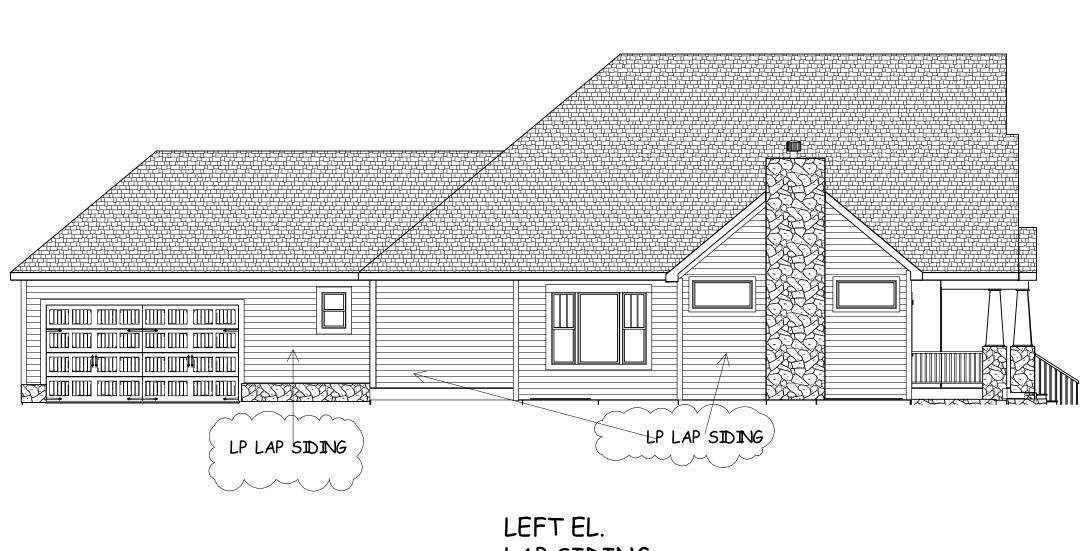
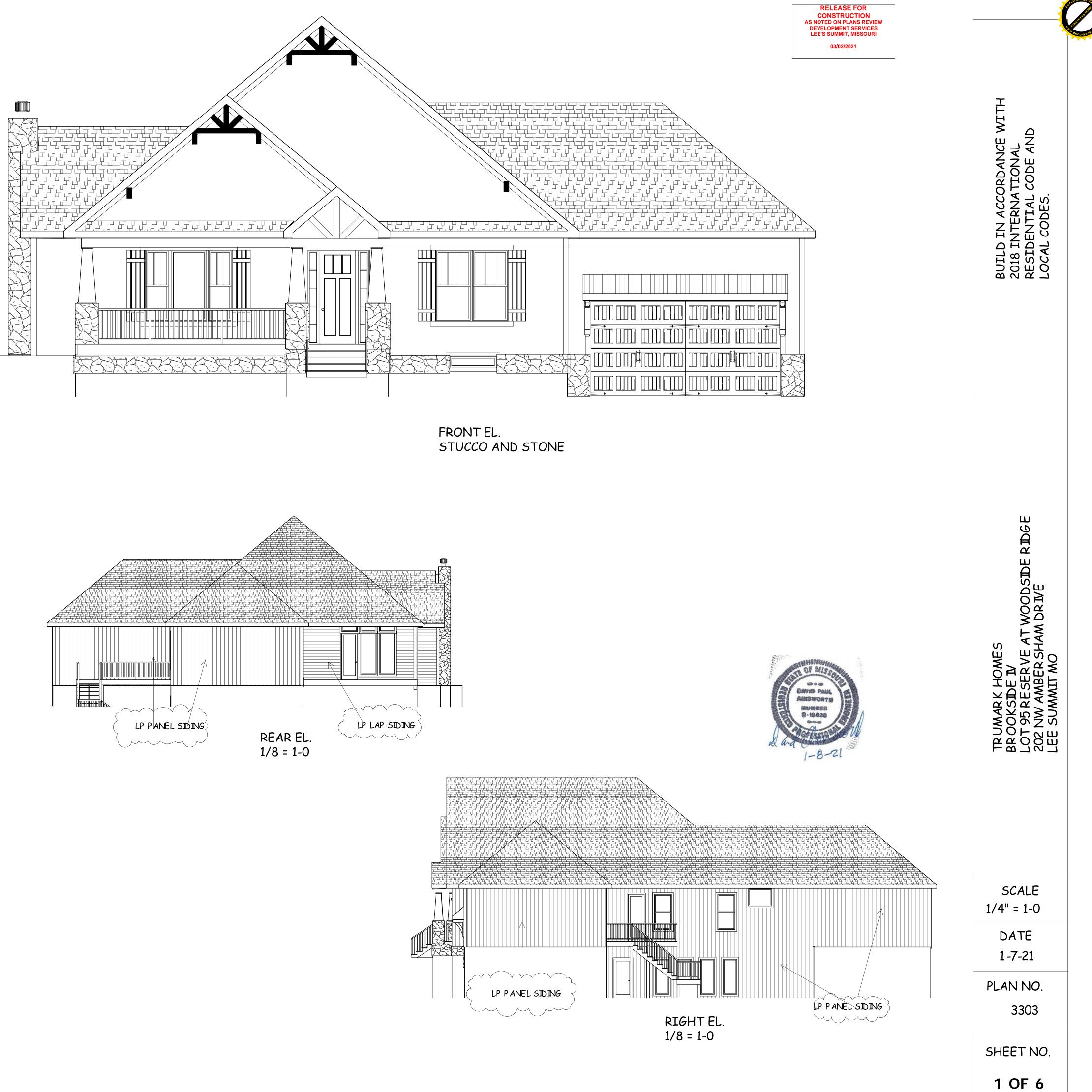
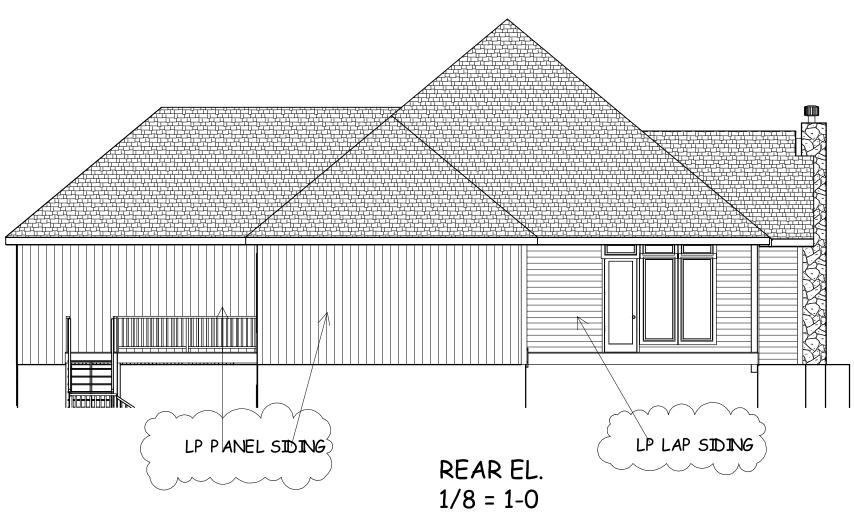


