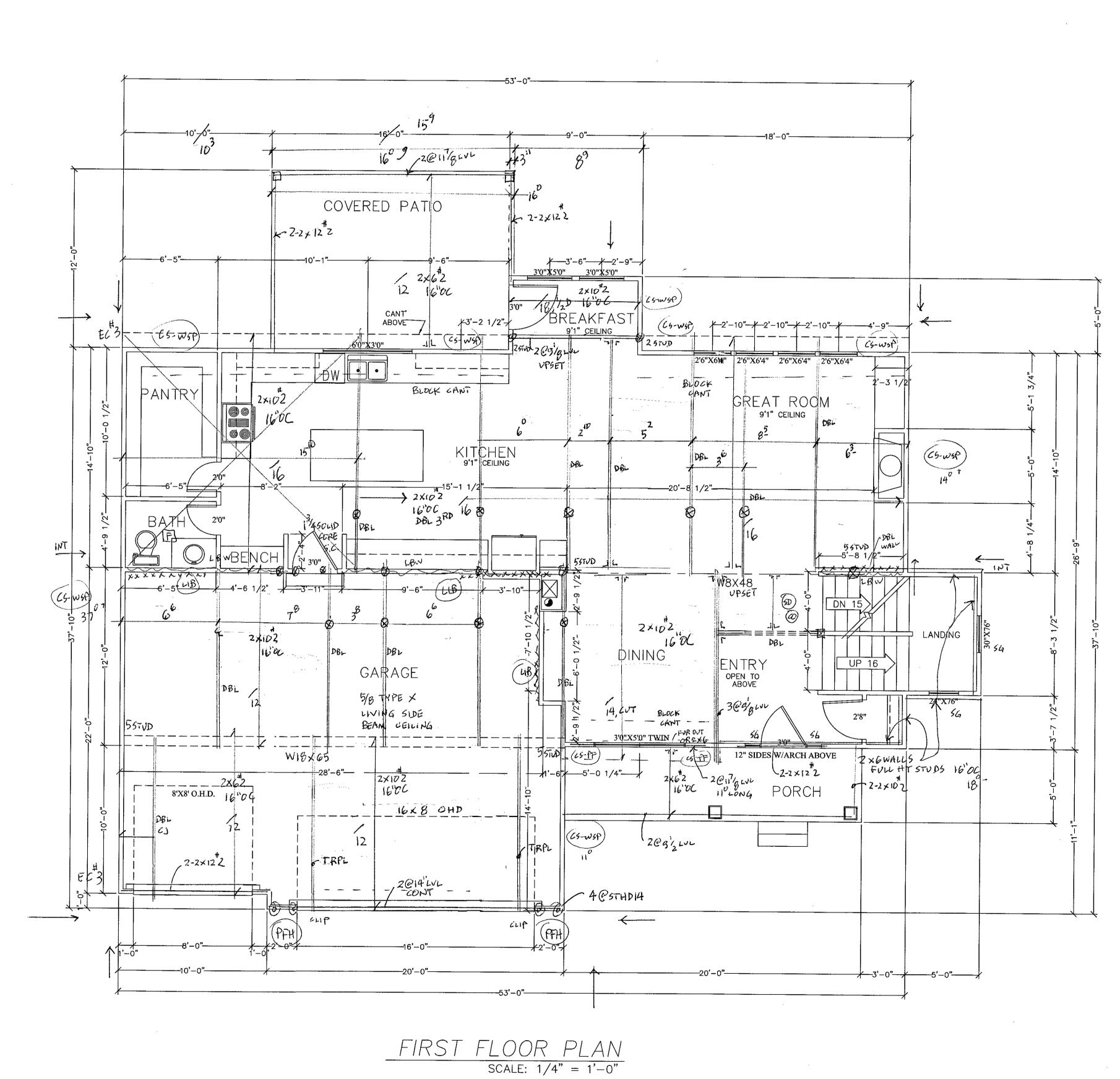
MODEL: COBEY CREEK

DATE:

ARCHITECT IS NOT RESPONSIBLE FOR THE STRUCTURAL ELEMENTS OF THESE PLANS. A STRUCTURAL ENGINEER MAY NEED TO VERIFY ALL STRUCTURAL ASPECTS OF THESE PRINTS BEFORE CONSTRUCTION BEGINS. FIELD CONDITIONS MAY BE DIFFERENT FROM PLAN. ALL STATE AND LOCAL CODES TAKE PRECIDENCE OVER THESE PLANS. CONTRACTOR WILL BE RESPONSIBLE FOR PLAN INTEGRITY AND CODE COMPLIANCE



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2 ® A35, 1 each Hdr.

DF/L MIN

HOUSE IS SHEATHED W/ 18" OSB APA PANELS, SMART PANEL OR EQUAL, INSTALLED PER MANU. SPECS, SHIP LAPPED PANELS REQUIRE NAILING OF OVER AND UNDER PANELS SEPARATELY.

LIB INT SHALL BE SIMPSON STRAP (CS16)

CS-PF HEADER LENGTHS ARE SHOWN

SIDING LAPS RIM

2x4, 9' PLATE, FULL HT. STUDS

S.C. = SELF CLOSING

D2 GN #25 FOR WINDOWS

CS = CONTINUOUSLY SHEATHED

EC = END CONDITION

SEE D2 FOR INSULATION VALUES

EC#5, 16" LONG CS16 STRAP,
CENTERED ON SUBFLOOR, FILL
ALL NAIL HOLES.

ARCHITECT IS NOT RESPONSIBLE FOR THE STRUCTURAL ELEMENTS OF THESE PLANS. A STRUCTURAL ELEMENTS OF THESE PLANS. A STRUCTURAL ELEMENTS OF THESE PLANS.

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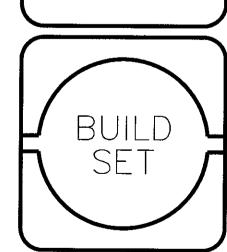
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STRUCTURAL ELEMENTS OF THESE PLANS. A STRUCTURAL ENGINEER MAY NEED TO VERIFY ALL STRUCTURAL ASPECTS OF THESE PRINTS BEFORE CONSTRUCTION BEGINS. FIELD CONDITIONS MAY BE DIFFERENT FROM PLAN. ALL STATE AND LOCAL CODES TAKE PRECIDENCE OVER THESE PLANS. CONTRACTOR WILL BE RESPONSIBLE FOR PLAN INTEGRITY AND CODE COMPLIANCE

528 SE David Rd. Cobey Creek Lot 7 Lee's Summit, MO

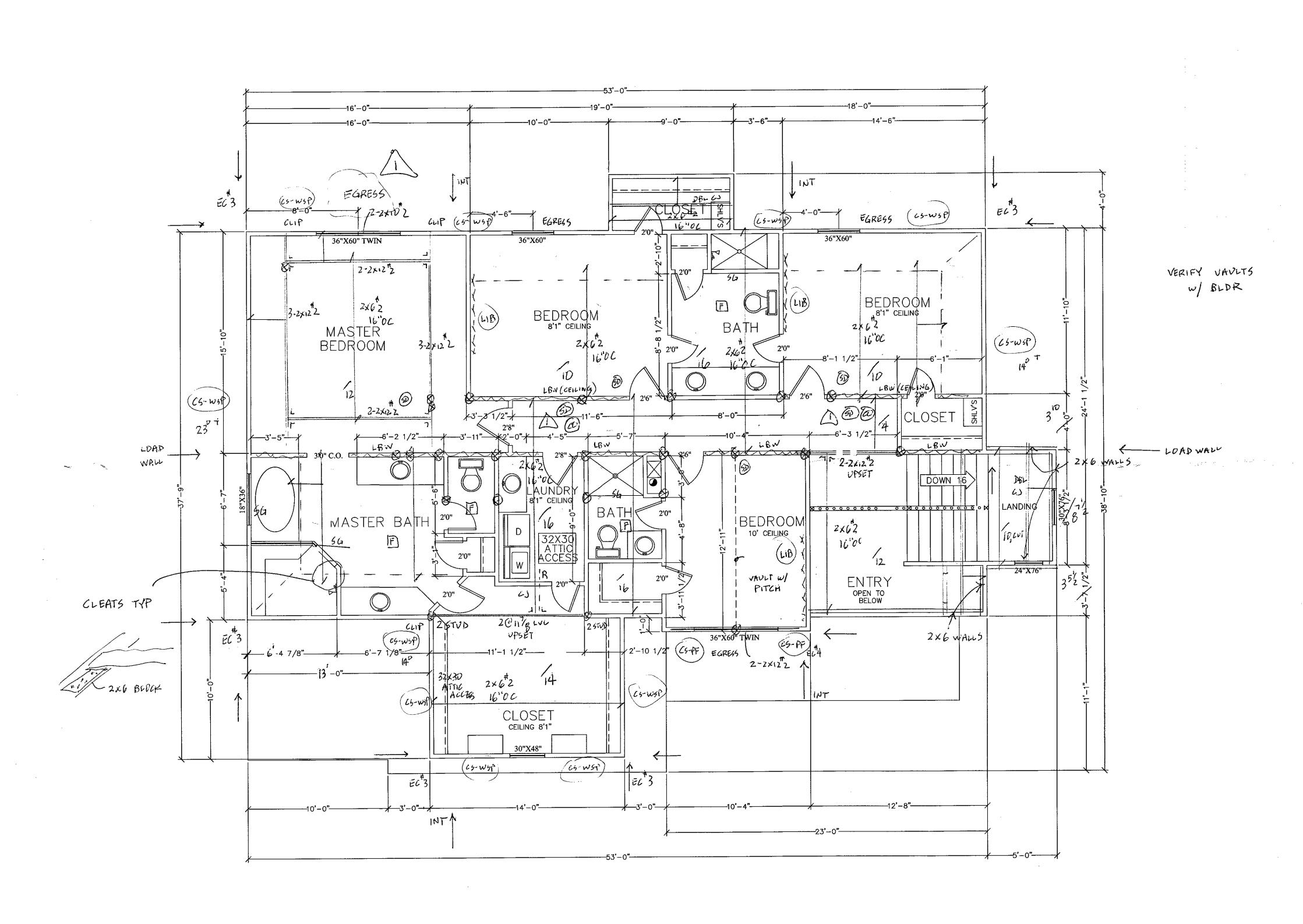


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DESCRIPTION:

