

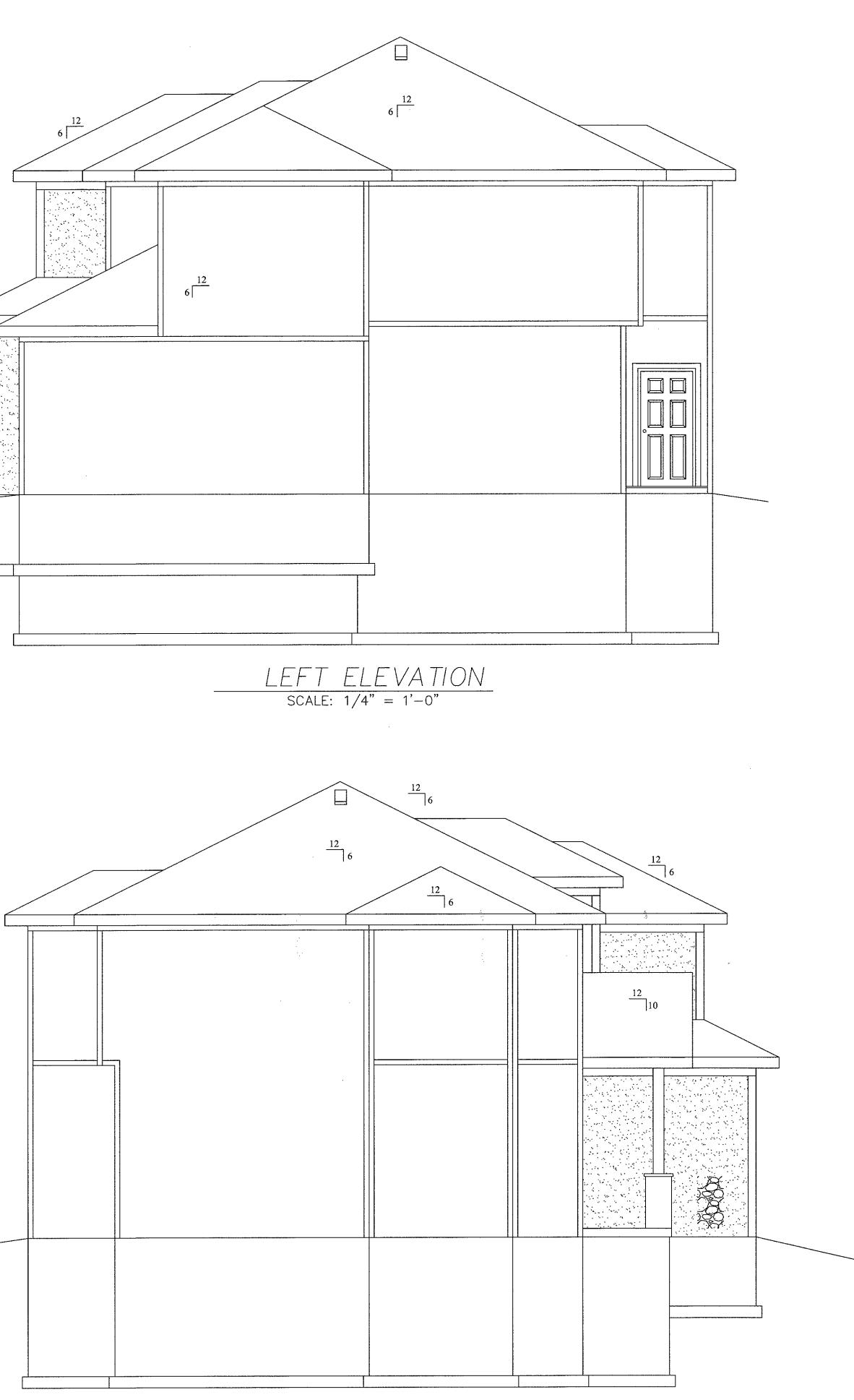
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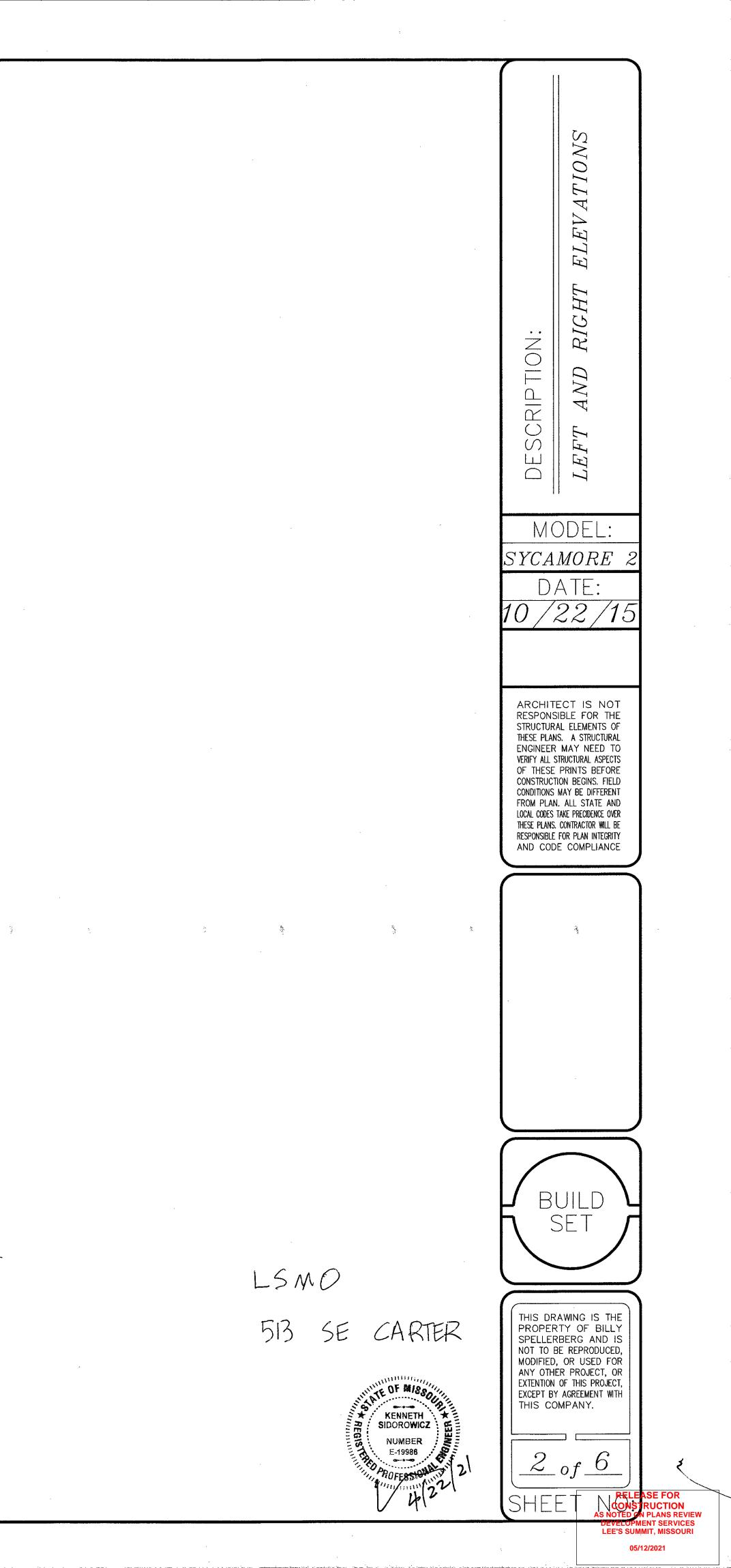
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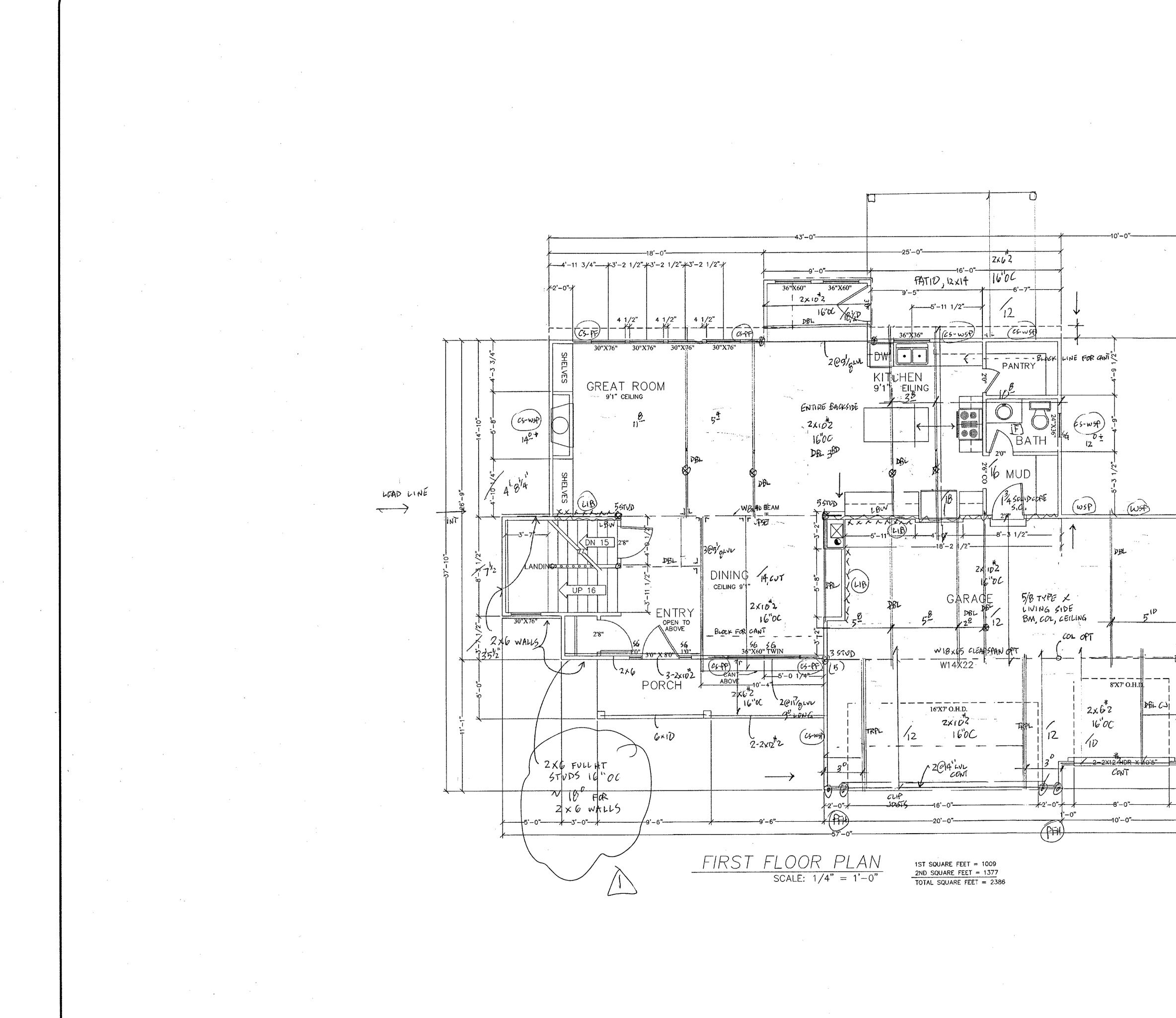
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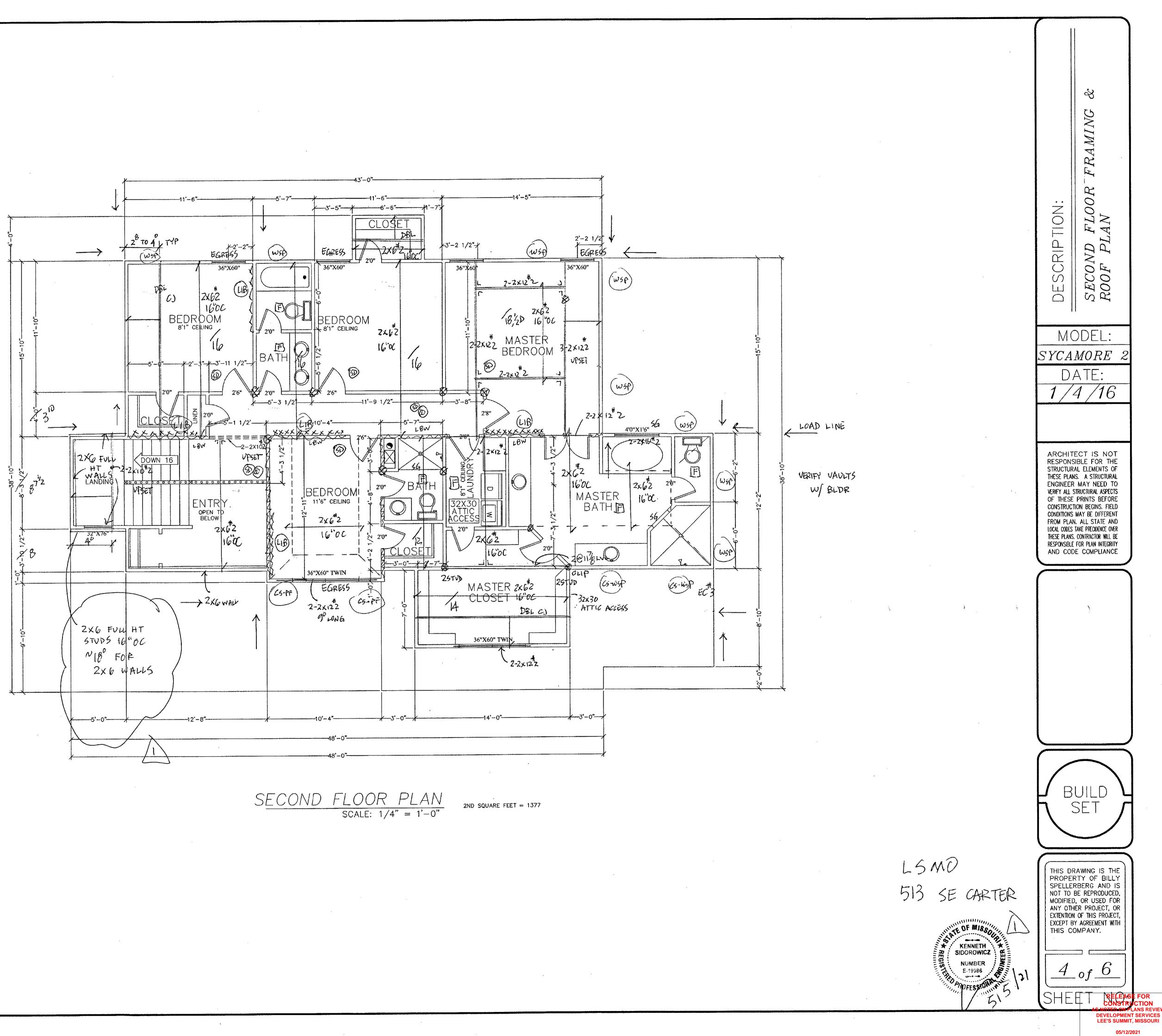
 $\frac{RIGHT ELEVATION}{SCALE: 1/4" = 1'-0"}$





