

MiTek USA, Inc. 16023 Swingley Ridge Rd Chesterfield, MO 63017 314-434-1200

Re: 2544696 Summit/17 Woodside

The truss drawing(s) referenced below have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Builders FirstSource (Valley Center).

Pages or sheets covered by this seal: I43733203 thru I43733290

My license renewal date for the state of Missouri is December 31, 2021.

Missouri COA: Engineering 001193



November 23,2020

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



a truss system and to use the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPI1 Quality Criteria**, **DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

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Job	Truss	Truss Type	Qty	Ply	Summit/17 Woodside	
						143733203
2544696	A01	Hip Girder	1	1		
					Job Reference (optional)	
Builders FirstSource (Valley	Center), Valley Center, K	S - 67147,	8.	240 s Mar	9 2020 MiTek Industries, Inc. Mon Nov 23 08:59:36 2020	Page 2

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NOTES-

- 13) Use Simpson Strong-Tie LUS24 (4-10d Girder, 2-10d Truss, Single Ply Girder) or equivalent at 2-4-0 from the left end to connect truss(es) to back face of bottom chord.
- 14) Fill all nail holes where hanger is in contact with lumber.
- 15) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.
 16) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 284 lb down and 44 lb up at 26-4-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 17) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- 1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
 - Uniform Loads (plf) Vert: 1-4=-80, 4-9=-80, 9-12=-80, 16-22=-20, 13-15=-20, 13-25=-20
- Concentrated Loads (lb)
 - Vert: 8=-107(B) 13=-170(B) 18=-44(B) 5=-107(B) 16=-44(B) 28=-107(B) 29=-107(B) 30=-107(B) 32=-107(B) 34=-107(B) 35=-107(B) 35=-37(B) 37=-237(B) 37=-237(B) 35=-107(B) 38=-185(B) 39=-44(B) 40=-44(B) 41=-44(B) 42=-44(B) 43=-44(B) 44=-44(B) 45=-99(B) 46=-284(B)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601





 	8-4-0			14-4-0		20-8-0			24-4-0	28-8-0	
Plate Offsets (X,	Y) [2:0-3-	8,Edge]		0-0-0		0	+-0		3-0-0	4-4-0	
LOADING (psf) TCLL (roof) Snow (Pf) TCDL BCLL BCDL	25.0 20.0 20.0 0.0 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2018/T	2-0-0 1.15 1.15 YES 'PI2014	CSI. TC 0.7 BC 0.6 WB 0.4 Matrix-AS	78 63 48 S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.08 14-16 -0.17 16-19 0.04 13	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 108 lb	GRIP 197/144 FT = 20%
LUMBER- TOP CHORD 2 BOT CHORD 2 WEBS 2 WEDGE Left: 2x4 SPF No	2x4 SPF No.2 2x4 SPF No.2 8-10: 2x6 SPF 2x4 SPF No.2 0.2	*Except* No.2			1	BRACING- TOP CHORD BOT CHORD	Structural wc 2-0-0 oc purl Rigid ceiling	ood shea ins (3-8- ⁻ directly a	thing direct 13 max.): 4 applied.	ly applied, except -6.	
REACTIONS.	(size) 2= Max Horz 2= Max Uplift 2= Max Grav 2=	0-4-0, 13=0-4-0, 8=0-4 34(LC 16) -83(LC 12), 13=-61(LC 1226(LC 41), 13=1716	-0 13), 8=-55(LC 1 (LC 2), 8=566(L0	3) C 41)							
FORCES. (lb) - TOP CHORD BOT CHORD WEBS	- Max. Comp. 2-3=-2106/2 7-8=-957/13 2-16=-221/1 11-13=-166 5-14801/1	/Max. Ten All forces 2 96, 3-4=-1780/245, 4-5 5 903, 14-16=-135/1628, 1/193, 6-11=-1498/176, 35, 7-11=-1132/141, 7-	250 (Ib) or less e =-1596/243, 5-6 10-11=-85/791, 3-16=-484/96, 4	xcept when show =-1596/243, 6-7 8-10=-87/856 I-16=0/327, 4-14 -173/1956	own. 7=0/411, 4=-417/25,						
NOTES- 1) Unbalanced rr 2) Wind: ASCE 7 MWFRS (enver- to 20-4-0, Extr right exposed; 3) TCLL: ASCE 7 Rough Cat C; 4) Unbalanced s 5) This truss has non-concurrer 6) Provide adequ 7) This truss has 8) Bearing at joir capacity of be 9) Provide mech 10) This truss is referenced s 11) This truss de sheetrock be 12) Graphical pu	oof live loads 7-16; Vult=11: elope) and C- erior(2R) 20-4 ;C-C for mem 7-16; Pr=25.0 Partially Exp snow loads ha so been design nt with other I uate drainage s been design nt with other I uate drainage s been design tt(s) 8 consid earing surface annical connec designed in a standard ANS asign requires e applied direu urlin represen	have been considered Simph (3-second gust) \ C Exterior(2E) -0-10-8 Lo to 24-6-15, Interior(bers and forces & MWF psf (roof LL: Lum DOL ; Ce=1.0; Cs=1.00; Ct= ve been considered for de for greater of min ro ve loads. to prevent water pondi ed for a 10.0 psf botton ers parallel to grain valu	for this design. /asd=91mph; TC to 2-1-8, Interior 1) 24-6-15 to 29- TRS for reaction: =1.15 Plate DOI =1.10 this design. of live load of 12 ng. n chord live load the using ANSI/T is to bearing plate 18 International I 5" structural woo	CDL=6.0psf; BCI (1) 2-1-8 to 8-4-(6-8 zone; cantile s shown; Lumbe _=1.15); Pf=20.0 .0 psf or 1.00 tim nonconcurrent v Pl 1 angle to gra e capable of with Residential Code d sheathing be a entation of the p	DL=4.2psf 0, Exterior ever left ar p DOL=1.6 0 psf (Lum mes flat ro with any o ain formula hstanding e sections applied dir purlin along	; h=20ft; Cat. II; f (2R) 8-4-0 to 12- da right exposed 50 plate grip DOL DOL=1.15 Plate of load of 20.0 ps ther live loads. I. Building design 100 lb uplift at joi R502.11.1 and F ectly to the top c g the top and/or b	Exp C; Enclose 6-15, Interior(1 ; end vertical le =1.60 DOL=1.15); Is of on overhangs ner should verif nt(s) 2, 13, 8. R802.10.2 and hord and 1/2" g	d;) 12-6-15 ff and =1.0; y yy	5	ANDREW THOMAS JOHA90 PE-2017018 November 23	55000000000000000000000000000000000000
	i - Verify design pa	arameters and READ NOTES (ON THIS AND INCLU	DED MITEK REFERE	ENCE PAGE	MII-7473 rev. 5/19/20	20 BEFORE USE.				

Design valid for use only with MiTeK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPH Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

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5-7-	12 11-0-0	17-8-0	20-8-0	24-4-0	28-8-0						
5-7-	12 5-4-4	6-8-0	3-0-0	3-8-0	4-4-0						
Plate Offsets (X,Y) [2:0-3-	8,Edge], [4:0-6-12,0-1-12], [5:0-7-8,0-2-	0]									
LOADING (psf) TCLL (roof) 25.0 Snow (Pf) 20.0 TCDL 20.0 BCLL 0.0 BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2018/TPI2014	CSI. DEFL. TC 0.96 Vert(LL) BC 0.58 Vert(CT) WB 0.48 Horz(CT) Matrix-AS Horz(CT)	in (loc) l/defl -0.07 14-16 >999 -0.15 14-16 >999 0.05 12 n/a	L/d 240 180 n/a	PLATES MT20 MT20HS Weight: 113 lb	GRIP 197/144 148/108 FT = 20%					
LUMBER- TOP CHORD 2x4 SPF No.2 BOT CHORD 2x4 SPF No.2 8-10: 2x6 SPF WEBS 2x4 SPF No.2 WEDGE Left: 2x4 SPF No.2	2 *Except* = No.2	BRACING- TOP CHORD BOT CHORD WEBS	Structural wood she 2-0-0 oc purlins (2-2 Rigid ceiling directly 1 Row at midpt	athing directly a -0 max.): 4-5. applied. 4-13	applied, except						
REACTIONS. (size) 2= Max Horz 2= Max Uplift 2= Max Grav 2=	0-4-0, 12=0-4-0, 8=0-4-0 44(LC 20) -80(LC 12), 12=-45(LC 13), 8=-58(LC 1 1321(LC 41), 12=1810(LC 41), 8=540(L	3) C 41)									
FORCES. (lb) - Max. Comp. TOP CHORD 2-3=-2260/2 BOT CHORD 2-16=-201/2 6-11=-1512 6-11=-1512 WEBS 3-14=-844/1 7-11=-1022 7-11=-1022	/Max. Ten All forces 250 (lb) or less e 80, 3-4=-1484/242, 4-5=-561/171, 5-6= 2023, 14-16=-201/2023, 13-14=-105/132 (162, 10-11=-71/662, 8-10=-72/714 06, 4-14=0/494, 4-13=-976/98, 5-13=-3 (114	xcept when shown. -573/165, 6-7=0/435, 7-8=-807/128 2, 12-13=-267/35, 11-12=-1837/179, 60/65, 6-13=-88/1186,									
 7-11=-1022/114 NOTES- Unbalanced roof live loads have been considered for this design. Winck: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=20ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 11-0-0, Exterior(2R) 11-0-0 to 15-2-15, Interior(1) 15-2-15 to 17-8-0, Exterior(2R) 17-8-0 to 21-10-15, Interior(1) 21-10-15 to 29-6-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp; Ce=1.0; Cs=1.00; Ct=1.10 Unbalanced snow loads have been considered for this design. This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads. Provide adequate drainage to prevent water ponding. All plates are MT20 plates unless otherwise indicated. This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. Bearing at joint(s) & considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface. Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 12, 8. This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TP1 1. This truss design equires that a minimum of 7/16''s structural wood sheathing be applied directly bo the top chord and 1/2' gypsum sheetrock be applied directly to the bottom chord. Correlic unrule memoremetation doe not not dive the cire on the cire not the circutating b											
WARNING - Verify design pa Design valid for use only with 1 a truss system. Before use, the building design. Bracing indici- ie obvious conjurat for arbitim	arameters and READ NOTES ON THIS AND INCLU WITek® connectors. This design is based only upo b building designer must verify the applicability of d ated is to prevent buckling of individual truss web a and to prevent collarse with possible negroapil ini-	DED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2 n parameters shown, and is for an individual building esign parameters and properly incorporate this desig nd/or chord members only. Additional temporary an used proceedit deproce. Ever concerd methodeps	020 BEFORE USE. component, not n into the overall d permanent bracing								

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bilding design. Bracing indicated is to prevent buckling of individual truss we basing humanicator and property incorporate and buckling indicated as to prevent buckling of individual truss we basing humanicator and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Job	Truss	Truss Type	Qtv	Ply	Summit/17 Woodside		
				-		14:	3733206
2544696	A04	ROOF SPECIAL GIRDER	1	2			
				_	Job Reference (optional)		
Builders FirstSource (Valley (Center). Vallev Center, K	S - 67147.	8.	240 s Mar	9 2020 MiTek Industries, Inc. Mon	Nov 23 08:59:45 2020 Pa	ade 2

8.240 s Mar 9 2020 MiTek Industries, Inc. Mon Nov 23 08:59:45 2020 Page 2 ID:wH4RYhEsTNeUP2dXvOfi1syQY8e-tY3fQTZJIRVk0hYG2NDF5uJ4JXcy9gVJ9WuxiMyGLgS

NOTES-

- 5) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 6) Unbalanced snow loads have been considered for this design.
- 7) Provide adequate drainage to prevent water ponding.
- 8) All plates are MT20 plates unless otherwise indicated.
- 9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 10) WARNING: Required bearing size at joint(s) 12 greater than input bearing size.
- 11) Bearing at joint(s) 9, 25 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 12=463, 25=288.
- 13) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 14) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 15) Use Simpson Strong-Tie HUS26 (14-10d Girder, 4-10d Truss) or equivalent spaced at 4-0-0 oc max. starting at 4-0-12 from the left end to 22-0-12 to connect truss(es) to front face of bottom chord.
- 16) Fill all nail holes where hanger is in contact with lumber.
- 17) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1934 lb down and 162 lb up at 2-0-12, and 699 lb down and 44 lb up at 26-0-12, and 800 lb down and 28 lb up at 28-0-12 on top chord, and 1419 lb down and 96 lb up at 12-0-12, and 865 lb down and 88 lb up at 24-2-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

- Uniform Loads (plf)
- Vert: 24-29=-40, 3-29=-80, 3-4=-80, 4-8=-80, 24-26=-140, 10-11=-20, 7-8=-20, 9-10=-101, 23-25=-140, 12-23=-20 Concentrated Loads (lb)
 - Vert: 8=-757 10=-847(F) 11=-1042(F) 21=-1151(F) 24=-1934(F) 34=-640(F) 35=-1107(F) 36=-1211(F) 37=-1315(F) 38=-1419 39=-1042(F) 40=-1042(F) 41=-1042(F) 42=-845(F)





a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designer must verify the applicability of design parameters and properly damage. For general guidance regarding the fabrication, storage, delivery, rection and bracing of trusses and truss systems, see **ADSUTPHI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

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Job	Truss	Truss Type	Qty	Ply	Summit/17 Woodside	
					14373	33207
2544696	B01	Hip Girder	1	1		
					Job Reference (optional)	
Builders FirstSource (Valley	Center), Valley Center, K	S - 67147,	8.	240 s Mar	9 2020 MiTek Industries, Inc. Mon Nov 23 08:59:50 2020 Page	2 (

8.240 s Mar 9 2020 MiTek Industries, Inc. Mon Nov 23 08:59:50 2020 Page 2 ID:wH4RYhEsTNeUP2dXvOfi1syQY8e-DVsYTBdS6_716SQDqxoQny0uDYOQqyq2JnciNayGLgN

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-3=-80, 3-7=-80, 7-8=-80, 15-18=-20

