LLC

Residential Builder Resource, LLC
Custom Home Drafting & Design Service
Kansas City Metro
Rural Missouri & Eastern Kansas
Ph. (816) 472-5072

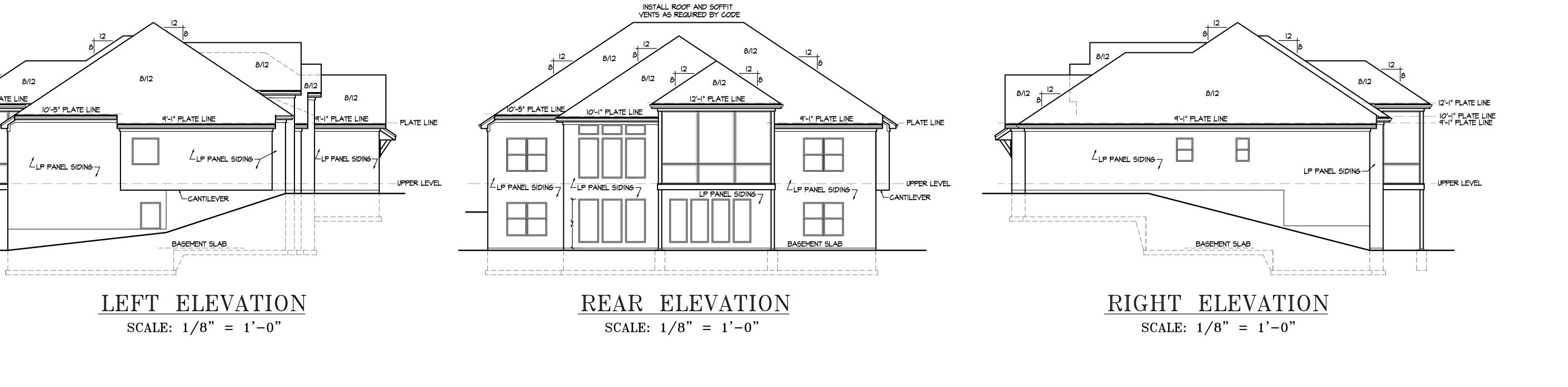
APEX ENGINEERS, INC. 1625 LOCUST ST KANSAS CITY, MO 64108 816.421.3222 STRUCTURAL DESIGN REVIEW KANSAS ENGINEERING LICENSE: E-992 MISSOURI ENGINEERING LICENSE:

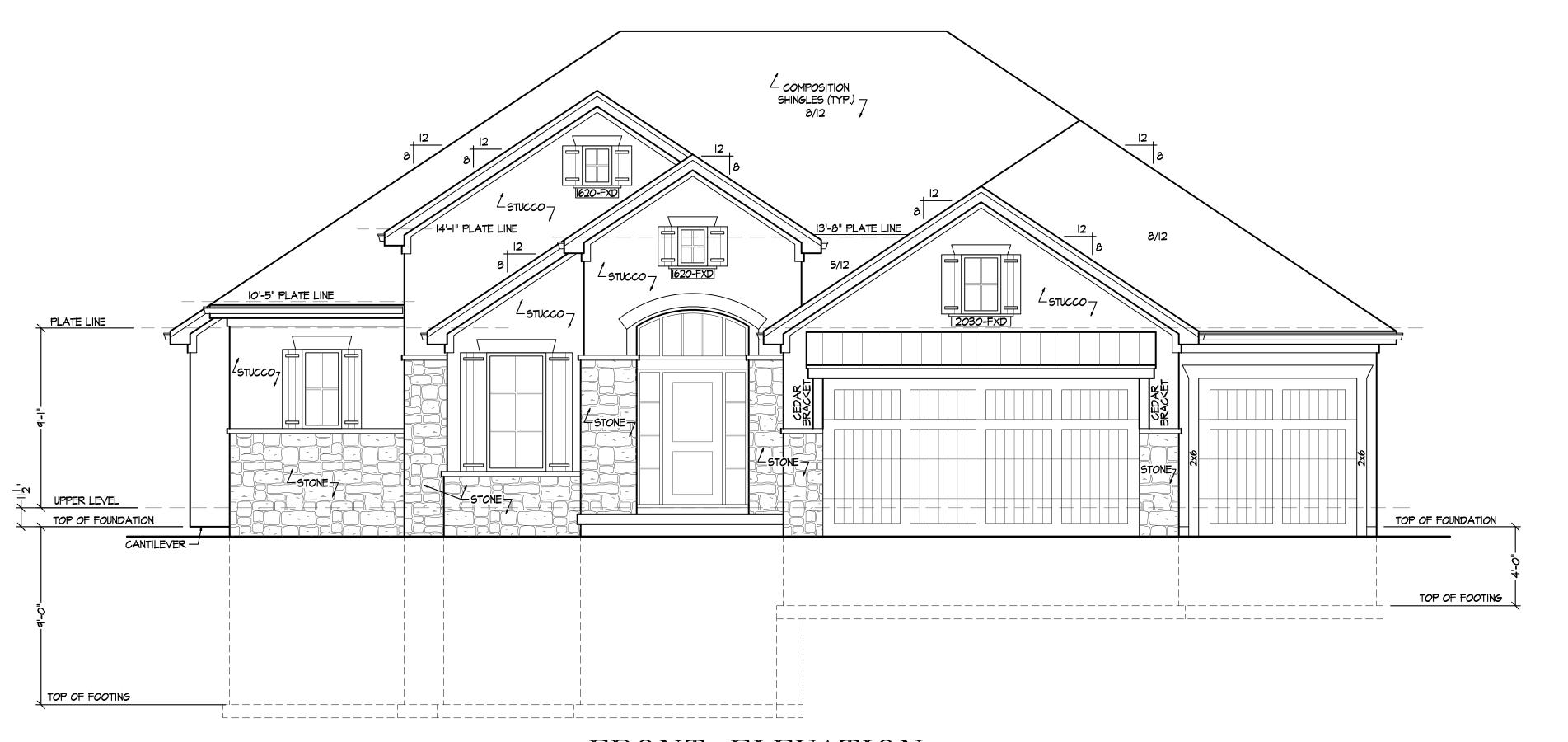
<u>DISCLAIMER</u> ACTUAL PLANS AND ELEVATIONS MAY VARY FROM ARCHITECTURAL DRAWINGS. DUE TO TERRAIN/BACKFILL PROCESS.

FRONT ELEVATIONS ARE ARCHITECTURAL DRAWINGS AND MAY VARY DUE TO MATERIAL AVAILABILITY.

DRAWN BY: RBR CHECKED BY: CA DATE: 12-18-20

PROJ. #20-133





FRONT ELEVATION
SCALE: 1/4" = 1'-0"

UPPER LEVEL PLAN
SCALE: 1/4" = 1'-0"

BRACED WALL METHODOLOGY
CONTINUOUS EXTERIOR SHEATHING PER WSP METHOD (BELOW)
UNLESS OTHERWISE NOTED ON THE PLAN

XXXX <u>EXTERIOR BRACED WALLS:</u>

MSP METHOD: WOOD STRUCTURAL PANEL SHEATHING WITH A THICKNESS NOT LESS THAN 3/8" WITH MINIMUM SPAN RATING OF 24/0 FOR 16" OC STUD SPACING WITH 6d COMMON NAILS AT 6" OC EDGES AND 12" OC FIELD OR SHEATHING THICKNESS NOT LESS THAN 3/6" WITH MINIMUM SPAN RATING OF 24/6 FOR 24" OC SPACING WITH 8d COMMON NAILS AT 6" OC EDGES AND 12" OC IN FIELD.

(NOTE: FRAMING MEMBERS 16" OC MAXJUNBLOCKED, AND WITH SHEATHING APPLIED DIRECTLY TO FRAMING MEMBERS)

/// INTERIOR BRACED WALLS (REF 2-S4.0):

GB METHOD: ½" MIN. GYPSUM BOARD OVER STUDS SPACED 24" MAX. FASTENED WITH No 6 - 1½" TYPE 'W' OR 'S' DRYWALL SCREWS AT 7" OC EDGES AND FIELD

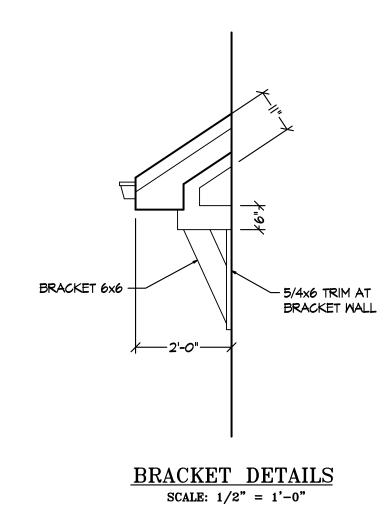
(MIN. 4'-0" SECTION FOR BOTH SIDES.)
OR

LIB METHOD: IX4 WOOD FASTENED WITH (3) 8d COMMON NAILS OR SIMPSON / USP 16 GA. TYPE WB (OR EQUAL) STL. X-BRACE(S) AT 45° TO 60° ANGLES, MAXIMUM 16" O.C. STUD FASTENED PER MANUFACTURER'S SPECIFICATIONS.

XXXX = EXTERIOR BRACED WALLS

//// = INTERIOR BRACED WALS (REF 2/54.0)

EC = END CONDTION (REF 2/54.1 FOR CONTINUOUS SHEATHED BRACED WALL END CONDTIONS



MAIN FLOOR - 2,084 SQ. FT.

LOWER LEVEL - 1,676 SQ. FT.

TOTAL 3,760 SQ. FT.

UNFINISHED BASEMENT 287 SQ. FT.

COVERED DECK 202 SQ. FT.

GARAGE 653 SQ. FT.



ALL WINDOWS SIZES ARE EXPRESSED
IN FEET AND INCHES TO THE UNIT
SIZE.

NOTE:
PLANS DESIGNED PER IRC AS
ADOPTED BY GOVERNING JURISDICTION

DRAWN BY: RBR

CHECKED BY: CA

DATE: 12-18-20

Builder Resource,

PROJ. #20-133

LOWER LEVEL PLAN

SCALE: 1/4" = 1'-0"

BRACED WALL METHODOLOGY
CONTINUOUS EXTERIOR SHEATHING PER MSP METHOD (BELOW)
UNLESS OTHERWISE NOTED ON THE PLAN

XXXX <u>EXTERIOR BRACED WALLS:</u>

WSP METHOD: WOOD STRUCTURAL PANEL SHEATHING WITH A THICKNESS NOT LESS THAN %" WITH MINIMUM SPAN RATING OF 24/O FOR 16" OC STUD SPACING WITH 6d COMMON NAILS AT 6" OC EDGES AND 12" OC FIELD OR SHEATHING THICKNESS NOT LESS THAN %" WITH MINIMUM SPAN RATING OF 24% FOR 24" OC SPACING WITH 8d COMMON NAILS AT 6" OC EDGES AND 12" OC IN FIELD.

(NOTE: FRAMING MEMBERS 16" OC MAXINBLOCKED, AND WITH SHEATHING APPLIED DIRECTLY TO FRAMING MEMBERS)

/// INTERIOR BRACED WALLS (REF 2-54.0):

GB METHOD: ½" MIN. GYPSUM BOARD OVER STUDS SPACED 24" MAX. FASTENED WITH No 6 - 1½" TYPE 'W' OR 'S' DRYWALL SCREWS AT 7" OC EDGES AND FIELD (MIN. 4'-0" SECTION FOR BOTH SIDES.)

LIB METHOD: IX4 WOOD FASTENED WITH (3) 8d COMMON NAILS OR SIMPSON / USP 16 GA. TYPE
WB (OR EQUAL) STL. X-BRACE(S) AT 45° TO 60° ANGLES, MAXIMUM 16" O.C. STUD
FASTENED PER MANUFACTURER'S SPECIFICATIONS.

XXXX = EXTERIOR BRACED WALLS

//// = INTERIOR BRACED WALS (REF 2/54.0)

EC = END CONDTION (REF 2/54.1 FOR CONTINUOUS SHEATHED BRACED WALL END CONDTIONS

COLUMN & PIER PAD SCHEDULE (REF. 5/52.0)								
COLUMN MARK	PAD SIZE	REINFORCEMENT	COLUMN SIZE	COLUMN TYPE				
A	30" x 30" x 12"	(4) #4 BAR E.M.	3" NOMINAL					
B	36" x 36" x 12"	(4) #4 BAR E.M.	3" NOMINAL	. 5				
<u> </u>	42" × 42" × 12"	(5) #4 BAR E.M.	3" NOMINAL	SCHEDUE STEEL PIPE 40 (FY = 36 KSI MIN.)				
À	48" × 48" × 12"	(6) #4 BAR E.M.	3" NOMINAL	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1				
<u></u>	54" × 54" × 16"	(8) #4 BAR E.M.	3½" NOMINAL	\$ \$ \frac{1}{2} \f				
A	60" × 60" × 16"	(IO) #4 BAR E.W.	3½" NOMINAL					

 COLUMN & PAD SIZES SHOWN ARE FOR MAXIMUM COLUMN HEIGHT OF IO'-O", REQUIRES SEPARATE ENGR'D DESIGN IF GREATER THAN IO'-O" TALL.
 COLUMN & PIER PAD SIZES SHOWN ARE BASED ON AN ASSUMED MINIMUM ALLOWABLE SOIL BEARING CAPACITY OF 2,000 PSF.

COLUMN & PIER SCHEDULE									
MARK	PIER DIA								
B	6x6	12"							
A	6×6	16"							
	6x6	18"							
A	6x6	24"							
\triangle	6x6	28"							

I. ALL PIERS TO BEAR ON ORIGINAL,
UNDISTURBED SOIL OF 2,000 PSF BEARING
CAPACITY OR FILL COMPACTED AND TESTED
TO CONFORM TO THE RECOMMENDATIONS OF A
GEOTECHNICAL ENGINEER.

2. PIERS SHALL EXTEND BELOW THE FROST LINE: MIN. DEPTH OF 36" BELOW GRADE.
3. POST SHALL BE TREATED OR CEDAR WITH

SIMPSON ABU66 POST BASE

DETAIL REFERENCES

1 TYPICAL FOUNDATION WALL DETAIL

2 TYPICAL "UNRESTRAINED" 52.0 FOUNDATION WALL DETAIL

(3) TYPICAL DEAD MAN DETAIL

4 52.0 FOUNDATION WALL JUMP DETAIL

5 52.0 COLUMN PAD DETAIL

TYPICAL STRUCTURAL GARAGE SLAB 52.1 PLAN

2 STRUCTURAL GARAGE SLAB 52.I PIER PAD DETAIL

3 STRUCTURAL GARAGE SLAB / S2.I WALL SECTION

6 TYPICAL OVERDIG DETAIL AT BASEMENT SLAB

ALTERNATE BRACED WALL PANEL DETAIL

APA NARROW WALL BRACING
S4.0 METHOD WITHOUT HOLD-DOWNS

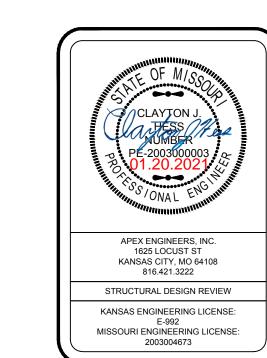
COLUMN AND PIER PAD SCHEDULE (SHEET S2.0)

EXPANSIVE SOILS DISCLAIMER

These plans have been prepared based on a presumptive allowable bearing capacity as allowed by IRC code and the local enforcing jurisdiction.

Apex Engineers, Inc. (APEX) recommends that all footing excavations be evaluated by a licensed geotechnical engineer prior to the placement of any foundation elements. Geotechnical investigation and/or testing is NOT a service provided or offered by APEX.

APEX has not been retained to determine the expansive soil characteristics of the subgrade soil and therefore cannot be held responsible for the volumetric changes of the soil (including below the basement slab). By use of these plans without an accompanying geotechnical engineering report, APEX shall not be held liable for any future movement and/or differential movement of the proposed structure and the possible damage that may be caused as a result of such movement.



ALL MINDOWS SIZES ARE EXPRESSED IN FEET AND INCHES TO THE UNIT

NOTE:
PLANS DESIGNED PER IRC AS
ADOPTED BY GOVERNING JURISDICTION

CLAYTON J.