FRAMING 2 @ A35, 1 each Hdr.----FLOORTION: Visible nations for inspection SCRIP. F DF/L MIN FIRS CS-WSP HOUSE IS SHEATHED W/ 78" OSB APA PANELS, SMART PANEL OR EQUAL, INSTALLED PER MANU. SPECS, SHIP LAPPED PANELS REQUIRE NAILING OF OVER AND UNDER PANELS SEPARATELY. \square INT SHALL BE SIMPSON STRAP (CS16) LIB HEADER LENGTHS ARE SHOWN FOR CS-PF CS-PF MODEL: SYCAMORE SIDING LAPS RIM 2x4, 9' PLATE, FULL HT. STUDS DATE: S.C. = SELF CLOSING D2 GN #25 FOR WINDOWS /16 4 CS = CONTINUOUSLY SHEATHED EC = END CONDITION SEE D2 FOR INSULATION VALUES EC#5, 16" LONG CS16 STRAP, CENTERED ON SUBFLOOR, FILL ALL NAIL HOLES. ARCHITECT IS NOT RESPONSIBLE FOR THE STRUCTURAL ELEMENTS OF THESE PLANS. A STRUCTURAL ENGINEER MAY NEED TO VERIFY ALL STRUCTURAL ASPECTS OF THESE PRINTS BEFORE CONSTRUCTION BEGINS. FIELD LOAD LINE _____ CONDITIONS MAY BE DIFFERENT FROM PLAN. ALL STATE AND LOCAL CODES TAKE PRECIDENCE OVER THESE PLANS, CONTRACTOR WILL BE RESPONSIBLE FOR PLAN INTEGRITY AND CODE COMPLIANCE . BUILD SET LSMO THIS DRAWING IS THE PROPERTY OF BILLY SPELLERBERG AND IS NOT TO BE REPRODUCED, MODIFIED, OR USED FOR ANY OTHER PROJECT, OR EXTENTION OF THIS PROJECT, EXCEPT BY AGREEMENT WITH THIS COMPANY. 513 SE CARTER KENNETH SIDOROWICZ NUMBER E-19986 w-1-40 **(**) 1EET RELEAS IS REVIEV DEVELOPMENT SERVICES LEE'S SUMMIT, MISSOURI 05/12/2021





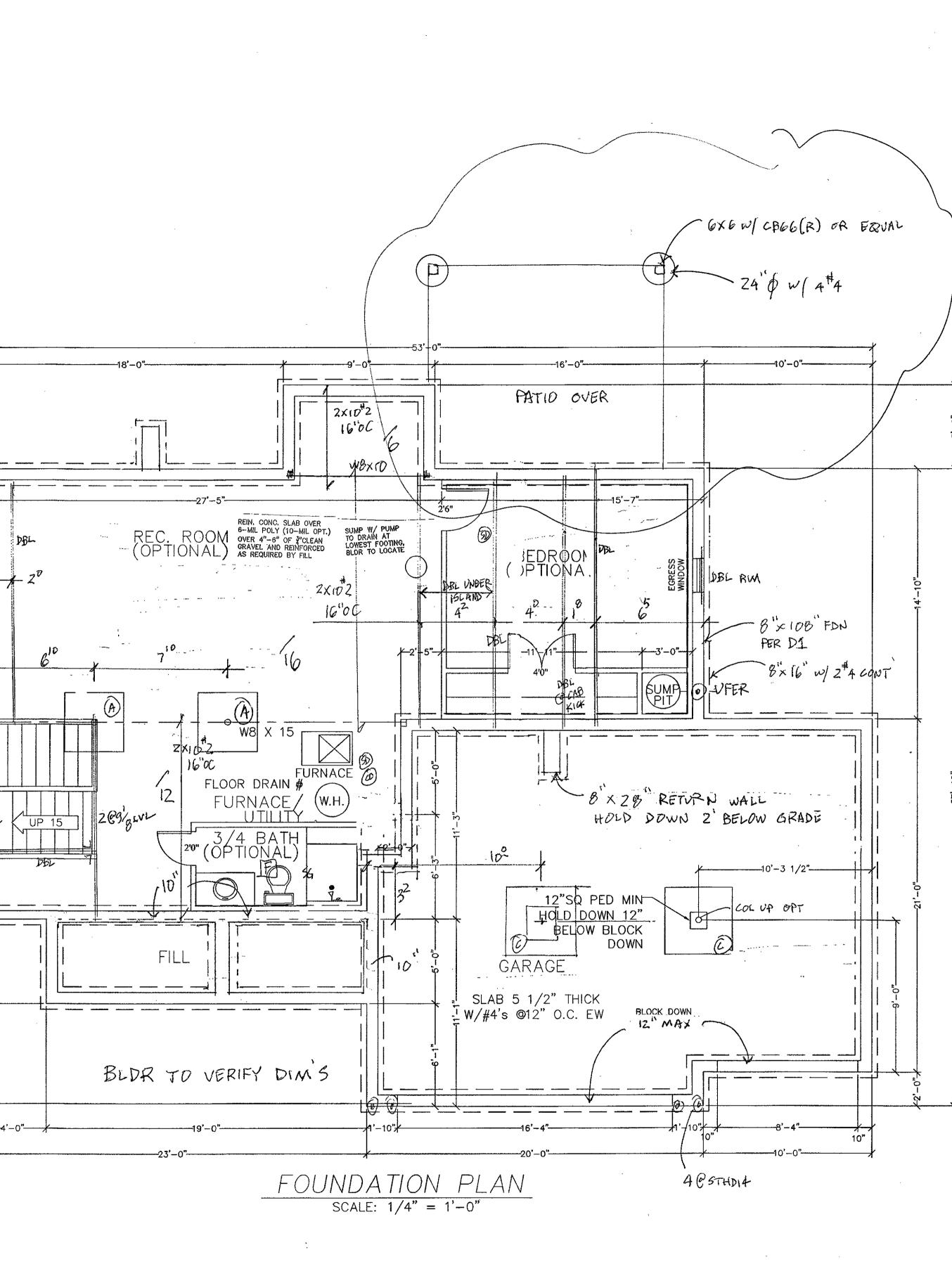
BEAMSFIELD VERIFY LENGTHLENGTHSIZE25'11"W8 X 1530'0"W14X22