Concentrated Loads (lb)

Vert: 21=32(B) 22=-95(B) 23=-95(B) 24=-95(B) 26=-95(B) 27=-95(B) 28=-95(B) 29=-95(B) 30=-95(B) 31=32(B) 32=-244(B) 33=-47(B) 34=-47(B) 35=-47(B) 36=-47(B) 37=-47(B) 38=-47(B) 38=-47(B) 41=-244(B)

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L	4-3-0	4-3-0 10-8-0					17-1-0					
I	4-3-0		6-5-	0			6-5-	-0		I	4-3-0	
LOADING (psi TCLL (roof) Snow (Pf) TCDL BCLL BCDL	f) 25.0 20.0 20.0 0.0 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2018/T	2-0-0 1.15 1.15 YES PI2014	CSI. TC BC WB Matri	0.83 0.89 0.38 x-AS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.17 -0.32 0.07	(loc) 9 8-9 7	l/defl >999 >796 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 78 lb	GRIP 197/144 FT = 20%
LUMBER- TOP CHORD BOT CHORD WEBS SLIDER	2x4 SPF No.2 3-5: 2x4 SPF 2x4 SPF No.2 2x4 SPF No.2 Left 2x4 SPF	2 *Except* 1650F 1.5E 2 2 No.2 2-6-0, Right 2x4 S	PF No.2 2-6-0			BRACING- TOP CHORD BOT CHORD	Struct 2-0-0 Rigid	tural wo oc purli ceiling	od sheat ins (2-7-(directly a	hing directly) max.): 3-5. pplied.	applied, except	
REACTIONS.	(size) 1= Max Horz 1= Max Uplift 1= Max Grav 1=	0-4-0, 7=Mechanical :22(LC 13) :-33(LC 13), 7=-33(LC 1 :1213(LC 39), 7=1213(L	2) C 39)									
FORCES. (Ib TOP CHORD BOT CHORD WEBS	DRCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. DP CHORD 1-3=-2098/273, 3-4=-3303/378, 4-5=-3303/378, 5-7=-2099/273 OT CHORD 1-11=-186/1852, 9-11=-189/1851, 8-9=-194/1851, 7-8=-191/1852 /EBS 3-9=-135/1552, 4-9=-958/151, 5-9=-135/1552											
NOTES- 1) Unbalanced 2) Wind: ASCE MWFRS (en 17-1-0, Exte and forces 8 3) TCLL: ASCE Rough Cat 0 4) Unbalanced 5) Provide ade 6) This truss ha 7) Refer to gird 8) Provide med 9) This truss a referenced s 10) This truss of sheetrock I 11) Graphical p	roof live loads 7-16; Vult=11 welope) and C prior(2E) 17-1-0 AWFRS for n 7-16; Pr=25.0 C; Partially Exp snow loads ha quate drainage as been design ter(s) for truss chanical connec designed in a design requires be applied dire purlin represen	have been considered to 5mph (3-second gust) V C Exterior(2E) 0-0-0 to to 21-4-0 zone; cantile eactions shown; Lumbe 0 psf (roof LL: Lum DOL .; Ce=1.0; Cs=1.00; Ct= two been considered for to to prevent water pondi ed for a 10.0 psf bottom to truss connections. ction (by others) of truss coordance with the 2018 TPI 1. that a minimum of 7/16 ctly to the bottom chord tation does not depict th	ior this design. asd=91mph; T(3-0-0, Interior(1) rer left and right = 1.15 Plate DO 1.10 this design. ng. chord live load to bearing plate International R " structural wood e size or the original to the original structural struc	CDL=6.0psf;) 3-0-0 to 4- exposed ; e grip DOL= L=1.15); Pf= nonconcurr e capable of esidential Cr d sheathing entation of t	BCDL=4.2p 3-0, Exterior end vertical I 1-60 20.0 psf (Lu ent with any withstandin ode sections be applied of he purlin alo	sf; h=20ft; Cat. II; E (2R) 4-3-0 to 8-5-1 eft and right expose m DOL=1.15 Plate other live loads. g 100 lb uplift at joi R502.11.1 and R8 directly to the top cl ng the top and/or b	Exp C; E 5, Interie ad;C-C f DOL=1 nt(s) 1, 7 102.10.2 hord and ottom cl	inclosed or(1) 8- or merr .15); Is= 7. 2 and d 1/2" g hord.	d; 5-15 to ibers =1.0; ypsum	O	ANDREY SANDE ANDREY THOMA JOHNSO PE-201701	S S O U R I X



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Scale = 1:36.6



L	5-9-0	15	-7-0		21-4-0		
	5-9-0	9-1	10-0			5-9-0	
Plate Offsets (X,Y) [1:0-3-	-8,Edge], [7:0-4-13,Edge]						
LOADING (psf) TCLL (roof) 25.0 Snow (Pf) 20.0 TCDL 20.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0- Plate Grip DOL 1.1. Lumber DOL 1.1. Rep Stress Incr YEs Code IRC2018/TPI2014	CSI. TC 0.62 BC 0.83 WB 0.44 Matrix-AS	DEFL. Vert(LL) - Vert(CT) - Horz(CT)	in (loc) -0.26 8-10 -0.58 8-10 0.08 7	l/defl L/d >979 240 >440 180 n/a n/a	PLATES MT20 Weight: 76 lb	GRIP 197/144 FT = 20%
LUMBER- TOP CHORD 2x4 SPF No.: BOT CHORD 2x4 SPF No.: WEBS 2x4 SPF No.: SLIDER Left 2x4 SPF	2 2 2 No.2 2-6-0, Right 2x4 SPF No.:	2-6-0	BRACING- TOP CHORD BOT CHORD	Structural wood 2-0-0 oc purlins Rigid ceiling di	d sheathing directl s (3-11-6 max.): 3- rectly applied.	ly applied, except -5.	
REACTIONS. (size) 1= Max Horz 1= Max Uplift 1= Max Grav 1=	=0-4-0, 7=Mechanical =30(LC 13) =-17(LC 13), 7=-17(LC 12) =1173(LC 2), 7=1173(LC 2)						
FORCES. (lb) - Max. Comp TOP CHORD 1-3=-1897/2 BOT CHORD 1-10=-158/2 WEBS 3-10=0/516	/Max. Ten All forces 250 (lb) 254, 3-4=-1635/257, 4-5=-1635/ 1655, 8-10=-246/2222, 7-8=-15 , 4-10=-698/102, 4-8=-698/102,	or less except when shown. 257, 5-7=-1897/254 /1655 5-8=0/516					
NOTES- 1) Unbalanced roof live loads 2) Wind: ASCE 7-16; Vult=11 MWFRS (envelope) and C to 15-7-0, Exterior(2R) 15- right exposed;C-C for men 3) TCLL: ASCE 7-16; Pr=25.0 Rough Cat C; Partially Exp 4) Unbalanced snow loads ha 5) Provide adequate drainage 6) This truss has been design 7) Refer to girder(s) for truss 8) Provide mechanical conne 9) This truss is designed in a referenced standard ANSSI 10) This truss design requires sheetrock be applied direc 11) Graphical purlin represen	have been considered for this 5mph (3-second gust) Vasd=91 -C Exterior(2E) 0-0-0 to 3-0-0, I 7-0 to 19-9-15, Interior(1) 19-9- bers and forces & MWFRS for 0 psf (roof LL: Lum DOL=1.15 F .; Ce=1.0; Cs=1.00; Ct=1.10 ave been considered for this dea e to prevent water ponding. led for a 10.0 psf bottom chord to truss connections. ction (by others) of truss to bea cocrdance with the 2018 Interna TPI 1. s that a minimum of 7/16" struct cty to the bottom chord. tation does not depict the size of	lesign. mph; TCDL=6.0psf; BCDL=4.2p iterior(1) 3-0-0 to 5-9-0, Exterior 5 to 21-4-0 zone; cantilever left eactions shown; Lumber DOL= ate DOL=1.15); Pf=20.0 psf (Lu ign. ive load nonconcurrent with any ing plate capable of withstandin tional Residential Code sections ural wood sheathing be applied of r the orientation of the purlin alc	esf; h=20ft; Cat. II; Ex r(2R) 5-9-0 to 9-11-19 and right exposed ; e 1.60 plate grip DOL= m DOL=1.15 Plate D other live loads. g 100 lb uplift at joint s R502.11.1 and R80 directly to the top cho	p C; Enclosed; 5, Interior(1) 9- end vertical left 1.60 9OL=1.15); Is=1 (s) 1, 7. 2.10.2 and ord and 1/2" gyp ttom chord.	11-15 and I.0; osum	STATE OF M. ANDRET THOMA JOHNSO PE-201701	ENGLAS

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a truss system and to use the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPI1 Quality Criteria**, **DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

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Job	Truss	Truss Type	Qty	Ply	Summit/17 Woodside
					143733214
2544696	C01	HIP GIRDER	1	2	
					Job Reference (optional)
Builders First Source, Valley Center	er, KS 67147				8.240 s Apr 4 2020 MiTek Industries, Inc. Mon Nov 23 16:05:37 2020 Page 2

ID:wH4RYhEsTNeUP2dXvOfi1syQY8e-u54x_daimNPMU736gIZF2obhDiaH10DKquY1XKyGFRC

NOTES-

13) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 630 lb down and 49 lb up at 0-7-4, 628 lb down and 61 lb up at 2-7-4, 712 lb down and 82 lb up at 4-7-4, 682 lb down and 102 lb up at 6-7-4, 682 lb down and 102 lb up at 8-7-4, 682 lb down and 102 lb up at 12-7-4, 799 lb down and 74 lb up at 14-7-4, 799 lb down and 74 lb up at 14-7-4, 799 lb down and 74 lb up at 14-7-4, 799 lb down and 74 lb up at 12-7-4, 799 lb down and 74 lb up at 20-7-4, 799 lb down and 74 lb up at 20-7-4, 799 lb down and 74 lb up at 20-7-4, 799 lb down and 74 lb up at 20-7-4, 799 lb down and 74 lb up at 20-7-4, 799 lb down and 65 lb up at 26-7-4, and 645 lb down and 64 lb up at 28-7-4, and 697 lb down and 63 lb up at 30-7-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-3=-80, 3-8=-80, 8-10=-80, 11-20=-20

Concentrated Loads (lb)

Vert: 18=-682(B) 17=-682(B) 21=-630(B) 22=-628(B) 23=-712(B) 24=-682(B) 25=-682(B) 26=-799(B) 27=-799(B) 28=-799(B) 29=-799(B) 30=-799(B) 31=-791(B) 32=-699(B) 33=-631(B) 34=-697(B)

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Scale = 1:61.8



	1-10-0	11-2-11	14-2-0	18-5-5	21-2-0	25-2-0)	32-0-0	<u></u>	
Plate Offsets ()	X,Y) [7:0-2-	12,0-2-0], [9:0-3-0,0-1-12], [16:0-2-8,0-	3-4]	4-3-3	2-0-11	4-0-0		0-10-0		
LOADING (psf TCLL (roof) Snow (Pf) TCDL BCLL BCDL	f) 25.0 20.0 20.0 0.0 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2018/TPI2014	CSI. TC BC WB Matrix	0.99 0.82 0.83 (-AS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.19 17-18 -0.42 17-18 0.12 10	l/defl >999 2 >900 7 n/a	L/d 240 180 n/a	PLATES MT20 MT20HS Weight: 175 lb	GRIP 197/144 148/108 FT = 20%
LUMBER- TOP CHORD BOT CHORD WEBS	2x4 SPF No.2 2x4 SPF No.2 2x4 SPF No.2	2		BR TO BC	ACING- IP CHORD	Structural wo 2-0-0 oc purli Rigid ceiling	od sheathin ins (3-4-14 r directly appl	g directly appl nax.): 4-6, 7-8 ied.	lied, except end ve	rticals, and
REACTIONS.	(size) 10 Max Horz 20 Max Uplift 10 Max Grav 10)=0-4-0, 20=0-4-0)=75(LC 13))=-83(LC 13), 20=-69(LC 12))=1754(LC 45), 20=1836(LC 45)								
FORCES. (Ib) TOP CHORD BOT CHORD) - Max. Comp. 1-2=-1436/2 6-7=-2261/3 2-18=-443/1 11-12=-344	/Max. Ten All forces 250 (lb) or less (443, 2-3=-1621/291, 3-4=-2523/406, 4-5 386, 7-8=-1687/306, 8-9=-1900/287, 1-2 10, 17-18=-413/2344, 16-17=-348/229 /2232	except when 5=-2487/441, 20=-1806/265 5, 5-16=-536	shown. 5-6=-2483/441, 5, 9-10=-1684/26 /100, 12-13=-34	;9 4/2233,					
WEBS	4-17=0/312 7-11=-934/1	, 4-16=-93/522, 13-16=-324/1973, 6-16 38, 9-11=-242/1772, 1-18=-248/1834,	=-113/731, 7· 3-18=-1224/2	-13=-390/92, 216						
NOTES- 1) Unbalanced 2) Wind: ASCE MWFRS (en 14-3-12 to 1: 31-10-4 zon- reactions shi 3) TCLL: ASCE Rough Cat (O 4) Unbalanced 5) Provide adee 6) All plates are 7) This truss is referenced s 10) This truss of sheetrock t 11) Graphical p	roof live loads 57-16; Vult=11 welope) and C 8-5-5, Exterior e; cantilever le own; Lumber I 57-16; Pr=25.0 C; Partially Exp snow loads br quate drainage e MT20 plates as been design chanical conne designed in ac standard ANSI/ design requires be applied dire purlin represen	have been considered for this design. 5mph (3-second gust) Vasd=91mph; Tr -C Exterior(2E) 0-1-12 to 3-1-12, Interior (2E) 18-5-5 to 21-2-0, Interior(1) 21-2-0 ft and right exposed ; end vertical left a DOL=1.60 plate grip DOL=1.60) psf (roof LL: Lum DOL=1.15 Plate DO ; Ce=1.0; Cs=1.00; Ct=1.10 ave been considered for this design. e to prevent water ponding. unless otherwise indicated. led for a 10.0 psf bottom chord live load ction (by others) of truss to bearing plat cocordance with the 2018 International R TPI 1. s that a minimum of 7/16" structural wood ctly to the bottom chord. tation does not depict the size or the or	CDL=6.0psf; r(1) 3-1-12 to to 25-2-0, E: and right expo L=1.15); Pf=: nonconcurre e capable of esidential Co ad sheathing ientation of th	BCDL=4.2psf; h o 11-2-11, Exteri xterior(2R) 25-2- sed;C-C for mer 20.0 psf (Lum D 20.0 psf (Lum D ode sections R50 be applied direc he purlin along th	=20ft; Cat. II; E or(2R) 11-2-11 0 to 28-2-0, Int nbers and force OL=1.15 Plate or live loads. 0 lb uplift at joir 02.11.1 and R8 tly to the top ch he top and/or b	txp C; Enclosed to 14-3-12, Int erior(1) 28-2-0 es & MWFRS fr DOL=1.15); Is= nt(s) 10, 20. 02.10.2 and nord and 1/2" g ottom chord.	d; erior(1) to or =1.0; ypsum	Ot Philos	NUMBER PE-2017018	SSOLINI SSOLINI SOUTH SOUTH SOUTH SOUTH SOUTH SOUTH SOUTH SOUTH SOUTH SOUTH SOUTH SOUTH SOUTH SOUTH SOUTH SSOLINI SSOL

