



01/26/2021

MiTek USA, Inc. 16023 Swingley Ridge Rd Chesterfield, MO 63017 314-434-1200

Re: 2523941

The truss drawing(s) referenced below have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Builders FirstSource (Valley Center).

Pages or sheets covered by this seal: I43385000 thru I43385096

My license renewal date for the state of Missouri is December 31, 2021.

Missouri COA: Engineering 001193



October 28,2020

**IMPORTANT NOTE:** The seal on these truss component designs is a certification that the engineer named is licensed in the  $\ensuremath{\mathsf{jurisdiction}}(s)$  identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

,Engineer





	2-4-12	7-3-4	11-7-5	15-9-1	1	20-0-0		24-2-5	26-0-0 28	28-4-11	32-8-12		37-8-8	40-0-0	ł
Plate Offsets (	(X,Y) [3:0-5-8.]	Edgel. [10:0-5-	4-4-1 4.Edae], [11	4-2-5 1:0-7-5.Edgel.	[11:0-1-2.0	-0-11]. [12	2:0-8-	-0.0-0-9]. [18:0	-5-10.0-2-	<u>0-0 0-4-11</u> 81	4-4-1		4-11-12	2-3-8	·
LOADING (ps TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	sf) 25.0 20.4/20.0 10.0 0.0 10.0	SPACIN Plate Gi Lumber Rep Stru Code IF	I <b>G-</b> ip DOL DOL ess Incr RC2018/TPI	2-0-0 1.15 1.15 NO  2014	CSI. TC BC WB Matri	0.83 1.00 0.40 x-MS		DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.70 -1.16 ) 0.45	(loc) 19 19 12	l/defl >685 >412 n/a	L/d 240 180 n/a		<b>PLATES</b> MT20 MT20HS Weight: 641 lb	<b>GRIP</b> 197/144 148/108 FT = 20%
LUMBER- TOP CHORD BOT CHORD WEBS	2x4 SPF No.2 * 1-3: 2x10 SP 24 2x4 SPF No.2 * 2-21: 2x6 SPF N 2x4 SPF No.2	Except* 400F 2.0E, 10-1 Except* No.2, 11-18,18-	3: 2x8 SP 2 21: 2x6 SP	2400F 2.0E F 2100F 1.8E			BR. TOI BO	<b>ACING-</b> P CHORD T CHORD	Structura 2-0-0 oc Rigid cei	al wood purlins ling dire	sheathing (4-0-12 m ectly appli	) directly a lax.): 3-10. ed or 10-0-	pplied or -0 oc bra	6-0-0 oc purlins cing.	, except
REACTIONS.	(size) 1=0- Max Horz 1=-8 Max Uplift 1=-6 Max Grav 1=48	-3-8, 12=0-3-8 34(LC 56) 526(LC 12), 12= 346(LC 34), 12=	-623(LC 12 -4499(LC 3	!) 4)											
FORCES. (II TOP CHORD	b) - Max. Comp./N 1-2=-2162/317 6-8=-18065/2 <sup>*</sup> 11-12=-2272/3	1ax. Ten All fo 7, 2-3=-11198/1 158, 8-9=-1732 337	orces 250 (l 343, 3-4=-´ 9/2077, 9-1	b) or less exce 14664/1758, 4 0=-14750/178	pt when sh 5=-17123/2 9, 10-11=-1	iown. 2041, 5-6: 1712/145	=-171 59,	123/2041,							
BOT CHORD	2-23=-1168/10 17-19=-1955/1 11-14=-48/443	0488, 22-23=-1 17329, 16-17=- 3	173/10545, 1667/14750	20-22=-1637/ ), 15-16=-1286	14664, 19-2 //11027, 11	20=-2036/ -15=-1276	/1806 6/109	85, 944,							
WEBS	3-23=-103/111 6-20=-1137/14 9-16=-2058/25	13, 3-22=-554/4 41, 6-19=-118/4 55, 9-17=-348/3	920, 4-22= 39, 10-15= 113, 8-19=	-2015/256, 4-2 -189/1487, 8-1 -98/888	20=-341/290 7=-869/12	68, 5-20≕ 1, 10-16≕	-350/ -456/	66, 4446,							
NOTES-															
<ul> <li>1) 3-ply truss to be connected together with 10d (0.131"x3") nails as follows: Top chords connected as follows: 2x10 - 2 rows staggered at 0-9-0 oc, 2x4 - 1 row at 0-7-0 oc, 2x8 - 2 rows staggered at 0-9-0 oc. Bottom chords connected as follows: 2x4 - 2 rows staggered at 0-9-0 oc, 2x4 - 1 row at 0-9-0 oc. Webs connected as follows: 2x4 - 1 row at 0-9-0 oc, 2x4 - 1 row at 0-9-0 oc.</li> <li>2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.</li> <li>3) Unbalanced roof live loads have been considered for this design.</li> <li>4) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=15ft; B=45ft; L=38ft; eave=5ft; Cat. II; Exp C; Enclosed; MWFRS (directional); cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60</li> <li>NYMBER</li> </ul>															
DOL=1.15) surfaces wi 6) Unbalanced 7) This truss h	; Is=1.0; Rough Ca ith slopes less that d snow loads have nas been designed	at C; Partially E n 0.500/12 in a been consider d for greater of r	xp.; Ce=1.0 ccordance v ed for this o min roof live	); Cs=1.00; Ct with IBC 1608. design. e load of 12.0 j	=1.10, Lu=5 3.4. osf or 1.00 t	50-0-0 Ra	ain su roof l	urcharge applie oad of 15.4 psf	d to all ex on overh	posed angs		Ŷ	Stress	VONAL E	
non-concur	rrent with other live	e loads.								3			~	amo	
8) Provide add	equate drainage to	o prevent water	ponding.										(	October 28,2	2020
WARNI Design val a truss sys building de is always r fabrication Safety Inf	NG - Verify design para lid for use only with MiT stem. Before use, the b esign. Bracing indicate required for stability and a, storage, delivery, erea ormation available fro	meters and READ N Fek® connectors. T uilding designer mu d is to prevent buck d to prevent collaps ction and bracing of om Truss Plate Insti	IOTES ON THI his design is bast verify the ap ling of individu e with possible trusses and tr tuste. 2670 Cra	S AND INCLUDED ased only upon pa oplicability of desig policability of desig truss web and/a personal injury a uss systems, see in Highway. Suite	MITEK REFE rameters show n parameters or chord memb nd property da ANS 203 Waldorf M	RENCE PAG vn, and is for and properly bers only. Ac mage. For g I/TPI1 Quali MD 20601	F MII- r an ind y incor ddition genera ity Crit	-7473 rev. 5/19/2020 dividual building co porate this design i al temporary and p al guidance regardin teria, DSB-89 and	D BEFORE U mponent, no nto the overa ermanent bra ng the BCSI Buildi	SE. t all acing ng Comp	ponent		16023 Chest	Tek* Swingley Ridge Rd	

		RELEASE FOR	
Job Truss	Truss Type		140005000
2523941 A1		AS NOTED ON PLANS REVIEW	143385000
		DEVELOPMENT SERVICES 3	Job Reference (optional)
Builders FirstSource (Valley Center),	Valley Center, KS - 67147,	LEE'S SUMMIT, MISSOURI8.240 s M	ar 9 2020 MiTek Industries, Inc. Tue Oct 27 18:38:45 2020 Page 2
		ID:VPVgvFnP0P0b1i2tZrl	DaezdKbx-avoDS3soSKbTZuid?fvSKKWkePLhK0l5FeRCP3vP7bu

01/26/2021

# NOTES-

10) All plates are 2x4 MT20 unless otherwise indicated.

11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

Haring at joint(s) 1 considers parallel to grain value using ANSI/TP1 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=626, 12=623.

- 14) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

15) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

16) Use Simpson Strong-Tie LUS24 (4-10d Girder, 2-10d Truss, Single Ply Girder) or equivalent spaced at 4-0-0 oc max. starting at 3-8-8 from the left end to 35-8-8 to connect truss(es) to front face of bottom chord.

17) Fill all nail holes where hanger is in contact with lumber.

18) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 594 lb down and 136 lb up at 1-8-8 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

# LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-29=-73, 3-29=-51, 3-10=-61, 10-11=-51, 11-13=-51, 11-28=-20, 14-31=-20

Concentrated Loads (lb)

Vert: 18=-334(F) 23=-338(F) 22=-334(F) 20=-334(F) 19=-334(F) 28=-594(F) 41=-292(F) 42=-338(F) 43=-334(F) 44=-334(F) 45=-334(F) 46=-339(F) 47=-339(F) 48=-339(F) 49=-300(F) 50=-298(F) 51=-404(F)





Scale = 1:72.3



2-4-	·12 9-3-4	16-5-11	23-6-5	26-2-0	28-0-0	30-8-12	37-8-8	40-0-0	
2-4-	-12 6-10-8	7-2-7	7-0-11	2-7-11	1-10-0	2-8-12	6-11-12	2-3-8	
Plate Offsets (X,Y)-	- [2:0-7-4,Edge], [3:0-5-8,Edg	e], [7:0-5-8,Edge], [8:0-7-14,0	0-0-1], [8:0-3-8,0-0-8], [9:0-8-	0,0-0-5], [	17:0-0-0	,0-1-12], [1	7:0-4-0,Edge], [18:0-1-12	,0-0-0],	
	[19:0-3-8,0-1-8]								

	/								
LOADING (psf) TCLL (roof) 25.0 Snow (Pf/Pg) 20.4/20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2018/TPI2014	<b>CSI.</b> TC 0.87 BC 1.00 WB 0.40 Matrix-AS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.51 18-19 -0.95 18-19 0.48 9	l/defl L/d >938 240 >504 180 n/a n/a	PLATES MT20 MT20HS Weight: 194 lb	<b>GRIP</b> 197/144 148/108 FT = 20%		
LUMBER- TOP CHORD 2x4 SPF 1650F 1-3: 2x10 SP 24 BOT CHORD 2x4 SPF No.2 *E 8-11: 2x6 SPF N WEBS 2x4 SPF No.2 REACTIONS. (size) 1=0- Max Horz 1=-9 Max Uplif 1=-1: Max Grav 1=18	1.5E *Except* 00F 2.0E, 7-10: 2x8 SP 2400F 2.0E Except* lo.2, 8-17,17-20: 2x4 SPF 1650F 1.5E 3-8, 9=0-3-8 6(LC 14) 38(LC 16), 9=-170(LC 16) 100(LC 2), 9=1866(LC 2)	BF TC BC	RACING- PP CHORD	Structural wood 2-0-0 oc purlins Rigid ceiling dire 2-2-0 oc bracing	sheathing direc (2-2-0 max.): 3- ectly applied. E : 2-21	ctly applied, except -7. xcept:			
FORCES. (lb) - Max. Comp./M TOP CHORD 1-2=-801/122, 7-8=-3706/411 BOT CHORD 2-21=-259/333 12-13=-262/3 WEBS 3-21=0/294, 7- 7-18=-131/156	FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown.         ICOP CHORD       1-2=-801/122, 2-3=-3639/410, 3-4=-4754/528, 6-7=-4754/528, 7-8=-3706/411, 8-9=-765/131         3OT CHORD       2-21=-259/330, 19-21=-257/3336, 18-19=-376/4754, 16-18=-262/3381, 13-16=-274/3326, 12-13=-262/3381, 8-12=-264/3373         WEBS       3-21=0/294, 7-12=0/323, 3-19=-136/1613, 4-19=-633/126, 6-18=-623/127, 7.14=0/322, 7.14=0/323, 3-19=-136/1613, 4-19=-633/126, 6-18=-623/127, 7.14=0/323,								
<ul> <li>NOTES-</li> <li>1) Unbalanced roof live loads ha</li> <li>2) Wind: ASCE 7-16; Vult=115m</li> <li>II; Exp C; Enclosed; MWFRS</li> <li>13-6-3, Interior(1) 13-6-3 to 31 exposed; end vertical left and grip DOL=1.60</li> <li>3) TCLL: ASCE 7-16; Pr=25.0 pr DOL=1.15); Is=1.0; Rough Ca surcharge applied to all exposed</li> <li>4) Unbalanced snow loads have</li> <li>5) This truss has been designed non-concurrent with other live</li> <li>6) Provide adequate drainage to</li> <li>7) All plates are MT20 plates un</li> <li>8) All plates are 2x4 MT20 unles</li> <li>9) This truss has been designed</li> <li>10) Bearing at joint(s) 1 conside</li> </ul>	ave been considered for this design. hph (3-second gust) Vasd=91mph; TCDL (directional) and C-C Exterior(2E) 0-1-1: 0-8-12, Exterior(2R) 30-8-12 to 34-11-11 d right exposed; C-C for members and for sf (roof LL: Lum DOL=1.15 Plate DOL=1 at C; Partially Exp.; Ce=1.0; Cs=1.00; Ct sed surfaces with slopes less than 0.500 been considered for this design. for greater of min roof live load of 12.0 p loads. prevent water ponding. less otherwise indicated. so otherwise indicated. for a 10.0 psf bottom chord live load no rs parallel to grain value using ANSI/TPI	=6.0psf; BCDL=4.2psf; h 2 to 2-10-9, Interior(1) 2- , Interior(1) 34-11-11 to 4 ces & MWFRS for reacti .15); Pg=20.0 psf; Pf=20 =1.10, Lu=50-0-0; Min. ft /12 in accordance with IE osf or 1.00 times flat roof nconcurrent with any oth 1 angle to grain formula.	h=15ft; B=45ft; L 10-9 to 9-3-4, E 40-11-0 zone; ca ons shown; Lum .4 psf (Lum DOI at roof snow load 3C 1608.3.4. load of 15.4 psf er live loads. . Building desig	=24ft; eave=5ft; C tterior(2R) 9-3-4 t antilever left and r uber DOL=1.60 pl _=1.15 Plate d governs. Rain on overhangs	Cat. o ight ate	ANDREW THOMAS JOHNSON NUMBER PE-201701899			

11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) Contifme@380@pate 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



October 28,2020

			RELEASE FOR	
Job	Truss	Truss Type	CONSTRUCTION Ply	
2523041	42	Hip	AS NOTED ON PLANS REVIEW	143385001
2020041	~ <u>~</u>	1 lip	DEVELOPMENT SERVICES	Job Reference (optional)
Builders FirstSource	(Valley Center),	Valley Center, KS - 67147,	LEE'S SUMMIT, MISSOURI8.240 s M	ar 9 2020 MiTek Industries, Inc. Tue Oct 27 18:38:52 2020 Page 2
			ID:VPVqvFnP0P0b1j2tZ	IOqezdKbx-zljtwSxBpUTUvykzvdX66pJwHDkHTBb7sDe499yP7bn
NOTES-			04/26/2024	

11/26/2021
 12) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 13) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the battom chord

bottom chord.

14) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.









				28-8-12	2		
2-4-12	11-3-4	20-0-0	23-3-3 26-	-2-0 28-0-0	33-2-10	37-8-8 40-0-0	1
2-4-12	8-10-8	8-8-12	3-3-3 2-1	<u>0-13 <sup> </sup> 1-10-0 0<sup> </sup>8-12</u>	4-5-14	4-5-14 2-3-8	1
Plate Offsets (X,Y) [3:0-4-0,	0-2-13], [7:0-4-0,0-2-13], [9:0-0-2,Edge]	[9:0-3-8,0-0-8], [10:0-8-	-0,0-0-5]				
LOADING (psf)	SPACING. 2-0-0	120	DEEL	in (loc)	l/defl L/d		CDID
TCLL (roof) 25.0		CSI.				I LAILS	407/444
Snow (Pf/Pg) 20 4/20 0	Plate Grip DOL 1.15	10 0.77	Vert(LL)	-0.38 14-16	>999 240	MT20	197/144
	Lumber DOL 1.15	BC 0.95	Vert(CT)	-0.76 14-16	>628 180	MT20HS	148/108
TODE 10.0	Rep Stress Incr YES	WB 0.86	Horz(CT	) 0.45 10	n/a n/a		
BCLL 0.0	Code IRC2018/TPI2014	Matrix-AS		,		Weight: 209 lb	FT - 20%
BCDL 10.0	Code 11(02010/11/12014	Matrix-A0				Weight. 209 lb	11 - 2070
LUMBER-		E	BRACING-				
TOP CHORD 2x4 SPF No.2 *	Except*	Т	OP CHORD	Structural wood	sheathing directly ap	oplied, except	
1-3: 2x10 SP 24	100F 2 0F 7-11 2x8 SP 2400F 2 0F			2-0-0 oc purlins	(2-2-0 max): 3-7		
	Except*			Digid coiling dir	(2 2 0 max). 0 7.		
BUTCHORD 2X4 SPF NU.2				Rigia celling and	ectiy applied.		
2-17,9-15: 2x4 \$	SPF 1650F 1.5E, 9-12: 2x6 SPF No.2						
WEBS 2x4 SPF No.2							
OTHERS 2x4 SPE No 2							
REACTIONS. (size) 1=0-	·3-8, 10=0-3-8						
Max Horz 1=-1	16(LC 14)						
Max Unlift 1=-1	38(I C 16) 10=-170(I C 16)						
Max Opint 1= 1	$210(1 \times 2)$ 10-1966(1 \times 2)						
	510(LC 2), 10=1000(LC 2)						
FORCES. (lb) - Max. Comp./M	lax. Ten All forces 250 (lb) or less exc	ept when shown.					
TOP CHORD 1-2=-801/123.	. 2-3=-3330/373. 3-4=-3003/392. 4-6=-3/	603/445. 6-7=-2990/396					
7-8-3/17//1	4 8-04263/485 0-10765/131		,				
	+, 0-9=-4200/400, 9-10=-700/101	10 11 070/0077 0 10	070/0077				
BOT CHORD 2-18=-204/298	32, 16-18=-282/3545, 14-16=-281/3552,	13-14=-370/3977, 9-13	=-370/3977				
WEBS 3-18=-5/816, 7	7-14=-100/1243, 4-18=-914/107, 6-14=-9	931/109, 8-14=-1245/19	0				
NOTES-							
1) Unbalanced root live loads ha	ave been considered for this design.						
<ol><li>Wind: ASCE 7-16; Vult=115n</li></ol>	nph (3-second gust) Vasd=91mph; TCD	L=6.0psf; BCDL=4.2psf;	h=15ft; B=45ft; L	_=24ft; eave=5ft; (	Cat.		
II; Exp C; Enclosed; MWFRS	(directional) and C-C Exterior(2E) 0-1-1	2 to 2-10-9, Interior(1) 2	-10-9 to 11-3-4, I	Exterior(2R) 11-3-	-4 to		
15-6-3 Interior(1) 15-6-3 to 2	28-8-12 Exterior(2R) 28-8-12 to 33-2-10	Interior(1) 33-2-10 to 4	0-11-0 zone: cant	tilever left and rid	ht		
	d right averaged (C C for mambers and for		tione chouse Lun		late		
exposed; end ventical left an	a right exposed;C-C for members and id	ices & IVIVERS for read	cuons snown; Lun	nber DOL=1.60 p	late	Jun	S
grip DOL=1.60						OF MIC	S
3) TCLL: ASCE 7-16; Pr=25.0 p	sf (roof LL: Lum DOL=1.15 Plate DOL=	1.15); Pq=20.0 psf; Pf=2	0.4 psf (Lum DO	L=1.15 Plate		B R OT MISS	1 AV
DOI =1 15): Is=1 0: Rough C	at C: Partially Exp : Ce=1.0: Cs=1.00: C	t=1 10 Lu=50-0-0 Min	flat roof snow loa	d governs Rain		935	U.V.
	and surfaces with slapes less than 0 500			a govorno. Train	a	AS /	New Y
surcharge applied to all expo	sed surfaces with slopes less than 0.500	J/12 in accordance with	IBC 1608.3.4.		H	S' ANDREW	15 M
<ol><li>Unbalanced snow loads have</li></ol>	been considered for this design.				A	THOMAS	V. X
<ol><li>This truss has been designed</li></ol>	for greater of min roof live load of 12.0	psf or 1.00 times flat roc	of load of 15.4 pst	f on overhangs		Inconst	N . N
non-concurrent with other live	e loads	•		•	/ α 🛪	C JOHNSON	1 × 0
6) Provide adequate drainage to	a provent water pending					R'a'a a hila	a a h de
o) Frovide adequate dialitage to	prevent water ponuling.						~ ~ ~ ~
<ol><li>All plates are M120 plates un</li></ol>	iless otherwise indicated.					NUMBER	IXU
<ol><li>This truss has been designed</li></ol>	for a 10.0 psf bottom chord live load no	nconcurrent with any ot	her live loads.		N7	DE 201701900	12121
9) Bearing at joint(s) 1 consider	s parallel to grain value using ANSI/TPL	1 angle to grain formula	. Building design	er should verifv	N.	C FE-201/01899	S/AD
capacity of bearing surface					XX	1001	154
(a) Drawids are a		and the state of the state	400 lb !!!!		N	1 Se	JU'A
10) Provide mechanical connec	tion (by others) of truss to bearing plate	capable of withstanding	100 ib uplift at jo	int(s) except (jt=ll	o)	VI ONIAT ET	A
1=138, 10=170.						WAL	4
11) This truss is designed in ac	cordance with the 2018 International Re	sidential Code sections	R502.11.1 and R	802.10.2 and		Vanne	-
referenced standard ANSI/						October 29 3	2020
Telefenceu Stanuaru ANSI/I	- 1 - 1 - E					OCIODEI 20,2	.020
Continued on page 2							
1 4N							



			RELEASE FOR	
Job	Truss	Truss Type	CONSTRUCTION Ply	140005000
2523941	A3	Hin	AS NOTED ON PLANS REVIEW	143385002
2020011			DEVELOPMENT SERVICES	Job Reference (optional)
Builders FirstSource (Valley	Center), Valley Center, K	S - 67147,	LEE'S SUMMIT, MISSOURI8.240 s M	ar 9 2020 MiTek Industries, Inc. Tue Oct 27 18:38:54 2020 Page 2
			ID:VPVqvFnP0P0b1j2tZrl0	qezdKbx-whqdK8zRL5kC8GuM12aaBEOIL1Qdx_IQKX7BE2yP7bl
NOTES-			01/26/2021	
<ol><li>This truss design requi</li></ol>	res that a minimum of 7/16" :	structural woo	d sheathing be applied directly to the top chord and	1/2" gypsum sheetrock be applied directly to the
bottom chord.				

bottom chord.
13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.





