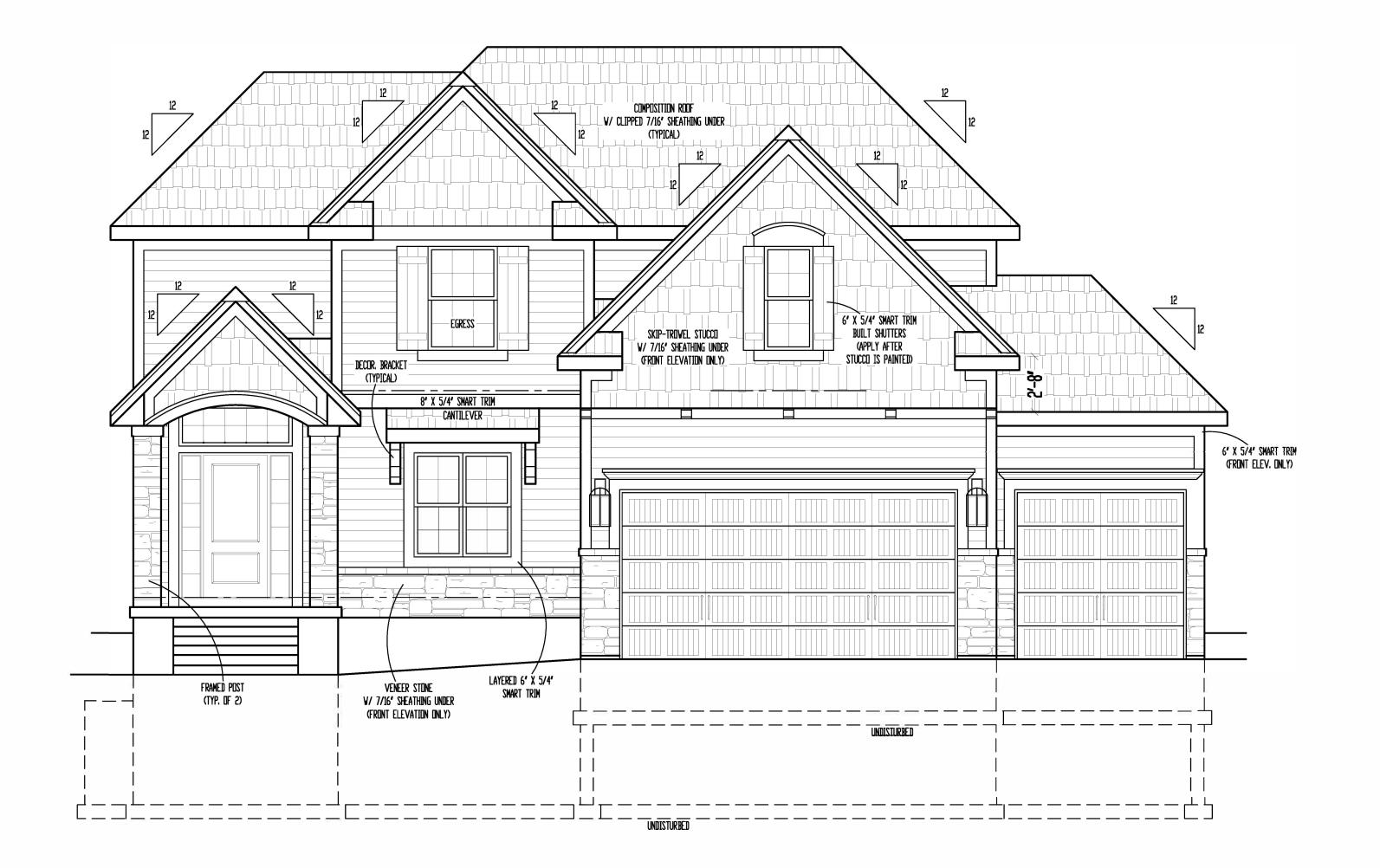
NOTE: GOVERNING CODES & GENERAL CONTRACTOR'S WRITTEN SPECIFICATIONS TAKE PRECEDENCE OVER THESE PLANS.



FRONT ELEVATION

SCALE: 1/4" = 1'-0"

RELEASE FOR
CONSTRUCTION
AS NOTED ON PLANS REVIEW
DEVELOPMENT SERVICES
LEE'S SUMMIT, MISSOURI

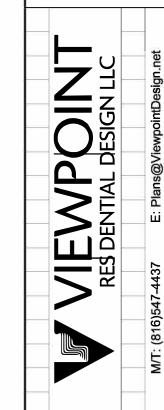
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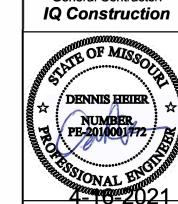
begotten Son, that whosoever believeth in him should not perish, but have



Drawing title:

The
OAKMONT

Elevation: F
Site Description:
Lot 738, Eagle
Creek 16th Plat
Street Address:
2362 SW Hickory
Ln., Lee's Summit,
Missouri
General Contractor:



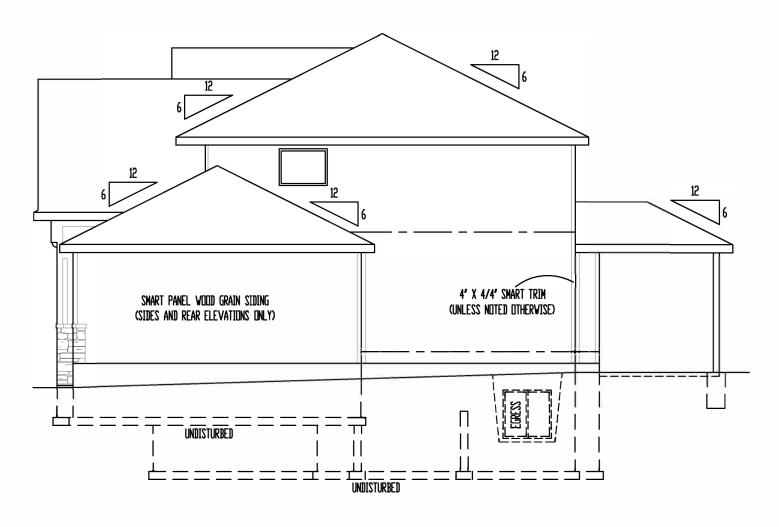
Date: 4 - 5 - 7 Rev. 1: Rev. 2: Rev. 3:

Sheet Title:
FRONT
ELEVATION

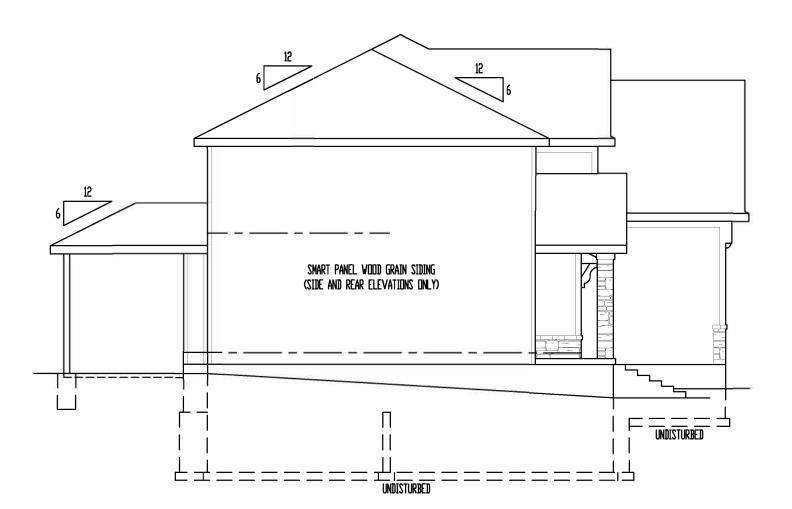
Sheet No.:

A-1

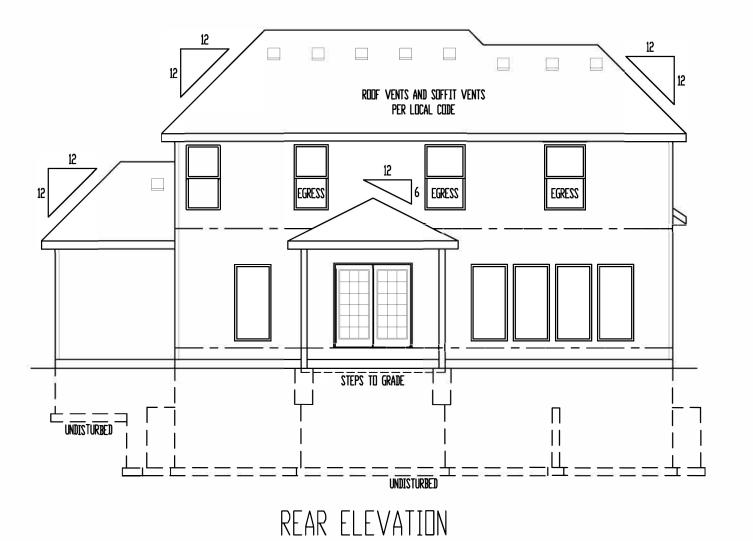
of 6



RIGHT ELEVATION SCALE: 1/8" = 1'-0"



LEFT ELEVATION SCALE: 1/8'' = 1'-0''

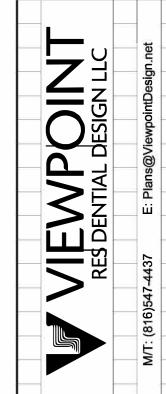


SCALE: 1/8" = 1'-0"

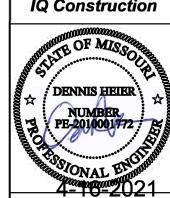
ELEVATIONS:
SMART PANEL VOOD GRAIN SIDING ON SIDE AND REAR ELEVATIONS
COMPOSITION ROOF SHINGLES
LOCATE ROOF AND SOFFIT VENTS PER CODE
ADJUST FOUNDATION TO GRADE

OPTIONAL DECK:

DECK CONSTRUCTION TO COMPLY WITH MUNICIPALITY'S RESIDENTIAL DECK STANDARDS
2' X 10" #2 TTD. @ 16" D.C. FLOOR JOISTS (MAX. SPAN: 14"-0") 2' X 6' TTD. DECKING
6' X 6' TTD. POSTS
2' X 2' TTD. SPINDLES
2' X 2' TTD. TOP RAIL
DETERMINE OPTIONAL STAIRS ON SITE



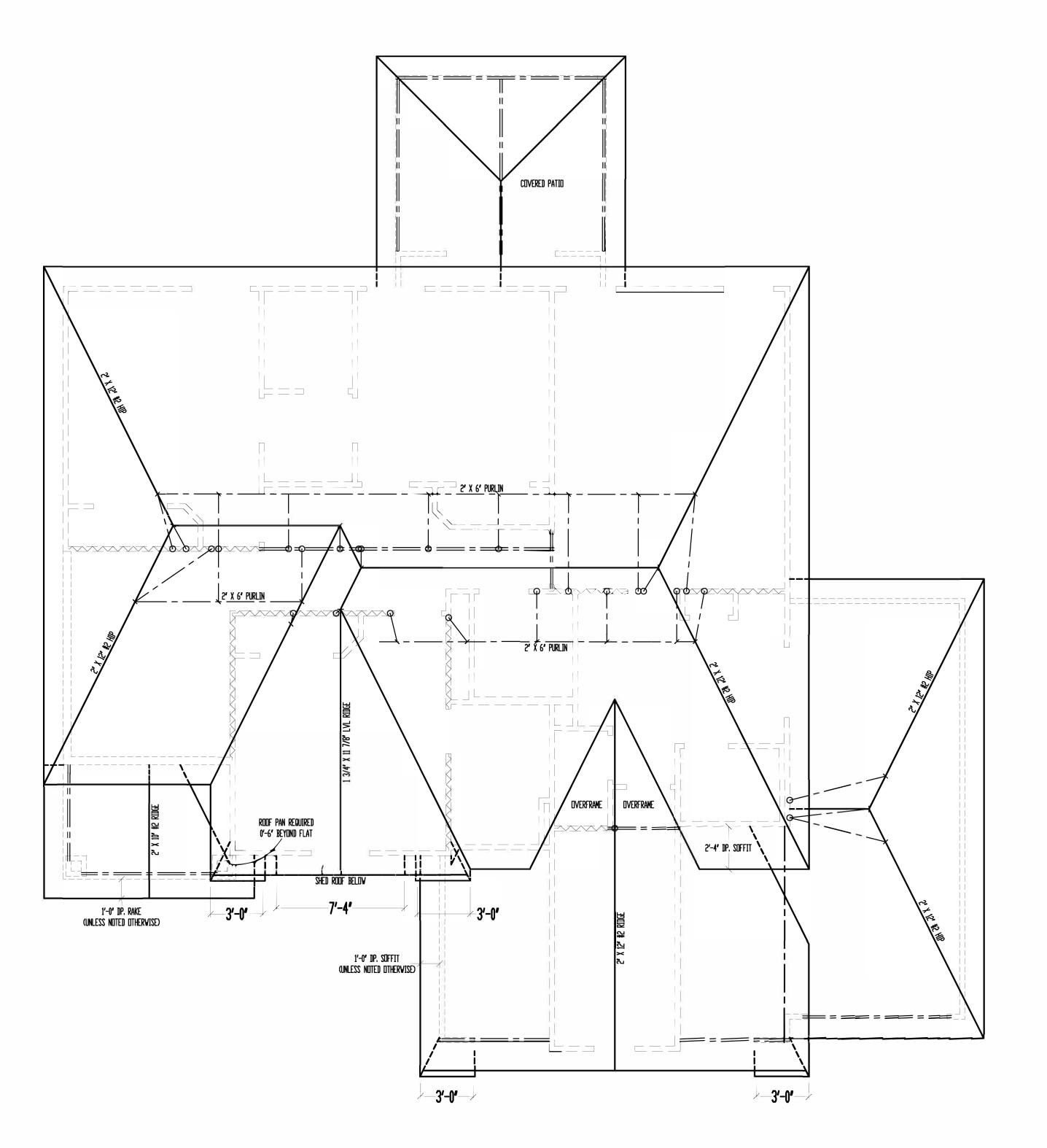
Drawing title: **The OAKMONT** Elevation: F
Site Description:
Lot 738, Eagle
Creek -16th Plat Street Address: 2362 SW Hickory Ln., Lee's Summit, Missouri General Contractor: **IQ** Construction



Rev. 1: Rev. 2: Rev. 3:

Sheet Title: SIDES & REAR **ELEVATIONS**





*ALL RAFTERS SHALL BE 2' X 6' #2 @ 16' D.C., UNLESS NOTED OTHERVISE.

Flashing note:

ROOF DESIGNED FOR LIGHT ROOF COVERING 30psf TOTAL LOAD [10psf DL, 20psf LL (SL)]

* RAFTERS (HEM-FIR, DOUG-FIR, OR EQUAL): SEE SPAN CHARTS BELOW

	CODE MINI	MIIM		
	RAFTERS	SPACING	MAX HORIZONTAL CLEARSPAN	7
	#2-2x6	€24" D.C.	11'-7'	1
>>>	#2-2x6	€16 ′ □.C.	14'-2'	///
	#2-2x8	624, D.C.	14'-8']
	#2-2x8	€16' D.C.	17'-11 '	
	#2-2x10	024′ □.C.	17'-10 '	
	#2-2x10	€16' D.C.	21′-11 ′	

NOTE: CODE MINIMUM ALLOWS FOR A RAFTER DEFLECTION OF L/180 TOTAL LOAD

HIGHER PE	rformance (ri	ECOMMENDED)
RAFTERS	SPACING	MAX HORIZONTAL CLEARSPAN
#2-2x6	024" D.C.	8'-6 '
#2-2x6	016 ′ □.C.	9'-9 '
#2-2x8	024" D.C.	11'-3 '
#2-2x8	916 ′ □.C.	12'-9 '
#2-2x10	024" D.C.	14'-3 '
#2-2x10	016 ° □.C.	16'-3 '
DEFLECTIO	N = L/360 LIV	VE LOAD, L/240 TOTAL LOAD

* ALL HIPS & VALLEYS ARE: (UNLESS OTHERWISE NOTED)

- #2- 2X8 UP TO 10/12 PITCH - #2- 2X10 OVER 10/12 PITCH

- PURLIN STRUTS ARE AT 4'-0' D.C.

- ALL PURLINS STRUTS SHALL HAVE A MAXIMUM UNBRACED LENGTH OF 8'-0'

i odni idomi	TION THE FOLLOWIN	
PURLIN STRUT	MAX PURLIN STRUT LENGTH	
(2) 2x4	8'-0 '	
D 2x4 & (1) 2x6	12'-0 '	

* RIDGE BRACES ARE SAME AS PURLIN BRACES-(SEE PURLIN BRACE NOTES ABOVE) * HIP & VALLEY BRACES ARE SAME AS PURLIN

ROOF

SCALE: 1/4'' = 1'-0''

DRIP EDGE, VALLEYS AND FLASHINGS TO BE METAL CLAD.

	CODE MINI	MUM		
	RAFTERS	SPACING	MAX HORIZONTAL CLEARSPAN	
	#2-2x6	024' D.C.	11'-7 '	Ĭ
>>>	#2-2x6	€16 ' D.C.	14'-2 '	///
	#2-2x8	024' D.C.	14'-8 '	
	#2-2x8	016 ′ □.C.	17'-11 '	
	#2-2x10	024' D.C.	17'-10 '	
	#2-2x10	016 ′ □.C.	21'-11 '	

HIGHER PERFORMANCE (RECOMMENDED)				
RAFTERS	SPACING	MAX HORIZONTAL CLEARSPAN		
#2-2x6	024' D.C.	8'-6 '		
#2-2x6	€16 ′ □.C.	9'-9 '		
#2-2x8	924' D.C.	11'-3 '		
#2-2x8	€16° D.C.	12'-9'		
#2-2x10	024" D.C.	14'-3'		
#2-2v10	916' TC	16'-3'		

* VAULTS TO BE 2x10 DEPTH

* RIDGE BOARDS ARE: (UNLESS OTHERVISE NOTED)

- #2- 2x8 up to 10/12 pitch

- #2- 2X10 OVER 10/12 PITCH

* PURLINS ARE 2X6 MIN.