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	1-10-0	7-10-5		14-2-0	15-9-5	19-9-11	23-10-0		27-10-0	32-0-0	
Plate Offcets (X V)	· 1-10-0	6-0-5 0 0-2-01 [10:Edge 0-1-9	1 [17:0-2-8 Ed	6-3-11	1-7-5	4-0-5	4-0-5		4-0-0	4-2-0	
	<u> [1.0-3-</u>	0,0-2-0], [10.Luge,0-1-0	J, [17.0-2-0,∟u	Jej, [20.0-5-0	5,0-5-0]						
LOADING (psf)TCLL (roof)2Snow (Pf)2TCDL2BCLL0DCDL4	25.0 20.0 20.0 0.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2018/T	2-0-0 1.15 1.15 YES PI2014	CSI. TC BC WB Matrii	0.68 0.64 0.60 x-AS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.15 18-19 -0.32 18-19 0.14 10	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 174 lb	GRIP 197/144 FT = 20%
BCDL 1	0.0										
LUMBER- TOP CHORD 2x BOT CHORD 2x WEBS 2x REACTIONS.	4 SPF No.2 4 SPF No.2 4 SPF No.2 (size) 10 ax Horz 22 ax Uplift 10 ax Grav 10	=0-4-0, 22=0-4-0 =69(LC 13) =-74(LC 13), 22=-59(LC =1891(LC 44), 22=2040	C 12) D(LC 44)			BRACING- TOP CHORD BOT CHORD WEBS	Structural wo 2-0-0 oc purl Rigid ceiling 1 Row at mic	ood sheat ins (3-5-1 directly a lpt	hing directly max.): 4-5 pplied. 7-11	y applied, except end ve , 7-8.	rticals, and
FORCES. (Ib) - N TOP CHORD 1 BOT CHORD 2 WEBS 2 1	Max. Comp 1-2=-1628/2 6-7=-2642/3 2-20=-1366/ 11-12=-341 2-19=-130/1 9-11=-231/1 13-17=-258/	/Max. Ten All forces 2 55, 2-3=-3090/384, 3-4 60, 7-8=-1416/241, 8-9 (225, 19-20=-327/1662, /2500 180, 3-19=-729/158, 15 769, 1-20=-262/2068, 4 (2426, 4-17=-76/350	250 (lb) or less e =-3091/464, 4-5 =-1587/231, 1-2 18-19=-302/23 -17=0/259, 5-1 -19=-110/656,	except when 5=-2368/376 22=-1970/26 37, 17-18=-2 7=-86/658, 7 7-13=-313/5	shown. , 5-6=-2600/3i 9, 9-10=-1847 242/2355, 12- ² -11=-1609/18 9, 6-13=-406/9	85, 7/254 13=-340/2502, 38, 55,					
 NOTES- 1) Unbalanced roo 2) Wind: ASCE 7 MWFRS (enveloped to the states of the states	of live loads 16; Vult=11! ope) and C- 9-5 to 18-9 nd right exp 60 plate gri 16; Pr=25.0 artially Exp ow loads ha te drainage been design- nical connec signed in ac dard ANSI/ n requires t oplied direct in represent	have been considered 1 5mph (3-second gust) V C Exterior(2E) 0-1-12 to 5, Interior(1) 18-9-5 to osed ; end vertical left a p DCL=1.60 psf (roof LL: Lum DOL ; Ce=1.0; Cs=1.00; Ct= ve been considered for to prevent water pondii ed for a 10.0 psf bottom ction (by others) of truss cordance with the 2018 TPI 1. hat a minimum of 7/16" ly to the bottom chord. tation does not depict th	for this design. Vasd=91mph; T(0 3-1-12, Interio 27-10-0, Exterio and right expose =1.15 Plate DO 1.10 this design. ng. o chord live load to bearing plat International R structural wood te size or the or	CDL=6.0psf; r(1) 3-1-12 t or(2R) 27-10 ed;C-C for m L=1.15); Pf= I nonconcurr e capable of esidential C d sheathing t ientation of t	BCDL=4.2ps o 13-10-11, E -0 to 30-10-0, embers and fo 20.0 psf (Lum ent with any c withstanding ode sections I be applied dire he purlin alon	f; h=20ft; Cat. II; E xterior(2E) 13-10. Interior(1) 30-10- orces & MWFRS i n DOL=1.15 Plate other live loads. 100 lb uplift at joi R502.11.1 and R5 exctly to the top cho g the top and/or b	Exp C; Enclose 11 to 15-9-5, 0 to 31-10-4 zc for reactions sh DOL=1.15); Is: nt(s) 10, 22. 02.10.2 and ord and 1/2" gy ottom chord.	d; pne; jown; =1.0; psum	0	STATE OF MI ANDREW THOMAS JOHNSON NUMBER PE-2017018	

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LOADING (ps TCLL (roof) Snow (Pf) TCDL BCLL BCDL	f) 25.0 20.0 20.0 0.0 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2018/T	2-0-0 1.15 1.15 YES Pl2014	CSI. TC BC WB Matri	0.94 0.90 0.56 x-AS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.24 -0.54 0.06	(loc) 8-10 8-10 7	l/defl >999 >625 n/a	L/d 240 180 n/a	PLATES MT20 MT20HS Weight: 140 lb	GRIP 197/144 148/108 FT = 20%
LUMBER- TOP CHORD BOT CHORD WEBS	2x4 SPF No.2 2x4 SPF No.2 2x4 SPF No.2	2 2 2				BRACING- TOP CHORD BOT CHORD WEBS	Struc 2-0-0 Rigid 1 Rov	tural wo oc purl ceiling w at mic	ood sheat ins (6-0-0 directly a lpt	hing directly a) max.): 5-6. pplied. 3-10	applied, except end ve	rticals, and
DEACTIONS	(cizo) 7-	-0 4 0 12-0 4 0										

REACTIONS. (size) 7=0-4-0, 12=0-4-0 Max Horz 12=-118(LC 14) Max Uplift 7=-64(LC 13), 12=-53(LC 12) Max Grav 7=1603(LC 40), 12=1542(LC 2)

- FORCES.
 (lb) Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown.

 TOP CHORD
 1-2=-1559/264, 2-3=-1493/327, 3-4=-2475/384, 4-5=-2446/286, 1-12=-1478/219

BOT CHORD 2-10=-639/160, 8-10=-174/1302, 7-8=-237/1647

1-10=-176/1451

3-10=-163/291, 3-8=-159/1288, 4-8=-920/202, 5-8=-18/547, 5-7=-2217/321, WFBS

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=20ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-1-12 to 3-1-12, Interior(1) 3-1-12 to 11-2-0, Exterior(2R) 11-2-0 to 14-2-0, Interior(1) 14-2-0 to 28-2-4 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) Provide adequate drainage to prevent water ponding.

6) All plates are MT20 plates unless otherwise indicated

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7, 12. 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

10) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.





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	L	6	-1-5	7-11-8	16	-2-11		22-1-9		1	28-4-0	
		6	-1-5	1-10-3	8	-3-3		5-10-15		I	6-2-7	1
Plat	te Offsets (2	X,Y) [8:0-3·	-0,0-1-12], [10:0-4	-0,0-2-4], [11:0-8-0,0	0-5-0], [12:0-8	-8,Edge], [15	5:0-4-8,0-3-8]				-	
LO/ TCL Sno TCL BCL	ADING (ps _L (roof) ww (Pf) DL _L	f) 25.0 20.0 20.0 0.0	SPACING- Plate Grip I Lumber DC Rep Stress Code IRC2	2-0-0 DOL 1.15 DL 1.15 Incr NO 2018/TPI2014	CSI. TC BC WB Matr	0.74 0.89 0.79 ix-MS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc -0.26 11-12 -0.53 11-12 0.11) I/defl 2 >999 2 >630 9 n/a	L/d 240 180 n/a	PLATES MT20 MT20HS Weight: 338 lb	GRIP 197/144 148/108 FT = 20%
BCI	DL	10.0									Trongina coo io	
LUN TOF BO ⁻	MBER- P CHORD T CHORD	2x4 SPF No.2 6-8: 2x4 SPF 2x6 SPF 210 13-15: 2x6 SF	2 *Except* 1650F 1.5E 0F 1.8E *Except* 2F No 2 4-13: 2x4	SPE No 2			BRACING- TOP CHORD BOT CHORD	Structural v end vertica Rigid ceilin 6-0-0 oc br	vood sheat Is, and 2-0 g directly a acing: 13-1	thing directly -0 oc purlins applied or 10-	applied or 4-2-9 oc purl (4-0-7 max.): 3-6. -0-0 oc bracing, Except	ins, except t:
WE	BS	2x4 SPF No.2 8-10: 2x4 SPI	2 *Except* F 1650F 1.5E				WEBS	1 Row at m	iidpt	2-15		
RE/	ACTIONS.	(lb/size) 15 Max Horz 15 Max Uplift 15 Max Grav 15	5=6466/0-6-0, 9=6 5=-129(LC 57) 5=-587(LC 8), 9=-5 5=6576(LC 36), 9=	816/0-4-0 557(LC 9) 7064(LC 36)								
FOF TOF	R CES. (Ib P CHORD) - Max. Comp 2-3=-5081/5	./Max. Ten All fo 525, 3-4=-6562/67	rces 250 (lb) or less 8, 4-5=-6634/685, 5	except when -16=-8161/84	shown. 1, 6-16=-815	8/842,					
BO	T CHORD	6-7=-8701/8 15-18=-248 12-20=-701 11-24=-742 27-28=-51/5	274, 7-17=-9271/8 /2940, 18-19=-248 /7640, 20-21=-701 2/8709, 24-25=-74 520, 28-29=-51/52	15, 8-1/=-9418/801 3/2940, 14-19=-248/ /7640, 21-22=-701/ 2/8709, 25-26=-742 0, 9-29=-51/520	, 8-9=-5658/4 2940, 13-14= 7640, 22-23= /8709, 10-26=	92 -462/113, 4-1 -701/7640, 1 -742/8709, 1	12=-489/61, 1-23=-701/7640, 0-27=-51/520,					
VVE	DO	2-14=-366/2 5-11=-182/1 8-10=-713/8	1212, 6-11=-242/2 3444	677, 7-11=-770/37,	96, 3-12=-012 7-10=-293/22	5, 2-15=-637	1727/185, 7/611,					
NO 1) 2 T E V 2) A p 3) L 4) V 5) T F 6) L 7) F 8) A 9) T 10)	TES- -ply truss to cop chords Bottom chor Vebs connection All loads are Jubalanced Vind: ASCE JWFRS (er CLL: ASCE AWFRS (er CLL: ASCE August Cat Cubalanced Provide ade All plates ar This truss ha Bearing at Bearing at tages to on p	o be connected connected as f rds connected as ected as follows e considered ec ons have been roof live loads E 7-16; Vult=11 twelope); cantil E 7-16; Pr=25.0 C; Partially Exp snow loads ha quate drainage e MT20 plates as been desigr joint(s) 15, 9 c f bearing surfac age 2	d together with 10c follows: 2x4 - 1 rov as follows: 2x6 - 2 s: 2x4 - 1 row at 0- qually applied to al provided to distrit have been conside 5mph (3-second g ever left and right 1 0 psf (roof LL: Lurr 0, ce=1.0; Cs=1.0 ave been consider to prevent water unless otherwise i led for a 10.0 psf to onsiders parallel to ce.	4 (0.131"x3") nails av v at 0-4-0 oc. rows staggered at 0 9-0 oc. I plies, except if not voute only loads note lered for this design (ust) Vasd=91mph; exposed ; end vertic to DOL=1.15 Plate D 0; Ct=1.10 ed for this design. ponding. ndicated. portom chord live load o grain value using /	s follows: -5-0 oc, 2x4 - ed as front (F) d as (F) or (B) TCDL=6.0psf; al left and rig OL=1.15); Pf= ad nonconcurr ANSI/TPI 1 ar	1 row at 0-9 or back (B) f , unless othe BCDL=4.2ps ht exposed; L 20.0 psf (Lur rent with any igle to grain f	-0 oc. face in the LOAD C erwise indicated. sf; h=20ft; Cat. II; E .umber DOL=1.60 m DOL=1.15 Plate other live loads. formula. Building d	ASE(S) secti Exp C; Enclos plate grip DO DOL=1.15); I esigner shou	on. Ply to ed; L=1.60 s=1.0; d verify		NUMBER PE-20170189 November 23,	SOLIN 93 NGTTO 2020
·com	WARNING Design valid a truss syste building desi is always rec fabrication, s Safety Inform	aye 2 G - Verify design pa for use only with N m. Before use, the gn. Bracing indica quired for stability a torage, delivery, er mation available f	rameters and READ NO liTek® connectors. This building designer must ted is to prevent bucklir and to prevent collapse rection and bracing of tr from Truss Plate Institu	TES ON THIS AND INCL s design is based only up verify the applicability of g of individual truss web with possible personal inj usses and truss systems te, 2670 Crain Highway, 1	UDED MITEK REI on parameters sh design paramete and/or chord mer ury and property , see AI Suite 203 Waldor	FERENCE PAGE own, and is for a rs and properly in mbers only. Add damage. For ge NSI/TPI1 Quality f, MD 20601	MII-7473 rev. 5/19/2020 an individual building con ncorporate this design in litional temporary and pe- neral guidance regardin r criteria, DSB-89 and	BEFORE USE. mponent, not nto the overall ermanent bracing g the BCSI Building C	omponent		16023 Swingley Ridge R Chesterfield, MO 63017	d

