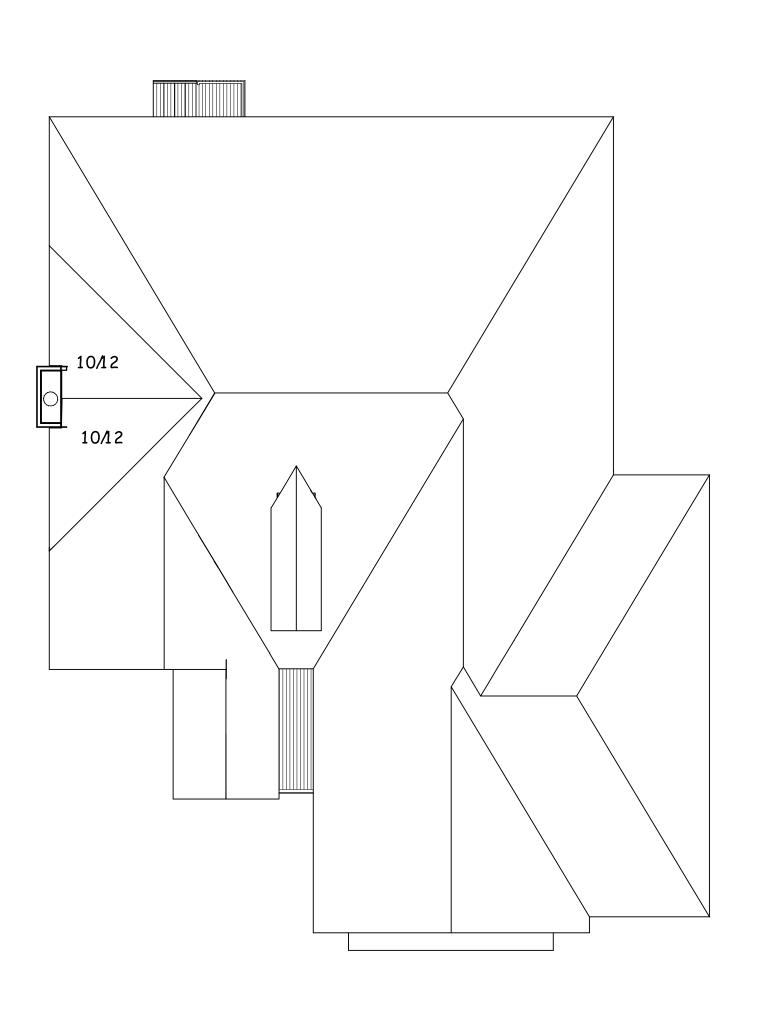
BUILD IN ACCORDANCE WITH 2018 INTERNATIONAL RESIDENTIAL CODE AND LOCAL CODES.

SHEET NO.

1 OF 5



ROOF PLAN

1/8 = 1-0

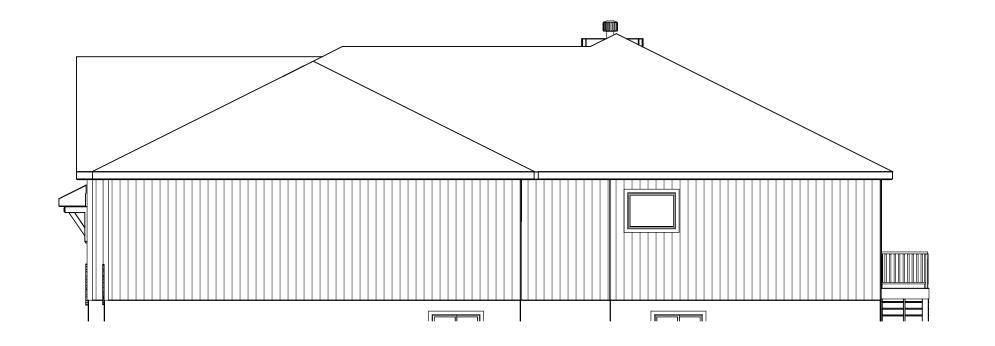
ROOF PITCHES FRONT TO BACK 6/12 TYP. U.N.O.

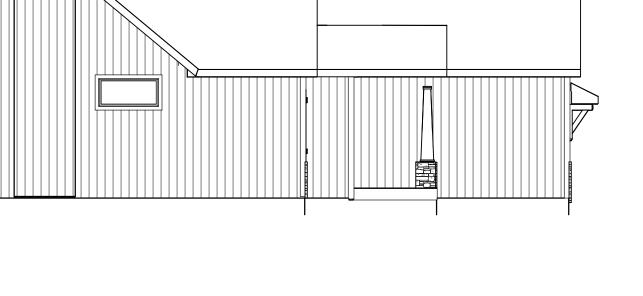
ROOF PITCHES SIDE TO SIDE 10/12 TYP. U.N.O

RAFTERS 2 X 6 DF NO 2 @ 16" OC TYP.