SECOND FLOOR FRAMI

ROOF FRAMING PLAN

MODEL: *cobey creek*

DATE: 5/7/21

ARCHITECT IS NOT RESPONSIBLE FOR THE STRUCTURAL ELEMENTS OF THESE PLANS. A STRUCTURAL ENGINEER MAY NEED TO VERIFY ALL STRUCTURAL ASPECTS OF THESE PRINTS BEFORE CONSTRUCTION BEGINS. FIELD CONDITIONS MAY BE DIFFERENT FROM PLAN. ALL STATE AND LOCAL CODES TAKE PRECIDENCE OVER THESE PLANS. CONTRACTOR WILL BE RESPONSIBLE FOR PLAN INTEGRITY AND CODE COMPLIANCE

BUILD SET

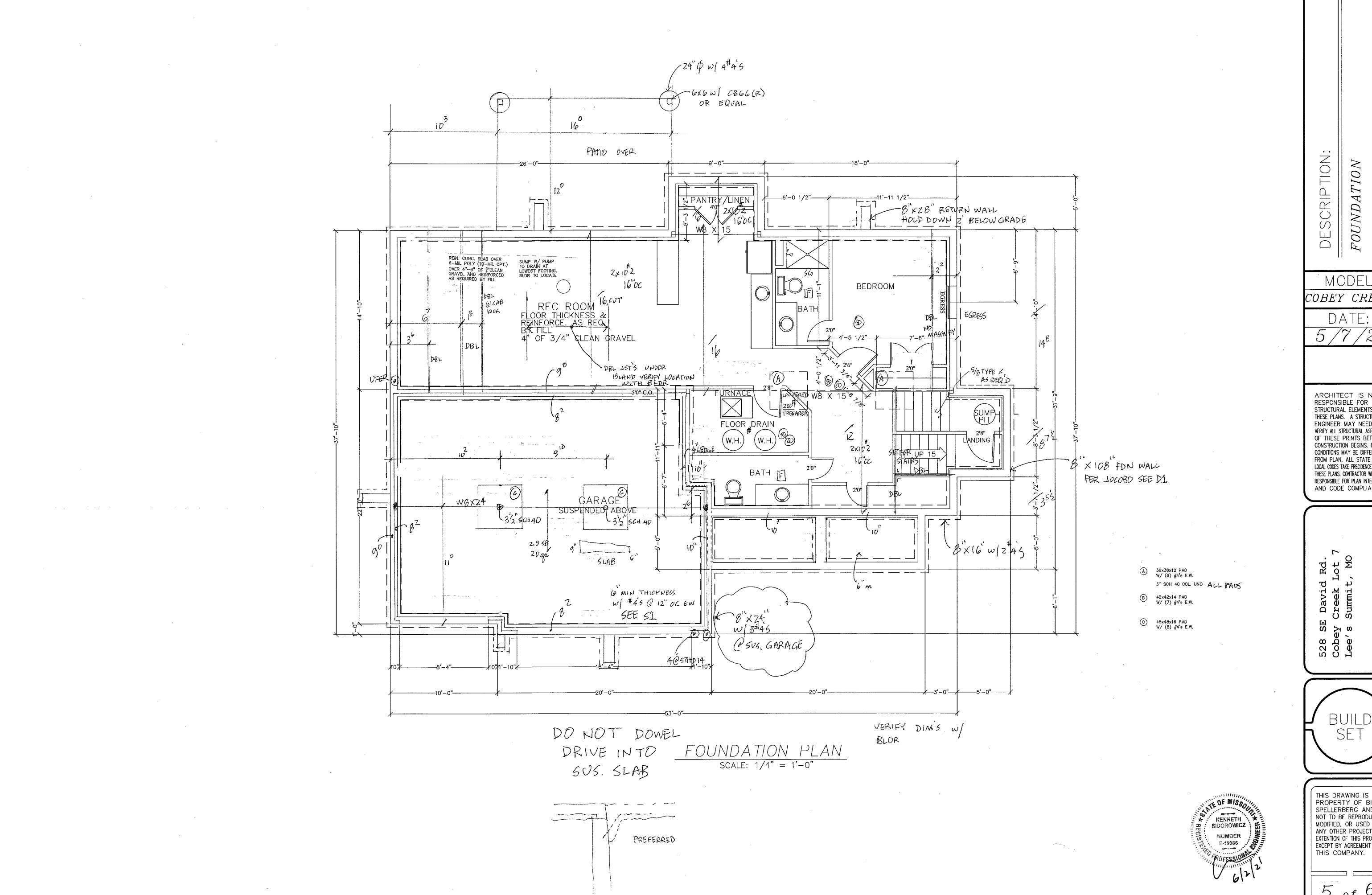
KENNETH SIDOROWICZ NUMBER E-19985

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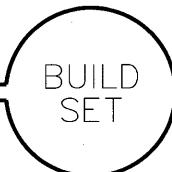
PELEASE FOR CONSTRUCTION
AS NOTED FOR PLAN REVIEW
DEVELOPMENT SERVICES
LEE'S SUMMIT, MISSOURI
06/07/2021

SECOND FLOOR PLAN SCALE: 1/4" = 1'-0"



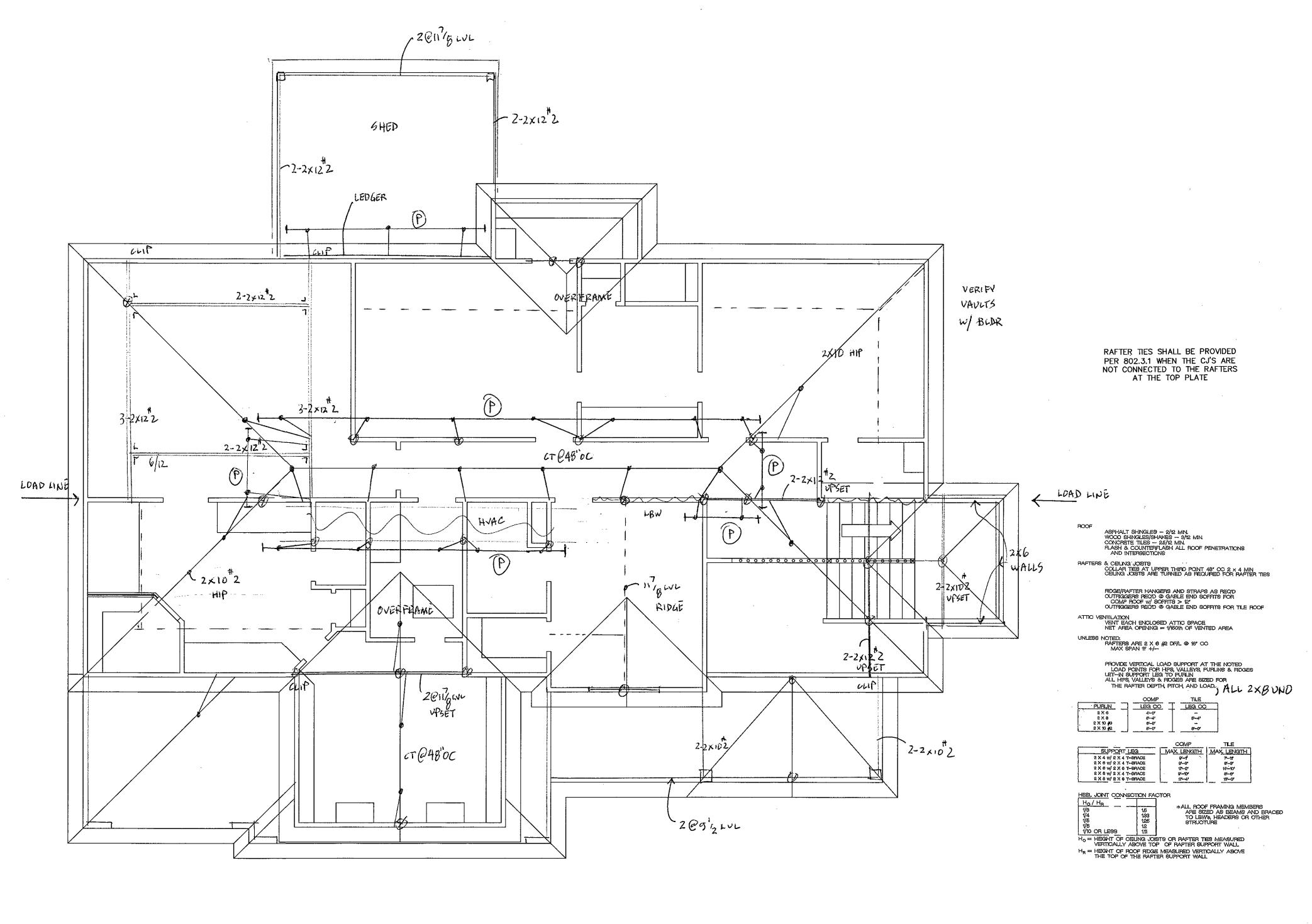
MODEL: COBEY CREEK

ARCHITECT IS NOT RESPONSIBLE FOR THE STRUCTURAL ELEMENTS OF THESE PLANS. A STRUCTURAL ENGINEER MAY NEED TO VERIFY ALL STRUCTURAL ASPECTS OF THESE PRINTS BEFORE CONSTRUCTION BEGINS. FIELD CONDITIONS MAY BE DIFFERENT FROM PLAN. ALL STATE AND THESE PLANS. CONTRACTOR WILL BE RESPONSIBLE FOR PLAN INTEGRITY AND CODE COMPLIANCE