L		13-3-4	20-1-12	26-2-0	26 <sub>1</sub> 8-12	33-2-10	40-0-0				
		13-3-4	6-10-8	6-0-4	0-6-12	6-5-14	6-9-6				
Plate Offsets (	(X,Y) [2:0-7-12	2,Edge], [3:0-5-4,Edge], [5:0-6-12,0-1-4],	[14:0-2-4,0-1-12]								
LOADING (ps TCLL (roof) Snow (Pf/Pg) TCDL	sf) 25.0 20.4/20.0 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	<b>CSI.</b> TC 0.89 BC 0.98 WB 0.65	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.36 17-21 -0.83 17-21 0.33 9	l/defl L/d >999 240 >578 180 n/a n/a	PLATES MT20 MT20HS	<b>GRIP</b> 197/144 148/108			
BCLL	0.0	Code IRC2018/TPI2014	Matrix-AS				Weight: 199 lb	FT = 20%			
LUMBER- TOP CHORD BOT CHORD WEBS SLIDER	2x4 SPF No.2 *I 1-3: 2x10 SP 24 2x4 SPF No.2 *I 2-16: 2x4 SPF 1 2x4 SPF No.2 Right 2x4 SPF N	Except* 00F 2.0E Except* 650F 1.5E No.2 2-6-0	B T( B(	RACING- DP CHORD DT CHORD	Structural wood 2-0-0 oc purlins Rigid ceiling dir 10-0-0 oc braci	d sheathing direct s (2-2-0 max.): 3-5 ectly applied. Ex ng: 2-17	ly applied, except 5. cept:				
REACTIONS.	(size) 1=0- Max Horz 1=-1 Max Uplift 1=-1 Max Grav 1=18	3-8, 9=0-3-8 33(LC 14) 36(LC 16), 9=-170(LC 16) 314(LC 2), 9=1867(LC 2)									
FORCES. (II TOP CHORD BOT CHORD WEBS	FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown.         FOP CHORD       1-2=-802/117, 2-3=-3102/362, 3-4=-3085/422, 4-5=-3087/423, 5-7=-3097/393, 7-9=-3102/362         3OT CHORD       2-17=-176/2759, 15-17=-174/2764, 14-15=-179/2708, 9-11=-246/2692         WEBS       3-17=0/372, 3-15=-51/611, 4-15=-650/127, 5-15=-54/686, 7-11=-315/100, 5-14=-18/599, 11-14=-246/2646, 7-14=-227/253										
<ul> <li>NOTES-</li> <li>1) Unbalanced roof live loads have been considered for this design.</li> <li>2) Wind: ASCE 7-16; Vul=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=15ft; B=45ft; L=24ft; eave=5ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) 0-1-12 to 2-10-9, Interior(1) 2-10-9 to 13-3-4, Exterior(2R) 13-3-4 to 17-6-3, Interior(1) 17-6-3 to 26-8-12, Exterior(2R) 26-8-12 to 30-11-11 to 40-11-0 zone; cantilever left and right exposed; C-C for members and forces &amp; MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60</li> <li>3) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=20.4 psf (Lum DOL=1.15 Plate DOL=1.67); Pi=25.0 psf (roof LL: Lind DOL=1.15); Pg=20.0 psf; Pf=20.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat c; Partially Exp; Ce=1.0; CS=1.00; Ct=-1.10, Lu=50-0-0; Min. flat roof now load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.</li> <li>4) Unbalanced snow loads have been considered for this design.</li> <li>5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.</li> <li>6) Provide adequate drainage to prevent water ponding.</li> <li>7) All plates are MT20 plates unless otherwise indicated.</li> <li>8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.</li> <li>9) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.</li> <li>10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (it=lb) 1=136, 9=170.</li> <li>11) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and Cottober 28,2020</li> </ul>											
🛝 WARNI	NG - Verify design parar	meters and READ NOTES ON THIS AND INCLUDE	MITEK REFERENCE PAGE M	II-7473 rev. 5/19/2020	BEFORE USE.						



			RELEASE FO	R	
Job	Truss	Truss Type	CONSTRUCTIO	Ply Ply	140005000
2523941	A4	Hin	AS NOTED ON PLANS	REVIEW	143385003
2020011			DEVELOPMENT SER	VICES	Job Reference (optional)
Builders FirstSource (Valley	Center), Valley Center, K	S - 67147,	LEE'S SUMMIT, MISS	SOURI <mark>8.240 s M</mark>	ar 9 2020 MiTek Industries, Inc. Tue Oct 27 18:38:56 2020 Page 2
				ID:VPVqvFnP0P0b	1j2tZrlOqezdKbx-s4yNlp_hsi_vOa1k8Tc2HfTbvq5aPxcinrcIIwyP7bj
NOTES-			01/26/2021		
<ol><li>This truss design requi</li></ol>	res that a minimum of 7/16" :	structural woo	d sheathing be applied directly to th	e top chord and	1/2" gypsum sheetrock be applied directly to the
bottom chord.					

13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.





Scale = 1:74.4



							26-0-8		31-2	-6			
2-4-12	8-9-10	15-3-4	20-	1-0	1	24-8-12	25-8-8	29-3-8	29 <sub>1</sub> 7 <sub>1</sub> 8	1	36-4-0	1	40-0-0
2-4-12	6-4-14	6-5-10	4-8	12	1	4-8-12	0-11-12	3-3-0	0-4-0	1	5-1-10	1	3-8-0
							0 4 0		164	1.4			

Plate Offsets (X	(,Y) [2:0-7-4,I	Edge], [10:0-4-3,0-2-0]									
LOADING (psf TCLL (roof) Snow (Pf/Pg) 2 TCDL BCLL BCDL	) 25.0 20.4/20.0 10.0 0.0 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2018/TI	2-0-0 1.15 1.15 YES PI2014	CSI. TC BC WB Matri	0.68 0.88 0.59 ix-AS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in ( -0.31 14 -0.74 14 ) 0.35	loc) l/defl -17 >999 -17 >650 10 n/a	L/d 240 180 n/a	PLATES MT20 MT20HS Weight: 201 lb	<b>GRIP</b> 197/144 148/108 FT = 20%
	1010										
TOP CHORD	2x4 SPF No.2 *I 1-3: 2x10 SP 24	Except* 00F 2.0E			T	OP CHORD	Structural v 2-0-0 oc pu	wood sheathi urlins (3-6-10	ng directly app max.): 4-6.	blied, except	
BOT CHORD	2x4 SPF 1650F 10-12: 2x8 SP 2	1.5E *Except* 400F 2.0E, 15-16: 2x4 S	PF No.2		E V	SOT CHORD VEBS	Rigid ceilin 1 Row at m	g directly app hidpt	olied. 3-17, 5-17	7, 5-14, 8-14	
WEBS OTHERS	2x4 SPF No.2 2x4 SPF No.2										
REACTIONS.	(size) 1=0- Max Horz 1=-1 Max Uplift 1=-1 Max Grav 1=18	3-8, 10=0-3-8 53(LC 14) 38(LC 16), 10=-173(LC 1 310(LC 2), 10=1859(LC 2	16) ?)								
FORCES. (lb) TOP CHORD	) - Max. Comp./N 1-2=-801/118, 6-8=-2860/385	lax. Ten All forces 250 2-3=-3637/427, 3-4=-28 5, 8-9=-3727/445, 9-10=-1	(lb) or less exc 60/385, 4-5=-24 5269/553	ept when sh 150/375, 5-6	nown. 6=-2456/379	,					
BOT CHORD	2-18=-286/332 10-12=-450/4	22, 17-18=-285/3327, 14- 707	17=-167/2584,	13-14=-291	1/3301, 12-1	3=-438/4547,					
WEBS	3-17=-1134/18 9-13=-1291/15	82, 4-17=-61/862, 5-17=- 51, 9-12=-39/851, 8-13=-	426/62, 5-14=-4 0/468, 8-14=-11	420/61, 6-14 101/176	4=-57/856,						
NOTES- 1) Unbalanced 2) Wind: ASCE II; Exp C; En 19-6-3, Interi exposed; en grip DOL=1.6 3) TCLL: ASCE DOL=1.15); I surcharge ap 4) Unbalanced 5) This truss ha non-concurre 6) Provide adee 7) All plates are	roof live loads ha 7-16; Vult=115m closed; MWFRS ior(1) 19-6-3 to 2 nd vertical left and 60 57-16; Pr=25.0 p Is=1.0; Rough Ca oplied to all expos snow loads have as been designed ent with other live quate drainage to a MT20 plates un	ave been considered for t nph (3-second gust) Vaso (directional) and C-C Ex 4-8-12, Exterior(2R) 24-8 d right exposed;C-C for n sf (roof LL: Lum DOL=1. at C; Partially Exp.; Ce=1 sed surfaces with slopes been considered for this for greater of min roof line bloads. p prevent water ponding. less otherwise indicated.	this design. d=91mph; TCDI terior(2E) 0-1-1 3-12 to 28-11-11 nembers and fo 15 Plate DOL=1 .0; Cs=1.00; Ct less than 0.500 s design. ve load of 12.0	L=6.0psf; B( 2 to 2-10-9, I, Interior(1) rces & MWI I.15); Pg=2( i=1.10, Lu=5 i/12 in acco psf or 1.00 f	CDL=4.2psf; , Interior(1) 2 ) 28-11-11 tc FRS for read 0.0 psf; Pf=2 50-0-0; Min. rdance with times flat roo	h=15ft; B=45ft; L -10-9 to 15-3-4, I 40-11-0 zone; c: ttions shown; Lun 0.4 psf (Lum DO flat roof snow loa IBC 1608.3.4. of load of 15.4 psf	=24ft; eave= Exterior(2R) antilever left hber DOL=1 L=1.15 Plate d governs.	=5ft; Cat. 15-3-4 to and right .60 plate Rain gs	Q.	ANDREW THOMAS JOHNON NUMBER PE-201701899	BURN *
<ul> <li>3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.</li> <li>3) Bearing at joint(s) 1, 10 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.</li> <li>10) Browide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (it-lb).</li> </ul>											

10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=138, 10=173.

#### Continued on page 2

🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED WITH KREPERENCE PAGE MIL-(4/5 fev. or 19/2/2/0/BEFORE USE. Design valid for use only with MITEK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/ITPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



			RELEASE FOR	
Job	Truss	Truss Type	CONSTRUCTION Ply	
25230/1	45	Hip	AS NOTED ON PLANS REVIEW	143385004
2020041	~ ~	1 lip	DEVELOPMENT SERVICES	Job Reference (optional)
Builders FirstSource (Valle	y Center),	Valley Center, KS - 67147,	LEE'S SUMMIT, MISSOURI8.240 s M	ar 9 2020 MiTek Industries, Inc. Tue Oct 27 18:38:58 2020 Page 2
			ID:VPVqvFnP0P0b1j2tZrlO	qezdKbx-oS48AV0yOKEddtB7GueWM4Y_kepXtr??F95ONpyP7bh
NOTES-			01/26/2021	

01/26/2021
 11) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 12) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the top chord and 1/2" gypsum sheetr

bottom chord.

13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



			RELEAS	SE FOR					
Job	Truss	Truss Type	CONSTR	UCTION Ply	'				
2523941	A6	Hip		LANS REVIEW	1				143385005
Builders FirstSource (Valley	Center). Valley Center, k	S - 67147.	LEE'S SUMMI	T. MISSOURI8.240	0 s Mai	Job Reference (c r 9 2020 MiTek Ir	ptional) idustries, Inc. Ti	Je Oct 27 18:39:0	0 2020 Page 1
	,	,		ID:VPVqvFnP0P0b1	1j2tZrC	QezdKbx-krCubE	31CwxULsBLWN	Jg_RVeHvSTPLo	sliTaVRiyP7bf
2-4-1	2 9-8-6	).	7-3-4 01/26	20-21 25-8-8	:	29-6-6 29-7-8	36-4-0	40-0-0	<u>40-11-</u> 0
2-4-1	2 ' 7-3-10	' <sup>7</sup>	-6-14 5-	5-8 2-11-12	'	3-9-14 0-1'-2	6-8-8	' 3-8-0	0-11-0

Scale = 1:76.0



2-4-12 2-4-12 17-3-4 29-6-6 40-0-0 9-8-6 22-8-12 25-8-8 29-7-8 36-4-0 7-3-10 7-6-14 0-1-2 5-5-8 2-11-12 3-9-14 6-8-8 3-8-0 Plate Offsets (X,Y)--[2:0-7-4,Edge], [9:0-4-3,0-2-0] LOADING (psf) SPACING-2-0-0 CSI. DEFL. in (loc) l/defl L/d PLATES GRIP 25.0 TCLL (roof) Plate Grip DOL 1.15 тс 0.89 Vert(LL) -0.32 11-12 >999 240 MT20 197/144 Snow (Pf/Pg) 20.4/20.0 Lumber DOL 1.15 BC 0.92 Vert(CT) -0.61 11-12 >790 180 MT20HS 148/108 TCDL 10.0 Rep Stress Incr YES WB 0.44 Horz(CT) 0.37 MT18HS 197/144 9 n/a n/a BCLL 0.0 Code IRC2018/TPI2014 Weight: 204 lb FT = 20% Matrix-AS BCDL 10.0 LUMBER-BRACING-TOP CHORD 2x4 SPF No.2 \*Except\* TOP CHORD Structural wood sheathing directly applied, except 1-3: 2x10 SP 2400F 2.0E 2-0-0 oc purlins (3-7-1 max.): 4-5. BOT CHORD 2x4 SPF No.2 \*Except\* BOT CHORD Rigid ceiling directly applied. 9-11: 2x8 SP 2400F 2.0E, 11-14: 2x4 SPF 1650F 1.5E WEBS 3-15, 4-13, 6-13, 8-12 1 Row at midpt WEBS 2x4 SPF No.2 2x4 SPF No.2 OTHERS REACTIONS. (size) 9=0-3-8, 1=0-3-8 Max Horz 1=-172(LC 14) Max Uplift 9=-173(LC 16), 1=-138(LC 16) Max Grav 9=1859(LC 2), 1=1810(LC 2) FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. TOP CHORD  $1\hbox{-}2\hbox{-}801/118, 2\hbox{-}3\hbox{-}3691/417, 3\hbox{-}4\hbox{-}2685/380, 4\hbox{-}5\hbox{-}2239/380, 5\hbox{-}6\hbox{-}2660/383, 5\hbox{-}2660/383, 5\hbox{-}2660/380, 5\hbox{-}2660/380, 5\hbox{-}2660/380, 5\hbox{-}2660/380, 5\hbox{-}2660/380, 5\hbox{-}2660/380, 5\hbox{-}2660/380,$ 6-8=-3643/423, 8-9=-5416/564 BOT CHORD 2-17=-269/3361, 15-17=-267/3367, 13-15=-103/2241, 12-13=-254/3202, 11-12=-454/4702, 9-11=-463/4864 WEBS 3-17=0/312, 3-15=-1270/192, 4-15=-37/744, 5-13=-53/777, 6-13=-1156/174, 8-12=-1577/203, 8-11=-28/883, 6-12=0/511

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=15ft; B=45ft; L=24ft; eave=5ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) 0-1-12 to 2-10-9, Interior(1) 2-10-9 to 17-3-4, Exterior(2R) 17-3-4 to 21-6-3, Interior(1) 21-6-3 to 22-8-12, Exterior(2R) 22-8-12 to 26-11-11, Interior(1) 26-11-11 to 40-11-0 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=20.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.

4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.

Provide adequate drainage to prevent water ponding.

7) All plates are MT20 plates unless otherwise indicated

8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads

9) Bearing at joint(s) 9, 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 9=173, 1=138.

#### Continued on page 2

🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design valid to use only design parameters and READ NOTES ON THIS AND INCLUDED WITH REPORT PAGE MIT 473 1647 301 192/2020 DEFORE USE. Design valid for use only with MITEK connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent colleges with possible personal injury and property damage. For general quidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11 Qu** Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





			RELEASE FOR	
Job	Truss	Truss Type	CONSTRUCTION Ply	
2523941	AG	Hin	AS NOTED ON PLANS REVIEW	143385005
2020011	, 10	1.112	DEVELOPMENT SERVICES	Job Reference (optional)
Builders FirstSource (\	/alley Center),	Valley Center, KS - 67147,	LEE'S SUMMIT, MISSOURI8.240 s M	ar 9 2020 MiTek Industries, Inc. Tue Oct 27 18:39:00 2020 Page 2
			ID:VPVqvFnP0P0b1j2tZr	OqezdKbx-krCubB1CwxULsBLWNJg_RVeHvSTPLosliTaVRiyP7bf
NOTES-			01/26/2021	

11) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 12) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord

bottom chord.

13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.





	2-4-12	10-6-0		19-3-4 8-9-4	20-8-12	28-8-12	2 29 <sub>7</sub> 2 <u>-0</u> 0-5-4	36-4-0		3-8-0	
Plate Offsets (X,Y)	[6:0-2-4,E	idge], [10:0-4-3,0-2-0]		004	100	000	004	120		000	
LOADING (psf) TCLL (roof) Snow (Pf/Pg) 20.4 TCDL BCLL BCDL	25.0 1/20.0 10.0 0.0 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2018/TPI	2-0-0 1.15 1.15 YES 2014	<b>CSI.</b> TC BC WB Matrix	0.85 0.96 0.50 (-AS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.36 12-13 -0.67 12-13 0.39 10	l/defl L >999 24 >709 18 n/a n	/d 40 80 //a	PLATES MT20 MT20HS MT18HS Weight: 215 lb	<b>GRIP</b> 197/144 148/108 197/144 FT = 20%
	10.0										
TOP CHORD 2x. 5-6 BOT CHORD 2x. 10 WEBS 2x.	4 SPF No.2 *E 5: 2x6 SPF No 4 SPF No.2 *E -12: 2x8 SP 24 4 SPF No.2	xcept* .2, 1-3: 2x10 SP 2400F 2. xcept* 400F 2.0E, 12-15: 2x4 SP		BR TC BC WE	P CHORD T CHORD BS	Structural wood 2-0-0 oc purlins Rigid ceiling dir 1 Row at midpt	sheathing dir (4-7-14 max. ectly applied. 4	rectly appli ): 5-6. I-16, 7-14,	ed, except 9-13, 6-16		
REACTIONS.       (size)       10=0-3-8, 1=0-3-8         Max Horz       1=-191(LC 14)         Max Uplift       10=-173(LC 16), 1=-138(LC 16)         Max Grav       10=1873(LC 39), 1=1845(LC 39)											
<b>FORCES.</b> (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 1-2=-818/115, 2-3=-3666/366, 3-4=-3771/466, 4-5=-2541/365, 5-6=-2195/349, 6-72642/356, 7-93704/393, 9-105560/538											
BOT CHORD 2	6-7=-2642/356, 7-9=-3704/393, 9-10=-5560/538 30T CHORD 2-18=-207/3312, 16-18=-134/2685, 14-16=-60/2209, 13-14=-220/3266, 12-13=-435/4823,										
WEBS 3	-18=-761/175 -14=-1241/18	92 , 4-18=-126/1127, 4-16=-8 9, 7-13=0/550, 9-13=-169	386/168, 5-16=- 2/218, 9-12=-19	-146/957, 6 9/919, 6-16	-14=-68/724, =-317/275						
NOTES- 1) Unbalanced roo 2) Wind: ASCE 7-1 II; Exp C; Enclos 20-8-12, Exterio and right exposs 3) TCLL: ASCE 7- DOL=1.15); Is=' surcharge applit 4) Unbalanced snc 5) This truss has b non-concurrent 6) Provide adequa 7) All plates are M 8) This truss has b 9) Bearing at joint (capacity of bear 10) Provide mecha 10=173, 1=138 11) This truss is de referenced sta	f live loads ha 6; Vult=115m sed; MWFRS i r(2R) 20-8-12 ad;C-C for mei 16; Pr=25.0 ps I.0; Rough Ca dt to all expose een designed with other live te drainage to T20 plates uni- een designed with other live te drainage to T20 plates uni- een designed s) 10, 1 consic ing surface. nical connecti 3. signed in acc ndard ANSI/TI	ve been considered for thi ph (3-second gust) Vasd= (directional) and C-C Exte to 24-11-11, Interior(1) 24 mbers and forces & MWF of (roof LL: Lum DOL=1.15 t C; Partially Exp.; Ce=1.0 ed surfaces with slopes le been considered for this been considered for this for greater of min roof live loads. prevent water ponding. ess otherwise indicated. for a 10.0 psf bottom choi fers parallel to grain value on (by others) of truss to l ordance with the 2018 Inte Pl 1.	is design. 91mph; TCDL= rior(2E) 0-1-12 I-11-11 to 40-17 RS for reactions Plate DOL=1. <sup>2</sup> y; Cs=1.00; Ct= ress than 0.500/1 design. I load of 12.0 ps rd live load none using ANSI/TF bearing plate ca ernational Resid	=6.0psf; BC to 2-10-9, 1-0 zone; c s shown; L 15); Pg=20 1.10, Lu=5 12 in accor sf or 1.00 ti concurrent Pl 1 angle t apable of w dential Cod	DL=4.2psf; h Interior(1) 2-1 antilever left : umber DOL= 0 psf; Pf=20. 0-0-0; Min. fla dance with IB mes flat roof with any othe o grain formu ithstanding 1 le sections R	=15ft; B=45ft; L 0-9 to 19-3-4, E and right expose 1.60 plate grip I 4 psf (Lum DOI tt roof snow load C 1608.3.4. load of 15.4 psf er live loads. la. Building des 00 lb uplift at joi	=24ft; eave=5ft; Exterior(2E) 19-3 ed ; end vertical I OOL=1.60 =1.15 Plate d governs. Rain on overhangs signer should ver nt(s) except (jt=ll 302.10.2 and	Cat. -4 to eft ify o)	A PROF	ANDREW THOMAS JOHNISAN NUMBER PE-201701899 Coctober 28,2	

# Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

16023 Swingley Ridge Rd Chesterfield, MO 63017

			RELEASE FOR	
Job	Truss	Truss Type		140005000
2523941	47	нр	AS NOTED ON PLANS REVIEW	143385006
2020041			DEVELOPMENT SERVICES	Job Reference (optional)
Builders FirstSource (Valley	Center), Valley Center, K	S - 67147,	LEE'S SUMMIT, MISSOURI8.240 s M	1ar 9 2020 MiTek Industries, Inc. Tue Oct 27 18:39:03 2020 Page 2
			ID:VPVqvFnP0P0b1j2tZrl	QqezdKbx-9Qt1DD45Dsswjf443REh37GomfUWY8ckORp921yP7bc
NOTES-			01/26/2021	
<ol><li>This truss design requi</li></ol>	res that a minimum of 7/16" :	structural woo	d sheathing be applied directly to the top chord and	1/2" gypsum sheetrock be applied directly to the
bottom chord.				

bottom chord.
13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.