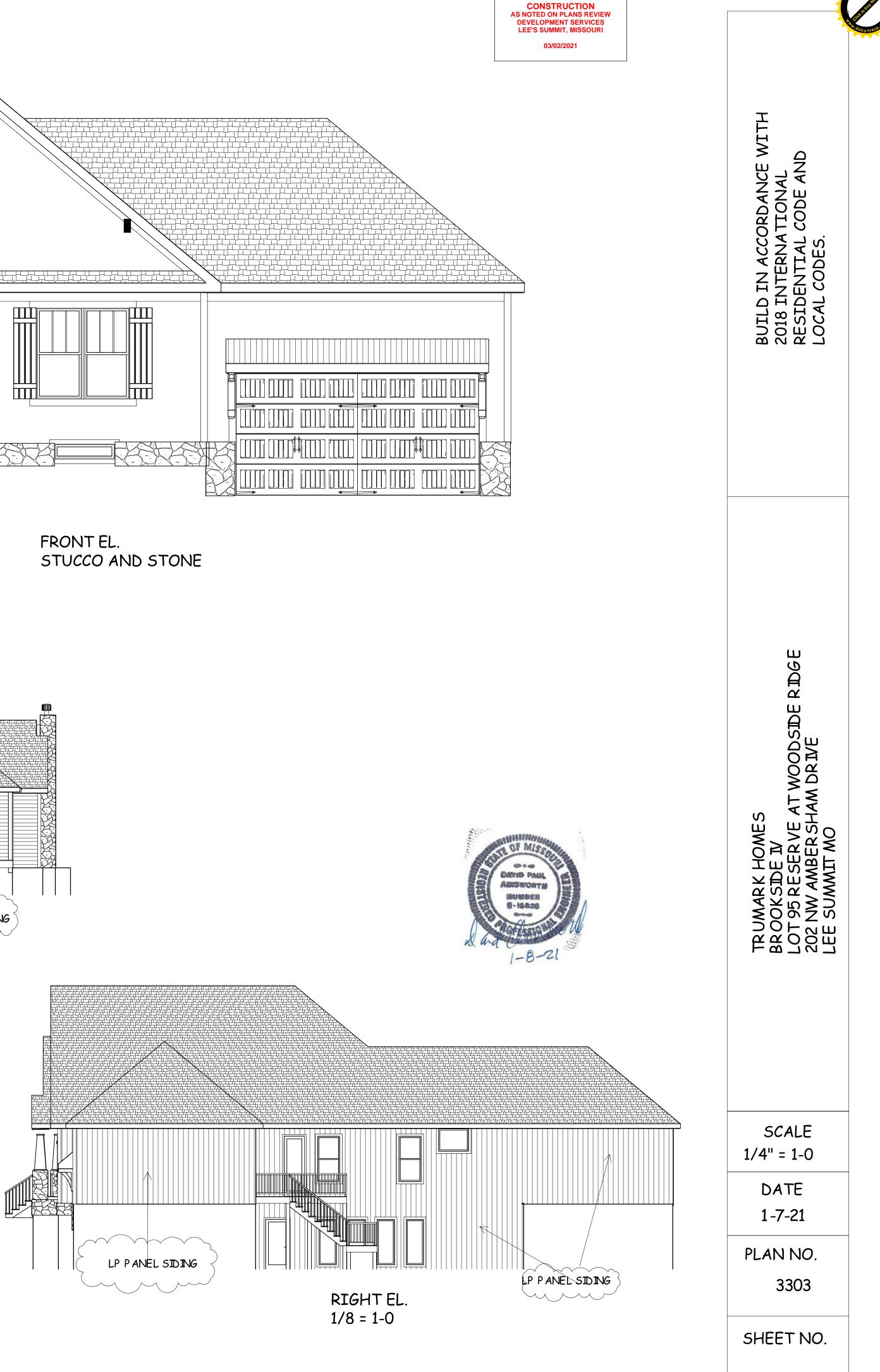
ROOF PLAN 1/8 = 1-0 ROOF PITCHES 10/12 RAFTERS2X6DFNO2@16"OCTYP. HIPSANDRIDGES2X8DFNO2