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LEE'S SUMMIT, MISSOURI
06/07/2021



ROOF FRAMING PLAN
SCALE: 1/4" = 1'-0"



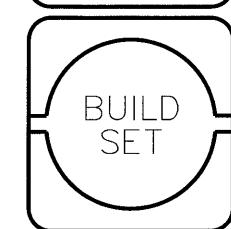
DESCRIPTION:
FIRST FLOOR FRAMING

MODEL: cobey creek

DATE: 5/7/21

ARCHITECT IS NOT RESPONSIBLE FOR THE STRUCTURAL ELEMENTS OF THESE PLANS. A STRUCTURAL ENGINEER MAY NEED TO VERIFY ALL STRUCTURAL ASPECTS OF THESE PRINTS BEFORE CONSTRUCTION BEGINS. FIELD CONDITIONS MAY BE DIFFERENT FROM PLAN. ALL STATE AND LOCAL CODES TAKE PRECIDENCE OVER THESE PLANS. CONTRACTOR WILL BE RESPONSIBLE FOR PLAN INTEGRITY AND CODE COMPLIANCE

528 SE David Rd. Cobey Creek Lot 7 Lee's Summit, MO

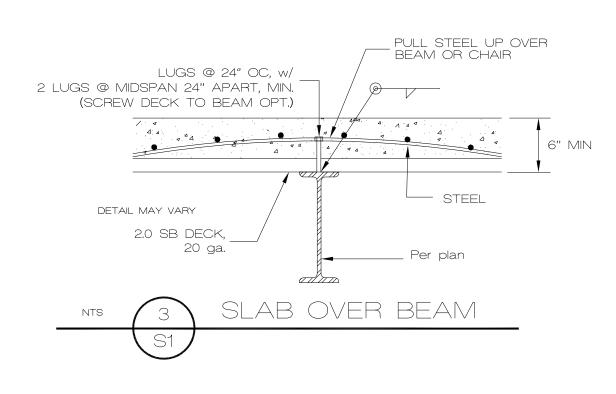


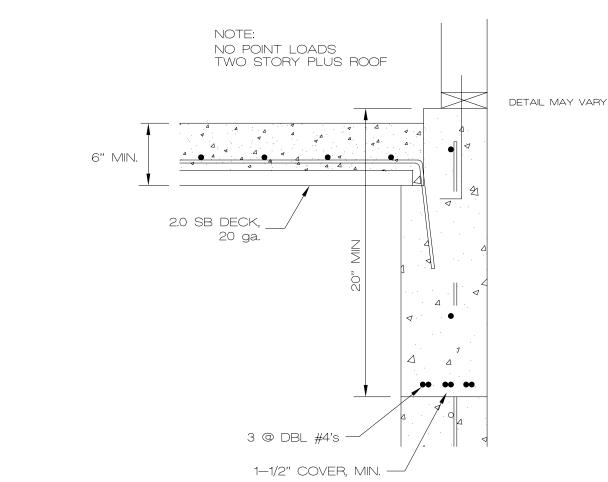
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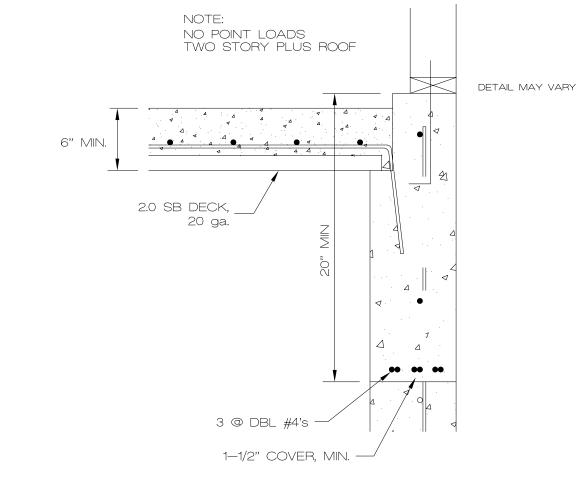
6 of 6

RELEASE FOR CONSTRUCTION
NOTED FOR PLAN REVIEW
DEVELOPMENT SERVICES
LEE'S SUMMIT, MISSOURI
06/07/2021

Total Slab







6'-0" MAX. OPENING HDR

VERIFY RO W/ BLDR

NOTES:

2.0 SB Normal Weight

145 pcf Normal Weight Concrete

Properties

Maximum Unshored Composite

Wt. Conc. Gage Single Double Triple lavg Sc

SET LEDGE 658 #'s OF CEMENT PER YD. MINIMUM (7 SACK) PROVIDE TEMPORARY DECK SUPPORT, READY AT INSPECTION PROVIDE TEMPORARY DECK SUPPORT, READY AT INSPECTION REBAR SHALL BE GR40 MIN.

TIE STEEL TO PREVENT DISPLACEMENT
SEAL OR WATERSTOP AS REQ'D
HOOK AND TIE STEEL AS POSSIBLE
SET STEEL ON CHAIRS AS REQ'D
SEAL AT PERIMETER AS REQ'D
SEAL ALL PENETRATIONS
DO NOT SAW CUT STRUCTURAL SLAB W/O APPROVAL
CONSTRUCTION SHALL MEET ALL APPLICABLE STANDARDS
CONSTRUCTION SHALL COMPLY WITH IRC

Section Properties (per ft. of width)

Span Span in⁴ / ft in³ / ft 7'-0" 7-6" 8'-0" 8'-6" 9'-0" 9'-6" 10'-0" 10'-6" 11'-0" 11'-6" 12'-0" 12'-6"

20 7'-2" 9'-1" 9'-5" 13.548 2.010 400 400 400 393 348 309 276 247 222 200 181

60.4 psf 18 8'-0" 10'-0" 10'-4" 14.981 2.589 400 400 400 400 359 320 285 256 230 207 187

42.7 in² | 16 | 9'-3" | 11'-4" | 11'-9" | 16.369 | 3.184 | 400 | 400 | 400 | 400 | 400 | 359 | 320 | 285 | 256 | 230 | 207 | 187

22 0.0295 2.0 0.257 0.258 0.317 0.309 0.472 50 20 0.0358 2.3 0.334 0.337 0.402 0.393 0.573 50

18 0.0474 3.0 0.507 0.517 0.557 0.552 0.759 40 16 | 0.0600 | 3.7 | 0.659 | 0.663 | 0.705 | 0.705 | 0.961 | 40

