

January 7, 2021

Washam Homes

Re: 1801 SW River Run Dr

Lot 21, Whispering Woods

Lee's Summit, MO

Apex Engineers Inc. observed the house under construction at the above referenced address. Our firm has been retained to address comments from the city rough-in inspection. For the purposes of this report the house will be referred to as facing east.

- 1. Load bearing 2x4 wall between stairs to basement and foundation wall is supporting two floors plus roof/ceiling assembly. Provide engineered solution for repair or comply with table R602.3(5).
 - After computations, our firm recommends the following:
 - Double each 2x4 stud in the wall of concern, so that there is (2)-2x4 studs at 16" OC.
 - The built-up wood column (stud pack) supporting the LVL spanning over the garage shall consist of a minimum of (4)-2x4 studs. The studs shall align above and below the wall plate line.
- **Built-up wood column Each 2x4 ply shall be fastened with 1 row of 10d nails at 9" on center alternating side to side, 1.4" min edge distance, and starting 2.5" from each end.

Contingent upon the repairs outline above, our firm recommends approval of these items.

Please call if our firm can of further assistance.

LIMITATIONS

The scope of our services includes only those items specifically addressed herein. All other items are outside the scope of this inspection; including any environmental assessment (such as, but not limited to mold, mildew, or presence of any other toxic substance or environmental risks).

In addition, the scope our services does not include any evaluation of the building or site for jobsite safety and/or hazardous conditions. All construction shall be performed in compliance with IRC and OSHA standards at all times. Our firm has not been retained to examine the site or building for any of these conditions. In addition, the contractor shall retain sole responsibility for the quality of work, for adhering to plans, specifications, appropriate codes, and, for repairing defects, deficiencies or omission, regardless of when they are found. By the use of this report it is understood the above conditions are agreed to.

Best Regards,

Apex Engineers, Inc.

Joshua M. Jensen, P.E.

Project Engineer

Clayton J. Hes Principal

CLAYTON JOHN DE PE-2003000003 2-2021.01.11



Apex Engineers, Inc 1625 Locust Kansas City, MO 64108 www.apex-engineers.com



MAL ENGINEERING

Project Title: Engineer: Project ID: Project Descr:

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File: 1801 SW River Run Dr Calcs.ec6 Software copyright ENERCALC, INC. 1983-2020, Build:12.20.8.17

APEX ENGINEERS INC

Wood Column

Lic. # : KW-06005244

DESCRIPTION: Walls Studs in Basement

Code References

Calculations per NDS 2018, IBC 2018, CBC 2019, ASCE 7-16

Load Combinations Used: ASCE 7-16

General Information

Analysis Method :	: Allowable	Stress Des	ign	\	Wood Section Name	2-2x4		
End Fixities	Top & Bo	ttom Pinned		1	Wood Grading/Manuf.	Gradeo	d Lumber	
Overall Column H	leight		9.0 ft	\	Wood Member Type	Sawn		
(Used for	non-slender cald	culations)			Exact Width	3.0 in	Allow Stress Modification Fact	fore
Wood Species	Douglas Fir	- Larch			Exact Depth	3.50 in	Cf or Cv for Bending	1.10
Wood Grade	Stud	_			Area	10.50 in^2	Cf or Cv for Compression	1.050
Fb +	700.0 psi	Fv	180.0 ps	İ	lx	10.719 in^4	Cf or Cv for Tension	1.10
Fb -	700.0 psi	Ft	450.0 ps	i	ly	7.875 in^4		1.0
Fc - Prll	85 0.0 psi	Density	31.210 pc	f	• 9	7.075 4	Ct : Temperature Factor	1.0
Fc - Perp	625.0 psi						Cfu : Flat Use Factor	1.0
E : Modulus of Elasticity x-x B		x-x Bending	y-y Bending	Axial			Kf : Built-up columns	1.0 NDS 15.3.2
	Basic	1,400.0	1,400.0	1,400.0 k	(Sİ		Use Cr : Repetitive ?	No
	Minimum	510.0	510.0	F	Brace condition for de	flection (bucklin	na) along columns ·	

Brace condition for deflection (buckling) along columns:

X-X (width) axis: Fully braced against buckling ABOUT Y-Y Axis

Y-Y (depth) axis: Unbraced Length for buckling ABOUT X-X Axis = 9.0 ft, K = 1.0

Service loads entered. Load Factors will be applied for calculations.

Applied Loads

Column self weight included: 20.482 lbs * Dead Load Factor

AXIAL LOADS . .