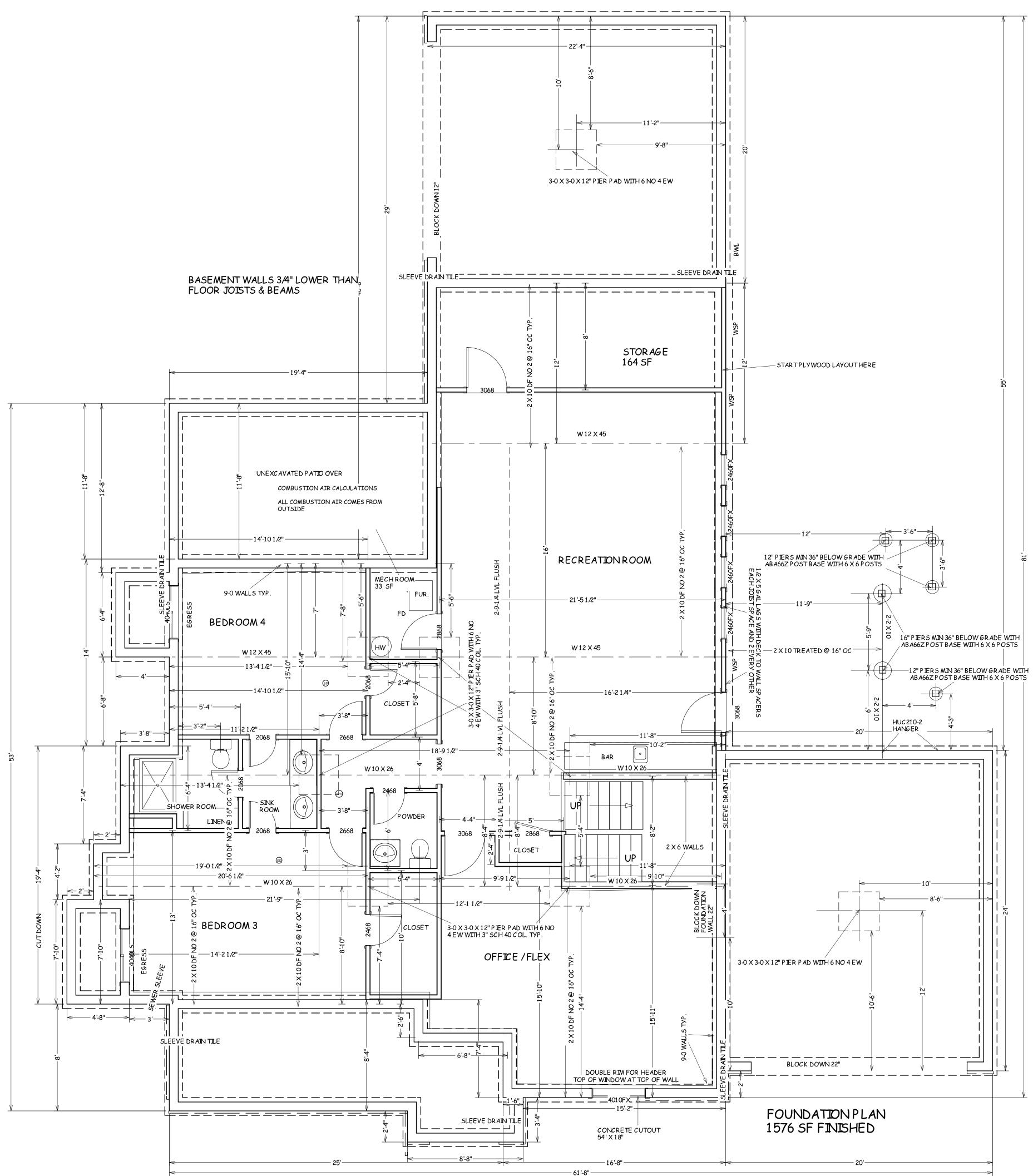
LAP SIDING 1/8 = 1-0













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 $\boldsymbol{\Omega}$ TRUMARK HOMES BROOKSIDE IV LOT 95 RESERVE AT WOODSI 202 NW AMBERSHAM DRIVE LEE SUMMIT MO

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ALTERNATIVE FOUNDATION WALL REINFORCEMENT NOTES:

AS AN ALTERNATIVE TO THE BASEMENT FOUNDATION WALL HORIZONTAL AND VERTICAL REINFORCEMENT, PROVIDE 9 lb/yd3 OF HELIX 5-25 DESIGNED IN ACCORDANCE WITH UNIFORM ES ER-0279.

THE HELIX ALTERNATE ALSO REQUIRES COUNTERFORTS TO BE INSTALLED AT BASEMENT WALLS LONGER THAN 16' AT 16' O.C. PER DETAIL 2, SHEET S103.