Superimposed Live Loads - psf: No Studs

DETAIL MAY VARY	12" MIN	— CAP WALL — DOWELS @ 12"
NS 6 S1	SLAB AT WALL	

DO NOT DOWEL DRIVE INTO STRUCTURAL GARAGE FLOORS

Metal Decking Details

ISSUE DATE 6/2/21 **REVISIONS**



06/07/2021

UFER GROUND NEAR PANEL <u>DIVISION 1</u> — GENERAL REQUIREMENTS <u>DIVISION 3</u> — CONCRETE DIVISION 4 - MASONRY <u>DIVISION 6</u> — ROUGH CARPENTRY DETAIL MAY VARY 1. DESIGN AND CONSTRUCTION WORK FOR THIS PROJECT SHALL 1. ALL ROUGH CARPENTRY WORK SHALL CONFORM TO THE 1. ALL CONCRETE WORK SHALL CONFORM TO THE REQUIREMENTS OF COMPRESSIVE STRENGTH OF CONCRETE MASONRY CONSTRUCTION J-BOLTS @ 3' OC REQUIREMENTS OF NFPA "NATIONAL DESIGN SPECIFICATION OF WOOD ACI 318 "BUILDING CODE REQUIREMENTS FOR REINFORCED CONFORM TO THE REQUIREMENTS OF THE 2018 IRC. (CMU) SHALL BE AS FOLLOWS (PSI). MASONRY STRENGTH NOT 7" EMBED CONCRETE" AND ACI 332 "REQUIREMENTS FOR RESIDENTIAL CONSTRUCTION". TPI "DESIGN SPECIFICATIONS FOR LIGHT METAL SPECIFICALLY NOTED ON PLAN SHALL BE (f'm) 1500 PSI. GRADE PLATE CONNECTED WOOD TRUSSES", APA "PLYWOOD DESIGN 2. FURNISH ALL LABOR, MATERIALS, AND EQUIPMENT NECESSARY TO CONCRETE CONSTRUCTION." MASONRY STRENGTH (F'm DESIGN)_ COMPLETE THE WORK AS SHOWN OR INFERRED BY THE DRAWINGS. SPECIFICATIONS", DOC PS 1 "PRODUCT STANDARD FOR CONSTRUCTION DAMP-PROOFING AND INDUSTRIAL PLYWOOD", DOC PS 56 "STRUCTURAL GLUED CONCRETE MATERIALS SHALL COMPLY WITH: BLOCK STRENGTH **VARIES** PER R406.1 3. DESIGN FACTORS: A) CEMENT - ASTM C 150 TYPE LAMINATED TIMBER", AND APPLICABLE SECTIONS OF THE MORTAR STRENGTH 1800 HORIZONTAL REINFORCEMENT A) GROUND SNOW LOAD (INCLUDING DRIFTING SNOW)__20 PSF INTERNATIONAL BUILDING CODE. B) AGGREGATE - ASTM C 33, MAXIMUM AGGREGATE SIZE 3/4" GROUT STRENGTH_ B) WIND SPEED (EXPOSURE B)_ C) WATER - POTABLE, WATER/CEMENT RATIO .5 (MAX.) 2. ROUGH CARPENTRY MATERIALS SHALL COMPLY WITH: C) SEISMIC CATEGORY (A), GROUND ACCELERATION = NA D) AIR-ENTRAINING ADMIXTURE - ASTM C 260 2. CONCRETE BLOCK SHALL BE HOLLOW LOAD-BEARING CONCRETE A) LUMBER - S4S, S-DRY, KD, OR S-GRN GRADE MARKED, MASONRY UNITS CONFORMING TO ASTM C 90, TYPE N-II. ALL FOUNDATION PER JOCOBO E) WATER-REDUCING ADMIXTURE - ASTM C 494, INCLUDING COMPLYING WITH PS 20, GRADED UNDER WWPA OR SPIB RULES: 4. DESIGN LOADS (PSF, UNLESS NOTED OTHERWISE): BLOCKS SHALL BE PLACED IN RUNNING BOND CONSTRUCTION SUPERPLASTICIZERS. STUDS: STUD GRADE (UNLESS OTHERWISE NOTED) WITH ALL VERTICAL CELLS IN A) ROOF (LL/DL)_ SEE TABLE VARIES F) FLY ASH - ASTM C 618, CLASS C #2 DOUGLAS FIR MIN TYPICAL HEADER: ALIGNMENT. B) FLOOR (LL/DL)_ _SEE TABLE 60 PCF EQUIVALENT #2 DOUGLAS FIR **RAFTER:** _SEE TABLE, (0/10 TRUSSES) 3. CONCRETE SHALL DEVELOP THE FOLLOWING MINIMUM 28 DAY C) CEILING (LL/DL)_ VERTICAL REINFORCEMENT, FLUID WEIGHT SOIL | 3. MORTAR MIX SHALL CONFORM TO THE REQUIREMENTS OF ASTM C PLATES: #2 DOUGLAS FIR 5" EMBED (NO DOWEL OPTION) DESIGN COMPRESSIVE STRENGTH (f'c): 270, TYPE M OR S. TYPE M MORTAR SHALL BE USED WHERE BLOCKING: #2 DOUGLAS FIR 5. DO NOT SCALE DRAWINGS. IF DIMENSIONS ARE IN QUESTION, COMP. STRENGTH (f'c) TYPE OF CONSTRUCTION CENTER PER 403.1.3.5.3 B) METAL FRAMING FASTENERS - ASTM A 153, HOT-DIP MASONRY IS IN CONTACT WITH SOIL. OBTAIN CLARIFICATION FROM A / E BEFORE CONTINUING A) FOOTINGS, WALLS, AND SLABS SEE TABLE GALVANIZED FASTENERS; EQUAL TO SIMPSON STRONG-TIE CONSTRUCTION. B) EXTERIOR SLABS AND CURBS SEE TABLE DOWEL w/ 5" EMBED 4. GROUT SHALL CONFORM TO THE REQUIREMENTS OF ASTM C 476. CONNECTORS COMPLYING WITH APPLICABLE ICC-ES REPORTS VARIES (AIR-ENTRAINED CONCRETE) (DOWEL OPTION) C) PLYWOOD — APA RATED SHEATHING, COMPLYING TO PS 1. USE SUFFICIENT WATER FOR GROUT TO FLOW INTO ALL JOINTS OF 6. THE CONTRACTOR SHALL EXAMINE ACTUAL JOB CONDITIONS AND THE MASONRY WITHOUT SEGREGATION. ALL CELLS IN CONCRETE D) LVL - LAMINATED VENEER LUMBER SHALL BE GRADE 2800 BE RESPONSIBLE FOR VERIFYING ALL DIMENSIONS AND ELEVATIONS CONCRETE PROPORTIONS SHALL BE ESTABLISHED ON THE BASIS F-2.0E AND SHALL MEET THE REQUIREMENTS OF APPLICABLE BLOCKS CONTAINING REINFORCING SHALL BE FILLED SOLID WITH SHOWN ON THE PLANS. IF ERRORS, OMISSIONS, OR DISCREPANCIES OF FIELD EXPERIENCE AND/OR TRIAL MIXTURES IN ACCORDANCE GROUT. ALL MASONRY BELOW FINISHED FLOOR OR GRADE SHALL ICC-ES REPORTS. ARE FOUND THEY SHALL BE REPORTED TO THE DESIGN WITH ACI 318-89 SECTIONS 5.2 AND 5.3. WHEN FLY ASH IS UTILIZED DRAIN TILE E) GLULAM BEAMS - COMBINATION 24F-V3 IN ACCORDANCE BE GROUTED SOLID. HOLD GROUT DOWN 1-1" BELOW TOP OF PROFESSIONAL BEFORE PROCEEDING WITH THE WORK. IN THE MIX, MIX SHALL CONTAIN A WATER-REDUCER. FLY ASH PER R405 ⁻ BLOCK AT GROUT LIFT JOINTS AND AT CONCRETE PLACED OVER WITH AITC A190.1 B" WALL REIN. PER JOCOBO SHALL BE ADDED AT THE RATE OF NOT MORE THAN 100 POUNDS DIMENSIONS FOR NEW CONSTRUCTION ARE TO FACE OF FINISH OR **VARIES** 9' MAX HEIGHT PER CUBIC YARD AND CEMENT SHALL BE REDUCED BY NOT MORE COLUMNS AND FACE OF CONCRETE, WOOD, OR MASONRY WALLS 3. EXTERIOR WALL AND ROOF SHEATHING SHALL BE 76" APA RATED RETURN WALL-THAN 15 PERCENT BY WEIGHT. UNLESS OTHERWISE INDICATED. DIMENSIONS INDICATE NOMINAL 5. MINIMUM LINTEL, WHERE NOT ON PLANS, SHALL HAVE A MINIMUM SHEATHING 24/0 EXTERIOR GLUED (MIN) FOR 16" OC STUD SPACING. DIMENSIONS RATHER THAN ACTUAL DIMENSIONS. OF 2 — #5's CONTINOUS HORIZONTAL BARS IN BOTTOM OF BOND NAIL SHEATHING TO SUPPORT MEMBERS WITH 8D COMMON NAILS AT 4. PROPORTION AND DESIGN MIXES TO RESULT IN CONCRETE SLUMP BEAM OR LINTEL BLOCK AND SHALL BE GROUTED SOLID TO A MIN. 6" ON CENTER ALONG EDGE SUPPORTS AND 12" ON CENTER ALONG AT A POINT OF PLACEMENT OF NOT MORE THAN 4" TO 5". 8. CONTRACTOR IS RESPONSIBLE FOR COORDINATING ALL TRADES DEPTH OF 24." ALL LINTEL REINFORCING AND GROUT SHALL FIELD SUPPORTS UNLESS NOTED OTHERWIDE. PROVIDE SOLID EVEN IF THE TRADE IS UNDER A SEPARATE CONTRACT EXTEND 2' MINIMUM PAST JAMBS UNLESS NOTED OTHERWISE ON BLOCKING AT ALL UNSUPPORTED PANEL EDGES; 4/8 GUN NAILS. 5. USE AIR-ENTRAINING ADMIXTURES IN EXTERIOR EXPOSED 3" MIN COVER PLANS OR DETAILS. CONCRETE TO RESULT IN CONCRETE AT POINT OF PLACEMENT 9. PROVIDE SUFFICIENT STUDS AND BLOCKING WHERE REQUIRED TO HAVING AIR CONTENT OF 5 TO 7 PERCENT ENTRAINED AIR. NOTE: ROOF SHEATHING SHALL BE §" APA RATED SHEATHING FOR 6. LAP REINFORCING 48 BAR DIAMETERS. STAGGER LAP SPLICES A SUPPORT EQUIPMENT AND/OR MISCELLANEOUS ITEMS, I.E., LOAD FTG REINFORCEMENT, 8" WALL TILE ROOF, OR AS REQUIRED BY MANUFACTURER. MINIMUM OF ONE LAP LENGTH. POINTS, TYPICAL CASEWORK, CABINETS, GRAB BARS ETC. $8" \times 16" \text{ w/}$ 6. ALL PLUMBING AND ELECTRICAL ROUGH-INS MUST BE COMPLETE, INSPECTED AND APPROVED BEFORE REQUESTING THE SLAB 2#4 CONTINUOUS 4. INTERIOR SHEAR WALL SHEATHING WHERE NOTED SHALL BE 点 APA 7. MASONRY VENEER SHALL BE ATTACHED TO SUPPORT WALL 10. PRETREAT FOUNDATION FOR TERMITES AS REQUIRED. RATED SHEATHING 24/0 EXTERIOR GLUED (MIN) FOR 16" OC STUD FRAMING WITH &" DIAMETER WALL TIES OR DOVETAIL-TYPE METAL WALL REINFORCEMENT SPACING. NAIL SHEATHING TO SUPPORT MEMBERS WITH 8D COMMON. 11. GARAGE DOORS AND FRAMES SHALL BE DESIGNED AND INSTALLED TIES OF EQUIVALENT STIFFNESS EMBEDDED INTO HORIZONTAL 7. CONCRETE WORK EXECUTION: NAILS AT 4" ON CENTER ALONG EDGE SUPPORTS AND 6" ON CENTER TO MEET THE 115 MPH WIND LOAD RESISTANCE REQUIREMENTS OF MORTAR JOINTS. MAXIMUM VERTICAL SPACING OF TIES SHALL BE A) MINIMUM CONCRETE COVER FOR REINFORCING SHALL BE, UNLESS ALONG FIELD SUPPORTS UNLESS NOTED OTHERWISE, PROVIDE SOLID DASMA 108 AND ASTM E 330. 