- PURLIN STRUTS SHALL BE INSTALLED AT NOT LESS THAN A 45 DEGREE ANGLE WITH THE HURIZUNTAL

- PURLINS STRUTS SHALL BE CONSTRUCTED IN A 'T' CONFIGURATION AND PER THE FOLLOWING CHART:

MAX PURLIN STRUT LENGTH
8′-0″
12'-0 '
20'-0 '
30′-0 ′
30′-0 ′

SPACING, SIZE, CONFIGURATION, & INSTALLATION SIZE, CONFIGURATION, & INSTALLATION (SEE PURLIN BRACE NOTES ABOVE)

* VERTICAL BRACE IF DOT IS UNDER HIP OR VALLEY

* SLASH IS TOP END OF BRACE (/), DOT IS BOTTOM OF BRACE (o). # DENOTES BEARING VALL

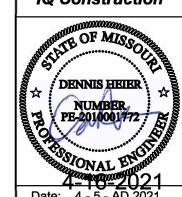
DENOTES ROOF BRACE

DENOTES PURLIN

DENOTES BEARING STRUCTURE

The **OAKMONT** Elevation: F Site Description: Lot 738, Eagle Creek -16th Plat Street Address: 2362 SW Hickory Ln., Lee's Summit, Missouri General Contractor: **IQ Construction**

Drawing title:



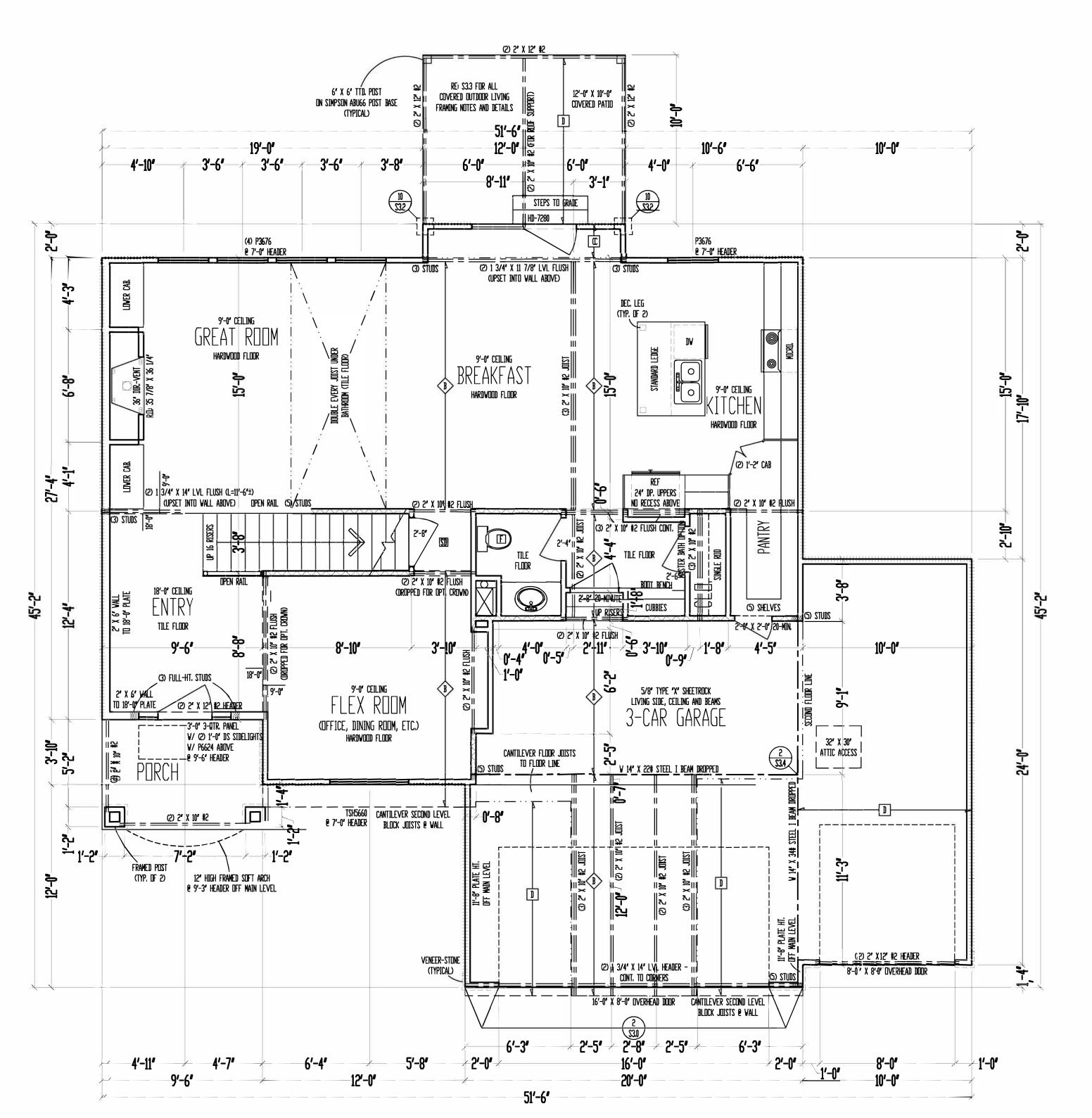
Rev. 3: Sheet Title:

ROOF PLAN

Rev. 1:

Rev. 2:





9'-0" CEILING
2" X 10" FLOOR SYSTEM
MAIN LEVEL
SCALE: 1/4" = 1'-0"

MAIN LEVEL: 1094 SQ. FT. SECOND LEVEL: 1225 SQ. FT. TOTAL: 2319 SQ. FT.

GARAGE: 667 SQ. FT.
UNFIN. BASEMENT: 1055 SQ. FT.
COV. DUT/LIV: 120 SQ. FT.

FRAMING NOTES

1. MAIN LEVEL EXTERIOR WALLS SHALL BE SHEATHED W/ 7/16' (I.S.B. A.P.A. PANELS W/ 8d COMMON NAILS @ 6' D.C. AT EDGES & @ 12' D.C. IN THE FIELD. SMART PANEL, DR EQUAL, INSTALLED PER MANUFACTURER'S SPECIFICATIONS.

2. \ \ \ \ \ \ \ \ \ \ \ = G.B.: 1/2' MIN. GYPSUM BOARD OVER STUDS SPACED 24' MAX FASTENED W/ NO. 6 - 1 1/4' TYPE W OR S DRYWALL SCREWS @ 7' D.C. EDGES & FIELD. (MIN. 8'-0' SECTIONS ONE SIDE OF WALL (OR) MIN. 4'-0' SECTION FOR BOTH SIDES)

3. /\/\/\/\/\/\/\/\/\/\/\/\ = LOAD BEARING INTERIOR WALL.

4. (2) 2' X 10' #2 HEADER AT ALL EXTERIOR AND LOAD BEARING WALLS, UNLESS NOTED OTHERWISE.

5. LOW TIES @ 4'-0' D.C. (TYPICAL)

6. RUN STUDS THE FULL HEIGHT OF RAISED PLATE WALLS.

7. BLOCK JOISTS ABOVE BEAMS, CANTILEVERS AND LOAD BEARING WALLS WITH JOIST MATERIAL (NOT REQUIRED WITH I-JOISTS).

8. PROVIDE MULTIPLE STUDS FOR SOLID BEARING BELOW ALL BEAMS.

9. ALL DESIGNATED 2' X 6' WALLS SHALL HAVE DOUBLE KING STUDS AT DOOR AND WINDOW OPENINGS.

10. ALL UNSQUARE VALLS SHALL BE 45°, UNLESS NOTED OTHERVISE.
11. ALL VALLS TO BE FRAMED V/ MIN. STUD GRADE 2' X 4'S @ 16' DC.,
UNLESS NOTED OTHERVISE.
12. EVICTORID VALL POTTEN DLATES SHALL BE NATLED TO EDAMINE.

12. EXTERIOR VALL BOTTOM PLATES SHALL BE NAILED TO FRAMING BELOW VITH 16d COMMON NAILS @ 8' D.C. MAX. (VHERE APPLICABLE.)

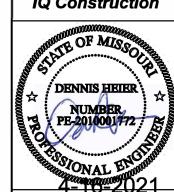
	JOIST SCHEDULE	
<u>A</u> >	2" X 10" #3 FLOOR JOIST @ 16" D.C.	
<u>B</u>	2' X 10' #2 FLOOR JOIST @ 16' D.C.	
С	2' X 6' #3 CEILING JOIST @ 16' D.C.	
D	2' X 6' #2 CEILING JOIST @ 16' D.C.	

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that whosoever believeth in him should not perish, but have everlasting life"



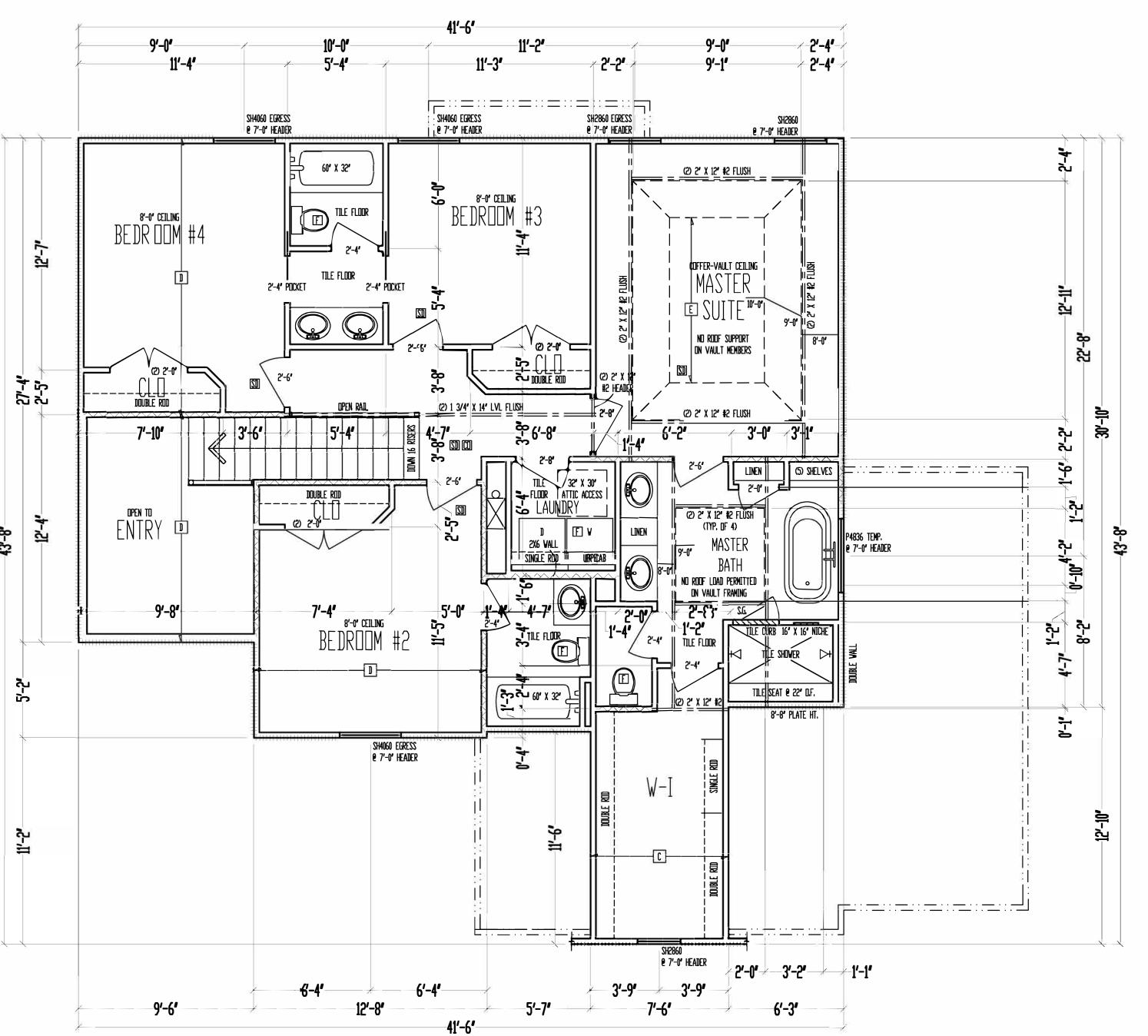
Drawing title:
The
OAKMONT
Elevation: F
Site Description:
Lot 738, Eagle
Creek 16th Plat
Street Address:
2362 SW Hickory
Ln., Lee's Summit,
Missouri
General Contractor:
IQ Construction

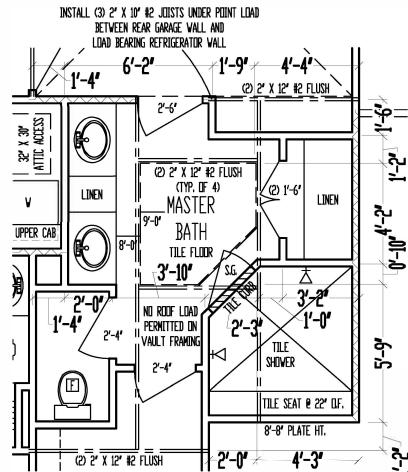


Date: 4 - Rev. 1: Rev. 2: Rev. 3:

Sheet Title:
MAIN LEVEL
PLAN







OPTION: NO WHIRLPOOL TUB OR WINDOW SCALE: 1/4" = 1'-0"

8'-0" CEILING

SECOND LEVEL

SCALE: 1/4" = 1'-0"

+++++++++++++++++++++ = WALL BRACING PER FRAMING NOTE #1
AND PER CALCULATIONS ON SHEET S1.1.

Framing No

1. SECOND LEVEL EXTERIOR VALLS SHALL BE SHEATHED W/ 7/16' [LS.B. APA. PANELS W/ 8d COMMON NAILS @ 6' [LC. AT EDGES & @ 12' [LC. IN THE FIELD. SMART PANEL, OR EQUAL, INSTALLED PER MANUFACTURER'S SPECIFICATIONS.

2. \ \ \ \ \ \ \ \ = G.B; 1/2" MIN. GYPSUM BOARD OVER
STUDS SPACED 24" MAX FASTENED W/ NO. 6 - 1 1/4" TYPE W OR S
DRYWALL SCREWS @ 7" O.C. EDGES & FIELD. OMIN. 8"-0" SECTIONS ONE
SIDE OF WALL (OR) MIN. 4"-0" SECTION FOR BOTH SIDES)
3. /\/\/\/\/\/\/\/\/\/\/\/\\\ = LOAD BEARING INTERIOR WALL.
4. (2) 2" X 10" #2 HEADER AT ALL EXTERIOR AND LOAD BEARING
WALLS, UNLESS NOTED OTHERWISE.
5. LOW TIES @ 4"-0" O.C. (TYPICAL)
6. RUN STUDS THE FULL HEIGHT OF RAISED PLATE WALLS.

7. BLOCK JOISTS ABOVE BEAMS, CANTILEVERS AND LOAD BEARING WALLS WITH JOIST MATERIAL (NOT REQUIRED WITH I-JOISTS).

8. PROVIDE MULTIPLE STUDS FOR SOLID BEARING BELOW ALL BEAMS.

9. ALL DESIGNATED 2" X 6" WALLS SHALL HAVE DOUBLE KING STUDS AT DOOR AND VINDOW OPENINGS.

10. ALL UNSQUARE WALLS SHALL BE 45", UNLESS NOTED OTHERWISE.

11. ALL VALLS TO BE FRAMED W/ MIN. STUD GRADE 2' X 4'S @ 16' D.C., UNLESS NOTED OTHERVISE.

12. EXTERIOR VALL BOTTOM PLATES SHALL BE NAILED TO FRAMING BELOW WITH 16d COMMON NAILS @ 16' D.C. MAX. (WHERE APPLICABLE.)

	JOIST SCHEDULE		
C 2' X 6' #3 CEILING JOIST			
D	2' X 6' #2 CEILING JOIST @ 16' O.C.		

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RES DENTIAL DESIGN LLC
M/T: (816)547-4437 E: Plans@ViewpointDesign.net

Drawing title:

The

OAKMONT

Elevation: F

Site Description:

Lot 738, Eagle

Creek
16th Plat

Street Address:

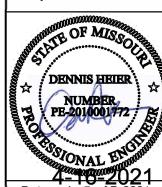
2362 SW Hickory

Ln., Lee's Summit,

Missouri

General Contractor:

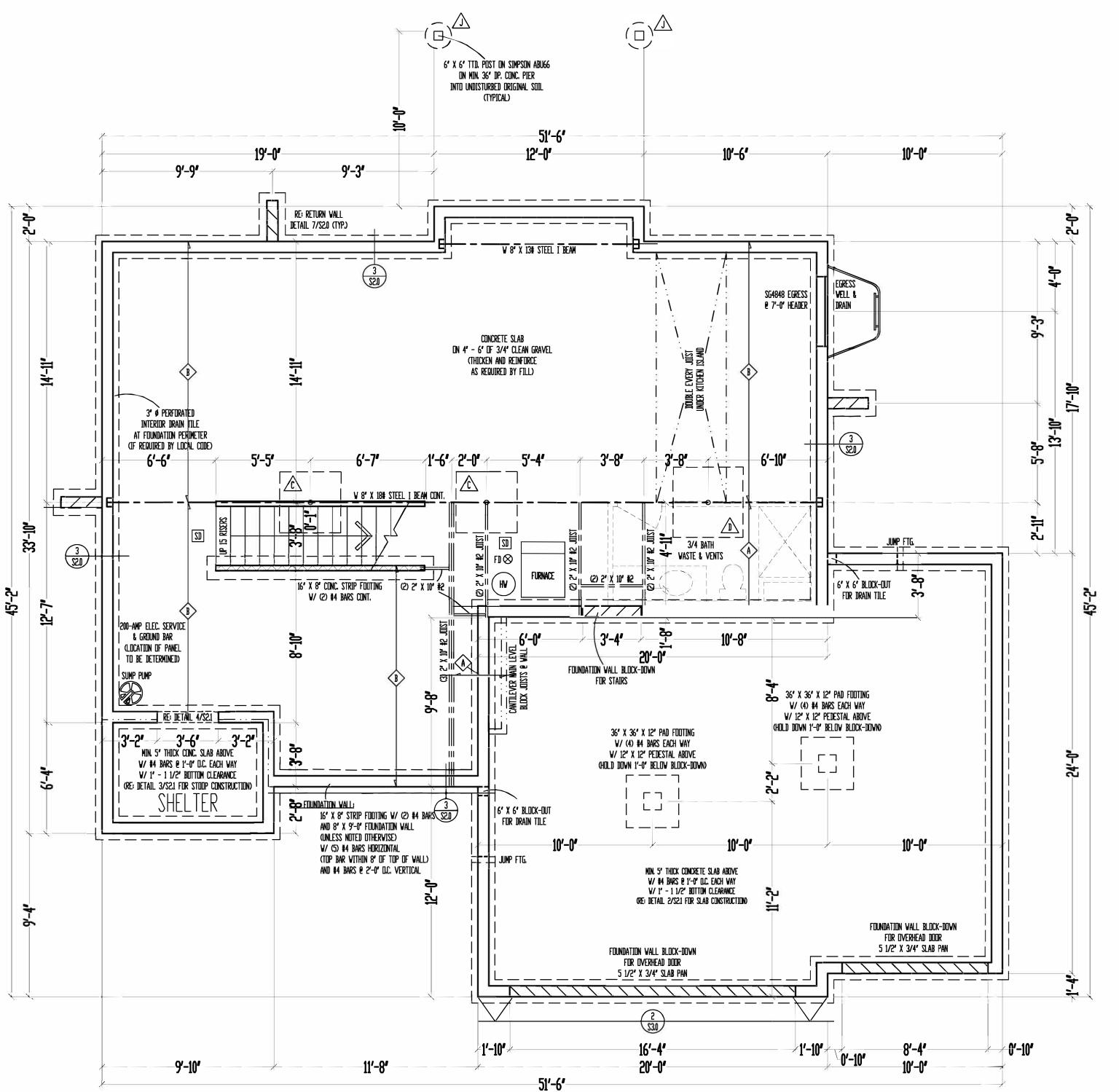
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Date: 4 - 5 - 7 Rev. 1: Rev. 2: Rev. 3:

Sheet Title: SECOND LEVEL PLAN





9'-0" FOUNDATION WALLS
(UNLESS NOTED OTHERWISE)
ON 16" X 8" STRIP FOOTINGS
(STEP WHERE GRADE REQUIRES)

2" X 10" FLOOR SYSTEM FOUNDATION SCALE: 1/4" = 1'-0"

+++++++++++++++++++++ = **W**all bracing per framing note: #1 and per calculations on sheet sl.1.