Design valid for use only with MTeKe connectors. This design is based only uppon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

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beigh valid for dise offly with with with each connectors. This design is based only upon parameters shown, and is for an individual funding component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

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			RELEASE FOR			
Job	Truss	Truss Type	CONSTRUCTION	Ply		
25230/1	A 0	Roof Special	AS NOTED ON PLANS REVIE	W 1	14	13385008
2323341	A9		DEVELOPMENT SERVICES	'	Job Reference (optional)	
Builders FirstSource (Valley	Center), Valley Center, K	S - 67147,	LEE'S SUMMIT, MISSOURI8	8.240 s Ma	ar 9 2020 MiTek Industries, Inc. Tue Oct 27 18:39:06 2020 P	age 2
			ID:VPVqvFnP0P0	)b1j2tZrlQ	qezdKbx-Z?Z9sE6zWnEVa6ofkZnOhmuLysW_IVMB4P1pfLy	P7bZ
NOTES-			01/26/2021			
<ol><li>This truss design require</li></ol>	res that a minimum of 7/16" s	structural woo	d sheathing be applied directly to the top ch	ord and 1	/2" gypsum sheetrock be applied directly to the	
bottom chord.						





besign value to be only with with ever connectors. This besign is based only upon parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

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			RELEA	SE FOR			
Job	Truss	Truss Type	CONSTR	RUCTION	Ply		
25230/1	A10		AS NOTED ON	PLANS REVI	EW 1	14	3385009
2323541		KOOF SPEC	DEVELOPME	NT SERVICE	S '	Job Reference (optional)	
Builders FirstSource (Valley	Center), Valley Center, K	S - 67147,	LEE'S SUMM	IT, MISSOUR	8.240 s M	ar 9 2020 MiTek Industries, Inc. Tue Oct 27 18:38:46 2020 Pa	age 2
				ID:VPVqvFnP0P	0b1j2tZrlOc	qezdKbx-99MbfPtQDejKB1HpZMQhtY3wRoh33SREUIBmxWy	P7bt
NOTES-			01/26	5/2021			
<ol><li>This truss design requi</li></ol>	res that a minimum of 7/16" s	structural woo	d sheathing be applied di	rectly to the top o	chord and 1	I/2" gypsum sheetrock be applied directly to the	
bottom chord.							





	2-3-9 4-6-4	i 9-8-0 j	17-8-0	)	1	26-6-13	1	33	-1-11	1	40-0-0	
	2-3-9 2-2-1	1 5-1-12	8-0-0		1	8-10-13	1	6-	6-13	1	6-10-5	
Plate Offsets (2	X,Y) [2:0-3-0,0-	2-3], [9:Edge,0-2-8], [1	5:0-6-4,Edge]									
LOADING (ps TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	f) 25.0 15.4/20.0 10.0 0.0 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2018/T	2-0-0 1.15 1.15 YES 'PI2014	<b>CSI.</b> TC BC WB Matrix	0.73 0.91 0.53 (-AS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.24 -0.52 0.24	(loc) 15-16 15-16 9	l/defl >999 >921 n/a	L/d 240 180 n/a	PLATES MT20 MT20HS Weight: 240 lb	<b>GRIP</b> 197/144 148/108 FT = 20%
LUMBER- TOP CHORD BOT CHORD	2x6 SPF No.2 *Ex 3-5: 2x4 SPF No.2 2x4 SPF No.2 *Ex 17-18: 2x4 SP 24	kcept* 2, 1-3: 2x10 SP 2400F kcept* 00F 2.0E, 13-14,9-13:	2.0E 2x6 SPF No.2			BRACING- TOP CHORD BOT CHORD WEBS	Structur Rigid ce 1 Row a 1 Row a	al wood eiling dire at midpt at midpt	sheathin ectly appl	g directly appl ied. Except: 4-15 3-15	lied.	

REACTIONS. (size) 1=0-3-8, 9=0-3-8 Max Horz 1=-200(LC 14) Max Uplift 1=-138(LC 16), 9=-173(LC 16) Max Grav 1=1810(LC 2), 9=1859(LC 2)

2x4 SPF No.2

 FORCES.
 (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 TOP CHORD
 1-2=-801/124, 2-3=-3562/403, 3-4=-2557/366, 4-5=-2437/433, 5-6=-2757/477, 6-8=-2721/362, 8-9=-3249/368

 BOT CHORD
 2-16=-247/3235, 15-16=-244/3241, 4-15=-398/164, 11-12=-240/2806, 9-11=-240/2806

WEBS 3-16=0/341, 3-15=-1209/188, 12-15=-21/1629, 5-15=-187/1270, 5-12=-198/990, 6-12=-538/189, 8-12=-554/112

# NOTES-

WEBS

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=15ft; B=45ft; L=24ft; eave=5ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) 0-1-12 to 2-10-9, Interior(1) 2-10-9 to 20-0-0, Exterior(2R) 20-0-0 to 23-0-0, Interior(1) 23-0-0 to 40-11-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.

6) All plates are MT20 plates unless otherwise indicated.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify

capacity of bearing surface. 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=138, 9=173.

10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

11) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.







WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designe. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSITP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

hitek 16023 Swingley Ridge Rd Chesterfield, MO 63017



- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) -0-11-0 to 2-1-0, Interior(1) 2-1-0 to 17-9-12 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

3) Unbalanced snow loads have been considered for this design.

- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8, 2.
- 7) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.







October 28,2020





WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent ocllapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses sand truss systems, see **ANSITTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601 16023 Swingley Ridge Rd Chesterfield, MO 63017



	5-10-0	11-4-0	17-11-0	
	5-10-0	5-6-8	6-7-0	
Plate Offsets (X,Y) [2:Edge	e,0-0-0], [6:0-3-5,Edge], [9:0-2-8,0-2-12]			
LOADING         (psf)           TCLL (roof)         25.0           Snow (Pf/Pg)         20.4/20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2018/TPI2014	CSI.         DEFL.           TC         0.45         Vert(LI           BC         0.34         Vert(C'           WB         0.82         Horz(C           Matrix-AS         Vertix-C         Vertix-C	in (loc) l/defl L/d -) -0.06 8-9 >999 240 T) -0.13 8-9 >999 180 T) 0.03 8 n/a n/a	PLATES         GRIP           MT20         197/144           Weight: 90 lb         FT = 20%

LUMBER-		BRACING-	
TOP CHORD	2x4 SPF No.2	TOP CHORD	Structural wood sheathing directly applied, except end verticals, and
BOT CHORD	2x4 SPF No.2		2-0-0 oc purlins (6-0-0 max.): 6-7.
WEBS	2x4 SPF No.2	BOT CHORD	Rigid ceiling directly applied.
SLIDER	Left 2x4 SPF No.2 2-6-0		

REACTIONS. (size) 8=0-3-8, 2=0-3-8 Max Horz 2=209(LC 13) Max Uplift 8=-85(LC 13), 2=-87(LC 16) Max Grav 8=800(LC 2), 2=939(LC 36)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- TOP CHORD 2-4=-1164/166, 4-5=-908/161, 5-6=-817/183
- BOT CHORD 2-11=-302/1110. 5-9=-285/97. 8-9=-209/573
- WEBS 9-11=-284/1099, 4-9=-429/93, 6-9=-121/789, 6-8=-774/219

## NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) -0-11-0 to 2-1-0, Interior(1) 2-1-0 to 12-3-12, Exterior(2R) 12-3-12 to 16-6-11, Interior(1) 16-6-11 to 17-9-12 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=20.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8, 2.
   9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and
- 9) This truss is designed in accordance with the 2016 international Residential Code sections R502.11.1 and R602.10.2 and referenced standard ANSI/TP1 1.
   10) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum
- sheetrock be applied directly to the bottom chord.
- 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.







#### REACTIONS. (size) 7=0-3-8, 2=0-3-8 Max Horz 2=173(LC 13) Max Uplift 7=-64(LC 16), 2=-85(LC 16) Max Grav 7=813(LC 2), 2=956(LC 36)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

- TOP CHORD 2-4=-1200/165, 4-5=-997/177, 5-6=-846/184, 6-7=-725/205
- BOT CHORD 2-11=-294/1073

WEBS 6-8=-228/905, 8-11=-297/1059, 4-8=-343/64

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) -0-11-0 to 2-1-0, Interior(1) 2-1-0 to 10-3-12, Exterior(2R) 10-3-12 to 14-6-11, Interior(1) 14-6-11 to 17-9-12 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=20.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4. 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7, 2. 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and
- referenced standard ANSI/TPI 1.
- 10) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.







- DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6, 2.
- 8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



MiTek 16023 Swingley Ridge Rd Chesterfield, MO 63017



	4-7-4	I	2-7-8		4-7-4		
Plate Offsets (X,Y)	[2:Edge,0-0-0], [7:Edge,0-0-0]						
LOADING (psf) TCLL (roof) 29 Snow (Pf/Pg) 20.4/20 TCDL 10 PCLL	SPACING-         2-0-0           .0         Plate Grip DOL         1.15           .0         Lumber DOL         1.15           .0         Rep Stress Incr         NO	<b>CSI.</b> TC 0.53 BC 0.81 WB 0.14	DEFL.         in           Vert(LL)         -0.07           Vert(CT)         -0.10           Horz(CT)         0.04	(loc) l/defl 8-9 >999 8-9 >999 7 n/a	L/d <b>P</b> 240 M 180 n/a	LATES IT20	<b>GRIP</b> 197/144
BCDL 10	0.0 Code IRC2018/TPI2014	Matrix-MS			W	/eight: 41 lb	FT = 20%
LUMBER- TOP CHORD 2x4 SI	PF No.2	BR. TO	ACING- P CHORD Structura	al wood sheathing	directly applied or 4	-0-1 oc purlins	s. except

SLIDER	Left 2x4 SPF No.2 2-6-0, Right 2x4 SPF No.2 2-6-0		
WEBS	2x4 SPF No.2	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
BOT CHORD	2x4 SPF No.2		2-0-0 oc purlins (4-10-11 max.): 4-5.
	EXTOT THOLE		Cirubiarar wood bribaaring arobary applied of 1 of 1 oo

REACTIONS. (size) 7=0-3-8, 2=0-3-8 Max Horz 2=49(LC 57) Max Uplift 7=-139(LC 12), 2=-168(LC 12) Max Grav 7=1048(LC 35), 2=1135(LC 35)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

- TOP CHORD 2-4=-1627/263, 4-5=-1381/245, 5-7=-1625/261
- BOT CHORD 2-9=-187/1412, 8-9=-183/1381, 7-8=-187/1412
- WEBS 4-9=-72/578, 5-8=-72/575

# NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (directional); cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=20.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0 Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 7=139, 2=168.
- 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 11) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 344 lb down and 84 lb up at 4-7-4, and 297 lb down and 68 lb up at 5-11-0, and 344 lb down and 84 lb up at 7-2-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

# LOAD CASE(S) Standard

# Continued on page 2





			RELEASE F	OR			
Job	Truss	Truss Type	CONSTRUCT	<b>FIØN</b>	Ply		
2523941	C2	Lin Cirdor	AS NOTED ON PLAN	SREVIE	Ν,	14	3385018
2020041	02		DEVELOPMENT SE	RVICES		Job Reference (optional)	
Builders FirstSource (Valley	Center), Valley Center, K	S - 67147,	LEE'S SUMMIT, MI	SSOURI8	.240 s Ma	ar 9 2020 MiTek Industries, Inc. Tue Oct 27 18:39:16 2020 Pa	age 2
			ID:	/PVqvFnP0F	P0b1j2tZr	IOqezdKbx-GwAxyfEE9sV4neZaJgzk4sI5Wuzt56VfOySL?myF	27bP
			01/26/2021				
LOAD CASE(S) Standard							
1) Dead + Snow (balanced)	): Lumber Increase=1.15, Pla	ate Increase=1	1.15				

Uniform Loads (plf) Vert: 1-4=-51, 4-5=-61, 5-7=-51, 10-14=-20 Concentrated Loads (lb)

Vert: 9=-344 8=-344 20=-297







1-11	-12 5-11-1	14	10-0-0	12-0-0			
<b>1-1</b> 1	-12 4-0-2	2	4-0-2	2-0-0			
Plate Offsets (X,Y) [3:0-3-5,Edge], [5:0-3-5,Edge], [8:0-4-8,0-2-0], [12:0-4-7,0-2-0]							
LOADING         (psf)           TCLL (roof)         25.0           Snow (Pf/Pg)         20.4/20.0           TCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Boo Strace Lar	CSI.         DE           TC         0.41         Ve           BC         0.45         Ve           WB         0.21         Ho	FL.         in         (loc)         l/defl           t(LL)         -0.06         10         >999           t(CT)         -0.09         10         >999           t(CT)         -0.09         10         >99	L/d <b>PLATES</b> 240 MT20 180	<b>GRIP</b> 197/144		
BCLL 0.0 BCDI 10.0	Code IRC2018/TPI2014	Matrix-MS	2(CT) 0.01 0 11/a	Weight: 45 lb	FT = 20%		
BCBL 10.0							
LUMBER- TOP CHORD 2x4 SPF No.2 BOT CHORD 2x4 SPF No.2 WEBS 2x4 SPF No.2 * 2-12,6-8: 2x6 SI	Except* PF No.2	BRACING- TOP CHOR BOT CHOR	<ul> <li>Structural wood sheathing except end verticals, and</li> <li>Rigid ceiling directly appli</li> </ul>	g directly applied or 6-0-0 oc purlin 2-0-0 oc purlins (4-7-15 max.): 3-5 ed or 10-0-0 oc bracing.	S,		
REACTIONS. (size) 12=0 Max Horz 12=- Max Uplift 12=- Max Grav 12=6	-3-8, 8=0-3-8 44(LC 10) 80(LC 12), 8=-80(LC 12) 601(LC 2), 8=601(LC 2)						
FORCES.         (lb) - Max. Comp./M           TOP CHORD         2-3=-701/54, 3           BOT CHORD         11-12=-30/58           WEBS         3-10=-73/853,	ax. Ten All forces 250 (lb) or less exc -4=-1404/116, 4-5=-1404/116, 5-6=-706 3, 10-11=-34/594, 9-10=-27/600, 8-9=-27 4-10=-437/82, 5-10=-73/846	æpt when shown. 8/54, 2-12=-487/75, 6-8=-489/75 3/594					
<ul> <li>NOTES-</li> <li>1) Unbalanced roof live loads ha</li> <li>2) Wind: ASCE 7-16; Vult=115m II; Exp C; Enclosed; MWFRS plate grip DOL=1.60</li> <li>3) TCLL: ASCE 7-16; Pr=25.0 p DOL=1.15); Is=1.0; Rough C: surfaces with slopes less that</li> <li>4) Unbalanced snow loads have</li> <li>5) This truss has been designed non-concurrent with other live</li> <li>6) Provide adequate drainage to</li> <li>7) This truss has been designed:</li> <li>8) Provide mechanical connectii</li> <li>9) This truss is designed in accorreferenced standard ANSI/TF</li> <li>10) Graphical purlin representat</li> <li>11) "NAILED" indicates 3-10d (0</li> <li>12) In the LOAD CASE(S) section</li> <li>LOAD CASE(S) Standard</li> <li>1) Dead + Snow (balanced): Lut Uniform Loads (plf) Vert: 1-2=-51, 2-3=5</li> </ul>	ave been considered for this design. ph (3-second gust) Vasd=91mph; TCD (directional); cantilever left and right exi- st (roof LL: Lum DOL=1.15 Plate DOL= at C; Partially Exp.; Ce=1.0; Cs=1.00; C 0.500/12 in accordance with IBC 1608 been considered for this design. for greater of min roof live load of 12.0 boads. prevent water ponding. for a 10.0 psf bottom chord live load no on (by others) of truss to bearing plate c ordance with the 2018 International Resi- 11. ion does not depict the size or the orien 0.148"x3") or 2-12d (0.148"x3.25") toe-n on, loads applied to the face of the truss mber Increase=1.15, Plate Increase=1.1 i1, 3-5=-61, 5-6=-51, 6-7=-51, 8-12=-20	L=6.0psf; BCDL=4.2psf; h=15ft; B= posed ; end vertical left and right ex 1.15); Pg=20.0 psf; Pf=20.4 psf (Lu t=1.10, Lu=50-0-0 Rain surcharge 3.3.4. psf or 1.00 times flat roof load of 18 proconcurrent with any other live loa apable of withstanding 100 lb uplift idential Code sections R502.11.1 at tation of the purlin along the top an ails per NDS guidlines. s are noted as front (F) or back (B).	I5ft; L=24ft; eave=4ft; Cat. posed; Lumber DOL=1.60 n DOL=1.15 Plate applied to all exposed .4 psf on overhangs ds. at joint(s) 12, 8. Id R802.10.2 and I/or bottom chord.	ANDREW THOMAS JOHNSON PE-20170189 October 28.	93 CT 2020		

Continued on page 2



		RELEASE FOR	
Job Truss	Truss Type	CONSTRUCTION Ply	1/00050/0
2523941	Hip Girder	AS NOTED ON PLANS REVIEW	143385019
		DEVELOPMENT SERVICES	Job Reference (optional)
Builders FirstSource (Valley Center), Va	alley Center, KS - 67147,	LEE'S SUMMIT, MISSOURI8.240 s Ma	ar 9 2020 MiTek Industries, Inc. Tue Oct 27 18:39:18 2020 Page 2
		ID:VPVqvFnP0P0b1j2tz	rlOqezdKbx-DJHiNLFVhTlo0yjzR5?CAHNTwik3Z_xyrGxS4fyP7bN
		01/26/2021	
LOAD CASE(S) Standard			

Concentrated Loads (lb) Vert: 11=-5(F) 10=0(F) 9=-5(F) 19=0(F) 20=0(F)





### Scale = 1:22.9



	L	3-11-12	8-0-0					12-0-0		
	1	3-11-12	4-0-4					4-0-0	I	
Plate Offsets ()	X,Y) [2:0-4-0,0	-0-6], [7:Edge,0-0-0]								
LOADING (psi TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	f) 25.0 20.4/20.0 10.0 0.0 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2018/TPI2014	<b>CSI.</b> TC 0.30 BC 0.25 WB 0.03 Matrix-AS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.04 -0.06 0.02	(loc) 9 9 7	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 42 lb	<b>GRIP</b> 197/144 FT = 20%
LUMBER- TOP CHORD BOT CHORD WEBS SLIDER	2x4 SPF No.2 2x4 SPF No.2 2x4 SPF No.2 Left 2x4 SPF No.2	2 2-6-0, Right 2x4 SPF No.2 2-6-0	BR/ TOP BO	ACING- P CHORD Si 2- T CHORD R	tructura -0-0 oc igid cei	I wood purlins ling dire	sheathin (6-0-0 m ectly appl	g directly app ax.): 4-5. ied.	blied, except	

REACTIONS. (size) 2=0-3-8, 7=0-3-8 Max Horz 2=-45(LC 14) Max Uplift 2=-72(LC 16), 7=-72(LC 16) Max Grav 2=608(LC 39), 7=608(LC 39)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD

2-4=-673/232, 4-5=-628/231, 5-7=-672/231 2-10=-123/633, 9-10=-124/628, 7-9=-122/632 BOT CHORD

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) -0-11-0 to 2-1-0, Interior(1) 2-1-0 to 3-11-12, Exterior(2E) 3-11-12 to 8-0-0, Exterior(2R) 8-0-0 to 12-0-0, Interior(1) 12-0-0 to 12-11-0 zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=20.4 psf (Lum DOL=1.15 Plate
- DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 7.
- 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 10) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.







TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.







		<u>2-4-12</u> 2-4-12		<u>3-11-4</u> 1-6-8				
Plate Offsets (X,Y) [2:0-0-14,	,0-1-12], [3:0-1-3,0-1-8], [7:0-0-14,0-1-1	2], [7:0-0-6,0-4-4]						
LOADING (psf) TCLL (roof) 25.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2018/TPI2014	<b>CSI.</b> TC 0.31 BC 0.52 WB 0.03 Matrix-MP	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) 0.04 6 -0.06 6 0.03 5	l/defl >999 >794 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 13 lb	<b>GRIP</b> 197/144 FT = 20%

BRACING-TOP CHORD

BOT CHORD

## LUMBER-

TOP CHORD2x4 SPF No.2BOT CHORD2x4 SPF No.2WEBS2x4 SPF No.2

REACTIONS. (size) 7=0-3-8, 5=Mechanical

Max Horz 7=90(LC 16) Max Uplift 7=-23(LC 16), 5=-58(LC 16)

Max Grav 7=264(LC 2), 5=171(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

### NOTES-

- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) -0-11-0 to 1-10-15, Interior(1) 1-10-15 to 3-11-4 zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Bearing at joint(s) 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7, 5.
   This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



Structural wood sheathing directly applied or 3-11-4 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.





	1	2-4-12		1	3-2-3	1			
	ſ	2-4-12		1	0-9-7				
Plate Offsets (X,Y) [2:0-	0-14,0-1-12], [3:0-1-4,Edge], [7:0-0-14,0-1-	-12], [7:0-0-6,0-4-4]							
LOADING         (psf)           TCLL (roof)         25.0           Snow (Pf/Pg)         15.4/20.0           TCDL         10.0           BCDL         0.0           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2018/TPI2014	CSI. TC 0.13 BC 0.28 WB 0.03 Matrix-MP	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.01 -0.02 0.01	(loc) 10 10 5	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 11 lb	<b>GRIP</b> 197/144 FT = 20%
LIMBER- BRACING-									

TOP CHORD

BOT CHORD

### LUMBER-

TOP CHORD 2x4 SPF No.2 BOT CHORD 2x4 SPF No.2 WEBS 2x4 SPF No.2

REACTIONS. (size) 7=0-3-8, 5=Mechanical

Max Horz 7=74(LC 16) Max Uplift 7=-23(LC 16), 5=-42(LC 16)

Max Grav 7=249(LC 21), 5=146(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

## NOTES-

- 1) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) -0-11-0 to 1-10-15, Interior(1) 1-10-15 to 3-2-3 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Refer to girder(s) for truss to truss connections.
- 7) Bearing at joint(s) 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7, 5. 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



Structural wood sheathing directly applied or 3-2-3 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.




WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

# 16023 Swingley Ridge Rd Chesterfield, MO 63017



	1-10-3		
LOADING (psf)         SPACING-         2-0-           TCLL (roof)         25.0         Plate Grip DOL         1.1           Snow (Pf/Pg)         15.4/20.0         Lumber DOL         1.1           TCDL         10.0         Rep Stress Incr         YE           BCDL         10.0         Code IRC2018/TPI2014	CSI. DEI TC 0.03 Ver BC 0.02 Ver WB 0.00 Hor Matrix-MR	FL.         in         (loc)         I/defl         L/d           t(LL)         -0.00         4         >999         240           t(CT)         -0.00         4         >999         180           z(CT)         -0.00         3         n/a         n/a	PLATES         GRIP           MT20         197/144           Weight: 6 lb         FT = 20%

TOP CHORD2x4 SPF No.2BOT CHORD2x4 SPF No.2WEBS2x4 SPF No.2

BRACING-TOP CHORD

Structural wood sheathing directly applied or 1-10-3 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 4=0-3-8, 3=Mechanical Max Horz 4=33(LC 13)

Max Uplift 3=-16(LC 13)

Max Grav 4=70(LC 2), 3=73(LC 27)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.







Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent oullapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

16023 Swingley Ridge Rd Chesterfield, MO 63017

MiTek

			RELEASE FOR		
Job	Truss	Truss Type		Ply	
2523941	145	Half Hip Girder	AS NOTED ON PLANS REVIEW	<b>/</b> 1	143385026
2020041	57.5		DEVELOPMENT SERVICES	'	Job Reference (optional)
Builders FirstSource (\	/alley Center),	Valley Center, KS - 67147,	LEE'S SUMMIT, MISSOURI8.2	240 s M	ar 9 2020 MiTek Industries, Inc. Tue Oct 27 18:39:32 2020 Page 2
			ID:VPVqvFnP0P0	b1j2tZrl	DqezdKbx-o?7_J7QHOmWoi6ofF1FUkEznFLPZrKt03SKBZryP7b9
			01/26/2021		
LOAD CASE(S) Star	ndard				
1) Dead + Snow (bala	anced): Lumber Inc	rease=1.15, Plate Increase=1.1	15		
Uniform Loads (plf)	)				

Vert: 1-2=-51, 2-3=-51, 3-4=-61, 2-7=-20, 2-5=-20 Concentrated Loads (lb)

Vert: 6=-333(B) 8=-146(B)





### Scale = 1:15.7



	1-11-4 1-11-4		<u>7-3-4</u> 5-4-0	
Plate Offsets (X,Y) [2:0-0-14	4,0-1-12], [8:0-3-1,0-0-8], [8:0-0-0,0-1-12]			
LOADING (psf)           TCLL (roof)         25.0           Snow (Pf/Pg)         20.4/20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr NO Code IRC2018/TPI2014	CSI.         DEFL.           TC         0.34         Vert(LL)           BC         0.36         Vert(CT)           WB         0.10         Horz(CT)           Matrix-MS         Horz(CT)	in (loc) l/defl L/d 0.05 6-7 >999 240 -0.07 6-7 >999 180 0.01 6 n/a n/a	PLATES         GRIP           MT20         197/144           Weight: 27 lb         FT = 20%
LUMBER- TOP CHORD 2x4 SPF No.2 BOT CHORD 2x4 SPF No.2 WEBS 2x4 SPF No.2 WEBS 2x4 SPF No.2 REACTIONS. (size) 6=M Max Horz 8=5' Max Uplift 6=-1 Max Grav 6=4:	lechanical, 8=0-3-8 1(LC 11) 22(LC 9), 8=-146(LC 12) 24(LC 31), 8=474(LC 2)	BRACING- TOP CHORD Si ex BOT CHORD R	tructural wood sheathing directly appli xcept end verticals, and 2-0-0 oc purli ligid ceiling directly applied or 10-0-0 c	ied or 6-0-0 oc purlins, ns (6-0-0 max.): 3-5. c bracing.
FORCES.         (lb)         Max. Comp./M           TOP CHORD         2-3=-551/200,           BOT CHORD         7-8=-190/456,           WEBS         4-6=-528/150	/lax. Ten All forces 250 (lb) or less exce , 3-4=-436/165, 2-8=-432/159 , 6-7=-172/544	ot when shown.		
<ul> <li>NOTES-</li> <li>1) Unbalanced roof live loads ha</li> <li>2) Wind: ASCE 7-16; Vult=115n II; Exp C; Enclosed; MWFRS plate grip DOL=1.60</li> <li>3) TCLL: ASCE 7-16; Pr=25.0 p DOL=1.15); Is=1.0; Rough C surcharge applied to all expod</li> <li>4) Unbalanced snow loads have</li> <li>5) This truss has been designed</li> <li>6) Provide adequate drainage to</li> <li>7) This truss has been designed</li> <li>8) Refer to girder(s) for truss to</li> <li>9) Provide mechanical connecti 6=122, 8=146.</li> <li>10) This truss is designed in act referenced standard ANSI/T</li> <li>11) Graphical purlin representai</li> <li>12) "NAILED" indicates 3-10d (f</li> <li>13) In the LOAD CASE(S) section</li> </ul>	ave been considered for this design. nph (3-second gust) Vasd=91mph; TCDL: (directional); cantilever left and right expr sef (roof LL: Lum DOL=1.15 Plate DOL=1. at C; Partially Exp.; Ce=1.0; Cs=1.00; Ct= sed surfaces with slopes less than 0.500/ be been considered for this design. d for greater of min roof live load of 12.0 p e loads. b prevent water ponding. d for a 10.0 psf bottom chord live load non truss connections. on (by others) of truss to bearing plate cal cordance with the 2018 International Resi TPI 1. tion does not depict the size or the orienta 0.148"x3") or 2-12d (0.148"x3.25") toe-nai on, loads applied to the face of the truss a	=6.0psf; BCDL=4.2psf; h=15ft; B=45ft; L=2- sed; end vertical left and right exposed; L 15); Pg=20.0 psf; Pf=20.4 psf (Lum DOL=1 1.10, Lu=50-0-0; Min. flat roof snow load g 12 in accordance with IBC 1608.3.4. sf or 1.00 times flat roof load of 15.4 psf on concurrent with any other live loads. bable of withstanding 100 lb uplift at joint(s) dential Code sections R502.11.1 and R802 tion of the purlin along the top and/or botto ls per NDS guidlines. ire noted as front (F) or back (B).	44ft; eave=4ft; Cat. .umber DOL=1.60 1.15 Plate governs. Rain n overhangs e) except (jt=lb) 2.10.2 and om chord.	ANDREW THOMAS OHNSON NUMBER PE-2017018993

October 28,2020



#### Continued on page 2

			RELEASE F	FOR			
Job	Truss	Truss Type	CONSTRUC	TIØN	Ply		
2523941	JA6	Half Hip Girder	AS NOTED ON PLAN	IS REVIE	N 1		43385027
	0,10		DEVELOPMENT S	ERVICES		Job Reference (optional)	
Builders FirstSource (Valley	Center), Valley Center, K	(S - 67147,	LEE'S SUMMIT, M	ISSOURI8.	.240 s M	ar 9 2020 MiTek Industries, Inc. Tue Oct 27 18:39:33 2020 F	Page 2
			ID:	/PVqvFnP0P	0b1j2tZr	OqezdKbx-GBhNWTRv94efKGMrpkmjHSV2EltzapM9l64l6H	/P7b8
			01/26/202				
LOAD CASE(S) Standard							
Uniform Loads (plf)		L					

Uniform Loads (plf) Vert: 1-2=-51, 2-3=-51, 3-5=-61, 6-8=-20 Concentrated Loads (lb) Vert: 7=-70(F) 10=-40(F) 11=-40(F)





TOP CHORD

BOT CHORD

TOP CHORD 2x4 SPF No.2 BOT CHORD 2x4 SPF No.2 WEBS 2x4 SPF No.2

REACTIONS. (size) 7=Mechanical, 9=0-3-8

Max Horz 9=131(LC 13) Max Uplift 7=-38(LC 13), 9=-50(LC 16)

Max Grav 7=354(LC 21), 9=388(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-9=-329/123

NOTES-

- 1) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) -0-11-0 to 2-1-0, Interior(1) 2-1-0 to 7-3-4 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Bearing at joint(s) 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7, 9.
- 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 10) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



Structural wood sheathing directly applied, except end verticals.

Rigid ceiling directly applied.





WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSITTPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

# 16023 Swingley Ridge Rd Chesterfield, MO 63017



LOADING (psf)           TCLL (roof)         25.0           Snow (Pf/Pg)         15.4/20.0           TCDL         10.0           BCLL         0.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.86 BC 0.27 WB 0.08	DEFL.inVert(LL)0.16Vert(CT)-0.24Horz(CT)0.19	(loc) 9 9 8	l/defl >536 >348 n/a	L/d 240 180 n/a	PLATES MT20	<b>GRIP</b> 197/144
BCDL 10.0	Code IRC2018/1P12014	Matrix-A5					weight: 29 lb	FI = 20%
LUMBER- TOP CHORD 2x4 SPF No.2 BOT CHORD 2x4 SPF No.2		BRACII TOP CH	IG- IORD Structu	ral wood	sheathin	g directly app	plied, except end vert	icals.

BOT CHORD 2x4 SPF No.2 WEBS 2x4 SPF No.2 SLIDER Left 2x4 SPF No.2 2-3-6

Rigid ceiling directly applied.

REACTIONS. (size) 8=Mechanical, 2=0-3-8 Max Horz 2=122(LC 13) Max Uplift 8=-36(LC 13), 2=-46(LC 16) Max Grav 8=359(LC 21), 2=383(LC 2)

 FORCES.
 (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 TOP CHORD
 3-11=-617/342, 3-4=-331/117

BOT CHORD 3-8=-287/348 WEBS 4-8=-472/346

NOTES-

1) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) -0-11-0 to 2-1-12, Interior(1) 2-1-12 to 7-3-4 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

Unbalanced snow loads have been considered for this design.

4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) Refer to girder(s) for truss to truss connections.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8, 2. 8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



MiTek 16023 Swingley Ridge Rd Chesterfield, MO 63017



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designe. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSITP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601 16023 Swingley Ridge Rd Chesterfield, MO 63017



- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) Refer to girder(s) for truss to truss connections.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6, 2.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 11) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.







16023 Swingley Ridge Rd Chesterfield, MO 63017



TCLL (roof)         25.0           Snow (Pf/Pg)         15.4/20.0           TCDL         10.0           PCLL         0.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.08 BC 0.02 WB 0.00	DEFL. Vert(LL) 0.0 Vert(CT) 0.0 Horz(CT) -0.0	in (loc) 00 5 00 5 00 3	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20	<b>GRIP</b> 197/144
BCDL 10.0	Code IRC2018/TPI2014	Matrix-MR					Weight: 4 lb	FT = 20%

E

TOP CHORD 2x4 SPF No 2 BOT CHORD 2x4 SPF No.2 WEBS 2x4 SPF No.2 BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 1-2-3 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. 5=0-3-8, 3=Mechanical, 4=Mechanical (size) Max Horz 5=48(LC 16) Max Uplift 5=-35(LC 16), 3=-9(LC 20), 4=-1(LC 13) Max Grav 5=159(LC 2), 3=11(LC 28), 4=16(LC 7)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

- 1) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 3, 4.
- 8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.







Flate Offsets (	<u>, t) [2.0-0-1</u> 2	F,0-1-12J, [5.0-2-13,0-0-12J, [5.0-0-0,0-	-12]						
LOADING (ps TCLL (roof) Snow (Pf/Pg) TCDL BCLL	f) 25.0 0.0/20.0 10.0 0.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYES	CSI. TC 0.22 BC 0.18 WB 0.00	<b>DEFL.</b> Vert(LL) Vert(CT) Horz(CT)	in (loc) 0.01 4-5 -0.00 4-5 0.00	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20	<b>GRIP</b> 197/144
BCDL	10.0	Code IRC2018/TPI2014	Matrix-MR					Weight: 6 lb	FT = 20%
LUMBER- TOP CHORD BOT CHORD	2x4 SPF No.2 2x4 SPF No.2		BF TC	ACING- P CHORD	Structural woo	d sheathir ticals.	ig directly ap	oplied or 1-11-4 oc p	urlins,
WEBS	2x4 SPF No.2		BC	T CHORD	Rigid ceiling di	rectly app	lied or 10-0-	0 oc bracing.	

**REACTIONS.** (size) 5=0-3-8, 4=Mechanical

Max Horz 5=78(LC 16) Max Uplift 5=-75(LC 16), 4=-53(LC 16)

Max Grav 5=182(LC 21), 4=60(LC 21)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

### NOTES-

 Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 2) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=0.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 4.
- 8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.







Flate Offsets (	<u>, t) [2.0-0-14</u>	i,0-1-12j, [5.0-2-13,0-0-12j, [5.0-0-0,0-1	-12]					
LOADING (ps TCLL (roof) Snow (Pf/Pg) TCDL BCLL	f) 25.0 0.0/20.0 10.0 0.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYES	CSI. TC 0.22 BC 0.18 WB 0.00	<b>DEFL.</b> Vert(LL) Vert(CT) Horz(CT)	in (loc) l/defl 0.01 4-5 >999 -0.00 4-5 >999 0.00 n/a	L/d 240 180 n/a	PLATES MT20	<b>GRIP</b> 197/144
BCDL	10.0	Code IRC2018/1PI2014	Matrix-MR				Weight: 6 lb	FI = 20%
LUMBER- TOP CHORD	2x4 SPF No.2		BI TC	RACING- OP CHORD	Structural wood sheathin	g directly app	blied or 1-11-4 oc pu	urlins,
WEBS	2x4 SPF No.2		В	OT CHORD	Rigid ceiling directly appl	ied or 10-0-0	oc bracing.	

REACTIONS. (size) 5=0-3-8, 4=Mechanical

Max Horz 5=78(LC 16) Max Uplift 5=-75(LC 16), 4=-53(LC 16)

Max Grav 5=182(LC 21), 4=60(LC 21)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

### NOTES-

 Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 2) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=0.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp; Ce=1.0; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 4.
- 8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.









BOT CHORD

Rigid ceiling directly applied.

 TOP CHORD
 2x4 SPF No.2

 BOT CHORD
 2x4 SPF No.2

 WEBS
 2x4 SPF No.2

REACTIONS. (size) 6=Mechanical, 8=0-3-8 Max Horz 8=123(LC 13)

Max Uplift 6=-38(LC 13), 8=-16(LC 16) Max Grav 6=358(LC 20), 8=308(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) 0-1-12 to 3-1-12, Interior(1) 3-1-12 to 7-3-4 zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Bearing at joint(s) 8 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6, 8.
- 8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.







LOADING         (psf)           TCLL (roof)         25.0           Snow (Pf/Pg)         15.4/20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2018/TPI2014	<b>CSI.</b> TC 0.08 BC 0.02 WB 0.00 Matrix-MR	DEFL. Vert(LL) Vert(CT) Horz(CT)	in 0.00 0.00 -0.00	(loc) 5 5 3	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 5 lb	<b>GRIP</b> 197/144 FT = 20%
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TOP CHORD2x4 SPF No.2BOT CHORD2x4 SPF No.2WEBS2x4 SPF No.2

BRACING-TOP CHORD

Structural wood sheathing directly applied or 1-2-3 oc purlins, except end verticals. Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS. (size) 5=0-3-8, 3=Mechanical, 4=Mechanical Max Horz 5=49(LC 16) Max Uplift 5=-33(LC 16), 3=-9(LC 20), 4=-2(LC 13) Max Grav 5=159(LC 2), 3=11(LC 28), 4=16(LC 7)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Bearing at joint(s) 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 3, 4.
   This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and
- Inis truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 an referenced standard ANSI/TPI 1.







			2-6-3			1			
LOADING (psf)           TCLL (roof)         25.0           Snow (Pf/Pg)         15.4/20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2018/TPI2014	CSI. TC 0.08 BC 0.05 WB 0.00 Matrix-MR	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.00 -0.00 -0.00	(loc) 4-5 4-5 3	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 8 lb	<b>GRIP</b> 197/144 FT = 20%

TOP CHORD2x4 SPF No.2BOT CHORD2x4 SPF No.2WEBS2x4 SPF No.2

BRACING-TOP CHORD

Structural wood sheathing directly applied or 2-6-3 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 5=0-3-8, 3=Mechanical, 4=Mechanical Max Horz 5=65(LC 16) Max Uplift 5=-30(LC 16), 3=-23(LC 16)

Max Grav 5=206(LC 21), 3=69(LC 21), 4=42(LC 7)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) -0-11-0 to 2-1-0, Interior(1) 2-1-0 to 2-5-7 zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 3.
- 8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.







		020		2012		0 + 0		
Plate Offsets (X,Y) [2:0-2-12	2,0-0-1]							
LOADING (psf) TCLL (roof) 25.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr NO Code IRC2018/TPI2014	CSI. TC 0.60 BC 0.40 WB 0.04 Matrix-MP	<b>DEFL.</b> ir Vert(LL) -0.07 Vert(CT) -0.13 Horz(CT) 0.03	n (loc) 7-10 7-10 7-10 2 2	l/defl >999 >544 n/a	L/d 240 180 n/a	<b>PLATES</b> MT20 Weight: 21 lb	<b>GRIP</b> 197/144 FT = 20%
		BBA	CINC					

TOP CHORD

BOT CHORD

#### LUMBER-

 TOP CHORD
 2x4 SPF No.2

 BOT CHORD
 2x4 SPF No.2

 WEBS
 2x4 SPF No.2

 SLIDER
 Left 2x4 SPF No.2 2-0-0

REACTIONS. (size) 2=0-4-9, 7=0-4-15 Max Horz 2=85(LC 12) Max Uplift 2=-46(LC 12), 7=-56(LC 12) Max Grav 2=361(LC 2), 7=372(LC 17)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-4=-273/63

WEBS 4-7=-263/77

NOTES-

- 1) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (directional); cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 7.
   7) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 8) "NAILED" indicates 3-10d (0.148"x3") or 2-12d (0.148"x3.25") toe-nails per NDS guidlines.
- 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

#### LOAD CASE(S) Standard

- 1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
  - Uniform Loads (plf)
  - Vert: 1-5=-51, 6-8=-20

Concentrated Loads (lb)

Vert: 14=-5(B) 15=-60(F) 16=0(F) 17=-1(B) 18=-18(F)



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.





LOADING (ps TCLL (roof) Snow (Pf/Pg) TCDL	sf) 25.0 15.4/20.0 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYES	CSI. TC 0.08 BC 0.03 WB 0.00	<b>DEFL.</b> ir Vert(LL) -0.00 Vert(CT) -0.00 Horz(CT) -0.00	(loc) 5 5 3	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20	<b>GRIP</b> 197/144
BCDL	10.0	Code IRC2018/TPI2014	Matrix-MR					Weight: 6 lb	FT = 20%

TOP CHORD2x4 SPF No.2BOT CHORD2x4 SPF No.2WEBS2x4 SPF No.2

BRACING-TOP CHORD

Structural wood sheathing directly applied or 1-9-3 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 5=0-3-8, 3=Mechanical, 4=Mechanical Max Horz 5=56(LC 16) Max Uplift 5=-32(LC 16), 3=-14(LC 16)

Max Grav 5=175(LC 21), 3=38(LC 21), 4=28(LC 7)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

3) Unbalanced snow loads have been considered for this design.

4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 3.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.







	ŀ		3-9-3						
LOADING (psf) TCLL (roof) 25.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2018/TPI2014	CSI. TC 0.19 BC 0.12 WB 0.00 Matrix-MR	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.01 -0.02 -0.01	(loc) 4-5 4-5 3	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 11 lb	<b>GRIP</b> 197/144 FT = 20%

TOP CHORD2x4 SPF No.2BOT CHORD2x4 SPF No.2WEBS2x4 SPF No.2

BRACING-TOP CHORD

Structural wood sheathing directly applied or 3-9-3 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 5=0-3-8, 3=Mechanical, 4=Mechanical Max Horz 5=83(LC 16) Max Uplift 5=-29(LC 16), 3=-36(LC 16)

Max Grav 5=271(LC 21), 3=121(LC 21), 4=67(LC 7)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) -0-11-0 to 2-1-0, Interior(1) 2-1-0 to 3-8-7 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 3.
- 8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.







	<u> </u>	3-2-5 3-2-5		6-0-1 2-9-12		6-4-10 0-4-9	
Plate Offsets (X,Y) [2:0-0-10	,0-1-12], [7:0-2-3,0-0-8], [7:0-0-0,0-1-12]						
LOADING (psf) TCLL (roof) 25.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0 PCU 0.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr NO	<b>CSI.</b> TC 0.54 BC 0.32 WB 0.04	DEFL. Vert(LL) - Vert(CT) - Horz(CT)	in (loc) -0.05 6-7 -0.10 6-7 0.00	l/defl L/d >999 240 >679 180 n/a n/a	PLATES MT20	<b>GRIP</b> 197/144
BCLL U.U	Code IRC2018/TPI2014	Matrix-AS				Weight: 19 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

#### LUMBER-

BCDL

TOP CHORD 2x4 SPF No.2 BOT CHORD 2x4 SPF No.2 2x4 SPF No.2 WEBS

REACTIONS. (lb/size) 7=281/0-4-9, 6=234/0-4-15

10.0

Max Horz 7=96(LC 16) Max Uplift 7=-50(LC 16), 6=-48(LC 16)

Max Grav 7=366(LC 2), 6=317(LC 21)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. TOP CHORD 2-7=-316/235

#### NOTES-

1) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Corner(3) -1-3-9 to 2-11-6, Exterior(2R) 2-11-6 to 6-4-10 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

3) Unbalanced snow loads have been considered for this design.

4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 50 lb uplift at joint 7 and 48 lb uplift at ioint 6.
- 7) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



Structural wood sheathing directly applied, except end verticals

Rigid ceiling directly applied.