3 POSTS ADJUSTIBLE

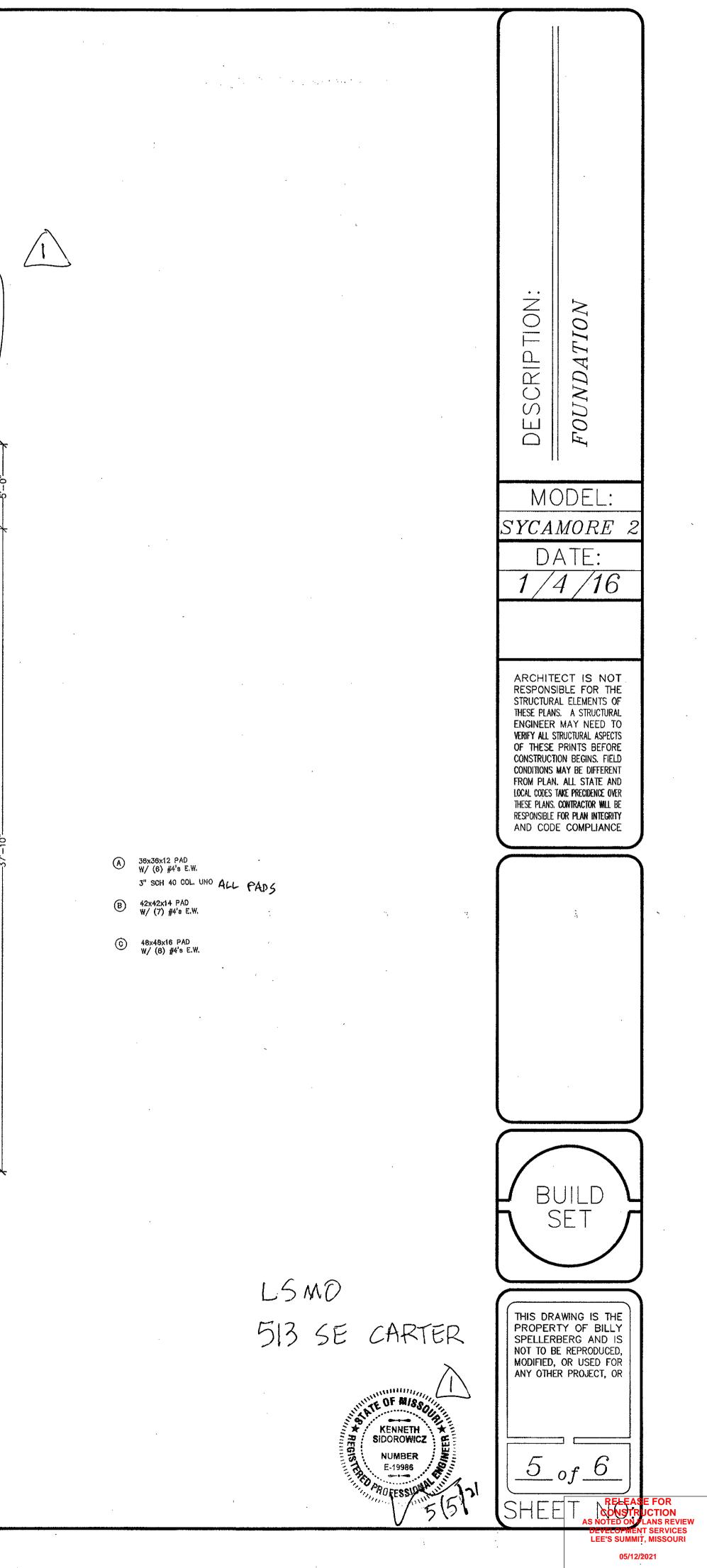


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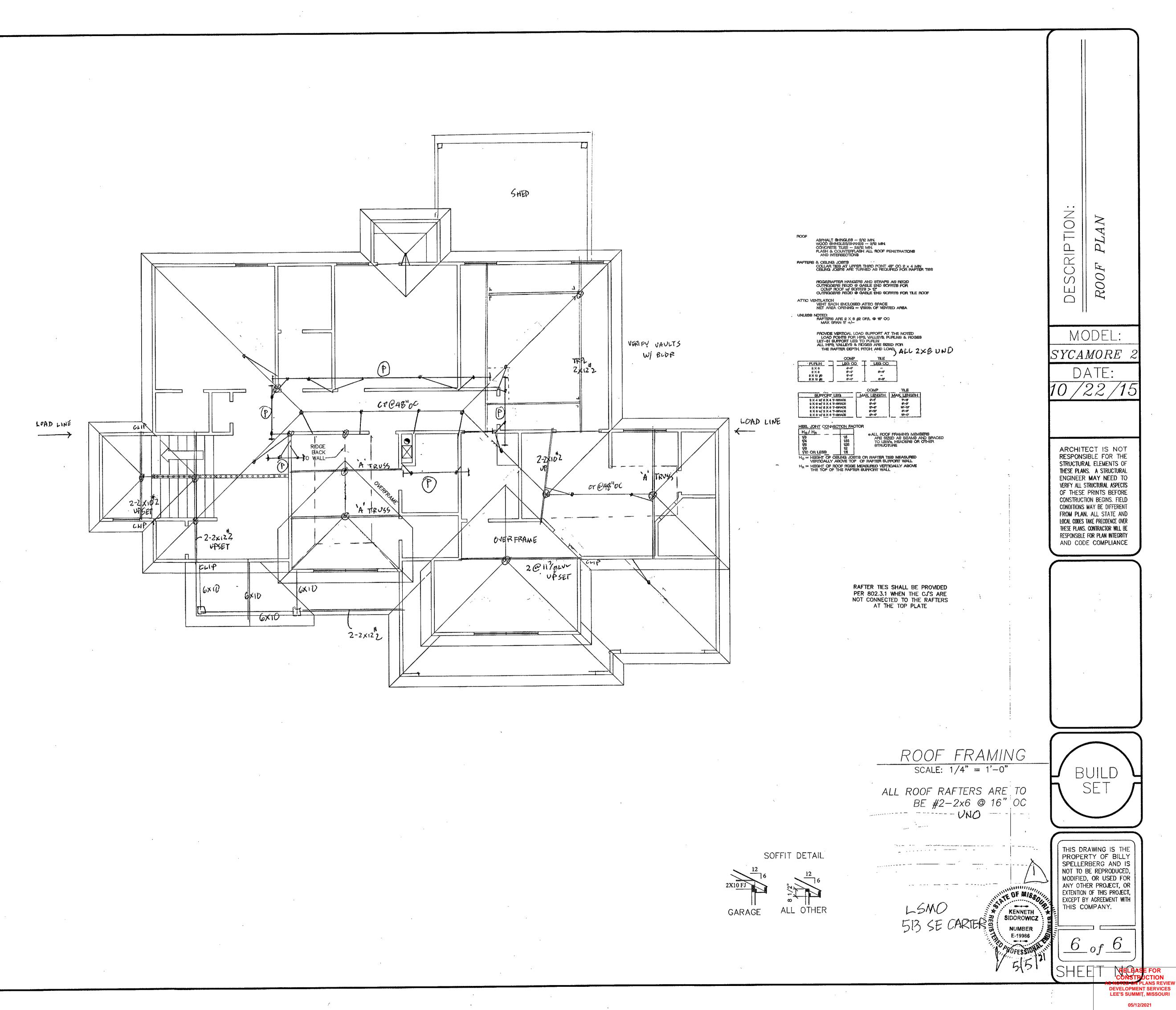
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- <u>DIVISION 1</u> GENERAL REQUIREMENTS
- 1. DESIGN AND CONSTRUCTION WORK FOR THIS PROJECT SHALL
- CONFORM TO THE REQUIREMENTS OF THE 2018 IRC.
- 2. FURNISH ALL LABOR, MATERIALS, AND EQUIPMENT NECESSARY TO COMPLETE THE WORK AS SHOWN OR INFERRED BY THE DRAWINGS.

