

1.	All design and construction work for this project shall conform to the 2018 edition of the International Building Code (IBC).	
2.	Design Loads	
A.	Roof Loads:	
	Dead load	20 psf (Per Existing Drawings)
	Roof live load	20 psf
	Ground snow load	20 psf
	Flat roof snow load (snow drifting additional)	20 psf
	Snow importance factor	$I_s = 1.0$
	Snow exposure factor	$C_e = 1.0$
	Thermal factor	$C_t = 1.0$
B.	Wind Loads:	
	Risk category	II
	Ultimate design wind speed (3 sec gust)	109 mph
	Service design wind speed (3 sec gust)	85 mph
	Wind exposure	B
	Wind internal pressure coefficient	+0.18
	Wind pressure for components and cladding	21.9 psf (Ultimate)
C.	Seismic Loads:	
	5% Damped mapped acceleration parameter	$S_s = 0.100$
	1-Sec period mapped acceleration parameter	$S_1 = 0.066$
	5% Damped spectral response coefficient	$S_{ds} = 0.106$
	1-Sec period spectral response coefficient	$S_{d1} = 0.100$
	Site class	D (Assumed)
	Risk category	II
	Seismic importance factor	$I_e = 1.0$
	Seismic design category	
3.	Shop drawings shall be submitted for review by the Architect and Engineer prior to fabrication.	

1. Furnish all labor, materials and equipment necessary to complete the work shown or implied by these drawings.
2. The General Contractor shall be responsible for verifying all dimensions and elevations with the Architectural and Mechanical drawings and the existing conditions. Ref: Architectural and Mechanical drawings for embedded items not shown herein and to verify size and location of all openings. Before proceeding with work, the General Contractor shall verify the actual job conditions and report to the Engineer any errors, omissions or difficulties affecting the work.
3. The structure is designed to be self-supporting and stable after erection of the structure has been fully completed. It is the General Contractor's responsibility to determine erection sequencing and to provide adequate bracing during construction.
4. The General Contractor shall provide adequate shoring or bracing during construction to resist forces such as wind and unbalanced loading due to construction.
5. Johnson Burkholder Associates, LLC, its employees, and representatives shall not be responsible for, and will have no control of, conditions, methods, techniques, sequences or procedures, or for safety precautions and programs in connection with construction; nor will they be responsible for any failure by the contractor to perform or complete construction in accordance with the drawings. Johnson Burkholder Associates, LLC shall not be responsible for any liability, awareness or liability for any hazardous conditions.
6. The General Contractor shall be responsible for protecting the existing building during construction.
7. Johnson Burkholder Associates assumes responsibility for the design of the new elements and systems shown on these drawings only. Johnson Burkholder Associates has not designed nor reviewed the design of the existing building in areas unaffected by new construction and accepts no responsibility for the design, adequacy or performance of the existing building in areas unaffected by new construction.

1. All concrete and reinforcement has been designed in accordance with the American Concrete Institute (ACI) Standard Building Code Requirements for Reinforced Concrete (ACI 318). All concrete and reinforcement shall conform to the ACI Specifications for Structural Concrete (ACI 301) and the latest applicable recommendations of the ACI Manual of Standard Practice for Detailing Reinforced Concrete (ACI 315).
2. Materials shall conform with:
 - A. Cement-ASTM C150 Type I or II
 - B. Aggregate-ASTM C33
 - C. Water- Potable
3. All concrete used in the work shall have the following properties:
 - A. Footings
 - 3000 psi strength (f_c) at 28 days
 - 0.50 maximum water/cement ratio
 - less than 3% air content
 - $4^{\circ} \pm 1^{\circ}$ slump at point of placement
 - B. Interior slabs-on-grade
 - 4000 psi strength (f_c) at 28 days
 - 0.53 maximum water/cement ratio
 - less than 3% air content
 - $4^{\circ} \pm 1^{\circ}$ slump at point of placement
 - C. Exterior slabs-on-grade
 - 4000 psi strength (f_c) at 28 days
 - 0.40 maximum water/cement ratio
 - 6% \pm 1.5% air content
 - $4^{\circ} \pm 1^{\circ}$ slump at point of placement
4. Chlorides in any form or concentration shall not be added to any concrete.