Job	Truss	Truss Type	Qty	Ply	Summit/17 Woodside
					143733221
2544696	C08	HIP GIRDER	1	2	
					Job Reference (optional)
Builders First Source, Valley Center, KS 67147					8.240 s Apr 4 2020 MiTek Industries, Inc. Mon Nov 23 16:06:09 2020 Page 2

ID:wH4RYhEsTNeUP2dXvOfi1syQY8e-yF?1iZznECRpK2WGY17lLYwCzbP5XXD2BtljeTyGFQi

NOTES-

- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 587 lb uplift at joint 15 and 557 lb uplift at joint 9.
- 12) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 14) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 718 lb down and 69 lb up at 1-8-12, 802 lb down and 70 lb up at 3-8-12, 777 lb down and 87 lb up at 5-8-12, 803 lb down and 88 lb up at 7-9-12, 742 lb down and 109 lb up at 9-8-12, 742 lb down and 109 lb up at 13-8-12, 742 lb down and 109 lb up at 13-8-12, 742 lb down and 109 lb up at 13-8-12, 742 lb down and 109 lb up at 13-8-12, 742 lb down and 97 lb up at 13-8-12, 704 lb down and 70 lb up at 13-8-12, 704 lb down and 58 lb up at 23-8-12, and 758 lb down and 60 lb up at 25-8-12, and 805 lb down and 56 lb up at 27-8-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-3=-80, 3-6=-80, 6-8=-80, 13-15=-20, 9-12=-20

Concentrated Loads (lb)

Vert: 13=-803(B) 14=-777(B) 18=-718(B) 19=-802(B) 20=-742(B) 21=-742(B) 22=-742(B) 23=-742(B) 24=-814(B) 25=-698(B) 26=-628(B) 27=-704(B) 28=-758(B) 29=-805(B) 29=-805(B) 20=-742(B) 24=-814(B) 25=-698(B) 26=-628(B) 27=-704(B) 28=-758(B) 29=-805(B) 20=-742(B) 20=-7

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Continued on page 2

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MITEK[®] 16023 Swingley Ridge Rd Chesterfield, MO 63017

Job	Truss	Truss Type	Qty	Ply	Summit/17 Woodside	
					14373	33222
2544696	D01	HALF HIP GIRDER	1	1		
					Job Reference (optional)	
Builders FirstSource (Valley	Center), Valley Center, K	S - 67147,	8.	240 s Mar	9 2020 MiTek Industries, Inc. Mon Nov 23 09:00:24 2020 Page	2

ID:wH4RYhEsTNeUP2dXvOfi1syQY8e-E2wPcp2n6QPCBh1mq4QPA7QmeFyrnOP274rVZbyGLfr

LOAD CASE(S) Standard

Uniform Loads (plf)

Vert: 1-4=-80, 4-6=-80, 7-10=-20

Concentrated Loads (lb) Vert: 14=32(B) 15=-95(B) 16=-95(B) 17=-95(B) 20=-244(B) 21=-47(B) 22=-47(B) 23=-47(B) 24=-47(B)

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- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 7.
- 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 10) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



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LOADING (psf TCLL (roof) Snow (Pf) TCDL BCLL BCDL) 25.0 20.0 20.0 0.0 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2018/TF	2-0-0 1.15 1.15 YES 212014	CSI. TC 0.8 BC 0.3 WB 0.6 Matrix-AS	89 33 60 S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.05 -0.09 0.01	(loc) 6-7 6-7 2	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 MT20HS Weight: 45 lb	GRIP 197/144 148/108 FT = 20%
LUMBER- TOP CHORD BOT CHORD WEBS SLIDER	 2x4 SPF No.2 2x4 SPF No.2 2x4 SPF No.2 Left 2x4 SPF No.2 1-6-0 				E F	BRACING- TOP CHORD BOT CHORD	Structural wood sheathing directly applied, except end verticals, and 2-0-0 oc purlins (4-10-14 max.): 4-5. Rigid ceiling directly applied.					
REACTIONS.	i. (size) 2=0-4-0, 6=0-4-0 Max Horz 2=98(LC 15) Max Uplift 2=-26(LC 16), 6=-42(LC 13) Max Grav 2=830(LC 38), 6=748(LC 37)											
FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 2-4=-851/196, 5-6=-393/105												

BOT CHORD 2-7=-254/701.6-7=-256/695

WEBS 4-6=-681/231

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=20ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 5-6-0, Exterior(2R) 5-6-0 to 9-8-15, Interior(1) 9-8-15 to 11-10-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.
- 7) All plates are MT20 plates unless otherwise indicated.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6. 10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1
- 11) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.





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 Satisfies
 Ansi/TPH Qu

 Safety Information
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BOT CHORD 2-10=-220/612, 9-10=-135/407, 7-8=-117/271, 6-7=-252/678

WEBS 4-6=-798/255

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=20ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 7-0-0, Exterior(2R) 7-0-0 to 11-2-15, Interior(1) 11-2-15 to 12-1-12 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
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- 6) Provide adequate drainage to prevent water ponding.
- 7) All plates are MT20 plates unless otherwise indicated.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) Refer to girder(s) for truss to truss connections.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6, 2.
- 11) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 12) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
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MWFRS (envelope) and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 12-1-12 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 2) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0;

Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

3) Unbalanced snow loads have been considered for this design.

4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) Refer to girder(s) for truss to truss connections.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6.

8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and

- referenced standard ANSI/TPI 1.
- 9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



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- MWFRS (envelope) and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 11-10-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0;
- Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 13.
- 8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and
- referenced standard ANSI/TPI 1.
- 9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.





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November 23,2020



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 Safety Information
 available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



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LOADING (ps TCLL (roof) Snow (Pf) TCDL BCLL BCDL	f) 25.0 20.0 20.0 0.0 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2018/TPl2	2-0-0 1.15 1.15 NO 2014	CSI. TC BC WB Matri:	0.95 0.96 0.60 ĸ-MS		DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.19 -0.30 0.08	(loc) 20-21 20-21 16	l/defl >833 >522 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 175 lb	GRIP 197/144 FT = 20%
LUMBER- TOP CHORD	2x4 SPF No.2 14-17: 2x6 SF	2 *Except* PF No.2				BRA TOP	CING- CHORD	Struct	tural wo	od sheath	hing directly a	applied or 4-1-11 oc pu	rlins,
BOT CHORD	2x4 SPF No.2 *Except* 15-22: 2x4 SPF 1650F 1.5E 2x4 SPF No.2					вот	CHORD	2-0-0 Rigid 10-0-0	2-0-0 oc purins (2-6-8 max.): 5-7, 8-9, 10-14. Rigid ceiling directly applied or 5-5-8 oc bracing. Except: 10-0-0 oc bracing: 19-20				
WEDGE Right: 2x4 SPF	No.2					WEB	S	1 Rov	v at mid	pt	13-22		
SLIDER	Left 2x4 SPF	No.2 1-6-0											
REACTIONS. All bearings 0-4-0 except (jt=length) 24=0-4-6 (input: 0-4-0 + bearing block). (lb) - Max Horz 2=-44(LC 121) Max Uplift All uplift 100 lb or less at joint(s) 2 except 16=-139(LC 13), 30=-175(LC 123), 24=-199(LC 13) Max Grav All reactions 250 lb or less at joint(s) except 2=759(LC 46), 16=1057(LC 45), 30=1629(LC 97), 24=2785(LC 45)													
FORCES. (Ib) - Max. Comp.	Max. Ten All forces 250) (lb) or less e	xcept when	shown.	470/40							

 TOP CHORD
 2-4=-804/66, 4-5=-604/65, 5-6=-521/73, 6-7=-474/146, 7-8=-557/170, 8-9=-476/191, 9-10=-623/191, 10-11=-176/2369, 11-12=-48/782, 12-13=-22/480, 13-14=-3348/335, 14-15=-2982/352, 15-16=-892/137

 BOT CHORD
 2-32=-57/672, 31-32=-57/672, 30-31=-268/84, 28-30=-268/84, 27-28=-84/430,

 WEBS
 4-31=-26/205, 6-31=-75/790, 6-30=-1489/227, 6-28=-182/836, 7-28=-605/120, 8-28=-46/393, 9-27=-315/63, 10-27=-114/1242, 13-22=-3858/364, 13-21=-41/527, 11-24=-971/101, 22-24=-2297/212, 11-22=-151/1734, 10-24=-1609/96, 14-20=-71/759, 14-21=-62/468

NOTES-

1) 2x4 SPF No.2 bearing block 12" long at jt. 24 attached to front face with 2 rows of 10d (0.131"x3") nails spaced 3" o.c. 8 Total fasteners. Bearing is assumed to be SPF No.2.

2) Unbalanced roof live loads have been considered for this design.

- 3) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=20ft; Cat. II; Exp C; Enclosed; MWFRS (envelope); cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 4) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

Torrende advance drainage to prevent water ponding

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Job	Truss	Truss Type	Qty	Ply	Summit/17 Woodside	
						43733236
2544696	D15	ROOF SPECIAL GIRDER	1	1		
					Job Reference (optional)	
Builders FirstSource (Valley	Center), Valley Center, K	S - 67147,	8.	240 s Mar	9 2020 MiTek Industries, Inc. Mon Nov 23 09:00:48 2020 F	Page 2

ID:wH4RYhEsTNeUP2dXvOfi1syQY8e-XfMUe0KqSoRo4NsNE6r18BQoDw5BPEAaepfnPCyGLfT

NOTES-

8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 except (jt=lb) 16=139, 30=175, 24=199.

10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

12) Use Simpson Strong-Tie LUS24 (4-10d Girder, 2-10d Truss, Single Ply Girder) or equivalent spaced at 2-0-0 oc max. starting at 29-11-4 from the left end to 35-11-4 to connect truss(es) to front face of bottom chord.

13) Fill all nail holes where hanger is in contact with lumber.

14) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.

15) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 244 lb down and 39 lb up at 37-9-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

16) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-5=-80, 5-7=-80, 7-8=-80, 8-9=-80, 9-10=-80, 10-14=-80, 14-17=-80, 23-33=-20, 19-22=-20, 18-40=-20

Concentrated Loads (lb) Vert: 19=-244(F) 48=-95(F) 49=41(F) 51=41(F) 53=41(F) 54=41(F) 55=32(F) 56=-47(F) 57=-232(F) 58=-232(F) 59=-232(F) 60=-232(F) 60=-

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	4-3-	-0	11-10-0			15-11-0	17-5-0	21-2-0		26-11-0			
Plate Offsets (X	(,Y) [2:0-4-	-0 -1,0-0-1], [11:0-4-13,Ec	[ge]			4-1-0	1-0-0	3-9-0		5-9-0			
LOADING (psf TCLL (roof) Snow (Pf) TCDL BCLL BCDL) 25.0 20.0 20.0 0.0 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2018/	2-0-0 1.15 1.15 YES TPI2014	CSI. TC BC WB Matrix-	0.51 0.39 0.55 -AS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.07 15-16 -0.14 15-16 0.02 11	l/defl >999 >990 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 105 lb	GRIP 197/144 FT = 20%		
LUMBER- TOP CHORD BOT CHORD WEBS SLIDER	2x4 SPF No.2 2x4 SPF No.2 2x4 SPF No.2 Left 2x4 SPF	2 2 2 No.2 2-6-0, Right 2x4 3	SPF No.2 2-6-0			BRACING- TOP CHORD BOT CHORD	Structural wo 2-0-0 oc purl Rigid ceiling	ood sheat lins (6-0-0 directly a	hing directl) max.): 4-7 pplied.	y applied, except , 8-9.			
REACTIONS.	(size) 11 Max Horz 2= Max Uplift 11 Max Grav 11	=0-4-0, 2=0-4-0, 15=0 =37(LC 20) =-31(LC 17), 2=-47(LC =832(LC 46), 2=692(L	-4-0 C 16), 15=-59(LC C 46), 15=1996(L	16) _C 45)									
FORCES. (Ib) TOP CHORD BOT CHORD WEBS	ORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. OP CHORD 2-4=-600/119, 4-5=-547/136, 5-6=-1/675, 6-7=-1/678, 7-8=-747/192, 8-9=-848/218, 9-11=-967/196 OT CHORD 2-16=-68/543, 15-16=-62/300, 13-15=-106/596, 12-13=-85/636, 11-12=-109/845 VEBS 6-15=-525/90, 7-15=-1355/202, 8-12=-48/279, 5-16=-13/343, 5-15=-1152/162												
NOTES- 1) Unbalanced 2) Wind: ASCE MWFRS (em 17-5-0, Exter cantilever lef Lumber DOL 3) TCLL: ASCE Rough Cat C 4) Unbalanced 5) This truss ha non-concurre 6) Provide adec 7) This truss ha 8) Provide meci 9) This truss is referenced si 10) This truss of sheetrock b 11) Graphical p	roof live loads 7-16; Vult=11 velope) and C rior(2R) 17-5-0 t and right exp =1.60 plate gr :7-16; Pr=25.0 C; Partially Exp snow loads ha is been design ent with other I quate drainage is been design hanical conne designed in at tandard ANSI/ design requires be applied dire burlin represen	have been considered 5mph (3-second gust) -C Exterior(2E) -0-10-8 0 to 20-5-0, Interior(1) 2 oosed ; end vertical left ip DOL=1.60 0 psf (roof LL: Lum DOI op sf (roof LL: Lum DOI v; Ce=1.0; Cs=1.00; Ct ave been considered fo teed for greater of min ro ive loads. a to prevent water pond ted for a 10.0 psf bottoo ction (by others) of trus ccordance with the 201 TPI 1. s that a minimum of 7/1 ctly to the bottom chord tation does not depict t	for this design. Vasd=91mph; TC to 2-1-8, Interior 0-5-0 to 21-2-0, I and right expose _=1.15 Plate DOL =1.10 r this design. of live load of 12 ing. n chord live load s to bearing plate 8 International Re 6" structural wood. he size or the original	CDL=6.0psf; E (1) 2-1-8 to 4 Exterior(2R) 2 d;C-C for me _=1.15); Pf=2 .0 psf or 1.00 nonconcurre capable of v esidential Coo d sheathing t entation of th	3CDL=4.2p -3-0, Exteri 21-2-0 to 24 mbers and 0.0 psf (Lu 0 times flat i nt with any withstanding de sections be applied o e purlin alo	sf; h=20ft; Cat. II; ior(2R) 4-3-0 to 7- 4-2-0, Interior(1) 2- forces & MWFRS m DOL=1.15 Plate roof load of 20.0 p other live loads. g 100 lb uplift at jo R502.11.1 and R directly to the top of ing the top and/or l	Exp C; Enclose 3-0, Interior(1) 7 4-2-0 to 26-11-(for reactions sh a DOL=1.15); Is sf on overhangs int(s) 11, 2, 15. 802.10.2 and shord and 1/2" (pottom chord.	d; 7-3-0 to 0 zone; 100wn; =1.0; s	C	ANDREW THOMAS JOHNSON NUMBER PE-2017018	SSOLIN-		