HIPS AND RIDGES 2 X 8 DF NO 2 TYP.





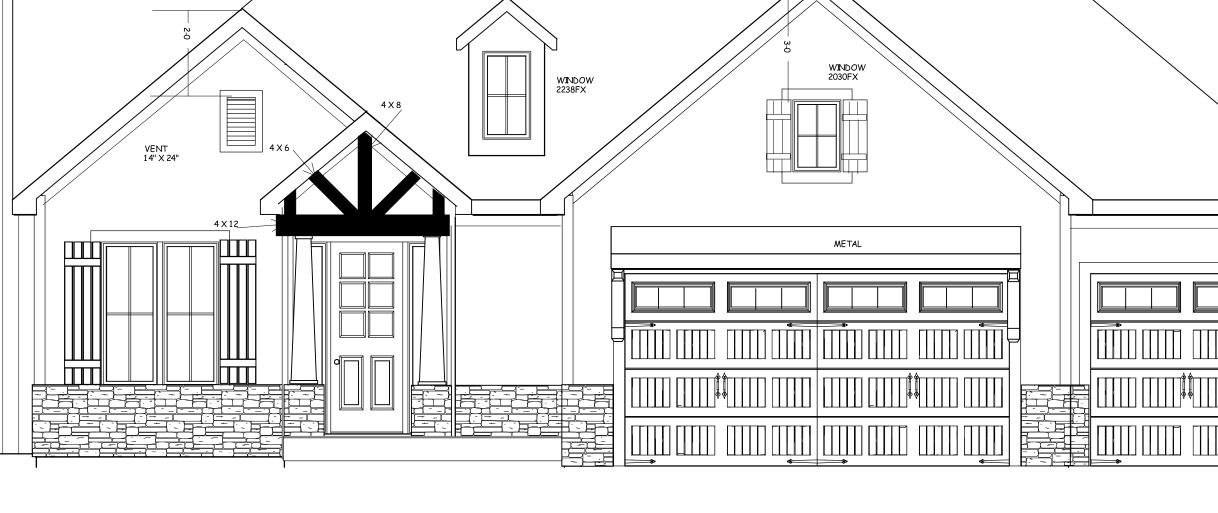


REAR ELEVATION 1/8 = 1-0

LEFT ELEVATION 1/8 = 1-0

RELEASE FOR
CONSTRUCTION
AS NOTED ON PLANS REVIEW
DEVELOPMENT SERVICES
LEE'S SUMMIT, MISSOURI