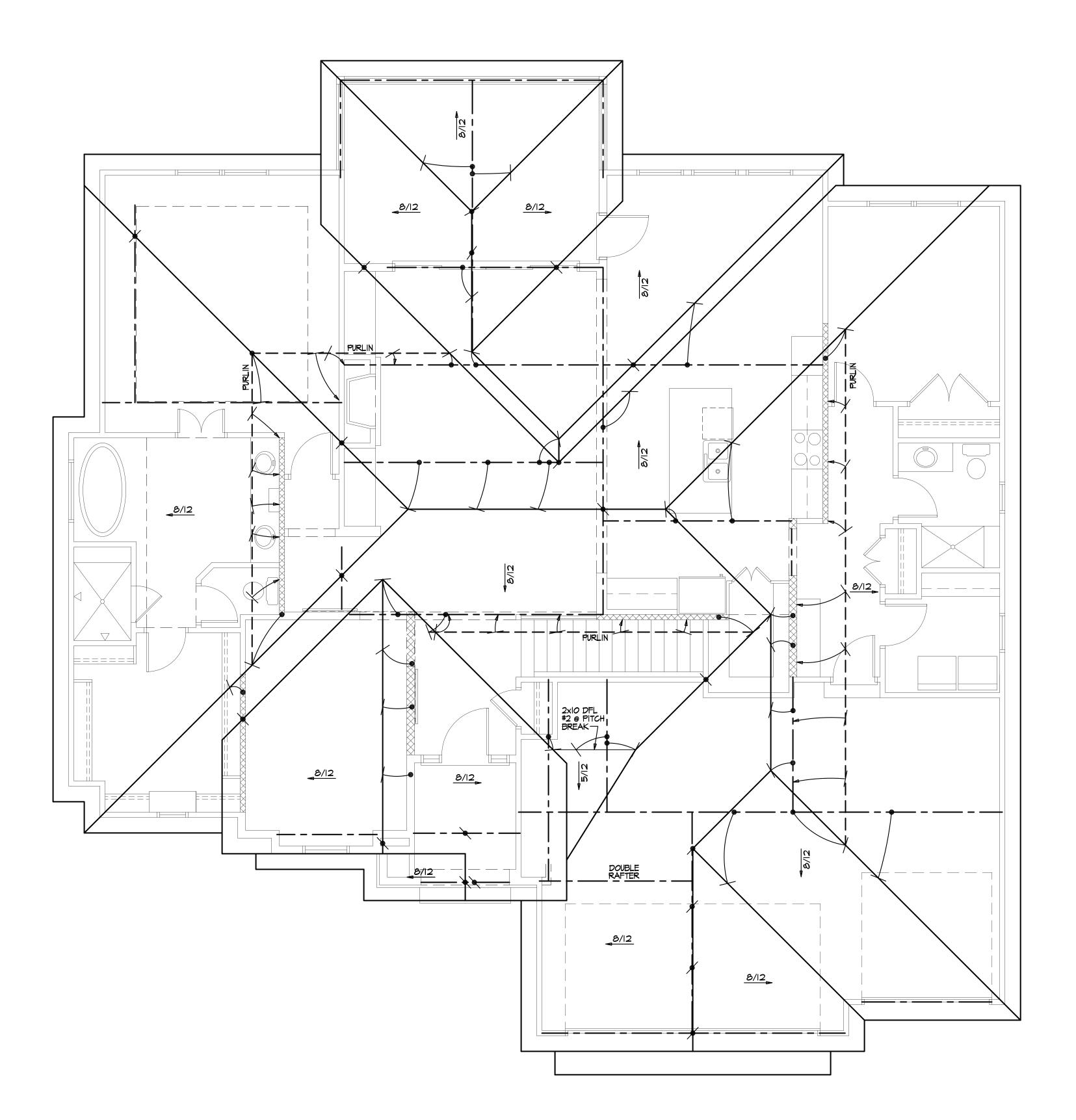
SOLATION J.

SOLATI

A3
PROJ. #20-133

CHECKED BY: CA

DATE: 12-18-20



ROOF PLAN SCALE: 1/4" = 1'-0"

ROOF FRAMING NOTES

ROOF DESIGNED FOR LIGHT ROOF COVERING 30psf TOTAL LOAD [IOpsf DL, 20psf LL (SL)]

ROOF SYSTEM IS DESIGNED TO MEET REQUIREMENTS OF

*RAFTERS (HEM-FIR, DOUG-FIR, OR EQUAL): SEE SPAN CHARTS BELOW

ODE MINIMUM		
RAFTERS	SPACING	MAX HORIZONTAL CLEARSPAN
#2-2×6	AT 24" OC	11'-7"
#2-2×6	AT 16" OC	14'-2"
#2-2×8	AT 24" OC	14'-8"
#2-2×8	AT 16" OC	17'-11"
#2-2xl0	AT 24" OC	17'-10"
#2-2xIO	AT 16" OC	2 '- "

NOTE: CODE MINIMUM ALLOWS FOR A RAFTER DEFLECTION OF L/180 TOTAL LOAD

HIGHER PERFORMANCE

SPACING	MAX HORIZONTAL CLEARSPAN
AT 24" OC	8'-6"
AT 16" OC	q'-q"
AT 24" OC	11'-3"
AT 16" OC	12'-9"
AT 24" OC	14'-3"
AT 16" OC	16'-3"
	AT 16" OC AT 24" OC AT 16" OC AT 24" OC

APEX ENGINEERS, INC. RECOMMENDED DEFLECTION = L/360 LIVE LOAD, L/240 TOTAL LOAD

*RIDGE BOARDS ARE (UNLESS OTHERWISE NOTED) #2-2x10 UP TO 9:12 PITCH

#2-2x12 OVER 9:12 PITCH

*ALL HIPS AND VALLEYS ARE (UNLESS OTHERWISE NOTED)

#2-2x10 UP TO 9:12 PITCH

#2-2x12 OVER 9:12 PITCH

NOTES ABOVE)

*PURLINS ARE 2x6 MIN - PURLIN STRUTS ARE AT 4'-0" OC

- PURLIN STRUTS SHALL BE INSTALLED AT NOT LESS THAN A 45 DEGREE ANGLE WITH THE HORIZONTAL

- ALL PURLIN STRUTS SHALL HAVE A MAX UNBRACED LENGTH OF 8'-0"

- PURLIN STRUTS SHALL BE CONSTRUCTED IN A "T" CONFIGURATION AND PER THE FOLLOWING CHART:

CONTIDURATION AND PER THE FOLLOWING CHART:							
PURLIN STRUT	MAX PURLIN STRUT LENGTH						
(2)2×4	8'-0"						
(1)2x4 AND (1)2x6	12'-0"						
(1)2x6 AND (1)2x8	20'-0"						
(2)2x6 AND (1)2x8	30'-O"						
CONSULT ARCH ENGR	>30'-0"						

*EACH END OF STRUT SHALL BE FASTENED WITH MIN (3)8d OR (2)16d NAILS

*RIDGE BRACERS ARE SAME AS PURLIN BRACES-SPACING, SIZE, CONFIGURATION, AND INSTALLATION (SEE PURLIN

BRACE NOTES ABOVE) *HIP AND VALLEY BRACES ARE THE SAME AS PURLINS SIZE, CONFIGURATION, AND INSTALLATION (SEE PURLIN BRACE

= ROOF BRACE/STRUT (PER CHART) -SLASH IS TOP END OF BRACE

= PURLIN STRUTS AT 48" OC (PER CHART) U.N.O. -SLASH IS TOP END OF BRACE -ARROW IS BEARING LOCATION

-CIRCLE IS BOTTOM END OF BRACE

DENOTES BEARING WALL - - - DENOTES PURLIN ----- DENOTES BEARING STRUCTURE

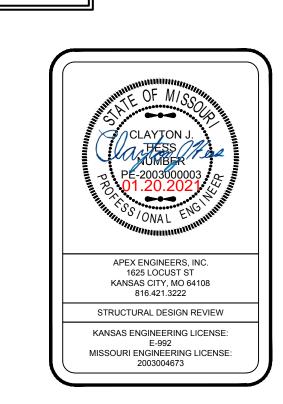
THIS IS AN ENGINEERED ROOF STRUCTURE DESIGNED FOR

ALL HIPS ARE DESIGNED TO BE CONTROLLED BY BENDING.

SHEAR AT BEARING WITH MIN 51/2" **DEPTH DOES NOT CONTROL DESIGN. FOR VALLEYS REF 4/S3.2**

COMPLIANCE WITH IRC 802.3, BUILD

AS SHOWN WITH NO DEVIATIONS.



NOTE:
PLANS DESIGNED PER IRC AS ADOPTED BY GOVERNING JURISDICTION

DRAWN BY: RBR CHECKED BY: CA

DATE: 12-18-20

PROJ. #20-133

01/20/2021

SHEATHING AND FRAMING FASTENING SCHEDULE BUILDING COMPONENT **FASTENING MATERIAL** 16 GA x 1-3/4" STAPLES AT 3" 7/16" PLYWOOD OC EDGES AND 6" OC IN FIELD ROOF SHEATHING¹ 1x4 #3 FURRING 1/2" CROWN STAPLES 8d COMMON NAILS AT 6" OC EDGES AND 12" OC IN THE FIELD 3/4" T&G YELLOW PINE PLYWOOD 14 GA x 2" STAPLES AT 4" OC APPLIED PERPENDICULAR TO FLOOR SHEATHING¹ EDGES AND 8" OC IN THE FIELD JOISTS AND ENDS STAGGERED 12.5 GA x 1-1/2" RING OR SCREW SHANK NAILS AT 6" OC EDGES AND 8" OC IN THE FIELD 7" OC NAILED / 12" OC SCREWED WITH 13 GA, 1-3/8" LONG, 19/64" HEAD; 0.098 DIA. 1-1/4" LONG. ANG.-RINGED: 5d CEILING COVERING¹ 1/2" GYPSUM SHEATHING COOLER NAIL, 0.086 DIA, 1-5/8" LONG, 15/64" HEAD; OR GYP BD NAIL, 0.086 DIA, 1-5/8" LONG, 9/32" HEAD 6d COMMON NAILS; 1-5/8" GALVANIZED STAPLES; 1-1/4" INTERIOR WALL 1/2" GYPSUM SHEATHING SCREWS, TYPE W OR S- AT 4" OC COVERING¹ EDGES AND 8" OC IN THE FIELD 8d COMMON NAILS AT 6" OC EDGES **EXTERIOR WAI** MIN 3/8" APA RATED SHEATHING AND 12" OC IN THE FIELD SHEATHING *TOE NAIL RIM JOIST TO SILL OR TOP 8d COMMON AT 6" OC; 3"x0.131" AT 6" OC; 3"x0.131" *SUPPORTING 2 FLOORS, ROOF, AND CEILING OR LESS. *TOE NAIL STUD TO TOP AND SOLE PLATE: (4) 8d COMMON; (4) 3"x0.131" *HEIGHT: 10'-0" OR LESS *END NAIL TOP AND SOLE PLATE TO STUD: (2) 16d COMMON; (3) 3"x0.131 16d AT 24" OC; 3"x0.131" AT 16" SIZE: NOM 2x4 (NOM 2x6 WHEN *FACE NAIL BUILT-UP CORNER STUDS SUPPORTING 2 FLOORS, CEILING, AT BRACED WALL PANELS): FACE NAIL JACK STUDS/TRIMMERS AND ROOF) 10d NAILS AT 6" OC CONVENTIONAL WOOD *SPECIES: DOUG-FIR, HEM-FIR, 16d COMMON AT 16" OC; 3"x0.131" AT 12" OC; *FACE NAIL DBL TOP PLATE: FRAMED WALLS 3"x0.128" AT 12" OC SOUTH PINE, SPRUCE-PINE-FIR *DBI_TOP PLATES WITH MIN 48" OFFSET *MAXIMUM SPACING 16" OC DF EACH. FACE NAIL LAPPED AREA WITH: (8) 16d COMMON; (12) 3"x0.131"; (12) 3"x0.128" *FACE NAIL DBI TOP PLATES AT LAPPED *STUDS 10' LENGTH OR LESS ORNERS AND INTERSECTIONS WITH: SHALL BE #3 STANDARD, OR STUD *FACE NAIL SOLE PLATE TO FRAMING 16d COMMON AT 16" OC; 3"x0.131" AT 12" OC FOENAIL BRIDGING TO JOIST, EACH END: (2) 8d COMMON; (2) 3"x0.131"; (3) 3"x0.128" *STUDS OVER 10' LENGTH SHALL *FACE NAIL LEDGER STRIPS SUPPORTING (3) 16d COMMON: (4) 3"x0 131": (4) 3"x0 128" BE MIN #2 GRADE OISTS OR RAFTERS WITH: *TOE NAIL HEADERS TO WALL STUDS WITH (4) 8d CONVENTIONAL WOOD NAILS AT EACH END. PER PLAN *FACE NAIL DOUBLE PIECE HEADERS WITH 16d **HEADER FRAMING** NAILS AT 16" CENTERS ALONG EACH EDGE. RAFTER TIES² MIN 2x4 MEMBERS AT EACH RAFTER REF TABLE R802.5.2 FACENAIL TO RAFTERS IN UPPER 1/3 OF **COLLAR TIES** MIN 1x4 MEMBERS AT 48" OC ATTIC SPACE WITH (3) 10d NAILS AT EACH

ATTIC SPACE WITH (3) TOU NAILS AT LACIT								
1. NOTE: ALL SHEATHING MATERIALS TO BE APPLIED PERPENDICULAR TO JOISTS AND ENDS STAGGERED.								
2. RAFTER TIES SHALL NOT BE REQUIRED WHEN A STRUCTURAL RIDGE HAS BEEN PROVIDED AND ADEQUATELY								
DESIGNED (AS IN A FULLY VAULTED ROOM). SUCH SHALL BE NOTED AS "STRUCTURAL" ON THE PLAN.								
BUILDING COMPONENT	FASTEN TO	FASTEN WITH						
RAFTERS	TO RIDGE/VALLEY/HIP RAFTERS	TOENAIL WITH (4) 16d ENDNAIL WITH (3) 16d						
TVALLENO	TO PLATE	TOENAIL WITH (2) 16d						
CEILING JOISTS	TO TOP PLATE	TOENAIL WITH (3) 8d AT EACH END						
CEILING JOISTS	WHERE CEILING JOISTS RUN PARALLEL TO RAFTERS FACENAIL TO RAFTERS WITH (3) 10d MIN							
FLOOR JOISTS	TO SILL OR GIRDER	TOENAL WITH: (3) 8d COMMON; (3) 3"x0.131"; (4) 3"x0.128"						
12001101010	TO RIM JOIST	ENDNAIL WITH: (3) 16d COMMON; (4) 3"x0.131"; (4) 3"x0.128"						
BRACED WALL PANELS PERP TO FRAMING	TO FRAMING MEMBER	SOLE PL, 16" OC WITH: (3) 16d COMMON; (4) 3"x0.131" TOP PL, 6" OC WITH: 8d COMMON; 3"x0.131"						
MEMBERS ABOVE/BELOW: PARALLEL TO FRAMING MEMBERS ABOVE/BELOW:	TO FRAMING AND BLOCKING AT 16" OC	SOLE PL, 16" OC WITH: (3) 16d COMMON; (4) 3"x0.131" AND AT EACH BLOCK: (3) 16d COMMON; (4) 3"x0.131" TOP PL, 6" OC WITH: 8d COMMON; 3"x0.131" AND AT EACH BLOCK: (3) 8d COMMON; 3"x0.131"						
NOTE: MEMBER THICKNESS AND FASTENING LISTED IN THIS SCHEDULE ARE MINIMUM IRC REQUIREMENTS. SPECIFIC PROJECT								

REQUIREMENTS NOTED WITHIN THE STRUCTURAL OR ARCHITECTURAL DRAWINGS, IF REQUIRED BY APEX ENGINEERS DESIGN

NEEDING TO BE MORE STRINGENT, SHALL BE FOLLOWED.