**MiTek** 16023 Swingley Ridge Rd Chesterfield, MO 63017



Concentrated Loads (lb) Vert: 4=-9(B) 5=-1(B)

WORTSSIONAL October 28,2020

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LOADING (psf) SPACING- 2-0-0 CSI. DEFL. in (loc) I/defl L/d PLATES GRIP				1.100	
Now (Pf/Pg)         15.4/20.0         Plate Grip DOL         1.15         TC         0.08         Vert(LL)         -0.00         5 >999         240         MT20         197/144           TCDL         10.0         Lumber DOL         1.15         BC         0.03         Vert(CT)         -0.00         5 >999         180           BCLL         0.0         Rep Stress Incr         YES         WB         0.00         Horz(CT)         -0.00         3 n/a         n/a           BCDL         10.0         Code IRC2018/TPI2014         Matrix-MR         Weight: 6 lb         FT = 20%	LOADING (psf)           TCLL (roof)         25.0           Snow (Pf/Pg)         15.4/20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2018/TPI2014	CSI. TC 0.08 BC 0.03 WB 0.00 Matrix-MR	DEFL.         in         (loc)         l/defl         L/d           Vert(LL)         -0.00         5         >999         240           Vert(CT)         -0.00         5         >999         180           Horz(CT)         -0.00         3         n/a         n/a	= 20%

TOP CHORD2x4 SPF No.2BOT CHORD2x4 SPF No.2WEBS2x4 SPF No.2

BRACING-TOP CHORD

Structural wood sheathing directly applied or 1-10-5 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 5=0-3-8, 3=Mechanical, 4=Mechanical Max Horz 5=57(LC 16) Max Uplift 5=-32(LC 16), 3=-15(LC 16)

Max Grav 5=179(LC 21), 3=42(LC 21), 4=30(LC 7)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

3) Unbalanced snow loads have been considered for this design.

4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 3.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.







LOADING (psf) TCLL (roof) 25.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0 PCUL 0.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYES	CSI. TC 0.08 BC 0.03 WB 0.00	<b>DEFL.</b> Vert(LL) -0.00 Vert(CT) -0.00 Horz(CT) -0.00	n (loc) 5 4-5 3	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20	<b>GRIP</b> 197/144
BCDL 0.0 BCDL 10.0	Code IRC2018/TPI2014	Matrix-MR					Weight: 6 lb	FT = 20%

TOP CHORD2x4 SPF No.2BOT CHORD2x4 SPF No.2WEBS2x4 SPF No.2

BRACING-TOP CHORD

Structural wood sheathing directly applied or 2-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 5=0-3-8, 3=Mechanical, 4=Mechanical Max Horz 5=59(LC 16) Max Uplift 5=-31(LC 16), 3=-17(LC 16)

Max Grav 5=185(LC 21), 3=49(LC 21), 4=33(LC 7)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

3) Unbalanced snow loads have been considered for this design.

4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 3.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.







BRACING-

TOP CHORD

BOT CHORD

<u>CI</u>		
SL	IDER	

LUMBER-

BCDL

2x4 SPF No.2 BOT CHORD Left 2x4 SPF No.2 2-0-0

TOP CHORD 2x4 SPF No.2

10.0

REACTIONS. (size) 1=0-3-8, 3=Mechanical, 4=Mechanical

Max Horz 1=36(LC 16) Max Uplift 3=-26(LC 16)

Max Grav 1=118(LC 20), 3=83(LC 20), 4=44(LC 7)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

1) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right

- exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.
- 7) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and
- referenced standard ANSI/TPI 1.



Structural wood sheathing directly applied or 2-7-11 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.





Max Uplift 2=-54(LC 12), 5=-37(LC 12) Max Grav 2=396(LC 2), 5=302(LC 17)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-4=-319/55

#### NOTES-

- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (directional); cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 5.
- 8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 9) "NAILED" indicates 3-10d (0.148"x3") or 2-12d (0.148"x3.25") toe-nails per NDS guidlines.
- 10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

## LOAD CASE(S) Standard

- 1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
  - Uniform Loads (plf)

Vert: 1-4=-51, 5-6=-20 Concentrated Loads (lb)









LOADING (psf)         SPACING-         2-0-0         CSI.           TCLL (roof)         25.0         Plate Grip DOL         1.15         TC         0.08           Snow (Pf/Pg)         15.4/20.0         Lumber DOL         1.15         BC         0.03           TCDL         10.0         Rep Stress Incr         YES         WB         0.00           BCDL         10.0         Code IRC2018/TPI2014         Matrix-MR	DEFL.         in         (loc)         l/defl         L/d           Vert(LL)         -0.00         5         >999         240           Vert(CT)         -0.00         5         >999         180           Horz(CT)         -0.00         3         n/a         n/a	PLATES         GRIP           MT20         197/144           Weight: 6 lb         FT = 20%
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TOP CHORD2x4 SPF No.2BOT CHORD2x4 SPF No.2WEBS2x4 SPF No.2

BRACING-TOP CHORD

Structural wood sheathing directly applied or 1-8-15 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 5=0-3-8, 3=Mechanical, 4=Mechanical Max Horz 5=55(LC 16) Max Uplift 5=-32(LC 16), 3=-14(LC 16)

Max Grav 5=175(LC 21), 3=37(LC 28), 4=28(LC 7)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

3) Unbalanced snow loads have been considered for this design.

4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 3.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.







				3-0-3						
LOADING (psf)SPACING-TCLL (roof)25.0Plate Grip DOLSnow (Pf/Pg)15.4/20.0Lumber DOLTCDL10.0Rep Stress IncrBCDL10.0Code IRC2018/	2-0-0 1.15 1.15 YES TPI2014	CSI. TC BC WB Matri:	0.11 0.07 0.00 x-MR	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.00 -0.01 -0.00	(loc) 4-5 4-5 3	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 9 lb	<b>GRIP</b> 197/144 FT = 20%

TOP CHORD2x4 SPF No.2BOT CHORD2x4 SPF No.2WEBS2x4 SPF No.2

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 3-0-3 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 5=0-3-8, 3=Mechanical, 4=Mechanical Max Horz 5=72(LC 16) Max Uplift 5=-30(LC 16), 3=-28(LC 16)

Max Grav 5=231(LC 21), 3=89(LC 21), 4=52(LC 7)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) -0-11-0 to 2-1-0. Interior(1) 2-1-0 to 2-11-7 zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 3.
- 8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.







LOADING (psf) TCLL (roof) Snow (Pf/Pg) 15.4 TCDL	25.0 4/20.0 10.0	<b>SPACING-</b> Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.15 1.15 YES	CSI. TC BC WB	0.32 0.18 0.00	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.02 -0.04 0.02	(loc) 4-5 4-5 3	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20	<b>GRIP</b> 197/144
BCDL	10.0	Code IRC2018/TP	12014	Matri	x-AS						Weight: 13 lb	FT = 20%

TOP CHORD2x4 SPF No.2BOT CHORD2x4 SPF No.2WEBS2x4 SPF No.2

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied, except end verticals. Rigid ceiling directly applied.

REACTIONS. (size) 5=0-3-8, 3=Mechanical, 4=Mechanical Max Horz 5=96(LC 16) Max Uplift 5=-29(LC 16), 3=-45(LC 16) Max Grav 5=286(LC 2), 3=162(LC 21), 4=84(LC 7)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-5=-251/154

#### NOTES-

 Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) -0-11-0 to 2-1-0, Interior(1) 2-1-0 to 4-7-14 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60

2) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

3) Unbalanced snow loads have been considered for this design.

4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 3.

 This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.







Max Horz 6=57(LC 13) Max Uplift 6=-24(LC 12), 4=-16(LC 13), 3=-6(LC 13) Max Grav 6=262(LC 34), 4=175(LC 34), 3=84(LC 35)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) 0-1-12 to 3-1-12, Interior(1) 3-1-12 to 5-3-1 zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=20.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 3) Unbalanced snow loads have been considered for this design.
- 4) Provide adequate drainage to prevent water ponding.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6, 4, 3.
- 8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and
- referenced standard ANSI/TPI 1.
- 9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 11) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.



16023 Swingley Ridge Rd Chesterfield, MO 63017



BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing

#### TOP CHORD 2x4 SPF No.2 2x4 SPF No.2 BOT CHORD

WEBS 2x4 SPF No.2

REACTIONS. All bearings Mechanical (lb) -

Max Horz 4=-59(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) 1, 2 Max Grav All reactions 250 lb or less at joint(s) 4, 1, 3, 2

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

- 1) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Corner(3) zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=20.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 2.
- 7) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 9) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.







- 2) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=20.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4. 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 4=123. 3=123.
- 7) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.







			5-0-3 5-0-3						
LOADING (psf)           TCLL (roof)         25.0           Snow (Pf/Pg)         15.4/20.0           TCDL         10.0           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2018/TPI2014	CSI. TC 0.36 BC 0.20 WB 0.03 Matrix-AS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.02 -0.05 0.00	(loc) 4-5 4-5	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 17 lb	<b>GRIP</b> 197/144 FT = 20%
LUMBER-		BR	ACING-						

TOP CHORD

BOT CHORD

# LUMBER-

TOP CHORD 2x4 SPF No 2 2x4 SPF No.2 BOT CHORD WEBS

2x4 SPF No.2

5=0-3-8, 4=Mechanical REACTIONS. (size) Max Horz 5=99(LC 16) Max Uplift 5=-29(LC 16), 4=-38(LC 16) Max Grav 5=296(LC 21), 4=229(LC 21)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. TOP CHORD 2-5=-259/155

### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) -0-11-0 to 2-1-0, Interior(1) 2-1-0 to 4-10-7 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) Refer to girder(s) for truss to truss connections.

- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 4.
- 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and
- referenced standard ANSI/TPI 1.
- 10) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



Structural wood sheathing directly applied, except end verticals.

Rigid ceiling directly applied.





LOADING (psf) TCLL (roof) 25.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.65 BC 0.40 WB 0.06	<b>DEFL.</b> Vert(LL) Vert(CT) Horz(CT)	in -0.08 -0.17 0.00	(loc) 4-5 4-5	l/defl >995 >443 n/a	L/d 240 180 n/a	PLATES MT20	<b>GRIP</b> 197/144
BCDL 0.0	Code IRC2018/TPI2014	Matrix-AS						Weight: 22 lb	FT = 20%
LUMBER-		BR	ACING-					ŀ	

TOP CHORD

BOT CHORD

#### LUMBER-

TOP CHORD 2x4 SPF No.2 BOT CHORD 2x4 SPF No.2 WEBS 2x4 SPF No.2

REACTIONS. (size) 5=0-3-8, 4=Mechanical

Max Horz 5=122(LC 16)

Max Uplift 5=-28(LC 16), 4=-51(LC 16) Max Grav 5=370(LC 2), 4=309(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-5=-317/165

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) -0-11-0 to 2-1-0, Interior(1) 2-1-0 to 6-6-14 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 4.
- 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 10) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



Structural wood sheathing directly applied, except end verticals.

Rigid ceiling directly applied.





Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

Mitek<sup>®</sup> 16023 Swingley Ridge Rd Chesterfield, MO 63017
			RELEAS	E FOR			
Job	Truss	Truss Type	CONSTRU	JCTIØN F	Ply		140005057
2523941	.IM11	Diagonal Hip	Girder AS NOTED ON PL		V 1		143385057
		Blagenarnip	DEVELOPMEN	T SERVICES	•	Job Reference (optional)	
Builders FirstSource (Valley	Center), Valley Center, I	KS - 67147,	LEE'S SUMMIT	, MISSOURI8.2	240 s Ma	ar 9 2020 MiTek Industries, Inc. Tue Oct 27 18:39:4	5 2020 Page 2
			ID:VF	PVqvFnP0P0b1j2t2	ZrlOqez	dKbx-wVQv2aaRKm9ym6H9WF_Xm_?5gbyuOBzw2	z_OWbyP7ay
			01/26/2	2021			
LOAD CASE(S) Standard							
Concentrated Loads (lb)							

Concentrated Loads (lb) Vert: 10=-12(F) 11=-20(B) 12=-89(F) 13=-11(F) 14=-4(B) 15=-37(F) 16=-201(B)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11 Quality Criteria**, **DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





LUM	BE	R-
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BCDL

TOP CHORD2x4 SPF No.2BOT CHORD2x4 SPF No.2SLIDERLeft 2x4 SPF No.2 2-0-0

10.0

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied. Rigid ceiling directly applied.

REACTIONS. (size) 1=0-3-8, 3=Mechanical, 4=Mechanical Max Horz 1=63(LC 16) Max Uplift 3=-44(LC 16) Max Grav 1=225(LC 20), 3=160(LC 20), 4=82(LC 7)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) 0-0-0 to 3-0-0, Interior(1) 3-0-0 to 4-6-15 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60
- TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.
- 7) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and
- referenced standard ANSI/TPI 1.
- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designe. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





		2-0-11		1			
LOADING (psf)         SPACING-         2-0-0           TCLL (roof)         25.0         Plate Grip DOL         1.15           Snow (Pf/Pg)         15.4/20.0         Lumber DOL         1.15           TCDL         10.0         Rep Stress Incr         YES           BCDL         10.0         Code IRC2018/TPI2014	CSI. TC 0.08 BC 0.03 WB 0.00 Matrix-MR	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (lo -0.00 -0.00 4 -0.00	oc) l/defl 5 >999 4-5 >999 3 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 7 lb	<b>GRIP</b> 197/144 FT = 20%

# LUMBER-

TOP CHORD2x4 SPF No.2BOT CHORD2x4 SPF No.2WEBS2x4 SPF No.2

BRACING-TOP CHORD

Structural wood sheathing directly applied or 2-0-11 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 5=0-3-8, 3=Mechanical, 4=Mechanical Max Horz 5=59(LC 16) Max Uplift 5=-31(LC 16), 3=-17(LC 16)

Max Grav 5=185(LC 21), 3=49(LC 21), 4=33(LC 7)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 3.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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#### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (directional); cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 4.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 10) "NAILED" indicates 3-10d (0.148"x3") or 2-12d (0.148"x3.25") toe-nails per NDS guidlines.
- 11) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 69 lb down and 45 lb up at 5-4-10 on top chord, and 24 lb down at 5-4-10 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 12) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

# LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-2=-51, 2-3=-51, 4-5=-20 Concentrated Loads (lb)

Vert: 3=-53(B) 4=-16(B) 8=0(B) 9=0(F)



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	4	4-0-0			
LOADING (psf)         SPACING-         2-0-0           TCLL (roof)         25.0         Plate Grip DOL         1.15           Snow (Pf/Pg)         15.4/20.0         Lumber DOL         1.15           TCDL         10.0         Rep Stress Incr         YES           BCDL         10.0         Code IRC2018/TPI2014	CSI. TC 0.21 BC 0.13 WB 0.00 Matrix-AS	DEFL.         in         (loc)           Vert(LL)         -0.01         4-5           Vert(CT)         -0.02         4-5           Horz(CT)         -0.01         3	l/defl L/d >999 240 >999 180 n/a n/a	PLATES MT20 Weight: 11 lb	<b>GRIP</b> 197/144 FT = 20%

### LUMBER-

TOP CHORD2x4 SPF No.2BOT CHORD2x4 SPF No.2WEBS2x4 SPF No.2

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied, except end verticals. Rigid ceiling directly applied.

REACTIONS. (size) 5=0-3-8, 3=Mechanical, 4=Mechanical Max Horz 5=86(LC 16) Max Uplift 5=-29(LC 16), 3=-38(LC 16) Max Grav 5=284(LC 21), 3=132(LC 21), 4=71(LC 7)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-5=-253/143

#### NOTES-

 Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) -0-11-0 to 2-1-0, Interior(1) 2-1-0 to 3-11-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60

2) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 3.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



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16023 Swingley Ridge Rd Chesterfield, MO 63017

# Design valid for use only design parameters during the both the bo

			RELEASE FOR	
Job	Truss	Truss Type		
2523941	IM16	Half Hin	AS NOTED ON PLANS REVIEW	143385062
2020041		rian rip	DEVELOPMENT SERVICES 2	Job Reference (optional)
Builders FirstSource (Valley	Center), Valley Center, k	S - 67147,	LEE'S SUMMIT, MISSOURI8.240 s M	ar 9 2020 MiTek Industries, Inc. Tue Oct 27 18:39:50 2020 Page 2
			ID:VPVqvFnP0P0b1j	2tZrlOqezdKbx-HTDo5HeZ9InFstA7JpaiT1ivfcdB3V?fCFh8BoyP7at
			01/26/2021	
LOAD CASE(S) Standard				
1) Dead + Snow (balanced	): Lumber Increase=1.15, Pl	ate Increase=	.15	
Listen and the second state (second		-		

Uniform Loads (plf) Vert: 1-2=-51, 2-4=-61, 5-8=-20

Concentrated Loads (lb)

Vert: 6=-938

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Plate Offsets (X,Y) [2:0-2-8,0	J-U-1]								
LOADING (psf) TCLL (roof) 25.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr NO	CSI. TC 0.17 BC 0.10 WB 0.00	<b>DEFL.</b> Vert(LL) Vert(CT) Horz(CT)	in -0.01 -0.01 0.00	(loc) 5-8 5-8 2	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20	<b>GRIP</b> 197/144
BCDL 0.0	Code IRC2018/TPI2014	Matrix-MP	· · · ·					Weight: 12 lb	FT = 20%
LUMBER-		BR	ACING-						

TOP CHORD

BOT CHORD

#### LUMBER-

TOP CHORD 2x4 SPF No.2 BOT CHORD 2x4 SPF No.2 SLIDER

Left 2x4 SPF No.2 2-0-0

REACTIONS.

(size) 4=Mechanical, 2=0-4-3, 5=Mechanical Max Horz 2=54(LC 12) Max Uplift 4=-24(LC 12), 2=-48(LC 12)

Max Grav 4=105(LC 17), 2=280(LC 17), 5=59(LC 7)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

- 1) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (directional); cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 2.
- 8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and
- referenced standard ANSI/TPI 1.
- 9) "NAILED" indicates 3-10d (0.148"x3") or 2-12d (0.148"x3.25") toe-nails per NDS guidlines.
- 10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

#### LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-4=-51, 5-6=-20 Concentrated Loads (lb)

Vert: 11=0(B)



Structural wood sheathing directly applied or 3-6-11 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

**MiTek** 16023 Swingley Ridge Rd Chesterfield, MO 63017

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BRACING-

TOP CHORD

BOT CHORD

2x4 SPF No 2 TOP CHORD 2x4 SPF No.2 BOT CHORD

WEBS

2x4 SPF No.2

REACTIONS. (size) 3=Mechanical, 4=Mechanical, 5=0-3-8 Max Horz 5=56(LC 16) Max Uplift 3=-14(LC 16), 5=-32(LC 16)

Max Grav 3=38(LC 21), 4=28(LC 7), 5=175(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

- 1) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Refer to girder(s) for truss to truss connections.
- 7) Bearing at joint(s) 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 5.
- 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



Structural wood sheathing directly applied or 1-9-3 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

🗼 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design valid for use only with MiTeK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





1				25-2-1							1
	25-2-1         25-2-1         25-0       SPACING-       2-0-0       CSI.       DEFL.       in       (loc)       I/defl       L/d       PLATES       GRIP         psf)       20.4/20.0       Plate Grip DOL       1.15       TC 0.06       Vert(LL)       n/a       -       n/a       999       MT20       197/144         10.0       Rep Stress Incr       YES       WB 0.04       Vert(CT)       n/a       -       n/a       999       MT20       197/144         0.0       Code IRC2018/TPI2014       Matrix-S       WB 0.04       Matrix-S       Vert(CT)       n/a       n/a       Meight: 105 lb       FT = 20%										
Plate Offs	ets (X,Y) [3:0-2-10,	Edge], [14:0-2-10,Edge]									
LOADING TCLL (roc Snow (Pf/ TCDL BCLL	i (psf) f) 25.0 Pg) 20.4/20.0 10.0 0.0	SPACING- 2 Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2018/TPI20	2-0-0 <b>CSI.</b> 1.15 TC 1.15 BC YES WB 014 Matr	0.06 0.03 0.04 ix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.00	(loc) - - 16	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 105 lb	<b>GRIP</b> 197/144 FT = 20%
LUMBER	RD 2x4 SPF No.2			BR.	ACING- P CHORD S	Structura	l wood	sheathing	directly app	lied or 6-0-0 oc purlins	except
DOT OUC						0.0	a contine a	(0 0 0			

BOT CHORD	2x4 SPF No.2		2-0-0 oc purlins (6-0-0 max.): 3-14.
OTHERS	2x4 SPF No.2	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS.** All bearings 25-2-1.

(lb) - Max Horz 1=-104(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) 1, 22, 23, 25, 26, 28, 21, 20, 19, 17

Max Grav All reactions 250 lb or less at joint(s) 1, 16, 22, 23, 25, 26, 27, 21, 20, 19, 18, 17 except 28=252(LC 23)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=15ft; B=45ft; L=25ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) 0-3-15 to 3-3-15, Interior(1) 3-3-15 to 3-10-9, Exterior(2R) 3-10-9 to 8-1-8, Interior(1) 8-1-8 to 21-3-7, Exterior(2E) 21-3-7 to 24-10-2 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=20.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 4) Provide adequate drainage to prevent water ponding.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 22, 23, 25, 26, 28, 21, 20, 19, 17.
- 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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- II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) 0-3-15 to 3-3-15, Interior(1) 3-3-15 to 8-7-0, Exterior(2R) 8-7-0 to 11-7-0, Interior(1) 11-7-0 to 16-10-2 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 9, 15, 14, 12, 11 except (jt=lb) 17=102, 10=102.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSITPTI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





NOTES-

 Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 3, 4.

6) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

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NUMBER PE-2017018993 October 28,2020



LOADING (psf)           TCLL (roof)         25.0           Snow (Pf/Pg)         20.4/20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2018/TPI2014	<b>CSI.</b> TC 0.25 BC 0.25 WB 0.04 Matrix-R	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc n/a n/a 0.00	i) l/defl - n/a - n/a 5 n/a	L/d 999 999 n/a	PLATES MT20 Weight: 20 lb	<b>GRIP</b> 197/144 FT = 20%
LUMBER- TOP CHORD 2x4 SPF No.2		BRA TOP	CING- CHORD Si	tructural wo	od sheathir	g directly ap	plied or 3-1-0 oc purlin	s,

BOT CHORD2x4 SPF No.2except end verticals, and 2-0-0 oc purlins: 2-4.WEBS2x4 SPF No.2BOT CHORDRigid ceiling directly applied or 8-8-3 oc bracing.OTHERS2x4 SPF No.2CHORDRigid ceiling directly applied or 8-8-3 oc bracing.

REACTIONS. All bearings 3-1-0.