CONTACT HELIX FOR PRICING, DELIVERY, AND INSTALLATION AT 734-322-2144 x1 OR SALES @HELIXSTEEL.COM



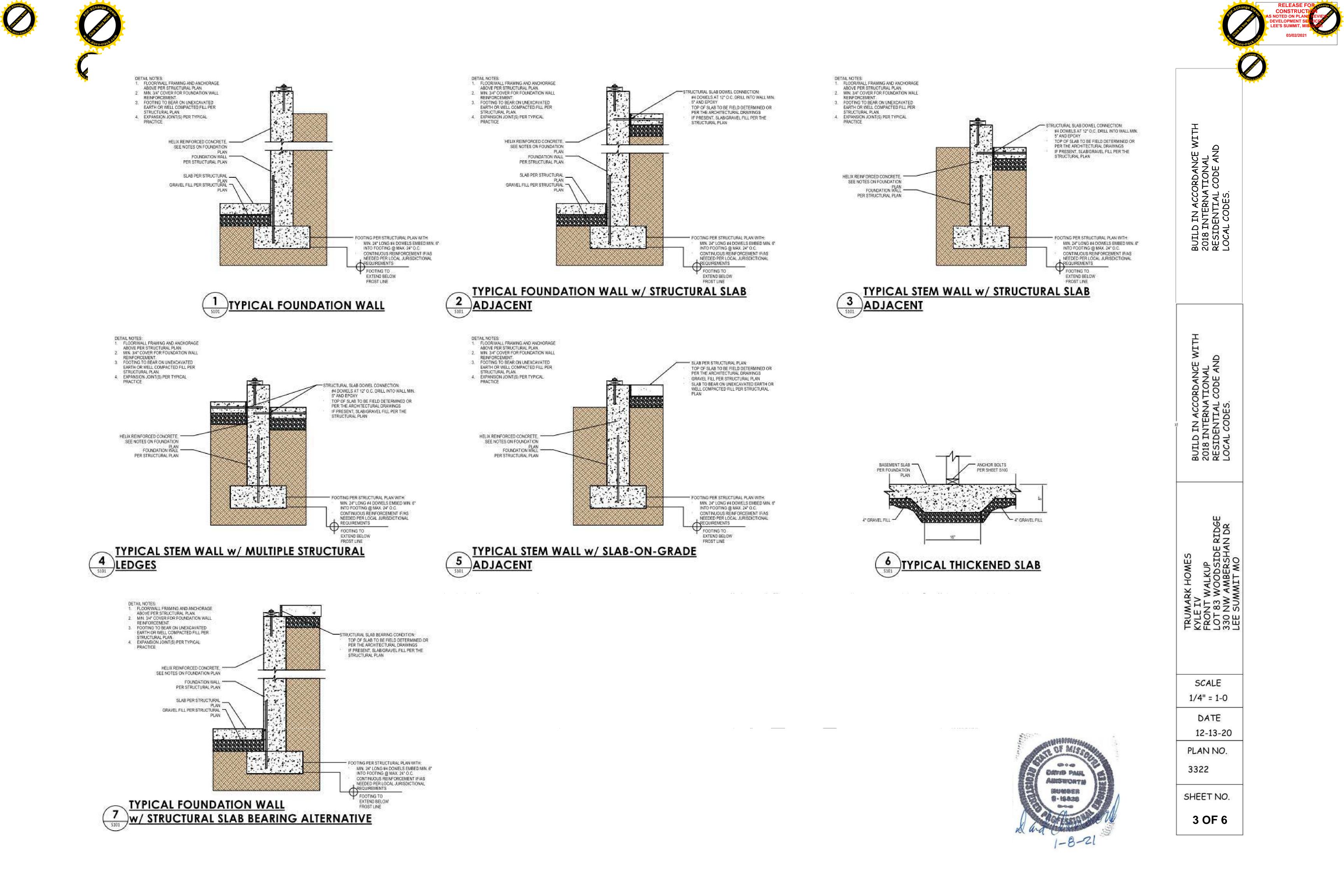
SCALE 1/4" = 1-0

> DATE 1-7-21