2

- 3. DESIGN FACTORS: A) GROUND SNOW LOAD (INCLUDING DRIFTING SNOW)__20 PSF B) WIND SPEED (EXPOSURE B)_ 115 MPH C) SEISMIC CATEGORY (A), GROUND ACCELERATION = NA
- 4. DESIGN LOADS (PSF, UNLESS NOTED OTHERWISE): A) ROOF (LL/DL) SEE TABLE B) FLOOR (LL/DL)_ _SEE TABLE _SEE TABLE, (0/10 TRUSSES) C) CEILING (LL/DL)_
- 5. DO NOT SCALE DRAWINGS, IF DIMENSIONS ARE IN QUESTION, OBTAIN CLARIFICATION FROM A / E BEFORE CONTINUING
- 6. THE CONTRACTOR SHALL EXAMINE ACTUAL JOB CONDITIONS AND BE RESPONSIBLE FOR VERIFYING ALL DIMENSIONS AND ELEVATIONS SHOWN ON THE PLANS. IF ERRORS, OMISSIONS, OR DISCREPANCIES ARE FOUND THEY SHALL BE REPORTED TO THE DESIGN PROFESSIONAL BEFORE PROCEEDING WITH THE WORK.
- 7. DIMENSIONS FOR NEW CONSTRUCTION ARE TO FACE OF FINISH OR COLUMNS AND FACE OF CONCRETE, WOOD, OR MASONRY WALLS UNLESS OTHERWISE INDICATED. DIMENSIONS INDICATE NOMINAL DIMENSIONS RATHER THAN ACTUAL DIMENSIONS.
- 8. CONTRACTOR IS RESPONSIBLE FOR COORDINATING ALL TRADES EVEN IF THE TRADE IS UNDER A SEPARATE CONTRACT.
- 9. PROVIDE SUFFICIENT STUDS AND BLOCKING WHERE REQUIRED TO SUPPORT EQUIPMENT AND/OR MISCELLANEOUS ITEMS, I.E., LOAD POINTS, TYPICAL CASEWORK, CABINETS, GRAB BARS ETC.
- 10. PRETREAT FOUNDATION FOR TERMITES AS REQUIRED.
- 11. GARAGE DOORS AND FRAMES SHALL BE DESIGNED AND INSTALLED TO MEET THE 115 MPH WIND LOAD RESISTANCE REQUIREMENTS OF DASMA 108 AND ASTM E 330.
- 12. ALL EXTERIOR DOORS, INCLUDING THE DOOR LEADING FROM THE GARAGE TO THE DWELLING UNIT, SHALL INCORPORATE THE PHYSICAL SECURITY PROVISIONS OF THE JURISDICTION IN WHICH THE CONSTRUCTION TAKES PLACE.

<u>DIVISION 2</u> – EARTHWORK

CONSTRUCTION.

- 1. ALL PROPERTY MARKERS SHALL BE EXPOSED.
- 2. ALL FOOTINGS ARE DESIGNED TO BEAR ON NATURAL UNDISTURBED SOIL CAPABLE OF ADEQUATELY SUSTAINING A MINIMUM BEARING PRESSURE OF 1,500 PSF. IF SUITABLE UNDISTURBED BEARING CAPACITY IS NOT ENCOUNTERED AT THE ELEVATION INDICATED ON THE DRAWINGS, CONTRACTOR SHALL NOTIFY THE ENGINEER OF RECORD IMMEDIATELY.
- 3. ALL TOPSOIL, ORGANIC MATERIAL, AND EXISTING STRUCTURES SHALL BE REMOVED FROM BUILDING AREA AND FROM AREAS TO BE PAVED. STOCKPILE ALL TOPSOIL FOR REUSE.
- 4. REFERENCE THE SOILS REPORT FOR ALL FILL CONDITIONS.
- 5. OVEREXCAVATE BUILDING AREA BELOW SLAB SUBGRADE ELEVATION AND REPLACE WITH MATERIAL PER SOILS REPORT, VERIEY
- 6. SITE EROSION CONTROL SHALL COMPLY WITH ALL STATE AND LOCAL ORDINANCES.
- 7. IN-SITU SOIL CONDITIONS, SEE SOILS REPORT OR 1,500 PSF BEARING & 60 PCF EQUIVALENT FLUID WEIGHT.

- 48" LONG MIN.

- 8. SOIL CONDITIONS AT THE DEPTH OF EXCAVATION FOR THE FOOTING SHALL BE UNIFORM AND CONSISTENT. NOTIFY THE ENGINEER OF RECORD OF ANY INCONSISTENCIES.
- 9. CONTRACTOR SHALL BE RESPONSIBLE FOR REMOVING AND DISPOSING OF ANY EXCESS EXCAVATION MATERIALS AND FOR OBTAINING AND SUPPLYING ADDITIONAL FILL MATERIAL AS REQUIRED.

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 $\underline{\text{DIVISION 3}}$ – CONCRETE

3

1. ALL CONCRETE WORK SHALL CONFORM TO THE REQUIREMENTS OF ACI 318 "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE" AND ACI 332 "REQUIREMENTS FOR RESIDENTIAL CONCRETE CONSTRUCTION."

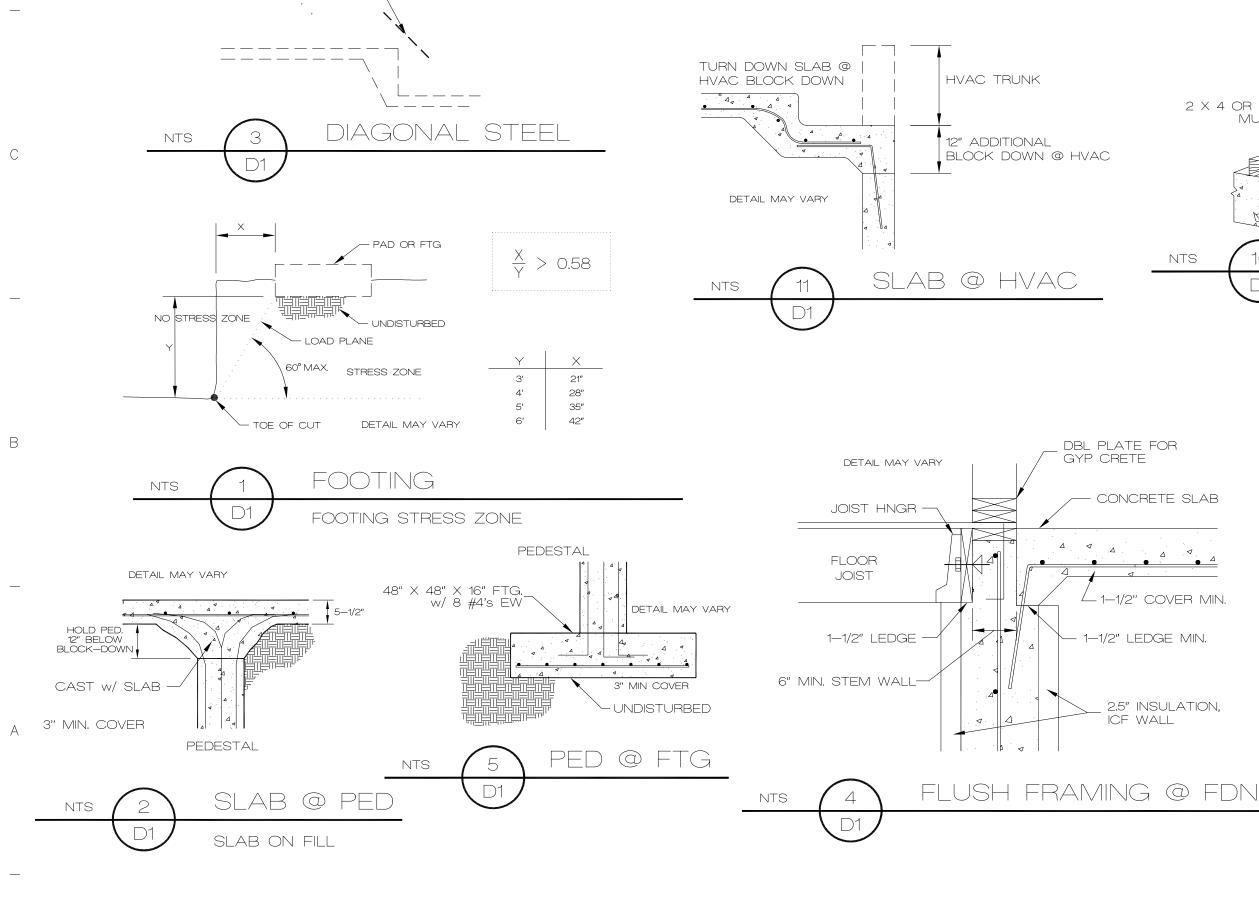
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- CONCRETE MATERIALS SHALL COMPLY WITH:
- A) CEMENT ASTM C 150 TYPE B) AGGREGATE – ASTM C 33, MAXIMUM AGGREGATE SIZE $\frac{3}{4}$ "