(lb) - Max Horz 8=123(LC 11)

5-6=-418/432

Max Uplift All uplift 100 lb or less at joint(s) 8 except 5=-291(LC 11), 6=-316(LC 12), 7=-137(LC 11) Max Grav All reactions 250 lb or less at joint(s) 8, 7 except 5=271(LC 12), 6=338(LC 11)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

# BOT CHORD

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=20.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
   4) Provide adequate drainage to prevent water ponding.
- Gable requires continuous bottom chord bearing.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8 except (jt=lb) 5=291, 6=316, 7=137.
- 8) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 5.
- 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



MiTek

16023 Swingley Ridge Rd Chesterfield, MO 63017

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			RELEAS	E FOR					
Job	Truss	Truss Type	CONSTRU		y I				140005000
2523941	LG5	GABLE	AS NOTED ON PL DEVELOPMEN	ANS REVIEW	1 Job Ref	ference (op	tional)		143385069
Builders FirstSource	(Valley Center),	Valley Center, KS - 67147,	LEE'S SUMMIT	, MISSOURI8.24	10 s Mar 9 2020	MiTek Inc	lustries, Inc.	Tue Oct 27 18:40:15 2	020 Page 1
			01/26/2	021	IJZIZNO QEZUKU	x-2GEFKQ	yrG_xiivijav		FZaryP7aU
			5-1-8	'					
		l	2x4						Scale = 1:35.7
			1						
		58-12 1	2 2%	13.42 <u> 12</u>					
			5 4	2x4 📏					
			2x4    2x4						
			ł						
LOADING (psf) TCLL (roof) Snow (Pf/Pg) 15.4 TCDL	25.0 /20.0 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.24 BC 0.05 WB 0.05	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) n/a - n/a - 0.00 3	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20	<b>GRIP</b> 197/144
BCDL	10.0	Code IRC2018/TPI2014	Matrix-P	. ,				Weight: 23 lb	FT = 20%
LUMBER- TOP CHORD 2x4 BOT CHORD 2x4 WEBS 2x4 OTHERS 2x4	SPF No.2 SPF No.2 SPF No.2 SPF No.2		BR TO BC	ACING- P CHORD Si ex T CHORD Ri	tructural wood ccept end verti igid ceiling dire	sheathing cals. ectly applie	directly ap ed or 10-0-0	plied or 5-1-8 oc purlin ) oc bracing.	S,

REACTIONS. (size) 5=5-1-8, 3=5-1-8, 4=5-1-8 Max Horz 5=-182(LC 10)

Max Uplift 5=-64(LC 12), 3=-27(LC 13), 4=-124(LC 14) Max Grav 5=71(LC 24), 3=167(LC 23), 4=312(LC 24)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-322/334

BOT CHORD 4-5=-250/257, 3-4=-250/257

WFBS 2-4=-292/216

NOTES-

1) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) 0-1-12 to 4-4-11, Interior(1) 4-4-11 to 4-9-10 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 3 except (jt=lb) 4=124.

6) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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 Satisfy for storage, delivery, erection and bracing of trusses and truss systems, see
 ANSI/TPI1 Qu

 Safety Information
 available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6 except (jt=lb) 4=248, 5=190.

7) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 4.

8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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			RELEAS	E FOR			
Job	Truss	Truss Type	CONSTR				1433850
2523941	LG7	GABLE	DEVELOPMEN	T SERVICES	1 Job Reference (d	optional)	
Builders FirstSource (Valle	ey Center), Val	ey Center, KS - 67147,	LEE'S SUMMI 01/25/ 5-5-4	<b>, MISSOURI</b> 8.240 : ID:VPVqvFnP0P0b1j <b>2021</b>	s Mar 9 2020 MiTek li 2tZrlOqezdKbx-SrvNz	ndustries, Inc. Tue O s_7ZvJHDAJVyh4Gb	Det 27 18:40:18 2020 Page 1 DxslruJJdAMnecTdBAyP7aR
			2x4    1				Scale = 1:3
		0-1-9	7 6 2x4    2x4    5-5-4	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$			
LOADING (psf) TCLL (roof) 25.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0	SPA Plate Luml	CING-         2-0-0           Grip DOL         1.15           per DOL         1.15	CSI. TC 0.27 BC 0.05	DEFL. Vert(LL) Vert(CT)	in (loc) l/defl n/a - n/a n/a - n/a	L/d 999 999	PLATES         GRIP           MT20         197/144
BCLL 0.0 BCDL 10.0	Rep Code	Stress Incr YES IRC2018/TPI2014	WB 0.04 Matrix-P	Horz(CT) 0	0.00 4 n/a	n/a	Weight: 26 lb FT = 20
LUMBER- TOP CHORD 2x4 SPF BOT CHORD 2x4 SPF WEBS 2x4 SPF OTHERS 2x4 SPF	No.2 No.2 No.2 No.2		BI TC BC	RACING- DP CHORD Stru exce DT CHORD Rigi	ctural wood sheathin ept end verticals. d ceiling directly app	g directly applied or ied or 10-0-0 oc bra	r 5-5-4 oc purlins, acing.
REACTIONS. All bea (lb) - Max Hor Max Upl Max Gra	rings 5-5-4. z 7=-194(LC 10) ift All uplift 100 lb w All reactions 25	or less at joint(s) 7, 4, 6 0 lb or less at joint(s) 7,	, 5 4, 6, 5				
FORCES.         (lb) - Max. C           TOP CHORD         2-3=-29           BOT CHORD         6-7=-20	omp./Max. Ten A 91/296, 3-4=-391/3 65/274, 5-6=-265/2	ll forces 250 (lb) or less 91 74, 4-5=-265/274	except when shown.				
NOTES- 1) Wind: ASCE 7-16; Vu	It=115mph (3-seco	nd gust) Vasd=91mph; 1	CDL=6.0psf; BCDL=4.2psf; I	n=15ft; B=45ft; L=24ft	; eave=4ft; Cat.		

- II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) 0-1-12 to 4-4-11, Interior(1) 4-4-11 to 5-1-6 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7, 4, 6, 5.
- 6) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- NUMBER PE-2017018993 October 28,2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





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 Satisfies
 Ansi/TPH Qu

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16023 Swingley Ridge Rd Chesterfield, MO 63017

October 28,2020



Max Uplift All uplift 100 lb or less at joint(s) 1, 9, 13, 16, 15, 14, 12, 11, 10

Max Grav All reactions 250 lb or less at joint(s) 1, 9, 13, 16, 15, 14, 12, 11 except 10=263(LC 2)

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FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-290/262
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#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) 0-3-15 to 3-3-15, Interior(1) 3-3-15 to 7-4-9, Exterior(2R) 7-4-9 to 10-4-9, Interior(1) 10-4-9 to 14-10-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=20.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 4) Provide adequate drainage to prevent water ponding.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 9, 13, 16, 15, 14, 12, 11, 10.
- 9) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 9, 12, 11, 10.
- 10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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Snow (Pf/Pg) TCDL BCLL BCDL	20.4/20.0 10.0 0.0 10.0	Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2018/TPI2014	TC 0.07 BC 0.04 WB 0.10 Matrix-S	Vert(LL) Vert(CT) Horz(CT)	n/a n/a -0.00	- - 9	n/a n/a n/a	999 999 n/a	MT20 Weight: 68 lb	197/144 FT = 20%
LUMBER- TOP CHORD BOT CHORD	2x4 SPF No.2 2x4 SPF No.2		BF TC	RACING- DP CHORD	Structural	wood : urlins (	sheathin 6-0-0 m	g directly ap ax.): 5-9.	plied or 6-0-0 oc purlir	s, except

LOWDER-		DIVACING-	
TOP CHORD	2x4 SPF No.2	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except
BOT CHORD	2x4 SPF No.2		2-0-0 oc purlins (6-0-0 max.): 5-9.
OTHERS	2x4 SPF No.2	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 14-4-8.

Max Horz 1=200(LC 14) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 1, 9, 13, 16, 15, 14, 12, 11, 10

Max Grav All reactions 250 lb or less at joint(s) 1, 9, 13, 16, 15, 14, 12, 11, 10

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 1-2=-269/243

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) 0-3-15 to 3-3-15, Interior(1) 3-3-15 to 6-7-15, Exterior(2R) 6-7-15 to 9-7-15, Interior(1) 9-7-15 to 14-1-6 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=20.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 4) Provide adequate drainage to prevent water ponding.
- All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 7)
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 9, 13, 16, 15, 14, 12, 11, 10.
- 9) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 9, 12, 11, 10.
- 10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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Max Horz 1=-113(LC 12) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 1 except 8=-104(LC 14), 6=-104(LC 14)

Max Grav All reactions 250 lb or less at joint(s) 1, 5, 7 except 8=253(LC 23), 6=253(LC 24)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) 0-3-15 to 3-3-15, Interior(1) 3-3-15 to 4-3-10, Exterior(2R) 4-3-10 to 7-3-10, Interior(1) 7-3-10 to 8-3-6 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 8=104 6=104

7) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design valid for use only with MiTeK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



<sup>3)</sup> TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10



BOT CHORD

TOP CHORD 2x4 SPF No.2 BOT CHORD

2x4 SPF No.2 OTHERS 2x4 SPF No.2

REACTIONS. All bearings 7-8-13.

Max Horz 1=-101(LC 12) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 1, 5, 8, 6 Max Grav All reactions 250 lb or less at joint(s) 1, 5, 7, 8, 6

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) 0-3-15 to 3-3-15, Interior(1) 3-3-15 to 3-10-6, Exterior(2R) 3-10-6 to 6-10-6, Interior(1) 6-10-6 to 7-4-14 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5, 8, 6.

7) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

🛦 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design valid for use only with MiTeK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



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		Γ	RELEAS	E FOR			
Job	Truss	Truss Type	CONSTRU				143385080
2523941	МЗ	Roof Special G	INDEVELOPMEN	ANS REVIEW		otional)	
Builders FirstSource (Valley	Center), Valley Center, K	S - 67147,	LEE'S SUMMIT	, MISSOURI8.240 s M	Mar 9 2020 MiTek In ZriOgezdKby-baynsy	dustries, Inc. Tue Oct 2	27 18:40:27 2020 Page 1
-0 <sub>-</sub> 11 0-11	<u>-0 4-0-0 7-1-12</u>	10-3-8 12	2-0-0 16-9-12 <b>01/26/2</b>	<b>021</b> 21-7-8	26-5-4	32-4-0	bvvDgL2301v3C: 3yr rai
0-11	-0 3-1-12	J-1-12	4-3-12	<del>4</del> -5-12		5-10-12	Scolo - 1:72 7
					4x4 =	6x8 =	Scale = 1.72.7
Тт					9	<u>33</u> 10 10 10 10 10 10 10 10 10 10 10 10 10	тт
				4x4 📁		/3x6 =	
				8			
			6x6 📁	32			
9-8		NAILED 2x4	7				8 6-8 6
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4	26 2	728 29 30				28 25	ļ.
			7 16 14	13	12	11	
0.4	≥ 20 <sup>34</sup>	19 <sup>35</sup> 18	36 15 15 2x4    8x16 MT20H	s = 4x4 =	4x8 =	2x4	
2	4x8 = 3x8 =	6x8 = 2x4	8x12 =				
	SUR26 NAILED	NAILED	2				
		1110020-	2				
	4-0-0 7-1-12	10-3-8 12	2-0-0 16-9-12	21-7-8	26-5-4	32-4-0	
Plate Offsets (X,Y) [2:0	-0-0,0-1-1], [6:0-4-12,0-3-0],	[7:0-1-8,0-3-0],	[10:0-1-8,0-3-0], [15:0-0-0	.0-2-12], [15:0-2-8,0-4-	-4-9-12 ·8], [17:0-4-12,0-4-4	], [19:0-3-8,0-3-0], [20	:0-3-8,0-1-8]
LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc) l/defl	L/d PI	ATES GRIP
Snow (Pf/Pg) 20.4/20.0	Plate Grip DOL Lumber DOL	1.15 1.15	TC 0.72 BC 0.74	Vert(LL) -0.4 Vert(CT) -0.8	l9 16 >789 37 16 >444	240 M 180 M	T20 197/144 T20HS 148/108
TCDL 10.0 BCLL 0.0	Rep Stress Incr	NO	WB 0.85	Horz(CT) 0.1	12 25 n/a	n/a	
BCDL 10.0	Code IRC2018/	I PI2014	Matrix-MS			VV	eight: 383 lb F I = 20%
LUMBER- TOP CHORD 2x4 SPF N	o.2 *Except*		BR TC	ACING- P CHORD Struct	ural wood sheathing	g directly applied or 5-	11-14 oc purlins,
3-6: 2x4 SF	PF 1650F 1.5E, 6-7: 2x6 SPF	No.2	BC	excep	t end verticals, and	2-0-0 oc purlins (3-4-	15 max.): 3-6, 9-10.
2-18: 2x6 S	SPF No.2		WE	EBS 1 Row	v at midpt	10-25	.9.
5-18: 2x4 SFF N	SPF 1650F 1.5E						
OTHERS 2x4 SPF N WEDGE	0.2						
Left: 2x4 SP No.3							
REACTIONS. (size)	2=0-3-8, 25=0-3-8						
Max Uplift	2=234(LC 12) 2=-228(LC 12), 25=-188(LC	12)					
Max Grav	2=2350(LC 38), 25=1692(LC	C 2)					
FORCES. (lb) - Max. Con TOP CHORD 2-3=-392	np./Max. Ten All forces 250 3/357, 3-4=-3345/327, 4-5=-	0 (lb) or less ex 12315/1113, 5-	cept when shown. 6=-12863/1166, 6-7=-5099	/371.			
7-8=-258	9/188, 8-9=-1260/112, 9-10=	-1038/120	6 17- 1219/12246	- ,			
14-16=-1	204/12245, 13-14=-478/4588	3, 12-13=-236/2	2228				
WEBS 17-18=-6 4-17=-64	4/747, 3-20=-107/1583, 4-20 7/6900, 6-17=-423/984, 6-16	)=-3070/222, 4- i=-853/119, 6-1	19=-1291/217, 17-19=-647 4=-8102/756, 7-14=-181/24	/5372, I85,			
7-13=-29	31/301, 8-13=-132/1718, 8-1	2=-1961/223, 1	0-12=-198/1755, 10-25=-1	693/188			
NOTES-	tod togothor with 10d (0 121	'v2") pails as fo	llowe			A S	OF MISS
Top chords connected a	s follows: 2x4 - 1 row at 0-7-	0 oc, 2x6 - 2 ro	ws staggered at 0-9-0 oc.			ALLE	
Bottom chords connecte Webs connected as follo	d as follows: 2x6 - 2 rows sta ws: 2x4 - 1 row at 0-9-0 oc.	aggered at 0-8-	0 oc.			ASI	ANDREW Y
2) All loads are considered	equally applied to all plies, e	except if noted a	as front (F) or back (B) face	in the LOAD CASE(S)	) section. Ply to		JOINSON +
<ol> <li>3) Unbalanced roof live loa</li> </ol>	ds have been considered for	this design.		se mulcaleu.		Jun	Hong
<ol> <li>Wind: ASCE 7-16; Vult= II; Exp C; Enclosed; MW</li> </ol>	115mph (3-second gust) Vas /FRS (directional); cantilever	sd=91mph; TCL left and right ex	DL=6.0psf; BCDL=4.2psf; h xposed ; end vertical left an	=15ft; B=45ft; L=24ft; e id right exposed; Lumb	eave=4ft; Cat. per DOL=1.60	AR DE	NUMBER
plate grip DOL=1.60 5) TCLL: ASCE 7-16 <sup>•</sup> Pr=2	5.0 psf (roof LL: Lum DOI =1	.15 Plate DOI =	=1.15): Pg=20.0 psf· Pf=20	4 psf (Lum DOI =1 15	Plate	A The	
DOL=1.15); Is=1.0; Rou	gh Cat C; Partially Exp.; Ce=	1.0; Cs=1.00; C	Ct=1.10, Lu=50-0-0; Min. fla	at roof snow load gover	rns. Rain	We'ssi	ONAL EN
6) Unbalanced snow loads	have been considered for th	is design.				D	
<ol> <li>I his truss has been des Comme of the property of the other</li> </ol>	igned for greater of min roof er live loads.	live load of 12.0	pst or 1.00 times flat roof	load of 15.4 psf on ove	ernangs	00	xoper 28,2020
WARNING - Verify desia	n parameters and READ NOTES ON	THIS AND INCLUD	ED MITEK REFERENCE PAGE MII	-7473 rev. 5/19/2020 BEFORI	E USE.		
Design valid for use only w a truss system. Before use	ith MiTek® connectors. This design i , the building designer must verify the	is based only upon e applicability of de	parameters shown, and is for an ir sign parameters and properly inco	dividual building component, porate this design into the over	, not verall		iļ,
building design. Bracing in is always required for stabil fabrication, storage, deliver	arcated is to prevent buckling of indiv lity and to prevent collapse with poss v erection and bracing of trusses or	vidual truss web and sible personal injury	avor chord members only. Addition and property damage. For generation ANSI/TPI1 Quality Cr	al temporary and permanent al guidance regarding the iteria DSB-89 and BCSI Pro-	ilding Component		€ Vingley Ridge Rd
Safety Information availa	ble from Truss Plate Institute. 2670	Crain Highway. Suit	te 203 Waldorf. MD 20601			Chesterfi	eld. MO 63017

			RELEASE F	OR	
Job Tru	ISS	Truss Type	CONSTRUCT		140005000
2523941 M3		Roof Special	Girder AS NOTED ON PLAN	SREVIEW	143385080
	,		DEVELOPMENT SE	RVICES	Job Reference (optional)
Builders FirstSource (Valley Cen	nter), Valley Center, KS	6 - 67147,	LEE'S SUMMIT, MI	<b>SSOURI</b> 8.240 s M	1ar 9 2020 MiTek Industries, Inc. Tue Oct 27 18:40:27 2020 Page 2
			ID:V	PVqvFnP0P0b1j2tz	rlOgezdKbx-haynsx5nRgR?oZVE 4kNTrkBbWDgE236iV9c?9yP7al

01/26/2021

## NOTES-

8) Provide adequate drainage to prevent water ponding.

9) All plates are MT20 plates unless otherwise indicated.

10) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 11) Bearing at joint(s) 25 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=228, 25=188.

13) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

14) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

15) Use Simpson Strong-Tie SUR26 (6-10d Girder, 6-10dx1 1/2 Truss) or equivalent at 4-0-0 from the left end to connect truss(es) to back face of bottom chord, skewed 45.0 deg to the right, sloping 0.0 deg. down.

16) Use Simpson Strong-Tie HHUS26-2 (14-10d Girder, 4-10d Truss) or equivalent at 9-10-2 from the left end to connect truss(es) to back face of bottom chord.

17) Fill all nail holes where hanger is in contact with lumber.

18) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.

#### LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-3=-51, 3-6=-61, 6-9=-51, 9-10=-61, 18-22=-20, 11-17=-20

Concentrated Loads (lb)

Vert: 20=-263(B) 27=-71(B) 29=-71(B) 34=-25(B) 35=-25(B) 36=-707(B)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





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			RELEASE FOR	
Job	Truss	Truss Type	CONSTRUCTION Ply	
2523941	MA	Roof Special	AS NOTED ON PLANS REVIEW	143385081
2020041	WI <del>T</del>		DEVELOPMENT SERVICES	Job Reference (optional)
Builders FirstSource	e (Valley Center),	Valley Center, KS - 67147,	LEE'S SUMMIT, MISSOURI8.240 s M	ar 9 2020 MiTek Industries, Inc. Tue Oct 27 18:40:29 2020 Page 2
			ID:VPVqvFnP0P0b1j2	ZrlOqezdKbx-ez4XHd61zHij2sec5VnrYGqWlKr5i_qPApej31yP7aG
NOTES-			04/26/2024	

01/26/2021
 11) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 12) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the top chord and 1/2" gypsum sheetr

bottom chord.