ENERGY REQUIREMENTS

1. LIGHTING FIXTURES PENETRATING THE THERMAL ENVELOPE SHALL BE IC-RATED, LEAKAGE RATED, AND SEALED TO THE GYPSUM WALLBOARD AS REQUIRED PER N1102.4.5. 2. PROGRAMMABLE THERMOSTATS SHALL BE INSTALLED AS REQUIRED PER

3. AIR HANDLERS SHALL BE RATED FOR MAXIMUM 2% AIR LEAKAGE RATE PER

N1103.3.2.1. 4. BUILDING FRAMING CAVITIES SHALL NOT BE USED AS DUCTS OR PLENUMBS PER N1103.3.5

5. HOT WATER PIPES SHALL BE INSULATED AS REQUIRED PER N1103.4. 6. ALL EXHAUST FANS SHALL TERMINATE TO THE BUILDING EXTERIOR AS REQUIRED PER M1501 1

7. MAKEUP AIR SYSTEMS SHALL BE INSTALLED FOR KITCHEN EXHAUST HOODS THAT EXCEED 400 CFM AS REQUIRED PER M1503.6. 8. AN AIR HANDLING SYSTEM SHALL NOT SERVE BOTH THE LIVING SPACE AND THE GARAGE PER M1601.6.

ENERGY CONSERVATION

THE ENERGY EFFICIENCY OF THE DWELLING SHALL COMPLY WITH THE FOLLOWING TABLE(S) (WHERE THERE ARE DISCREPANCIES BETWEEN THIS TABLE AND THE PLANS, THE MOST RESTRICTIVE SHALL APPLY). IF TABLE 1 IS NOT COMPLETED AND ACCOMPANIED BY RESCHECK CALCULATIONS, THEN TABLE 2 SHALL BE APPLIED. TABLE 1 - ResCheck COMPLIANCE SOFTWARE (FILL IN APPLICABLE

VALUES FROM ResCheck CALCS. **MIN VALUE BUILDING ELEMENT** WALLS - FRAMED WALLS - BASEMENT FLOORS - UNCONDITIONED SPACE FLOORS - OVER OUTSIDE AIR FLOORS - CRAWL SPACE **SLAB - PERIMETER CEILING - FLAT CEILING - CATHEDRAL DOORS - GLASS** DOORS - SOLID WINDOWS - OPERABLE WINDOWS - FIXED WINDOWS - OTHER FURNACE AFUE-AIR CONDITIONER SFFR-NOTE: FOR USE OF TABLE 1 A ResCheck COMPLIANCE FORM MUST BE

SUBMITTED WITH PLANS. TABLE 2 -PRESCRIPTIVE ENVELOPE (MIN PRESCRIPTIVE APPROACH

ACCEPTABLE FOR ANY DWELLING.)

BIIII DING ELEMENT

BUILDING ELEWENT	IVIIIN VALUE
CEILING - FLAT	R-49
CEILING - CATHEDRAL**	R-30
CEILING - CATHEDRAL	R-38
FLOORS - UNCONDITIONED SPACED	R-19
FLOORS - OVER OUTSIDE AIR	R-30
WALLS - BASEMENT	R-10 (CONT) OR R-13 (CAVITY)
CONCRETE SLAB ON GRADE	R-10 (FOR 2FT)
SKYLIGHTS	U=0.55
WALLS - EXTERIOR (2x4)	R-13 (CAVITY) + R-5 (CONT)
WALLS - EXTERIOR (2x6)	R-20
WALLS - CRAWL SPACE	R-19
GLAZING*	U<=0.32
GLAZING*	SHGF<=0.40
NOTE:	
TABLE 2 DED IDC TABLE N1102 1 2	

MIN VALUE

TABLE 2 PER IRC TABLE N1102.1.2 *DEFAULT U-FACTOR FOR DOUBLE PANE, ARGON FILLED LOW-E TREATMENT IS U=0.35 **LIMITED TO AREAS LESS THAN 500 SQ-FT OR 20% OF CEILING AREA.

DEFERRED SUBMITTALS 1. THE ARCHITECT OR ENGINEER OF RECORD SHALL LIST THE DEFERRED SUBMITTALS ON THE PLANS FOR REVIEW BY THE BUILDING OFFICIAL DOCUMENTS FOR DEFERRED SUBMITTAL ITEMS SHALL BE SUBMITTED TO THE ARCHITECT OR ENGINEER OF RECORD WHO SHALL REVIEW THEM AND FORWARD THEM TO THE BUILDING OFFICIAL WITH A NOTATION INDICATING THAT THE DEFERRED SUBMITTAL DOCUMENTS HAVE BEEN REVIEWED AND FOUND TO BE IN THE GENERAL CONFORMANCE TO THE DESIGN OF THE BUILDING. THE DEFERRED SUBMITTAL ITEMS SHALL NOT BE INSTALLED UNTIL THE DEFERRED SUBMITTAL DOCUMENTS HAVE BEEN APPROVED BY THE BUILDING OFFICIAL. DEFERRED SUBMITTALS ARE DEFINED AS THOSE PORTIONS OF THE DESIGN THAT ARE NOT SUBMITTED AT THE TIME OF THE APPLICATION AND THAT ARE TO BE SUBMITTED TO THE BUILDING OFFICIAL WITH A SPECIFIED PERIOD. DEFERRAL OF ANY SUBMITTAL ITEMS SHALL HAVE THE PRIOR

APPROVAL OF THE BUILDING OFFICIAL. 2. DEFERRED SUBMITTAL ITEMS (WHEN APPLICABLE):

- A. TRUSSES
- B. I-JOISTS C. GUARDRAILS AND HANDRAILS
- D. STEEL FABRICATED STAIRS
- E. PRE-MANUFACTURED CANOPIES AND AWNINGS F. PRECAST HOLLOW CORE SLABS
- G. GROUND IMPROVEMENT AND/OR STRUCTURAL FOUNDATION SOLUTIONS (SUCH AS DRILLED PIERS)

CONCRETE

CONCRETE SHALL BE AIR ENTRAINED WITH A MINIMUM COMPRESSIVE STRENGTH OF 28 DAYS OF 2,500 PSI FOR BASEMENT AND INTERIOR FLOOR SLABS, 3,000 PSI FOR BASEMENT AND FOUNDATION WALLS, AND 3,500 FOR PORCHES, CARPORTS, AND GARAGE FLOOR SLABS.

GLAZING

GLAZING IN HAZARDOUS LOCATIONS AS IDENTIFIED IN IRC SECTION R308.4 SHALL BE OF APPROVED SAFETY GLAZING MATERIALS: GLASS IN STORM DOORS: INDIVIDUAL FIXED OR OPENABLE PANELS ADJACENT TO A DOOR WHERE THE NEAREST VERTICAL EDGE IS WITHIN A 24" ARCH OF THE DOOR IN A CLOSED POSITION AND WHOSE BOTTOM EDGE IS WITHIN 60" OF THE FLOOR: WALLS ENCLOSING STAIRWAYS AND LANDINGS WHERE THE GLAZING IS WITHIN 60" OF THE TOP OR BOTTOM OF THE STAIR; ENCLOSURES FOR SPAS, TUBS, SHOWERS, AND WHIRLPOOLS; GLAZING IN FIXED OR OPENABLE PANELS EXCEEDING 9 SQUARE FEET AND WHOSE BOTTOM EDGE IS LESS THAN 18" ABOVE THE FLOOR OR WALKING SURFACE WITHIN 36".

EMERGENCY EGRESS AND RESCUE

1. PROVIDE ONE WINDOW FROM EACH BEDROOM THAT HAS A MINIMUM OPENABLE AREA OF 5.7 SQUARE FEET WITH A MINIMUM OPENABLE HEIGHT OF 24 INCHES AND WIDTH OF 20 INCHES.

2. BASEMENT EGRESS TO MEET THE REQUIREMENTS OF IRC SECTION 310. 3. SMOKE ALARMS SHALL BE INSTALLED AS REQUIRED PER IRC 2018 SECTION R314. 4. PROVIDE SMOKE ALARMS IN EACH SLEEPING ROOM, OUTSIDE OF EACH SLEEPING AREA. ON EACH FLOOR INCLUDING BASEMENTS AND HABITABLE ATTICS, AND NOT LESS THAN 3'-0" HORIZONTALLY FROM DOOR OR OPENING OF A BATHROOM THAT CONTAINS A BATHTUB OR SHOWER. ALARMS SHALL BE INTERCONNECTED IN SUCH A MANNER THAT THE ACTUATION OF ONE ALARM WILL ACTIVATE ALL OF THE ALARMS IN THE DWELLING 5. CARBON MONOXIDE ALARMS SHALL BE INSTALLED AS REQUIRED PER IRC 2018

6. CARBON MONOXIDE ALARMS SHALL BE INSTALLED OUTSIDE OF EACH SEPARATE SLEEPING AREA. WHERE A FUEL-BURNING APPLIANCE IS LOCATED WITHIN A BEDROOM OR ITS ATTACHED BATHROOM, A CARBON MONOXIDE ALARM SHALL BE INSTALLED WITHIN THE BEDROOM.

SECTION R315.

FRAMING GENERAL

1. ALL LUMBER SIZES ARE FOR DOUGLAS FIR-LARCH UNLESS NOTED OTHERWISE. 2. ALL HEADERS TO BE MIN (2) #2-2x10 UNLESS NOTED OTHERWISE. 3. BLOCK CANTILEVERS, DOORJAMBS, AND OVER BEAMS.

4. ALL HEADERS TO BEAR ON A MINIMUM OF (2) 2x4 STUD POSTS UNLESS NOTED 5. INTERIOR NON-BEARING WALLS, OTHER THAN THOSE RESTING DIRECTLY ON

THE FOOTING SHALL BE ISOLATED FROM THE FLOOR FRAMING ABOVE. 6. WHERE JOISTS RUN PARALLEL TO FOUNDATION WALLS, SOLID BLOCKING FOR A MINIMUM OF (2) JOIST SPACES BE PROVIDED TO A MAXIMUM OF 2'-0" CENTERS TO TRANSFER LATERAL LOADS ON THE WALL TO THE FLOOR DIAPHRAGM. THE BLOCKING SHALL BE SECURELY NAILED TO THE JOISTS AND FLOORING. NAIL JOISTS AND BLOCKING TO SILL PLATE WITH (3) 10d NAILS (IRC SECTION R602.3.(1) 7. IF DUCTS ARE INSTALLED IN THE FIRST JOIST SPACE(S), NAIL 2x4s FLAT AT 2'-Ö" CENTERS WITHIN THE JOIST SPACE(S) AND THEN PROVIDE SOLID BLOCKING. INSTALLED UPRIGHT, IN THE NEXT TWO JOIST SPACES. SECURE THE 2x4s TO THE SILL PLATE WITH (4) 10d NAILS.

8. ALL SILLS AND SLEEPERS SUPPORTED ON CONCRETE OR MASONRY AND FURRING ATTACHED TO CONCRETE OR MASONRY SHALL BE OF DECAY RESISTANT MATERIALS

9. JOISTS UNDER BEARING PARTITIONS SHALL BE DOUBLED AND COMPLY WITH IRC SECTION R502.4.

10. JOISTS FRAMING FROM OPPOSITE SIDES OVER BEARING SUPPORTS SHALL LAP A MINIMUM 3" AND SHALL BE NAILED TOGETHER WITH A MINIMUM 10d FACE NAILS. 11. JOISTS FRAMING INTO A WOOD GIRDER OR BEAM SHALL BE SUPPORTED BY APPROVED FRAMING ANCHORS OR MINIMUM 2"x2" LEDGER STRIPS. 12. FRAMING OF OPENINGS - HEADERS AND TRIMMERS SHALL BE OF SUFFICIENT CROSS SECTION TO SUPPORT THE FLOOR FRAMING. TRIMMER JOISTS SHALL BE DOUBLED WHEN THE HEADER IS SUPPORTED MORE THAN 3'-0" FROM THE TRIMMER JOIST BEARING. WHEN THE HEADER SPAN EXCEEDS 4'-0", THE HEADER AND TRIMMER SHALL BE DOUBLED.

13. JOISTS AT SUPPORTS SHALL BE SUPPORTED LATERALLY AT THE ENDS BY FULL-DEPTH SOLID BLOCKING NOT LESS THAN 2" NOMINAL THICKNESS OR BY ATTACHMENT TO A HEADER, BAND OR RIM JOIST OR TO AN ADJOINING STUD OR OTHERWISE PROVIDED WITH LATERAL SUPPORT TO PREVENT ROTATION. 14. WATER-RESISTIVE BARRIER SHALL BE PROVIDED OVER ALL EXTERIOR WALLS. ONE LAYER OF No 15 ASPHALT FELT OR ANY OTHER BARRIER THAT MEETS ASTM D226 TYPE 1 FELT. (R703.2)

15. WHERE CEILING JOISTS ARE NOT INSTALLED CONNECTED TO THE RAFTERS AT THE TOP PLATE AND/OR WHERE CEILING JOISTS ARE NOT INSTALLED PARALLEL TO THE RAFTERS, RAFTER TIES SHALL BE INSTALLED IN THE LOWER 1/3 OF THE ATTIC SPACE AND IN ACCORDANCE WITH TABLE 1-S1.0. 16. COLLAR TIES SHALL BE PROVIDED IN THE UPPER 1/3 OF THE ATTIC SPACE IN ACCORDANCE WITH TABLE 1-S1.0.

GARAGE

1. THE GARAGE FLOOR SHALL SLOPE TOWARDS THE GARAGE DOORWAYS. 2. DOORS BETWEEN THE GARAGE AND THE DWELLING - MINIMUM 1-3/8" SOLID CORE OR HONEY COMBED STEEL DOOR OR 20-MINUTE FIRE RATED. 3. THE GARAGE SHALL BE SEPARATED FROM THE RESIDENCE AND ITS ATTIC AREA BY 5/8", TYPE X GYPSUM BOARD, OR EQUIVALENT MATERIALS APPROVED FOR ONE-HOUR FIRE-RESISTIVE CONSTRUCTION, APPLIED TO GARAGE SIDE. WHERE THE SEPARATION IS A FLOOR-CEILING ASSEMBLY, THE STRUCTURE SUPPORTING THE SEPARATION SHALL ALSO BE PROTECTED BY 5/8", TYPE X GYPSUM BOARD, OR MATERIALS APPROVED FOR ONE-HOUR FIRE-RESISTIVE CONSTRUCTION OR EQUIVALENT, APPLIED TO THE GARAGE SIDE. PULL DOWN STAIRS LOCATED WITHIN GARAGE SHALL BE RATED TO BE ADEQUATELY PROTECTED WITH MATERIALS APPROVED FOR ONE-HOUR FIRE-RESISTIVE CONSTRUCTION. ATTIC ACESS PANELS LOCATED WITHIN GARAGE SHALL BE OF 5/8", TYPE X GYPSUM BOARD, OR MATERIALS FOR ONE-HOUR FIRE-RESISTIVE CONSTRUCTION.
4. GARAGE DOOR AND FRAME- THE H-FRAME FOR THE ATTACHMENT OF THE

TRACK AND COUNTER BALANCE SHALL CONSIST OF THE FOLLOWING: 2x6 VERTICAL JAMBS RUNNING FROM THE FLOOR TO CEILING ATTACHED WITH 1-3/4" x 0.120" NAILS AT 7" OC STAGGERED WITH (7) 3-1/4" x 0.120" NAILS THRU THE JAMB INTO THE HEADER, MINIMUM 2x8 HEADER FOR ATTACHMENT OF COUNTER BALANCE SYSTEM.

STAIRWAYS

1. STAIRWAYS SHALL PROVIDE A MAXIMUM 7-3/4" RISE AND MINIMUM 10" RUN. 2. PROVIDE MINIMUM 36" GUARDRAILS ON THE OPEN SIDES OF RAISED FLOORS, PORCHES, AND BALCONIES: MINIMUM 34" GUARDRAILS ON THE OPEN SIDES OF STAIRWAYS LOCATED MORE THAN 30" ABOVE THE FLOOR OR GRADE BELOW. GUARDRAIL ENCLOSURES SHALL HAVE INTERMEDIATE RAILS OR ORNAMENTAL PATTERNS THAT DO NOT ALLOW PASSAGE OF A SPHERE 4" IN DIAMETER. 3. EACH STAIRWAY OF THREE OR MORE RISERS SHALL PROVIDE A CONTINUOUS HANDRAIL ON AT LEAST ONE SIDE BETWEEN 34" AND 38" ABOVE THE NOSING OF THE TREADS

4. HANDRAILS SHALL HAVE A CIRCULAR CROSS SECTION OF 1-1/4" MINIMUM TO 2" MAXIMUM OR OTHER APPROVED GRASPABLE SHAPER PER IRC SECTION

5. PROVIDE A MINIMUM 6'-8" OF HEADROOM CLEARANCE IN STAIRWAYS. 6. ENCLOSED ACCESSIBLE SPACE UNDER STAIRWAYS SHALL HAVE WALLS AND THE UNDERSIDE OF THE STAIR AND LANDING PROTECTED WITH 1/2" GYPSUM

BOARD ON ENCLOSURE SIDE PER IRC SECTION 302.7. 7. SPIRAL STAIRS TO BE CONSTRUCTED PER IRC SECTION 311.7.10.1. 8. SPACE STRINGERS AT 16" OC MAX.