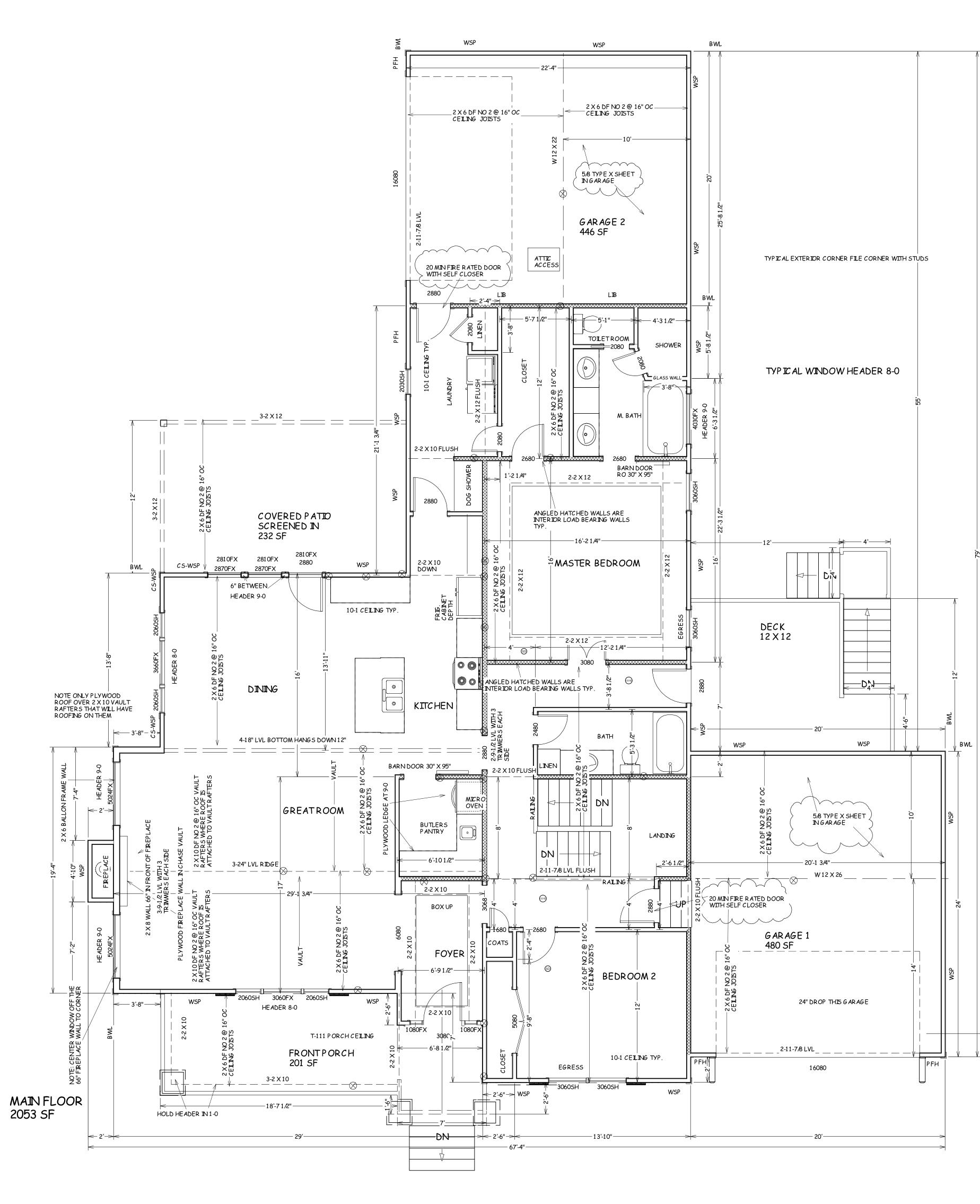
PLAN NO. 3303

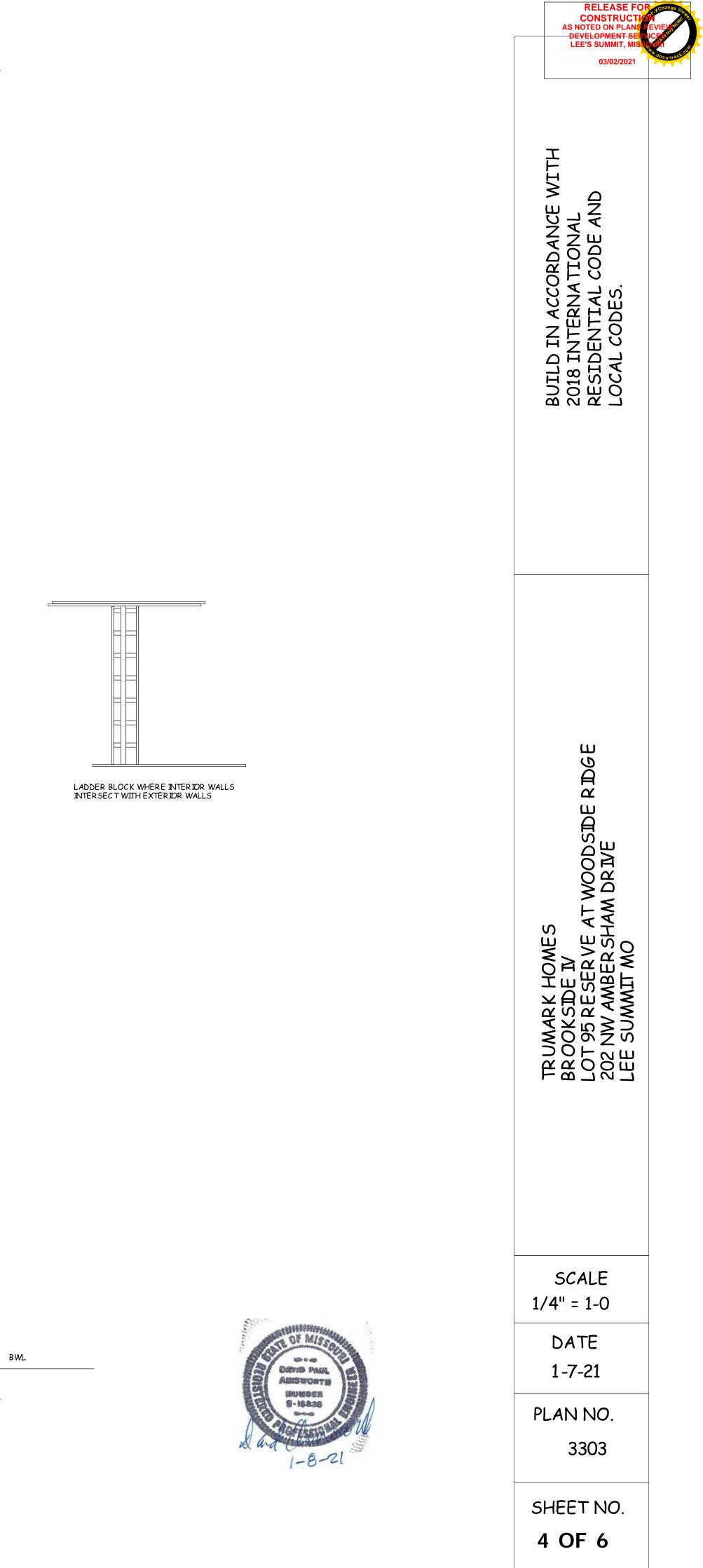
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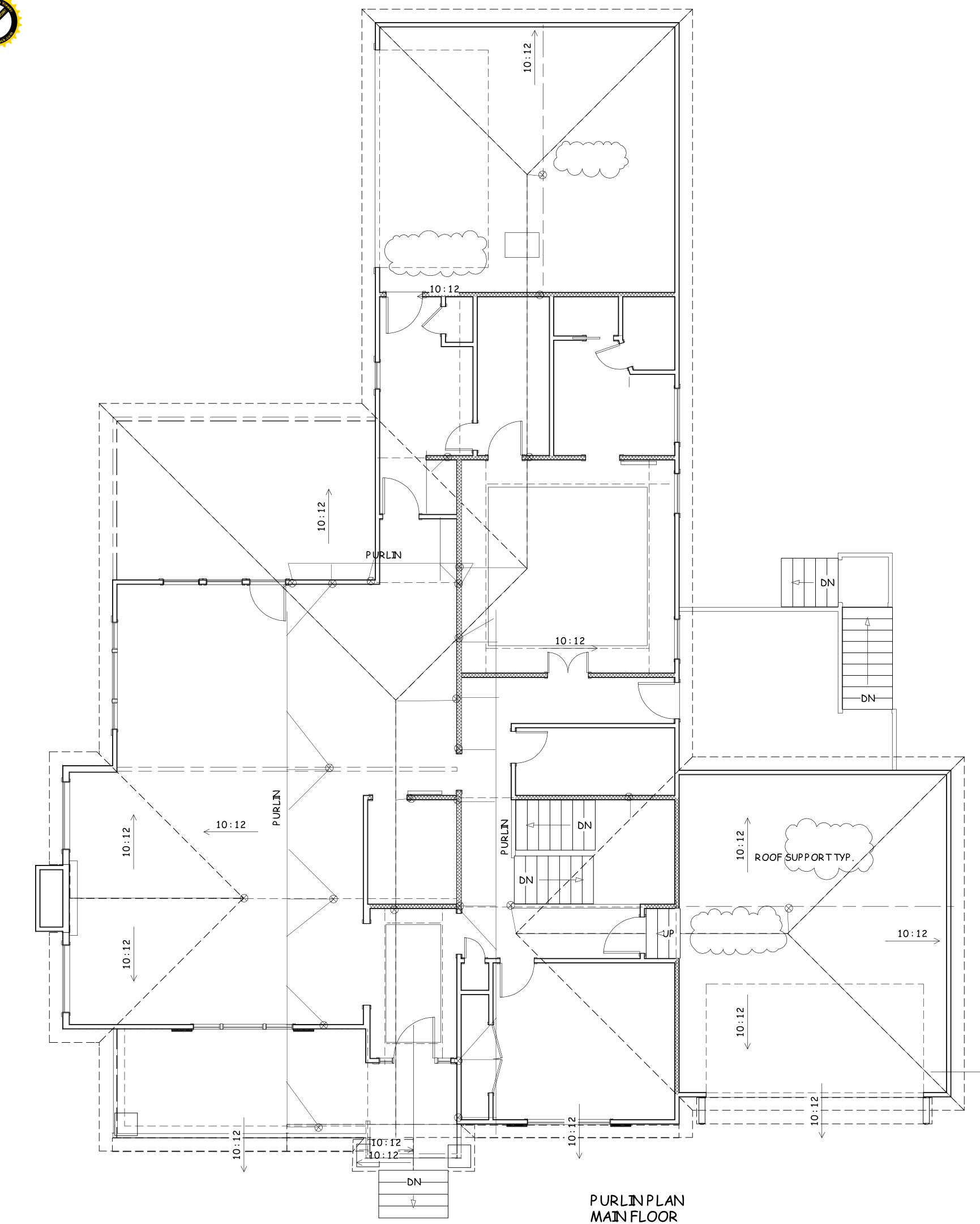
2 OF 6