November 23,2020

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Plate Offsets (X,Y) [2:0-4	-13,Edge], [12:0-4-1,0-0-1]						
LOADING (psf) TCLL (roof) 25.0 Snow (Pf) 20.0 TCDL 20.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr NO Code IRC2018/TPI2014	CSI. TC 0.85 BC 0.56 WB 0.41 Matrix-MS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) l/defl -0.05 18-19 >999 -0.10 18-19 >999 0.04 12 n/a	L/d 240 180 a n/a	PLATES MT20 Weight: 107 lb	GRIP 197/144 FT = 20%
LUMBER- TOP CHORD 2x4 SPF No. BOT CHORD 2x4 SPF No. WEBS 2x4 SPF No. SLIDER Left 2x4 SPF REACTIONS. (size) 12 Max Horz 22 Max Uplift 12 Max Grav 12	2 2 2 No.2 2-6-0, Right 2x4 SPF No.2 2-6-0 2=0-4-0, 2=0-4-0, 16=0-4-0 =30(LC 111) 2=-38(LC 108), 2=-110(LC 12), 16=-100 2=726(LC 86), 2=988(LC 42), 16=2506(L	(LC 8) .C 41)	BRACING- TOP CHORD BOT CHORD WEBS	Structural wood she 2-0-0 oc purlins (4- Rigid ceiling directly 6-0-0 oc bracing: 15 1 Row at midpt	sathing directly a 3-14 max.): 4-7, / applied or 10-0 5-16. 5-16	applied or 4-9-2 oc purl 8-10. I-0 oc bracing, Excep	ins, except
FORCES. (lb) - Max. Comp TOP CHORD 2-4=-1259/ 8-9=-495/13 BOT CHORD 2-19=-123/ 13-14=-49// WEBS 4-19=0/319 9-15=-761//	./Max. Ten All forces 250 (lb) or less e 132, 4-5=-1097/140, 5-6=-32/1225, 6-7= 39, 9-10=-1061/133, 10-12=-974/89 1117, 18-19=-106/1227, 16-18=-106/122 878, 12-13=-47/882 , 5-18=0/269, 5-16=-2574/145, 6-16=-59 41, 10-14=-83/267	xcept when shown. -31/1225, 7-8=-552/157, ?7, 15-16=-186/273, 14-1 96/84, 7-16=-1449/34, 7-1	5=-101/1058, 15=-13/668,				
NOTES- 1) Unbalanced roof live loads 2) Wind: ASCE 7-16; Vult=11 MWFRS (envelope); cantil 3) TCLL: ASCE 7-16; Pr=25. Rough Cat C; Partially Exy 4) Unbalanced snow loads hi 5) This truss has been design non-concurrent with other 6) Provide adequate drainagy 7) This truss has been design 8) Provide mechanical conner (it=lb) 2=110. 9) This truss is designed in a referenced standard ANSI 10) Graphical purlin represer 11) Use Simpson Strong-Tie truss(es) to front face of II 2) Fill all nail holes where h 13) "NAILED" indicates 3-100 14) In the LOAD CASE(S) set	thave been considered for this design. 5mph (3-second gust) Vasd=91mph; TC ever left and right exposed ; end vertical 0 psf (roof LL: Lum DOL=1.15 Plate DOI); Ce=1.0; Cs=1.00; Ct=1.10 ave been considered for this design. ted for greater of min roof live load of 12 live loads. a to prevent water ponding. ted for a 10.0 psf bottom chord live load ction (by others) of truss to bearing plate ccordance with the 2018 International Re (TPI 1. ttation does not depict the size or the ori LUS24 (4-10d Girder, 2-10d Truss, Sing bottom chord. anger is in contact with lumber. d (0.148"x3") or 3-12d (0.148"x3.25") toe ction, loads applied to the face of the true	CDL=6.0psf; BCDL=4.2ps left and right exposed; L _=1.15); Pf=20.0 psf (Lun .0 psf or 1.00 times flat ro nonconcurrent with any of e capable of withstanding esidential Code sections entation of the purlin alor le Ply Girder) or equivale -nails per NDS guidlines. iss are noted as front (F)	if; h=20ft; Cat. II; E umber DOL=1.60 p n DOL=1.15 Plate I pof load of 20.0 psf bther live loads. 100 lb uplift at join R502.11.1 and R80 ng the top and/or ba ent at 2-0-12 from the or back (B).	xp C; Enclosed; olate grip DOL=1.60 DOL=1.15); Is=1.0; f on overhangs at(s) 12, 16 except 02.10.2 and ottom chord. he left end to connec	t	ANDREW THOMAS JOHNSON NUMBER PE-2017018 November 23	993 ENGT
Communications pageszandard							

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Job	Truss	Truss Type	Qty	Ply	Summit/17 Woodside	
					4	43733238
2544696	D17	ROOF SPECIAL GIRDER	1	1		
					Job Reference (optional)	
Builders FirstSource (Valley	Center), Valley Center, K	S - 67147,	8.	240 s Mar	9 2020 MiTek Industries, Inc. Mon Nov 23 09:00:55 2020 P	Page 2

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LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-4=-80, 4-7=-80, 7-8=-80, 8-10=-80, 10-12=-80, 20-24=-20

Concentrated Loads (lb)

Vert: 16=-51(F) 6=-96(F) 3=32(F) 28=-95(F) 29=-95(F) 30=-95(F) 32=-95(F) 36=-244(F) 37=-47(F) 38=-47(F) 39=-47(F) 40=-47(F) 40

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- EACTIONS. (SIZE) 8=0-4-0, 14=0-4-0 Max Horz 14=80(LC 13) Max Uplift 8=-46(LC 13), 14=-20(LC 16) Max Grav 8=883(LC 37), 14=837(LC 38)
- FORCES. (Ib) Max. Comp./Max. Ten. All forces 250 (Ib) or less except when shown.
- TOP CHORD 2-3=-554/122, 3-4=-1555/291, 4-5=-1367/295, 5-6=-1025/152, 6-7=-937/150,
- 7-8=-846/156, 2-14=-818/220
- BOT CHORD 13-14=-158/264, 3-12=-187/1147, 11-12=-346/1396, 10-11=-354/1704, 6-10=-358/106
- WEBS 4-11=-13/375, 7-10=-207/1179, 5-11=-373/92, 5-10=-745/209

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=20ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 4-1-14, Exterior(2R) 4-1-14 to 8-4-13, Interior(1) 8-4-13 to 13-3-4 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8, 14.
 This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and
- referenced standard ANSI/TPI 1. 10) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum
- sheetrock be applied directly to the bottom chord.
- 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.





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		-4-0	4	-7-14 -3-14			<u>11-1-0</u> 6-5-2					2-3-0	
Plate Offsets (X,Y) [2:0-0-	14,0-1-12], [3:0-2	<u>۔</u> 2-14,0-0-8	3], [9:0-3-0,0-0)-12], [9:0-4	-8,0-1-8],	[12:0-3-0,0-2-8], [14:	0-0-0,0-	1-12]			200	
LOADING (ps TCLL (roof) Snow (Pf) TCDL BCLL BCDL	sf) 25.0 20.0 20.0 0.0 10.0	SPACING Plate Grip Lumber D Rep Stres Code IRC	DOL OL s Incr 2018/TPI	2-0-0 1.15 1.15 YES 2014	CSI. TC BC WB Matri	0.49 0.69 0.48 x-AS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.08 -0.17 0.05	(loc) 10-11 10-11 18	l/defl >999 >929 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 53 lb	GRIP 197/144 FT = 20%
LUMBER- TOP CHORD BOT CHORD WEBS OTHERS	2x4 SPF No.2 2x4 SPF No.2 2x4 SPF No.2 2x4 SPF No.2 2x4 SPF No.2						BRACING- TOP CHORD BOT CHORD	Struc 2-0-0 Rigid	tural wo oc purl ceiling	ood sheat ins (4-10 directly a	hing directly -13 max.): 4- pplied.	applied, except end v 6.	erticals, and

REACTIONS. (size) 14=0-4-0, 18=Mechanical Max Horz 14=67(LC 13) Max Uplift 14=-19(LC 16), 18=-41(LC 13) Max Grav 14=860(LC 38), 18=820(LC 37)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

- TOP CHORD 2-3=-587/101, 3-4=-1437/261, 4-5=-1270/276, 5-6=-270/0, 6-9=-86/578, 2-14=-839/217
- BOT CHORD 13-14=-153/299, 3-12=-141/1031, 11-12=-292/1288, 10-11=-265/1412, 9-10=-213/1432

WEBS 4-11=0/296, 5-9=-1241/316, 6-18=-844/133

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=20ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 4-7-14, Exterior(2R) 4-7-14 to 8-8-7, Interior(1) 8-8-7 to 12-10-12 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) Refer to girder(s) for truss to truss connections.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 14, 18.
- 10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 11) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.





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- FORCES. (lb) Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown.
- TOP CHORD 2-3=-1342/299, 3-4=-1036/223, 4-5=-888/220, 6-9=-106/573, 2-14=-933/186
- BOT CHORD 2-12=-274/990, 11-12=-380/1156, 10-11=-191/836, 9-10=-166/843
- WEBS 5-11=-58/279, 5-9=-844/225, 3-11=-377/167, 6-15=-776/141

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=20ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 6-1-14, Exterior(2R) 6-1-14 to 10-4-13, Interior(1) 10-4-13 to 12-10-12 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
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- 6) Provide adequate drainage to prevent water ponding.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) Refer to girder(s) for truss to truss connections.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 14, 15.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 11) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.





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NOTES-

- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=20ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 13-4-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0;
- Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 9.
- 8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and
- referenced standard ANSI/TPI 1.
- 9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



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- 3-6=0/289, 3-5=-855/227
- WEBS

NOTES-

- 1) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=20ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-0-0 to 3-0-0, Interior(1) 3-0-0 to 13-2-4 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0;
- Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) Refer to girder(s) for truss to truss connections.

- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5.
- 7) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and

referenced standard ANSI/TPI 1.

8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



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11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



November 23,202

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E. I cing g Component Chesterfield, MO 63017

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	2-9-0	7-7-8		12-6-0	15-3-0
	2-9-0	4-10-8		4-10-8	2-9-0
LOADING (pst) TCLL (roof) 25.0 Snow (Pf) 20.0 TCDL 20.0 BCLL 0.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr NO Code IRC2018/TPI2014	CSI. TC 0.72 BC 0.95 WB 0.42 Matrix-MS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) I/defl L -0.13 10 >999 24 -0.24 10-11 >770 18 0.04 8 n/a n	/d PLATES GRIP 10 MT20 197/144 10 /a Weight: 59 lb FT = 20%
BCDL 10.0					
LUMBER- TOP CHORD 2x4 SPF No.2 4-6: 2x4 SPF BOT CHORD 2x4 SPF No.2 WEBS 2x4 SPF No.2 SLIDER Left 2x4 SPF	? *Except* 1650F 1.5E ? ? No.2 2-6-0, Right 2x4 SPF No.2 2-6	В Т В О	BRACING- TOP CHORD BOT CHORD	Structural wood sheathing except 2-0-0 oc purlins (3-0-0 ma: Rigid ceiling directly applie	directly applied or 3-0-14 oc purlins, x.): 4-6. d or 10-0-0 oc bracing.
REACTIONS. (size) 8= Max Horz 2= Max Uplift 8= Max Grav 8=	.0-4-0, 2=0-4-0 :22(LC 85) :-119(LC 8), 2=-121(LC 9) :1372(LC 36), 2=1465(LC 37)				
FORCES. (lb) Max. Comp. TOP CHORD 2-4=-2253/1 BOT CHORD 2-11=-161/1 WEBS 4-11=0/260,	/Max. Ten All forces 250 (lb) or les 74, 4-5=-3599/245, 5-6=-3599/245, 982, 10-11=-164/1964, 9-10=-149/1 , 4-10=-90/1730, 5-10=-924/126, 6-1	s except when shown. 6-8=-2261/174 972, 8-9=-147/1991 0=-89/1721, 6-9=0/262			
 NOTES- 1) Unbalanced roof live loads 2) Wind: ASCE 7-16; Vult=111 MWFRS (envelope); cantile 3) TCLL: ASCE 7-16; Pr=25.0 Rough Cat C; Partially Exp 4) Unbalanced snow loads hat 5) This truss has been design non-concurrent with other li 6) Provide adequate drainage 7) This truss has been design 8) Provide mechanical connect 8=119, 2=121. 9) This truss is designed in ac referenced standard ANSI/ 10) Graphical putlin represent 11) Use Simpson Strong-Tie I 2-0-12 from the left end to 12) Fill all nail holes where hat 13) "NAILED" indicates 3-10d 14) In the LOAD CASE(S) Standard 	have been considered for this desig 5mph (3-second gust) Vasd=91mph; aver left and right exposed ; end vert 1) psf (roof LL: Lum DOL=1.15 Plate II ; Ce=1.0; Cs=1.00; Ct=1.10 ive been considered for this design. ed for greater of min roof live load of ive loads. • to prevent water ponding. ed for a 10.0 psf bottom chord live lo ction (by others) of truss to bearing p excordance with the 2018 Internationa TPI 1. tation does not depict the size or the LUS24 (4-10d Girder, 2-10d Truss, §) 13-2-4 to connect truss(es) to back inger is in contact with lumber. I (0.148"x3") or 3-12d (0.148"x3.25") ction, loads applied to the face of the	n. TCDL=6.0psf; BCDL=4.2psf; ical left and right exposed; Lur DOL=1.15); Pf=20.0 psf (Lum 12.0 psf or 1.00 times flat roc vad nonconcurrent with any ot late capable of withstanding 1 I Residential Code sections R orientation of the purlin along single Ply Girder) or equivalen face of bottom chord. toe-nails per NDS guidlines. truss are noted as front (F) o	h=20ft; Cat. II; E mber DOL=1.60 p DOL=1.15 Plate of load of 20.0 pst her live loads. 00 lb uplift at join 502.11.1 and R8 the top and/or but t spaced at 11-1- r back (B).	xp C; Enclosed; plate grip DOL=1.60 DOL=1.15); Is=1.0; tron overhangs t(s) except (jt=lb) D2.10.2 and bttom chord. 8 oc max. starting at	ANDREW THOMAS JOHNOO PE-2017018993

Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



November 23,2020

Job	Truss	Truss Type	Qty	Ply	Summit/17 Woodside	
					14373325	j2
2544696	G01	HIP GIRDER	1	1		
					Job Reference (optional)	
Builders FirstSource (Valley	Center), Valley Center, K	S - 67147,	8.	240 s Mar	9 2020 MiTek Industries, Inc. Mon Nov 23 09:01:16 2020 Page 2	

9 2020 MiTek Industries, Inc. Mon Nov 23 09:01:16 203 10 s Mar ID:wH4RYhEsTNeUP2dXvOfi1syQY8e-i233WbgOsPzqRg?lt?LbH4adiDgyzyoi4ARFObyGLf1

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-4=-80, 4-6=-80, 6-8=-80, 12-16=-20

Concentrated Loads (lb)

Vert: 10=-47(B) 5=-95(B) 3=32(B) 7=32(B) 7=32(B) 20=-95(B) 21=-95(B) 24=-95(B) 25=-95(B) 26=-244(B) 27=-47(B) 28=-47(B) 29=-47(B) 30=-47(B) 31=-244(B) 27=-47(B) 28=-47(B) 28=-4

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Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

MITEK[®] 16023 Swingley Ridge Rd Chesterfield, MO 63017

Job	Truss	Truss Type	Qty	Ply	Summit/17 Woodside	
					14373	3253
2544696	H01	HIP GIRDER	1	1		
					Job Reference (optional)	
Builders FirstSource (Valley	Center), Valley Center, K	S - 67147,	8.	240 s Mar	9 2020 MiTek Industries, Inc. Mon Nov 23 09:01:19 2020 Page	2

8.240 s Mar 9 2020 MiTek Industries, Inc. Mon Nov 23 09:01:19 2020 Page 2 ID:wH4RYhEsTNeUP2dXvOfi1syQY8e-7dkC9ciH9KLOI8kKY7vlujCHQQpVAOH8n8fw?vyGLf_

LOAD CASE(S) Standard

Uniform Loads (plf)

Vert: 1-4=-80, 4-5=-80, 5-8=-80, 11-15=-20

Concentrated Loads (b) Vert: 19=32(B) 20=-95(B) 21=32(B) 22=-244(B) 23=-47(B) 24=-244(B)

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- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=20ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 3-8-0, Exterior(2R) 3-8-0 to 6-8-0, Interior(1) 6-8-0 to 8-2-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for
- reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 3) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
 - Unbalanced snow loads have been considered for this design.
 - 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
 - 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8, 6.
 8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 - 9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



16023 Swingley Ridge Rd Chesterfield, MO 63017

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LUMBER-

TOP CHORD 2x4 SPF No.2 BOT CHORD 2x4 SPF No.2 WEBS 2x4 SPF No.2 BRACING-TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 2-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 5=0-4-0, 3=Mechanical, 4=Mechanical

Max Horz 5=34(LC 13) Max Uplift 5=-5(LC 16), 3=-19(LC 16)

Max Grav 5=253(LC 23), 3=71(LC 23), 4=33(LC 7)

NOTES-

- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=20ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

Unbalanced snow loads have been considered for this design.

- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 3.
- 8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.





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FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.



				3-1	-8				1	
LOADING (psf) TCLL (roof) Snow (Pf) TCDL	25.0 20.0 20.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYES	CSI. TC 0.25 BC 0.07 WB 0.00	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.00 -0.01 0.00	(loc) 4-5 4-5 3	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20	GRIP 197/144
BCDL	10.0	Code IRC2018/TPI2014	Matrix-MR						Weight: 9 lb	FT = 20%
LUMBER-				BRACING-						

TOP CHORD 2x4 SPF No.2 2x4 SPF No.2 BOT CHORD WEBS 2x4 SPF No.2

TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 3-2-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. 5=0-5-11, 3=Mechanical, 4=Mechanical (size) Max Horz 5=38(LC 12) Max Uplift 5=-59(LC 12), 3=-22(LC 16) Max Grav 5=398(LC 23), 3=115(LC 23), 4=52(LC 7)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. TOP CHORD 2-5=-363/215

NOTES-

1) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=20ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Corner(3) -1-5-8 to 2-9-7, Exterior(2R) 2-9-7 to 3-1-4 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0;

Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

3) Unbalanced snow loads have been considered for this design.

4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

Refer to girder(s) for truss to truss connections.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 3.

8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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Plate Offsets (X,Y) [2:0-2-0	0,0-4-11]										
LOADING (ps TCLL (roof) Snow (Pf) TCDL BCLL BCDL	sf) 25.0 20.0 20.0 0.0 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2018/TF	2-0-0 1.15 1.15 NO Pl2014	CSI. TC BC WB Matrix	0.25 0.39 0.02 x-MP	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.03 -0.06 0.03	(loc) 8-11 8-11 5	l/defl >999 >854 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 14 lb	GRIP 197/144 FT = 20%
LUMBER- TOP CHORD	2x4 SPF No.2					BRACING- TOP CHORD	Struc	tural wo	od sheat	hing directly	applied or 4-0-0 oc pu	rlins,

 BOT CHORD
 2x4 SPF No.2
 except end verticals, and 2-0-0 oc purlins: 3-5.

 WEBS
 2x4 SPF No.2
 BOT CHORD
 Rigid ceiling directly applied or 6-0-0 oc bracing.

 WEDGE
 Left: 2x4 SPF No.2
 Left: 2x4 SPF No.2

REACTIONS. (size) 5=Mechanical, 2=0-4-0, 6=Mechanical Max Horz 2=44(LC 11) Max Uplift 5=-67(LC 44), 2=-35(LC 8), 6=-19(LC 12)

Max Oplift 5=-67(LC 44), 2=-35(LC 8), 6=-19(LC 12)Max Grav 5=48(LC 30), 2=373(LC 34), 6=264(LC 34)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=20ft; Cat. II; Exp C; Enclosed; MWFRS (envelope); cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
 3) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0;
- Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) Refer to girder(s) for truss to truss connections.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 2, 6.
- 10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
 Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 63 lb down and 31 lb up at
- 3-11-4, and 48 lb down and 23 lb up at 3-0-0 on top chord, and 29 lb down and 12 lb up at 3-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 13) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-3=-80, 3-4=-80, 4-5=-80, 6-9=-20

Continued on page 2

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Job	Truss	Truss Type	Qty	Ply	Summit/17 Woodside	
						143733257
2544696	J03	HALF HIP GIRDER	9	1		
					Job Reference (optional)	
Builders FirstSource (Valley	Center), Valley Center, K	S - 67147,	8.2	240 s Mar	9 2020 MiTek Industries, Inc. Mon Nov 23 09:01:24 2020	Page 2

ID:wH4RYhEsTNeUP2dXvOfi1syQY8e-TbY5CKmPzszhOvclLgUTbmv7ARWcrf8twPNgg7yGLev

LOAD CASE(S) Standard

Concentrated Loads (lb) Vert: 3=-6(F) 5=-49(F) 8=0(F)

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			-			4-0-0						
Plate Offsets (X,Y) [2:0-2-0,0-4-11]												
LOADING (ps TCLL (roof) Snow (Pf) TCDL BCLL BCDL	f) 25.0 20.0 20.0 0.0 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2018/TF	2-0-0 1.15 1.15 YES Pl2014	CSI. TC BC WB Matrix	0.27 0.24 0.00 x-AS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.02 -0.04 0.01	(loc) 4-7 4-7 2	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 12 lb	GRIP 197/144 FT = 20%
LUMBER- TOP CHORD BOT CHORD	2x4 SPF No.2 2x4 SPF No.2					BRACING- TOP CHORD BOT CHORD	Struc Rigid	tural wo ceiling	od sheat directly a	hing directly	applied.	

WEDGE Left: 2x4 SPF No.2

REACTIONS. (size) 3=Mechanical, 2=0-4-0, 4=Mechanical

Max Horz 2=50(LC 12) Max Uplift 3=-28(LC 16), 2=-25(LC 12) Max Grav 3=175(LC 23), 2=359(LC 23), 4=77(LC 7)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=20ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 3-11-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2.
- 8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



16023 Swingley Ridge Rd Chesterfield, MO 63017

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TOP CHORD

BOT CHORD

TOP CHORD 2x4 SPF No.2 BOT CHORD 2x4 SPF No.2 WEDGE Left: 2x4 SPF No.2

REACTIONS. (size) 3=Mechanical, 2=0-4-0, 4=Mechanical Max Horz 2=34(LC 12) Max Uplift 3=-14(LC 16), 2=-26(LC 12) Max Grav 3=82(LC 23), 2=243(LC 23), 4=41(LC 7)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=20ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2.
- 8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



Structural wood sheathing directly applied or 2-3-4 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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						2-3-4						
Plate Offsets (X,Y) [2:0-1-1	4,0-0-0], [2:0-2-8,0-3-4]	, [5:0-0-10,0-1	-10]								
LOADING (ps TCLL (roof) Snow (Pf) TCDL BCLL BCDL	sf) 25.0 20.0 20.0 0.0 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2018/TP	2-0-0 1.15 1.15 YES Pl2014	CSI. TC BC WB Matri	0.10 0.03 0.00 x-MR	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.00 -0.00 -0.00	(loc) 5 4-5 4	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 7 lb	GRIP 197/144 FT = 20%
LUMBER- TOP CHORD BOT CHORD WEBS	2x4 SPF No.2 2x4 SPF No.2 2x4 SPF No.2					BRACING- TOP CHORD BOT CHORD	Struct excep Rigid	tural wo ot end v ceiling	od sheat erticals. directly a	hing directly	applied or 2-3-4 oc p)-0 oc bracing.	purlins,

REACTIONS. (size) 4=Mechanical, 5=0-4-0

Max Horz 5=35(LC 13) Max Uplift 4=-10(LC 13), 5=-35(LC 12)

Max Grav 4=94(LC 23), 5=253(LC 23)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=20ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; PI=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

3) Unbalanced snow loads have been considered for this design.

- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) Refer to girder(s) for truss to truss connections.
- Bearing at joint(s) 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 5.
- 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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			2-3-0
LOADING (psf) TCLL (roof) 25.0 Snow (Pf) 20.0 TCDL 20.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2018/TPI2014	CSI. TC 0.25 BC 0.06 WB 0.00 Matrix-MR	DEFL. in (loc) l/defl L/d Vert(LL) 0.00 4-5 >999 240 Vert(CT) 0.00 4-5 >999 180 Horz(CT) -0.00 3 n/a n/a Weight: 7 lb FT = 20%

LUMBER-

TOP CHORD2x4 SPF No.2BOT CHORD2x4 SPF No.2WEBS2x4 SPF No.2

BRACING-TOP CHORD

Structural wood sheathing directly applied or 2-3-8 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 5=0-5-11, 3=Mechanical, 4=Mechanical Max Horz 5=31(LC 12) Max Uplift 5=-61(LC 12), 3=-14(LC 16) Max Grav 5=355(LC 23), 3=59(LC 23), 4=32(LC 7)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-5=-318/202

NOTES-

- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=20ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Corner(3) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

3) Unbalanced snow loads have been considered for this design.

- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) Refer to girder(s) for truss to truss connections.
- Bearing at joint(s) 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 3.
- 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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LOADING (psf) TCLL (roof) 25.0 Snow (Pf) 20.0 TCDL 20.0 BCLL 0.0 BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2018/TPI2014	CSI. TC 0.25 BC 0.06 WB 0.00 Matrix-MR	DEFL. in Vert(LL) 0.00 Vert(CT) 0.00 Horz(CT) -0.00	(loc) 4-5 4-5 3	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 8 lb	GRIP 197/144 FT = 20%
			PRACING					

LUMBER-

TOP CHORD2x4 SPF No.2BOT CHORD2x4 SPF No.2WEBS2x4 SPF No.2

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 2-3-8 oc purlins, except end verticals. Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS. (size) 5=0-4-11, 3=Mechanical, 4=Mechanical Max Horz 5=30(LC 12) Max Uplift 5=-61(LC 12), 3=-15(LC 16) Max Grav 5=355(LC 23), 3=59(LC 23), 4=33(LC 7)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-5=-318/202

NOTES-

- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=20ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Corner(3) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

3) Unbalanced snow loads have been considered for this design.

- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) Refer to girder(s) for truss to truss connections.
- Bearing at joint(s) 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 3.
- 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601




Flate Olisets (∧, t) [2.0-2·	-0,0-0-1], [3.0-0-0,0-0-15										
LOADING (ps TCLL (roof) Snow (Pf) TCDL BCLL BCDL	f) 25.0 20.0 20.0 0.0 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2018/TF	2-0-0 1.15 1.15 NO PI2014	CSI. TC BC WB Matri	0.20 0.09 0.03 ĸ-MP	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.00 -0.00 0.00	(loc) 5-6 5-6 5	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 17 lb	GRIP 197/144 FT = 20%
LUMBER- TOP CHORD BOT CHORD WEBS SLIDER	2x4 SPF No.2 2x4 SPF No.2 2x4 SPF No.2 Left 2x4 SPF	2 2 2 No.2 1-4-3				BRACING- TOP CHORD BOT CHORD	Struct excep Rigid	tural wo ot end v ceiling	ood sheat erticals, a directly a	thing directly and 2-0-0 oc pplied or 10-	applied or 4-0-0 oc pu purlins: 3-4. -0-0 oc bracing.	rlins,

REACTIONS. (size) 2=0-4-0, 5=Mechanical Max Horz 2=34(LC 54)

Max Uplift 2=-15(LC 12), 5=-14(LC 9) Max Grav 2=355(LC 34), 5=257(LC 33)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=20ft; Cat. II; Exp C; Enclosed; MWFRS (envelope); cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
 TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0;
- Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) Refer to girder(s) for truss to truss connections.

Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 5.
 This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

- 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 12) "NAILED" indicates 3-10d (0.148"x3") or 2-12d (0.148"x3.25") toe-nails per NDS guidlines.
- 13) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-3=-80, 3-4=-80, 5-7=-20 Concentrated Loads (lb) Vert: 6=1(F) 11=-7(F) 12=-16(F)











LUMBER-BRACING-TOP CHORD2x4 SPF No.2TOP CHORDStructural wood sheathing directly applied or 4-0-0 oc purlins,
except end verticals, and 2-0-0 oc purlins: 4-5.WEBS2x4 SPF No.2BOT CHORDBOT CHORDSLIDERLeft 2x4 SPF No.2 2-0-0BOT CHORDRigid ceiling directly applied or 10-0- oc bracing.

REACTIONS. (size) 2=0-4-0, 6=Mechanical Max Horz 2=58(LC 15) Max Uplift 2=-16(LC 16), 6=-17(LC 13) Max Grav 2=383(LC 38), 6=205(LC 38)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=20ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 2-11-7, Exterior(2E) 2-11-7 to 3-10-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 6) Provide adequate drainage to prevent water ponding.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) Refer to girder(s) for truss to truss connections.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.







12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.



NUMBER

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16023 Swingley Ridge Rd Chesterfield, MO 63017



Plate Offsets (X,Y) [2:0-2-	·12,0-0-1]									
LOADING (psf) TCLL (roof) 25.0 Snow (Pf) 20.0 TCDL 20.0 BCLL 0.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYES	CSI. TC 0.32 BC 0.22 WB 0.00	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.02 -0.04 0.01	(loc) 5-8 5-8 2	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20	GRIP 197/144	
BCDL 10.0	Code IRC2018/TPI2014	Matrix-MP						Weight: 13 lb	FT = 20%	
LUMBER-			BRACING-							

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SPF No.2 BOT CHORD 2x4 SPF No.2 SLIDER Left 2x4 SPF No.2 2-0-0

REACTIONS. (size) 4=Mechanical, 2=0-4-0, 5=Mechanical

Max Horz 2=63(LC 16) Max Uplift 4=-36(LC 16), 2=-3(LC 16)

Max Grav 4=187(LC 23), 2=372(LC 23), 5=76(LC 7)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=20ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 3-11-4 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0;

Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

3) Unbalanced snow loads have been considered for this design.

- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 2.
- 8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



Structural wood sheathing directly applied or 4-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.





LUMBER-	
TODOUODE	-

TOP CHORD2x4 SPF No.2BOT CHORD2x4 SPF No.2WEBS2x4 SPF No.2

BRACING-TOP CHORD Str exi BOT CHORD Rig

Structural wood sheathing directly applied or 4-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS. (size) 7=0-4-0, 4=Mechanical, 5=Mechanical

Max Horz 7=59(LC 16) Max Uplift 7=-3(LC 16), 4=-11(LC 16), 5=-22(LC 16)

Max Grav 7=383(LC 23), 4=117(LC 23), 5=119(LC 23)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-7=-336/134

NOTES-

- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=20ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 3-11-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

3) Unbalanced snow loads have been considered for this design.

4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) Refer to girder(s) for truss to truss connections.

7) Bearing at joint(s) 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7, 4, 5.
 This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.







TOP CHORD2x4 SPF No.2BOT CHORD2x4 SPF No.2WEBS2x4 SPF No.2WEDGE

Left: 2x4 SPF No.2

REACTIONS. (size) 5=Mechanical, 2=0-4-0, 6=Mechanical Max Horz 2=50(LC 12) Max Uplift 5=-9(LC 23), 2=-25(LC 12), 6=-28(LC 16) Max Grav 5=5(LC 16), 2=359(LC 23), 6=252(LC 23)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. WEBS 4-7=-293/173

NOTES-

- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=20ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-10-8 to 2-1-1, Interior(1) 2-1-1 to 3-11-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 2, 6.
- 8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.







					4-0-0						
Plate Offsets (X,Y) [2:0-2-0),0-4-11]									
LOADING (ps TCLL (roof) Snow (Pf) TCDL BCLL	f) 25.0 20.0 20.0 0.0	SPACING- 2 Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 CSI. 1.15 TC 1.15 BC YES WB	0.27 0.24 0.00	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.02 -0.04 0.01	(loc) 4-7 4-7 2	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20	GRIP 197/144
BCDL	10.0	Code IRC2018/TPI20	014 Mati	ix-AS						Weight: 12 lb	FT = 20%
LUMBER- TOP CHORD BOT CHORD WEDGE	2x4 SPF No.2 2x4 SPF No.2				BRACING- TOP CHORD BOT CHORD	Struc Rigid	tural wo ceiling	ood sheat directly a	thing directly pplied.	applied.	

Left: 2x4 SPF No.2

REACTIONS. (size) 3=Mechanical, 2=0-4-0, 4=Mechanical

Max Horz 2=50(LC 12) Max Uplift 3=-28(LC 16), 2=-25(LC 12) Max Grav 3=175(LC 23), 2=359(LC 23), 4=77(LC 7)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=20ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior(1) 2-1-8 to 3-11-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2.
- 8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.







TOP CHORD 2x4 SPF No.2 BOT CHORD 2x4 SPF No.2 WEBS 2x4 SPF No.2 BRACING-TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 2-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 4=0-4-0, 2=Mechanical, 3=Mechanical

Max Horz 4=26(LC 13) Max Uplift 2=-21(LC 16)

Max Grav 4=111(LC 22), 2=85(LC 22), 3=37(LC 7)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

 Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=20ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for

members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0;

Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

3) Unbalanced snow loads have been considered for this design.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2.

7) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.







Snow (Pf) TCDL BCLL	20.0 20.0 0.0	Lumber DOL 1.15 Rep Stress Incr YES Code IRC2018/TPI2014	BC 0.28 WB 0.00 Matrix-AS	Vert(CT) Horz(CT)	-0.02 3-6 -0.04 3-6 0.01 1	>999 >999 n/a	240 180 n/a	Weight: 10 lb	FT = 20%
LUMBER- TOP CHORD BOT CHORD	2x4 SPF No.2 2x4 SPF No.2		1	BRACING- TOP CHORD BOT CHORD	Structural wo Rigid ceiling	ood shea directly a	thing directly a applied.	applied.	

BOT CHORD 2x4 SPF No.2 WEDGE Left: 2x4 SPF No.2

REACTIONS. (size) 1=0-4-0, 2=Mechanical, 3=Mechanical

Max Horz 1=40(LC 16) Max Uplift 1=-3(LC 16), 2=-29(LC 16) Max Grav 1=248(LC 22), 2=176(LC 22), 3=79(LC 7)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=20ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-0-0 to 3-0-0, Interior(1) 3-0-0 to 3-11-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0;
- Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 2.
- 7) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.





Job	Truss	Truss Type	Qty	Ply	Summit/17 Woodside	
2544696	1 G01	GABLE	1	1		143733273
2011000					Job Reference (optional)	
Builders FirstSource (Valley	Center), Valley Center, F	KS - 67147, ID	8. 3seZTaShN avhe:	240 s Mar IaPBpz4m	9 2020 MiTek Industries, Inc. vNXMX-?aW8ZovSCn_QJMrl	Mon Nov 23 09:01:40 2020 Page 1 NH1nDF8ZuGu4ibrUEcvFXECvGLef
		$\begin{array}{cccccccccccccccccccccccccccccccccccc$	3-4-14 <u>11 10-4-14 12-4-14</u> <u>4 2-0-0 2-0-0</u> 0-7-3 x6	<u>14-4-14</u> 2-0-0	15-7-5 1-2-7	Scale = 1:68.7
				9	10 11 3x4	
		3x4 4⁄2.7 0111-11 3-2-7 5-2-7 7-2-7 8-4 0 ¹ 11-11 2-0-0 2-0-0 2-0-0 1- 0-2-12	<u>4-14 10-4-14 12-4-14</u> 2-7 2-0-0 2-0-0	13 14-4-14 2-0-0	12 <u>15-7-5</u> <u>1-2-7</u>	
LOADING (psf) TCLL (roof) 25.0 Snow (Pf) 20.0 TCDL 20.0 BCLL 0.0 BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2018/TP	2-0-0 CSI. 1.15 TC 0.11 1.15 BC 0.05 YES WB 0.25 I2014 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (l- n/a n/a 0.01	oc) l/defl L/d - n/a 999 - n/a 999 11 n/a n/a	PLATES GRIP MT20 197/144 Weight: 97 lb FT = 20%
LUMBER- TOP CHORD 2x4 SPF N BOT CHORD 2x4 SPF N WEBS 2x4 SPF N	0.2 0.2 0.2	-	BRACING- TOP CHORD BOT CHORD WEBS	Structura Rigid cei 1 Row at	al wood sheathing directly ap ling directly applied or 10-0- r midpt 5-16, 7-7	pplied or 6-0-0 oc purlins. •0 oc bracing. 15
REACTIONS. All bearin (lb) - Max Horz Max Uplift Max Grav	gs 15-7-5. 1=235(LC 13) All uplift 100 lb or less at jc 18=-124(LC 14), 17=-139(L All reactions 250 lb or less 18=266(LC 25), 17=273(LC	bint(s) 16, 15 except 1=-190(LC 12), 11= C 14), 12=-105(LC 15), 13=-124(LC 15) at joint(s) 19, 16, 12, 15 except 1=309(I 25), 13=266(LC 26), 14=275(LC 26)	166(LC 13), 19=- , 14=-140(LC 15) LC 14), 11=298(LC	105(LC 1 C 15),	4),	
FORCES. (lb) - Max. Con TOP CHORD 1-2=-441 BOT CHORD 1-19=-23 14-15=-23 WEBS 3-18=-270	np./Max. Ten All forces 25 (410, 2-3=-305/290, 9-10=-3 2/263, 18-19=-232/263, 17- 32/263, 13-14=-232/263, 12 6/189, 4-17=-299/211, 9-13=	0 (lb) or less except when shown. 05/290, 10-11=-441/410 18=-232/263, 16-17=-232/263, 15-16=-2 -13=-232/263, 11-12=-232/263 276/189, 8-14=-299/211	32/263,			
 NOTES- 1) Unbalanced roof live loa 2) Wind: ASCE 7-16; Vult= MWFRS (envelope) and 10-9-11 to 15-3-15 zone MWFRS for reactions sh 3) TCLL: ASCE 7-16; Pr=2: Rough Cat C; Partially E 4) All plates are 2x4 MT20 5) Gable requires continuou 6) This truss has been desi 7) Provide mechanical contr (jt=lb) 1=190, 11=166, 11 8) This truss is designed in referenced standard ANS 	ds have been considered fo 115mph (3-second gust) Va C-C Exterior(2E) 0-3-6 to 3 ; cantilever left and right exp own; Lumber DOL=1.60 pla 5.0 psf (roof LL: Lum DOL=' xp.; Ce=1.0; Cs=1.00; Ct=1 unless otherwise indicated. Js bottom chord bearing. gned for a 10.0 psf bottom of hection (by others) of truss t 3=105, 18=124, 17=139, 12 accordance with the 2018 I SI/TPI 1.	r this design. sd=91mph; TCDL=6.0psf; BCDL=4.2psf :2-7, Interior(1) 3-2-7 to 7-9-11, Exterior(osed ; end vertical left and right expose te grip DOL=1.60 I.15 Plate DOL=1.15); Pf=20.0 psf (Lum 10 chord live load nonconcurrent with any o b bearing plate capable of withstanding =105, 13=124, 14=140. hternational Residential Code sections F	; h=20ft; Cat. II; E: (2R) 7-9-11 to 10- d;C-C for member DOL=1.15 Plate I ther live loads. 100 lb uplift at join R502.11.1 and R80	κp C; Enc -11, Inter s and forc DOL=1.15 t(s) 16, 15 D2.10.2 ar	losed; ior(1) es &); ls=1.0; 5 except id	STATE OF MISSOL ANDREW THOMAS JOHNSON NUMBER PE-2017018993 STONAL ENGINE