FRAMING NOTE

1. BASEMENT LEVEL EXTERIOR VOOD-FRAMED VALLS SHALL BE SHEATHED V/ 7/16' D.S.B. A.P.A. PANELS V/ 8d COMMON NAILS @ 6' D.C. AT EDGES & @ 12' D.C. IN THE FIELD. SMART PANEL, DR EQUAL, INSTALLED PER MANUFACTURER'S SPECIFICATIONS.

2. \ \ \ \ \ \ \ \ = G.B.: 1/2'' min. Gypsum board over studs spaced 24" max fastened w/ No. 6 - 1 1/4" type w or s drywall screws 0 7" d.C. edges & field. (Min. 8'-0' sections one side of wall (or) min. 4'-0' section for both sides)

3. //////////// = Load bearing interior wall.

4. (2) 2" X 10" #2 HEADER AT ALL EXTERIOR AND LOAD BEARING WALLS, UNLESS NOTED OTHERWISE.

5. LOW TIES @ 4'-0" D.C. (TYPICAL)

6. RUN STUDS THE FULL HEIGHT OF RAISED PLATE WALLS.
7. BLOCK JOISTS ABOVE BEAMS, CANTILEVERS AND LOAD BEARING WALLS WITH JOIST MATERIAL (NOT

8. PROVIDE MULTIPLE STUDS FOR SOLID BEARING BELOW ALL BEAMS.
9. ALL DESIGNATED 2' X 6' WALLS SHALL HAVE DOUBLE KING STUDS AT DOOR AND VINDOW

ENINGS,
ALL INVENTAGE MALLS SHALL BE 45° LINLESS NOTED OTHERWISE

11. ALL VALLS TO BE FRAMED V/ MIN. STUD GRADE 2' X 4'S @ 16' D.C., UNLESS NOTED OTHERVISE.

12. 1/2' Ø ANCHOR BOLTS V/ MIN. 7' EMBEDMENT @ 48' D.C. MAX. & VITHIN 6' - 12' OF END OF FACH PLATE LENGTH.

13. NEW FOUNDATION SHALL BEAR ON ORIGINAL SOIL WITH MINIMUM BEARING CAPACITY OF 1500 PSF. A GEOTECHNICAL ENGINEER IS RECOMMENDED FOR VERIFICATION OF THESE CONDITIONS DURING THE EXCAVATION PHASE, ENGINEER OF RECORD ASSUMES NO RESPONSIBILITY FOR CONSTRUCTION NOT VERIFIED TO BE FOUNDED ON ANYTHING SHORT OF THE AFOREMENTIONED REQUIREMENTS.

	STEEL COLUMN &
P	AD FOOTING SCHEDULE
Â	3" X 11 GA. STEEL COLUMN ON 30" X 30" X 10" PAD FOOTING W/ (4) #4 BARS EACH WAY (12.5k)
B	3 1/2" X 11 GA. STEEL COLUMN DN 36" X 36" X 10" PAD FOOTING W/ (4) #4 BARS EACH WAY (18.0k)
Ĉ	3' SCH. 40 STEEL COLUMN ON 42' X 42' X 12' PAD FOOTING W/ (5) #4 BARS EACH WAY (24.5k)
Î	3 1/2" SCH. 40 STEEL COLUMN ON 48" X 48" X 12" PAD FOOTING W/ (6) #4 BARS EACH WAY (32.0k)
É	3 1/2" SCH. 40 STEEL COLUMN ON 54" X 54" X 14" PAD FOOTING V/ (7) #4 BARS EACH WAY (40.5k)

3 1/2" SCH. 40 STEEL COLUMN On 60" X 60" X 14" PAD FOOTING

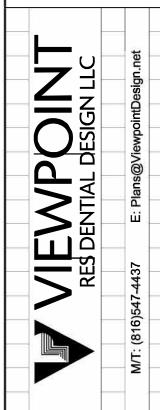
V/ (8) #4 BARS EACH WAY (50.0k)

	PIER FOOTING SCHEDULE		
_	A	12" Ø PIER FTG.	
i ()		16" Ø PIER FTG.	
i ()	\triangle	18' Ø PIER FTG.	
	Ŕ	24" Ø PIER FTG.	
ן מ			

-	6.1
JOIST SCHEDULE	
2" X 10" #3 FLOOR JOIST @ 16" D.C.	
2" X 10" #2 FLOOR JOIST @ 16" D.C.	
	2" X 10" #3 FLOOR JOIST @ 16" D.C. 2" X 10" #2 FLOOR JOIST

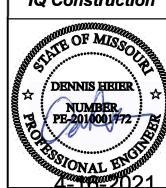
.S SE) NGS

> begonen son, that whosoever believeth in him should not perish, but have everlasting life"



Drawing title:

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IQ Construction



Date: <u>4 - 5 - AD</u> Rev. 1: Rev. 2: Rev. 3:

Sheet Title:
FOUNDATION
PLAN



	FASTENER SCHEDULE FOR STRUCTURAL MEMBERS	
DESCRIPTION OF BUILDING ELEMENTS	NUMBER AND TYPE OF FASTENER	SPACING AND LOCATION
	ROOF 1	
BLOCKING BETWEEN JOISTS OR RAFTERS TO TOP PLATE, TOE NAIL	4-8d (2½" x 0.113")	TOENAIL
CEILING JOISTS TO PLATE, TOE NAIL	4-8d (2½" x 0.113")	PER JOIST, TOENAIL
CEILING JOISTS NOT ATTACHED TO PARALLEL RAFTER, LAPS OVER PARTITIONS, FACE NAIL	4-10d (3" x 0.128")	FACE NAIL
CEILING JOIST TO PARALLEL RAFTER (HEEL JOINT)	TBLE R802.5.2	FACE NAIL
COLLAR TIE TO RAFTER, FACE NAIL OR 1 $\frac{1}{4}$ " x 20 GA. RIDGE STRAP TO RAFTER	4-10d (3" x 0.128")	FACE NAIL, EACH RAFTER
RAFTER OR ROOF TRUSS TO PLATE	3-16d BOX NAILS (3½" x 0.135") OR 3-10d COMMON NAILS (3" x 0.148")	2 TOE NAILS ON ONE SIDE AND 1 TOE NAIL ON OPPOSITE SIDE OF EACH RAFTER OR TRUSS
ROOF RAFTERS TO RIDGE, VALLEY, OR HIP RAFTERS OR ROOF RAFTER TO MINIMUM 2" RIDGE BEAM	4-16d (3 ½" x 0.135") - TOENAIL; 3-16d BOX (3 ½" x 0.135") - END NAIL	TOENAIL, END NAIL
	WALL	
STUD TO STUD (NOT AT BRACED WALL PANELS)	10d (3" x 0.128")	16" O.C. FACE NAIL
STUD TO STUD AND ABUTTING STUDS AT INTERSECTING WALL CORNERS (AT BRACED WALL PANELS)	16d (3½" x 0.135")	12" O.C. FACE NAIL
BUILT-UP HEADER, TWO PIECES WITH ½" SPACER	16d (3½" x 0.135")	12" O.C. EACH EDGE FACE NAIL
CONTINUOUS HEADER TO STUD	4-8d (2½" x 0.131")	TOENAIL
TOP PLATE TO TOP PLATE	10d (3" x 0.128")	12" O.C. FACE NAIL
DOUBLE TOP PLATE SPLICE	8-16d COMMON (3 ½" x 0.162")	FACE NAIL ON EACH SIDE OF END JOINT (MIN. 24' LAP SPLICE LENGTH EACH SIDE OF END JOINT)
BOTTOM PLATE TO JOIST, RIM JOIST, BAND JOIST, OR BLOCKING (NOT AT BRACED WALL PANELS)	16d COMMON (3 $\frac{1}{2}$ " x 0.162")	16" O.C. FACE NAIL
BOTTOM PLATE TO JOIST, RIM JOIST, BAND JOIST, OR BLOCKING (AT BRACED WALL PANEL)	3-16d BOX (3 ½" x 0.135")	3 EACH 16" O.C. FACE NAIL
TOP OR SOLE PLATE TO STUD, END NAIL	4-8d BOX (2 $\frac{1}{2}$ " x 0.113") - TOENAIL; 3-16d BOX (3 $\frac{1}{2}$ " x 0.135") - END NAIL	TOENAIL, END NAIL (SEE LEFT)
TOP PLATES, LAPS AT CORNERS AND INTERSECTIONS	3-10d BOX (3" x 0.128")	FACE NAIL
1" BRACE TO EACH STUD AND PLATE	3-8d BOX (2 ½" x 0.113")	FACE NAIL
1"x6" SHEATHING TO EACH BEARING	3-8d BOX (2 ½" x 0.113")	FACE NAIL
1"x8" SHEATHING TO EACH BEARING	3-8d BOX (2 $\frac{1}{2}$ " x 0.113") - FACE NAIL; WIDER THAN 1"x8" - 4-8d BOX (2 $\frac{1}{2}$ " x 0.113")	FACE NAIL
	FLOOR	
JOIST TO SILL, TOP PLATE, OR GIRDER	4-8d BOX (2 ½" x 0.113")	TOE NAIL
RIM JOIST, BAND JOIST, OR BLOCKING TO SILL OR TOP PLATE (ROOF APPLICATIONS ALSO)	8d BOX (2 ½" x 0.113")	4" O.C. TOE NAIL
1" x 6" SUBFLOOR OR LESS TO EACH JOIST	3-8d BOX (2 ½" x 0.113")	FACE NAIL
2" SUBFLOOR TO JOIST OR GIRDER	3-16d BOX (3 ½" x 0.135")	BLIND AND FACE NAIL
2" PLANKS (PLAN & BEAM - FLOOR AND ROOF)	3-16d BOX (3 ½" x 0.135")	AT EACH BEARING, FACE NAIL
BAND OR RIM JOIST TO JOIST	3-16d COMMON (3 ½" x 0.162")	END NAIL
BUILT-UP GIRDERS AND BEAMS, 2-INCH LUMBER LAYERS	10d BOX (3" x 0.128")	24" O.C. FACE NAIL AT TOP AND BOTTOM STAGGERED ON OPPOSITE SIDES
LEDGER STRIP SUPPORTING JOISTS OR RAFTERS	4-16d BOX (3 ½" x 0.135")	AT EACH JOIST OR RAFTER, FACE NAIL
BRIDGING OR BLOCKING TO JOIST	2-10d BOX (3" x 0.128")	EACH END, TOENAIL

DESCRIPTION OF BUILDING MATERIALS	FASTNER SCHEDULE FOR DESCRIPTION OF FASTENER	STRUCTURAL MEMBERS EDGE SPACING (INCHES)	INTERMEDIATE SUPPORTS (INCHES)			
WOOD STRUCTURAL PANELS, SUBFLOOR, ROOF AND INTERIOR WALL SHEATHING TO FRAMING AND PARTICLEBOARD WALL SHEATHING TO FRAMING						
¾" - ½"	6d COMMON (2" x 0.113") NAIL (SUBFLOOR, WALL) 8d COMMON NAIL (ROOF)	6	12			
¹⁹ / ₃₂ " - 1"	8d COMMON NAIL (2½" x 0.131")	6	12			
11/8" - 11/4"	10d COMMON (3" x 0.148") NAIL OR 8d (2½" x 0.131") DEFORMED NAIL	6	12			
	OTHER WALL	SHEATHING 1				
½" STRUCTURAL CELLULOSIC FIBERBOARD SHEATHING	$1\frac{1}{2}$ " GALVANIZED ROOFING NAIL, $\frac{7}{16}$ " HEAD DIAMETER, OR $1\frac{1}{4}$ " LONG 16 GA. STAPLE WITH $\frac{7}{16}$ " OR 1" CROWN	3	6			
25" STRUCTURAL CELLULOSIC FIBERBOARD SHEATHING	$1\frac{3}{4}$ " GALVANIZED ROOFING NAIL, $\frac{7}{16}$ " HEAD DIAMETER, OR $1\frac{1}{2}$ " LONG 16 GA. STAPLE WITH $\frac{7}{16}$ " OR 1" CROWN	3	6			
½" GYPSUM SHEATHING	1½" GALVANIZED ROOFING NAIL; STAPLE GALVANIZED, 1½" LONG; 1½" SCREWS, TYPE W OR S	7	7			
%" GYPSUM SHEATHING	1¾" GALVANIZED ROOFING NAIL; STAPLE GALVANIZED, 1½" LONG; 1½" SCREWS, TYPE W OR S	7	7			
We	OOD STRUCTURAL PANELS, COMBINATIO	N SUBFLOOR UNDERLAYMENT TO FRAM	ING			
¾" AND LESS	6d DEFORMED (2" x 0.120") NAIL OR 8d COMMON (2½" x 0.131") NAIL	6	12			
⅓" - 1"	8d COMMON (2½" x 0.131") NAIL OR 8d DEFORMED (2½" x 0.120") NAIL	6	12			
1½" - 1½"	10d COMMON (3" x 0.148") NAIL OR 8d DEFORMED (2½" x 0.120") NAIL	6	12			

1. IF INFORMATION LISTED ON PLAN SHEETS CONTRADICTS INFORMATION IN THIS TABLE, INFORMATION ON PLANS TAKES PRECEDENCE OVER INFORMATION

LISTED IN THIS TABLE

FOUNDATION NOTES

- CONCRETE SHALL BE AIR-ENTRAINED BETWEEN 5%-7% WITH A MINIMUM 28-DAY COMPRESSIVE STRENGTH OF 2500 PSI FOR BASEMENT AND INTERIOR FLOOR SLABS-ON-GRADE, 3000 PSI FOR FOUNDATION WALLS, AND 3500 PSI FOR PORCHES AND GARAGE FLOOR SLARS
- THE FOUNDATION DESIGN SHALL COMPLY WITH THE ENFORCING JURISDICTION'S RESIDENTIAL FOUNDATION STANDARDS
- PROVIDE A MINIMUM 4"-DIAMETER PERFORATED DRAIN PIPE ALONG PERIMETER OF USABLE SPACE AT FOOTING LEVEL OR OTHER EQUIVALENT MATERIALS PER IRC SECTION R405.1. THE PIPE SHALL BE COVERED WITH A MINIMUM OF 6" OF GRAVEL OR CRUSHED ROCK. THE DRAIN SHALL DAYLIGHT BELOW FOOTING LEVEL OR TERMINATE IN A MINIMUM 20 GALLON SUMP PIT
- FOUNDATION SHALL BE DESIGNED FOR A BEARING CAPACITY OF 1500 PSF AND FOUNDED ON COMPETENT ORIGINAL SOIL AS DETERMINED AND CONFIRMED BY A LICENSED GEOTECHNICAL ENGINEER OR ENGINEERING GEOLOGIST. ENGINEER OF RECORD ASSUMES NO RESPONSIBILITY FOR CONSTRUCTION NOT VERIFIED TO BE FOUNDED ON ANY SOIL WITH THE AFOREMENTIONED MINIMUM PROPERTIES.
- 5. FOOTINGS SHALL BE A MINIMUM OF 16" WIDE x 8" DEEP AND SHALL HAVE A MINIMUM OF (2) CONTINUOUS GRADE 40 #4 BARS WITH 3" BOTTOM CLERANCE. BOTTOM OF FOOTING SHALL BE LOCATED A MINIMUM OF 3'-0" BELOW GRADE
- FOR FROST PROTECTION. CONCRETE PADS SUP0PORTING COLUMN LOADS SHALL BE NO SMALLER THAN 2'-0" x 2'-0" x 1'-0" DEEP WITH A
- MINIMUM OF (4) GRADE 40 #4 BARS EACH WAY WITH 3" BOTTOM CLEARANCE FOUNDATION WALLS SHALL BE A MINIMUM OF 8" NOMINAL WIDTH AND SHALL HAVE HOIZONTAL GRADE 40 #4 BARS AT 2'-0" O.C. MAX. WITH VERTICAL #4 BARS AS REQUIRED ON FOUNDATION CROSS SECTION ON SHEET S2.0
- REINFORCEMENT SHALL LAP A MINIMUM OF 2'-0" (CLASS B SPLICE)
- INTERIOR BEARING WALLS AND COLUMNS SHALL BE ISOLATED FROM THE BASEMENT FLOOR SLAB BASEMENT FLOOR SLAB SHALL BE A MINIMUM OF 4" THICK ON A MINIMUM BASE COURSE OF 4" TO 6" OF SAND. GRAVEL OR CRUSHED ROCK. BETWEEN THE BASE COURSE AND FLOOR SLAB SHALL BE PLACED A 6-MIL POLY VAPOR RETARDER WITH MINIMUM OVERLAP OF 6" AT DISCONTINUITIES
- 11. IF A FLOOR IS TO BE SUPPORTED BY A MINIMUM OF 2'-0" OF GRANULAR FILL OR 8" OF EARTH, BASEMENT SLAB SHALL BE DESIGNED BY A LICENSED ENGINEER
- SILL PLATES SHALL BE ANCHORED TO THE FOUNDATION WALL WITH ½" Ø ANCHOR BOLTS EMBEDDED A MINIMUM OF 7" INTO CENTER OF WALL STEM AND SHALL BE INSTALLED AT A MAXIMUM OF 6'-0" O.C. (OR AS NOTED ON PLANS) AND SHALL BE INSTALLED WITHIN 6" TO 12" OF EACH END OF EACH SILL PLATE LENGTH, PER IRC SECTION R403.1.6
- 13. FOUNDATION WINDOW WELLS SHALL BE PROVIDED WITH MINIMUM DIMENSIONS AS SHOWN IN DETAIL ON SHEET
- 14. THE GARAGE FLOOR SHALL SLOPE TOWARD THE VEHICLE DOORS OR TO A TRENCH OR UNTRAPPED DRAIN THAT DISCHARGES TO THE EXTERIOR. ABOVE GRADE

FRAMING NOTES

- 15. ALL DIMENSIONAL LUMBER SHALL BE DOUGLAS-FIR-LARCH GRADE #2, UNLESS NOTED OTHERWISE ON PLANS ALL INTERIOR LOAD-BEARING AND EXTERIOR WALL HEADERS SHALL BE (2) #2 - 2x10's, UNLESS NOTED OTHERWISE
- BLOCK OVER BEAMS AND AT CANTILEVERS AND DOOR JAMBS INTERIOR NON-BEARING WALLS RESTING ON BASEMENT SLAB SHALL BE ISOLATED FROM ABOVE FRAMING BY A
- MINIMUM OF 1/2 ALL HEADERS/BEAMS SHALL BEAR ON A MINIMUM OF (2) 2x4 POSTS (KING AND JACK STUDS), UNLESS NOTED
- OTHERWISE WHERE JOISTS SPAN PARALLEL TO FOUNDATION, BLOCKING SHALL BE PROVIDED IN THE TWO SPACES MOST ADJACENT TO THE FOUNDATION WALL AT 4'-0" O.C. FOR THE PURPOSE OF TRANSFERRING LATERAL FOUNDATION WALL LOAD TO THE FLOOR DIAPHRAGM. FASTEN JOISTS AND BLOCKING TO SILL PLATE WITH (4) 10d NAILS. IF
- SECURE 2x4's TO JOIST(S)/SILL PLATE WITH (4) 10d NAILS ALL WOOD MATERIAL SUPPORTED ON CONCRETE OR MASONRY SHALL BE TREATED OR OF DECAY-RESISTANT

MECHANICAL DUCTWORK IS INSTALLED IN ONE OF THESE FIRST TWO BAYS, FASTEN 2x4's FLAT AT 4'-0" O.C. BETWEEN JOIST(S) AND/OR SILL AND PROVIDE BLOCKING AS PRESCRIBED ABOVE IN THE NEXT TWO JOIST BAYS.