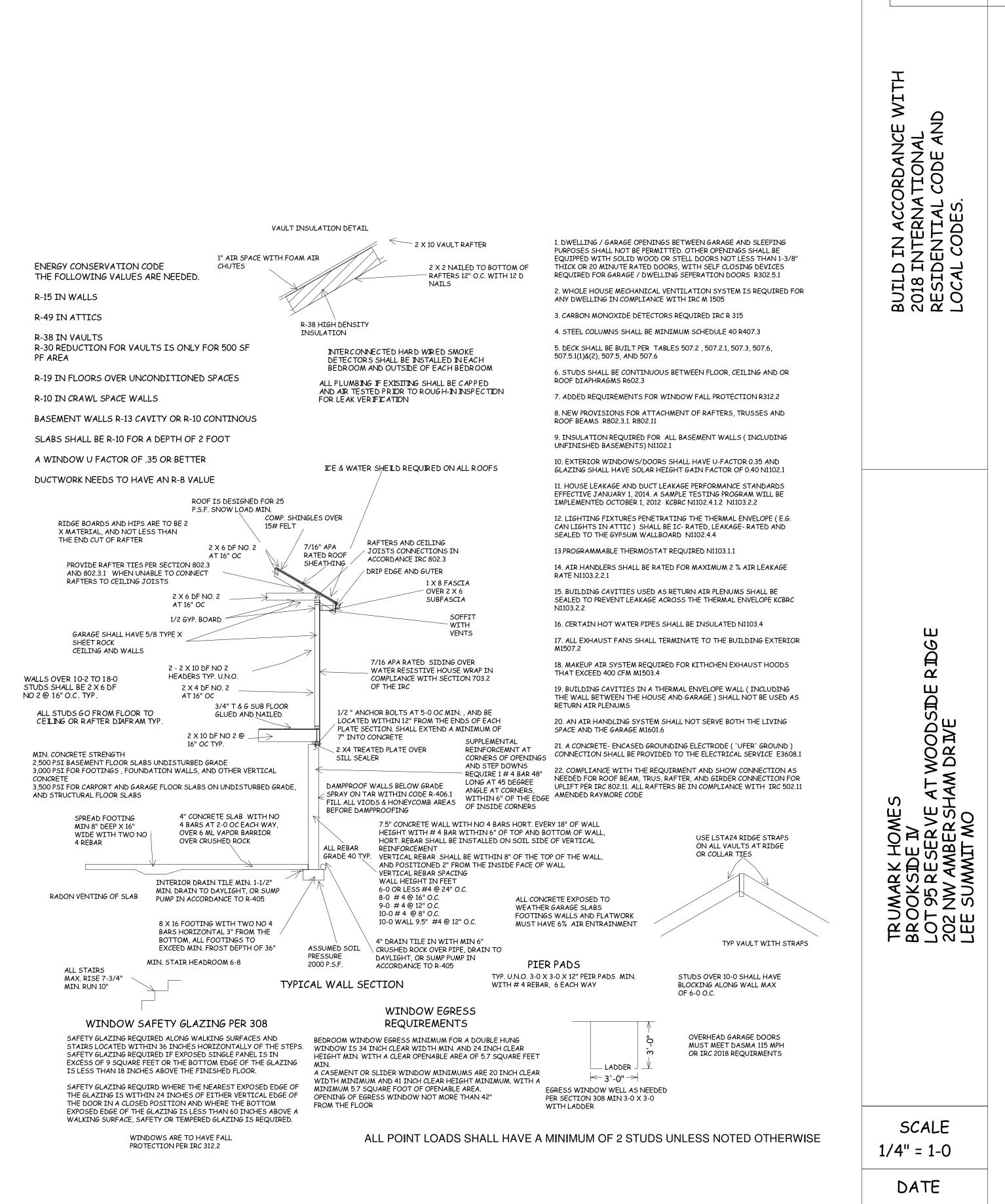














SHEET NO. 5 OF 6

3303

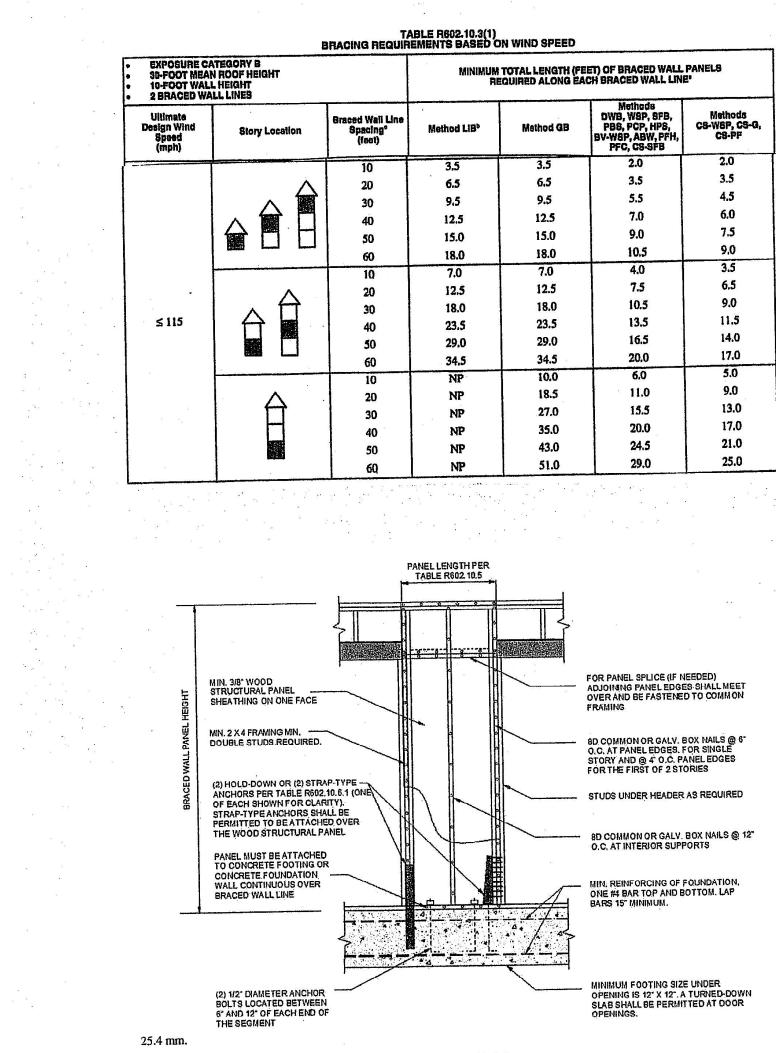
1-7-21

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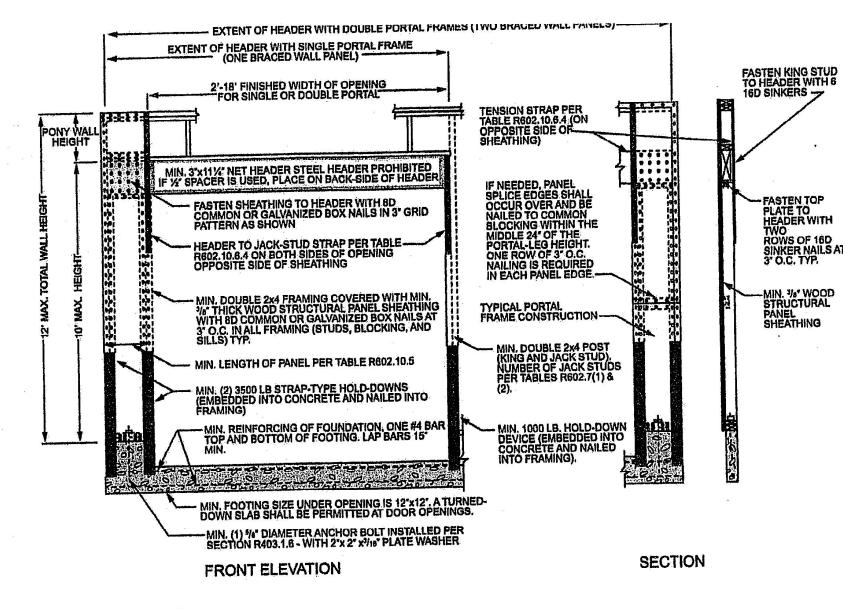


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FIGURE R602.10.6.1 METHOD ABW---ALTERNATE BRACED WALL PANEL



4 mm, 1 foot = 304.8 mm.