- C) WATER POTABLE, WATER/CEMENT RATIO .5 (MAX.)
- D) AIR-ENTRAINING ADMIXTURE ASTM C 260 E) WATER-REDUCING ADMIXTURE - ASTM C 494, INCLUDING
- SUPERPLASTICIZERS.
- F) FLY ASH ASTM C 618, CLASS C
- 3. CONCRETE SHALL DEVELOP THE FOLLOWING MINIMUM 28 DAY DESIGN COMPRESSIVE STRENGTH (f'c): <u>COMP. STRENGTH (f'c)</u> TYPE OF CONSTRUCTION
- A) FOOTINGS, WALLS, AND SLABS SEE TABLE B) EXTERIOR SLABS AND CURBS SEE TABLE (AIR-ENTRAINED CONCRETE)
- CONCRETE PROPORTIONS SHALL BE ESTABLISHED ON THE BASIS OF FIELD EXPERIENCE AND/OR TRIAL MIXTURES IN ACCORDANCE WITH ACI 318-89 SECTIONS 5.2 AND 5.3. WHEN FLY ASH IS UTILIZED IN THE MIX, MIX SHALL CONTAIN A WATER-REDUCER. FLY ASH SHALL BE ADDED AT THE RATE OF NOT MORE THAN 100 POUNDS PER CUBIC YARD AND CEMENT SHALL BE REDUCED BY NOT MORE THAN 15 PERCENT BY WEIGHT.
- 4. PROPORTION AND DESIGN MIXES TO RESULT IN CONCRETE SLUMP AT A POINT OF PLACEMENT OF NOT MORE THAN 4" TO 5".
- 5. USE AIR-ENTRAINING ADMIXTURES IN EXTERIOR EXPOSED CONCRETE TO RESULT IN CONCRETE AT POINT OF PLACEMENT HAVING AIR CONTENT OF 5 TO 7 PERCENT ENTRAINED AIR.
- 6. ALL PLUMBING AND ELECTRICAL ROUGH-INS MUST BE COMPLETE, INSPECTED AND APPROVED BEFORE REQUESTING THE SLAB INSPECTION.
- 7. CONCRETE WORK EXECUTION: A) MINIMUM CONCRETE COVER FOR REINFORCING SHALL BE, UNLESS NOTED OTHERWISE ON DRAWINGS: CAST AGAINST AND EXPOSED TO EARTH_
- EXPOSED TO EARTH OR WEATHER_ NOT EXPOSED TO EARTH OR WEATHER B) IN CORNERS OF GRADE BEAMS PROVIDE CORNER REINFORCEMENT
- LAP TWO FEET EACH DIRECTION IN OUTSIDE FACE, MATCHING SIZE AND SPACING OF HORIZONTAL REINFORCEMENT. C) PROVIDE CONTROL JOINTS IN SLABS-ON-GRADE AT NOT GREATER THAN 20 FEET ON CENTER IN EACH DIRECTION. SAW CUT CONTROL JOINTS MINIMUM $\frac{1}{4}$ OF THE SLAB DEPTH, AS SOON AFTER SLAB FINISHING AS POSSIBLE WITHOUT DISLODGING
- AGGREGATE. (DO NOT SAW CUT STRUCTURAL SLABS w/o APPROVAL).
- 8. BATCH TICKETS SHALL BE SUBMITTED TO A CONTRACTORS REPRESENTATIVE PRIOR TO OFF LOADING. ANY CONCRETE MORE THAN 45 MINUTES OUT PRIOR TO STARTING PLACEMENT SHALL BE REJECTED.
- 9. THE MAXIMUM ADDITION OF WATER SHALL BE LIMITED TO 1 GALLON PER YARD; NOTE THAT THIS ADDITION SHALL BE USED TO CONTROL HEAT ONLY (NOT SLUMP).
- 10. PUMPS SHALL NOT BE PRIMED IN FORMS.
- 11. REINFORCEMENT

DAMAGE

- A) ALL REINFORCING BARS SHALL BE A615, GR40 MIN. LAP SPLICES 18" MIN FOR #4 BAR. SEE TABLE B) WELDED WIRE FABRIC SHALL BE ASTM A185, LAP AT LEAST ONE
- FULL MESH AND LACE SPLICES WITH WIRE. C) REBAR SHALL BE CLEAN, AND FREE FROM RUST AND OIL PRIOR TO THE PLACEMENT OF CONCRETE. REBAR SHALL BE TIED AND
- SECURED AS REQUIRED TO PREVENT DISPLACEMENT IN THE FORMS. D) TIE STEEL TO PREVENT DISPLACEMENT. HOOK AND TIE STEEL AS POSSIBLE. TIES, CHAIRS, OR OTHER PRODUCTS SHALL BE
- PROTECTED WHEN LOCATED NEAR EXPOSED SURFACES E) STEEL SHALL BE STORED ON SITE ABOVE GRADE, AND COVERED AS REQUIRED FOR PROTECTION FROM RAIN AND OTHER POSSIBLE
- 12. ADJUST FOUNDATION FOR SITE AND SOIL CONDITIONS AND VERIFY WITH EOF



DIVISION 4 - MASONRY

5

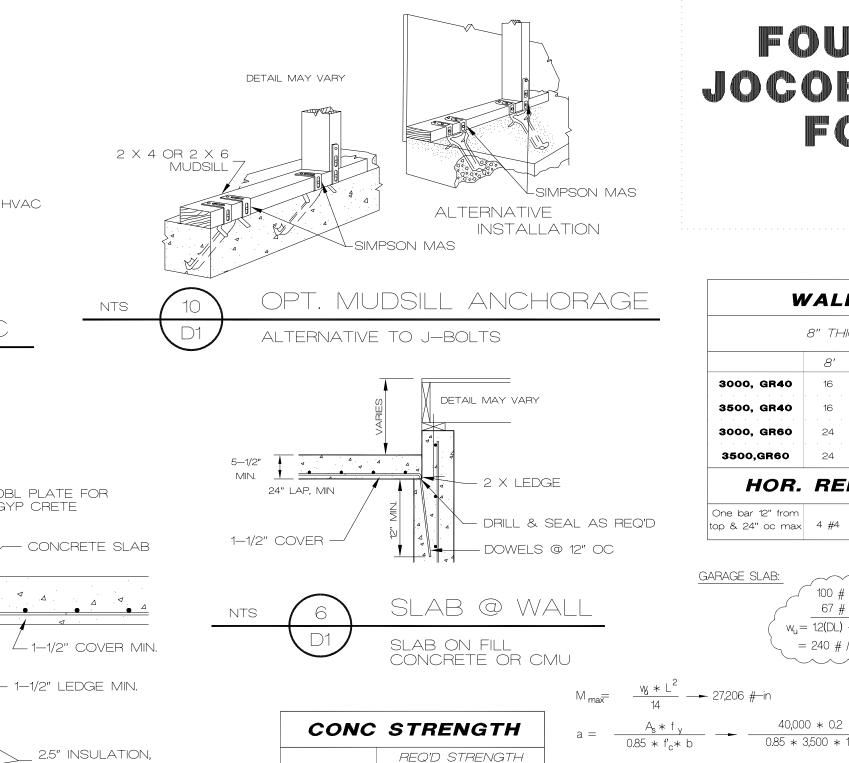
1. COMPRESSIVE STRENGTH OF CONCRETE MASONRY CONSTRUCTION (CMU) SHALL BE AS FOLLOWS (PSI). MASONRY STRENGTH NOT SPECIFICALLY NOTED ON PLAN SHALL BE (f'm) 1500 PSI.

6

MASONRY STRENGTH (F'm DESIGN)	1500
BLOCK STRENGTH	
MORTAR STRENGTH	1800
GROUT STRENGTH	2000

- 2. CONCRETE BLOCK SHALL BE HOLLOW LOAD-BEARING CONCRETE MASONRY UNITS CONFORMING TO ASTM C 90, TYPE N-11. ALL BLOCKS SHALL BE PLACED IN RUNNING BOND CONSTRUCTION (UNLESS OTHERWISE NOTED) WITH ALL VERTICAL CELLS IN ALIGNMENT.
- 3. MORTAR MIX SHALL CONFORM TO THE REQUIREMENTS OF ASTM C 270, TYPE M OR S. TYPE M MORTAR SHALL BE USED WHERE MASONRY IS IN CONTACT WITH SOIL.
- 4. GROUT SHALL CONFORM TO THE REQUIREMENTS OF ASTM C 476. USE SUFFICIENT WATER FOR GROUT TO FLOW INTO ALL JOINTS OF THE MASONRY WITHOUT SEGREGATION. ALL CELLS IN CONCRETE BLOCKS CONTAINING REINFORCING SHALL BE FILLED SOLID WITH GROUT. ALL MASONRY BELOW FINISHED FLOOR OR GRADE SHALL BE GROUTED SOLID. HOLD GROUT DOWN 1-3" BELOW TOP OF BLOCK AT GROUT LIFT JOINTS AND AT CONCRETE PLACED OVER MASONRY.
- 5. MINIMUM LINTEL, WHERE NOT ON PLANS, SHALL HAVE A MINIMUM OF 2 - #5'S CONTINOUS HORIZONTAL BARS IN BOTTOM OF BOND BEAM OR LINTEL BLOCK AND SHALL BE GROUTED SOLID TO A MIN. DEPTH OF 24." ALL LINTEL REINFORCING AND GROUT SHALL EXTEND 2' MINIMUM PAST JAMBS UNLESS NOTED OTHERWISE ON PLANS OR DETAILS.
- 6. LAP REINFORCING 48 BAR DIAMETERS. STAGGER LAP SPLICES A MINIMUM OF ONE LAP LENGTH.
- 7. MASONRY VENEER SHALL BE ATTACHED TO SUPPORT WALL FRAMING WITH $\frac{3}{6}$ " DIAMETER WALL TIES OR DOVETAIL-TYPE METAL TIES OF EQUIVALENT STIFFNESS EMBEDDED INTO HORIZONTAL MORTAR JOINTS. MAXIMUM VERTICAL SPACING OF TIES SHALL BE 16." MAXIMUM HORIZONTAL SPACING SHALL BE 24." TIES IN ALTERNATE COURSES SHALL BE STAGGERED. PROVIDE #9 WIRE REINFORCING IN HORIZONTAL MORTAR JOINTS AT 16" OC. ENGAGE #9 WIRE WITH WALL ANCHOR TIES. CONSTRUCTION JOINTS IN MASONRY VENEER WALLS SHALL BE LOCATED PER THE DRAWINGS.
- 8. WATERPROOFING, DRAINAGE PLANE, AND INSTALLATION PER ADOPTED BUILDING CODE.
- <u>DIVISION 5.5</u> MISC. STRUCTURAL STEEL
- 1. ALL MISCELLANEOUS STRUCTURAL STEEL WORK SHALL CONFORM TO THE REQUIREMENTS OF AISC "SPECIFICATIONS FOR DESIGN, FABRICATION, AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS.
- MISCELLANEOUS STRUCTURAL STEEL MATERIAL SHALL COMPLY WITH A) STRUCTURAL STEEL – ASTM A992
- B) STEEL PIPE COLUMNS ASTM A53 GRADE B(Sch 40 TYP) C) ANCHOR BOLTS - ASTM A307 GRADE A, NON-HEADED TYPE UNLESS OTHERWISE NOTED.
- 3. FLITCH PLATES SHALL HAVE $\frac{1}{2}$ " DIA. BOLTS @ 16" OC, STAGGERED OP AND BOTTOM BETWEEN DOIST LAYOUT

WALL HT.	RETURN SPACING		
ABOVE FLOOR	(HOLD DOWN 24" BELOW GRADE)		
LESS THAN 4'	RETURN WALLS NOT REQ'D		
>4' TO 9'	16'—4" ON CENTER (MAX.), AND WITHIN		
74 10 3	8' OF STEP DOWN OR AS SHOWN		