GENERAL

1. PLANS SHALL COMPLY WITH THE 2018 INTERNATIONAL RESIDENTIAL CODE WITH AMENDMENTS AS ADOPTED BY THE GOVERNING JURISDICTION. IF ANY CHANGES OR DEVIATIONS FROM THE PLANS ARE MADE DURING CONSTRUCTION, CONTRACTOR SHALL NOTIFY THE APPROPRIATE AUTHORITY AND ENGINEER OF RECORD, EITHER (OR BOTH) OF WHOM MAY REQUIRE REVISED DRAWINGS OR CALCULATIONS AT ITS

2. REPRODUCTION, ALTERATION, OR RE-USE BY ANY METHOD OF ALL OR PORTIONS OF THESE STRUCTURAL PLANS OR VARIATIONS THEREOF WITHOUT WRITTEN PERMISSION FROM APEX ENGINEERS, INC IS STRICTLY PROHIBITED. THE DRAWINGS AND DETAILS OF THIS SHEET SET. BEING INSTRUMENTS OF SERVICE. ARE AND SHALL REMAIN THE PROPERTY OF APEX ENGINEERS, INC. AN UNSEALED VERSION, OR A VERSION VOID OF APEX ENGINEERS LOGO AND/OR TITLE BLOCK, SHALL BE CONSIDERED AN UNAUTHORIZED REPRODUCTION.

3. WHERE DISCREPENCIES EXIST BETWEEN THE STANDARD COMMENTS, NOTES FROM THE DESIGN PROFESSIONAL OR THE CODE, THE MOST RESTRICTIVE SHALL

APPLY. THE DWELLING SHALL COMPLY WITH THE FOLLOWING LOAD CONDITIO						
AREA	MIN DEAD LOAD	MIN LIVE LOAD				
EXTERIOR BALCONIES	10 PSF	60 PSF				
DECKS	10 PSF	40 PSF				
CEILING JOISTS/ATTICS NO STORAGE - SCUTTLE ACCESS ONLY ROOF SLOPE 3:12 OR LESS	5 PSF	10 PSF				
CEILING JOISTS/ATTICS WITHOUT STORAGE - SCUTTLE ACCESS ONLY ROOF SLOPE OVER 3:12 OR LESS	10 PSF	10 PSF				
CEILING JOISTS/ATTICS WITH STORAGE - DOOR/PULL DOWN LADDER ACCESS	10 PSF	20 PSF				
ROOMS - NON-SLEEPING	10 PSF	40 PSF				
ROOMS - SLEEPING	10 PSF	30 PSF				
ROOF - LIGHT ROOF COVERING	10 PSF	20 PSF				
ROOF - HEAVY ROOF COVERING CONCRETE/TILE/SLATE	20 PSF	20 PSF				
NOTE: HEAVY ROOF COVERING WILL NOT BE INSTALLED OR USED IN						

THE DESIGN CALCULATIONS UNLESS IT IS SPECIFICALLY NOTED ON THE PLANS THAT THE DESIGN IS FOR HEAVY ROOF COVERINGS.

FOUNDATIONS

1. THE FOUNDATION DESIGN SHALL BE BASED ON A MINIMUM SOIL BEARING CAPACITY OF 2000 PSF, UNLESS OTHERWISE INDICATED ON THE PLANS OR IF MODIFIED BY AN ENGINEERING REPORT BASED ON ACTUAL SITE CONDITIONS. 2. CONCRETE SHALL MEET THE FOLLOWING SPECIFIED DESIGN STRENGTH

- 2500 PSI FOR BASEMENT FLOOR SLABS ON UNDISTURBED SOIL - 3000 PSI FOR FOOTINGS AND FOUNDATION WALLS
- 3500 PSI FOR GARAGE FLOOR SLABS 3. FOOTINGS SHALL EXTEND BELOW THE FROST LINE; MINIMUM DEPTH 36 INCHES BELOW GRADE.
- 4. UNLESS OTHERWISE NOTED ON THE PLANS OR IF SITE CONDITIONS REQUIRE OTHERWISE, FOOTINGS SHALL BE A MINIMUM OF 16" WIDE AND 8" DEEP WITH (2) #4 BARS CONTINUOUS.
- 5. COLUMN PADS SHALL BE A MINIMUM 30"x30"x12" WITH (4) #4 BARS EACH WAY UNLESS NOTED OTHERWISE. 6. UNLESS NOTED OTHERWISE ON THE PLANS, FOUNDATION WALLS SHALL BE MINIMUM 8" THICK x 8'-0" (OR 9'-0") TALL AND REINFORCED PER DETAIL 1-S2.0 (AND 2-S2.0 WHERE APPLICABLE). FOUNDATION WALLS GREATER THAN 10'-0" TALL REQUIRE A SEPERATE ENGINEERED DESIGN. PROVIDE A 2'-0" LONG
- INTERIOR OR EXTERIOR DEAD-MAN FOR ANY STRAIGHT WALL PANELS EXCEEDING 20'-0" IN LENGTH (REF 3-S2.0) 7. REINFORCEMENT SHALL BE MINIMUM GRADE 40 UNLESS NOTED OTHERWISE. REINFORCEMENT SHALL LAP A MINIMUM OF 24" AT ENDS, SPLICES, AND AROUND
- CORNERS 8. FOUNDATION WALLS SHALL BE BACKFILLED WITH A CLEAN LEAN CLAY (OR BETTER) LOW VOLUME CHANGE MATERIAL. ON-SITE MATERIAL MAY BE USED IF DEEMED ACCEPTABLE BY THE GEOTECHNICAL ENGINEER OF RECORD 9. FOUNDATION WALLS WILL NOT ACHIEVE FULL STRENGTH UNTIL THE BASEMENT SLAB AND THE FIRST FLOOR DECK HAVE BEEN PROPERLY PLACED. IF BACKFILLING THE INTERIOR OF THE FOUNDATION WALL WITH GREATER THAN 8" OF EARTHEN FILL OR 24" OF GRANULAR FILL, A STRUCTURAL BASEMENT SLAB (TO BE DESIGNED OR DESIGN REVIEWED BY APEX ENGINEERS). OR ALTERNATE ENGINEERED SOLUTION (i.e. ENGINEERED FILL) WILL BE REQUIRED. 10. WHERE JUMPS OR STEPS IN ELEVATION OCCUR FOUNDATION WALLS AND FOOTINGS SHALL BE FORMED CONTINUOUS AND POURED PER DETAIL 4-S2.0. 11. CONCRETE FLOOR SLABS SHALL BE A MINIMUM 4" THICK OVER A MINIMUM 4" BASE OF 1/2" OR 3/4" CLEAN GRADED ROCK, UNLESS NOTED OTHERWISE OR IF
- SITE CONDITIONS REQUIRE OTHERWISE 12. PROVIDE A MIN 6 MIL THICK POLYETHYLENE MOISTURE BARRIER OVER POURUS GRAVEL BASE UNDER BASEMENT FLOOR SLAB PER R406.2. LAP JOINTS MINIMUM 6" (NOT REQUIRED FOR GARAGE SLABS OR DETACHED ACCESSORY
- BUILDINGS) 13. FOR A STRUCTURAL REINFORCED CONCRETE FLOOR OVER A USABLE AREA, SUCH AS A GARGE FLOOR LOCATED OVER A STORAGE AREA, SUBMIT SEALED ENGINEERED DETAILS AND CALCULATIONS. 14. GARAGE SLABS AND BASEMENT OVERDIGS SUPPORTED BY FILL
- CONSISTING OF MORE THAN 24" OF GRANULAR FILL OR 8" OF EARTH SHALL BE REINFORCED PER DETAILS 1-S2.1 AND 6-S2.1 RESPECTIVELY. WHERE THE LIMITATIONS OF DETAILS 1-S2.1 AND 6-S2.1 ARE NOTE MET. A SEPERATE ENGINEERED DESIGN SHALL BE REQUIRED.
- 15. BASEMENT FOUNDATION SILL PLATES SHALL BE BOLTED TO THE FOUNDATION WITH A MINIMUM OF 1/2" ANCHOR BOLTS EMBEDDED AT LEAST 7" INTO THE CONCRETE AND SPACED NOT MORE THAN 3'-0" ON CENTER AND WITHIN 12" OF EACH END PIECE. 16. FOUNDATION WALLS SHALL BE DAMP-PROOFED PER IRC SECTION R406.
- 17. PROVIDE A MINIMUM 4" PERFORATED DRAIN AROUND USABLE SPACE BELOW GRADE OR OTHER EQUIVALENT MATERIALS PER IRC SECTION 405.1. THE PIPE SHALL BE PLACED ON A MINIMUM OF 2" OF WASHED GRAVEL OR CRUSHED ROCK AND COVERED WITH NOT LESS THAN 6". THE DRAIN SHALL DAYLIGHT TO THE EXTERIOR BELOW THE FLOOR LEVEL OR TERMINATE IN A MINIMUM 20 GALLON
- 18. INTERIOR BEARING WALLS AND COLUMNS SHALL BE ISOLATED FROM THE BASEMENT FLOOR SLAB.
- 19. INTERIOR NON-BEARING WALLS, OTHER THAN THOSE RESTING DIRECTLY ON THE FOOTING, SHALL BE ISOLATED FROM THE FLOOR FRAMING ABOVE. 20. ALL EARTH RETAINING STRUCTURES ON SITE GREATER THAN 4'-0" TALL (EXCLUDING CONCRETE FOUNDATION WALLS RESTRAINED AT BOTH THEIR TOP AND BOTTOM) SHALL REQUIRE A SEPARATE ENGINEERED DESIGN AS REQUIRED BY THE CODE AUTHORITY.
- 21. ANY GEOTECHNICAL IMPROVEMENT METHODS AND/OR STRUCTURAL SOLUTIONS (SUCH AS DRILLED PIERS) EMPLOYED TO ADDRESS UNACCEPTABLE SUBGRADE CONDITIONS SHALL BE SUBMITTED TO EOR AS ENGINEERED SHOP DRAWINGS FOR REVIEW AND APPROVAL.

EXPANSIVE SOILS DISCLAIMER:

THESE PLANS HAVE BEEN PREPARED BASED ON A PRESUMPTIVE ALLOWABLE BEARING CAPACITY AS ALLOWED BY IRC CODE AND THE LOCAL ENFORCING JURISDICTION.

APEX ENGINEERS, INC. (APEX) RECOMMENDS THAT ALL FOOTING EXCAVATIONS BE EVALUATED BY A LICENSED GEOTECHNICAL ENGINEER PRIOR TO THE PLACEMENT OF ANY FOUNDATION ELEMENTS. GEOTECHNICAL INVESTIGATION AND/OR TESTING IS NOT A SERVICE PROVIDED OR OFFERED BY APEX.

APEX HAS NOT BEEN RETAINED TO DETERMINE THE EXPANSIVE SOIL CHARACTERISTICS OF THE SUBGRADE SOIL AND THEREFORE CANNOT BE HELD RESPONSIBLE FOR THE VOLUMETRIC CHANGES OF THE SOIL (INCLUDING BELOW THE BASEMENT SLAB). BY USE OF THESE PLANS WITHOUT AN ACCOMPANYING GEOTECHNICAL ENGINEERING REPORT, APEX SHALL NOT BE HELD LIABLE FOR ANY FUTURE MOVEMENT AND/OR DIFFERENTIAL MOVEMENT OF THE PROPOSED STRUCTURE AND THE POSSIBLE DAMAGE THAT MAY BE CAUSED AS A RESULT OF SUCH MOVEMENT. DAMAGE FROM EXPANSIVE SOILS AND/OR SETTLEMENT CAN RESULT IN AMONGST OTHER THINGS, THE FOLLOWING: BASEMENT SLAB HEAVE, SHEETROCK CRACKS, WINDOWS AND DOOR BECOMING OUT OF PLUMB AND STICKING AND/OR NOT OPENING. DAMAGE TO TILE, MOULDING, AND OTHER COSMETIC FINISHES.



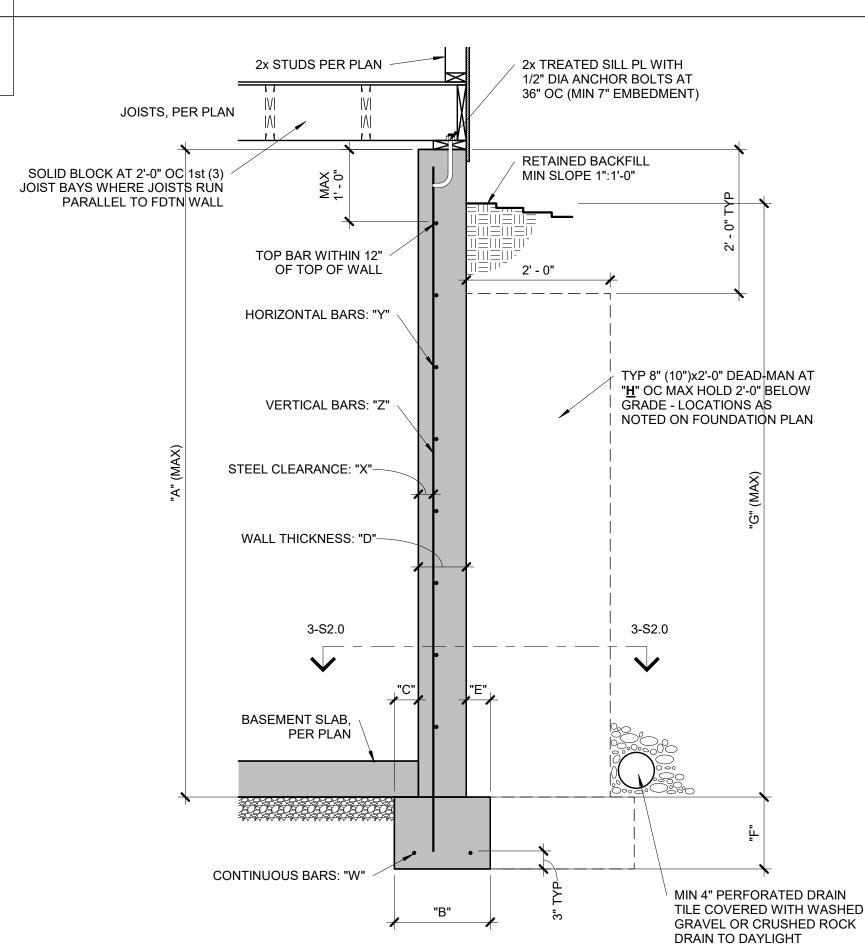


STRUCTURAL DESIGN REVIEW KANSAS ENGINEERING LICENSE MISSOURI ENGINEERING LICENSE: 2003004673

PROJECT #: 20-133 DRAWN BY: TDA CHECKED BY BDC SUBMITTAL DATE: 2021.01.20

SHEET:

GENERAL NOTES



CONCRETE DIMENSIONS REINFORCING BARS(GRADE 40 BARS) "W" "X" "Y" 8'-0" 1'-4" 4" 8" 4" 8" 7'-6" 20'-0" | │ (2) #4 │ 2 1/2" │ #4 BARS AT 24" OC │ #4 BARS AT 24" OC 9'-0" | 1'-4" | 4" | 8" | 4" | 8" | 8'-6" | 20'-0 | (2) #4 | 2 1/2" | #4 BARS AT 24" OC | #4 BARS AT 24" OC 10'-0" 1'-8" 5" 10" 5" 10" 9'-6" 20¹-0" (2) #4 | 2 1/2" | #4 BARS AT 18" OC | #4 BARS AT 18" OC

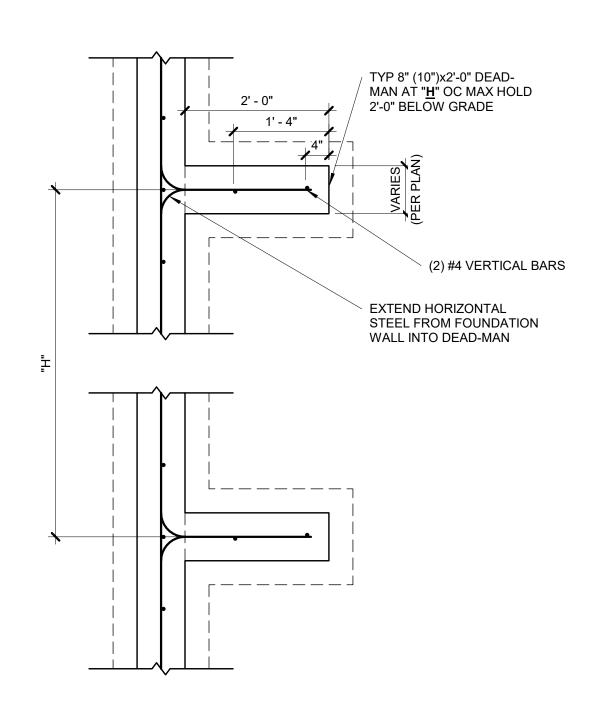
1. DIMENSION SHOWN IS FOR MAXIMUM UNINTERRUPTED WALL PANEL LENGTH BEFORE A DEAD-MAN SHALL BE INSTALLED. NOTE, A MINIMUM 2'-0" RETURN OR OFFSET IN THE FOUNDATION WALL SHALL SUBSTITUTE AS A DEAD-MAN AND/OR BREAK IN THE WALL PANEL LENGTH. 2. VERTICAL REINFORCING STEEL TO EXTEND TO WITHIN 8" OF TOP WALL. MINIMUM (1) #4 HORIZONTAL BAR WITHIN 12" OF TOP AND BOTTOM OF WALL.