- All concrete is reinforced unless specifically noted as "unreinforced". Reinforce all concrete not otherwise shown with the same steel as shown in similar sections. Comply with ACI 304.
- "Reinforcing Practice": Reinforcing Bars, Welding, Transporting and Handling Concrete
- Cold weather conditions: When the average daily air temperature for 3 consecutive days is expected to be below 40° F and the air temperature does not exceed 50° F for more than 12 consecutive hours during the curing period, concrete placement shall comply with the provisions of ACI 306 and as herein specified. Protect concrete work from physical damage or reduced strength that could be caused by frost, freezing actions, or low temperatures.
- Water conditioning: If the ambient temperature or other environmental factors exist, all concrete shall comply with the provisions of ACI 305 and as here is specified.
- Perform curing of concrete by curing and sealing compound, by moist curing, moisture-retarding compound, or other approved method.
- Maintain minimum concrete coverage for reinforcing as indicated unless otherwise noted in the drawings:
 - A. Formed/cast directly against soil 3"
 - B. Cast against forms but exposed to earth or water:
 - a. #8 and larger 3"
 - b. #4 and smaller 2" 1/2"
 - C. Slabs and walls not exposed to earth/water 3/4"
- Other details in slab-on-grade shall be as shown on the drawings. Where not shown, limit controlled areas to not more than 12'-0" on any side. Do not intersect construction and saw joints where a particular joint detail is specified on the drawings. A saw joint must terminate at a corner.
- Coordinate concrete finishes, recessed areas, reveals, embedded items, special joint patterns, etc. with the Architectural drawings and specifications. Provide a 3/4" chamfer at all exposed edges of concrete. No concrete shall be placed over a finished surface of concrete.
- QUALITY CONTROL TESTING DURING CONSTRUCTION**
 - A. General: The Owner shall employ a testing laboratory to perform tests and to submit test results to the Architect.
 - a. Sampling and testing for quality control during placement of concrete shall include the following, as directed by the Architect.
 - i. Slump: Fresh Concrete ASTM C 172; except filled slump to comply with ASTM C 94.
 - ii. Slump: ASTM C 143; one test at point of discharge for each day pour of each type of concrete; additional when concrete consistency appears to have changed.
 - iii. Air Content: ASTM C 173; 28 test per set for dry weight or normal weight concrete.
 - iv. ASTM C 231 pressure method for normal weight concrete; one for each day pour of each type of air-entrained concrete.
 - v. Concrete Temperature: ASTM C 1064; one test per set for dry weight or normal weight concrete; and below, when 80 deg F (27 deg C) and above, and each time a set of compression test specimens is made.
 - b. Compression Test Specimen: ASTM C 31; one set of 4 standard cylinders for each compressive strength test; unless otherwise directed. Mold and store cylinders for laboratory-cured test specimens except when field-cure test specimens are required.
 - c. Field-Cure Strength Tests: ASTM C 38; one set per set for dry weight plus additional sets for each 50 cu. yards; more than the first 25 cu. yards, of each concrete class placed in any one day; one specimen tested at 7 days, two specimens tested at 28 days.
 - D. Specimen retention: Retain one set of specimens for each compressive strength class + strength level of concrete will be considered satisfactory if averages of sets of three consecutive strength test results equal or exceeded specified compressive strength, no individual strength test result was less than specified compressive strength by more than 500 psi.

1. Post-installed anchors shall be installed in accordance with the Manufacturers Printed Installation Instructions (MPII). If anchors are to be installed in a horizontal or upward direction personnel shall be trained to install adhesive anchors through the ACI/CRSI Adhesive Anchor Installer Certification Program.
2. Post-installed anchors shall only be used where noted on the drawings. The use of post-installed anchors for repairs requires approval from the Engineer of Record.

1. All structural steel plate shall be ASTM A36 for shapes and plates, ASTM A573 Grade B for pipes, ASTM A500, Grade B or C for rectangular and square HSS members and ASTM 1085 for round HSS members, unless noted otherwise. W shapes shall be ASTM A992 or ASTM A572, Grade 50. Fabrication and erection shall be in accordance with the latest edition of the American Institute of Steel Construction (AISC) Manual of Steel Construction.
2. All structural steel shall be painted and the current American Welding Society Specifications and be performed by certified welders.
3. All structural steel shall have one shop coat of rust inhibitor primer paint conforming to the specifications. Field touch up all unpainted, nicked and welded areas.
4. All unpainted, exterior steel plate shall be galvanized.
5. No permanent suspended loads are to be supported by metal deck.