16." MAXIMUM HORIZONTAL SPACING SHALL BE 24." TIES IN NOTED OTHERWISE ON DRAWINGS: BLOCKING AT ALL UNSUPPORTED PANEL EDGES. CAST AGAINST AND EXPOSED TO EARTH_ ALTERNATE COURSES SHALL BE STAGGERED. PROVIDE #9 WIRE 12. ALL EXTERIOR DOORS, INCLUDING THE DOOR LEADING FROM THE REINFORCING IN HORIZONTAL MORTAR JOINTS AT 16" OC. ENGAGE EXPOSED TO EARTH OR WEATHER_ 5. ATTACH METAL FRAMING FASTENERS TO FRAMING MEMBERS WITH GARAGE TO THE DWELLING UNIT, SHALL INCORPORATE THE #9 WIRE WITH WALL ANCHOR TIES. CONSTRUCTION JOINTS IN NOT EXPOSED TO EARTH OR WEATHER. MINIMUM NUMBER AND SIZE OF NAILS LISTED IN THE APPLICABLE PHYSICAL SECURITY PROVISIONS OF THE JURISDICTION IN WHICH Less than 10" OC equivalent MASONRY VENEER WALLS SHALL BE LOCATED PER THE DRAWINGS. B) IN CORNERS OF GRADE BEAMS PROVIDE CORNER REINFORCEMENT ICC-ES REPORTS. THE CONSTRUCTION TAKES PLACE. LAP TWO FEET EACH DIRECTION IN OUTSIDE FACE, MATCHING SIZE 8. WATERPROOFING, DRAINAGE PLANE, AND INSTALLATION PER AND SPACING OF HORIZONTAL REINFORCEMENT. WOOD TRUSS SYSTEM; TRUSS JOIST SYSTEM AND GLULAM SYSTEM <u>DIVISION 2</u> — EARTHWORK ADOPTED BUILDING CODE. C) PROVIDE CONTROL JOINTS IN SLABS-ON-GRADE AT NOT GREATER FOR ROOFS: \(\) THAN 20 FEET ON CENTER IN EACH DIRECTION. SAW CUT A) DESIGN, FABRICATE, AND ERECT IN ACCORDANCE WITH BCSI 1. ALL PROPERTY MARKERS SHALL BE EXPOSED. <u>DIVISION 5.5</u> — MISC. STRUCTURAL STEEL STANDARDS AND NDS SPECIFICATIONS. CONTROL JOINTS MINIMUM 1/4 OF THE SLAB DEPTH, AS SOON AFTER SLAB FINISHING AS POSSIBLE WITHOUT DISLODGING B) DESIGN LOADS: 2. ALL FOOTINGS ARE DESIGNED TO BEAR ON NATURAL UNDISTURBED 1. ALL MISCELLANEOUS STRUCTURAL STEEL WORK SHALL CONFORM AGGREGATE. (DO NOT SAW CUT STRUCTURAL SLABS w/o 25 PSF SNOW LIVE LOAD SOIL CAPABLE OF ADEQUATELY SUSTAINING A MINIMUM BEARING TO THE REQUIREMENTS OF AISC "SPECIFICATIONS FOR DESIGN, 10 PSF DEAD LOAD TOP CHORD (20 TILE) PRESSURE OF 1,500 PSF. IF SUITABLE UNDISTURBED BEARING FABRICATION, AND ERECTION OF STRUCTURAL STEEL FOR 10 PSF DEAD LOAD BOTTOM CHORD DECK LEDGER CAPACITY IS NOT ENCOUNTERED AT THE ELEVATION INDICATED ON BATCH TICKETS SHALL BE SUBMITTED TO A CONTRACTORS C) SUBMIT SHOP DRAWINGS, INCLUDING DESIGN CALCULATIONS, THE DRAWINGS, CONTRACTOR SHALL NOTIFY THE ENGINEER OF REPRESENTATIVE PRIOR TO OFF LOADING. ANY CONCRETE MORE MATERIAL STRESSES, GRADE AND SPECIES OF WOOD, AND RECORD IMMEDIATELY MISCELLANEOUS STRUCTURAL STEEL MATERIAL SHALL COMPLY 18' max Joist Spar THAN 45 MINUTES OUT PRIOR TO STARTING PLACEMENT SHALL BE PLACEMENT DRAWING. 3. ALL TOPSOIL, ORGANIC MATERIAL, AND EXISTING STRUCTURES A) STRUCTURAL STEEL - ASTM A992 7. DEFAULT HEADER SIZE NOT SPECIFIED SPANNING 8'-0" MAX SHALL BE SHALL BE REMOVED FROM BUILDING AREA AND FROM AREAS TO B) STEEL PIPE COLUMNS - ASTM A53 GRADE B(Sch 40 TYP) THE MAXIMUM ADDITION OF WATER SHALL BE LIMITED TO 1 $2-2 \times 10$ #2, WITH 2 STUD SUPPORT. BE PAVED. STOCKPILE ALL TOPSOIL FOR REUSE. C) ANCHOR BOLTS - ASTM A307 GRADE A, NON-HEADED TYPE GALLON PER YARD; NOTE THAT THIS ADDITION SHALL BE USED TO UNLESS OTHERWISE NOTED. 8. ALL HEADERS OVER 4'-0" SHALL HAVE DOUBLE TRIMMER @ EACH CONTROL HEAT ONLY (NOT SLUMP). 4. REFERENCE THE SOILS REPORT FOR ALL FILL CONDITIONS. 1/2" \$\phi \times 6" Lag @ 16" OC, SECTION SUPPORT, OR AS SPECIFIED, UNO. DBL EVERY OTHER -3. FLITCH PLATES SHALL HAVE ½" DIA. BOLTS @ 16" OC, STAGGERED 10. PUMPS SHALL NOT BE PRIMED IN FORMS. 5. OVEREXCAVATE BUILDING AREA BELOW SLAB SUBGRADE (SEE SECTION VIEW) TOP AND BOTTOM BETWEEN JOIST LAYOUT. 9. SOLID BLOCKING BETWEEN JOISTS @ 36" OC FOR JOISTS PARALLEL ELEVATION AND REPLACE WITH MATERIAL PER SOILS REPORT, 11. REINFORCEMENT TO THE EXTERIOR FOUNDATION WALL, MIN. 48" OR 3 JOIST SPACES. A) ALL REINFORCING BARS SHALL BE A615, GR40 MIN. LAP SPLICES 18" LUS210 RETURN WALLS SHEATHING — MIN FOR #4 BAR. SEE TABLE 10. ALL FLUSH FRAMING @ HEADERS OR GIRDERS SHALL BE HANGERED. 6. SITE EROSION CONTROL SHALL COMPLY WITH ALL STATE AND B) WELDED WIRE FABRIC SHALL BE ASTM A185, LAP AT LEAST ONE LOCAL ORDINANCES. 11. BLOCK BETWEEN JOISTS @ SUPPORTS OR OVER BEAMS. FULL MESH AND LACE SPLICES WITH WIRE. C) REBAR SHALL BE CLEAN, AND FREE FROM RUST AND OIL PRIOR TO 7. IN-SITU SOIL CONDITIONS, SEE SOILS REPORT OR 1,500 PSF BEARING RETURN SPACING ABOVE FLOOR 12. RATED CONSTRUCTION FOR PROJECTIONS INTO SETBACKS AS REQ'D. THE PLACEMENT OF CONCRETE. REBAR SHALL BE TIED AND & 60 PCF EQUIVALENT FLUID WEIGHT. (HOLD DOWN 24" BELOW GRADE) SECURED AS REQUIRED TO PREVENT DISPLACEMENT IN THE FORMS. 13. DOUBLE JOIST BELOW PARALLEL NONBEARING WALLS ON LAYOUT, 600# @ 9' < 790# OK 8. SOIL CONDITIONS AT THE DEPTH OF EXCAVATION FOR THE D) TIE STEEL TO PREVENT DISPLACEMENT. HOOK AND TIE STEEL AS LESS THAN 4' RETURN WALLS NOT REQ'D SINGLE JOIST OFF LAYOUT. STRUCTURE BELOW LOAD-BEARING FOOTING SHALL BE UNIFORM AND CONSISTENT. NOTIFY THE POSSIBLE. TIES, CHAIRS, OR OTHER PRODUCTS SHALL BE 11' JOISTS WALLS AS NOTED ON PLANS. ENGINEER OF RECORD OF ANY INCONSISTENCIES. PROTECTED WHEN LOCATED NEAR EXPOSED SURFACES 16'-4" ON CENTER (MAX.), AND WITHIN >4' TO 9' E) STEEL SHALL BE STORED ON SITE ABOVE GRADE, AND COVERED CANT 8' OF STEP DOWN OR AS SHOWN 9. CONTRACTOR SHALL BE RESPONSIBLE FOR REMOVING AND AS REQUIRED FOR PROTECTION FROM RAIN AND OTHER POSSIBLE DISPOSING OF ANY EXCESS EXCAVATION MATERIALS AND FOR * RETURN WALLS ALLOW FOR BACKFILL W/O FLOOR DECK OBTAINING AND SUPPLYING ADDITIONAL FILL MATERIAL AS IN PLACE FOR 60 PCF EQUIVALENT FLUID WEIGHT SOIL. 12. ADJUST FOUNDATION FOR SITE AND SOIL CONDITIONS AND VERIFY DETAIL MAY VARY NO HEAVY EQUIPMENT OR SURCHARE LOADING. 48" LONG MIN. - WALL BELOW -2 X 10 @ 16" OC <u>PLAN</u> FOUNDATION PER DETAIL MAY VARY JOCOBO RESIDENTIAL DECK @ CANTILEVER ______ ______ TURN DOWN SLAB @ **FOUNDATION** HVAC BLOCK DOWN HVAC TRUNK 2 X 4 OR 2 X 6 STRUCT. SLAB MODULE SPACING **GUIDELINE** DIAGONAL STEEL 12" ADDITIONAL LSIMPSON MAS SLAB TYPE | MODULE SPACING BLOCK DOWN @ HVAC ALTERNATIVE BASEMENT 15'—6" INSTALLATION GARAGE DETAIL MAY VARY (MODULE ALSO APPLIES @ OVERDIG) OPT. MUDSILL ANCHORAGE WALL REINFORCING $\frac{x}{y} > 0.58$ 12" PED. MIN. LAB @ HVAC HOLD PED. DOWN 12" BELOW 8" THICK 10" THICK ALTERNATIVE TO J—BOLTS GAR. DOOR BLOCK DOWN AND/OR BOTTOM OF SLAB 8' - LOAD PLANE MODULE STRESS ZONE - TOE OF CUT DETAIL MAY VARY -2 X LEDGE MIN. GR40 #4 24" LAP, MIN DOWELS, TYP. DBL PLATE FOR GYP CRETE ---------DETAIL MAY VARY - DRILL & SEAL AS REQ'D FOOTING top & 24" oc max 4 #4 -#4's @ 12" OC EW 1-1/2" COVER CONCRETE SLAB - DOWELS @ 12" OC JOIST HNGR -FOOTING STRESS ZONE w/ 8 #4's EW 100 # /中' (LL) ¹ 40 # /中' (LL) **PEDESTAL** SLAB @ WALI 67 # /中'(DL) FLOOR 67 # /中'(DL) NTS DETAIL MAY VARY JOIST $w_u = 1.2(DL) + 1.6(LL)$ w = 1.2(DL) + 1.6(LL)SLAB ON FILL CONCRETE OR CMU $\frac{1}{2}$ 1—1/2" COVER MIN. = 240 # / 由' (TL) MODULE = 144 # / 武 (TL) w/ 8 #4's EW DETAIL MAY VARY SEE TABLE 1-1/2" LEDGE - 1—1/2" LEDGE MIN. – – 25,951 #-in 24" LAP MIN., TYP. - - 27,206 #-in 40,000 * 0.2 0.85 * 3,500 * 12 = 0.22" CONC STRENGTH 6" MIN. STEM WALL-CAST w/ SLAB 3" MIN COVER 2.5" INSULATION, -UNDISTURBED REQ'D STRENGTH $\phi M_N = *\phi A * f(d - a/2)$ A 3" MIN. COVER ICF WALL $\phi M_N = *\phi A * f(d - a/2)$ TRUCTURAL SLAB ON FILL 3,000 psi = 0.9(0.2)(40000)(4-0.22/2)= 0.9(0.2)(40000)(4-0.22/2)PEDESTAL PED @ FTG 3,500 psi = 28,008 # in > 25,951 (OKAY)= 28,008 # in > 27,206 (OKAY)DO NOT SAW CUT STRUCTURAL SLABS W/O APPROVAL 3,500 psi FLUSH FRAMING @ FDN ∴ Use #4 @ 12" OC EW SLAB @ PEI ∴ Use #4 @ 12" OC EW VERIFY ALL STRUCTURAL SLAB DETAILS W/ ENGINEER SUS-SLAB 7 SACK MIX 12'-6" (+/-) MODULE 15'-6" (+/-) MODULE DO NOT ISOLATE COLUMNS FROM STRUCTURAL SLABS