3. BURIED CONCRETE FOUNDATION WALLS UP TO 9'-0" TALL MAY BE 8" NOMINAL THICKNESS WITH #4 BARS AT 24" OC BOTH WAYS OVER 16"x8" CONCRETE FOOTINGS WITH (2) #4 BARS CONTINUOUS, UNLESS OTHERWISE REQUIRED BY ENGINEERING REPORT BASED ON ACTUAL SITE CONDITIONS. 4. WALL WILL NOT ACHIEVE FULL STRENGTH UNTIL FIRST FLOOR DECK AND BASEMENT SLAB HAVE BEEN PLACED.

TYPICAL FOUNDATION WALL

1 DETAIL

S2.0 3/4" = 1'-0"



1. MIN 3000 PSI FOOTING COMPRESSIVE CONCRETE STRENGTH.

2. MIN 3000 PSI WALL COMPRESSIVE CONCRETE STRENGTH. 3. AIR ENTRAINED BETWEEN 5% & 7% OF CONCRETE VOLUME.

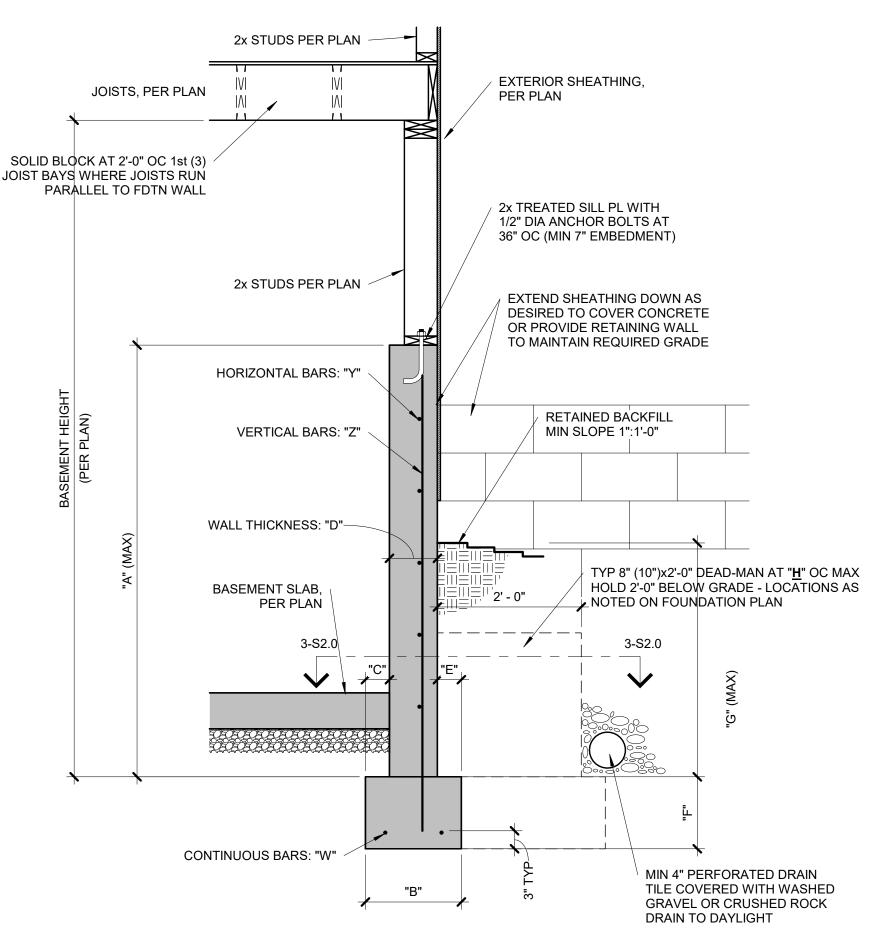
4. GRADE 40 REINFORCING STEEL UNLESS OTHERWISE NOTED. 5. LAP SPLICES 24" MIN.

6. WALL SHALL BE BACK-FILLED WITH CLEAN, LEAN CLAY (OR BETTER) LOW VOLUME CHANGE MATERIAL. ON-SITE MATERIAL MAY BE USED IF DEEMED ACCEPTABLE BY THE GEOTECHNICAL ENGINEER.

7. ASSUMED 2,000 PSF BEARING (TO BE VERIFIED BY GEOTECHNICAL ENGINEER).

3 TYPICAL DEAD-MAN SECTION

S2.0 3/4" = 1'-0"



CONCRETE DIMENSIONS					RE	INF	ORCING BAI	RS(GRADE 40 BARS)			
"A"	"B"	"C"	"D"	"E"	"F"	"G"	"H" ¹	"W"	"X"	"Y"	"Z"
4'-0"	1'-4"	4"	8"	4"	8"	3'-4"	20'-0"	(2) #4	N/A	#4 BARS AT 24" OC	#4 BARS AT 24" OC
6'-0"	1'-4"	4"	8"	4"	8"	4'-4"	20'-0	(2) #4	N/A	#4 BARS AT 24" OC	#4 BARS AT 24" OC
9'-0"	1'-8"	5"	8"	4"	8"	4'-4"	20"-0"	(2) #4	N/A	#4 BARS AT 24" OC	#4 BARS AT 24" OC

REINFORCING BARS(GRADE 40 BARS)								
"W"	"Z"							
(2) #4	N/A	#4 BARS AT 24" OC	#4 BARS AT 24" OC					
(2) #4	N/A	#4 BARS AT 24" OC	#4 BARS AT 24" OC					

1. DIMENSION SHOWN IS FOR MAXIMUM UNINTERRUPTED WALL PANEL LENGTH BEFORE A DEAD-MAN SHALL BE INSTALLED. NOTE, A MINIMUM 2'-0" RETURN OR OFFSET IN THE FOUNDATION WALL SHALL SUBSTITUTE AS A DEAD-MAN AND/OR BREAK IN THE WALL PANEL LENGTH.

2. VERTICAL REINFORCING STEEL TO EXTEND TO WITHIN 8" OF TOP WALL. MINIMUM (1) #4 HORIZONTAL BAR WITHIN 12" OF TOP AND BOTTOM OF WALL. 3. THE BASEMENT SLAB IS AN INTEGRAL PART OF THE 'UNRESTRAINED' FOUNDATION WALL DESIGN THEREFORE, IF THE

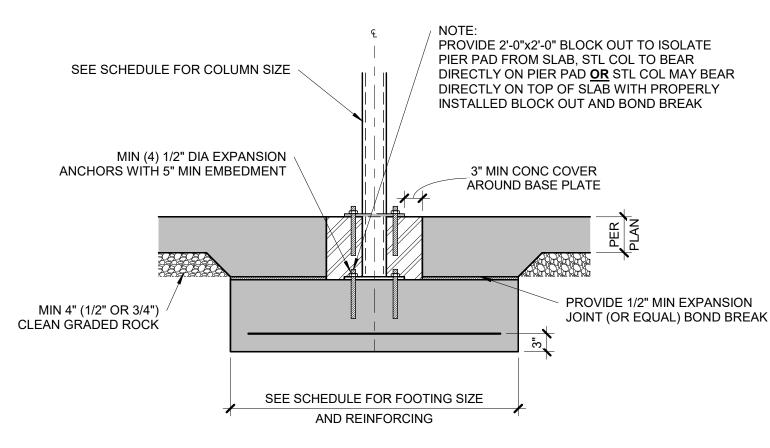
WALL IS BACKFILLED PRIOR TO PLACEMENT OF THE BASEMENT SLAB, THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROPERLY BRACING THE WALL UNTIL THE BASEMENT SLAB HAS BEEN PLACED.

TYPICAL 'UNRESTRAINED' 2 FOUNDATION WALL DETAIL

S2.0 3/4" = 1'-0"

COLUMN AND PIER PAD SCHEDULE COLUMN MARK PAD SIZE REINFORCING COL SIZE 30"x30"x12" (4) #4 BARS E-W 36"x36"x12" (4) #4 BARS E-W 42"x42"x12" (5) #4 BARS E-W 3" NOMINAL 48"x48"x12" (6) #4 BARS E-W 3" NOMINAL 3 1/2" NOMINAL 54"x54"x16" (8) #4 BARS E-W 60"x60"x16" (10) #4 BARS E-W 1. COLUMN AND PIER PAD SIZES SHOWN ARE FOR MAXIMUM COLUMN HEIGHT

OF 10'-0", REQUIRES SEPERATE ENGINEERED DESIGN IF GREATER THAN 10'-0" 2. COLUMN AND PIER PAD SIZES SHOWN ARE BASED ON AN ASSUMED MINIMUM ALLOWABLE SOIL BEARING CAPACITY OF 2,000 PSF.



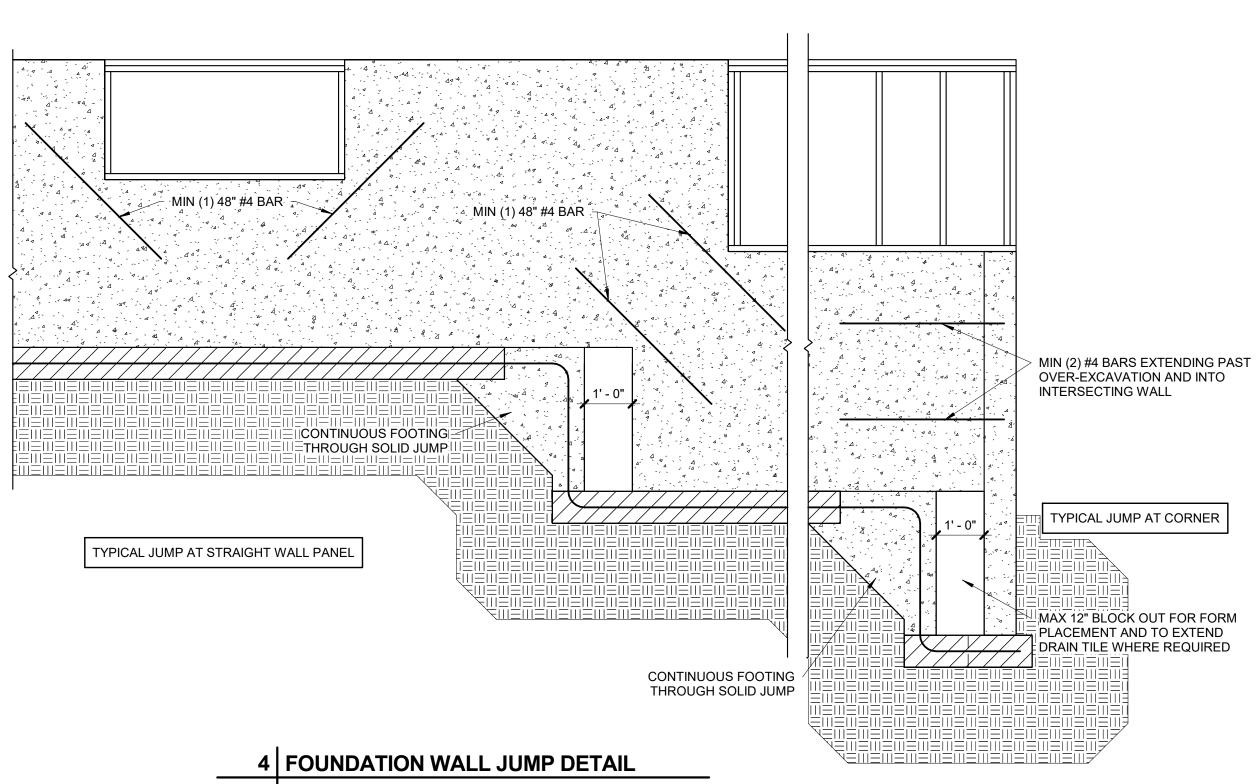
5 COLUMN PAD DETAIL **\$2.0** 3/4" = 1'-0"

EXPANSIVE SOILS DISCLAIMER:

THESE PLANS HAVE BEEN PREPARED BASED ON A PRESUMPTIVE ALLOWABLE BEARING CAPACITY AS ALLOWED BY IRC CODE AND THE LOCAL ENFORCING JURISDICTION.

APEX ENGINEERS, INC. (APEX) RECOMMENDS THAT ALL FOOTING EXCAVATIONS BE EVALUATED BY A LICENSED GEOTECHNICAL ENGINEER PRIOR TO THE PLACEMENT OF ANY FOUNDATION ELEMENTS. GEOTECHNICAL INVESTIGATION AND/OR TESTING IS NOT A SERVICE PROVIDED OR OFFERED BY APEX.

APEX HAS NOT BEEN RETAINED TO DETERMINE THE EXPANSIVE SOIL CHARACTERISTICS OF THE SUBGRADE SOIL AND THEREFORE CANNOT BE HELD RESPONSIBLE FOR THE VOLUMETRIC CHANGES OF THE SOIL (INCLUDING BELOW THE BASEMENT SLAB). BY USE OF THESE PLANS WITHOUT AN ACCOMPANYING GEOTECHNICAL ENGINEERING REPORT, APEX SHALL NOT BE HELD LIABLE FOR ANY FUTURE MOVEMENT AND/OR DIFFERENTIAL MOVEMENT OF THE PROPOSED STRUCTURE AND THE POSSIBLE DAMAGE THAT MAY BE CAUSED AS A RESULT OF SUCH MOVEMENT. DAMAGE FROM EXPANSIVE SOILS AND/OR SETTLEMENT CAN RESULT IN AMONGST OTHER THINGS, THE FOLLOWING: BASEMENT SLAB HEAVE, SHEETROCK CRACKS, WINDOWS AND DOOR BECOMING OUT OF PLUMB AND STICKING AND/OR NOT OPENING, DAMAGE TO TILE, MOULDING, AND OTHER COSMETIC FINISHES.