1. Existing steel joists and joist girders are assumed to be in good condition and in compliance with the Steel Joist Institute Specifications. Any damage to existing joists or joist girders shall be reported prior to adding load to framing.

- A. All light gauge structural joists, studs, track and accessories shall be designed in accordance with the latest edition of the American Iron and Steel Institute (AISI) "Specification for the Design of Cold-Formed Steel Structural Members", and shall be of the type, size, gauge and spacing shown on the drawings.
- A. All 16 gauge and heavier studs and joists shall be formed from corrosion-resistant steel conforming to the requirements of ASTM A1003 or A653, with a minimum yield strength of 50 ksi. All 18 gauge and lighter studs, joists, track and accessories shall be formed from corrosion-resistant steel corresponding to the requirements of ASTM A1003 or A653, with a minimum yield strength of 33 ksi.
- C. The design and details provided on these drawings are for the final in-place conditions. The General Contractor shall be responsible for temporary bracing, as required, prior to completion of all lateral bracing supports.
- D. Fastening of components shall be with self-drilling screws, powder actuated fasteners (PAF) or welding. Screws and PAFs shall be installed such that a minimum 3/4" on center spacing and 3/4S of AISI 5100 Welding electrode shall be fasteners in concrete shall have a minimum spacing of 3". All welding shall be according to the American Welding Society (AWS) D11.3 specification, for "Structural Welding Code - sheet steel", and shall be performed by certified welders. Consult AWS D11.3 and AWS D1.1 for details. Also consult AWS D11.3 and AWS D1.1 for information regarding safe welding procedures. Weld sizes shall match the thickness of the thinner part. All welds shall be touched up with a zinc-rich paint on each side of the weld.
- D. Prior to fabrication of the General Contractor shall submit fabrication and erection drawings to the Architect/Engineer for review.
- D. All framing components shall be cut square for attachment to perpendicular members or, as required, for an in-situ cut or angled cut through all members. All cutting shall be done by sawing and shearing.
- D. Unless noted otherwise, abutting lengths of track shall be spliced together using a piece of stud of the same size and type as the track. All #10 screws per flange on each side of the splice.
- D. Axially loaded studs shall be installed in a manner which will ensure that their ends are positioned against the inside of the track web prior to fastening. Studs shall be securely fastened to both flanges of the track web bottom track with (1) #10 screw.
- D. Wall stud bridging shall be attached in a manner to prevent stud rotation. Bridging rods shall be installed at 4'-0" on center unless otherwise noted on drawings.
- D. All wall stud and joist bracing shall be installed at the time of light gauge erection. Member punch outs shall be aligned to allow bridging installation.
- D. Sheathing shall be attached to the flange of each framing member as noted on the drawings.
- D. Wall attachment is required to be made at the time of attachment spacing of 12" on center shall be used.
- D. Sample Member Designation: 600S162-43
 - 600 - Web size (W)
 - S - Member type: S- stud, T - track
 - 162 - Flange size (1/8")
 - 43 - Member thickness (in): 33 - 20 gauge, 43 - 18 gauge, 54 - 16 gauge
4. Acceptable Fasteners (UNO): (Substitutions shall be submitted for review).
- A. Screws:
 - Light gauge to light gauge
 - o Buldex Tech
 - o Hilti Kwik-Pro
 - o Simpson XS or FHPSD
 - Light gauge Stud/Track to Concrete; 1/4" diameter
 - o Simpson Titan HD-Mini
 - o Hilti Kwik-Con
 - o Buldex Tapcon
 - Powers Tapper
- B. Powder Actuated Fasteners (PAF):
 - Light gauge Stud/Track to Concrete; 0.145" dia x 1" (7" Minimum)
 - o Rammed Power
 - o Simpson PDP
 - o Hilti Power Point SP114
 - Light gauge Stud/Track to Steel; 0.157" diameter with length based on steel thickness
 - o Hilti X-U
 - o Simpson PDP