SLAB ON FILL

ville, Missouri 64152

P.O. Box 12089, Parkville, Mi

ISSUE DATE
REVISIONS 11/2/15

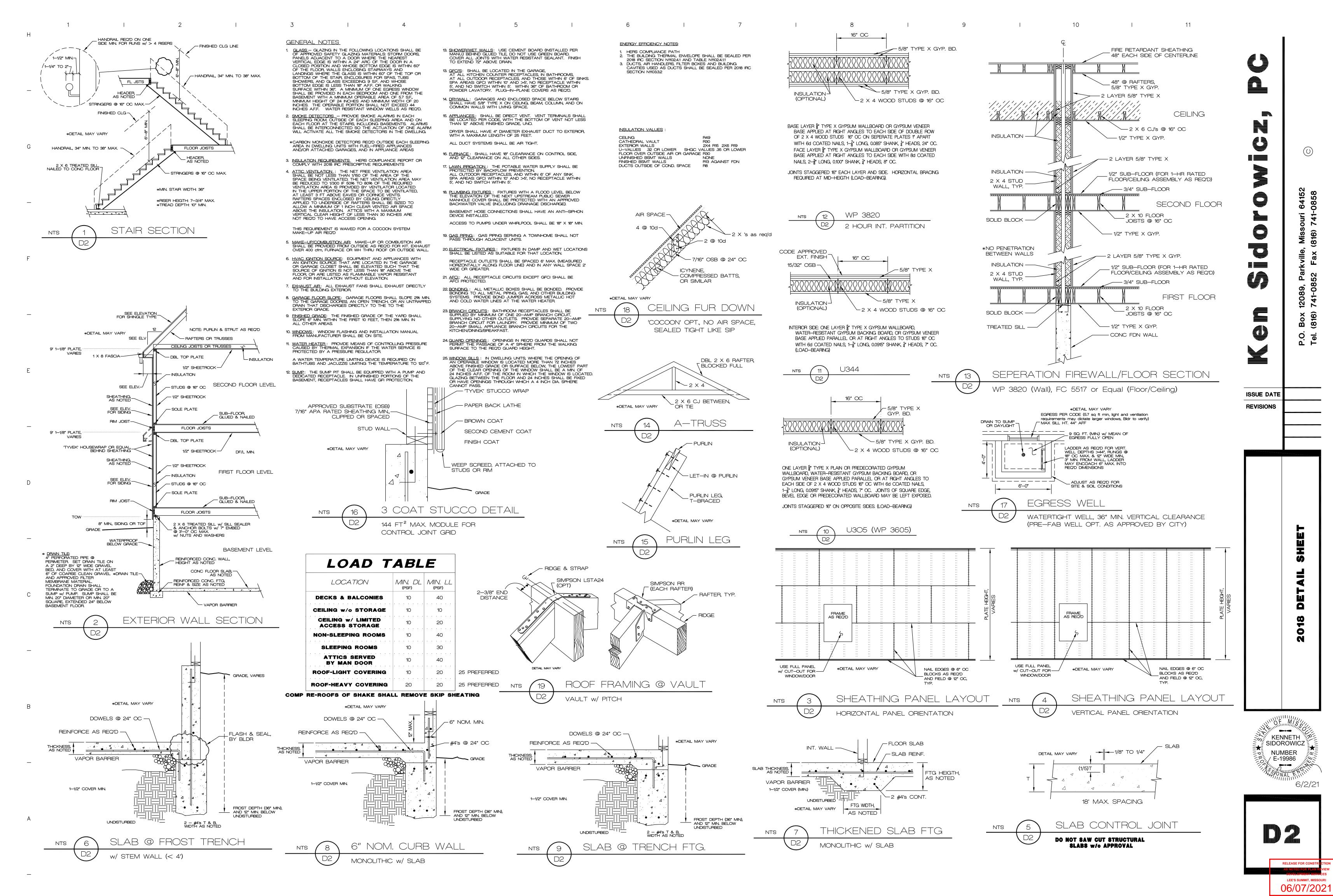
2018 DETAIL

KENNETH SIDOROWICZ NUMBER E-19986

RELEASE FOR CONSTRUCTION

-AS NOTED FOR PLAN REVIEW
DEVELOPMENT SERVICES
LEE'S SUMMIT, MISSOURI

06/07/2021

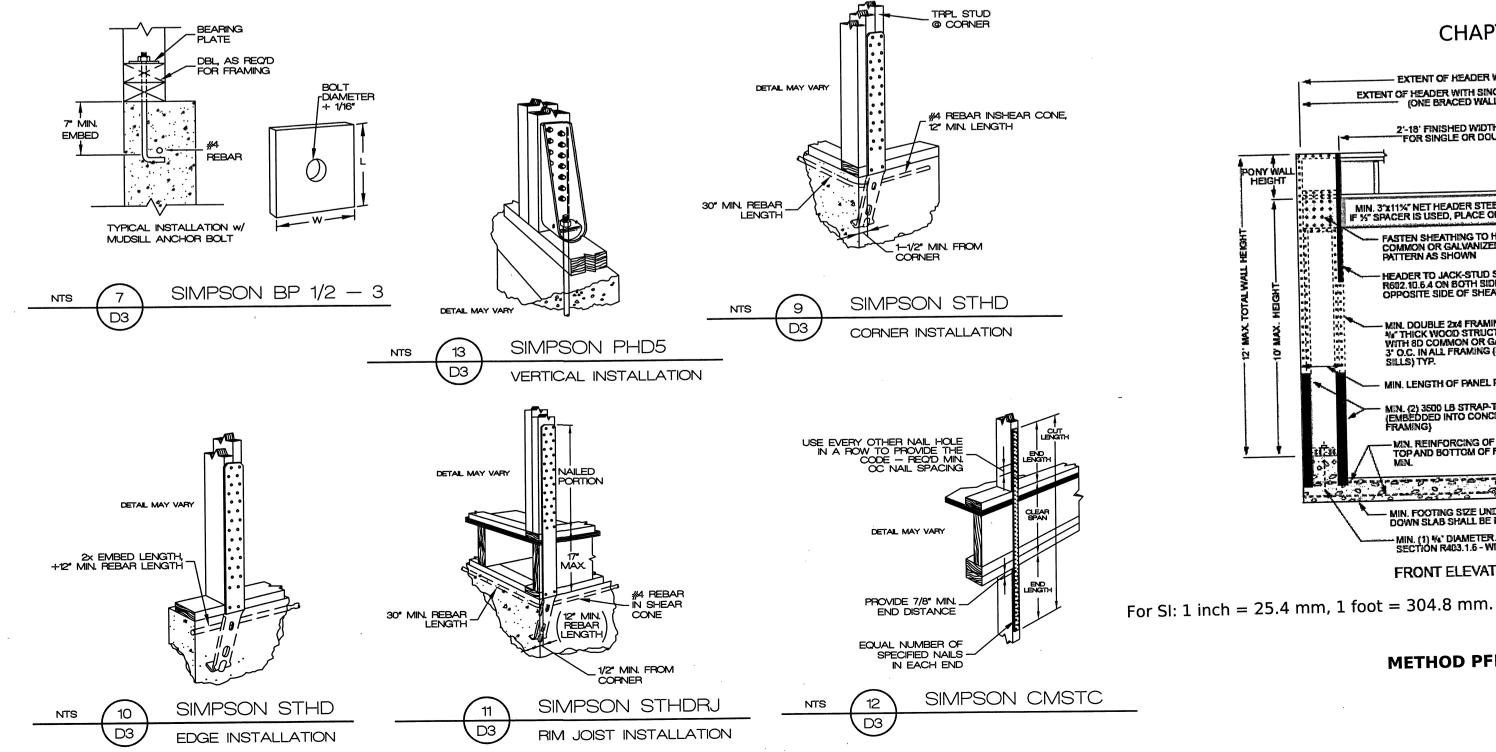


STAPLES NOT PERMITTED IN KCMO

		TENER SCHEDULE FO		
tem	Description of building el	ements	Number & type of fastener (notes: a, b, c)	Spacing of fasteners
;			L	
1	Blocking between joists or rafters to	top plate, toe nail	3-8d (2-1/2" × 0.113")	
[2]	Ceiling joists to plate, toe nail		3-8d (2-1/2" × 0.113")	
.3	Ceiling joists not attached to parallel		3-10d 3-10d (3" × 0.128")	<u>-</u>
. 4 5	Collar tie rafter, face nail or 1-1/4" x	s use STC clips at NLB walls and special holdowns	3-16d or 3-10d (3-1/2" x 0.135", 0.148")	2 toe nails side 1, 1 toe nail side 2 (note j)
. 6	Roof rafters to ridge, valley or hip ra	afters:	1	<u> </u>
	Toe nail:		4-16d (3-1/2" × 0.135")	<u> </u>
	Face nail:		3-16d (3-1/2" × 0.135")	
 	I D the second to see and		10d (3" × 0.128")	24° o.c.
.7 8	Built-up studs-face nail Abutting studs at intersecting wall o		16d (3-1/2" × 0.135")	12° o.c.
.9	Built—up header, two pieces w/ 1/2" spacer		16d (3-1/2" x 0.135")	16" o.c. along each eage
10	Continued header, two pieces		16d (3-1/2" × 0.135")	16° o.c. along each edge
.11	Continuous header to stud, toe nail		4-8d (2-1/2" × 0.113") 10d (3" × 0.128")	
.12 _. .	Double studs, face nail Double top plates, face nail		10d (3" × 0.128")	24" o.c.
13 14		end joints, face nail in lapped area	8-16d (3-1/2" × 0.135")	
.15	Sole plate to joist or blocking, face	nail	16d (3–1/2" × 0.135")	
16 17	Sole plate to joist or blocking at bra	aced wall panels	3-16d (3-1/2" × 0.135") 3-8d (2-1/2" × 0.113") or	16° a.c
17	Stud to sole plate, toe nail		2-16d (3-1/2" × 0.135")	
18	Top or sole plate to stud, end nail		2-16d (3-1/2" × 0.135")	.
19	Top plates, laps at corners and inte		2-10d (3" × 0.128")	<u>-</u>
20	1" brace to each stud and plate, fac	ce nail	2-8d (2-1/2" x 0.113") 2 staples 1-3/4"	
21	1" x 6" sheathing to each bearing, f	ace nail	2-8d (2-1/2" × 0.113")	
.21		r x 6 sneathing to each bearing, race rial		<u> </u>
22	1" x 8" sheathing to each bearing, f	ace nail	2-8d (2-1/2" × 0.113")	<u>-</u>
	NATIONAL AND	th bearing food poil	3 staples 1-3/4" 3- \$ cb(\$20 6\$2"1 ×3 0% "3")	_
23	Wider than 1" x 8" sheathing to each	on bearing, tace hall	o ecomposit (Norme)	
or	Joist to sill or girder, toe nail		3-8d (2-1/2" × 0.113")	-
24 25	Rim joist to top plate, toe nail (roof	applications also)	8d (2-1/2" × 0.113")	6° o.c.
26	26 Rim joist or blocking to sill plate, toe nail		8d (2-1/2" x 0.113")	6° o.c.
27	1" x 6" subfloor or less to each jois	t, face nail	2-8d (2-1/2" x 0.113") 2 staples 1-3/4"	_
	2" subfloor to joist of girder, blind a		2-16d (3-1/2" × 0.135")	[
28 2" subfloor to joist of girder, blind and face nail 29 2" planks (plank & beam — floor and roof)		2-16d (3-1/2" × 0.135")	@ each bearing	
30	Built-up girders and beams, 2" lum	nber layers	10d (3" x 0.128")	Nail each layer as follows: 32" o.c. at top and bottom and staggered. Two nails at ends
				and at each splice
31	Ledger strip supporting joists or raf	ters	3-16d (3-1/2" × 0.135")	@ each joist or rafter
			Spacing of	f Fasteners
		D I II of factors	Edges (inches)	Intermediate supports (inches)
	Description of building	Description of fastener	(note: i)	(notes: c, e)
	materials	(notes: b, c, e) wall sheathing to framing and particleboard wall sheath		
32 32	3/8" to 1/2"	6d common (2" × 0.113") nail (subfloor, wall) (note j)	6	12 (note: g)