13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601







F	8-1-12	10-3-8	16-1-12	22-5-4		32-4-0		
Plate Offsets (X,Y) [2:0-4-10.	Edgel. [6:0-4-12.0-2-0].	[14:0-5-4.0-3-4].	[16:0-3-8.0-2-0]	0-5-0		5-10-12		
LOADING (psf)           TCLL (roof)         25.0           Snow (Pf/Pg)         20.4/20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2018/T	2-0-0 1.15 1.15 YES PI2014	CSI. TC 0.76 BC 0.95 WB 0.89 Matrix-AS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.24 10-11 -0.52 10-11 0.12 22	l/defl L/d >999 240 >742 180 n/a n/a	PLATES MT20 Weight: 153 lb	<b>GRIP</b> 197/144 FT = 20%
LUMBER- TOP CHORD 2x4 SPF No.2 BOT CHORD 2x4 SPF No.2 WEBS 2x4 SPF No.2 OTHERS 2x4 SPF No.2 SLIDER Left 2x4 SPF No.2 REACTIONS. (size) 2=0-3 Max Horz 2=19 Max Uplift 2=-12	2 2-6-0 -8, 22=0-3-8 7(LC 16) 2(LC 16), 22=-138(LC	16)		BRACING- TOP CHORD BOT CHORD WEBS	Structural wood 2-0-0 oc purlins Rigid ceiling dir 1 Row at midpt	sheathing directly app (2-6-5 max.): 4-6, 7-9. ectly applied. 6-11, 8-10	lied, except end vertio	als, and
Max Grav 2=151 FORCES. (lb) - Max. Comp./Ma TOP CHORD 2-4=-2268/272, 10-17=-161/127 BOT CHORD 2-16=-403/2037 WEBS 4-16=-803/206, 7-11=0/456, 8-1	13(LC 2), 22=1421(LC 2 ax. Ten All forces 250 4-5=-2986/393, 5-6=-3 70, 9-17=-161/1270 7, 5-14=-552/75, 13-14= 14-16=-398/2147, 4-14 11=-146/1032, 8-10=-14	2) (lb) or less exceµ 054/405, 6-7=-18 =-528/3465, 11-13 I=-220/1869, 6-14 I48/244, 9-22=-14	ot when shown. 153/219, 7-8=-1562 3=-526/3469, 10-1 4=-493/3, 6-11=-2 <sup>-</sup> 424/193	2/234, 1=-158/935 172/321,				
<ul> <li>NOTES-</li> <li>1) Unbalanced roof live loads hav</li> <li>2) Wind: ASCE 7-16; Vult=115m;</li> <li>II; Exp C; Enclosed; MWFRS (11-1-12, Interior(1) 11-1-12 to exposed; end vertical left and grip DOL=1.60</li> <li>3) TCLL: ASCE 7-16; Pr=25.0 ps DOL=1.15); Is=1.0; Rough Cai surcharge applied to all expose</li> <li>4) Unbalanced snow loads have</li> <li>5) This truss has been designed in non-concurrent with other live</li> <li>6) Provide adequate drainage to 7) This truss has been designed in capacity of bearing surface.</li> <li>9) Provide mechanical connection 2=122, 22=138.</li> <li>10) This truss is designed in accorreferenced standard ANSI/TF</li> <li>11) This truss design requires the Contification constrained to the function of the standard ANSI/TF</li> </ul>	ve been considered for oh (3-second gust) Vas directional) and C-C Ex 22-5-4, Exterior(2R) 22 right exposed;C-C for r f (roof LL: Lum DOL=1. t C; Partially Exp.; Ce= d surfaces with slopes been considered for this for greater of min roof li loads. prevent water ponding. for a 10.0 psf bottom ch s parallel to grain value n (by others) of truss to prdance with the 2018 I P1. at a minimum of 7/16" s to the bottom chord.	this design. d=91mph; TCDL= terior(2E) -0-11- -5-4 to 25-5-4, In nembers and ford 15 Plate DOL=1. I.0; Cs=1.00; Ct= less than 0.500; Ct= less than 0.500; Ct= s design. ve load of 12.0 p ord live load non using ANSI/TPI - bearing plate cap nternational Resi tructural wood sh	=6.0psf; BCDL=4.2 ) to 2-1-0, Interior( terior(1) 25-54 to 3 ces & MWFRS for 15); Pg=20.0 psf; f 1.10, Lu=50-0-0; N 12 in accordance v sf or 1.00 times fla concurrent with an 1 angle to grain for pable of withstandi dential Code section eathing be applied	tpsf; h=15ft; B=45ft; L 1) 2-1-0 to 8-1-12, Ex 31-10-12 zone; cantil reactions shown; Lun Pf=20.4 psf (Lum DO) Alin. flat roof snow loa vith IBC 1608.3.4. t roof load of 15.4 psf y other live loads. mula. Building desig ng 100 lb uplift at join ons R502.11.1 and Ri I directly to the top ch	=24ft; eave=4ft; terior(2R) 8-1-12 ever left and righ hber DOL=1.60 p L=1.15 Plate d governs. Rain on overhangs ner should verify t(s) except (jt=lb) 802.10.2 and ord and 1/2" gyp	Cat. to late	ANDREW THOMAS TOHNSON PE-201701899 PE-201701899 October 28,2	
WARNING - Verify design param Design valid for use only with MiTe a truss system. Before use, the bui building design. Bracing indicated is always required for stability and fabrication, storage, delivery, erect Safety Information available from	eters and READ NOTES ON T k® connectors. This design is diding designer must verify the is to prevent buckling of indivi to prevent collapse with possi on and bracing of trusses and n Truss Plate Institute, 2670 C	HIS AND INCLUDED based only upon par- applicability of desigr dual truss web and/or ble personal injury and truss systems, see rain Highway, Suite 2	MITEK REFERENCE P/ ameters shown, and is i parameters and prope chord members only. d property damage. Fo <b>ANS//TPI1 Qu</b> 03 Waldorf, MD 20601	AGE MII-7473 rev. 5/19/2020 for an individual building co ry incorporate this design i Additional temporary and p r general guidance regardir ality Criteria, DSB-89 and	D BEFORE USE. mponent, not nto the overall ermanent bracing ng the BCSI Building Com	ponent	NITEK 16023 Swingley Ridge Rd Chesterfield, MO 63017	

			RELEASE FO	DR			
Job	Truss	Truss Type	CONSTRUCT	<b>ØN</b> Ply			
25230/1	M5	Roof Special	AS NOTED ON PLANS	REVIEW	1	143;	385082
2020041	WIS .		DEVELOPMENT SEF	RVICES	'	Job Reference (optional)	
Builders FirstSource (Valley	Center), Valley Center, K	S - 67147,	LEE'S SUMMIT, MIS	SOURI8.240	)sMa	r 9 2020 MiTek Industries, Inc. Tue Oct 27 18:40:31 2020 Pag	ge 2
			ID:VP	VqvFnP0P0b1	j2tZr	)qezdKbx-aLCIhI8HVuyRHAo?DwpJdhvsx7XEArJhd77p8wyP7	7aE
NOTES-	entation does not depict the	size or the or	01/26/2021	and/or botton	n cha	rd	
						u.	

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601







	5-2-10	10-3-8	18-1-12	21-10-12	2 26-1	1-10	32-4-0				
Plate Offsets (X	Y) [2:0-0-0,0	0-0-11], [5:0-6-0,0-2-0], [10:Edge,0-2-0],	[11:0-6-0,0-2-4], [14:0-7-	·12,0-2-12], [17:0	)-3-8,0-2-0]	-14	5-4-0				
LOADING (psf) TCLL (roof) Snow (Pf/Pg) 2 TCDL BCLL BCLL	25.0 20.4/20.0 10.0 0.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2018/TPI2014	CSI. TC 0.65 BC 0.86 WB 0.80 Matrix-AS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.30 10-11 -0.63 10-11 0.09 22	l/defl L/d >999 240 >610 180 n/a n/a	PLATES MT20 Weight: 172 I	<b>GRIP</b> 197/144 b FT = 20%			
LUMBER- TOP CHORD BOT CHORD WEBS OTHERS WEDGE Left: 2x4 SP No.	10.0 2x4 SPF No.2 *E 1-4,4-5: 2x6 SPF 2x4 SPF No.2 2x4 SPF No.2 2x4 SPF No.2 3	Except* F No.2	BF TC BC WI	Racing- Racing- DP Chord DT Chord EBS	Structural wood a 2-0-0 oc purlins ( Rigid ceiling dire 10-0-0 oc bracine 1 Row at midpt	sheathing directly (3-5-2 max.): 4-5, ctly applied. Exc g: 14-16 4-12,	/ applied, except end ver , 7-9. .ept: 8-10	rticals, and			
REACTIONS.	KEACTIONS.         (size)         2=0-3-8, 22=0-3-8           Max Horz         2=172(LC 16)           Max Uplift         2=-127(LC 16), 22=-132(LC 16)           Max Grav         2=1517(LC 2), 22=1473(LC 43)										
FORCES. (Ib) TOP CHORD BOT CHORD WEBS NOTES- 1) Unbalanced r 2) Wind: ASCE II; Exp C; End 13-0-0, Interir exposed ; end grip DOL=1.6 3) TCLL: ASCE DOL=1.15); Is surcharge ap 4) Unbalanced s 5) This truss has non-concurre 6) Provide adeq 7) This truss has non-concurre 6) Provide adeq 7) Provide med	Max Grav 2=1517(LC 16), 22=132(LC 16) Max Grav 2=1517(LC 2), 22=1473(LC 43) <b>FORCES.</b> (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-2511/273, 3-4=-2693/366, 4-5=-2645/348, 5-6=-1969/263, 6-7=-2034/293, 7-8=-1881/267, 10-18=-155/1288, 018=-155/1288 30T CHORD 2-17=-200/2131, 4-14=-32/246, 12-14=-342/2421, 11-12=-412/2640, 10-11=-190/1175 WEBS 3-17=-388/130, 14-17=-381/2057, 3-14=-14/439, 4-12=-17/334, 5-11=-1511/251, 6-11=-195/1723, 7-11=-1090/162, 8-11=-129/1038, 8-10=-1588/261, 9-22=-1477/188 <b>NOTES-</b> 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; IWVFRS (directional) and C-C Exterior(2E) -0-11-0 to 2-1-0, Interior(1) 2-1-0-10-2, Exterior(2R) 10-0-0 to 13-0-0, Interior(1) 13-0-0 to 21-2-0, Exterior(2E) -0-11-0 to 2-1-0, Interior(1) 2-1-0-12, const-10-12, const-10-13, const-10,										
2=127, 22=13 10) This truss is Continuieteoneologie	32. s designed in acc stenzdard ANSI/T	ordance with the 2018 International Re PI 1.	sidential Code sections R	502.11.1 and R8	302.10.2 and		October 28	3,2020			

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

16023 Swingley Ridge Rd Chesterfield, MO 63017

				RELEASE FOR	2		
J	lob	Truss	Truss Type		N Ply		
2	2523941	M6	Roof Special	AS NOTED ON PLANS R	EVIEW	1433	385083
Ľ				DEVELOPMENT SERVI	ICES	Job Reference (optional)	
	Builders FirstSource (Valley	Center), Valley Center, K	S - 67147,	LEE'S SUMMIT, MISSO	<b>DURI</b> 8.240 s Ma	r 9 2020 MiTek Industries, Inc. Tue Oct 27 18:40:34 2020 Pag	je 2
				ID:VPVqvFr	nP0P0b1j2tZrlOq	ezdKbxwtQKKAAnpK?8eXau2M0FJXOyLaLNDU8J5LTIFyP7	'aB
	NOTES-			01/26/2021			
	<ol><li>This truss design requir</li></ol>	res that a minimum of 7/16" s	structural woo	d sheathing be applied directly to the t	top chord and 1	/2" gypsum sheetrock be applied directly to the	
	bottom chord.						

bottom chord.
12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11 Quality Criteria**, **DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601







	5-3-8	10-3-8	18-8-12		23-10-12	25-4-10	32-4-0		
Plate Offsets ()	5-3-8 (.Y) [5:0-3-12	5-0-0 .0-3-0]. [6:0-6-0.0-2-0]. [8:0-3-6.Edge]. [	8-5-4 15:0-7-12.0-2-12], [17:0-3-3	8.0-2-01	5-2-0	'1-5-14 '	6-11-6	•	
LOADING (psf	f)	<b>SPACING.</b> 2-0-0			in (loc)	l/defl	l /d	PI ATES	GRIP
TCLL (roof)	25.0	Plate Grip DOL 1.15	TC 0.74	Vert(LL)	-0.20 13-15	>999	240	MT20	197/144
TCDL	10.0	Lumber DOL 1.15	BC 0.75	Vert(CT)	-0.47 13-15	>818	180		
BCLL	0.0	Rep Stress Incr YES	WB 0.67 Matrix AS	Horz(CT)	0.11 11	n/a	n/a	Woight: 150 lb	ET - 20%
BCDL	10.0		Matrix-AS					weight. 159 b	FT = 20%
LUMBER-			BRA	ACING-					
TOP CHORD	2x4 SPF No.2 *I	Except*	TOF	P CHORD	Structural woo	d sheathing	g directly applied	, except end vertic	als, and
	5-6: 2X6 SPF NG 2x4 SPF No 2	0.2	BOT	CHORD	2-0-0 oc purin Rigid ceiling di	s (3-2-4 Ma rectly appli	ax.): 5-6, 8-10. ed		
WEBS	2x4 SPF No.2		WEI	BS S	1 Row at midp	t	5-13, 9-11		
SLIDER	Left 2x4 SPF No	0.2 2-6-0					,		
REACTIONS.	(size) 2=0- Max Horz 2=19 Max Uplift 2=-1 Max Grav 2=15	3-8, 11=0-3-8 17(LC 15) 43(LC 16), 11=-120(LC 16) i14(LC 2), 11=1448(LC 2)							
FORCES. (Ib)	) - Max. Comp./N	ax. Ten All forces 250 (lb) or less exc	ept when shown. 2773/429 7-82259/355						
	8-9=-2006/290	)	2113/429, 1-0=-2239/333,						
BOT CHORD	2-17=-444/208	30, 5-15=-29/544, 13-15=-453/2317, 12-	13=-305/1764, 11-12=-221	/1186					
WEBS	4-17=-385/132	2, 15-17=-400/2007, 4-15=-6/346, 5-13=	-79/251, 6-13=-1613/294,						
	7-13=-272/179	91, 9-12=-142/1197, 9-11=-1691/277, 8-	12=-1206/219, 7-12=-90/54	16					
NOTES-									
1) Unbalanced	roof live loads ha	we been considered for this design.							
2) Wind: ASCE	7-16; Vult=115m	hph (3-second gust) Vasd=91mph; TCD	L=6.0psf; BCDL=4.2psf; h=	15ft; B=45ft; L	=32ft; eave=4ft;	Cat.			
to 13-4-9 In	terior(1) 13-4-9 to	21-2-0 Exterior(2E) 21-2-0 to 23-10-12	-0 to 2-3-13, intend (1) 2-3 2 Interior(1) 23-10-12 to 32	-13 to 10-1-12	tilever left and i	right			
exposed ; er	nd vertical left and	d right exposed;C-C for members and for	rces & MWFRS for reaction	ns shown; Lum	ber DOL=1.60	plate		man	
grip DOL=1.	60						6	OF MIC	D
3) TCLL: ASCE	= 7-16; Pr=25.0 p	st (root LL: Lum DOL=1.15 Plate DOL=1	1.15); Pg=20.0 pst; Pt=20.4 t=1.10_Lu=50_0.0; Min_flat	pst (Lum DOL	=1.15 Plate		A.	LE	N.O.
surcharge at	pplied to all expos	sed surfaces with slopes less than 0.500	0/12 in accordance with IBC	1608.3.4.	a governs. Trail		BAS	ANDREW	N &
4) Unbalanced	snow loads have	been considered for this design.					871	THOMAS	1~ 8
5) This truss ha	as been designed	for greater of min roof live load of 12.0	psf or 1.00 times flat roof lo	ad of 15.4 psf	on overhangs		$\mathcal{N} \star \mathcal{I}$	IOHNSON	1+4
6) Provide ade	ent with other live	loads.					1 tak	mal	win
7) This truss ha	as been designed	for a 10.0 psf bottom chord live load no	enconcurrent with any other	live loads.			May -	NUMBER	128
<ol> <li>Provide med 2=143. 11=1</li> </ol>	chanical connection	on (by others) of truss to bearing plate c	apable of withstanding 100	lb uplift at join	t(s) except (jt=lt	)	N ROL	PE-201701899	13 / E
9) This truss is referenced s	designed in acco	rdance with the 2018 International Resi	dential Code sections R502	2.11.1 and R80	02.10.2 and		N.S.	19.19.2	NOT
10) This truss of sheetrock h	design requires the	hat a minimum of 7/16" structural wood s	sheathing be applied directl	y to the top ch	ord and 1/2" gy	psum	A.	CONAL E	5
11) Graphical p	ourlin representat	ion does not depict the size or the orien	tation of the purlin along the	e top and/or bo	ottom chord.			October 28,2	2020
WARNIN Design valid	IG - Verify design para I for use only with MiT	neters and READ NOTES ON THIS AND INCLUDE	D MITEK REFERENCE PAGE MII-7 arameters shown, and is for an ind	7473 rev. 5/19/2020 ividual building co	BEFORE USE.				

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Design valid for use only with MTeKe connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

			RI	ELEASE FOR				
Job	Truss	Truss Type	CO	NSTRUCTION	Ply			
2523941	M7	Roof Special	AS NOTE DEVEL	D ON PLANS REVIE OPMENT SERVICES	≣ <b>W</b> 1 S 1	Job Reference (o	ptional)	143385085
Builders FirstSource (Valley Center), Valley Center, KS - 6		KS - 67147,	LEE'S	SUMMIT, MISSOUR	8.240 s M	ar 9 2020 MiTek In	dustries, Inc. Tue Oct 27 18:40	:40 2020 Page 1
				ID:VPVqvFn	P0P0b1j2t2	rlOqezdKbx-p4Eia	NFxNf49sZ_kEJTQUanNVmffnv	/U0i1ooyvyP7a5
-Q <u>-11-Q</u> 4	4-6-2 8-8-12	10-3-8	16-8-12	01/26/2021 20-8-12	2 <sub>1</sub> -7-4	25-10-12	32-4-0	
0-11-0 4	4-6-2 4-2-10	1-6-12	6-5-4	4-0-0	0-10-8	4-3-8	6-5-4	
								0 1 1 1 1 1

Scale = 1:61.8

16023 Swingley Ridge Rd Chesterfield, MO 63017



	L	8-8-12	10-3-8	16-8-12	20-8-12	2 <sub>1</sub> -7-4	25-10-12	- 3	32-4-0	
	1	8-8-12	'1-6-12 '	6-5-4	4-0-0	0-10-8	4-3-8	1	6-5-4	
Plate Offsets (X,Y	<u>′) [5:0-4-10</u>	,Edge], [12:Edge,0-1-8], [	13:0-3-8,0-2-0	], [18:0-6-12,0-3-4]						
LOADING (psf) TCLL (roof) Snow (Pf/Pg) 20 TCDL BCLL	25.0 .4/20.0 10.0 0.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.15 1.15 YES	CSI. TC 0.84 BC 0.74 WB 0.90	DEFL. Vert(LL) Vert(CT) Horz(CT	in -0.23 1 -0.44 1 ) 0.14	(loc) l/defl  7-18 >999  7-18 >873  12 n/a	L/d 240 180 n/a	PLATES MT20	<b>GRIP</b> 197/144
BCDL	10.0	Code IRC2018/TP	12014	Matrix-AS					Weight: 159 lb	FT = 20%
LUMBER- TOP CHORD 2: BOT CHORD 2: WEBS 2: SLIDER LG REACTIONS.	x4 SPF No.2 x4 SPF No.2 x4 SPF No.2 eft 2x4 SPF No (size) 2=0- Max Horz 2=17 Max Uplift 2=-1 Max Grav 2=15	0.2 2-6-0 3-8, 12=0-3-8 '9(LC 15) 44(LC 16), 12=-119(LC 16 i14(LC 2), 12=1448(LC 2)	5)		BRACING- TOP CHORD BOT CHORD	Structural 2-0-0 oc p Rigid ceili	I wood sheathi purlins (2-2-0 r ing directly ap	ng directly app nax.): 5-7, 8-9, blied.	lied, except end vertio 10-11.	cals, and
FORCES. (Ib) - TOP CHORD BOT CHORD WEBS	Max. Comp./M 2-4=-2396/348 8-9=-1897/339 2-20=-450/207 13-14=-332/21 5-20=-1063/24 8-15=-137/793 11-13=-328/20	lax. Ten All forces 250 (l 9, 4-5=-2255/334, 5-6=-27/ 9, 9-10=-2114/340, 10-11= 77, 6-18=-727/148, 17-18= 107 13, 18-20=-374/2241, 5-18 3, 9-15=-107/576, 9-14=-3- 302	b) or less exca 60/426, 6-7=-3 -2066/311, 11 -508/2827, 15 =-320/2043, 6 4/320, 10-14=-	ept when shown. 1185/464, 7-8=-2214/36 -12=-1379/229 -17=-525/3186, 14-15= -17=-37/435, 7-15=-17 -420/85, 10-13=-1041/2	64, =-306/1814, 59/275, 220,					
NOTES- 1) Unbalanced ro 2) Wind: ASCE 7- II; Exp C; Encld 11-11-9, Interior zone; cantilever shown; Lumbe 3) TCLL: ASCE 7 DOL=1.15); Is= surcharge appl 4) Unbalanced sm 5) This truss has non-concurrent 6) Provide adequa 7) This truss has 8) Provide mechan 2=144, 12=119 9) This truss is der referenced star 10) This truss des sheetrock be 11) Graphical pur Design valid for Design valid for Design valid for	of live loads ha -16; Vult=115m bsed; MWFRS or(1) 11-11-9 to r left and right r DOL=1.60 pla -16; Pr=25.0 p =1.0; Rough Ca lied to all expose to wo loads have been designed to with other live ate drainage to been designed unical connection assigned in acco ndard ANSI/TF sign requires th applied directly tin representat	ave been considered for the hph (3-second gust) Vasde (directional) and C-C Exterior (2E) 20-8-12, Exterior (2E) 20-8-12, Exterior (2E) 20-8-12, Exterior (2E) 20-8-12, exposed ; end vertical left ate grip DOL=1.60 sf (roof LL: Lum DOL=1.19 at C; Partially Exp.; Ce=1.01 sed surfaces with slopes to been considered for this e been considered for this e loads. P prevent water ponding. I for a 10.0 psf bottom cho on (by others) of truss to be rdance with the 2018 Inte P1 1. That a minimum of 7/16" string to the bottom chord. ion does not depict the size the size and READ NOTES ON THE sed sed son the size the sed son the size the sed son the set of the size the sed son the set of the sed son the set of the s	is design. =91mph; TCDI rior(2E) -0-11 8-12 to 21-7-4 and right expr 5 Plate DOL=1 (5 S=1.00; Ct eass than 0.500 design. e load of 12.0   rd live load no earing plate ca include the orient s AND INCLUDET S AND INCLUDET	L=6.0psf; BCDL=4.2psf -0 to 2-3-13, Interior(1) 4, Exterior(2R) 21-7-4 to bosed;C-C for members -1.10, Lu=50-0-0; Min. /12 in accordance with psf or 1.00 times flat ro nconcurrent with any o apable of withstanding dential Code sections F heathing be applied dia- tation of the purlin alone MITEK REFERENCE PAGE rameters shown, and is for a	; h=15ft; B=45ft; L 2-3-13 to 8-8-12, o 24-10-1, Interior and forces & MW 20.4 psf (Lum DO flat roof snow loa IBC 1608.3.4. of load of 15.4 ps ther live loads. 100 lb uplift at joir 8502.11.1 and R8 rectly to the top ch g the top and/or b MII-7473 rev. 5/19/202	=32ft; eave Exterior(2F (1) 24-10-1 FRS for rea L=1.15 Pla d governs. f on overha ht(s) except 02.10.2 and hord and 1/2 ottom chord	e=4ft; Cat. R) 8-8-12 to to 32-2-4 actions te Rain ungs t (jt=lb) d 2" gypsum d. SE.	O A A PROVINCIAL	ANDREW THOMAS JOHNSON NUMBER PE-201701899 PE-201701899 PE-201701899 October 28,2	
Design valid for a truss system. building design	r use only with MiT Before use, the but Bracing indicated red for stability and	ek® connectors. This design is b uilding designer must verify the a d is to prevent buckling of individu	ased only upon pa oplicability of designation al truss web and/ opersonal injury of	arameters shown, and is for a gn parameters and properly in or chord members only. Add	n individual building concorporate this design itional temporary and p	omponent, not into the overal permanent bra	ll Icing		MiTek <sup>°</sup>	

billing design. Bilandig indicates to be prevent buckning of individual duss web and/of viola infinite only. Additional employed and a billing and permanent blanding is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS//TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