RIGHT ELEVATION 1/8 = 1-0



FRONT ELEVATION " A " STUCCO AND STONE

1/4" = 1-0

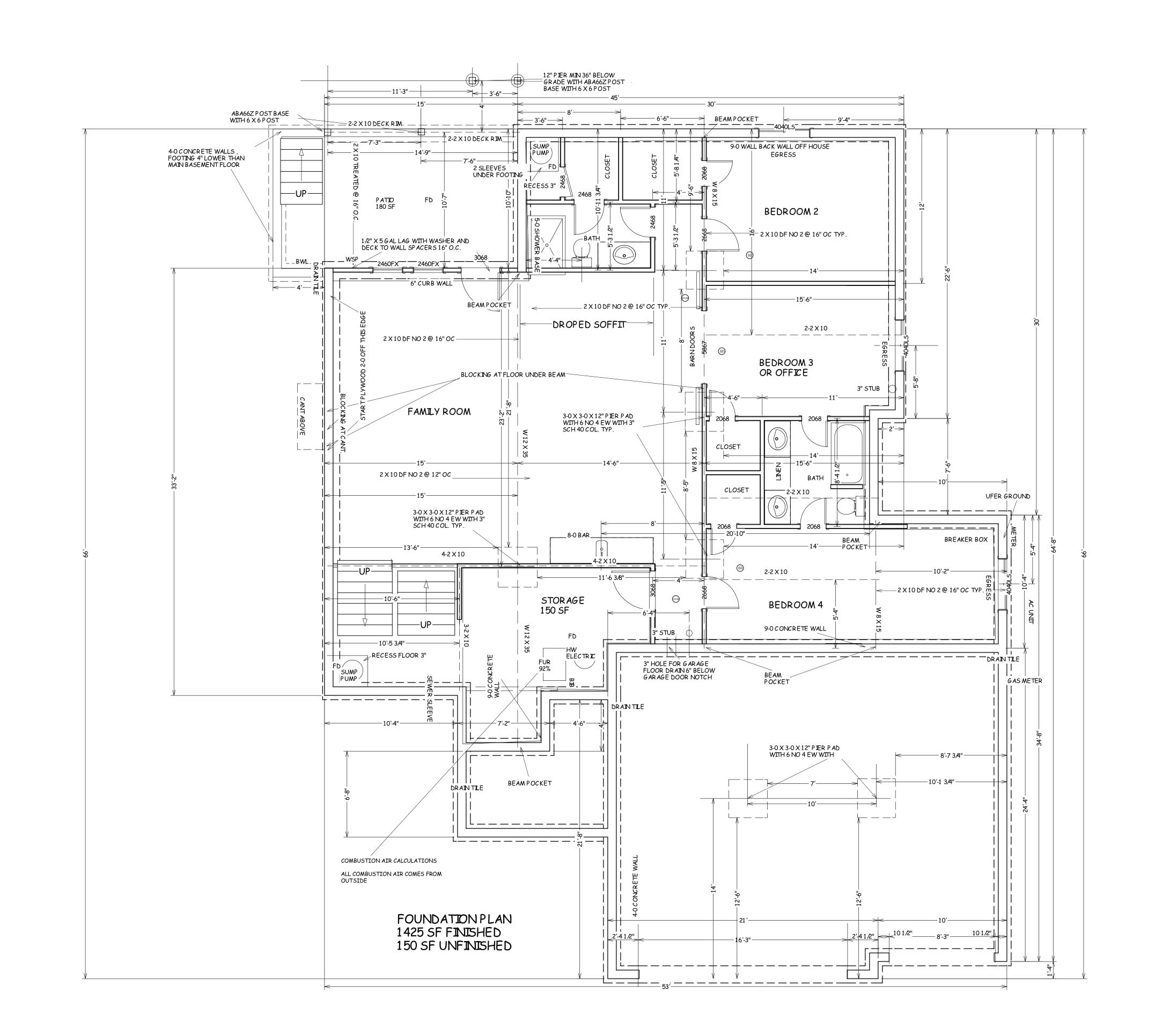
DATE 1-26-21

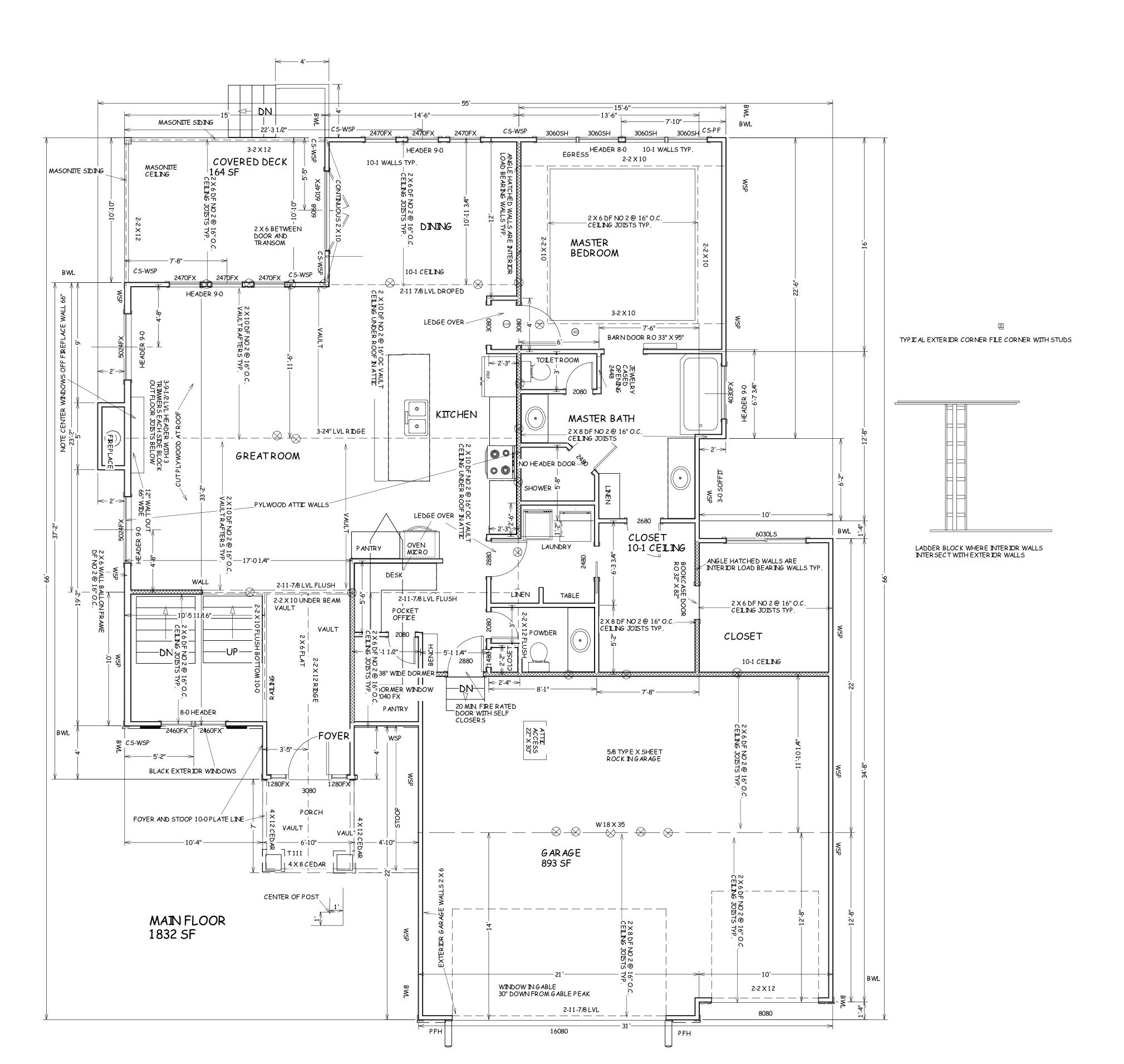
DAVID PAUS AMSWORTED PLAN NO.