PROJECT #: 20-133 TDA DRAWN BY: **CHECKED BY** BDC 2021.01.20 SUBMITTAL DATE: SHEET:

ENGINEERS,INC

1625 LOCUST ST

KANSAS CITY, MO 64108

816.421.3222

www.apex-engineers.com

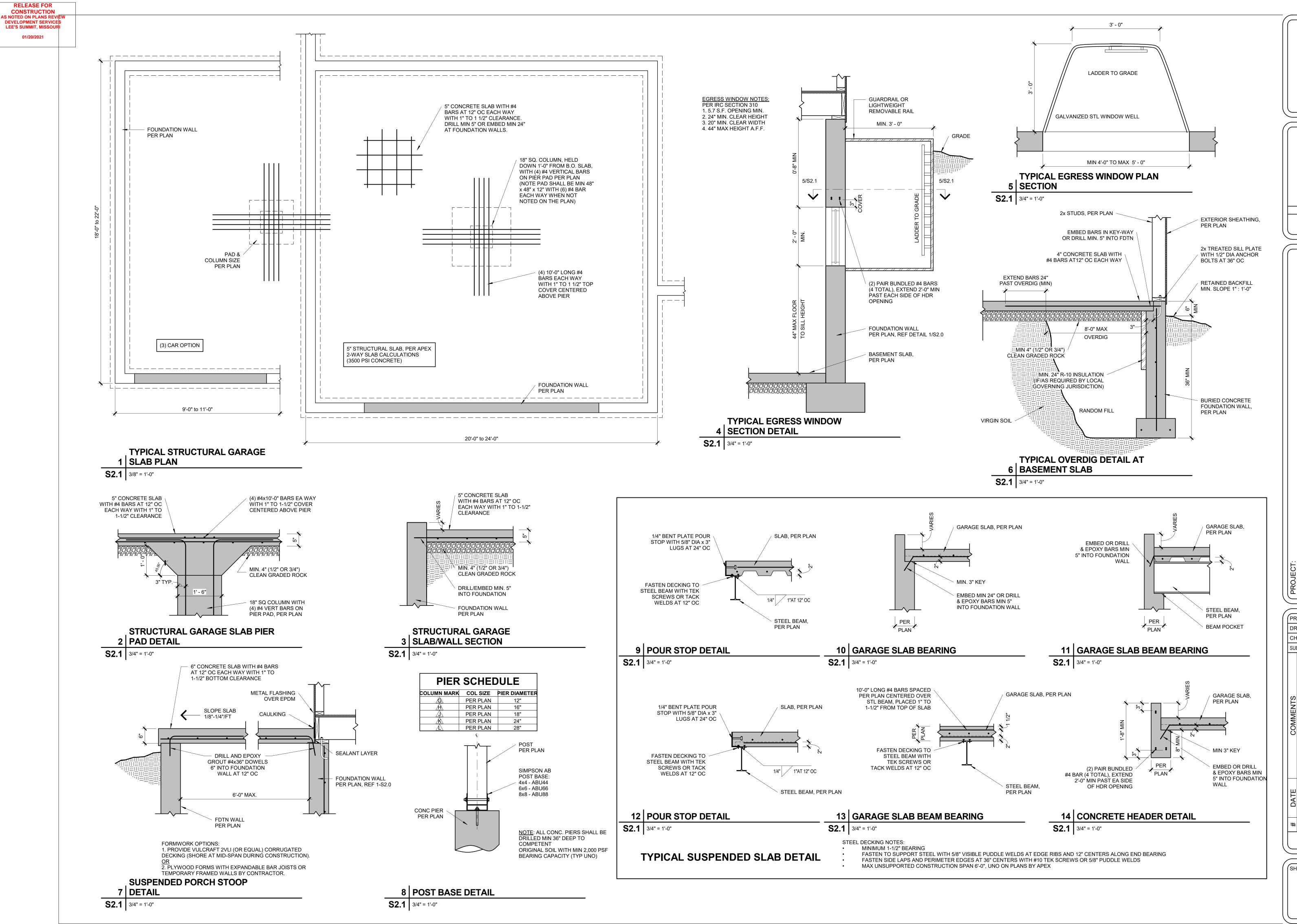
STRUCTURAL DESIGN REVIEW

MISSOURI ENGINEERING LICENSE:

2003004673

FOUNDATION DETAILS

S2.0 1/2" = 1'-0"



APEX
ENGINEERS, INC.

1625 LOCUST ST
KANSAS CITY, MO 64108
816.421.3222
www.apex-engineers.com

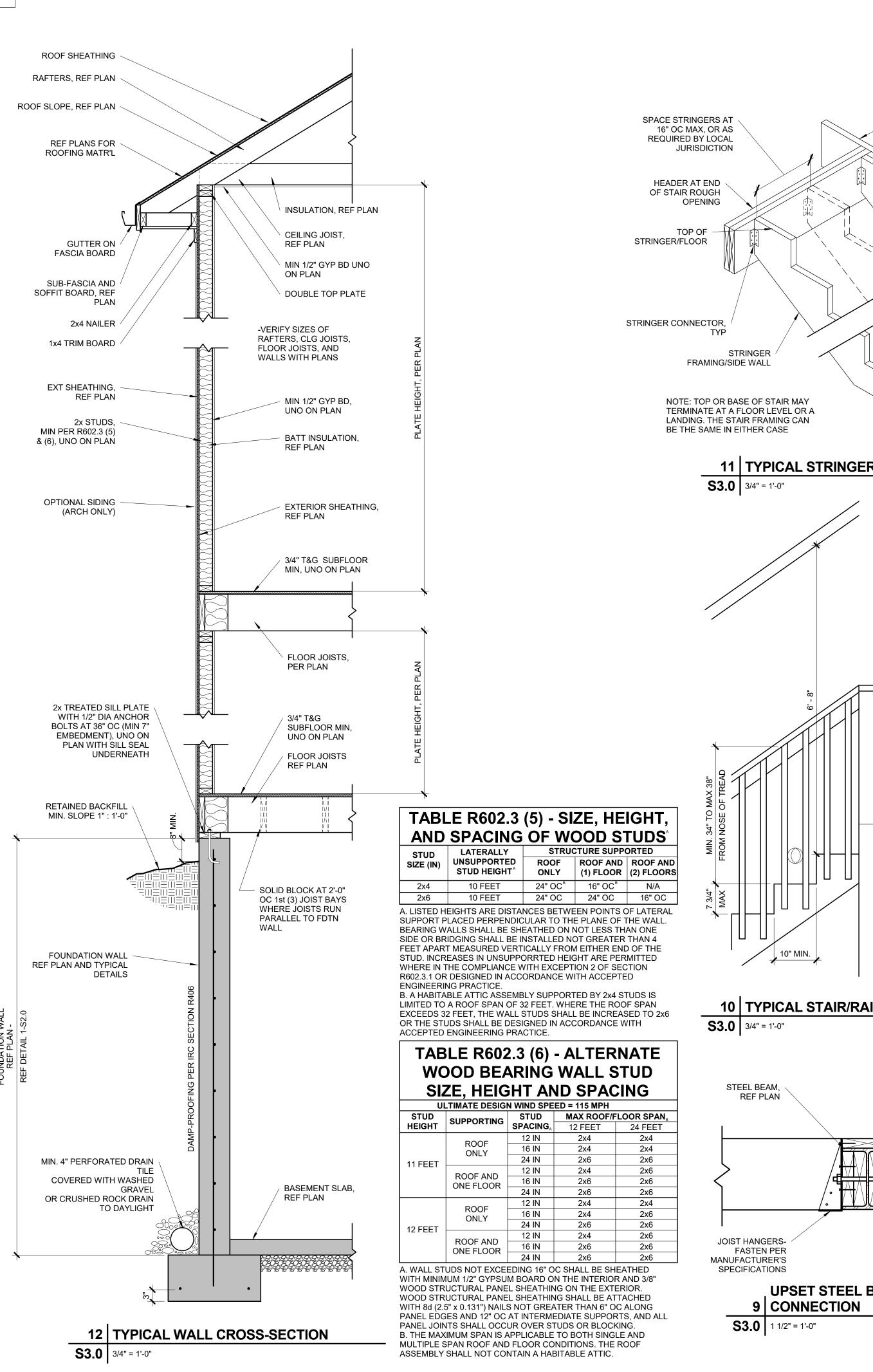


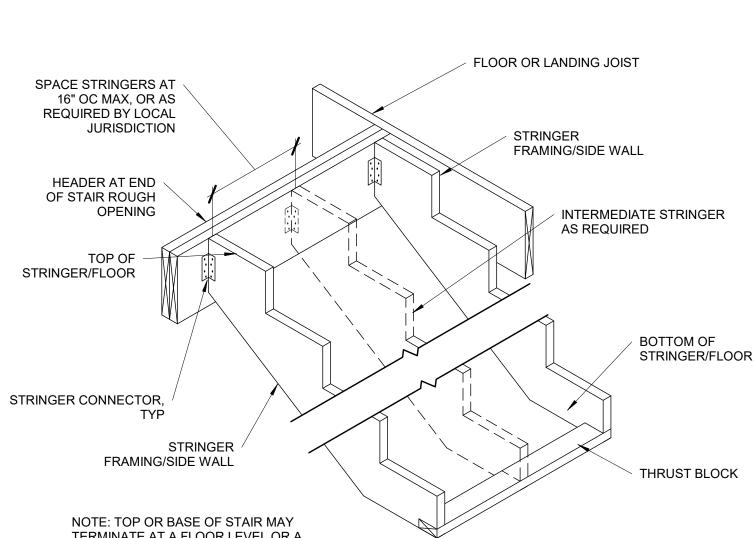
STRUCTURAL DESIGN REVIEW

KANSAS ENGINEERING LICENSE:
E-992
MISSOURI ENGINEERING LICENSE:
2003004673

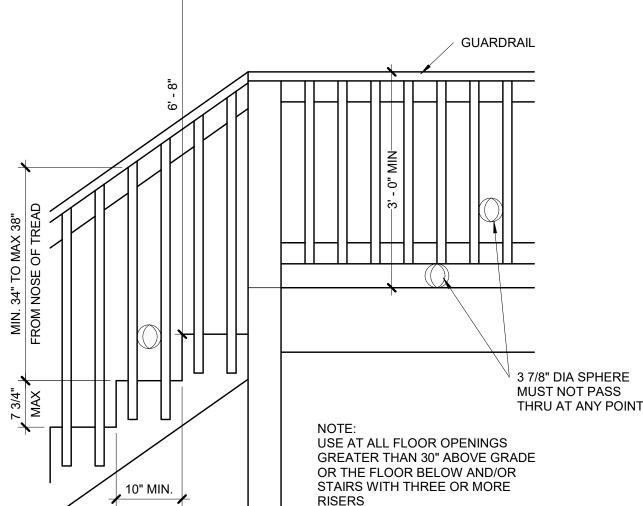
nbersham Dr mit, Missouri Homes

SHEET:
FOUNDATION DETAILS

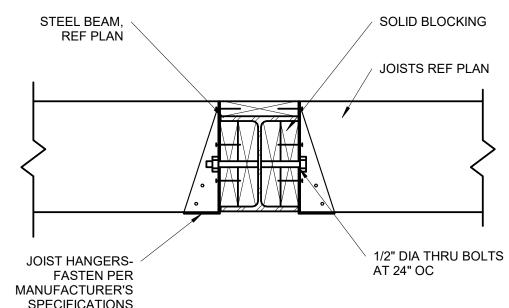




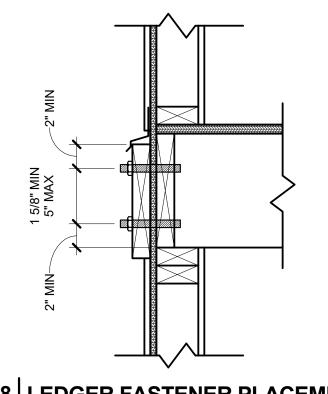




10 TYPICAL STAIR/RAIL DETAIL

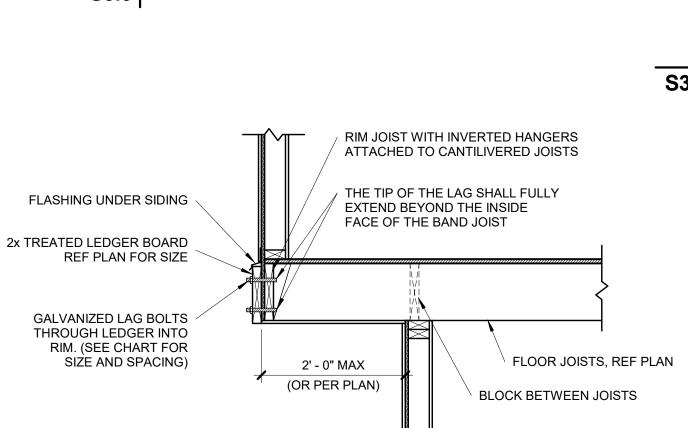


UPSET STEEL BEAM/JOIST



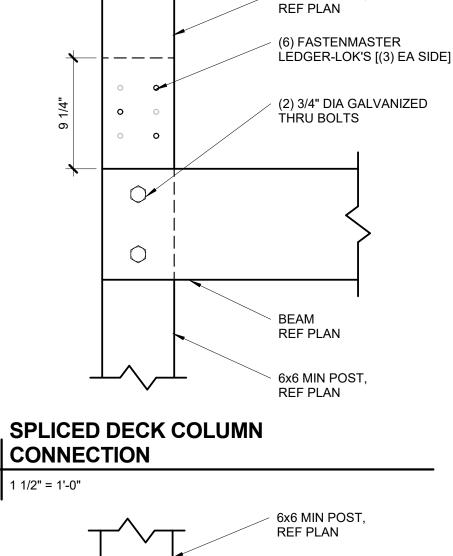
LEDGER FASTENER PLACEMENT

S3.0 1 1/2" = 1'-0"



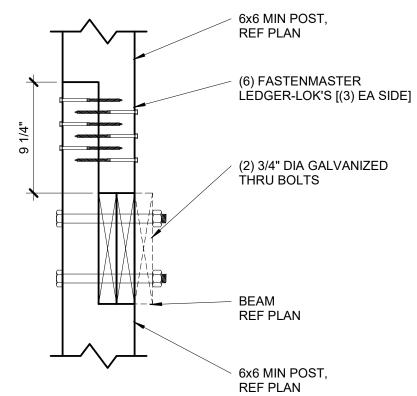
TYPICAL CANTILEVER FRAMING 7 WITH DECK ATTACHMENT

S3.0 3/4" = 1'-0"



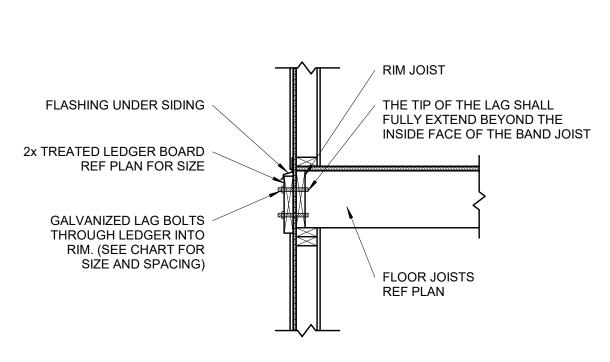
6x6 MIN POST,

4 CONNECTION **S3.0** 1 1/2" = 1'-0"



SPLICED DECK COLUMN 3 CONNECTION

S3.0 1 1/2" = 1'-0"



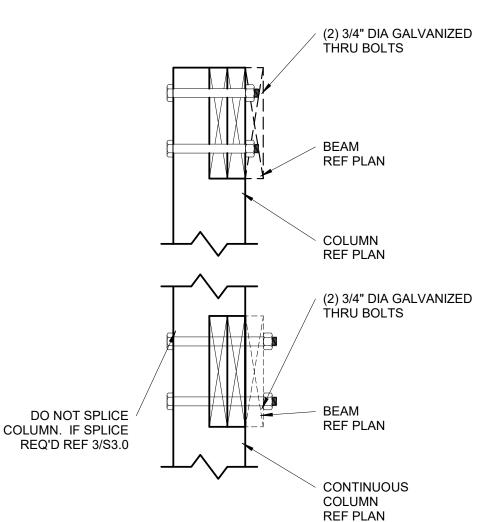
DECK JOIST SPAN 1/2" DIA LAG SPACING FOR 16" OC JOIST BAYS							
UP TO 10'-0"	16" OC	N/A					
10'-1" TO 12'-0"	15" OC	16" OC DBL EVERY OTHER					
12'-1" TO 14'-0"	13" OC	16" OC DBL EVERY OTHER					
14'-1" TO 16'-0"	11" OC	16" OC DBL EVERY JOIST BAY					
16'-1" TO 18'-0"	10" OC	16" OC DBL EVERY JOIST BAY					