- 22. JOISTS UNDER BEARING PARTITIONS ON PLANS HAVE BEEN SIZED TO SUPPORT THE DESIGN LOAD. JOISTS FRAMING INTO THE FACE OF A STEEL OR WOOD BEAM SHALL BE SUPPORTED WITH APPROPRIATE
- COLD-FORMED STEEL JOIST HANGERS JOISTS FRAMED ON TOP OF STRUCTURAL MEMBER SHALL BE SUPPORTED AT EN DS BY FULL-DEPTH SOLID BLOCKING MIN. 1/4" IN THICKNESS OR BY FASTENING RIM TO JOISTS PER FASTENING TABLE TO LEFT
- ALL WALL COVERINGS SHALL COMPLY WITH IRC SECTION R702.3
- ALL RAFTERS AND COLLAR TIES SHALL COMPLY WITH IRC SECTION R802.3.
- ALL RAFTERS SHALL HAVE 2x4 COLLAR TIES @ 4'-0" O.C. IN UPPER ⅓ OF VERTICAL DISTANCE BETWEEN CEILING AND
- BLOCKING BETWEEN JOISTS UNDER A LOAD-BEARING WALL IS NOT REQUIRED
- PER IRC SECTION 501.3, BOTTOM OF ALL FLOOR ASSEMBLIES ABOVE UNFINISHED AREAS SHALL BE PROVIDED WITH A 1/2" GYPSUM BOARD MEMBRANE OR RESIDENTIAL FIRE SPRINKLER SYSTEM WHEN FLOOR SYSTEM IS CONSTRUCTED OF OTHER THAN DIMENSION LUMBER OR STRUCTURAL COMPOSITE LUMBER EQUAL TO OR
- GREATER THAN 2x10 NOMINAL DIMENSION(WHERE REQUIRED BY ENFORCING JURISDICTION) ENGINEERED LVL's SHALL HAVE MINIMUM PROPERTIES OF Fb = 2600 psi, E=1900 ksi, AND Fv=285 psi
- ENGINEERED PARALLAMS SHALL HAVE MINIMUM PROPERTIES OF Fb = 2600 psi, E = 2000 ksi, AND Fv = 290 psi COLUMN CONNECTION TO STEEL BEAMS SHALL BE WITH A CLIP POST CAP WITH ALL FOUR TAB EARS BENT AROUND
- THE BOTTOM FLANGE OF THE BEAM. FOR A BEARING PLATE, FOUR HOLES SHALL BE DRILLED IN THE BOTTOM FLANGE OF THE STEEL BEAM TO MATCH THE HOLE PATTERN OF THE PLATE. $\frac{1}{2}$ " x 2" BOLTS SHALL THEN BE INSTALLED WITH A FLAT WASHER, LOCK WASHER, AND A NUT IN EACH OF THE HOLES. THE POST CAP MAY BE WELDED TO THE STEEL BEAM IN ACCORDANCE WITH AWS D1.1-92 AS AN ALTERNATIVE, AND WOULD NEED TO BE INSPECTED BY AN AWS-CERTIFIED INSPECTOR.
- 33. WHEN MECHANICAL EQUIPMENT IS LOCATED IN AN ENCLOSED ROOM, THERE SHALL BE (2) 14"x12" VENTS LOCATED IN A WALL COMMON WITH ADDITIONAL LIVING AREA. ONE VENT SHALL BE LOCATED SUCH THAT THE BOTTOM OF THE VENT BEGINS 12" FROM THE FLOOR AND THE OTHER VENT SHALL BE LOCATED SUCH THAT THE TOP OF THE VENT BEGINS 12" FROM THE CEILING.
- 34. ALL ROOF SHEATHING SHALL BE $\frac{7}{16}$ " OSB WITH 8d COMMON NAILS @ 6" O.C. AT PANEL EDGES AND @ 12" O.C. IN FIELD

- 35. GLAZING IN HAZARDOUS LOCATIONS AS IDENTIFIED IN IRC SECTION R308.4 SHALL BE OF APPROVED SAFETY GLAZING MATERIALS. GLASS IN STORM DOORS, INDIVIDUAL FIXED OR OPENABLE PANELS ADJACENT TO A DOOR WHERE THE NEAREST VERTICAL EDGE IS WITHIN A 2'-0" ARC OF THE DOOR IN A CLOSED POSITION AND FOR WHICH THE BOTTOM EDGE IS WITHIN 5'-0" OF THE FLOOR, WALLS ENCLOSING STAIRWAYS AND LANDINGS WHERE THE GLAZING IS WITHIN 5'-0" OF THE TOP OR BOTTOM OF THE STAIR, ENCLOSURES FOR SPAS, TUBS, SHOWERS, AND WHIRLPOOLS, GLAZING IN FIXED OR OPENABLE PANELS EXCEEDING NINE SQUARE FEET AND FOR WHICH THE BOTTOM EDGE IS LESS THAN 1'-6" ABOVE THE FLOOR OR WALKING SURFACE WITHIN 3'-0"
- ALL OPERABLE WINDOWS SHALL HAVE FALL PROTECTION PER IRC SECTION R612.2

37. ENCLOSED ATTICS SHALL HAVE CROSS VENTILATION FOR EACH SEPARATE SPACE BY VENTILATING OPENINGS PROTECTED AGAINST THE ENTRANCE OF RAIN OR SNOW. VENTILATING OPENINGS SHALL BE PROVIDED WITH CORROSION-RESISTANT WIRE MESH, WITH 1/8" TO 1/2" OPENINGS. THE TOTAL FREE VENTILATING AREA SHALL NOT BE LESS THAN χ_{50} OF THE AREA OF SPACE VENTILATED, EXCEPT WHERE THE VENTILATORS ARE LOCATED IN THE UPPER PORTION OF THE SPACE TO BE VENTILATED - THE REQUIRED AREA MAY BE REDUCED TO 1/300.

EMERGENCY EGRESS

- 38. PROVIDE A MINIMUM OF ONE WINDOW FOR EACH BEDROOM THAT HAS A MINIMUM OPENABLE AREA OF 5.7 SQUARE FEET WITH A MINIMUM OPENABLE HEIGHT OF 2'-0" AND A MINIMUM WIDTH OF 1'-9". IN ADDITION, THE OPENABLE
- PORTION OF EGRESS WINDOWS SHALL NOT EXCEED 3'-8" ABOVE THE ADJOINING FLOOR OR PERMANENT STEP. 39. PROVIDE SMOKE ALARMS IN EACH SLEEPING ROOM, OUTSIDE OF EACH SLEEPING AREA AND ON EACH FLOOR, INCLUDING BASEMENT (IF APPLICABLE). ALARMS SHALL BE HARDWIRED TOGETHER SO THAT THE ACTIVATION OF ONE SMOKE ALARM WILL ACTIVATE ALL SMOKE ALARMS IN THE DWELLING. PROVIDE CARBON MONOXIDE DETECTORS OUTSIDE EACH SLEEPING AREA.

MASONRY VENEER

- 40. MASONRY VENEER SHALL BE ANCHORED TO THE SUPPORTING WALL STUDS WITH CORROSION-RESISTANT METAL TIES EMBEDDED IN MORTAR OR GROUT AND EXTENDING INTO THE VENEER A MINIMUM OF 1½", WITH NOT LESS THAN 5/8" MORTAR OR GROUT COVER TO OUTSIDE FACE
- 41. VENEER TIES, IF STRAND WIRE, SHALL NOT BE LESS IN THICKNESS THAN NO. 9 U.S. GAGE WIRE AND SHALL HAVE A HOOK EMBEDDED IN THE MORTAR JOINT, OR IF SHEET METAL, SHALL BE NOT LESS THAN NO. 22 U.S. GAGE BY 1/8"
- EACH TIE SHALL SUPPORT NOT MORE THAN 2.67 SQUARE FEET OF WALL AREA AND SHALL BE SPACED NOT MORE THAN 32 INCHES ON CENTER HORIZONTALLY AND 24 INCHES ON CENTER VERTICALLY.
- VENEER TIES AROUND WALL OPENINGS: ADDITIONAL METAL TIES SHALL BE PROVIDED AROUND ALL WALL OPENINGS GREATER THAN 16 INCHES IN EITHER DIMENSION. METAL TIES AROUND THE PERIMETER OF OPENINGS SHALL BE SPACED NOT MORE THAN 3 FEET ON CENTER AND PLACED WITHIN 12 INCHES OF THE WALL OPENING.

- 44. DOOR(S) BETWEEN THE GARAGE AND DWELLING SHALL BE MINIMUM 1%" SOLID CORE OR HONEY-COMBED STEEL DOOR WITH 20-MINUTE FIRE RATING EQUIPPED WITH A SELF-CLOSING DEVICE
- 45. VEHICLE DOORS AND FRAMES SHALL BE DESIGNED AND INSTALLED TO MEET THE 115-MPH 3-SECOND GUST LOADING PER DASMA 108 AND ASTM E 330-96 PER IRC 2018

- THE GARAGE SHALL BE SEPARATED FROM THE DWELLING AND ITS ATTIC AREAS BY MINIMUM 5/4" GYP. BOARD APPLIED TO THE GARAGE SIDE OF FRAMING. WHERE HABITABLE SPACE OCCURS ABOVE THE GARAGE. THE GARAGE CEILING ASSEMBLY SHALL BE PROTECTED WITH A MINIMUM 3/8" TYPE X GYP. BOARD. WHERE A FLOOR/CEILING SPACE IS PROVIDED ABOVE THE GARAGE COLUMNS AND BEAMS SUPPORTING THE SEPARATION SHALL ALSO BE PROTECTED WITH %" GYP. BOARD.
- GARAGE DOOR H-FRAME FOR THE ATTACHMENT OF THE TRACK AND COUNTER BALANCE SHALL CONSIST OF THE FOLLOWING: 2x6 VERTICAL JAMBS RUNNING FROM FLOOR TO CEILING AND SHALL BE FASTENED WITH 2%"" x 0.120" NAILS AT 7" O.C. STAGGERED WITH (7) 31/4" x 0.120" NAILS THROUGH THE JAMB INTO THE HEADER. MINIMUM 2x8 HEADER FOR ATTACHMENT OF COUNTER BALANCE SYSTEM.

DESIGN LOADING (PER TABLE R301.5)

GARAGE NOTES (CONTINUED)

MINIMUM UNIFORMLY DISTRIBUTED LIVE LOADS (PSF)							
USE	LIVE LOAD	DEAD LOAD					
UNINHABITABLE ATTICS WITHOUT STORAGE	10	10					
UNINHABITABLE ATTICS WITH LIMITED STORAGE	20	10					
HABITABLE ATTICS AND ATTICS SERVED WITH FIXED STAIRS	30	10					
BALCONIES (EXTERIOR) AND DECKS	40	10 ^d					
FIRE ESCAPES	40	10					
GUARDRAILS AND HANDRAILS ^a	200 ^c	-					
GUARDRAIL IN-FILL COMPONENTS ^b	50 ^c	-					
PASSENGER VEHICLE GARAGES	50	DEPENDENT UPON SLAB CONSTRUCTION					
ROOMS OTHER THAN SLEEPING ROOM	40	10 ^d					
SLEEPING ROOM	30	10 ^d					
STAIRS	40	10 ^d					

a. A single concentrated load applied in any direction at any point along the top.

b. Guard in-fill components (all those except the handrail), ballusters and panel fillers shall be designed to withstand a horizontally applied normal load of 50 pounds on an area equal to one square foot. This load need not be assumed to act concurrently with any other live load requirement.

c. Glazing used in handrail assemblies and guards shall be designed with a safety factor of 4. The safety factor shall be applied to each of the concentrated loads applied to the top of the rail, and to the load on the infill components. These loads shall be determined independently of one another, and loads are assumed not to occur with any other live load.

d. An additional dead loading of 10 psf shall be applied where thinset tile floor is to be installed. An additional dead loading of 50 psf shall be applied where mudset tile floor is to be installed

INSULATION/EFFICIENCY

- BUILDING ENVELOPE INSULATION SHALL COMPLY WITH IRC TABLE N1102 1.1 OR THE 2012 IECC (SEE SHEET S3.1 FOR FRAMING DETAILS AND TABLES ON THIS SHEET FOR MORE INFORMATION)
- CATHEDRAL -VAULTED CEILING FRAMING SHALL BE FRAMED WITH A MINIMUM INSULATION VALUE OF R-38. IF VAULTED RAFTERS DO NOT PROVIDE REQUIRED DEPTH TO ACHIEVE R-38 INSULATION BUILDER SHALL FUR DOWN RAFTERS PER DETAILS PROVIDED ON

INSULATION AND FENESTRATION REQUIRE	MINITE DV COMPONENT /TADLE NI4402 4 4
CLIMATE ZONE	4-A
FENESTRATION U-FACTOR	0.35
SKYLIGHT U-FACTOR	0.55
GLAZED FENSTRATION SHGC	0.40
CEILING R-VALUE	49
WOOD FRAME WALL R-VALUE	15
MASS WALL R-VALUE	8 / 13
FLOOR R-VALUE	19
BASEMENT WALL R-VALUE	10-CONTINUOUS OR 13-CAVITY
SLAB R-VALUE AND DEPTH	10 AT 2'-0"
CRAWL SPACE WALL R-VALUE	10-CONTINUOUS OR 13-CAVITY
DUCTWORK EXPOSED TO OUTSIDE AIR R-VALUE	8
DUCTWORK NOT EXPOSED TO OUTSIDE AIR R-VALUE	6
CATHEDRAL VAULTED CEILING R-VALUE	38

DUCT SEALING

N1103.2.2 (R403.2.2) SEALING (MANDATORY). DUCTS, AIR HANDLERS, AND FILTER BOXES SHALL BE SEALED. JOINTS AND SEAMS SHALL COMPLY WITH SECTION M1601.4.1 OF 2018 IRC **EXCEPTIONS:**

- AIR-IMPERMEABLE SPRAY FOAM PRODUCTS SHALL BE PERMITTED TO BE APPLIED WITHOUT ADDITIONAL JOINT SEALS
- WHERE A DUCT CONNECTION IS MADE THAT IS PARTIALLY INACCESSIBLE, THREE SCREWS OR RIVETS SHALL BE EQUALLY SPACED ON THE EXPOSED PORTION OF THE JOINT SO AS TO PREVENT A HINGE EFFECT.
- CONTINUOUSLY WELDED AND LOCKING-TYPE LONGITUDINAL JOINTS AND SEAMS IN DUCTS OPERATING AT STATIC PRESSURES LESS THAN 2 INCHES OF WATER COLUMN PRESSURE CLASSIFICATION SHALL NOT REQUIRE ADDITIONAL CLOSURE SYSTEMS.

DUCT TIGHTNESS SHALL BE VERIFIED BY EITHER OF THE FOLLOWING:

- POST-CONSTRUCTION TEST: TOTAL LEAKAGE SHALL BE LESS THAN OR EQUAL TO 4 CFM PER 100 SQUARE FEET OF CONDITIONED FLOOR AREA WHEN TESTED AT A PRESSURE DIFFERENTIAL OF 0.1 INCHES W.G. ACROSS THE ENTIRE SYSTEM, INCLUDING THE MANUFACTURER'S AIR HANDLER ENCLOSURE. ALL REGISTER BOOTS SHALL BE TAPED OR OTHERWISE SEALED DURING THE TEST.
- ROUGH-IN TEST: TOTAL LEAKAGE SHALL BE LESS THAN OR EQUAL TO 4 CFM PER 100 SQUARE FEET OF CONDITIONED FLOOR AREA WHEN TESTED AT A PRESSURE DIFFERENTIAL OF 0.1 INCHES W.G. ACROSS THE SYSTEM, INCLUDING THE MANUFACTURER'S AIR HANDLER ENCLOSURE. ALL REGISTERS SHALL BE TAPED OR OTHERWISE SEALED DURING THE TEST. IF THE AIR HANDLER IS NOT INSTALLED AT THE TIME OF THE TEST, TOTAL LEAKAGE SHALL BE LESS THAN OR EQUAL TO 3 CFM PER 100 SQUARE FEET OF CONDITIONED FLOOR AREA

EXCEPTION: THE TOTAL LEAKAGE TEST IS NOT REQUIRED FOR DUCTS AND AIR HANDLERS LOCATED ENTIRELY WITHIN THE BUILDING THERMAL ENVELOPE.