		8d common (2-1/2" x 0.131") nail (roof)	1	Commence of the State of the Commence of the C
33	19/32" to 1"	8d common nail (2-1/2" x 0.131")	6 6	12 (note: g)
34	1-1/8° to 1-1/4°	10d common (3" x 0.148") nail or 8d deformed (2-1/2" x 0.131") nail	6	_
or=#	sheathing (note h)	ou delottieu (Z-1/2 X U.DI) itali		
er wall	1/2" structural cellulosic fiberboard	1-1/2" galv. roofing nail, 7/16" crown or	3	6
~	sheathing	1" crown staple 16 ga., 1-1/4" long	1	
36	25/32" structural cellulosic	1-3/4" galv. roofing nail, 7/16" crown or	3	0
37	fiberboard sheathing	1" crown staple 16 ga, 1-1/2" long 1-1/2" galvanized roofing nail; staple galv,	\ \cdot \cdo	h
37	1/2" gypsum sheathing (note d)	1-1/2" long; 1-1/4" screws, Type W or S		1
38	5/8" gypsum sheathing (note d)	1-3/4" galvanized roofing nail; staple galv.	7	7
		1-5/8" long; 1-5/8" screws, Type W or S		
	ctural panels, combination subfloor und		6	12
39	3/4" and less	6d deformed (2" x 0.120") nail or 8d common (2-1/2" x 0.131") nail		
<i>.</i>	7/8° to 1°	8d common (2-1/2" x 0.131") nail or	6	12
40	7/8 10 1	8d deformed (2-1/2" × 0.120") nail	1	
	1-1/8° to 1-1/4°	1	6	12

For St. 1 inch = 25.4 mm, 1 foot = 304.8 mm, 1 mile per hour = 0.447 m/s; 1 ksi = 6.895 MPa.

- a. All nails are smooth-common, box or deformed shanks except where otherwise stated. Nails used for framing and sheathing connections shall have minimum average bending yield strengths as shown: 80 ksi (551 MPa) for shank diameter of 0.192 inch (20d common nail), 90 ksi (620 MPa) for shank diameters larger than 0.142 inch but not larger than 0.177 inch, and 100 ksi (689 MPa) for shank diameters of 0.142 inch or less.
- Staples are 16 gage wire and have a minimum 7/16-inch on diameter crown width. c. Nails shall be spaced at not more than 6 inches on center at all supports where spans are 48 inches or greater.
- Four-foot-by-8-foot or 4-foot-by-9-foot panels shall be applied vertically. e. Spacing of fasteners not included in this table shall be verified w/ EOR.
- f. For regions having basic wind speed of 110 mph or greater, 8d deformed nails shall be used for attaching plywood and wood structural panel roof sheathing to framing within minimum 48-inch distance from gable end walls, if mean roof height is more than 25 feet, up to 35 feet maximum.
- g. For regions having basic wind speed of 100 mph or less, nails for attaching wood structural panel roof sheathing to gable endwall framing shall be spaced 6 inches on center. When basic wind speed is greater than 100 mph, nails for attaching panel roof sheathing to intermediate supports shall be spaced 6 inches on center for minimum 48-inch distance from ridges, eaves and gable end walls; and 4 inches on center to gable end wall framing. Gypsum sheathing shall conform to ASTMC 1396 and shall be installed in accordance with GA 253. Fiberboard sheathing shall conform to ASTM C 208.
- Spacing of fasteners on floor sheathing panel edges applies to panel edges supported by framing members and at all floor perimeters only. Spacing of fasteners on roof sheathing panel edges applies to panel edges supported by framing members and at all roof plane perimeters. Blocking of roof or floor sheathing panel edges perpendicular to the framing members shall not be required except at intersection of adjacent roof planes. Floor and roof perimeter shall be supported by
- Where a rafter is fastened to an adjacent parallel ceiling joist in accordance with this schedule, provide two toe nails on one side of the rafter and toe nails from the ceiling joist to top plate in accordance with this schedule. The toe nail on the opposite side of the rafter shall not be required.