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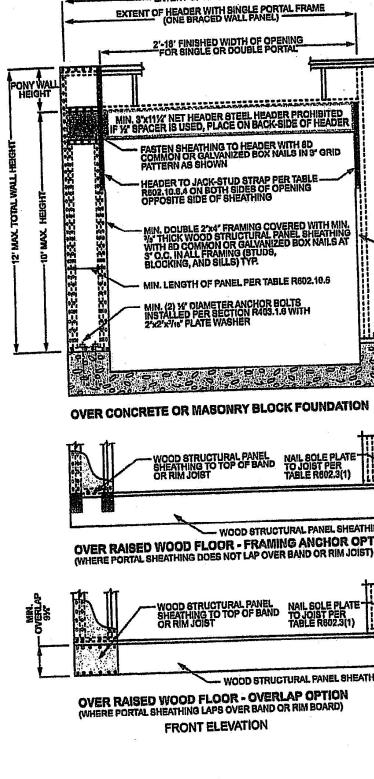
FIGURE R602.10.6.2 METHOD PFH—PORTAL FRAME WITH HOLD-DOWNS

| | | | BRACING METHO | CONNECTION CRITERI | A ^a |
|------------------------------|--|---|------------------------|---|---|
| METHODS, MATERIAL | | MINIMUM THICKNESS | FIGURE | Fasteners | Spacing |
| Intermittent Bracing Methods | | 1×4 wood or approved metal straps | | Wood: 2-8d common nails or 3-8d (2 ¹ / ₂ " long x 0.113" dia.) nails | Wood: per stud and top and bottom plates |
| | Let-in-bracing | at 45° to 60° angles for maximum 16" stud spacing | | Metal strap: per manufacturer | Metal: per manufacturer |
| | DWB Diagonal wood boards | ³ / ₄ " (1" nominal) for maximum 24" stud spacing | | 2-8d $(2^{1}/_{2}^{"} \log \times 0.113^{"} \text{ dia.})$ nails or 2 - $1^{3}/_{4}^{"} \log \text{ staples}$ | Per stud |
| | WSP Wood | 3/8" | TRAILING | Exterior sheathing per Table R602.3(3) | 6" edges 12" field |
| | structural panel (See Section R604) | | | Interior sheathing per Table R602.3(1) or R602.3(2) | Varies by fastener |
| | BV-WSP ^e Wood structural panels with stone or masonry veneer (See Section R602.10.6.5) | 7/ ₁₆ " | See Figure R602.10.6.5 | 8d common $(2^{1}/_{2}'' \times 0.131)$ nails | 4" at panel edges 12" at intermediate supports 4" at braced wall panel end posts |
| | SFB Structural fiberboard sheathing | ¹ / ₂ " or ²⁵ / ₃₂ " for maximum 16" stud spacing | | $1^{1}/_{2}$ " long x 0.12" dia. (for $1^{1}/_{2}$ " thick sheathing) $1^{3}/_{4}$ " long x 0.12" dia. (for $2^{25}/_{32}$ " thick sheathing) galvanized roofing nails | 3" edges 6" field |
| mittent | | 1/2" | | Nails or screws per Table R602.3(1) for exterior locations | For all braced wall panel locations: 7" edges (including top |
| Inter | GB Gypsum board | | | Nails or screws per Table R702.3.5 for interior locations | and bottom plates) 7 field |
| | PBS Particleboard sheathing (See Section R605) | ³ / ₈ " or ¹ / ₂ " for maximum 16" stud spacing | | For ${}^{3}/{}_{8}$ ", 6d common (2" long × 0.113" dia.) nails For ${}^{1}/{}_{2}$ ", 8d common (2"/ $_{2}$ " long × 0.131" dia.) nails | 3" edges 6" field |
| | PCP Portland cement plaster | See Section R703.7 for maximum 16" stud spacing | | $1^{1}/_{2}^{"}$ long, 11 gage, $7^{'}/_{16}^{"}$ dia. head nails or $7^{'}/_{8}^{"}$ long, 16 gage staples | 6" o.c. on all framing members |
| | HPS Hardboard panel siding | ⁷ / ₁₆ " for maximum 16" stud spacing | | 0.092" dia., 0.225" dia. head nails with length to accommodate 1 ¹ / ₂ " penetration into studs | 4" edges 8" field |
| | ABW Alternate braced wall | ³ / ₈ " | | See Section R602.10.6.1 | See Section R602.10.6 |

| | RELEASE FOR COMPACE CONSTRUCTION AS NOTED ON PLANS REVIEW OF DEVELOPMENT SERVICES LEE'S SUMMIT, MISSING OF 03/02/2021 |
|--|--|
| TABLE R602.10.4continued BRACING METHODS | HTIM Q |
| CONNECTION CRITERIA' | |
| Fasteners Spacing | DRDANCE V TIONAL CODE AND |
| PFH Portal frame with hold-downs Ng" Image: Constraint of the section R602.10.6.2 See Section R602.10.6.2 Portal frame at garage γ_{16} " Image: Constraint of the section R602.10.6.3 See Section R602.10.6.3 | |
| CS-WSP Exterior sheathing per Table R602.3(3) 6" edges 12" field | |
| Continuously sheathed wood structural panel CS-G ^{5, c} Continuously sheathed wood structural panel adjacent to garage | BUILD IN ACC 2018 INTERN RESIDENTIAL LOCAL CODES |
| Image: Second | α ν α Τ |
| $\frac{1}{2} \frac{Continuously sheathed}{portal frame} \frac{7'_{16}"}{Continuously sheathed} \frac{1}{2}" or \frac{25}{32}" for maximum 16"} See Section R602.10.6.4 Se$ | |
| For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm, 1 degree = 0.0175 rad, 1 pound per square foot = 47.8 N/m ² , 1 mile per hour = 0.447 m/s. a. Adhesive attachment of wall sheathing, including Method GB, shall not be permitted in Seismic Design Categories C, D ₀ , D ₁ and D ₂ . b. Applies to panels next to garage door opening where supporting gable end wall or roof load only. Shall only be used on one wall of the garage. In Seismic | |
| Design Categories D₀, D₁ and D₂ roof covering deal load shall not exceed 5 psr. c. Garage openings adjacent to a Method CS-G panel shall be provided with a header in accordance with Table R602.7(1). A full-height clear opening shall not be permitted adjacent to a Method CS-G panel. d. Method CS-SFB does not apply in Seismic Design Categories D₀, D₁ and D₂. e. Method applies to detached one- and two-family dwellings in Seismic Design Categories D₀ through D₂ only. | |
| e. Method applies to detached one- and two-namily dweinings in Seisine Design Categories D ₀ anolge D ₁ and t | |
| CONTINUOUSLY 8HEATHED BRACED WALL PANEL MEETING MINUM LENGTH REQUIREMENTS OF TABLE R602, 10.5 | |
| EXTENT OF CONCRETE OR MASCINRY BLOCK FOUNDATION | OMES IV ERVE AT WOODSIDE RIDGE ERSHAM DRIVE MO |
| WOOD STRUCTURAL PANEL SHEATHING TO TO POP BAND OR RIM JOIST WOOD STRUCTURAL PANEL SHEATHING JOIST PER TABLE R802.3(1) WOOD STRUCTURAL PANEL SHEATHING OVER APPROVED BAND OR RIM JOIST WOOD STRUCTURAL PANEL SHEATHING OVER APPROVED BAND OR RIM JOIST WOOD STRUCTURAL PANEL SHEATHING OVER APPROVED BAND OR RIM JOIST WOOD STRUCTURAL PANEL SHEATHING OVER APPROVED BAND OR RIM JOIST WOOD STRUCTURAL PANEL SHEATHING OVER APPROVED BAND OR RIM JOIST WOOD STRUCTURAL PANEL SHEATHING OVER APPROVED BAND OR RIM JOIST WHERE PORTAL SHEATHING DOES NOT LAP OVER BAND OR RIM JOIST) | TRUMARK HO BROOKSIDE I LOT 95 RESER 202 NW AMBE LEE SUMMIT A |
| Wood STRUCTURAL PANEL SHEATHING TO TOP OF BAND OR RIM JOIST WOOD STRUCTURAL FANEL SHEATHING OVER APPROVED BAND OR RIM JOIST WOOD STRUCTURAL FANEL SHEATHING OVER APPROVED BAND OR RIM JOIST WOOD STRUCTURAL FANEL SHEATHING OVER APPROVED BAND OR RIM JOIST WOOD STRUCTURAL FANEL SHEATHING OVER APPROVED BAND OR RIM JOIST WHERE PORTAL SHEATHING LAPS OVER BAND OR RIM BOARD) SECTION SECTION | |
| FRONT ELEVATION For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm. | |
| FIGURE R602.10.6.4 METHOD CS-PF-CONTINUOUSLY SHEATHED PORTAL FRAME PANEL CONSTRUCTION | |
| | SCALE 1/4" = 1-0 |
| and the second | DATE |
| Sure on our of the second | 1-7-21 |
| Autorate Autorate | |
| a had been all | PLAN NO. 3303 |
| 1-0-01 | |