1. All concrete masonry units shall conform to ASTM C 90 specifications and shall be placed in running bond unless noted otherwise. Concrete masonry units above finished grade shall be either light weight or medium weight density. Concrete masonry units below finished grade shall be fully dense, normal weight concrete.
2. Concrete masonry shall have a minimum prism compressive strength, f_m , of 2000 psi. Minimum unit compressive strength shall be 1900 psi. All mortar for masonry shall be Type S, mortar shall be fully mixed, and shall be applied in accordance with the manufacturer's instructions. Bed on web between grouted cell and hollow cell.
3. Masonry grout shall have a 28 day compressive strength of 2000 psi. Masonry Contractor shall be fully responsible for the design and construction of grout. Grout cells shall be placed in low lifts not to exceed 5'4". High lift grouting may be used if Masonry Contractor submits written permission to Engineer for review prior to construction of walls. All grouted cells shall be fully consolidated using a vibrator. Grout strength shall be greater than 1000 psi. Diameter: Do not use concrete vibrator.
4. All reinforcing steel shall meet ASTM A618 Grade 60 and ASTM A706 Grade 50 and located in accordance with the wall design. Grout shall be placed in grouted cells. Use wire reinforced devices where approved means to ensure proper location and secure reinforcement in place.
5. All bond beams and cells containing reinforcing steel, anchors, embedments, etc. shall be filled with concrete. Hollow cells containing no reinforcement shall be filled with grout.
6. All vertical reinforcement and continuous steel shall be lapped 52 bar diameters (2.7" minimum) at splices unless noted otherwise on the drawings.
7. The Masonry Contractor shall be responsible for the temporary bracing of masonry walls before permanent lateral support is in place.
8. Masonry may be placed using normal procedures when temperatures are between 40-90 degrees Fahrenheit.
 - A. When temperatures are above or below this range, hot and cold weather masonry procedures outlined in the Specification for Masonry Structures (TMS 602) shall be followed. When temperatures are above 90 degrees or 90 degrees with an 8 mph wind, "Hot weather procedures" must be followed.
 - B. When temperatures are below 40 degrees, "Cold weather procedures" must be followed. When temperatures are below 40 degrees with an 8 mph wind, "Cold weather procedures" must be followed.
9. All masonry walls shall be constructed using a plastic or polyethylene sheeting to retain moisture. Walls shall be covered at the end of each day.

Work in progress shall be inspected and materials, equipment and procedures evaluated for compliance with the Specifications for Masonry Structures (TMS 602). The final construction of masonry shall be in accordance with the Specifications for Masonry Structures (TMS 602).

1. **Special inspections shall be performed in accordance with Chapter 17 of the 2018 International Building Code (IBC).** All special inspectors shall be qualified for inspection of the particular type of construction to be inspected. The special inspector shall be approved by the building department. All special inspectors shall perform the duties and responsibilities outlined in Chapter 17 of the 2018 edition of the International Building Code. The special inspector shall be submitted to the building official, architect, and engineer of record in a timely manner.
2. **Types of work requiring special inspection:**
 - a. Concrete per Section 1705.3
 - a. Periodic inspection of the placement of reinforcing steel.
 - b. Periodic inspection of all post-installed anchors and continuous inspection of adhesive and grout installed between anchors and concrete. Inspections shall include hole size and depth, cleaning procedure, materials, and location. All anchors installed in hardened concrete are subject to inspection.
 - b. Periodic verification of concrete test results as required by Section 1705.3.2.
 - c. Continuous sampling of fresh concrete. Perform tests for slump, air content, and temperature. Cast specimens for strength tests (see Cast-in-Place Concrete - Execution notes).
 - c. Continuous inspection of concrete placement for proper placement techniques.
 - d. Continuous inspection of masonry for proper construction and placement techniques.
 - d. Periodic inspection of formwork for shape, strength, and dimensions.
 3. **Masonry per Section 1706.4 and The Masonry Society (TMS) Building Code Requirements for Masonry Structures, 2013 Edition, and Addendum 2014.**
 - a. At the beginning of masonry construction, one masonry unit and one grout sample shall be taken for testing. Masonry units shall be tested in accordance with ASTM C140.
 - a. Grout shall be tested in accordance with ASTM C1119.
 - b. If high lift grouting procedures are used, continuous inspection is required.
 - c. During construction, three masonry units and three grout samples shall be taken.
 - c. The placement of masonry placement and the grouting of walls shall be observed during the initial and final grout pours.
 - d. CMU mortar samples and inspection of joints shall occur at the beginning of masonry construction.
 - d. Anchor bolt placement and grouting below beams or bearing plates at all beam bearing locations shall be inspected.
 - e. Periodic inspection of the preparation, construction, and protection of masonry during cold or hot weather conditions.
 4. **Structural Steel per Section 1708.2:**
 - a. Periodic inspection for material verifications.