6 TYPICAL LEDGER ATTACHMENT **S3.0** 3/4" = 1'-0"

*DISTANCE SHALL BE PERMITTED TO BE REDUCED TO 4 1/2" IF LAG SCREWS 5 1/2" MIN ARE USED OR BOLT SPACING IS REDUCED TO THAT OF LAG SCREWS TO 6 1/2" MIN 2x12 7 1/2" MIN REDUCED TO THAT OF EACH STATE ATTACH 2x8 LEDGERS TO 2x8 BAND STAGGER **FASTENERS** LAG SCREW OR BOLT IN 2 ROWS LEDGER, REF PLAN FOR SIZE

5 TYPICAL LEDGER BOLT SPACING

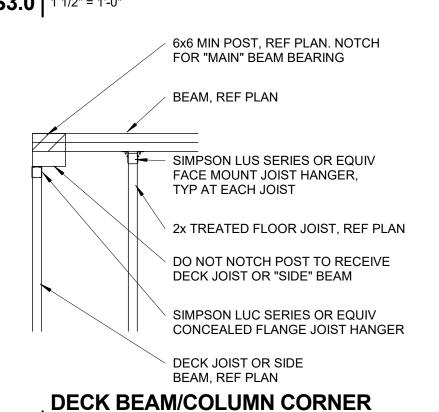
S3.0 3/4" = 1'-0"



DECK BEAM/COLUMN 2 CONNECTION

S3.0 1 1/2" = 1'-0"

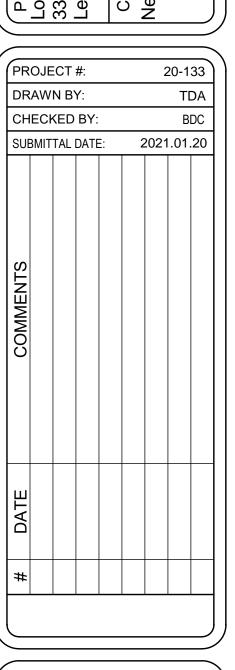
1 CONDITION



ENGINEERS,INC 1625 LOCUST ST KANSAS CITY, MO 64108 816.421.3222 www.apex-engineers.com

STRUCTURAL DESIGN REVIEW

KANSAS ENGINEERING LICENSE: MISSOURI ENGINEERING LICENSE: 2003004673



SHEET: FRAMING DETAILS



2x PLATE T&B, UNO

FASTEN PLATE TO EACH

VERT MEMBER WITH 10d

2x PACKOUT AT EACH END

THROUGHOUT HEADER SPAN

FASTEN VERT MEMBERS TO

PACKOUT WITH (3) 10d NAILS

NAILS AT 16" OC UNO

HEADER VERT MEMBERS, REF PLAN

AND 4'-0" OC MAX

HEADERS WITH GREATER THAN 1"
GAP BETWEEN VERT MEMBERS

S3.1 NOT TO SCALE

2 - PLY

10 SCHEDULE

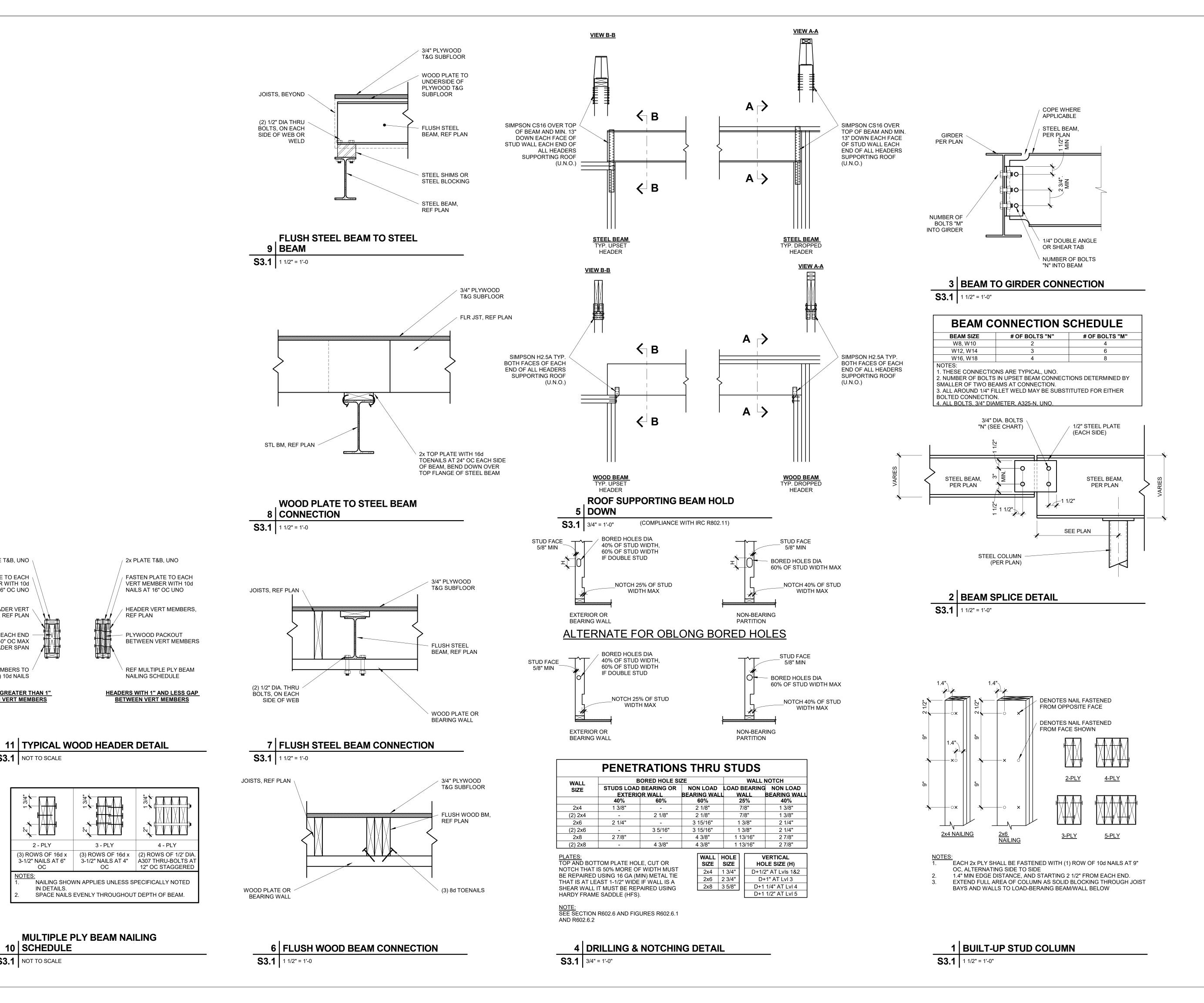
S3.1 NOT TO SCALE

IN DETAILS.

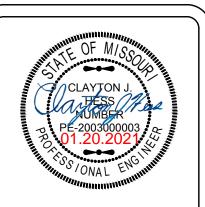
3 - PLY

MULTIPLE PLY BEAM NAILING

REF PLAN



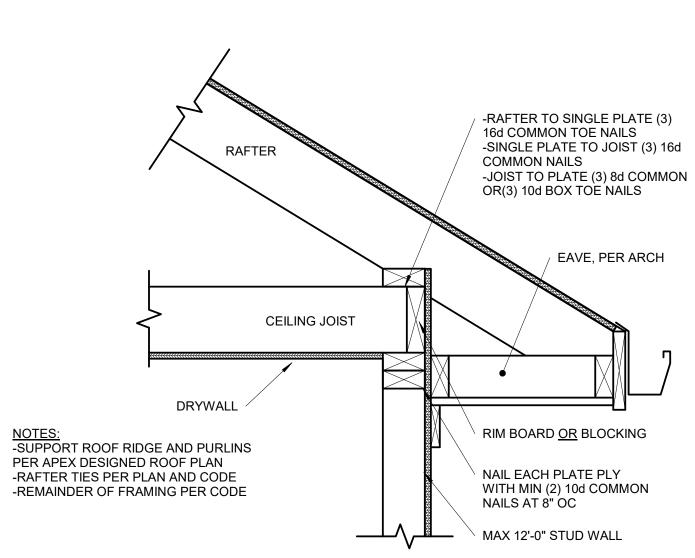
ENGINEERS,INC 1625 LOCUST ST KANSAS CITY, MO 64108 816.421.3222 www.apex-engineers.com



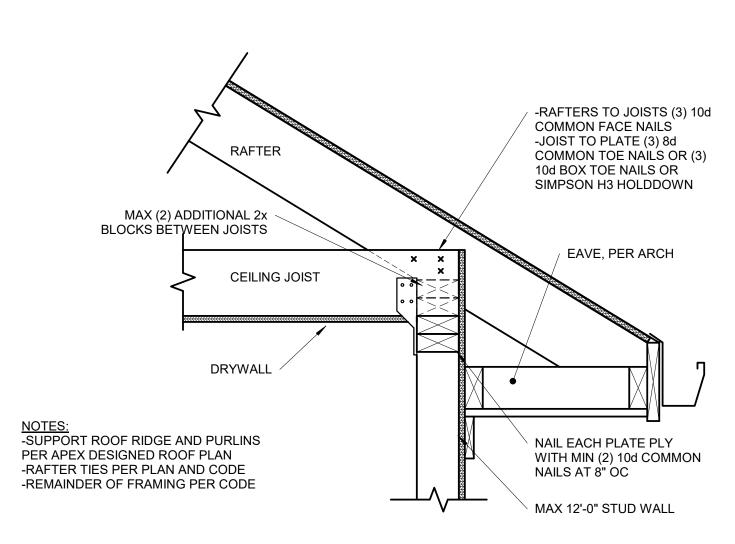
STRUCTURAL DESIGN REVIEW KANSAS ENGINEERING LICENSE: E-992 MISSOURI ENGINEERING LICENSE: 2003004673

PROJECT #: 20-133 TDA DRAWN BY: CHECKED BY: BDC 2021.01.20 SUBMITTAL DATE:

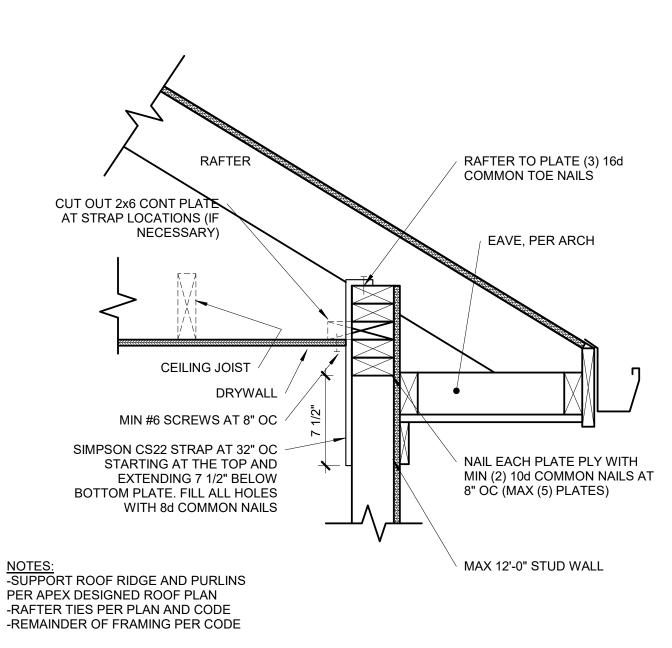
> SHEET: FRAMING DETAILS



8 OPTIONAL RAFTER BEARING S3.2 1 1/2" = 1'-0



7 OPTIONAL RAFTER BEARING S3.2 1 1/2" = 1'-0



6 OPTIONAL RAFTER BEARING
S3.2 1 1/2" = 1'-0

UPLIFT CONNECTION SCHEDULE									
OVERHANG SPAN: 1'-1" TO 1'-9"									
RAFTER SPACING	UPLIFT CONNECTOR	EXPOSURE B	EXPOSURE C						
12" OC	SIMPSON H2.5A	(1) AT 24" OC	(1) AT 24" OC						
16" OC	SIMPSON H2.5A	(1) AT 32" OC	(1) AT 16" OC						
24" OC	SIMPSON H2.5A	(1) AT 24" OC	(1) AT 24" OC						
OVERHANG SPAN: 1'-10" TO 2'-6"									
RAFTER SPACING	UPLIFT CONNECTOR	EXPOSURE B	EXPOSURE C						
12" OC	SIMPSON H2.5A	(1) AT 12" OC	(1) AT 12" OC						
16" OC	SIMPSON H2.5A	(1) AT 16" OC	(2) AT 16" OC						
24" OC	SIMPSON H2.5A	(2) AT 24" OC	(2) AT 24" OC						
	OVERHANG	SPAN: 2'-7" TO 3'-9	9"						
RAFTER SPACING	UPLIFT CONNECTOR	EXPOSURE B	EXPOSURE C						
12" OC	SIMPSON H2.5A	(2) AT 12" OC	(2) AT 12" OC						
16" OC	SIMPSON H2.5A	(2) AT 16" OC	(2) AT 16" OC						
24" OC	SIMPSON H2.5A	(2) AT 24" OC	N/A						

RAFTER BEAM SUPPORTING

OVERHANG SPAN	MIN BACKSPAN LENGTH
≤1'-0"	1'-0"
1'-1" to 2'-0"	EQUALS OVERHANG SPAN
≥2'-1"	OVERHANG SPAN x2
NOTES:	

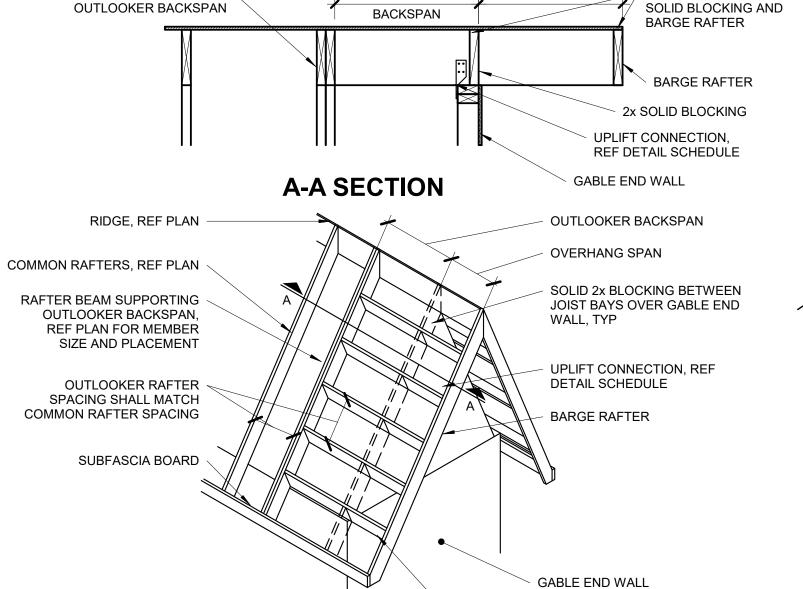
NOTES:
-CHART IS ONLY APPLICABLE IF NO
RAFTER BEAM SHOWN ON PLAN.
-CONTACT EOR IF OVERHANG LENGTH
EXCEEDS CHART OPTIONS.

OVERHANG SPAN

OUTLOOKER RAFTERS,
MATCH COMMON RAFTER

SIZE, REF PLAN

4" OC NAIL SPACING AT



OUTLOOKER

	S3.2	NOT TO SO	CALE						
F	REQUI	RED N	IUMBE	ΞR	OF SIS	TER P	LIES		
			LIGH	TR	OOF				
2x VALLEY						LVL VAL	.LEY		
# OF SISTER	OF SISTER RAFTER SIZE			# OF SISTER	F	RAFTER SIZE			
PLIES	2x6	2x8	2x10		PLIES	2x6	2x8	2x10	
0	4'-8"	6'-2"	7'-11"		0	8'-8"	11'-5"	14'-7"	
1	9'-5"	*	*		1	*	*	*	
2	*	N/A	N/A		2	N/A	N/A	N/A	
•			HEA\	/Y R	OOF			•	
2x VALLEY					LVL VALLEY				
# OF SISTER	RAFTER SIZE				# OF SISTER	RAFTER SIZE			
PLIES	2x6	2x8	2x10		PLIES	2x6	2x8	2x10	
0	3'-6"	4'-7"	5'-11"		0	6'-6"	8'-7"	10'-11"	
1	7'-1"	9'-3"	*		1	13'-1"	*	*	
2	*	*	N/A		2	*	N/A	N/A	

OUTLOOKER RAFTERS ROOF

5 FRAMING

*VALLEYS OF A LENGTH GREATER THAN THAT FOUND IN THE CELL ABOVE ARE CONTROLLED BY BENDING. APPLY THE NUMBER OF SISTER PLIES CORRESPONDING TO THIS ROW.