3369

SHEET NO.









BUILD IN ACCORDANCE WITH 2018 INTERNATIONAL RESIDENTIAL CODE AND LOCAL CODES.

> ELEVATION A ELEVATION A LOT 1482 WINTER SET 3053 NW THOREAU LANE

SCALE 1/4" = 1-0

DATE

1-26-21

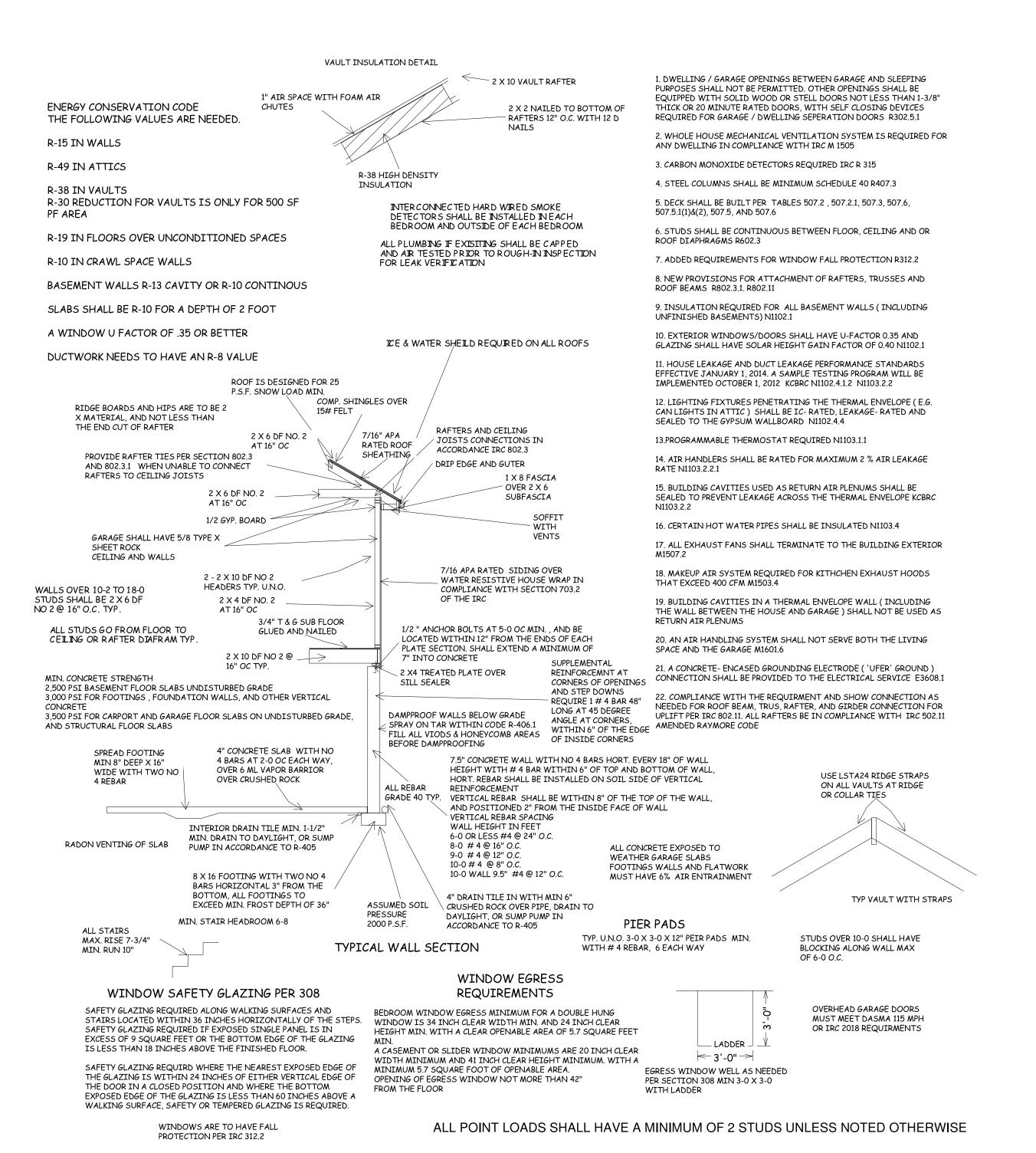
PLAN NO.