DATE
BY

AB	ANCHOR BOLT
ACI	AMERICAN CONCRETE INSTITUTE
AFF	ABOVE FINISHED FLOOR
AISI	AMERICAN INSTITUTE OF STEEL CONSTRUCTION
AI	AMERICAN IRON AND STEEL INSTITUTE
ARCH	ARCHITECTURAL
ASTM	AMERICAN SOCIETY FOR TESTING AND MATERIALS
ASW	AMERICAN WELDING SOCIETY
BFF	BELOW FINISHED FLOOR
BL	BELOW LOCK
BM	BEAM
BO	BOTTOM
BOM	BOTTOM OF MASONRY
BOS	BOTTOM OF STEEL
BRG	BEARING
CJ	CONTRACTION JOINT
CL	CENTER LINE
CLR	CLEAR
CMU	CONCRETE MASONRY UNIT
COL	COLUMN
CONC	CONCRETE
COND	CONDENSER UNIT
CONST	CONSTRUCTION
CONT	CONTINUOUS
DIA	DIAMETER
EF	EXHAUST FAN
EIFS	EXTERIOR INSULATION AND FINISH SYSTEM
EJ	EXPANSION JOINT
EL	ELEVATION
ELEC	ELECTRICAL
EQ	EQUAL
ETR	EXISTING TO REMAIN
EW	EACH WAY
FDN	FOUNDATION
FF	FINISHED FLOOR
FS	FAR SIDE
FTG	FOOTING
FV	FIELD VERIFY
GA	GAUGE
GC	GENERAL CONTRACTOR
H	HEIGHT
HORIZ	HORIZONTAL
HSA	HEADED STUD ANCHOR
HSS	HOLLOW STRUCTURAL SHAPE
INFO	INFORMATION
ISO	ISOLATION
JBE	JOIST BEARING ELEVATION
JST	JOIST
JT	JOINT
KSI	KIPS PER SQUARE INCH
L	LENGTH
LL	LONGS
LLH	LONG LEG HORIZONTAL
LLV	LONG LEG VERTICAL
LONG	LONGITUDINAL
MAX	MAXIMUM
MECH	MECHANICAL
MFR	MANUFACTURER
MIN	MINIMUM
MISC	MISCELLANEOUS
MO	MASONRY OPENING
MTL	METAL
NOT	NOT IN CONTRACT
NO	NUMBER
NS	NEAR SIDE
NTS	NOT TO SCALE
OC	ON CENTER
OD	OUTSIDE DIAMETER
OH	OPPOSITE HAND
PAF	POWER ACTUATED FASTENER
PCF	POUNDS PER CUBIC FOOT
PL	PLATE
PLF	POUNDS PER LINEAR FOOT
PLUMB	PLUMBING
PMEJ	PREMOLDED EXPANSION JOINT
PSF	POUNDS PER SQUARE FOOT
PSI	POUNDS PER SQUARE INCH
QTY	QUANTITY
RACK	REFRIGERATION RACK
REF	REFER TO
REINF	REINFORCING
REQD	REQUIRED
REV	REVERSE
RO	ROUGH OPENING
RTU	ROUGH TOP UNIT
SCHED	SCHEDULE
SDI	STEEL DECK INSTITUTE
SIM	SIMILAR
SJL	STEEL JOIST INSTITUTE
SPCS	SPACES
SPCSPEC	SPECIFICATIONS
STRUC	STRUCTURAL
T&B	TOP AND BOTTOM
THK	THICKNESS
TO	TOP OF
TOC	TOP OF CONCRETE
TOF	TOP OF FOOTING
TOM	TOP OF MASONRY
TOS	TOP OF STEEL
TOW	TOP OF WALL
TRANS	TRANSVERSE
TYP	TYPICAL
UN	UNLESS NOTED OTHERWISE
VERT	VERTICAL
W	WIDTH
WP	WORK POINT
WWF	WELDED WIRE FABRIC

STATE OF MISSOURI
BART HALVERSON
NUMBER
E-20439
REGISTERED PROFESSIONAL ENGINEER
10/22/20

10/22/20

Hydrex®

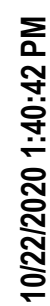


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DRAWN BY: JBA	DATE: 10/20/2020
SCALE: 12" = 1'-0"	JOB NUMBER 62930547