			R	RELEASE FOR				
Job	Truss	Truss Type	C	ONSTRUCTIO	N Ply			
2523941	M8	Roof Special	AS NOT DEVE	ED ON PLANS RI	EVIEW 1	Job Reference (optional)		143385086
Builders FirstSource (Valley	Center), Valley Center,	KS - 67147,	LEE'S	S SUMMIT, MISSO	URI8.240 s M	ar 9 2020 MiTek Industries, Inc.	Tue Oct 27	18:40:42 2020 Page 1
				ID:VPVqvF	nP0P0b1j2tZrlQ	qezdKbx-ITMS?3GBvHKt5s86M	/kWua?sn6Z	GbFq3J9LHv1nyP7a3
-Q-11-Q	6-8-12	10-3-8	14-8-12	01/28/9191	23-7-4	27-10-12	31-3-8	32-4-0
0-11-0	6-8-12	3-6-12	4-5-4	4-0-0	4-10-8	4-3-8	3-4-12	1-0-8

Scale = 1:61.1



	6-	8-12	10-3-8	14-8-12		18-8-12		23-7-4	1	31-3-8	32-4-0	
Plate Offsets (X,Y)	) [9:0-4-0.0		3-6-12 ael. [18:0-3-8.0-2-0]	4-5-4		4-0-0		4-10-8		7-8-4	1-0-8	
LOADING (psf) TCLL (roof) Snow (Pf/Pg) 20. TCDL BCLL BCDL	25.0 4/20.0 10.0 0.0 10.0	SPACING- Plate Grip D Lumber DOI Rep Stress Code IRC20	2-0-0 OL 1.15 - 1.15 Incr YES 018/TPI2014	CSI. TC BC WB Matr	0.64 0.97 0.89 ix-AS	DE Vei Vei Ho	FL. t(LL) t(CT) rz(CT)	in (loc) -0.31 15 -0.56 15-16 0.18 11	l/defl >999 >688 n/a	L/d 240 180 n/a	PLATES MT20 MT20HS Weight: 141 lb	<b>GRIP</b> 197/144 148/108 FT = 20%
LUMBER- TOP CHORD 2x BOT CHORD 2x WEBS 2x SLIDER Le REACTIONS. M M	44 SPF No.2 44 SPF No.2 44 SPF No.2 eft 2x4 SPF No (size) 11=0 (size) 11=0 lax Horz 2=14 lax Uplift 11=- lax Grav 11=1	.2 2-6-0 -3-8, 2=0-3-8 4(LC 15) 118(LC 16), 2=-144 448(LC 2), 2=1514	4(LC 16) 4(LC 2)			BRACING- TOP CHORI BOT CHORI WEBS	D 9 2 D F	Structural woc 2-0-0 oc purlir Rigid ceiling d 1 Row at midp	d sheathii s (2-4-3 n irectly app t	ng directly app nax.): 4-6, 7-8 Jlied. 9-11	blied, except end vertic , 9-10.	als, and
FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown.         TOP CHORD       2-4=-2404/359, 4-5=-3806/573, 5-6=-4286/636, 6-7=-2824/453, 7-8=-1966/358, 8-9=-2282/364         BOT CHORD       2-18=-392/2086, 5-16=-696/129, 15-16=-641/3880, 13-15=-693/4293, 12-13=-405/2434, 11-12=-356/2095         WEBS       4-18=-507/144, 16-18=-376/1985, 4-16=-290/2154, 5-15=-57/484, 6-13=-2219/346, 7-13=-172/1353, 7-12=-734/130, 8-12=-48/659, 9-11=-2359/397												
<ul> <li>T-13=-172/1353, 7-12=-734/130, 8-12=-48/659, 9-11=-2359/397</li> <li>NOTES-         <ol> <li>Unbalanced roof live loads have been considered for this design.</li> <li>Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=15ft; B=45ft; L=31ft; eave=4ft; Cat.</li></ol></li></ul>												
WARNING - Design valid for a truss system. building design. is always requiri fabrication, storz Safety Informat	Verify design paran use only with MiTo Before use, the bu Bracing indicated ed for stability and age, delivery, erec tion available from	neters and READ NOTE ek® connectors. This de ilding designer must ve l is to prevent buckling of to prevent collapse with tion and bracing of fruss m Truss Plate Institute,	S ON THIS AND INCLUDE sign is based only upon p- rify the applicability of desi of individual truss web and possible personal injury a ses and truss systems, see 2670 Crain Highway, Suite	D MITEK REFE arameters shov gn parameters for chord memb and property da a ANS 203 Waldorf, I	RENCE PAGI wn, and is for and properly pers only. Ad- image. For g i/TP11 Qualit MD 20601	E MII-7473 rev. 5/ an individual buil incorporate this d ditional temporar eneral guidance y Criteria, DSB-	19/2020 E ding com design into y and per regarding 89 and B	BEFORE USE. ponent, not o the overall manent bracing the CSI Building Col	nponent		NITEK 16023 Swingley Ridge Rd Chesterfield, MO 63017	





	4-8-10	7-6-1	10-3-8 12	-8-10 16-8-10		21-2-0	21-2-0 25-7-6			28-3-11	31-3-8 32-4-0	
Diata Offecto (	<u>4-8-10</u>		<u>2-9-7 2</u>	<u>-5-2 ' 4-0-0</u>		<u>4-5-6</u>		4-5-0	<u>5</u>	2-8-5	2-11-13 1-0-8	
Plate Offsets (X,Y) [2:0-0-0,-2-5], [4:0-2-0,0-1-8], [6:0-5-8,0-3-4], [7:0-4-10,Edge], [14:0-3-8,0-1-8], [16:0-4-12,0-5-0], [19:0-5-8,0-2-4]												
LOADING (ps TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDI	f) 25.0 20.4/20.0 10.0 0.0 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2018/	2-0-0 1.15 1.15 NO TPI2014	CSI. TC 0.94 BC 0.94 WB 0.70 Matrix-MS		DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.56 -0.95 0.19	(loc) 17 17 12	l/defl >690 >405 n/a	L/d 240 180 n/a	PLATES MT20 MT20HS Weight: 322 lb	<b>GRIP</b> 197/144 148/108 FT = 20%
LUMBER- TOP CHORD BOT CHORD WEBS WEDGE Left: 2x4 SP N	2x4 SPF No.2 *1 3-6: 2x4 SPF 16 2x6 SPF No.2 *1 2-20,15-18: 2x6 2x4 SPF No.2 *1 4-18: 2x4 SPF 1 0.3	Except* 550F 1.5E, 6-7: 2x6 SPF Except* SPF 2100F 1.8E Except* 650F 1.5E	= No.2		BF TC BC	RACING- DP CHORD	Structura except e 10-11. Rigid cei	al wood nd verti ling dire	sheathing cals, and ectly appli	g directly app 2-0-0 oc purl ed or 10-0-0	lied or 4-8-4 oc purlins ins (2-4-13 max.): 3-6, oc bracing.	s, 7-9,
REACTIONS.	(size) 12=0 Max Horz 2=10 Max Uplift 12=- Max Grav 12=3	0-3-8, 2=0-3-8 06(LC 11) 530(LC 12), 2=-469(LC 3346(LC 41), 2=2982(L0	12) C 2)									
FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown.         TOP CHORD       2-3=-5344/829, 3-4=-7551/1170, 4-5=-14754/2361, 5-6=-15968/2675, 6-7=-9621/1681, 7-8=-8450/1499, 8-9=-7789/1341, 9-10=-6300/1058         BOT CHORD       2-21=-723/4756, 19-21=-727/4774, 17-18=-2350/14855, 16-17=-2665/16090, 14-16=-1248/7786, 13-14=-869/5589, 12-13=-817/4958, 18-19=-1194/7963         WEBS       3-21=-316/71, 3-19=-556/3799, 4-19=-4638/792, 7-16=-671/3987, 8-16=-202/955, 8-14=-1019/209, 9-14=-488/2830, 9-13=-105/904, 10-13=-127/746, 10-12=-5815/992, 4-18=-1366/8258, 5-18=-1086/270, 6-17=-1316/279, 6-16=-8064/1326, 5-17=-358/1420												
NOTES- 1) 2-ply truss t Top chords Bottom choi Webs conne 2) All loads are ply connecti 3) Unbalanced 4) Wind: ASCE II; Exp C; Et plate grip D 5) TCLL: ASCI DOL=1.15); surcharge a 6) Unbalanced 7) This truss h non-concurr (8)rfitnuideoacter	o be connected to connected as follows: 2 ected as follows: 2 e considered eque ons have been pr roof live loads ha E 7-16; Vult=115m closed; MWFRS OL=1.60 E 7-16; Pr=25.0 p Is=1.0; Rough Cz pplied to all expidence as been designed ent with other live agents drainage to	bgether with 10d (0.131' ows: 2x4 - 1 row at 0-4- follows: 2x6 - 2 rows st 2x4 - 1 row at 0-9-0 oc. ally applied to all plies, e rovided to distribute only ave been considered for ph (3-second gust) Var (directional); cantilever sf (roof LL: Lum DOL=1 at C; Partially Exp.; Ce= sed surfaces with slope been considered for th d for greater of min roof b loads. prevent water ponding	"x3") nails as fol 0 oc, 2x6 - 2 rov aggered at 0-9-( except if noted a y loads noted as r this design. sd=91mph; TCD left and right ex 1.15 Plate DOL= =1.0; Cs=1.00; C s less than 0.50 is design. live load of 12.0	lows: ws staggered at 0-7- ) oc. s front (F) or back (E s (F) or (B), unless of posed ; end vertical 1.15); Pg=20.0 psf; t=1.10, Lu=50-0-0; f 0/12 in accordance psf or 1.00 times flage	0 oc. B) face therwi 2psf; ł left ar Pf=20 Min. fla with IE at roof	e in the LOAD C. se indicated. n=15ft; B=45ft; L nd right exposed 1.4 psf (Lum DOI at roof snow load 3C 1608.3.4. load of 15.4 psf	ASE(S) se =31ft; eav ; Lumber =1.15 Pla d governs on overha	ection. F re=4ft; ( DOL=1 ate . Rain angs	Ply to Cat. 60	O the second	ANDREW THOMAS JOHNSON NUMBER PE-201701899 HISSIONAL E October 28,2	
Design vali a truss syst building des	IG - Verify design parar d for use only with MiT em. Before use, the bu sign. Bracing indicated	meters and READ NOTES ON ek® connectors. This design uilding designer must verify th d is to prevent buckling of indi	THIS AND INCLUDE is based only upon p e applicability of des vidual truss web and	D MITEK REFERENCE P parameters shown, and is ign parameters and prope l/or chord members only.	AGE MI for an i erly inco Additio	II-7473 rev. 5/19/2020 ndividual building co prporate this design in anal temporary and p	BEFORE U mponent, no nto the overa ermanent bra	SE. t all acing				

billing design. Bilandig indicates to be prevent buckning of individual duss web and/of viola infinite only. Additional employed and a billing and permanent blanding is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS//TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

16023 Swingley Ridge Rd Chesterfield, MO 63017
				RELEASE FOR	
	Job	Truss	Truss Type	CONSTRUCTION Ply	140005007
252	2523941	М9	Roof Special Gird	Girder AS NOTED ON PLANS REVIEW	143385087
	20011			DEVELOPMENT SERVICES 2	Job Reference (optional)
	Builders FirstSource (Valley	Center), Valley Center, K	S - 67147,	LEE'S SUMMIT, MISSOURI8.240 s Ma	ar 9 2020 MiTek Industries, Inc. Tue Oct 27 18:40:48 2020 Page 2
				ID:\/P\/avEnP0P0b1i2t7	rlOgezdKby-adiiG6LyLl750ppbGi_clpG6iy_KTf7LlCYGkCDRyP77z

# NOTES-

01/26/2021

- 9) All plates are MT20 plates unless otherwise indicated.
- 10) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 12=530, 2=469.
   This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 14) Use Simpson Strong-Tie SUL26 (6-10d Girder, 6-10dx1 1/2 Truss) or equivalent at 4-8-10 from the left end to connect truss(es) to front face of bottom chord, skewed 45.0 deg.to the left, sloping 0.0 deg. down.
- 15) Use Simpson Strong-Tie SUL26 (6-10d Girder, 6-10dx1 1/2 Truss) or equivalent at 16-8-10 from the left end to connect truss(es) to front face of bottom chord, skewed 45.0 deg.to the left, sloping 0.0 deg. down.
- 16) Use Simpson Strong-Tie LUS24 (4-10d Girder, 2-10d Truss, Single Ply Girder) or equivalent spaced at 2-0-0 oc max. starting at 19-2-0 from the left end to 25-2-0 to connect truss(es) to front face of bottom chord.
- 17) Fill all nail holes where hanger is in contact with lumber.
- 18) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.
- 19) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 44 lb down at 5-2-0, and 289 lb down and 71 lb up at 17-2-0, and 615 lb down and 119 lb up at 25-7-6 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

# LOAD CASE(S) Standard

- 1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
- Uniform Loads (plf) Vert: 1-3=-51, 3-6=-61, 6-7=-51, 7-9=-61, 9-10=-51, 10-11=-61, 19-22=-20, 19-20=-20, 12-18=-20, 18-19=-20
- Concentrated Loads (lb)

Vert: 21=-273(F) 16=-615(F) 14=-289(F) 13=-615(F) 25=-104(F) 28=-101(F) 29=-101(F) 35=-36(F) 36=-36(F) 37=-36(F) 38=-155(F) 39=-11(F) 40=-25(F) 41=-289(F) 42=-289(F) 43=-289(F) 44=-289(F)





## Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601 NITEK 16023 Swingley Ridge Rd Chesterfield, MO 63017

			RELEASE FOR	
Job	Truss	Truss Type		
2523941	P1	Hip Girder	AS NOTED ON PLANS REVIEW	143385088
2020011			DEVELOPMENT SERVICES	Job Reference (optional)
Builders FirstSource (Valley C	Center), Valley Center, K	S - 67147,	LEE'S SUMMIT, MISSOURI8.240 s	Mar 9 2020 MiTek Industries, Inc. Tue Oct 27 18:40:49 2020 Page 2
			ID:VPVqvFnP0P0b1j2tZrl	DqezdKbx-2pH5TSMaFQDtRxASGi8YMUf5rOtMOBTLmwUlmtyP7Zy
			01/26/2021	
LOAD CASE(S) Standard				
Concentrated Loads (lb)				

Concentrated Loads (lb) Vert: 3=-30(B) 4=-30(B) 7=-14(B) 8=-14(B)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11 Quality Criteria**, **DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





REACTIONS. (size) 2=0-3-8, 4=0-3-8 Max Horz 2=37(LC 15) Max Uplift 2=-42(LC 16), 4=-42(LC 16) Max Grav 2=333(LC 2), 4=332(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-263/163, 3-4=-263/163

# NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) -0-7-8 to 2-4-8, Interior(1) 2-4-8 to 3-2-8, Exterior(2R) 3-2-8 to 6-5-0, Interior(1) 6-5-0 to 7-0-8 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.
   This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designe. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSIVTPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





	3-2-8		3-9-12	6-5-0		_	
	3-2-8		0-7-4	2-7-4		1	
Plate Offsets (X,Y) [2:0-3-0,0	0-1-2], [4:0-3-0,0-1-2], [5:Edge,0-1-8]						
LOADING         (psf)           TCLL (roof)         25.0           Snow (Pf/Pg)         20.4/20.0           TCDL         10.0           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2018/TPI2014	<b>CSI.</b> TC 0.24 BC 0.29 WB 0.02 Matrix-AS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) l/defl -0.04 7 >999 -0.07 7 >999 0.00 6 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 20 lb	<b>GRIP</b> 197/144 FT = 20%
LUMBER-		BI	RACING-				

TOP CHORD

BOT CHORD

TOP CHORD 2x4 SPF No.2

BOT CHORD2x4 SPF No.2WEBS2x4 SPF No.2

REACTIONS. (size) 8=0-3-8, 6=0-3-8 Max Horz 8=54(LC 15)

Max Uplift 8=-28(LC 12), 6=-28(LC 13)

Max Grav 8=294(LC 40), 6=294(LC 40)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

# NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) 0-1-12 to 2-7-4, Interior(1) 2-7-4 to 3-2-8, Exterior(2E) 3-2-8 to 3-9-12, Interior(1) 3-9-12 to 6-3-4 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=20.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 4) Unbalanced snow loads have been considered for this design.

5) Provide adequate drainage to prevent water ponding.

- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8, 6.
   This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



Structural wood sheathing directly applied, except end verticals, and

2-0-0 oc purlins (6-0-0 max.): 1-2, 4-5.

Rigid ceiling directly applied

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#### 5-9-12 Plate Offsets (X,Y)--[2:0-3-0,Edge], [3:Edge,0-1-8] LOADING (psf) SPACING-DEFL. GRIP 2-0-0 CSI. in (loc) l/defl L/d PLATES 25.0 TCLL (roof) Plate Grip DOL 1.15 тс 0.22 Vert(LL) 999 MT20 197/144 n/a n/a Snow (Pf/Pg) 20.4/20.0 Lumber DOL 1.15 BC 0.19 Vert(CT) n/a n/a 999 TCDL 10.0 Rep Stress Incr YES WB 0.00 Horz(CT) 0.00 4 n/a n/a BCLL 0.0 Code IRC2018/TPI2014 FT = 20% Weight: 14 lb Matrix-R BCDL 10.0 LUMBER-BRACING-TOP CHORD 2x4 SPF No.2 TOP CHORD Structural wood sheathing directly applied or 5-9-12 oc purlins, BOT CHORD 2x4 SPF No.2 except end verticals, and 2-0-0 oc purlins: 2-3.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

REACTIONS. (size) 1=5-9-4, 4=5-9-4 Max Horz 1=36(LC 15)

2x4 SPF No.2

Max Uplift 1=-18(LC 16), 4=-23(LC 13)

Max Grav 1=227(LC 35), 4=240(LC 34)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

# NOTES-

WEBS

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=20.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 4) Unbalanced snow loads have been considered for this design.
- 5) Provide adequate drainage to prevent water ponding.
- 6) Gable requires continuous bottom chord bearing.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 4.
   This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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LOADING         (psf)           TCLL (roof)         25.0           Snow (Pf/Pg)         20.4/20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2018/TPI2014	CSI. TC 0.79 BC 0.42 WB 0.00 Matrix-R	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (l n/a n/a 0.00	(loc) - - 4	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 MT20HS Weight: 21 lb	<b>GRIP</b> 197/144 148/108 FT = 20%
LUMBER- TOP CHORD 2x4 SPF No.2		<b>BR</b> TO	ACING- P CHORD S	structural v	wood s	heathing	g directly ap	plied or 6-0-0 oc purlin	S,

BOT CHORD

TOP CHORD 2x4 SPF No.2

BOT CHORD 2x4 SPF No.2 WEBS 2x4 SPF No.2

REACTIONS. (size) 1=7-9-4, 4=7-9-4 Max Horz 1=106(LC 15) Max Uplift 1=-22(LC 16), 4=-29(LC 13)

Max Grav 1=376(LC 35), 4=337(LC 35)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

# NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) 0-7-9 to 3-7-9, Interior(1) 3-7-9 to 6-11-0, Exterior(2E) 6-11-0 to 7-8-0 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=20.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- 4) Unbalanced snow loads have been considered for this design.

5) Provide adequate drainage to prevent water ponding.

All plates are MT20 plates unless otherwise indicated.

7) Gable requires continuous bottom chord bearing.

- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 4. 10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 2-3.

Rigid ceiling directly applied or 10-0-0 oc bracing

👠 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design valid to use only design parameters and READ NOTES ON THIS AND INCLUDED WITH REPORT PAGE MIT 475 169 (2020 BEFORE USE). Design valid for use only with MITEK deconnectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent colleges with possible personal injury and property damage. For general quidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component 
 Satisfies
 Ansi/TPH Qu

 Safety Information
 available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





BCLL BCDL	10.0 0.0 10.0	Rep Stress Incr YES Code IRC2018/TPI2014
LUMBER- TOP CHORD	2x4 SPF No.2	

2x4 SPF No.2 BOT CHORD 2x4 SPF No.2 WEBS 2x4 SPF No.2 OTHERS 2x4 SPF No.2

#### REACTIONS. (size) 1=9-9-4, 4=9-9-4, 5=9-9-4

Max Horz 1=158(LC 13) Max Uplift 4=-23(LC 13), 5=-81(LC 16)

Max Grav 1=189(LC 2), 4=141(LC 20), 5=512(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 2-5=-386/243WEBS

### NOTES-

- 1) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) 0-7-9 to 3-7-9, Interior(1) 3-7-9 to 9-8-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL=1.15 Plate

YES

WB

Matrix-S

0.06

Horz(CT)

BRACING-

TOP CHORD

BOT CHORD

-0.00

4

except end verticals.

n/a

Rigid ceiling directly applied or 10-0-0 oc bracing.

n/a

Structural wood sheathing directly applied or 6-0-0 oc purlins,

- DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 5.
- 7) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



Weight: 30 lb

FT = 20%

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1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.

5) Gable requires continuous bottom chord bearing.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3, 4.

8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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2x4 💋

BOT CHORD

2x4 🛸

Rigid ceiling directly applied or 10-0-0 oc bracing.

3-6-13 0-0-8 3-6-5 Plate Offsets (X,Y)--[2:0-3-0,Edge] LOADING (psf) SPACING-DEFL. 2-0-0 CSI in (loc) l/defl L/d PLATES GRIP 25.0 TCLL (roof) Plate Grip DOL 1.15 тс 0.03 Vert(LL) 999 MT20 197/144 n/a n/a Snow (Pf/Pg) 15.4/20.0 Lumber DOL 1.15 BC 0.05 Vert(CT) n/a n/a 999 TCDL 10.0 Rep Stress Incr YES WB 0.00 Horz(CT) 0.00 3 n/a n/a BCLL 0.0 Code IRC2018/TPI2014 FT = 20% Matrix-P Weight: 7 lb BCDL 10.0 LUMBER-BRACING-TOP CHORD Structural wood sheathing directly applied or 3-6-13 oc purlins.

TOP CHORD 2x4 SPF No.2 2x4 SPF No.2 BOT CHORD

REACTIONS. 1=3-5-13, 3=3-5-13 (size) Max Horz 1=11(LC 15) Max Uplift 1=-8(LC 16), 3=-8(LC 16) Max Grav 1=104(LC 2), 3=104(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Enclosed; MWFRS (directional) and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) Gable requires continuous bottom chord bearing.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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