MECHANICAL VENTILATION SYSTEM FAN EFFICACY					
FAN LOCATION	AIR FLOW RATE MINIMUM (CFM)	MINIMUM EFFICACY (CFM/WATT)	AIR FLOW RATE MAXIMUM (CFM)		
RANGE HOODS	ANY	2.8	ANY		
IN-LINE FAN	ANY	2.8	ANY		
BATHROOM, UTILITY ROOM	10	1.4	90		
BATHROOM, UTILITY ROOM	90	2.8	ANY		



AGLE CREEK ICKORY LANE MISSOUR SUMMIT, 38, SW LOT 7 2362 $\overline{\mathbb{H}}$ TITLE:

VIEWPOINT



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		CONSTRU	CTION

RESIDENTIAL SEISMIC & WIND ANALYSIS

				INPUT
DETERMINE WEIGHT OF HOUSE:				CALCULATED VALUE
LOCATION		DEAD LOAD (psf)	AREA (ft ²)	WEIGHT (lbs.)
ROOF		10	3039	30390
CEILING		10	3039	30390
SECOND FLOOR		10	1221	12210
FIRST FLOOR		10	3039	30390
	WALL LENGTH (ft)	WALL HEIGHT (ft)	WALL UNIT WT. (psf)	WEIGHT (lbs)
SECOND FLOOR EXT. WALL DL	169.34	9	9	13716.54
FIRST FLOOR EXT. WALL DL	193.34	10	10	19334
		DEAD LOAD (psf)	AREA (ft2)	WEIGHT (lbs)
SECOND FLOOR INT. PARTITION WALL DL		6	1221	7326
FIRST FLOOR INT. PARTITION WALL DL		6	3039	18234

_	PROJECTED AREAS (WIND DESIGN PER 115 MPH 3-SECOND GUST, EXPOSURE C AND MEAN ROOF HEIGHT <= 30 FT ASSUMED)						
	FRONT-TO-BACK			SIDE-TO-SIDE			
	AREA	LOAD			AREA	LOAD	
SLOPED ROOF	274	2305		SLOPED ROOF	204	1736	
VERT. ROOF	26	320	CUMULATIVE	VERT. ROOF	8	99	CUMULATIVE
2ND	415	5202	7826	2ND	431.7	5390	7225
1ST	566.5	6963	14789	1ST	496.87	6177	13402
			PRESSURE (PSI	F) - PER ASCE CH. 6			
SLOPED ROOF ZONE B S WALL/VERT. ROOF ZONE A 1 MEAN ROOF HT., h 24			9.7	ZONE C	11.3	2a (FIG. 28.6-1, ASCE7)	
		14.2	ZONE D	7.7	9.034		
				-	-		

a) If there is a walkout wall to be sheathed, determine tributary wind area and enter here. If no walkout, enter 0 for area.

 $q_{z10_ASD} = 0.6 q_{z10} \quad \text{(Design Velocity Pressure for ASD analysis under ASCE7-10 and IRC/IBC 2012)}$ q_{z10} =0.00256 $K_zK_{zt}K_dV^2$ (ASCE7-10 Velocity Pressure)

2ND FLOOR TRIBUTARY WEIGHT 1ST FLOOR TRIBUTARY WEIGHT S_S (SITE GROUND MOTION - %g - FROM ASCE7 SEISMIC MAP) F_a (from ASCE7 Table 11.4-1)

 S_{DS} (= 2/3 * S_{S} * F_{a})

R (from ASCE7 Table 12.2-1)

67638.27 103699.54 12.0% 1.6 0.128 6.5

		OLIGINIO GITERIA			
LOCATION		From	n ASCE7 (Eq. 12.8-1):	V (= 1.2 * S _{DS} * W	R) (lbs.)
2ND FLOOR				1598	
1ST FLOOR				2450	
Sheathing Location	Min. Sheathing Schedule	Fastening Schedule	Allowab	ole Shear (#/LF)	Code Reference
Exterior (Ontion #1)	7/16" APA Rated Plywood/OSB	1-1/2" 16ga. Staples w/ 1" penetration@ 6" OC Edges, 6" OC Field		155	per IBC, Table

Sheathing Location	Min. Sheathing Schedule	Fastening Schedule	Allowable Shear (#/LF)	Code Reference
Exterior (Option #1)	7/16" APA Rated Plywood/OSB	1-1/2" 16ga. Staples w/ 1" penetration@ 6" OC Edges, 6" OC Field For 24" stud spacing, 12" OC Field For 16" stud spacing	155	per IBC, Table 2306.3(1)
Exterior (Option #2)	7/16" APA Rated Plywood/OSB	1-1/2" 16ga. Staples w/ 1" penetration@ 4" OC Edges, 6" OC Field For 24" stud spacing, 12" OC Field For 16" stud spacing	230	per IBC, Table 2306.3(1)
Exterior (Option #3)	7/16" APA Rated Plywood/OSB	1-1/2" 16ga. Staples w/ 1" penetration@ 3" OC Edges, 6" OC Field For 24" stud spacing, 12" OC Field For 16" stud spacing	310	per IBC, Table 2306.3(1)
Exterior (Option #4)	7/16" APA Rated Plywood/OSB or shiplap panel sheathing, or 3/8" shiplap panel sheathing with tighter nail spacing	8d Common Nails w/ 1-3/8" penetration @ 6" O.C. Edges, 12" O.C. Field for 7/16" APA-rated plywood/OSB or shiplap panel sheathing OR @ 4" O.C. Edges, 12" O.C. Field for 3/8" shiplap panel sheathing	220	AF&PA SDPW Table 4.3A
Exterior (Option #5)	7/16" APA Rated Plywood/OSB or shiplap panel sheathing, or 3/8" shiplap panel sheathing with tighter nail spacing	8d Common Nails w/ 1-3/8" penetration @ 4" O.C. Edges, 12" O.C. Field for 7/16" APA-rated plywood/OSB or shiplap panel sheathing OR @ 3" O.C. Edges, 12" O.C. Field for 3/8" shiplap panel sheathing	320	AF&PA SDPW Table 4.3A
Exterior (Option #6)	7/16" APA Rated Plywood/OSB or shiplap panel sheathing, or 3/8" shiplap panel sheathing with tighter nail spacing and double studs at each panel edge	8d Common Nails w/ 1-3/8" penetration @ 3" O.C. Edges, 12" O.C. Field	410	AF&PA SDPW Table 4.3A
Interior	1/2" Gypsum Board	No. 6- 1 ¹ / ₄ " Type W or S Screws @ 8" O.C. Edges, 12" O.C. Field	60	per IBC, Table 2306.4.4
Interior	16 Ga. Simpson/USP Type WB Steel X-Brace (or equal)	(3) 16d @ end studs & (1) 8d @ intermediate studs (per manufacturer specifications - see detail on sheet S3)	325	

EXTERIOR SHEATHING OPTION FOR SECOND FLOOR	4
EXTERIOR SHEATHING OPTION FOR FIRST FLOOR	4

		_	
WIDTH OF 1ST STORY (FT.)	51.5		WIDTH OF 2ND S
DEPTH OF 1ST STORY (FT.)	45.17		DEPTH OF 2ND S
BACK WALL OF GARAGE (FT.)	20	<u>'</u>	
GAR WALL 1=F-B 2=S-S	2	1	

WIDTH OF 1ST STORY (FT.)	51.5	WIDTH OF 2ND STORY (FT.)	41.5
DEPTH OF 1ST STORY (FT.)	45.17	DEPTH OF 2ND STORY (FT.)	43.17
ACK WALL OF GARAGE (FT.)	20		
GAR. WALL: 1=F-B, 2=S-S	2		

EXTERIOR STRUCTURAL WALL LENGTHS (ft.) & RESISTANCES											
		SE	ISMIC		WIND						
	FRONT-TO-BACK	RESISTANCE (lbs.)	SIDE-TO-SIDE	RESISTANCE (lbs.)	FRONT-TO-BACK	RESISTANCE (lbs.)	SIDE-TO-SIDE	RESISTANCE (lbs.)			
2ND FLOOR	50	14000	44	12320	50	19600	44	17248			
1ST FLOOR	71 19880 35 9800 71 27832 35										

	ADDITIONAL RESIS	TANCE REQUIRED
	SEISMIC	WIND
2ND FLOOR FRONT-TO-BACK	0	0
2ND FLOOR SIDE-TO-SIDE	0	0
1ST FLOOR FRONT-TO-BACK	0	0
1ST FLOOR SIDE-TO-SIDE	0	0

Anchor Bolt Spacing	(in.)		16d Nail Spacing req'd at b	oottom plate (in.)
diameter (in.)	0.5	lſ	2nd Floor F-B	31
Shear value (per NDS) 944			2nd Floor S-S	39
Spacing F-B (inches)	110.7		1st Floor F-B	17
spacing S-S (inches)	139.3		1st Floor S-S	21

DECIDITANCE REQUIRED IN ADDITION TO DECIDITANCE PROVIDED BY EXTERIOR WALLOW									
RESISTANCE REQUIRED IN ADDITION TO RESISTANCE PROVIDED BY EXTERIOR WALLS**									
ADDITIONAL RESISTANCE									
2ND FLOOR FRONT-TO-BACK	0					0	YES		
2ND FLOOR SIDE-TO-SIDE	0					0	YES		
1ST FLOOR FRONT-TO-BACK	0					0	YES		
1ST FLOOR SIDE-TO-SIDE	0					Ö	YES		

**NOTES: 1) SEE ATTACHED CALCULATIONS FOR PORTAL FRAME OR PERFORATED SHEAR WALL RESISTANCE CAPACITIES (IF APPLICABLE),

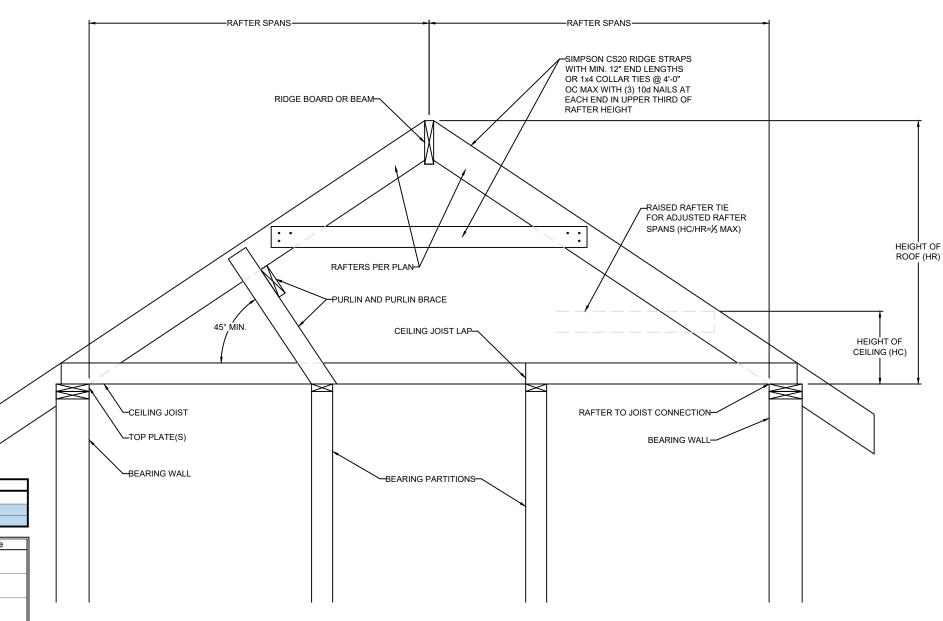
2) SEE SHEET ST FOR INTERIOR STEEL X-BRACE INSTALLATION, 3) INTERIOR WALLS SHEATHED WITH OSB SHALL BE ATTACHED WITH SAME STAPLE/NAILING
PATTERN AS EXTERIOR OSB ON SAME FLOOR (SEE TABLE ABOVE) AND ARE ONLY APPLICABLE FOR FULL-HEIGHT SECTIONS OF 2'-8" OR LONGER
ALL LATERAL BRACING ACHIEVED AT EXTERIOR WALLS AND WALLS DIRECTLY ON FOUNDATIONS; THEREFORE, NO INTERIOR BRACING PER 2012 IRC SECTION R502.2.1 IS REQUIRE

ALL LATERAL BRACING ACHIEVED AT EXTERIOR WALLS AND WALLS DIRECTLY ON FOUNDATIONS; THEREFORE, NO INTERIOR BRACING PER 2012 IRC SECTION 8502.2:1 IS REQUIRE									
WIND UPLIFT ANALYSIS									
	X/12 DEGREES								
ROOF PITCH (MAX)	8	33.7	PITCH OF 6 OR LESS:	EOH -13.3, E -7.2, G -5.2					
	ASCE 7								
	LENGTH (FT.) PRESSURE (PSF) LINEAL FT. OF OH UPLIFT PER FT* (LBS)								
OVERHANG	1	-1.08	195.34	-1.08					
	TOTAL AREA (FT ²)	ZONE E AREA (FT ²)	ZONE G AREA (FT ²)	PRESSURE ZN. E (PSF)	PRESSURE ZN. G (PSF)	TOTAL FORCE (LBS)	FORCE PER LINEAL FT @ PERIMETER (LBS)		
MAIN ROOF**	2326.255	1203.364936	1122.890064	-1.08	-0.36	-1704	-8.8		
*ALONG PERIMETER	ALONG PERIMETER TOTAL UPLIFT PER LINEAL FOOT ALONG EXTERIOR (POUNDS) -9.9 UPLIFT OK								
**INSIDE EXTERIOR V	VALLS	RESISTANCE DUE TO DEAD	WEIGHT & (3) 10d TOENAILS	3	251.6				

NOTE FOR CONSTRUCTION:
THE CONTINUOUS STRUCTURAL PANEL SHEATHING BRACING METHOD REQUIRES USE OF THE ABOVE TABLE FOR SHEATHING OF THE ENTIRE STRUCTURE. IN ADDITION, FRAMING MEMBERS SHALL BE @ 16" O.C. MAX., UNBLOCKED, AND W/ SHEATHING APPLIED DIRECTLY TO FRAMING MEMBERS

NOTE FOR DESIGN:
ALL WALLS USED IN THE CALCULATION OF THE RESISTANCE FOR THIS STRUCTURE SHALL HAVE A MINIMUM UNINTERRUPTED HEIGHT OF 8'-0" AND LENGTH OF 2'-8". ALLOWABLE RESISTANCES HAVE BEEN #/FT AND INCREASED BY 40% FOR WIND LOADS, PER VALUES IN 2012 IBC SECTION 2306 AND AF&PA SDPWS TABLE 4.3A. FOR EXAMPLE, 7/16" APA-RATED SHEATHING WITH 8d @ 6" & 12" HAS A SEISMIC SHEAR VALUE OF 24 A WIND SHEAR VALUE OF 335#/FT - 40% GREATER THAN THAT OF SEISMIC)

NOTE: SOIL SITE CLASS ASSUMED TO BE CLASS D. IF SITE CONDITIONS ARE DETERMINED TO BE CLASS E OR F, CONSULT ENGINEER BEFORE PROCEEDING



1 BRACED RAFTER CONSTRUCTION S1.1 SCALE: 1" = 1'-0" (18x24) OR 1½" = 1'-0" (24x36)



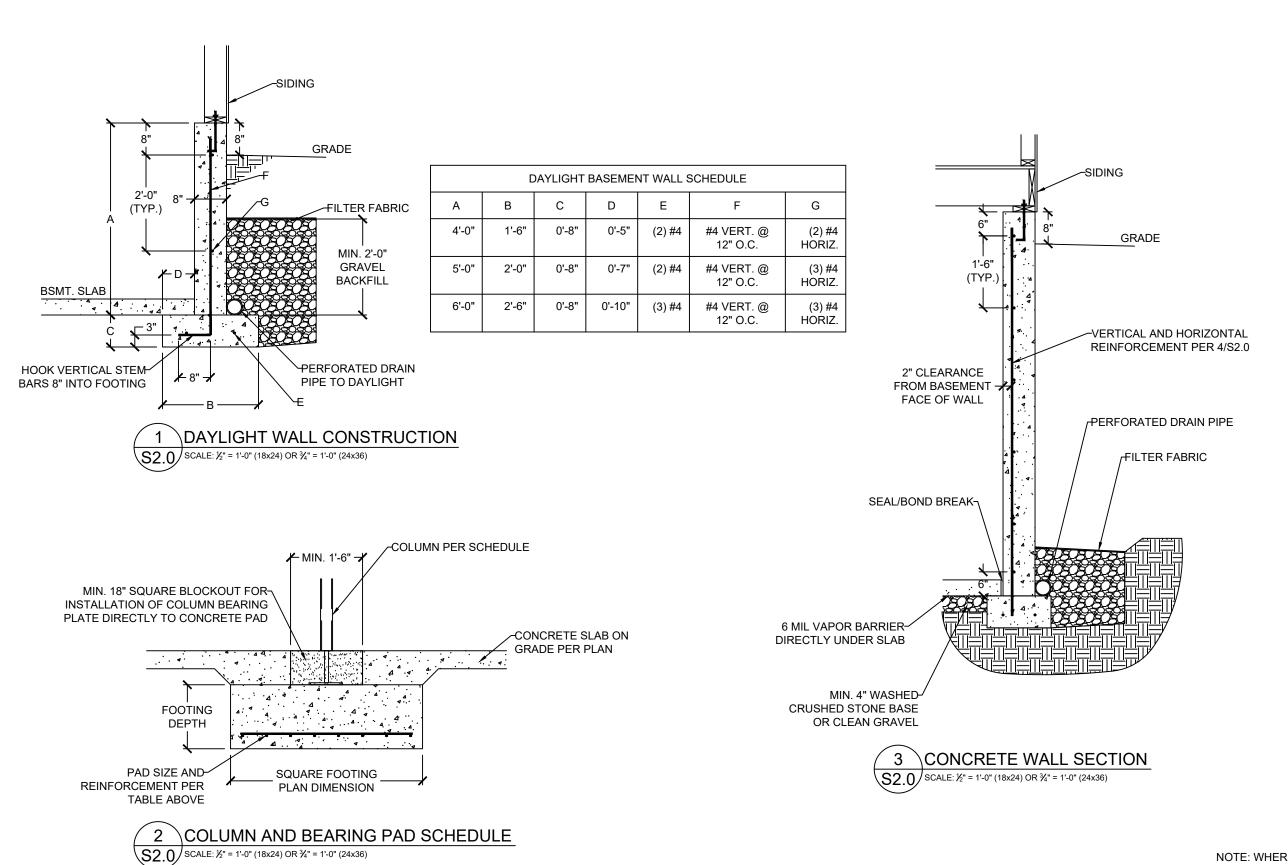
CLIENT: VIEWPOINT RESIDENTIAL DESIGN, LLC TITLE: LOT 738, EAGLE CREEK 2362 SW HICKORY LANE