3,000 psi

3,500 psi

3,500 psi

7 SACK MIX

FTG

WALL

SLAB

SUS-SLAB

1. ALL ROUGH CARPENTRY WORK SHALL CONFORM TO THE

7

- CONSTRUCTION", TPI "DESIGN SPECIFICATIONS FOR LIGHT METAL PLATE CONNECTED WOOD TRUSSES", APA "PLYWOOD DESIGN SPECIFICATIONS", DOC PS 1 "PRODUCT STANDARD FOR CONSTRUCTION AND INDUSTRIAL PLYWOOD", DOC PS 56 "STRUCTURAL GLUED LAMINATED TIMBER", AND APPLICABLE SECTIONS OF THE INTERNATIONAL BUILDING CODE.
- 2. ROUGH CARPENTRY MATERIALS SHALL COMPLY WITH: A) LUMBER – S4S, S–DRY, KD, OR S–GRN GRADE MARKED, COMPLYING WITH PS 20, GRADED UNDER WWPA OR SPIB RULES:
 - STUDS: STUD GRADE HEADER:
 - RAFTER: #2 DOUGLAS FIR
 - PLATES: BLOCKING: #2 DOUGLAS FIR
 - B) METAL FRAMING FASTENERS ASTM A 153, HOT-DIP
 - CONNECTORS COMPLYING WITH APPLICABLE ICC-ES REPORTS C) PLYWOOD - APA RATED SHEATHING, COMPLYING TO PS 1.
 - D) LVL LAMINATED VENEER LUMBER SHALL BE GRADE 2800 F-2.0E AND SHALL MEET THE REQUIREMENTS OF APPLICABLE ICC-ES REPORTS.
 - E) GLULAM BEAMS COMBINATION 24F-V3 IN ACCORDANCE WITH AITC A190.1
- 3. EXTERIOR WALL AND ROOF SHEATHING SHALL BE $\frac{7}{16}$ " APA RATED SHEATHING 24/0 EXTERIOR GLUED (MIN) FOR 16" OC STUD SPACING. NAIL SHEATHING TO SUPPORT MEMBERS WITH 8D COMMON NAILS AT 6" ON CENTER ALONG EDGE SUPPORTS AND 12" ON CENTER ALONG FIELD SUPPORTS UNLESS NOTED OTHERWIDE. PROVIDE SOLID BLOCKING AT ALL UNSUPPORTED PANEL EDGES; 4/8 GUN NAILS.
- NOTE: ROOF SHEATHING SHALL BE §" APA RATED SHEATHING FOR TILE ROOF, OR AS REQUIRED BY MANUFACTURER.
- 4. INTERIOR SHEAR WALL SHEATHING WHERE NOTED SHALL BE $\frac{7}{16}$ " APA RATED SHEATHING 24/0 EXTERIOR GLUED (MIN) FOR 16" OC STUD SPACING. NAIL SHEATHING TO SUPPORT MEMBERS WITH 8D COMMON NAILS AT 4" ON CENTER ALONG EDGE SUPPORTS AND 6" ON CENTER ALONG FIELD SUPPORTS UNLESS NOTED OTHERWISE. PROVIDE SOLID BLOCKING AT ALL UNSUPPORTED PANEL EDGES.
- 5. ATTACH METAL FRAMING FASTENERS TO FRAMING MEMBERS WITH MINIMUM NUMBER AND SIZE OF NAILS LISTED IN THE APPLICABLE ICC-ES REPORTS.
- 6. WOOD TRUSS SYSTEM; TRUSS JOIST SYSTEM AND GLULAM SYSTEM FOR ROOFS: A) DESIGN, FABRICATE, AND ERECT IN ACCORDANCE WITH BCSI
 - B) DESIGN LOADS
 - 25 PSF SNOW LIVE LOAD 10 PSF DEAD LOAD TOP CHORD (20 TILE)
 - 10 PSF DEAD LOAD BOTTOM CHORD C) SUBMIT SHOP DRAWINGS, INCLUDING DESIGN CALCULATIONS, MATERIAL STRESSES, GRADE AND SPECIES OF WOOD, AND PLACEMENT DRAWING.
- 7. DEFAULT HEADER SIZE NOT SPECIFIED SPANNING 8'-0" MAX SHALL BE
- 8. ALL HEADERS OVER 4'-0" SHALL HAVE DOUBLE TRIMMER @ EACH SUPPORT, OR AS SPECIFIED, UNO.
- 9. SOLID BLOCKING BETWEEN JOISTS @ 36" OC FOR JOISTS PARALLEL
- 11. BLOCK BETWEEN JOISTS @ SUPPORTS OR OVER BEAMS.
- 12. RATED CONSTRUCTION FOR PROJECTIONS INTO SETBACKS AS REQ'D.
- 13. DOUBLE JOIST BELOW PARALLEL NONBEARING WALLS ON LAYOUT, SINGLE JOIST OFF LAYOUT. STRUCTURE BELOW LOAD-BEARING WALLS AS NOTED ON PLANS.

FOUNDATION PER **JOCOBO RESIDENTIAL** FOUNDATION GUIDELINE

40,000 * 0.2

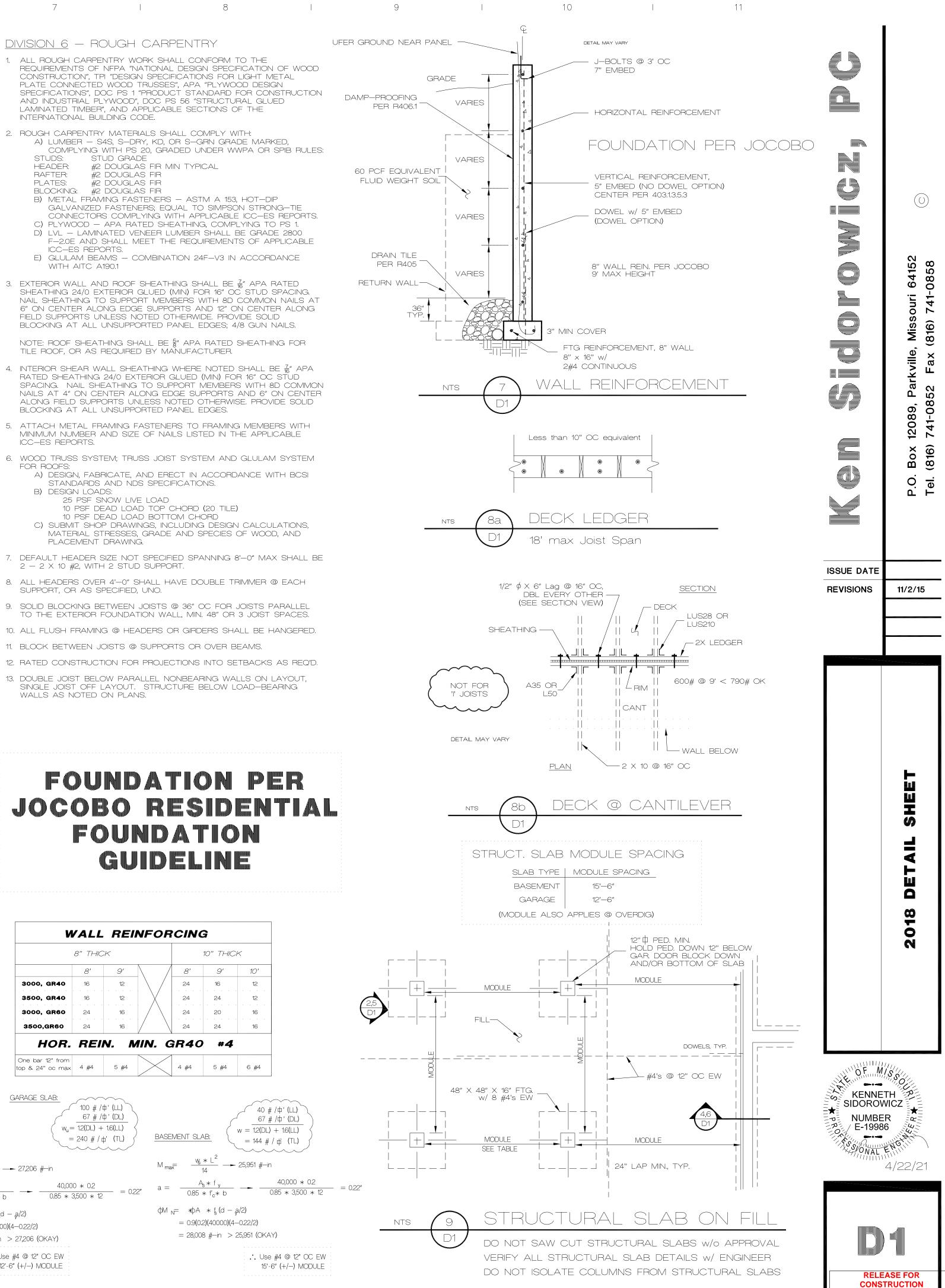
 $\phi M_N = *\phi A * f_s (d - a/2)$

= 0.9(0.2)(40000)(4-0.22/2)