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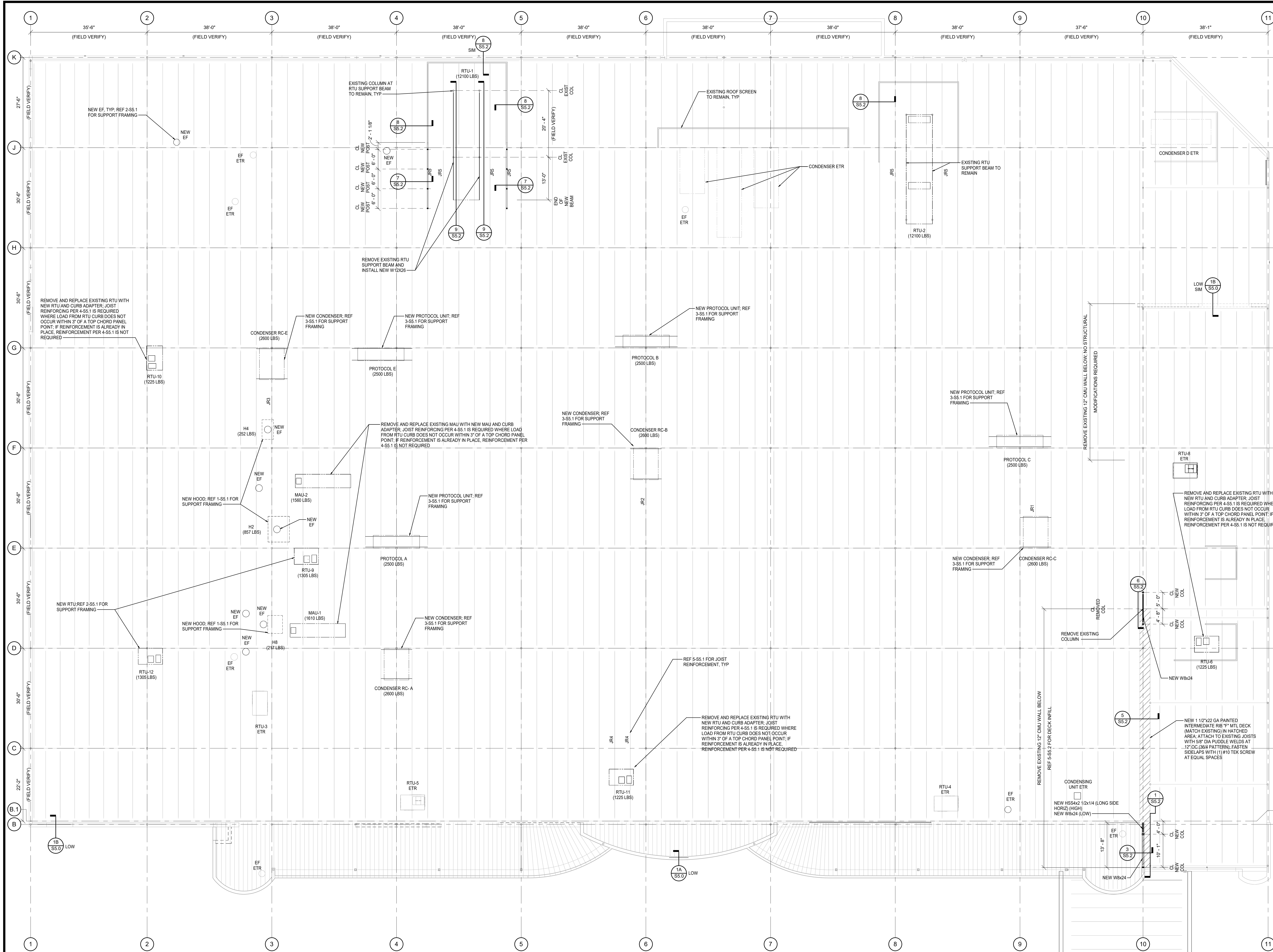
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DRAWN BY: JBA	DATE: 10/20/2020
SCALE: 3/32" = 1'-0"	JOB NUMBER 62930547

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1 ROOF FRAMING PLAN
SCALE: 3/32" = 1'-0"