SUMMIT, MISSOURI

DENNIS HEIER PE-2010001772

JOB

NO.	DATE	REVISION		BY
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S CA ENGIN	TRU LC NEER: DM 10. 338 : 04-16	ULATION CHECKED BY STREET CONSTRU	ON BYDM DM E FO	H H OR
S CA ENGIN	TRU LC NEER: DM 10. 338 : 04-16	ULATION CHECKED BY SAME DEPARTMENT OF THE PROPERTY OF THE PROP	ON DM DM	H H OR



INTO INTERSECTING WALL

4 4 4 4

-CONTINUOUS FOOTING

AND REBAR THROUGH

SOLID JUMP

MAX. 12" BLOCKOUT FOR

FORM PLACEMENT AND

TO EXTEND DRAIN TILE

5 \SOLID JUMP

S2.0/SCALE: $\frac{1}{2}$ " = 1'-0" (18x24) OR $\frac{3}{4}$ " = 1'-0" (24x36)

VERTICAL REINFORCEMENT SPACING CONCRETE STRENGTH/GRADE 8" THICK WALL 10" THICK WALL REINFORCEMENT (#4 BARS) 9' 9' 10' 8' 10' 8' 3.000 PSI/ GRADE 40 24 24 16 24 24 18 3,500 PSI/ GRADE 40 24 24 18 16 24 24 3,000 PSI/ GRADE 60 24 24 24 18 16 24 3,500 PSI/ GRADE 60 24 24 24 18 24 16 HORIZONTAL REINFORCEMENT - MINIMUM GRADE 40 STEEL ONE BAR 12" FROM TOP OF WALL; 6-#4 7-#4 7-#4 6-#4 7-#4 7-#4 MAX. SPACING 24" OC

FOOTNOTES:

1) WALL HEIGHT IS MEASURED FROM THE TOP OF THE WALL TO THE TOP OF THE FLOOR SLAB 2) VERTICAL REINFORCEMENT FOR CONCRETE WALLS THAT ARE NOT FULL HEIGHT, AND FOR REINFORCEMENT SPACING 24" OC, REINFORCEMENT MAY BE PLACED IN THE MIDDLE OF THE WALL. OTHER WALLS SHALL HAVE VERTICAL REINFORCEMENT AS FOLLOWS:

A) 8" WALL - MINIMUM 5" FROM THE OUTSIDE FACE

B) 10" WALL - MINIMUM 63/4" FROM THE OUTSIDE FACE C) EXTEND BARS TO WITHIN 8" OF THE TOP OF THE WALL

3) REINFORCEMENT CLEARANCES:

A) CONCRETE EXPOSED TO EARTH - MINIMUM 11/2"

B) NOT EXPOSED TO WEATHER (INTERIOR SIDE OF WALLS) -3/4"

C) CONCRETE EXPOSED TO WEATHER (TOP CLEARANCE IN GARAGE AND DRIVEWAY SLABS) - 1½"

4) HORIZONTAL REINFORCEMENT:

A) ONE BAR SHALL BE PLACED WITHIN 12" OF THE TOP OF THE WALL

B) OTHER BARS SHALL BE EQUALLY SPACED WITH SPACING NOT TO EXCEED 24" OC C) HORIZONTAL BARS SHOULD BE AS CLOSE TO THE TENSION FACE AS POSSIBLE (INTERIOR) AND BEHIND THE VERTICAL REINFORCEMENT (I.E. 2" TOWARD THE

D) SUPPLEMENTAL REINFORCEMENT AT CORNERS - PLACE (1) #4 BAR 48" LONG AT 45 DEGREE ANGLE AT CORNERS OF OPENINGS. PLACE REINFORCEMENT WITHIN 6" OF THE EDGE OF INSIDE CORNERS.

5) REINFORCEMENT SHALL BE LAPPED A MINIMUM 24" AT ENDS, SPLICES, AND AROUND CORNERS.

6) AT MASONRY LEDGES THE MINIMUM WALL THICKNESS SHALL BE 3\%". LEDGES SHALL NOT EXCEED A DEPTH OF MORE THAN 24" BELOW THE TOP OF THE WALL. FOR WALL THICKNESSES LESS THAN 4" PROVIDE #4 BARS AT MAX. 24" OC TO WITHIN 8" OF THE TOP

7) STRAIGHT WALLS MORE THAN 5' TALL AND MORE THAN 16 FEET LONG SHALL BE PROVIDED WITH EXTERIOR BRACED RETURN WALLS. WALL LENGTH SHALL BE MEASURED USING INSIDE THE SHORTEST DIMENSION BETWEEN INTERSECTING WALLS

8) WALL SHALL NOT BE BACKFILLED UNTIL FLOOR SYSTEM AND DIAPHRAGM ARE IN PLACE

\FOUNDATION WALL REINFORCEMENT TABLE





RESIDENTIAL

-SLAB PER PLAN, IF APPLICABLE

PER PLAN

-REBAR PER

5" INTO WALL

✓ PER PLAN

✓

CONCRETE GRADE BEAM

S2.0 SCALE: 1" = 1'-0" (18x24) OR $1\frac{1}{2}$ " = 1'-0" (24x36)

3" CLEAR (TYP.)

CLEAR-

(TYP.)

EAGLE CREEK HICKORY LANE

738, 2 SW

LOT 7 2362

TITLE:

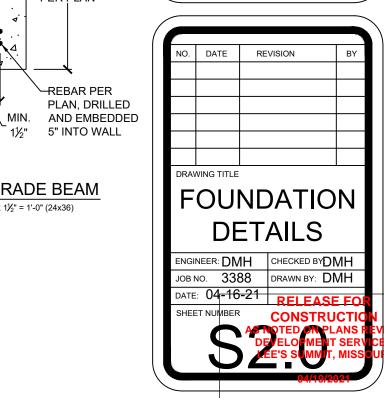
JOB

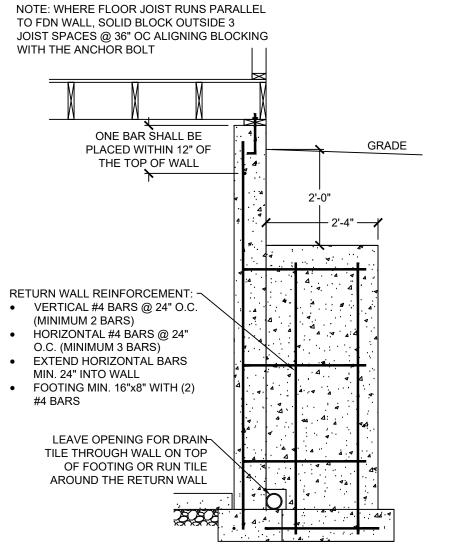
MISSOUR

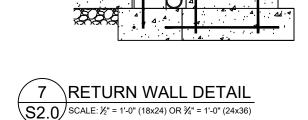
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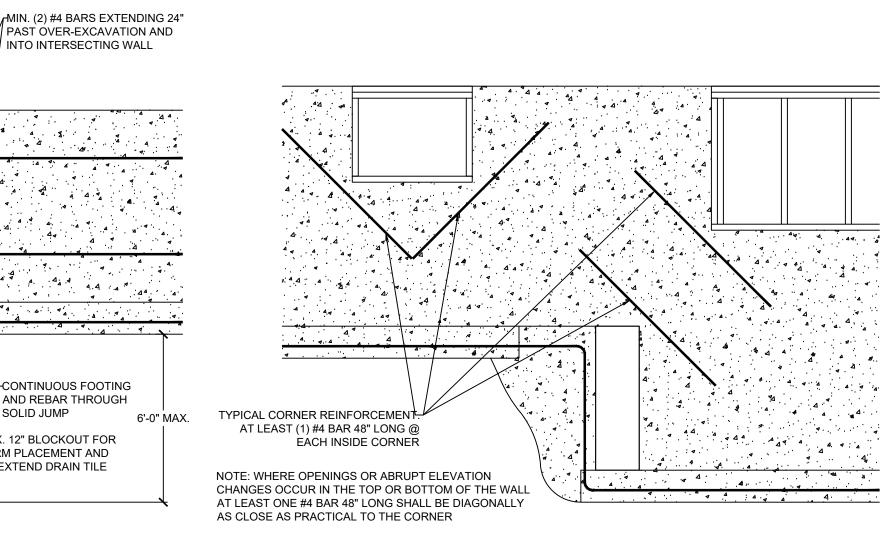
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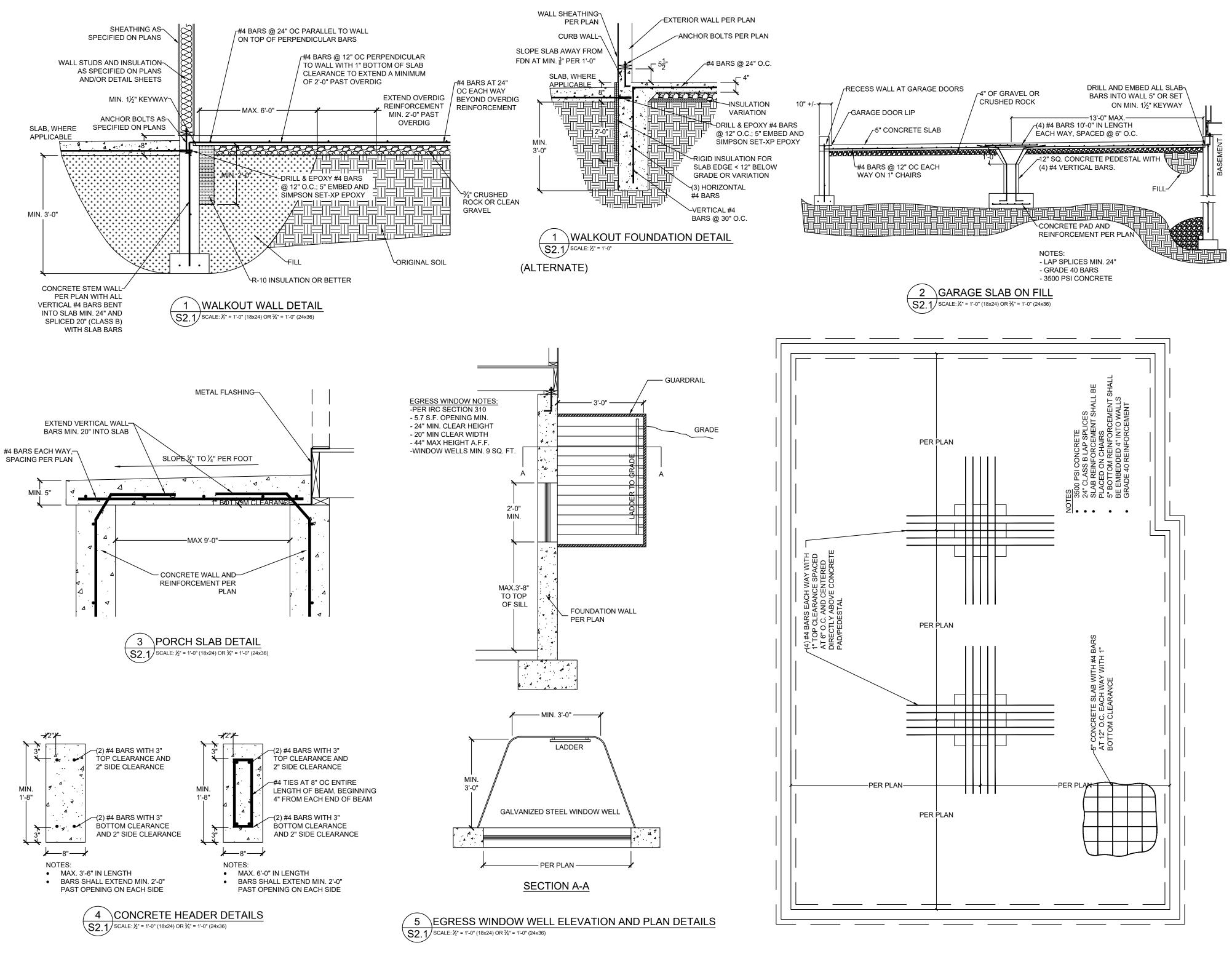






6 \REINFORCEMENT AT OPENING CORNERS \S2.0/AND STEP CORNERS @ INSIDE CORNERS

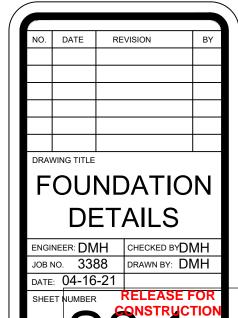
SCALE: ½" = 1'-0" (18x24) OR ¾" = 1'-0" (24x36)

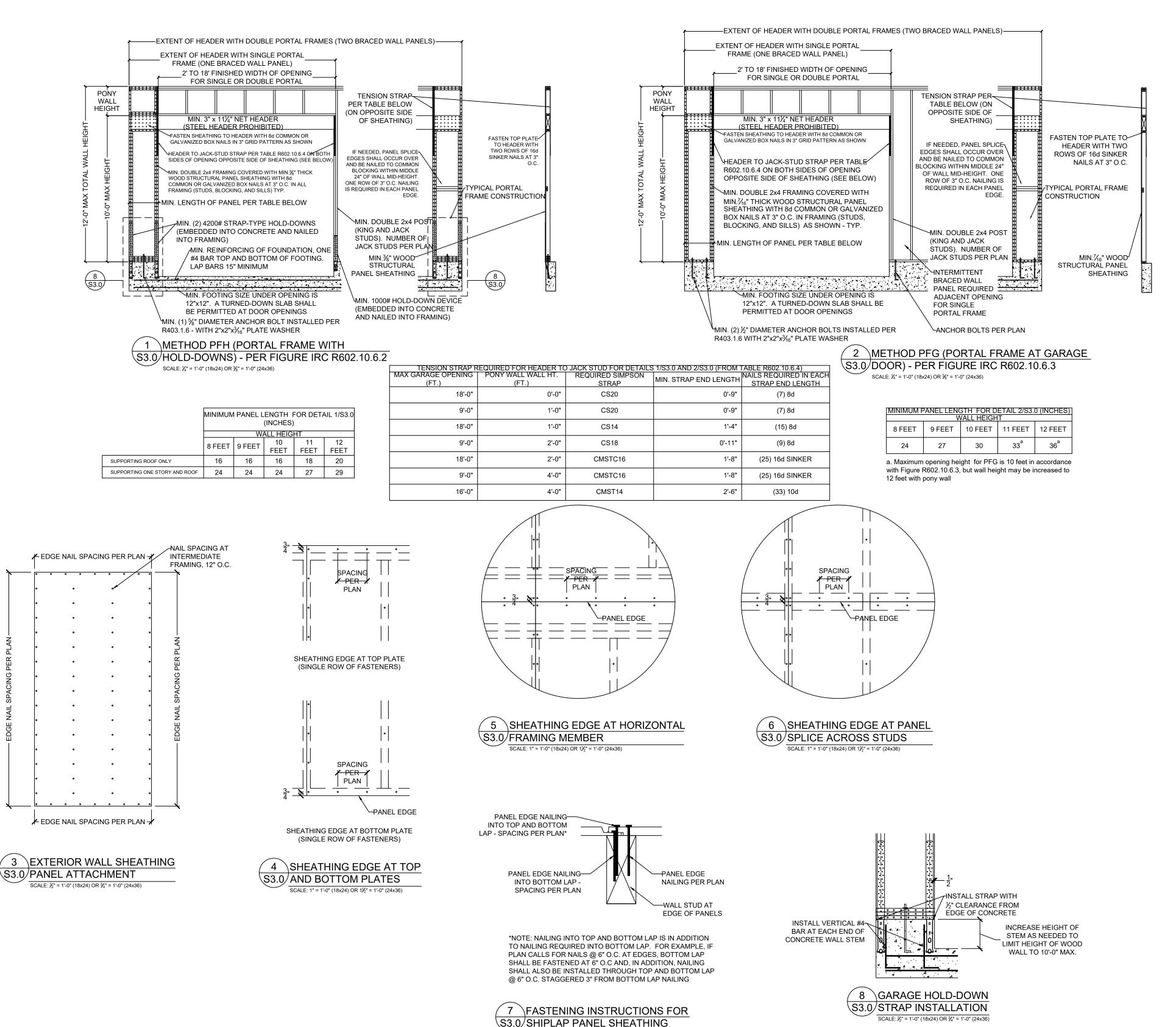




CLIENT: VIEWPOINT RESIDENTIAL DESIGN, LLC
JOB TITLE: LOT 738, EAGLE CREEK
2362 SW HICKORY LANE
LOCATION: LEE'S SUMMIT, MISSOURI







SCALE: 4" = 1'-0" (18x24) OR 6" = 1'-0" (24x36)