 $= 28,008 \ \#\text{--in} > 27,206 \ (OKAY)$

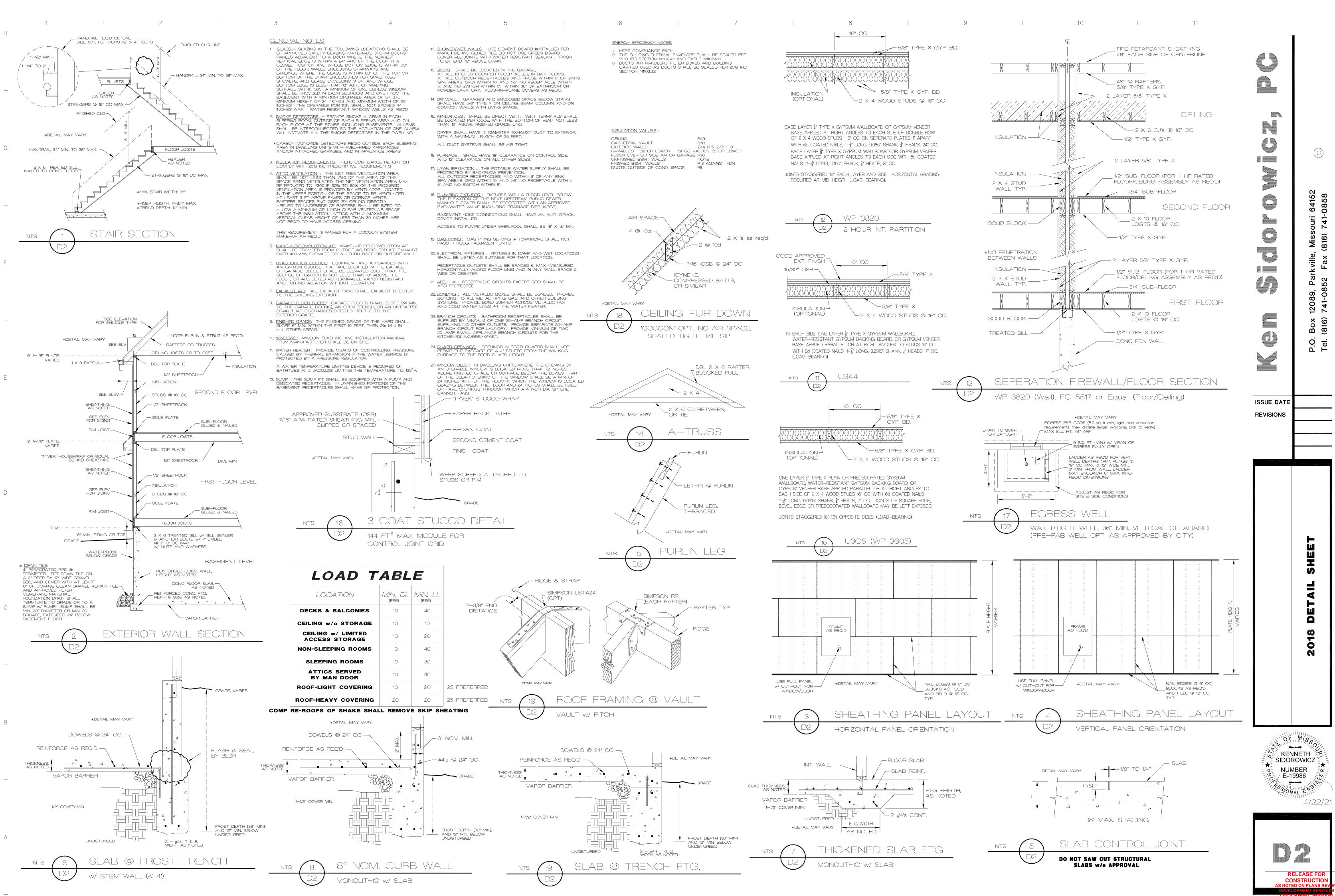
... Use #4 @ 12" OC EW

12'-6" (+/-) MODULE



LEE'S SUMMIT, MISS

05/12/2021



05/12/2021

3 | I 2 i STAPLES NOT PERMITTED IN KCMO

1

4

	FAST	ENER SCHEDULE FC	OR STRUCTURAL	
ltem	Description of building eler	ments	Number & type of fastener (notes: a, b, c)	Spacing of fasteners
Roof				
1	Blocking between joists or rafters to to	p plate, toe nail	3-8d (2-1/2" × 0.113")	.
2	Ceiling joists to plate, toe nail		3-8d (2-1/2" × 0.113")	.
3	Ceiling joists not attached to parallel ra	fter, laps over partitions, face nail	3–10d	
4	Collar tie rafter, face nail or 1-1/4" × 20) ga. ridge strap	3-10d (3" × 0.128")	2 toe nails side 1, 1 toe nail side 2 (note j)
5		se STC clips at NLB walls and spec'd holdowns	3-16d or 3-10d (3-1/2" × 0.135", 0.148")	
6	Roof rafters to ridge, valley or hip rafte Toe nail:	1S.	4-16d (3-1/2" × 0.135")	-
	Face nail:		3-16d (3-1/2" × 0.135")	
Vall			10d (3" × 0.128")	24° o.c.
7	Built-up studs-face nail		16d (3-1/2" × 0.135")	12° o.c.
8	Abutting studs at intersecting wall corn Built-up header, two pieces w/ 1/2" sp		16d (3-1/2" × 0.135")	16" o.c. along each edge
9	Continued header, two pieces		16d (3-1/2" × 0.135")	16" o.c. along each edge
. <u>10</u> 11	Continuous header to stud, toe nail		4-8d (2-1/2" × 0.113")	$\frac{1}{2}$
" 12	Double studs, face nail		10d (3" × 0.128")	24ª o.c.
≌ 13	Double top plates, face nail		10d (3" × 0.128")	24° o.c.
13 14	Double top plates, nin. 48" offset of er	nd joints, face nail in lapped area	8-16d (3-1/2" × 0.135")	· · · · · · · · · · · · · · · · · · ·
** 15	Sole plate to joist or blocking, face nai		16d (3-1/2" × 0.135")	16" o.c.
. <u>16</u>	Sole plate to joist or blocking at brace	d wall panels	3-16d (3-1/2" × 0.135")	16° o.c.
	Stud to sole plate, toe nail		3-8d (2-1/2" × 0.113") or	-
			2-16d (3-1/2" × 0.135")	
18	Top or sole plate to stud, end nail		2-16d (3-1/2" × 0.135")	.
19	Top plates, laps at corners and interse	ections, face nail	2-10d (3" × 0.128")	.
20	1" brace to each stud and plate, face	nail	2-8d (2-1/2" × 0.113")	.
	1" \times 6" sheathing to each bearing, face	a nail	2 staples 13/4" 28d (21/2" × 0.113")	
21			2 staples 1-3/4" 2-8d (2-1/2" × 0.113")	
	1" × 8" sheathing to each bearing, face	e nail	3 staples 1-3/4"	· · · · · · · · · · · · · · · · · · ·
23	Wider than 1" x 8" sheathing to each	bearing, face nail	3- 8 05 (2014/2"1-x3(0/1"13")	
=loor				
. 24	Joist to sill or girder, toe nail		3-8d (2-1/2" × 0.113") 8d (2-1/2" × 0.113")	6° oc
25	Rim joist to top plate, toe nail (roof ap		8d (2-1/2" × 0.113")	6° o.c.
26	Rim joist or blocking to sill plate, toe r 1" \times 6" subfloor or less to each joist, f		2-8d (2-1/2" × 0.113")	· · · · · · · · · · · · · · · · · · ·
27	T x 6" subtion or less to each joist, it		2 staples 1-3/4"	-
28	2" subfloor to joist of girder, blind and	face nail	2-16d (3-1/2" × 0.135")	· · · · · · · · · · · · · · · · · · ·
28 29	2" planks (plank & beam - floor and	roof)	2-16d (3-1/2" × 0.135")	@ each bearing
	Built-up girders and beams, 2" lumbe		10a (3" × 0.128")	Nail each layer as follows: 32" o.c. at top
		-		and bottom and staggered. Two nails at er
			3-16d (3-1/2" × 0.135")	and at each splice @ each joist or rafter
31	Ledger strip supporting joists or rafters	3		
			Spacing U	f Fasteners
	Description of building	Description of fastener	Edges (inches)	Intermediate supports (inches
	materials	(notes: b, c, e)	(note: i)	(notes: c, e)
		Il sheathing to framing and particleboard wall sheath	ing to framing	
32	3/8" to 1/2"	6d common (2" × 0.113") nail (subfloor, wall) (note j)	6	12 (note: g)
32	3/8 10 1/2	8d common $(2-1/2" \times 0.131")$ nail (roof)		
	19/32" to 1"	8d common nail (2-1/2" × 0.131")	6	12 (note: g)
. 33 	1-1/8" to 1-1/4"	10d common (3" × 0.148") nail or	6	12
04		8d deformed (2-1/2" × 0.131") nail		l
Other wall st	heathing (note h)			
35	1/2" structural cellulosic fiberboard	1-1/2" galv. roofing nail, 7/16" crown or	3	6
	sheathing	1" crown staple 16 ga, 1-1/4" long	1	
36	25/32" structural cellulosic	1-3/4" galv. roofing nail, 7/16" crown or	3	6
	fiberboard sheathing	1" crown staple 16 ga, 1-1/2" long		
37	1/2" gypsum sheathing (note d)	1-1/2" galvanized roofing nail; staple galv.,	7	/
		1-1/2" long; 1-1/4" screws, Type W or S		
38	5/8" gypsum sheathing (note d)	1-3/4" galvanized roofing nail; staple galv, 1-5/8" long; 1-5/8" screws, Type W or S	7	,
Wood etuct	ural panels, combination subfloor underla		· ·	
		6d deformed (2" × 0.120") nail or	6	12
39	3/4" and less	8d common (2-1/2" × 0.120") hall 01		
			4	1
	7/8° to 1°		6	E E
40 	7/8" to 1"	8d common (2-1/2" × 0.131") nail or	6	
40 		8d common (2-1/2" × 0.131") nail or 8d deformed (2-1/2" × 0.120") nail	6 	12
	7/8" to 1" 1-1/8" to 1-1/4"	8d common (2-1/2" × 0.131") nail or	6 	

For St 1 inch = 25.4 mm, 1 foot = 304.8 mm, 1 mile per hour = 0.447 m/s; 1 ksi = 6.895 MPa

a. All naits are smooth-common, box or deformed shanks except where otherwise stated. Naits used for framing and sheathing connections shall have minimum average bending yield strengths as shown: 80 ksi (551 MPa) for shank diameter of 0.192 inch (20d common nail), 90 ksi (620 MPa) for shank diameters larger than 0.142 inch but not larger than 0.177 inch, and 100 ksi (689 MPa) for shank diameters of 0.142 inch or less.