NAIL TOP AND BOTTOM PLATES OF BP's TO JOISTS ABOVE AND BELOW W/ 3 @ 16d @ 16" OC ALL METHODS CONSTRUCTION DESCRIPTION SHEAR WALL SIMPSON CS16 STRAP NAILED TO STUDS SPACED METAL STRAP METHOD AT 16" OC MAXIMUM. STRAPS SHALL BE INSTALLED IN 'V' OR 'X' PATTERN AT THE BRACE LOCATION AND FOR THE SPECIFIED LENGTH, ALTERNATIVE TO *DBL JOIST MIN. BELOW BRACED WALL WHEN FRAMING BELOW IS PARALLEL TO WALL LINE, OR SOLID BLOCK @ 16" OC BELOW BRACED WALL WHEN FRAMING BELOW IS PERPENDICULAR TO WALL LINE. COLLECTOR OR DRAG STRUT OVER. 7/16" STRUCTURAL SHEATHING OVER STUDS SPACED 16" OC W/ 8d COMMON NAILS AT 6" OC EDGE AND 12" FIELD, HORIZONTAL JOINTS SHALL BE SHEATHING METHOD BLOCKED FOR ANCHORAGE. *DBL JOIST MIN. BELOW BRACED WALL WHEN FRAMING BELOW IS PARALLEL TO WALL LINE, OR SOLID BLOCK @ 16" OC BELOW BRACED WALL WHEN FRAMING BELOW IS PERPENDICULAR TO WALL LINE. 6 TO 1 ASPECT RATIO, HEADER LENGTH AS SPECIFIED WITH FULL PANEL SHEATHING AT UPPER CORNERS CUTOUT FOR THE OPENING. BLOCKING AT HORIZONTAL JOINTS. NOTE FULL 4' WIDTH CUTOUT PANELS REQ'D AT CORNERS. STHD10 & LSTA STRAPS GARAGE DOOR PORTAL PFH HEADER LENGTH AS SPECIFIED EXTENDED TO NEXT LAYOUT STUD, 18" MINIMUM WIDTH. FULL PANEL SHEATHING REQ'D WITH CUTOUTS FOR OPENINGS. PORTALS CS-PF HORIZONTAL BLOCKING AT EDGES. 'J' BOLT SPACING FOR SHEAR WALLS IS 3' OC WITH STRAPS AS NOTED.

SHEAR WALL SCHEDULE

CHAPTER 6 WALL CONSTRUCTION

FIGURE R602.10.6.2

METHOD PFH—PORTAL FRAME WITH HOLD-DOWNS

TYPICAL PORTAL FRAME CONSTRUCTION -

SECTION

--- EXTENT OF HEADER WITH DOUBLE PORTAL FRAMES (TWO BRACED WALL PANELS)---------

EXTENT OF HEADER WITH SINGLE PORTAL FRAME
(ONE BRACED WALL PANEL)

MIN. 3"x11%" NET HEADER STEEL HEADER PROHIBITED IF %" SPACER IS USED, PLACE ON BACK-SIDE OF HEADEL

FASTEN SHEATHING TO HEADER WITH 8D COMMON OR GALVANIZED BOX NAILS IN 3" GRED PATTERN AS SHOWN

HEADER TO JACK-STUD STRAP PER TABLE —— R602, 10,64 ON BOTH SIDES OF OPENING OPPOSITE SIDE OF SHEATHING

MIN. DOUBLE 2X4 FRAMING COVERED WITH MIN.
NOT THICK WOOD STRUCTURAL PANEL SHEATHING
WITH 8D COMMON OR GALVANIZED BOX NAILS AT
3" O.C. IN ALL FRAMING (STUDS, BLOCKING, AND

IIN. LENGTH OF PANEL PER TABLE RE02.10.5

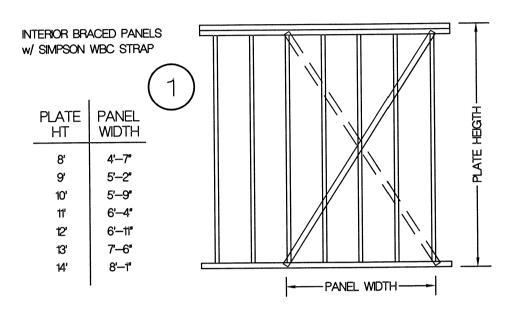
TOP AND BOTTOM OF FOOTING, LAP BARS 15"

Alcon has hoon has harman has had A service of the serv — MIN. FOOTING SIZE UNDER OPENING IS 12"x12". A TURNED DOWN SLAB SHALL BE PERMITTED AT DOOR OPENINGS.

FRONT ELEVATION

--- MIN. (1) % DIAMETER ANCHOR BOLT INSTALLED PER SECTION R403.1.5 - WITH 2"X 2" X*16" PLATE WASHER

LOAD TABLE LOCATION MIN. DL | MIN. LL (PSF) (PSF) 60 10 **EXTERIOR BALCONIES** 10 40 **DECKS** 10 CEILING w/o STORAGE 10 20 CEILING W/ STORAGE 40 10 NON-SLEEPING ROOMS SLEEPING ROOMS 30 10 10 25 ROOF-LIGHT COVERING 20 25 ROOF-HEAVY COVERING

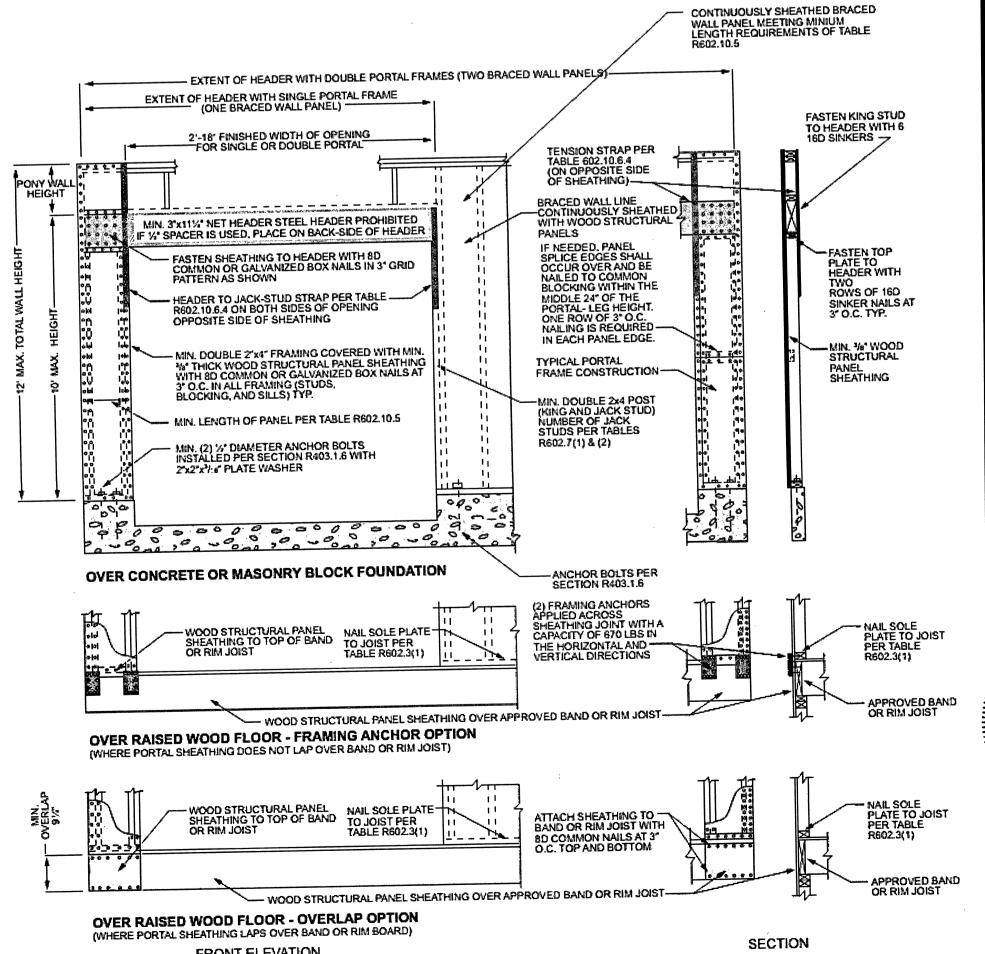


INT. BRACED WALL PANEL LIB, METAL STRAP ALT. TO LET IN 1 X 4

> 2018 International Residential Code Third Printing: Sep 2019

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CHAPTER 6 WALL CONSTRUCTION



For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm.

DIGITAL CODES

FIGURE R602.10.6.4 METHOD CS-PF—CONTINUOUSLY SHEATHED PORTAL FRAME PANEL CONSTRUCTION

REVISIONS TE OF MISS ---KENNETH SIDOROWICZ NUMBER

0

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ISSUE DATE

FRONT ELEVATION

AS NOTED FOR PLAN REVIEW DEVELOPMENT SERVICES LEE'S SUMMIT, MISSOURI 06/07/2021