CLIENT: VIEWPOINT RESIDENTIAL DESIGN, LLC

JOB TITLE: LOT 738, EAGLE CREEK 2362 SW HICKORY LANE

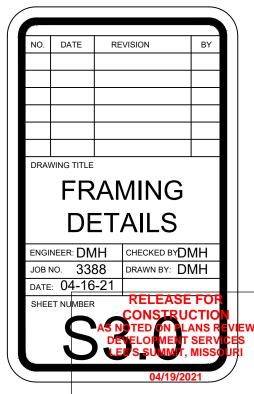
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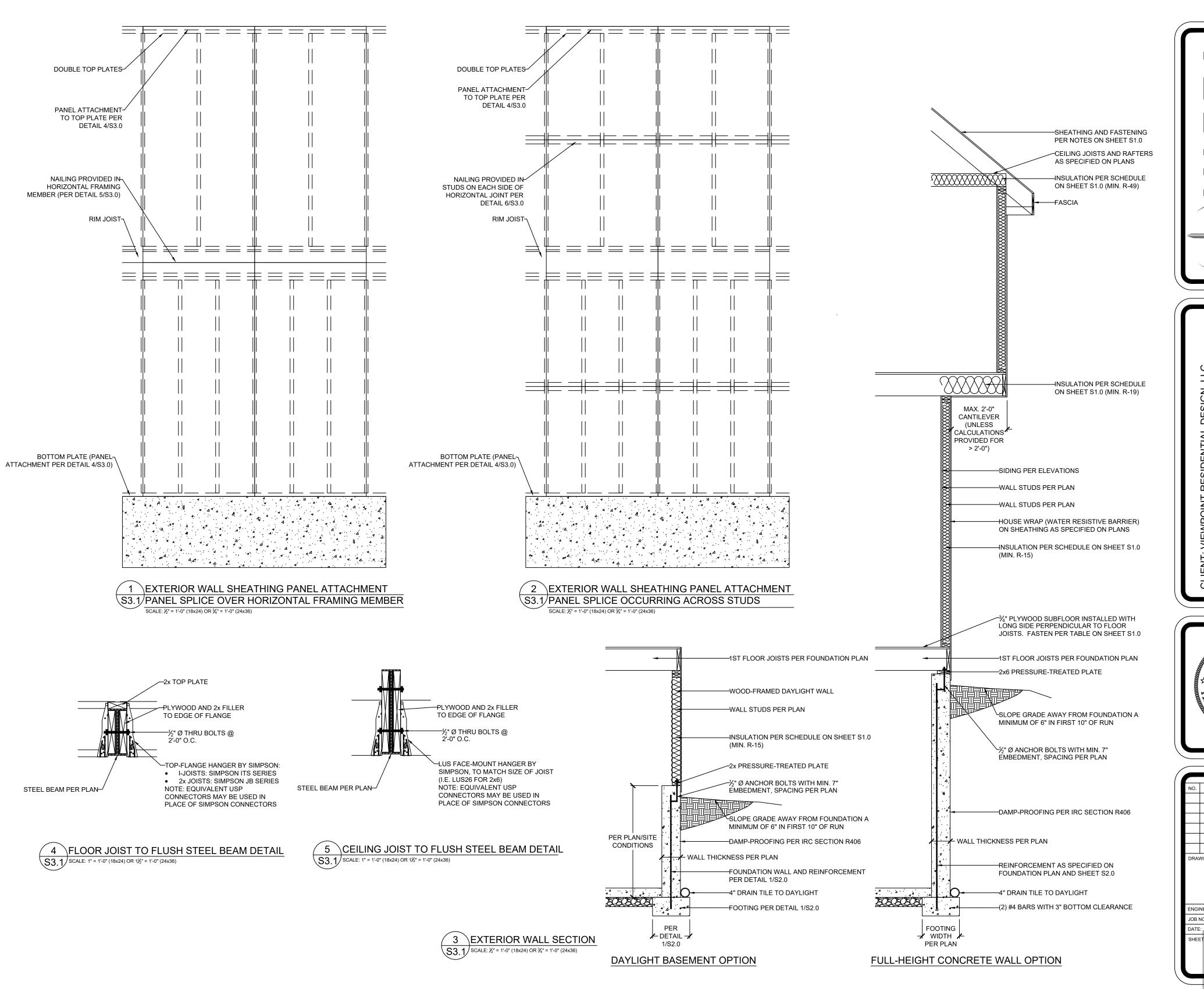
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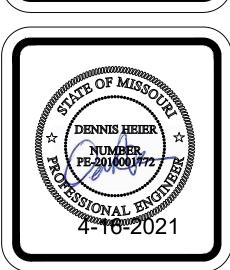


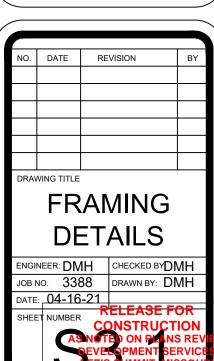


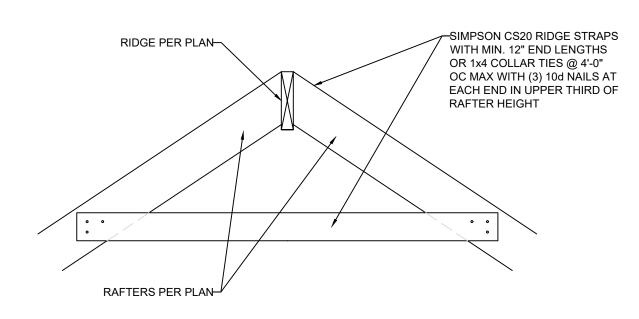


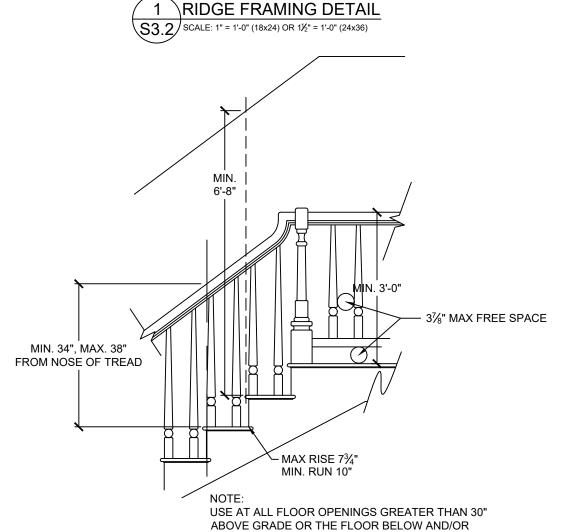


CLIENT: VIEWPOINT RESIDENTIAL DESIGN, LLC
JOB TITLE: LOT 738, EAGLE CREEK
2362 SW HICKORY LANE
LOCATION: LEE'S SUMMIT, MISSOURI





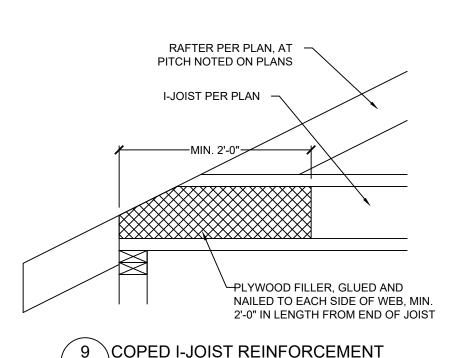




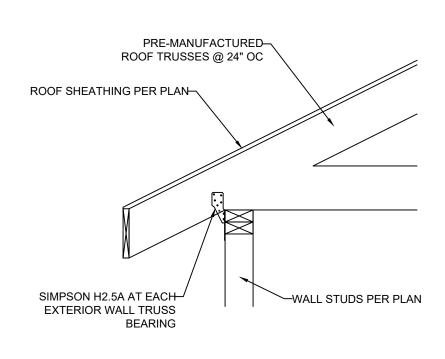
STAIRS WITH THREE OR MORE RISERS

4 \STAIR AND HANDRAIL/GUARDRAIL DETAIL

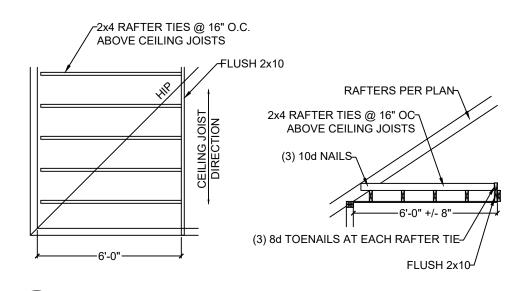
S3.2/SCALE: $\frac{1}{2}$ " = 1'-0" (18x24) OR $\frac{3}{4}$ " = 1'-0" (24x36)



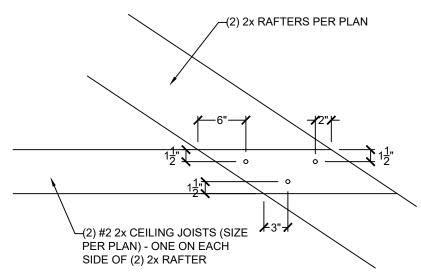
SCALE: 1" = 1'-0" (18x24) OR $1\frac{1}{2}$ " = 1'-0" (24x36)



2 TRUSS CONNECTION TO EXT. WALL BEARING S3.2 SCALE: 1" = 1'-0" (18x24) OR 1½" = 1'-0" (24x36)



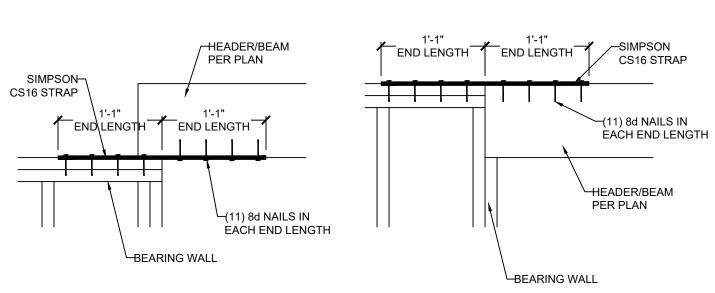
5 RAFTER TIES AT CEILING JOISTS PERP. TO RAFTERS S3.2 SCALE: ¼" = 1'-0" (18x24) OR ¾" = 1'-0" (24x36)

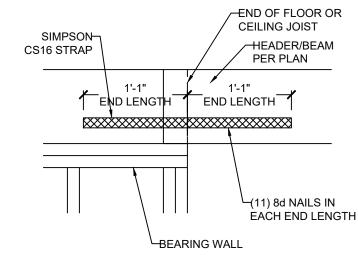


6 FIELD-CONSTRUCTED A-FRAME DETAIL S3.2 SCALE: 1" = 1'-0" (18x24) OR 1½" = 1'-0" (24x36)

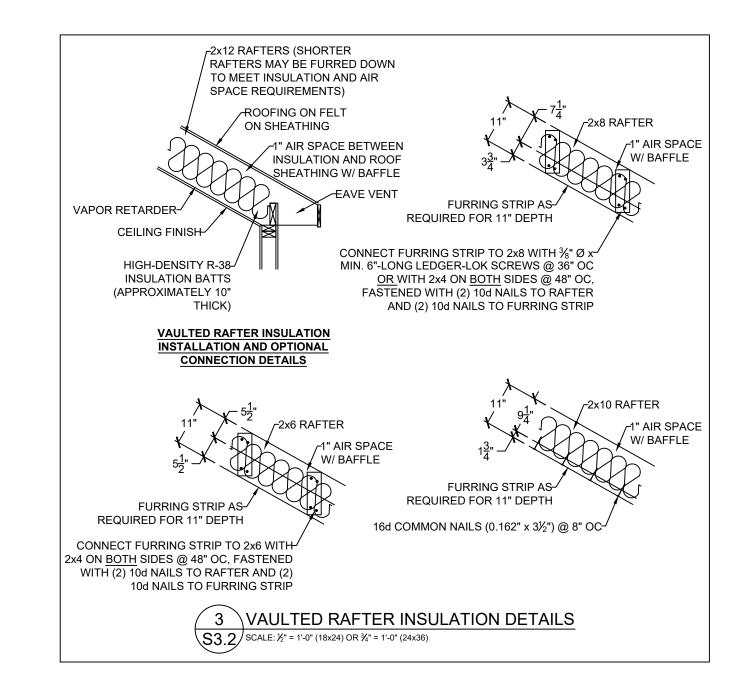
7 DETAIL S3.2 SCALE: 1" = 1'-0" (18x24) OR 1½" = 1'-0" (24x36)

NOT USED





10 HEADER/BEAM CONNECTION OPTIONS AT OUTDOOR/OPEN SPACE \$3.2 SCALE: 1" = 1'-0" (18x24) OR 1½" = 1'-0" (24x36)



LIEIOLIT (ET.)	SPACING (INCHES O.C.)					
HEIGHT (FT.)	24	16	12	8		
	SUPPORT	ING A ROOF	ONLY			
10 OR LESS	2x4	2x4	2x4	2x4		
12	2x6	2x4	2x4	2x4		
14	2x6	2x6	2x6	2x4		
16	2x6	2x6	2x6	2x4		
18	DR	2x6	2x6	2x6		
20	DR	DR	2x6	2x6		
SUPI	PORTING O	NE FLOOR	AND A ROO	F		
10 OR LESS	2x6	2x4	2x4	2x4		
12	2x6	2x6	2x6	2x4		
14	2x6	2x6	2x6	2x6		
16	DR	2x6	2x6	2x6		
18	DR	2x6	2x6	2x6		
20	DR	DR	2x6	2x6		
SUPP	ORTING TV	VO FLOORS	AND A ROC)F		
10 OR LESS	2x6	2x6	2x4	2x4		
12	2x6	2x6	2x6	2x6		
14	2x6	2x6	2x6	2x6		
16	DR	2x6	2x6	2x6		
18	DR	DR	2x6	2x6		
20	DR	DR	DR	2x6		

NOTES:

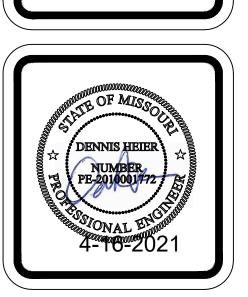
1) DR = DESIGN REQUIRED

2) UTILITY, STANDARD, STUD AND #3 GRADE LUMBER OF ANY SPECIES ARE NOT PERMITTED

3) THIS TABLE DOES NOT APPLY FOR STUDS SUPPORTING MEMBERS WITH A TRIB. LENGTH GREATER THAN 6'-0"

8 MAXIMUM ALLOWABLE LENGTH OF WOOD WALL STUDS (IRC TABLE 602.3.1)





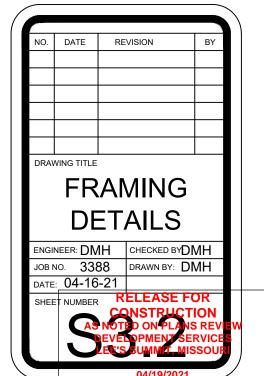
SUMMIT, MISSOURI

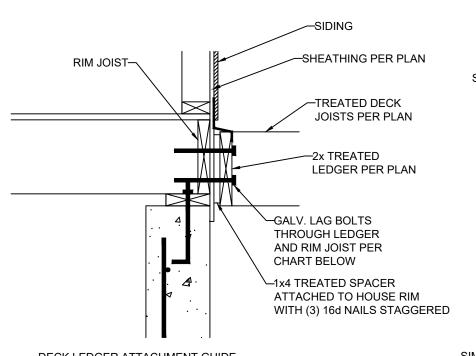
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TITLE:

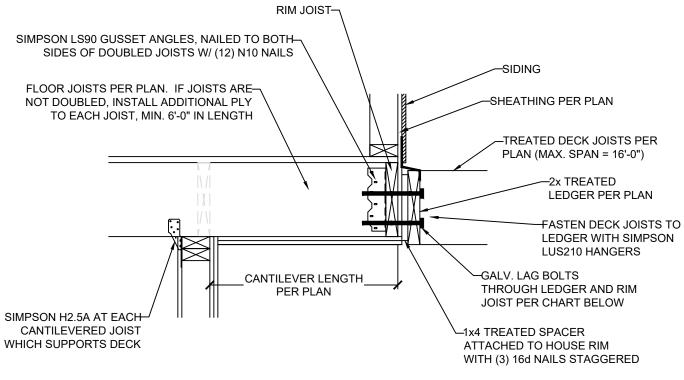
JOB

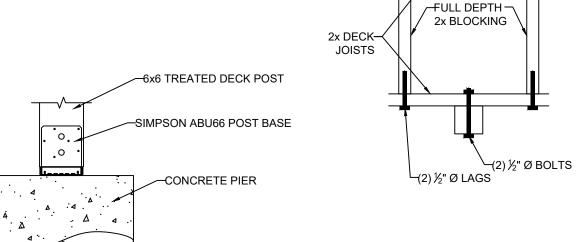


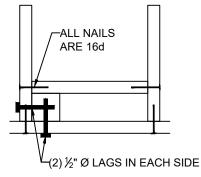


DECK LEDGER ATTACHMENT GUIDE

	DECK JOIST SPAN	½" Ø GALV. LAG OR ¾" Ø LEDGER-LOK SPACING
	10'-0" OR LESS	16" OC
	10'-0" - 13'-11"	12" OC OR @ 16" OC DOUBLED EVERY OTHER
	14'-0" - 18'-0"	8" OC OR @ 16" OC DOUBLED
-		



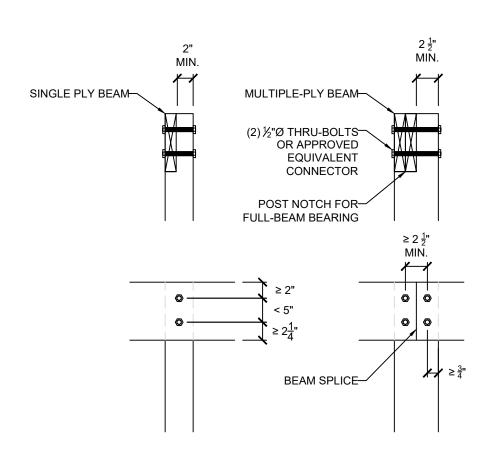




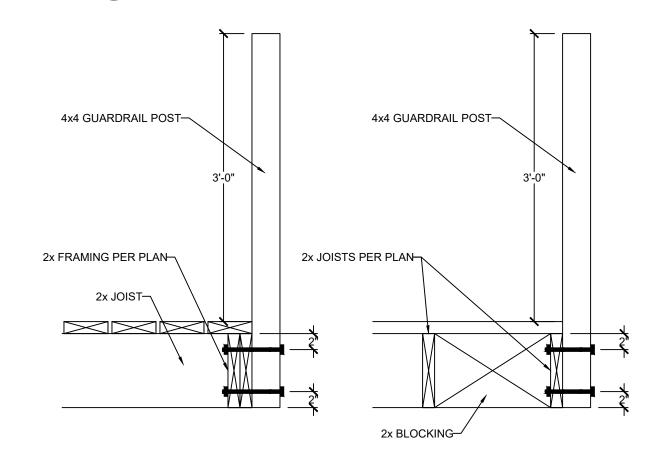
\DECK POST BASE \$3.3 SCALE: 1" = 1'-0" (18x24) OR 1½" = 1'-0" (24x36)

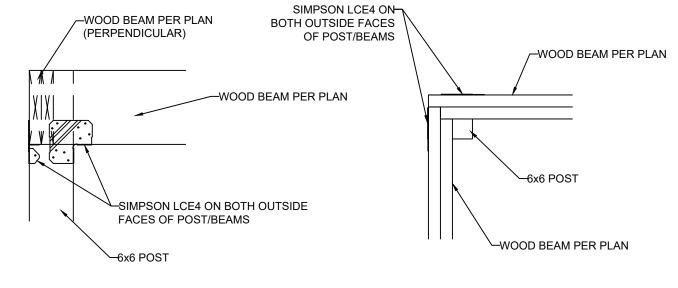
REINF. POST CONNECTIONS \$3.3 SCALE: 1" = 1'-0" (18x24) OR 1½" = 1'-0" (24x36)