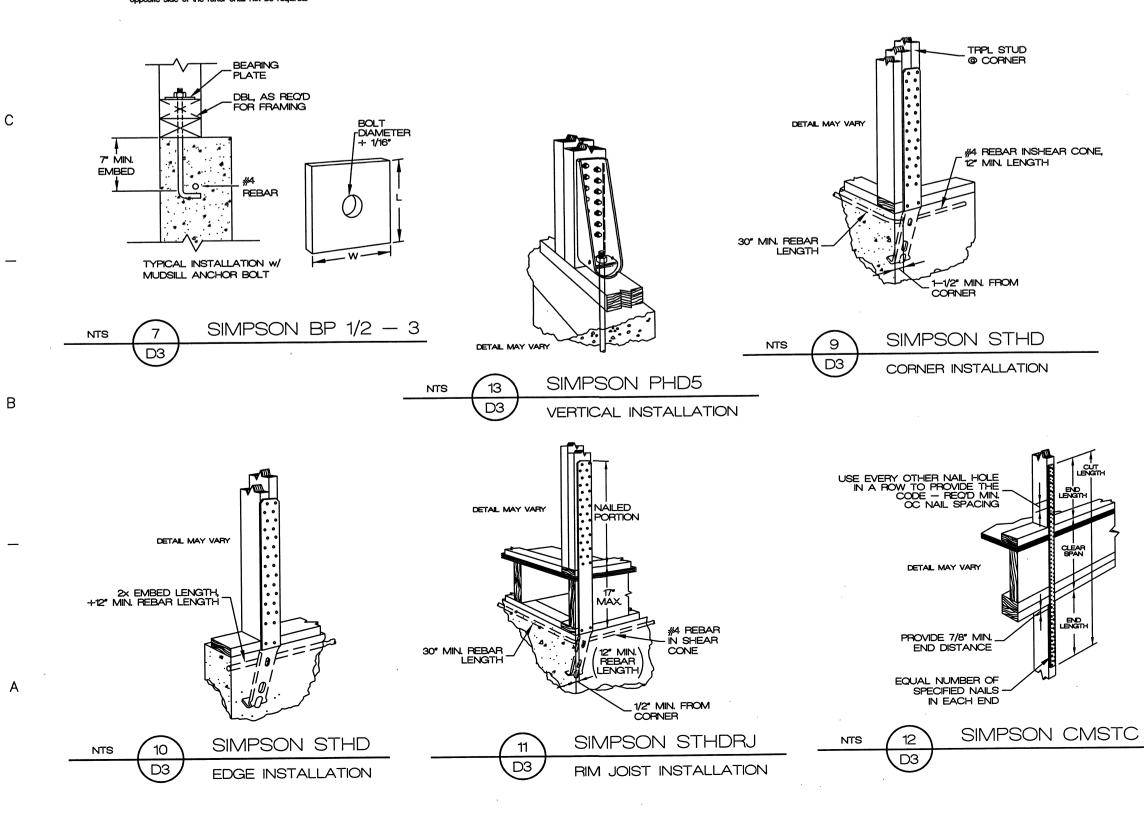
b. Staples are 16 gage wire and have a minimum 7/16-inch on diameter crown width. c. Nails shall be spaced at not more than 6 inches on center at all supports where spans are 48 inches or greater.

Four-foot-by-8-foot or 4-foot-by-9-foot panels shall be applied vertically. e. Spacing of fasteners not included in this table shall be verified w/ EOR

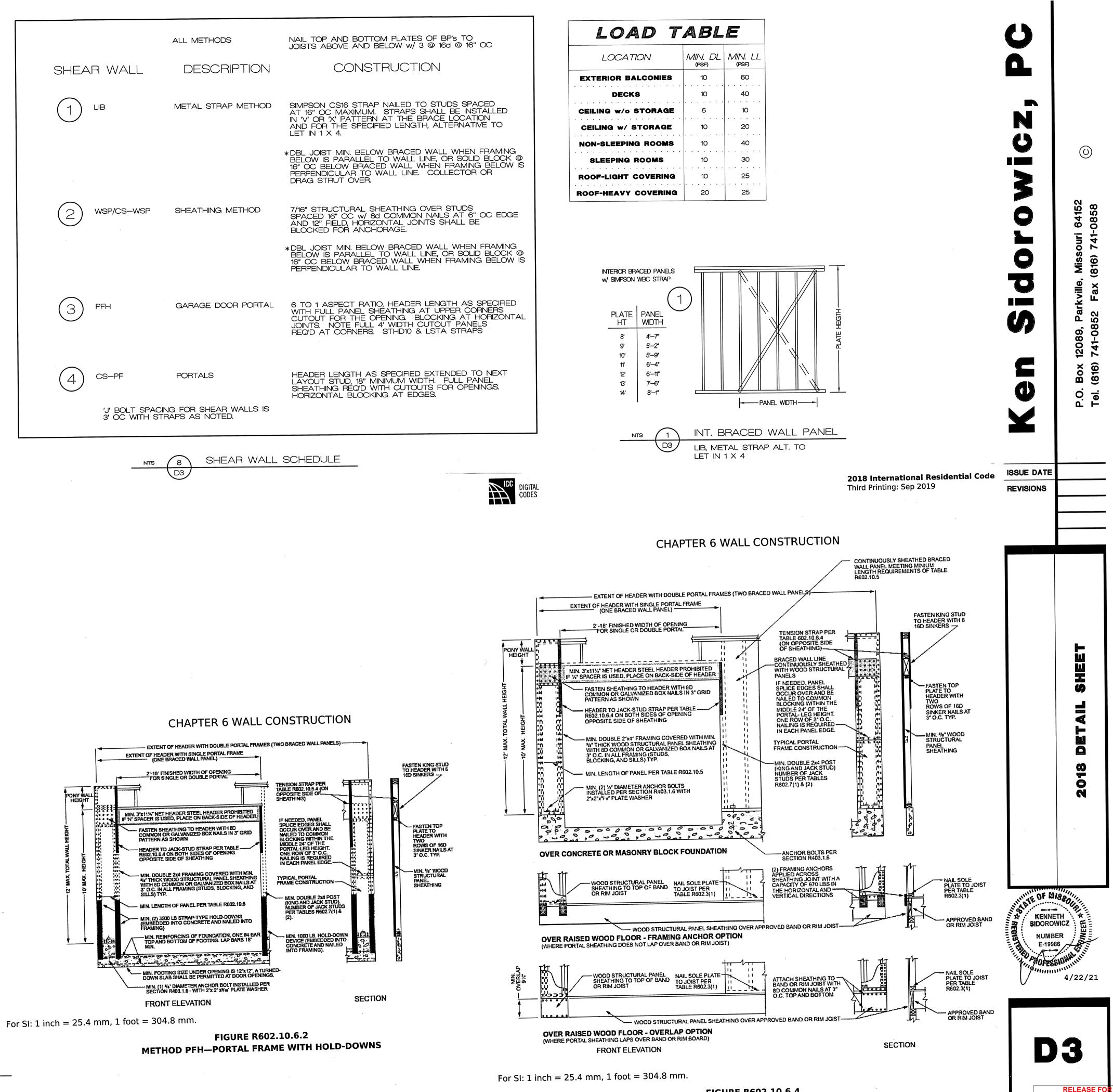
f. For regions having basic wind speed of 110 mph or greater, 8d deformed nails shall be used for attaching plywood and wood structural panel roof sheathing to framing within minimum 48-inch distance from gable end walls, if mean roof height is more than 25 feet, up to 35 feet maximum. g. For regions having basic wind speed of 100 mph or less, nails for attaching wood structural panel roof sheathing to gable endwall framing shall be spaced 6 inches on center. When basic wind speed is greater than 100 mph, nails for attaching panel root sheathing to intermediate supports shall be spaced 6 inches on center for minimum 48-inch distance from ridges, eaves and gable end walls; and 4 inches on center to gable end wall framing.

h. Gypsum sheathing shall conform to ASTMC 1396 and shall be installed in accordance with GA 253. Fiberboard sheathing shall conform to ASTM C 208. i Spacing of fasteners on floor sheathing panel edges applies to panel edges supported by framing members and at all floor perimeters only. Spacing of fasteners on roof sheathing panel edges applies to panel edges supported by framing members and at all roof plane perimeters. Blocking of roof or floor sheathing panel edges perpendicular to the framing members shall not be required except at intersection of adjacent roof planes. Floor and roof perimeter shall be supported by

framing members or solid blocking. Where a ratiler is fastened to an adjacent parallel ceiling joist in accordance with this schedule, provide two toe nails on one side of the ratiler and toe nails from the ceiling joist to top plate in accordance with this schedule. The toe nail on the opposite side of the rafter shall not be required.



F BP's TO > 16d @ 16" OC	NAIL TO	ALL METHODS		
N		DESCRIPTION	R WALL	SHEA
GTUDS SPACED ALL BE INSTALLED ICE LOCATION ALTERNATIVE TO	AT 16" (IN 1/1 O	METAL STRAP METHO	LIB	$\left(1\right)$
ALL WHEN FRAMING E, OR SOLID BLOCK @ EN FRAMING BELOW IS COLLECTOR OR	BELOW			
ER STUDS AILS AT 6" OC EDGE 3 SHALL BE	SPACE	SHEATHING METHOD	WSP/CS-WSP	(2)
ALL WHEN FRAMING E, OR SOLID BLOCK @ EN FRAMING BELOW IS				~
NGTH AS SPECIFIED UPPER CORNERS CKING AT HORIZONTAL OUT PANELS STA STRAPS	WITH F CUTOU	GARAGE DOOR PORTA	PFH	3
(TENDED TO NEXT 1. FULL PANEL 2. FOR OPENINGS. 3.	LAYOU SHEAT HORIZC	PORTALS G FOR SHEAR WALLS IS	CS-PF	4
		APS AS NOTED.	3' OC WITH STF	
	L SCHED	B SHEAR WAL	NTS	



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FIGURE R602.10.6.4 METHOD CS-PF—CONTINUOUSLY SHEATHED PORTAL FRAME PANEL CONSTRUCTION

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