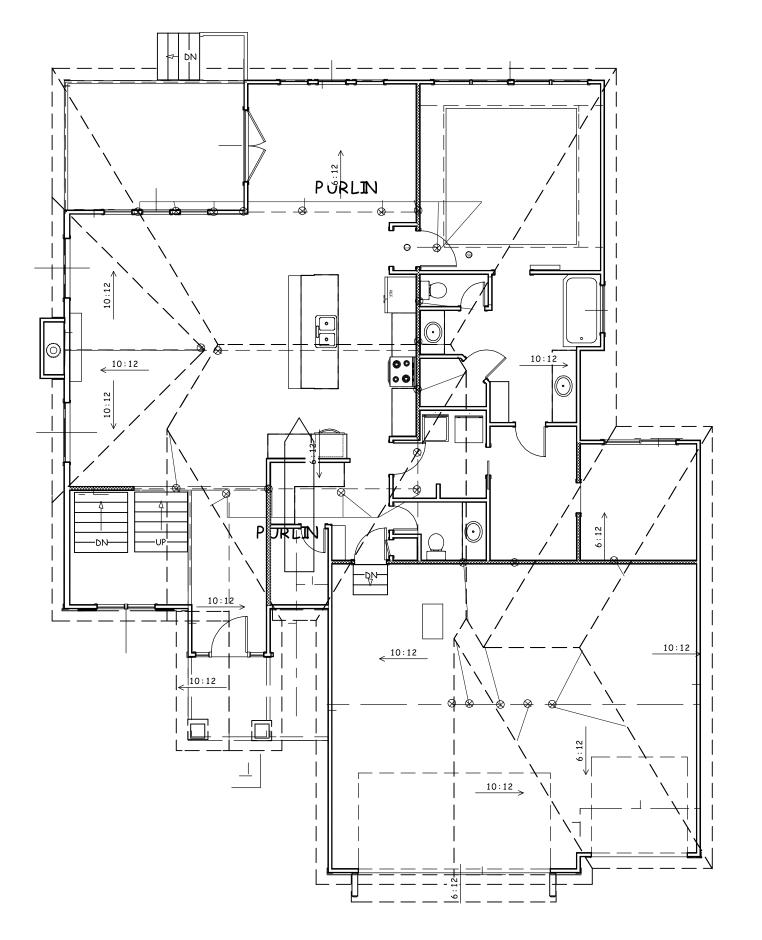
3369

SHEET NO.

3 OF RELEASE FOR
CONSTRUCTION
AS NOTED ON PLANS REVIEW
DEVELOPMENT SERVICES
LEE'S SUMMIT, MISSOURI

02/11/2021





PURLINPLAN 1/8" = 1-0 RAFTER SPANMAX. 14-0



BUILD IN ACCORDANCE WITH 2018 INTERNATIONAL RESIDENTIAL CODE AND

KYLE V ELEVATION A LOT 1482 WINTERSET 3053 NW THOREAU LANE

*SCALE* 1/4" = 1-0

DATE

1-26-21

PLAN NO.