Axial Load at 9.0 ft, D = 1.681 k

DESIGN SUMMARY

Bending & Shear Check Results

PASS Max. Axial+Bending Stress Ratio = Load Combination Governing NDS Forumla	0.4324 : 1 +D+H Comp Only, fc/Fc'	Maximum SERVIC Top along Y-Y Top along X-X	CE Lateral Load R 0.0 k 0.0 k	Reactions Bottom along Y-Y Bottom along X-X	0.0 k 0.0 k
Location of max.above base At maximum location values are Applied Axial Applied Mx Applied My Fc : Allowable	0.0 ft 1.701 k 0.0 k-ft 0.0 k-ft 374.739 psi	Along X-X	0.0 in at bination: n/a 0.0 in at bination: n/a	0.0 ft above base0.0 ft above base	
PASS Maximum Shear Stress Ratio = Load Combination Location of max.above base Applied Design Shear Allowable Shear	0.0 : 1 +0.60D+0.70E+H 9.0 ft 0.0 psi 288.0 psi	2.000 0000 0		Bending Compression	<u>Tension</u>

Load Combination Results

	0	0	Maximum Axial + Bending Stress Ratios			<u>Maximu</u>	m Shear Ra	<u>Ratios</u>	
Load Combination	C _D	CP	Stress Ratio	Status	Location	Stress Ratio	Status	Location	
+D+H	0.900	0.467	0.4324	PASS	0.0 ft	0.0	PASS	9.0 ft	
+D+L+H	1.000	0.429	0.4233	PASS	0.0 ft	0.0	PASS	9.0 ft	
+D+Lr+H	1.250	0.355	0.4086	PASS	0.0 ft	0.0	PASS	9.0 ft	
+D+S+H	1.150	0.382	0.4135	PASS	0.0 ft	0.0	PASS	9.0 ft	
+D+0.750Lr+0.750L+H	1.250	0.355	0.4086	PASS	0.0 ft	0.0	PASS	9.0 ft	
+D+0.750L+0.750S+H	1.150	0.382	0.4135	PASS	0.0 ft	0.0	PASS	9.0 ft	
+D+0.60W+H	1.600	0.286	0.3975	PASS	0.0 ft	0.0	PASS	9.0 ft	
+D+0.750Lr+0.750L+0.450W+H	1.600	0.286	0.3975	PASS	0.0 ft	0.0	PASS	9.0 ft	
+D+0.750L+0.750S+0.450W+H	1.600	0.286	0.3975	PASS	0.0 ft	0.0	PASS	9.0 ft	
+0.60D+0.60W+0.60H	1.600	0.286	0.2385	PASS	0.0 ft	0.0	PASS	9.0 ft	
+D+0.70E+0.60H	1.600	0.286	0.3975	PASS	0.0 ft	0.0	PASS	9.0 ft	
+D+0.750L+0.750S+0.5250E+H	1.600	0.286	0.3975	PASS	0.0 ft	0.0	PASS	9.0 ft	



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APEX ENGINEERS INC

Wood Column

Lic. # : KW-06005244

DESCRIPTION: Walls Studs in Basement

Load Combination Results

Load Combination	C _D	С _Р		Maximum Axia Stress Ratio			Ratios ation	Stre	Maxim ss Ratio		near Ratios atus Lo	cation
+0.60D+0.70E+H	1.600	0.286		0.2385	PAS	SS	0.0 ft		0.0	P	ASS	9.0 ft
Maximum Reactions								Note: O	nly non-	zero r	eactions a	are listed.
	X-X Axis F	Reaction	k	Y-Y Axis Rea	ction	Axial Reac	tion	My - End Mo	ments	k-ft	Mx - End	Moments
Load Combination	@ Base	@ Top		@ Base @	Top	@ Base	е	@ Base	@ Top		@ Base	@ Top
+D+H						1.7	701					
+D+L+H						1.7	701					
+D+Lr+H						1.7	701					
+D+S+H						1.7	701					
+D+0.750Lr+0.750L+H						1.7	701					
+D+0.750L+0.750S+H						1.7	701					
+D+0.60W+H						1.7	701					
+D+0.750Lr+0.750L+0.450W+H						1.7	701					
+D+0.750L+0.750S+0.450W+H						1.7	701					
+0.60D+0.60W+0.60H						1.0)21					
+D+0.70E+0.60H						1.7	701					
+D+0.750L+0.750S+0.5250E+H						1.7	701					
+0.60D+0.70E+H						1.0)21					
D Only						1.7	701					
Lr Only												
L Only												
S Only												
W Only												
E Only												
H Only												