1. THIS TABLE IS INTENDED TO BE USED IN CONJUNCTION WITH THE STAMPED, ENGINEERED PLANS AS THEY ARE DRAWN BY APEX. BRACING LOCATIONS SHALL DETERMINE HORIZONTAL, UNSUPPORTED SPAN FROM VALLEY BEARING AND BE USED TO DETERMINE THE NUMBER OF SISTERS REQUIRED. BRACING LOCATIONS ARE NOT TO

BE INFERRED USING THIS TABLE.

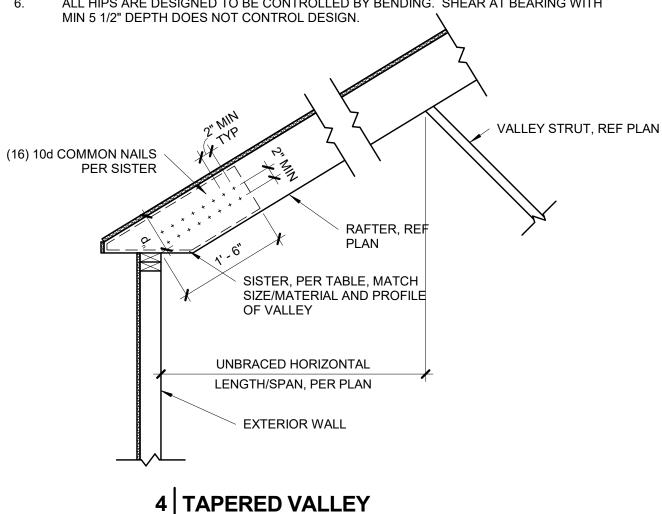
TABLE VALUES ARE BASED ON A DEPTH OF MEMBER REMAINING, d., EQUAL TO THE DEPTH OF THE RAFTERS. IF d IS OBSERVED TO BE LESS THAN THE DEPTH OF THE

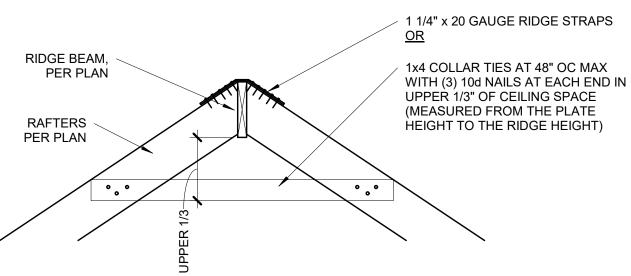
RAFTER, THE VALLEY WILL NEED TO BE EITHER REPLACED OR ANALYZED BY APEX.

TABLE VALUES ARE VALID FOR TAPERED CUTS ONLY, REF DETAIL 4/S3.2.

IF MULTI-PLY VALLEY IS SPECIFIED ON PLAN TREAT EACH ADDITIONAL PLY AS A SISTER PLY WHEN LOOKING UP MAX SPAN.

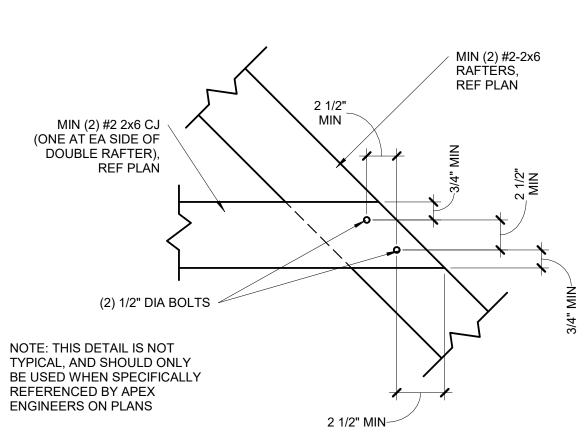
 MAX 14'-0" HORIZONTAL RAFTER SPAN IN BOTH DIRECTIONS FROM VALLEY.
 ALL HIPS ARE DESIGNED TO BE CONTROLLED BY BENDING. SHEAR AT BEARING WITH MIN 5 1/2" DEPTH DOES NOT CONTROL DESIGN





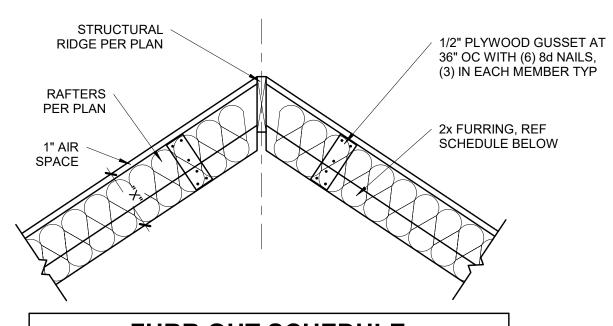
3 RIDGE BEAM DETAIL

S3.2 3/4" = 1'-0"



BOLTED RAFTER HIP 2 CONNECTION

S3.2 1 1/2" = 1'-0"



2x6 2x6 2x8 2x8 2x4 2x6 2x10 NOT REQUIRED 2x4 2x12 NOT REQUIRED REQUIRED	RAFTER SIZE	R-30C INSULATION (X= 9 1/4")	R-38C INSULATION (X=11 1/4")		
2x10 NOT REQUIRED 2x4 2x12 NOT REQUIRED REQUIRED	2x6	2x6	2x8		
2x12 NOT REQUIRED REQUIRED	2x8	2x4	2x6		
	2x10	NOT REQUIRED	2x4		
NOTES	2x12	NOT REQUIRED	REQUIRED		
NOTES:	NOTES:				
1. ALL VAULTED RAFTERS SHALL BE #2-2x6 DF-L, MINIMUM, AT 16" OC, PEI					
SPAN CHART, UNLESS NOTED OTHERWISE.	2. ALL VAULTS SHALL BE FURRED DOWN WITH 2x FRAMING TO THE				

1. ALL VAULTED RAFTERS SHALL BE #Z-2x6 DF-L, MINIMUM, AT 16° OC, PER SPAN CHART, UNLESS NOTED OTHERWISE.

2. ALL VAULTS SHALL BE FURRED DOWN WITH 2x FRAMING TO THE REQUIRED DEPTH OF INSULATION, PLUS 1" AIR SPACE.

3. R-30C INSULATION = 8 1/4" THICK

4. R-38C INSULATION = 10 1/4" THICK

5. INSULATION REQUIREMENTS MAY BE REDUCED TO R30 IF

5. INSULATION REQUIREMENTS MAY BE REDUCED TO R30 IF ROOF/CEILING ASSEMBLY DOES NOT ALLOW SUFFICIENT SPACE BUT IS LIMITED TO VAULTED CEILING AREAS THAT ARE LESS THAN 500 SQUARE FEET OR 20 PERCENT OF THE TOTAL INSULATED CEILING AREA, WHICHEVER IS LESS. (PER N1102.2.2)

VAULTED RAFTER INSULATION
1 | FURR OUT

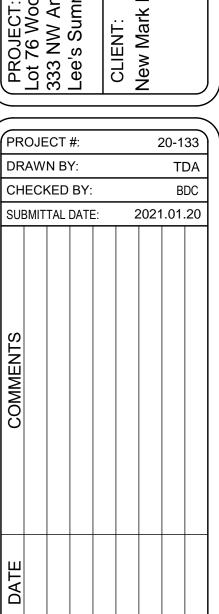
S3.2 3/4" = 1'-0"





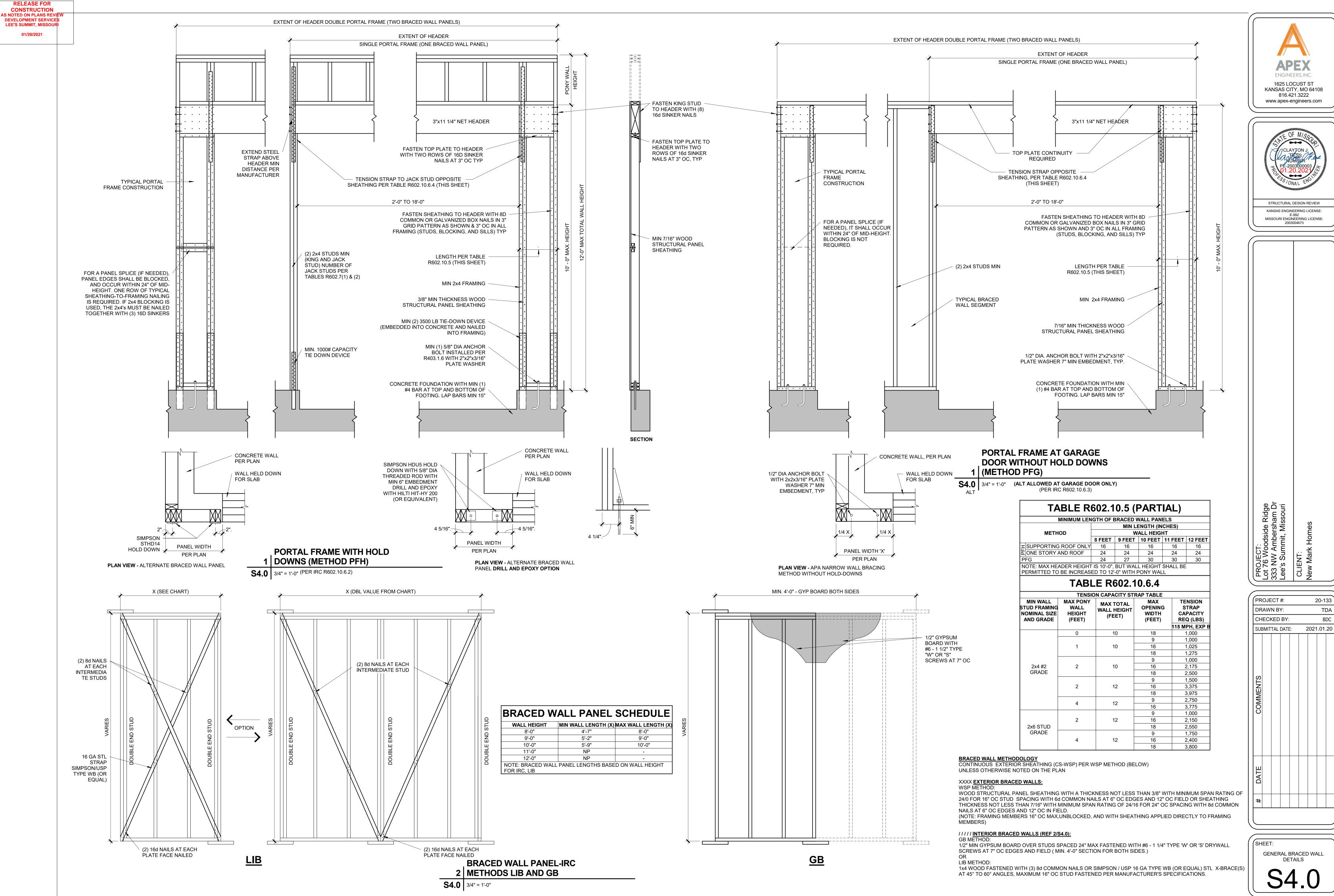
STRUCTURAL DESIGN REVIEW

KANSAS ENGINEERING LICENSE:
E-992
MISSOURI ENGINEERING LICENSE:
2003004673



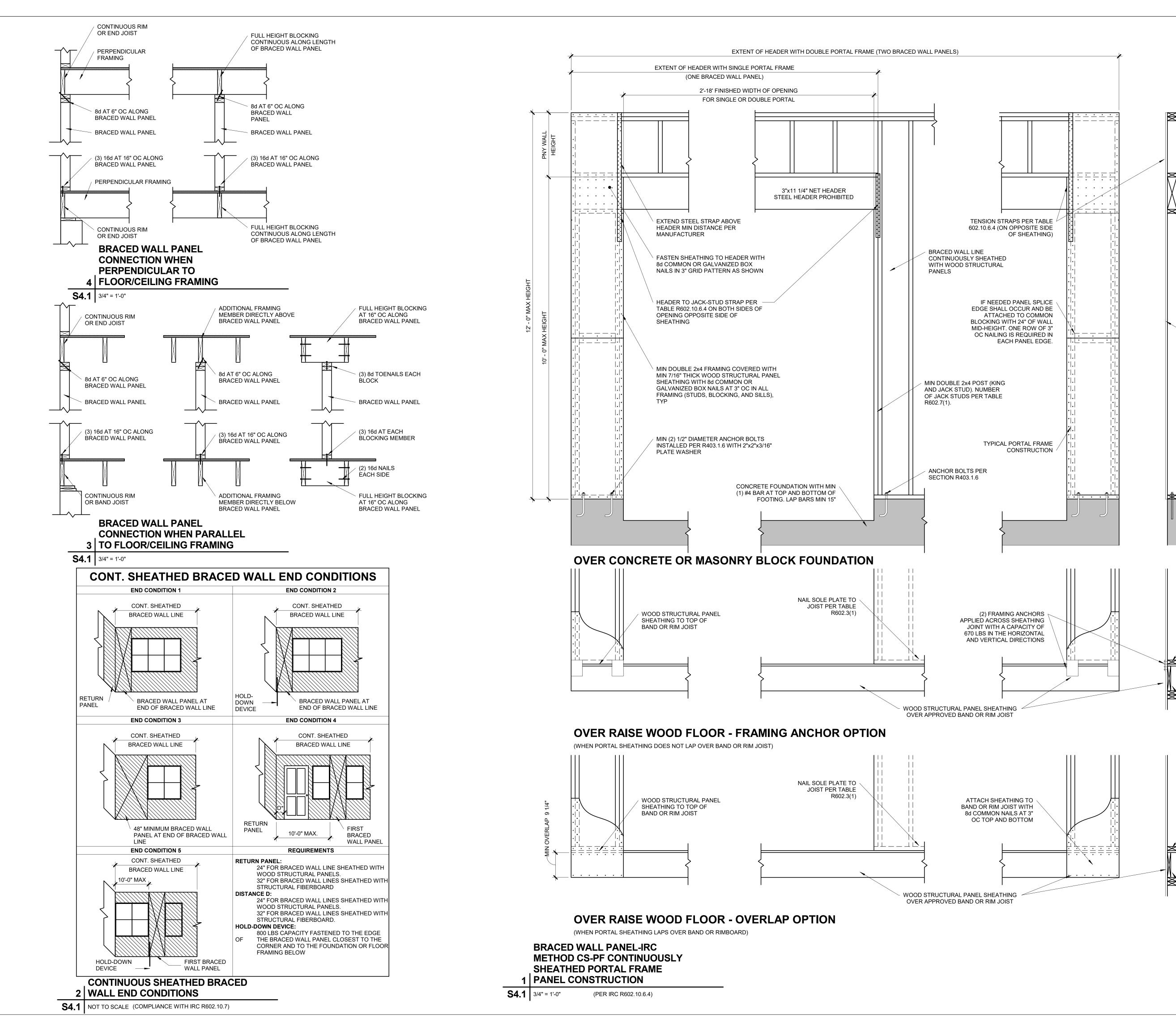
SHEET:
FRAMING DETAILS

S3 2





TDA BDC 2021.01.20



APEX ENGINEERS,INC. 1625 LOCUST ST KANSAS CITY, MO 64108 816.421.3222 www.apex-engineers.com



STRUCTURAL DESIGN REVIEW

KANSAS ENGINEERING LICENSE:
E-992
MISSOURI ENGINEERING LICENSE:
2003004673

FASTEN TOP PLATE TO

HEADER WITH TWO ROWS OF

MIN 7/16" WOOD STRUCTURAL

PANEL SHEATHING

NAIL SOLE PLATE TO JOIST

APPROVED BAND OR RIM

NAIL SOLE PLATE TO JOIST

APPROVED BAND OR RIM

PER TABLE R602.3(1)

PER TABLE R602.3(1)

16d SINKER NAILS AT 3" OC,

ot 76 Woodside Ridge 33 NW Ambersham Dr se's Summit, Missouri se's Homes

20-133

PROJECT #:

CHECKED BY: BDC

SUBMITTAL DATE: 2021.01.20

SUBMITTAL DATE: 2021.01.20

SHEET:

GENERAL BRACED WALL

DETAILS

S4.1