3369

SHEET NO.

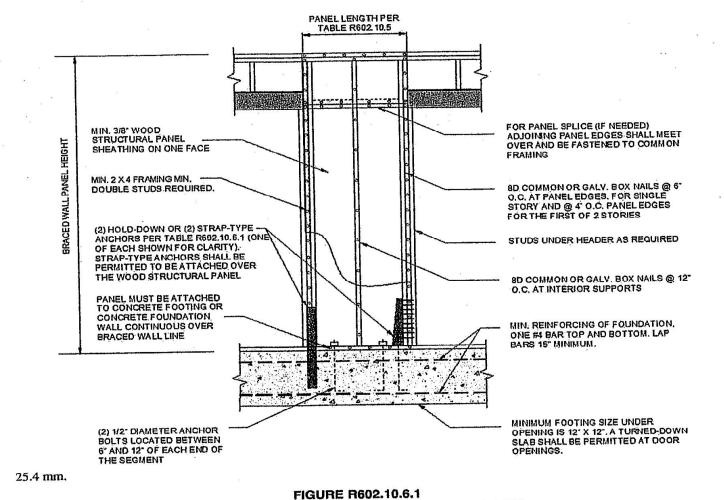


3369

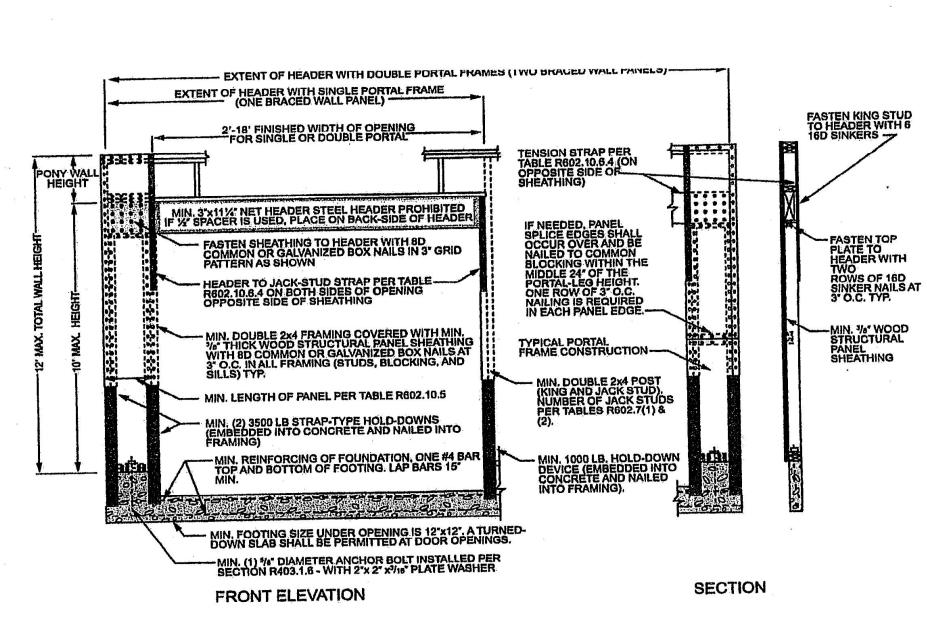
SHEET NO.

RELEASE FOR DEVELOPMENT SERVICES
LEE'S SUMMIT, MISSOURI 02/11/2021

9.5 12.5 12.5 9.0 15.0 9.0 10.5 18.0 18.0 18.0 23.5 13.5 23.5 14.0 29.0 29.0 17.0 20.0 34.5 34.5 13.0 27.0 17.0 35.0 21.0 43.0 50 25.0