Maximum Deflections for Load Combinations

Load Combination	Max. X-X Deflection	Distance	Max. Y-Y Deflection	Distance	
+D+H	0.0000 in	0.000 ft	0.0000 in	0.000 ft	
+D+L+H	0.0000 in	0.000 ft	0.0000 in	0.000 ft	
+D+Lr+H	0.0000 in	0.000 ft	0.0000 in	0.000 ft	
+D+S+H	0.0000 in	0.000 ft	0.0000 in	0.000 ft	
+D+0.750Lr+0.750L+H	0.0000 in	0.000 ft	0.0000 in	0.000 ft	
+D+0.750L+0.750S+H	0.0000 in	0.000 ft	0.0000 in	0.000 ft	
+D+0.60W+H	0.0000 in	0.000 ft	0.0000 in	0.000 ft	
+D+0.750Lr+0.750L+0.450W+H	0.0000 in	0.000 ft	0.0000 in	0.000 ft	
+D+0.750L+0.750S+0.450W+H	0.0000 in	0.000 ft	0.0000 in	0.000 ft	
+0.60D+0.60W+0.60H	0.0000 in	0.000 ft	0.0000 in	0.000 ft	
+D+0.70E+0.60H	0.0000 in	0.000 ft	0.0000 in	0.000 ft	
+D+0.750L+0.750S+0.5250E+H	0.0000 in	0.000 ft	0.0000 in	0.000 ft	
+0.60D+0.70E+H	0.0000 in	0.000 ft	0.0000 in	0.000 ft	
D Only	0.0000 in	0.000 ft	0.0000 in	0.000 ft	
Lr Only	0.0000 in	0.000 ft	0.0000 in	0.000 ft	
L Only	0.0000 in	0.000 ft	0.0000 in	0.000 ft	
S Only	0.0000 in	0.000 ft	0.0000 in	0.000 ft	
W Only	0.0000 in	0.000 ft	0.0000 in	0.000 ft	
E Only	0.0000 in	0.000 ft	0.0000 in	0.000 ft	
H Only	0.0000 in	0.000 ft	0.0000 in	0.000 ft	



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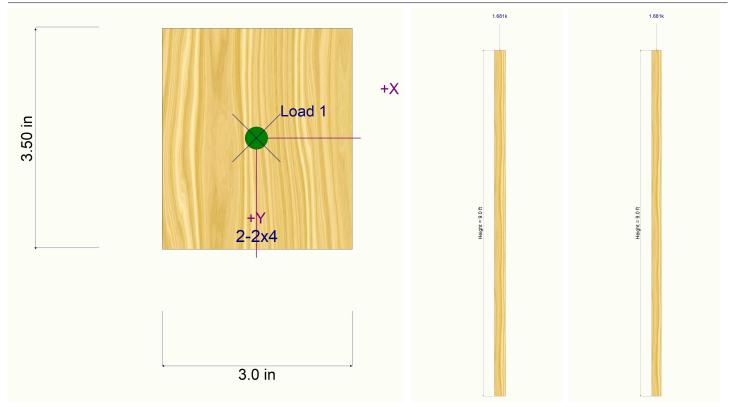
APEX ENGINEERS INC

Wood Column

Lic. # : KW-06005244

DESCRIPTION: Walls Studs in Basement

Sketches





Apex Engineers, Inc. 1625 Locust St. Kansas City, Missouri 64108 www.apex-engineers.com Job Address: 1801 SW River Run Dr.

Sheet # ____ of ___

Engineer:

Date: 1/8/2021

Wall Uniform Loads
• Dead load of wall = 92 165/ft • Second Floor Over Garage = 240 165/ft • 1st to Lower Stair Landing = 225 165/ft
1st to Lower Stair Landing = 225 1/05/Ft
Wall Print Loads
· Ceiling Beam Over Entry = 340 106
· Coiling Beam Over hallman = COO Nes
· Second floor (4) 1349 × 9/44 Beam = 1877 165
- Ceiling Beam over Entry = 540 165 • Ceiling Beam over Entry = 540 165 • Ceiling Beam over hallway = 600 165 • Second Floor (4) 1348 × 9148 Beam = 1877 165
Total load Per Stud
(16/12) (92+240+225) + (1877 165/2) = 1681 65/5700
(See enercale file for stud Capacity Calculations)
15 ee enercale tile tox 5 to0 capacity (acularions)
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CLAYFON J.
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4 of 4