Job		Truss	Truss Type	Qty	Ply	Summit/17 Woodside	
2544696		LG03	GABLE	1	1		143733274
Builders FirstSour	rce (Valley (Center), Valley Center, K	 S - 67147,	8.	240 s Mar	Job Reference (optional) 9 2020 MiTek Industries, Ind	c. Mon Nov 23 09:01:41 2020 Page 1
			1-2-7 3-2-7	D:3seZTgShN_qvhe	lqPBpz4m	iyNXMX-Ts3Wm8z4z46HxW	QZqIISnM62BIQIKImNqZ?4meyGLee
			1-2-7 2-0-0	2-0-0 ' 2-0-0 ' 1-6	i-8 '		0
			I		6 /		Scale = 1:67.7
				1			
				5			
			(7.02)				
			17.09 12	4			
			2-5-7	12			
				´			
			11 10 4x4 //	9 8	7		
			1 <u>-2-7</u> + <u>3-2-7</u> 1-2-7+ <u>2-0-0</u> +	5-2-7 7-2-7 8-8- 2-0-0 2-0-0 1-6	- <u>15</u> 8		
LOADING (psf)	25.0	SPACING-	2-0-0 CSI.	DEFL.	in (I	loc) I/defl L/d	PLATES GRIP
Snow (Pf)	20.0	Plate Grip DOL Lumber DOL	1.15 TC 0.16 1.15 BC 0.02	Vert(LL) Vert(CT)	n/a n/a	- n/a 999 - n/a 999	MT20 197/144
BCLL	0.0	Rep Stress Incr Code IRC2018/TPI	YES WB 0.25 2014 Matrix-P	Horz(CT)	0.00	n/a n/a	Weight: 65 lb FT = 20%
LUMBER-	10.0			BRACING-			
TOP CHORD 2 BOT CHORD 2	2x4 SPF No 2x4 SPF No	5.2 5.2		TOP CHORD	Structura except e	al wood sheathing directly a and verticals.	applied or 6-0-0 oc purlins,
WEBS 2	2x4 SPF No	b.2		BOT CHORD WEBS	Rigid ce 1 Row a	iling directly applied or 10-0 t midpt 6-7, 5-8	D-0 oc bracing. B
REACTIONS. (lb) - 1	All bearing Max Horz	gs 8-8-15. 1=321(LC 14)					
1	Max Uplift	All uplift 100 lb or less at jo 9=-126(LC 14), 8=-112(LC 1	int(s) 7 except 1=-226(LC 12), 11=-10- 4)	4(LC 14), 10=-126(LC 14),		
r	Max Grav	All reactions 250 lb or less	at joint(s) 7, 11, 8 except 1=445(LC 14	4), 10=267(LC 25),	9=266(LC	25)	
FORCES. (lb) -	- Max. Com 1-2=-831/	np./Max. Ten All forces 250 720 2-3=-651/585 3-4=-42!) (lb) or less except when shown. 5/409				
WEBS	3-10=-296	6/252, 4-9=-296/217, 5-8=-26	65/187				
NOTES-	7-16: \/ult-1	115mph (3-second quet) Vas	d-91mph TCDI -6 0pcf BCDI -4 2pc	ef: h=20ft: Cat. II: E		losod.	
MWFRS (enve	elope) and	C-C Corner(3) 0-3-6 to 4-6-4	4, Exterior(2R) 4-6-4 to 8-7-3 zone; car	ntilever left and right -1.60 plate grid	nt exposed	d ; end	
2) TCLL: ASCE 7	7-16; Pr=25	5.0 psf (roof LL: Lum DOL=1	.15 Plate DOL=1.15); Pf=20.0 psf (Lur	m DOL=1.15 Plate	DOL=1.15	5); ls=1.0;	
3) All plates are 2	2x4 MT20	unless otherwise indicated.	10				
4) Gable requires5) This truss has	s continuou been desi	gned for a 10.0 psf bottom c	hord live load nonconcurrent with any	other live loads.			
6) Provide mecha 1=226, 11=10	anical conr 14, 10=126,	9=126, 8=112.	b bearing plate capable of withstanding	j 100 ib upliπ at joir	it(s) / exc	ept (Jt=ID)	and a
7) This truss is de referenced sta	andard ANS	accordance with the 2018 In SI/TPI 1.	iternational Residential Code sections	R502.11.1 and R8	02.10.2 ar	nd	SE OF MISSO
						Å	ANDREW E
						A	
						()	mithing
						~X	PE-2017018993
						X	ATT ATT
							NONAL EN
							November 23 2020

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



November 23,2020



NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=20ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-4-13 to 3-4-13, Interior(1) 3-4-13 to 6-3-3, Exterior(2R) 6-3-3 to 9-3-3, Interior(1) 9-3-3 to 12-1-9 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 13, 12, 10, 9.
- 8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.







- TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7, 9.8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and







lob	Truce		0	DIV	Summit/17 Woodside	
2544606			Qty	r iy	Summiv 17 WoodSide	143733277
2544696			1	1	Job Reference (optional)	
Builders FirstSource (Valley	Center), Valley Center, K	S - 67147, <u>1-2-7, 3-2-7</u> 1-2-7 2-0-0	8 ID:3seZTgShN_qvl <u>5-2-7 7-2-7 9-2-</u> 2-0-0 2-0-0 2-0-	240 s Mar nelqPBpz4 <u>7 10-5-15</u> 0 1-3-8	9 2020 Millek Industries, Ind myNXMX-uRleO9?yG?Uro_{	2. Mon Nov 23 09:01:44 2020 Page 1 38Wtr9P_kZAVR_XfjqWXDkNzyGLeb
		17.09 3x6 // 4 3 1 1 4x4 //	7 12 6 5 15 8 12 11	8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8		Scale = 1:75.8
		<u>1-2-7</u> <u>3-2-7</u> 1-2-7 2-0-0	5-2-7 + 7-2-7 + 9-2- 2-0-0 + 2-0-0 + 2-0-	7 10-5-15 0 1-3-8		
LOADING (psf) TCLL (roof) 25.0 Snow (Pf) 20.0 TCDL 20.0 BCLL 0.0 BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2018/TPI	2-0-0 CSI. 1.15 TC 0.18 1.15 BC 0.02 YES WB 0.23 2014 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (l n/a n/a 0.00	oc) l/defl L/d - n/a 999 - n/a 999 9 n/a n/a	PLATES GRIP MT20 197/144 Weight: 87 lb FT = 20%
LUMBER- TOP CHORD 2x4 SPF N BOT CHORD 2x4 SPF N WEBS 2x4 SPF N 2x4 SPF N REACTIONS. All bearin Max Horz Max Uplift Max Grav	lo.2 lo.2 lo.2 lo.2 1=388(LC 14) All uplift 100 lb or less at jc 12=-121(LC 14), 11=-129(Li All reactions 250 lb or less 11=272(LC 25)	int(s) 9, 10 except 1=-278(LC 12), 14= C 14) at joint(s) 9, 14, 10 except 1=541(LC	BRACING- TOP CHORD BOT CHORD WEBS =-104(LC 14), 13=- ⁻ 14), 13=269(LC 25)	Structura except e Rigid ce 1 Row a 27(LC 14 , 12=257(al wood sheathing directly a ind verticals. iling directly applied or 10-0 t midpt 8-9, 6-1), LC 25),	applied or 6-0-0 oc purlins,)-0 oc bracing. 1, 7-10
FORCES. (lb) - Max. Con TOP CHORD 1-2=-957 WEBS 4-13=-29	np./Max. Ten All forces 25 /841, 2-4=-792/717, 4-5=-57 /3/244, 5-12=-279/203, 6-11=	0 (lb) or less except when shown. 3/543, 5-6=-388/380 297/208				
 NOTES- 1) Wind: ASCE 7-16; Vult- MWFRS (envelope) and members and forces & I 2) TCLL: ASCE 7-16; Pr=2 Rough Cat C; Partially 3) All plates are 2x4 MT20 4) Gable requires continue 5) This truss has been des 6) Provide mechanical cor (jt=lb) 1=278, 14=104, 1 7) This truss is designed in referenced standard AN 	e115mph (3-second gust) Va: I C-C Corner(3) 0-3-6 to 4-6- WWFRS for reactions shown 5.0 psf (roof LL: Lum DOL=1 Exp.; Ce=1.0; Cs=1.00; Ct=1. unless otherwise indicated. us bottom chord bearing. igned for a 10.0 psf bottom con nection (by others) of fruss to 3=127, 12=121, 11=129. accordance with the 2018 In SI/TPI 1.	sd=91mph; TCDL=6.0psf; BCDL=4.2p 4, Exterior(2R) 4-6-4 to 10-4-3 zone; c Lumber DOL=1.60 plate grip DOL=1. .15 Plate DOL=1.15); Pf=20.0 psf (Lu 10 hord live load nonconcurrent with any bearing plate capable of withstanding aternational Residential Code sections	sf; h=20ft; Cat. II; E antilever left and rig 60 m DOL=1.15 Plate other live loads. g 100 lb uplift at joir R502.11.1 and R8	xp C; Enc ht expose DOL=1.15 t(s) 9, 10 D2.10.2 at	losed; ed;C-C for ;;; Is=1.0; except nd	ANDREW THOMAS JOHNOON NUMBER PE-2017018993

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11 Quality Criteria**, **DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



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- MWFRS (envelope) and C-C Exterior(2E) 0-3-6 to 3-3-6, Interior(1) 3-3-6 to 4-9-3, Exterior(2R) 4-9-3 to 7-9-3, Interior(1) 7-9-3 to 9-2-15 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=168, 6=167.

7) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.















👠 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE Design valid to use only design parameters and READ NOTES ON THIS AND INCLUDED WITH REPORT PAGE MIT 475 169 (2020 BEFORE USE). Design valid for use only with MITEK deconnectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent colleges with possible personal injury and property damage. For general quidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component
 Satisfies
 Ansi/TPH Qu

 Safety Information
 available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



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E



0-0 ₀ 11					1	6-9-6						1	
0-0 ⁻ 11					16	6-8-12						1	
LOADING (psi TCLL (roof) Snow (Pf) TCDL BCLL BCDL	f) 25.0 20.0 20.0 0.0 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2018/TF	2-0-0 1.15 1.15 YES Pl2014	CSI. TC BC WB Matri	0.31 0.10 0.06 x-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.00	(loc) - - 5	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 43 lb	GRIP 197/144 FT = 20%	
LUMBER-						BRACING-							

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD2x4 SPF No.2BOT CHORD2x4 SPF No.2OTHERS2x4 SPF No.2

REACTIONS. All bearings 16-8-1.

(lb) - Max Horz 1=-28(LC 21)

Max Uplift All uplift 100 lb or less at joint(s) 1, 5, 9, 6

Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 7=379(LC 2), 9=563(LC 22), 6=563(LC 23)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. WEBS 3-7=-305/72, 2-9=-472/163, 4-6=-472/163

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=20ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-10-1 to 3-10-1, Interior(1) 3-10-1 to 8-4-11, Exterior(2R) 8-4-11 to 11-4-11, Interior(1) 11-4-11 to 15-11-6 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) Gable requires continuous bottom chord bearing.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5, 9, 6.
- 8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and
- referenced standard ANSI/TPI 1.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.





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HESSIONAL



LOADING (ps TCLL (roof) Snow (Pf) TCDL BCLL BCDL	sf) 25.0 20.0 20.0 0.0 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2018/TP	2-0-0 1.15 1.15 YES Pl2014	CSI. TC BC WB Matri:	0.19 0.07 0.00 x-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.00	(loc) - - 2	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 9 lb	GRIP 197/144 FT = 20%
LUMBER- TOP CHORD BOT CHORD WEBS	2x4 SPF No.2 2x4 SPF No.2 2x4 SPF No.2					BRACING- TOP CHORD BOT CHORD	Struct excep Rigid	tural wo ot end vo ceiling	od sheat erticals. directly a	hing direc pplied or 1	tly applied or 3-8-11 oc	purlins,

REACTIONS. (size) 3=3-8-0, 2=3-8-0 Max Horz 3=-34(LC 12) Max Uplift 3=-12(LC 17), 2=-7(LC 17) Max Grav 3=167(LC 23), 2=167(LC 23)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=20ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.

4) Gable requires continuous bottom chord bearing.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2.
- 7) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and
- referenced standard ANSI/TPI 1.







BOT CHORD 1-3=-154/263

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=20ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) Gable requires continuous bottom chord bearing.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3. 8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and

referenced standard ANSI/TPI 1.







Plate Offsets (X,Y)-- [2:0-1-12,0-0-10], [4:0-0-2,0-2-0], [4:0-1-12,0-0-0]

	//	/ /-	- / /									
LOADING (ps TCLL (roof) Snow (Pf) TCDL BCLL BCDL	if) 25.0 20.0 20.0 0.0 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2018/TI	2-0-0 1.15 1.15 YES PI2014	CSI. TC BC WB Matri	0.34 0.12 0.06 x-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.00	(loc) - - 3	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 16 lb	GRIP 197/144 FT = 20%
LUMBER- TOP CHORD BOT CHORD	2x4 SPF No.2 2x4 SPF No.2					BRACING- TOP CHORD	Struct	tural wo	od sheat erticals.	hing directly	y applied or 6-0-0 oc pu	rlins,
WEBS	2x4 SPF No.2 2x4 SPF No.2					BOT CHORD	Rigid	ceiling	directly a	pplied or 10)-0-0 oc bracing.	

REACTIONS. (size) 5=6-4-11, 3=6-4-11, 4=6-4-11 Max Horz 5=-66(LC 12) Max Uplift 5=-12(LC 17), 3=-91(LC 23), 4=-47(LC 17) Max Grav 5=215(LC 23), 3=23(LC 17), 4=573(LC 23)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. WEBS 2-4=-484/263

NOTES-

- Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=6.0psf; BCDL=4.2psf; h=20ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-1-12 to 4-3-8, Interior(1) 4-3-8 to 5-6-10 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0;
- Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 3, 4.
- 7) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and
 - referenced standard ANSI/TPI 1.







- MWFRS (envelope) and C-C Exterior(2E) 0-1-12 to 4-4-11, Interior(1) 4-4-11 to 8-2-10 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 2) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0;
- Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 4.
- 7) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.













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- 3) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) Gable requires continuous bottom chord bearing.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6, 4.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.