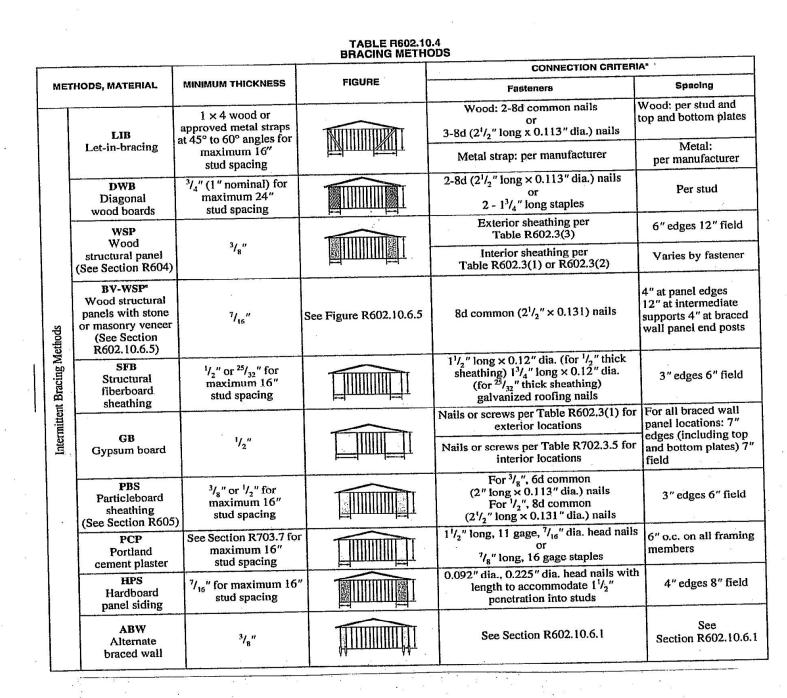


METHOD ABW-ALTERNATE BRACED WALL PANEL



4 mm, 1 foot = 304.8 mm.

FIGURE R602.10.6.2 METHOD PFH—PORTAL FRAME WITH HOLD-DOWNS



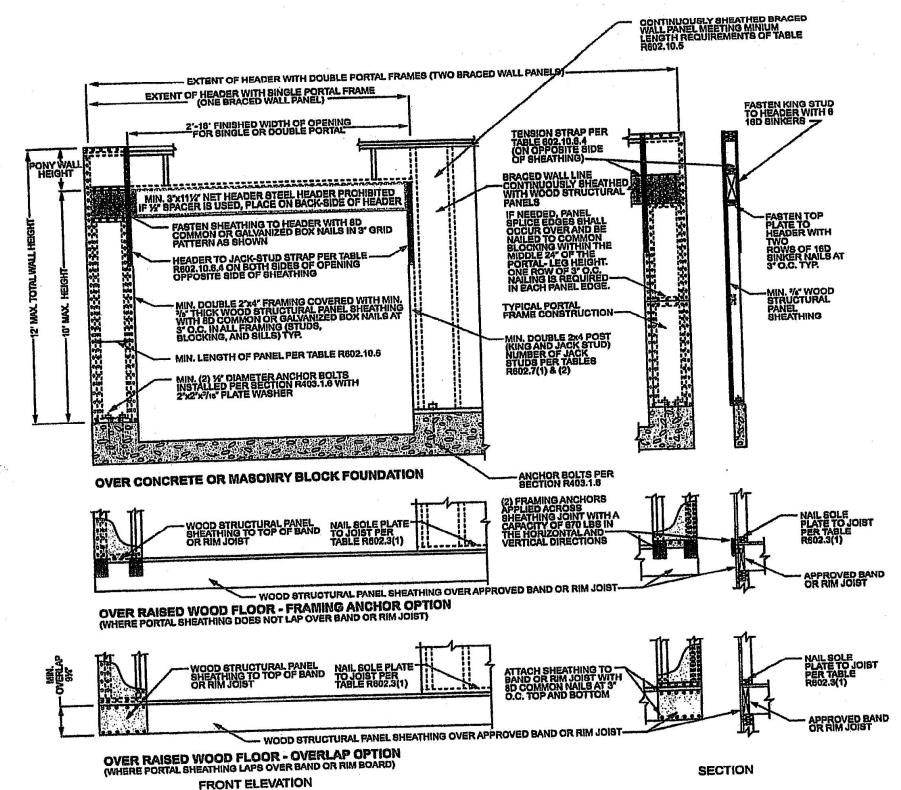
METHOD (See Table R602.10.4)		NGTH OF BRACED WALL PANELS  MINIMUM LENGTH'  (Inches)  Wall Height					CONTRIBUTING LENGTH (Inches)
		DWB, WSP, SFB, PBS, PCP, HPS, BV-WSP		48	48	48	53
GB		48	48	48	53	58	Double sided = Actual Single sided = 0.5 × Actua
LIB		55	62	69	NP	NP	Actual <sup>6</sup>
ABW	SDC A, B and C, ultimate design wind speed < 140 mph	28	32	34	38	42	48
	SDC D <sub>0</sub> , D <sub>1</sub> and D <sub>2</sub> , ultimate design wind speed < 140 mph	32	32	34	NP	NP	
CS-G		24	27	30	33	36	Actual <sup>b</sup>
	Adjacent clear opening height (inches)	- 10/2 II II I					
	≤ 64	24	27	30	33	36	Actual <sup>b</sup>
	68	26	27	30	33	36	
	72	27	27	30	33	36	
	76	30	29	30	33	-36	
CS-WSP, CS-SFB	80	32	30	30	33	36	
	84	35	32	32	33	36	
	88	38	35	33	33	36	
	92	43	37	35	35	36	
	96	48	41	38	36	36	
	100		44	40	38	38	
	104		49	43	40	39	
	108		54	46	43	41	
	112		T	50	45	43	
	116			55	48	45	
	120	_		60	52	48	
	124		_		56	51	
	128				61	54	
	132			T	66	58	
	136					62	
	140					66	
	144				T -	72	
METHOD		Portal header height					
	Table R602,10.4)	8 feet	9 feet	10 feet	11 feet	12 feet	
	Supporting roof only	16	16	16	Note c	Note c	c 46
PFH	Supporting one story and roof		24	24	Note c	Note c	
PFG		24	27	30	Note d	Note d	
	SDC A, B and C	16	18	20	Note e	Note e	
CS-PF	SDC $D_0$ , $D_1$ and $D_2$ 1 foot = 304.8 mm, 1 mile per hour =	16	18	20	Note e	Note e	Actual <sup>b</sup>