LEDGER ATTACHMENT \$3.3\rightarrow\$SCALE: 1" = 1'-0" (18x24) OR 1\frac{1}{2}" = 1'-0" (24x36)







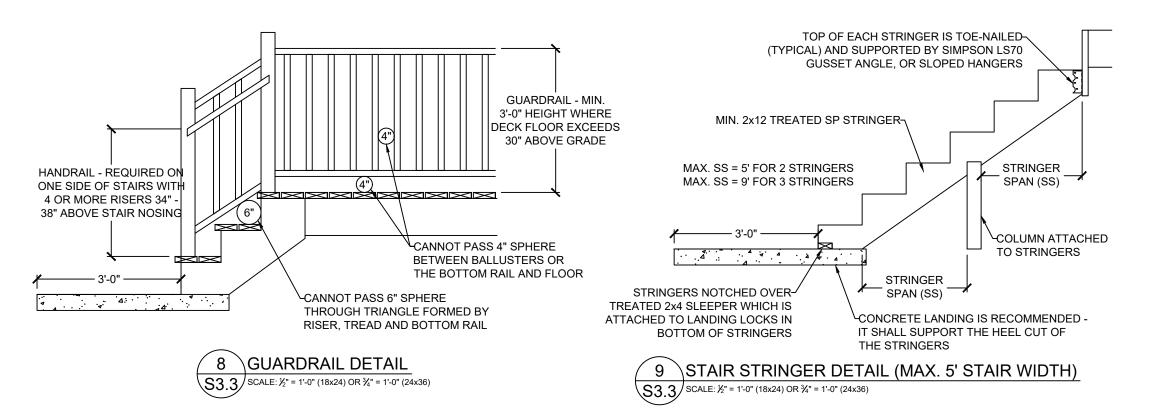


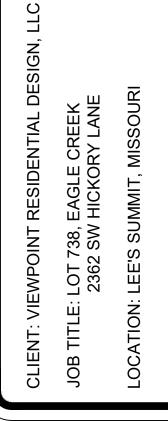
GUARDRAIL CONNECTION \$\S3.3\ SCALE: 1" = 1'-0" (18x24) OR $1\frac{1}{2}$ " = 1'-0" (24x36)



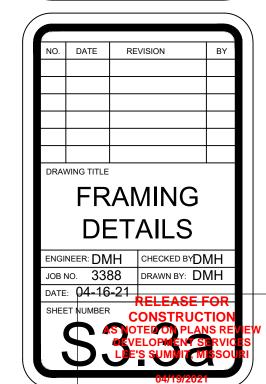
SCALE: 1" = 1'-0" (18x24) OR 1½" = 1'-0" (24x36)

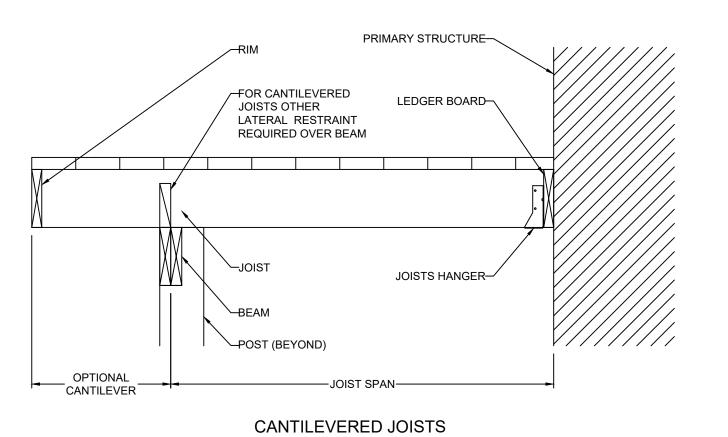


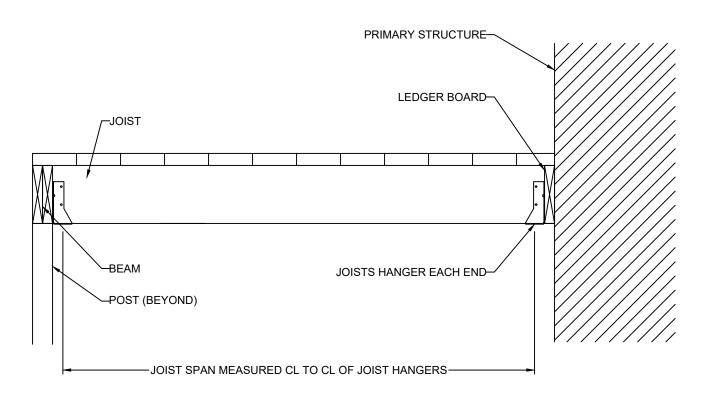




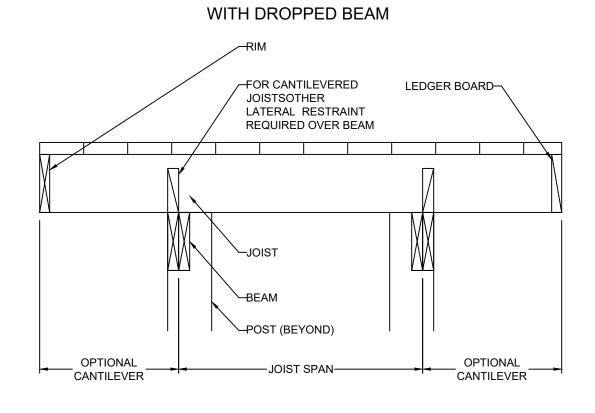


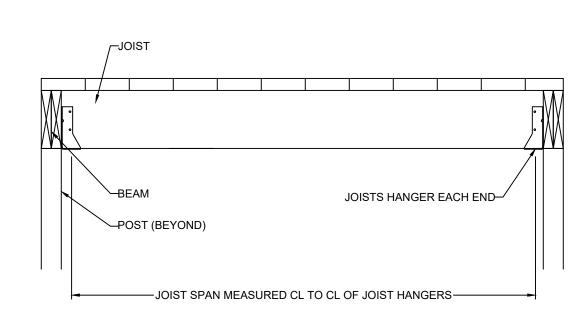






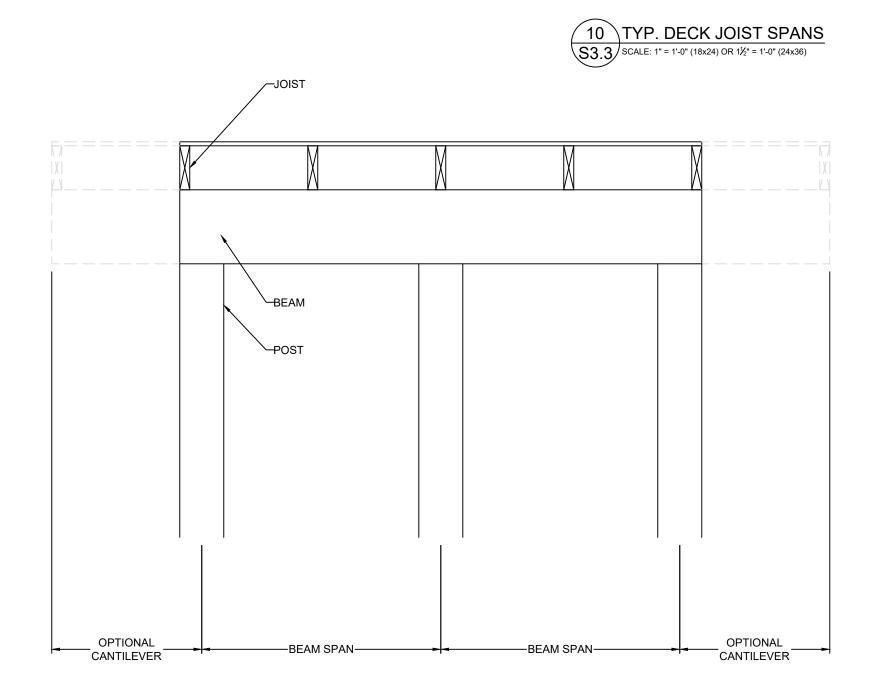
JOISTS WITH FLUSH BEAM

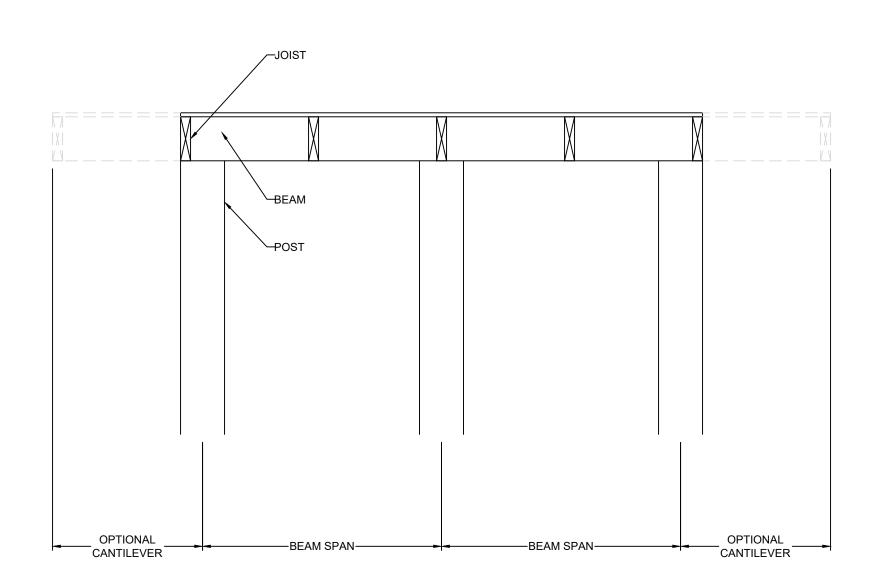




JOISTS ON FREE-STANDING DECK WITH DROPPED BEAM

JOISTS WITH FLUSH BEAM





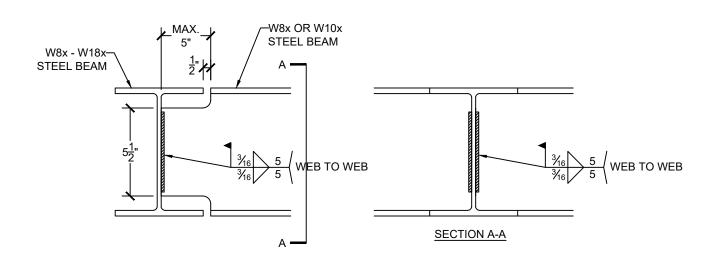


CLIENT: VIEWPOINT RESIDENTIAL DESIGN, LLC
JOB TITLE: LOT 738, EAGLE CREEK
2362 SW HICKORY LANE
LOCATION: LEE'S SUMMIT, MISSOURI



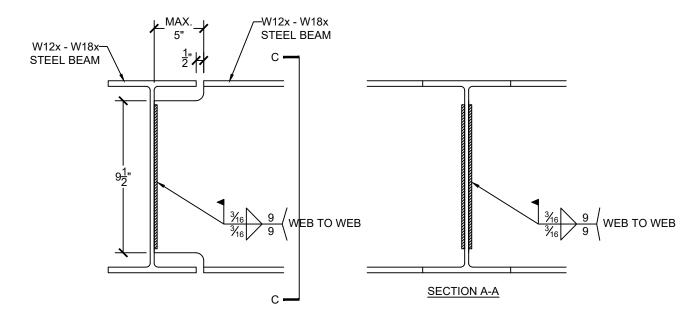
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			04/	9/202		/

DROPPED BEAM FLUSH BEAM



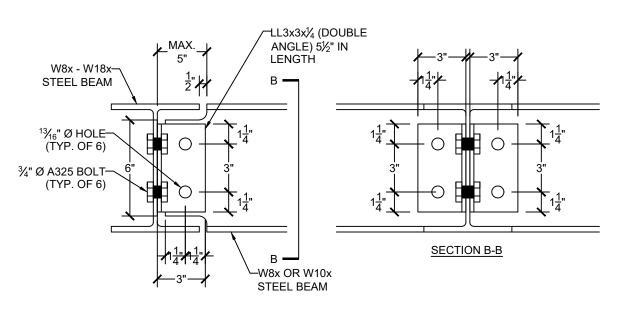
1 WELDED T-BEAM CONNECTION FOR W8x AND W10x BEAMS S3.4 SCALE: 2" = 1'-0" (18x24) OR 3" = 1'-0" (24x36)

(OPTION #1)

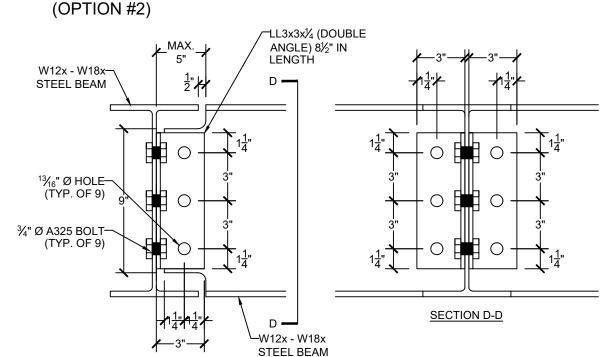


2 WELDED T-BEAM CONNECTION FOR W12x, W14x, W16x & W18x BEAMS S3.4 SCALE: 2" = 1'-0" (18x24) OR 3" = 1'-0" (24x36)

(OPTION #1)



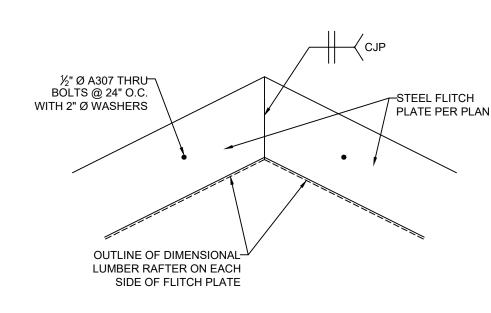
BOLTED T-BEAM CONNECTION FOR W8x AND W10x BEAMS SCALE: 2" = 1'-0" (18x24) OR 3" = 1'-0" (24x36)



2 BOLTED T-BEAM CONNECTION FOR W12x, W14x, W16x & W18x BEAMS

SCALE: 2" = 1'-0" (18x24) OR 3" = 1'-0" (24x36)

(OPTION #2)



 $-5\frac{1}{2}$ " x $6\frac{1}{2}$ " x $\frac{3}{8}$ "
STEEL PLATE ON

SECTION A-A

EACH SIDE OF

BEAM SPLICE

ALTERNATE SPLICE: COMPLETE JOINT PENETRATION WELD

\BEAM SPLICE CONNECTION FOR W8x AND W10x BEAMS

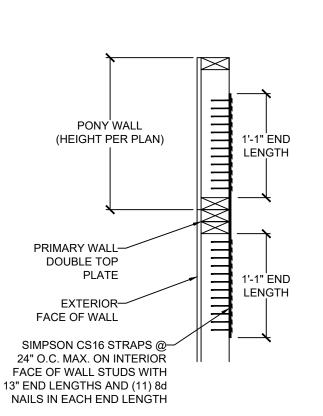
W8x OR W10x-

STEEL BEAM

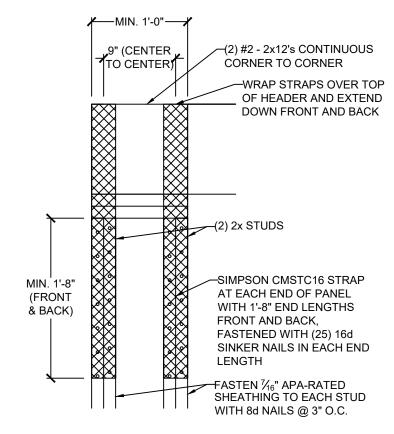
3/4" Ø A325 BOLT-

(TYP. OF 6)



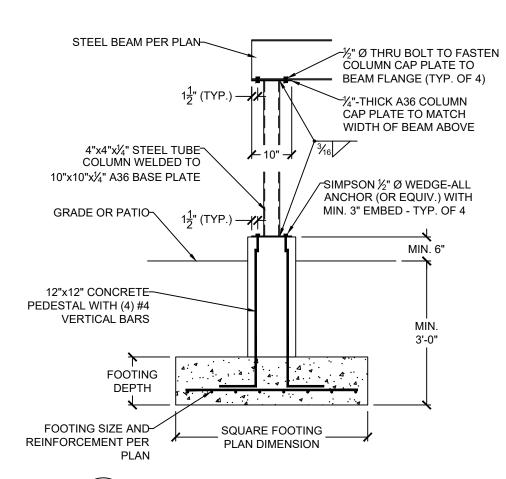


5 SPLICED WALL CONNECTION
S3.4 SCALE: 1" = 1'-0" (18x24) OR 1½" = 1'-0" (24x36)

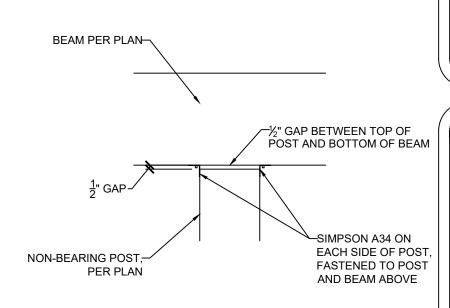


NOTE: SILL PLATE OF PANEL SHALL BE MIN. (1) 2x AND FASTENED WITH $\frac{1}{2}$ " Ø ANCHOR BOLT AND 2" Ø WASHER PLATE

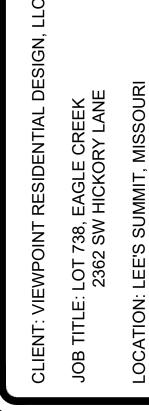
6 3RD CAR STALL BRACING S3.4 SCALE: 1" = 1'-0" (18x24) OR 1½" = 1'-0" (24x36)



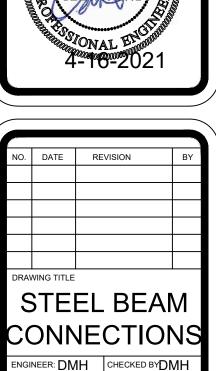
7 EXTERIOR STEEL COLUMN CONNECTIONS S3.4 SCALE: ½" = 1'-0" (18x24) OR ¾" = 1'-0" (24x36)



8 NON-BEARING POST CONNECTION S3.4 SCALE: 1" = 1'-0" (18x24) OR 1½" = 1'-0" (24x36)



☼



JOB NO. 3388 DRAWN BY: DMH

DATE: **04-16-21**

OF MISS

DENNIS HEIER

PE-2010001472