| | | IGTH OF BRACED WALL PANELS MINIMUM LENGTH* (Inchea) | | | | | CONTRIBUTING LENGTH |
|--------------------------------------|---|---|----------------------|---------|---------|---------|---|
| METHOD (See Table R602.10.4) | | | Wall Height | | | | (inches) |
| | | | 9 feet | 10 feet | 11 feet | 12 feet | |
| DIVD WCD CED D | | 8 feet 48 | 48 | 48 | 53 | 58 | Actual ^b |
| DWB, WSP, SFB, PBS, PCP, HPS, BV-WSP | | 48 | 48 | 48 | 53 | 58 | Double sided = Actual Single sided = 0.5 × Actu |
| GB | | | | | NP | NP | Single sided = $0.5 \times Actual^6$ |
| LIB | | 55 | 62 | 69 | NP | INF | / Icultu |
| | SDC A, B and C, ultimate design wind speed < 140 mph | 28 | 32 | 34 | 38 | 42 | 48 |
| ABW | SDC D ₀ , D ₁ and D ₂ , ultimate design wind speed < 140 mph | 32 | 32 | 34 | NP | NP | |
| | CS-G | 24 | 27 | 30 | 33 | 36 | Actual ^b |
| | Adjacent clear opening height (inches) | | | | | | |
| | ≤ 64 | 24 | 27 | 30 | 33 | 36 | Actual ^b |
| | 68 | 26 | 27 | 30 | 33 | 36 | |
| | 72 | 27 | 27 | 30 | 33 | 36 | |
| | 76 | 30 | 29 | 30 | 33 | 36 | |
| | 80 | 32 | 30 | 30 | 33 | 36 | |
| | 84 | 35 | 32 | 32 | 33 | 36 | |
| | 88 | 38 | 35 | 33 | 33 | 36 | |
| | 92 | 43 | 37 | 35 | 35 | 36 | |
| | 96 | 48 | 41 | 38 | 36 | 36 | |
| CS-WSP, CS-SFB | 100 | | 44 | 40 | 38 | 38 | |
| | 104 | | 49 | 43 | 40 | 39 | |
| | 108 | | 54 | 46 | 43 | 41 | |
| | 112 | | - | 50 | 45 | 43 | |
| | 116 | | | 55 | 48 | 45 | |
| | 120 | | | 60 | 52 | 48 | |
| | 124 | | | | 56 | 51 | |
| | 128 | - | | - | 61 | 54 | |
| | 132 | | - | | 66 | 58 | |
| | 136 | · · · · · · · · · · · · · · · · · · · | | | — | 62 | |
| | 140 | | - | - | - | 66 | |
| | 144 | | | - | — | 72 | |
| METHOD | | | Portal header height | | | | _ |
| (See Table R602.10.4) | | 8 feet | 9 feet | 10 feet | 11 feet | 12 feet | |
| | Supporting roof only | 16 | 16 | 16 | Note c | Note c | - 40 |
| PFH | Supporting one story and roof | 24 24 | 24 | 24 | Note c | Note c | 1 F 1 -tw-th |
| PFG | | | 27 | 30 | Note d | Note d | and the second se |
| | SDC A, B and C | 16 | 18 | 20 | Note e | Note e | |
| CS-PF | SDC D_0 , D_1 and D_2 | 16 | 18 | 20 | Note e | Note e | Actual ^b |

а. С. р. – ,



For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm, 1 mile per hour = 0.447 m/s. NP = Not Permitted. a. Linear interpolation shall be permitted.

IEADER WITH

ROWS OF 16D SINKER NAILS AT 3" O.C. TYP.

MIN. 3/6" WOOD STRUCTURAL PANEL SHEATHING

a. Linear interpolation shall be permuted.
b. Use the actual length where it is greater than or equal to the minimum length.
c. Maximum header height for PFH is 10 feet in accordance with Figure R602.10.6.2, but wall height shall be permitted to be increased to 12 feet with pony wall.
d. Maximum header height for PFG is 10 feet in accordance with Figure R602.10.6.3, but wall height shall be permitted to be increased to 12 feet with pony wall.
e. Maximum header height for PFG is 10 feet in accordance with Figure R602.10.6.4, but wall height shall be permitted to be increased to 12 feet with pony wall.

BRACE WALL DETAILS WIND SPEED 115 MPH WIND EXPOSURE A SEISMIC DESIGNCAEGORY A

SHEET NO.

6 OF 6