b. Use the actual length where it is greater than or equal to the minimum length.
c. Maximum header height for PFH is 10 feet in accordance with Figure R602.10.6.2, but wall height shall be permitted to be increased to 12 feet with pony wall.
d. Maximum header height for PFG is 10 feet in accordance with Figure R602.10.6.3, but wall height shall be permitted to be increased to 12 feet with pony wall.
e. Maximum header height for CS-PF is 10 feet in accordance with Figure R602.10.6.4, but wall height shall be permitted to be increased to 12 feet with pony wall.

BRACE WALL DETAILS WIND SPEED 115 MPH WIND EXPOSURE A SEISMIC DESIGN CAEGORY A

				CONNECTION CRITERIA'			
M	ETHODS, MATERIAL	MINIMUM THICKNESS	FIGURE	Fasteners	Specing		
g Methods	PFH Portal frame with hold-downs	³/ <sub>8</sub> ″		See Section R602.10.6.2	See Section R602.10.6.2		
Intermittent Bracing Methods	PFG Portal frame at garage	<sup>7</sup> / <sub>16</sub> "		See Section R602.10.6.3	See Section R602.10.6.3		
Continuous Sheathing Methods	CS-WSP	3/8"		Exterior sheathing per Table R602.3(3)	6" edges 12" field		
	Continuously sheathed wood structural panel			Interior sheathing per Table R602.3(1) or R602.3(2)	Varies by fastener		
	CS-G <sup>b,c</sup> Continuously sheathed wood structural panel adjacent to garage openings	3/8"		See Method CS-WSP	See Method CS-WSP		
	CS-PF Continuously sheathed	<sup>7</sup> / <sub>16</sub> "		See Section R602.10.6.4	See Section R602.10.6.4		
	CS-SFB <sup>d</sup> Continuously sheathed structural fiberboard	1/2" or <sup>25</sup> /32" for maximum 16" stud spacing		$1\frac{1}{2}$ " long × 0.12" dia. (for $\frac{1}{2}$ " thick sheathing) $1\frac{3}{4}$ " long × 0.12" dia. (for $\frac{25}{22}$ " thick sheathing) galvanized roofing nails	3" edges 6" field		
a. Ad b. Ap De c. Gs	hesive attachment of wall splies to panels next to gara	heathing, including Methonge door opening where sured to the D <sub>2</sub> roof covering dead load the ball the CS-G panel shall thou CS-G panel.	d GB, shall not be permitted porting gable end wall or re i shall not exceed 3 psf. be provided with a header in	to foot = 47.8 N/m <sup>2</sup> , 1 mile per hour = in Seismic Design Categories C, $D_0$ , of load only. Shall only be used on accordance with Table R602.7(1). A	one wall of the garage. In Seismic		

d. Method CS-SFB does not apply in Seismic Design Categories  $D_0$ ,  $D_1$  and  $D_2$ . e. Method applies to detached one- and two-family dwellings in Seismic Design Categories  $D_0$  through  $D_2$  only.



For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm.

FIGURE R802.10.6.4
METHOD CS-PF—CONTINUOUSLY SHEATHED PORTAL FRAME PANEL CONSTRUCTION