REVISION		DATE BY

JBA JOHNSTON BURKHOLDER ASSOCIATES
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816-471-4700 www.jba-usa.com
JBA PROJ #: 20332 - JSH

PROFESSIONAL SEAL

STATE OF MISSOURI
BART HALVORSON
NUMBER E-30439
EXPIRATION 10/22/2020

LOCATION
LEE'S SUMMIT, MO

HY-VEE, INC.
5820 WESTTOWN PARKWAY
WEST DES MOINES, IOWA 50266
PHONE (515) 267-2600
FAX (515) 267-2636

Hy-Vee
EMPLOYEE OWNED

NORTH

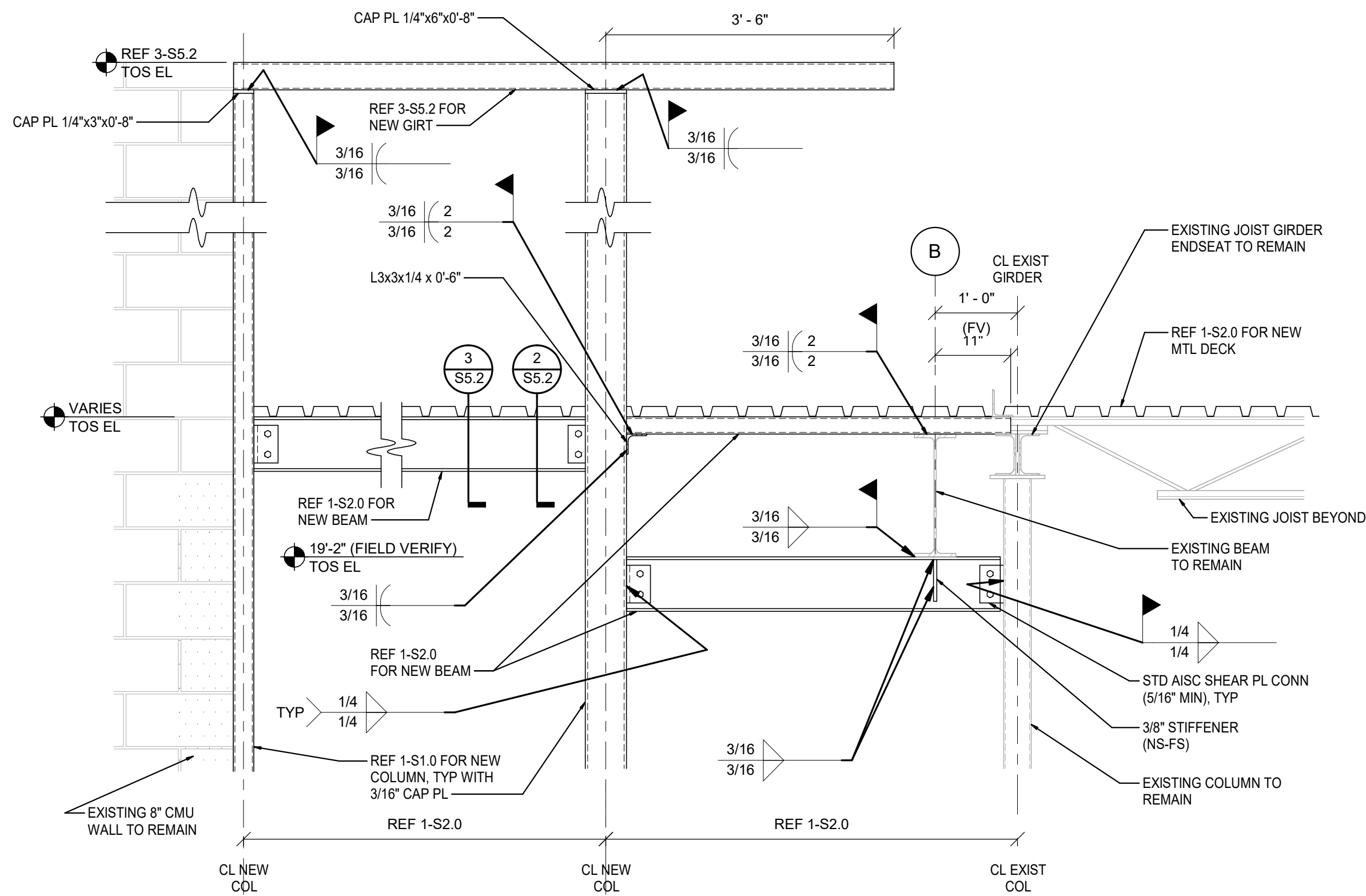
ROOF FRAMING PLAN

PROJECT MANAGER	CHECKED BY
JSH	BJH

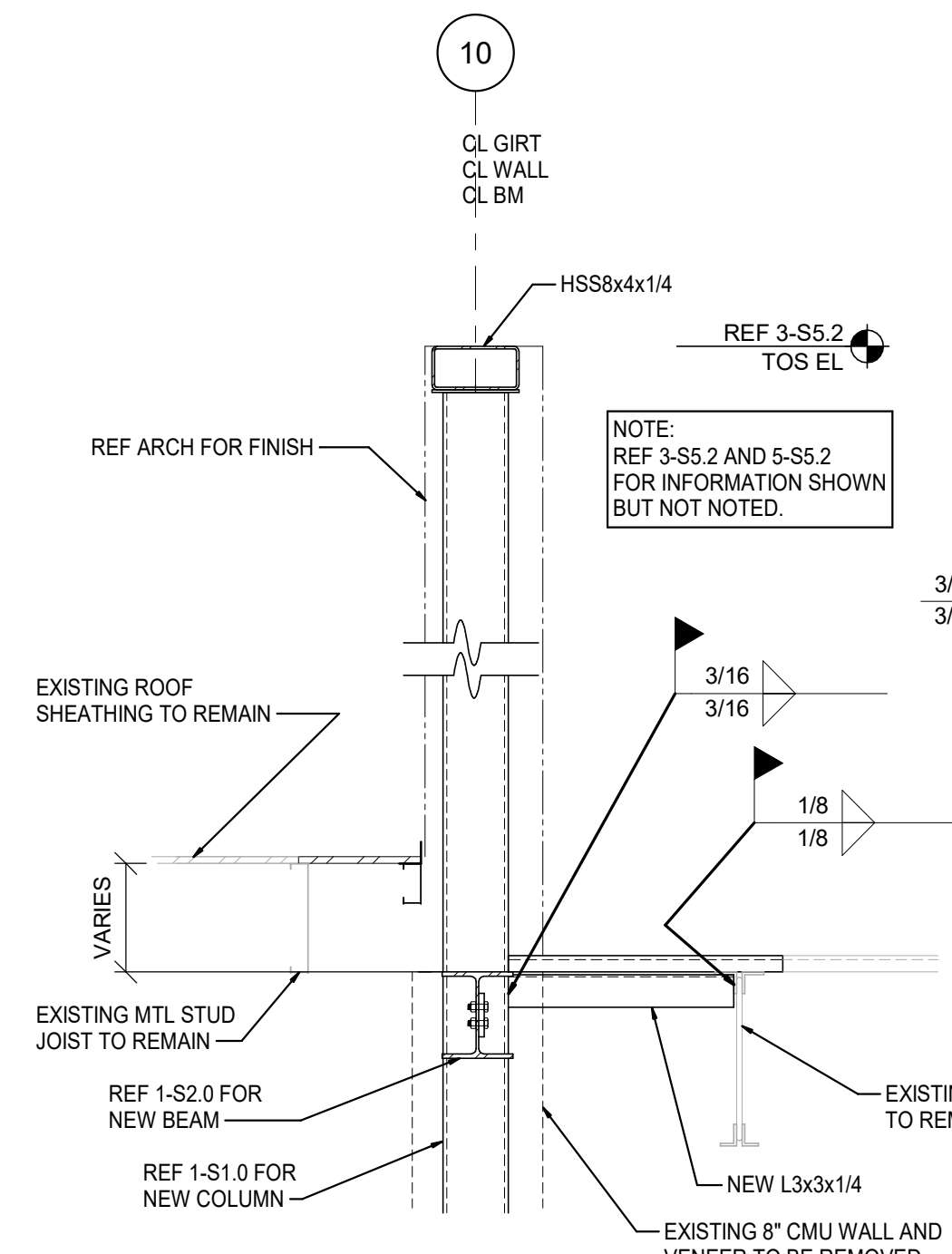
DRAWN BY	DATE
JBA	10/20/2020

SCALE	JOB NUMBER
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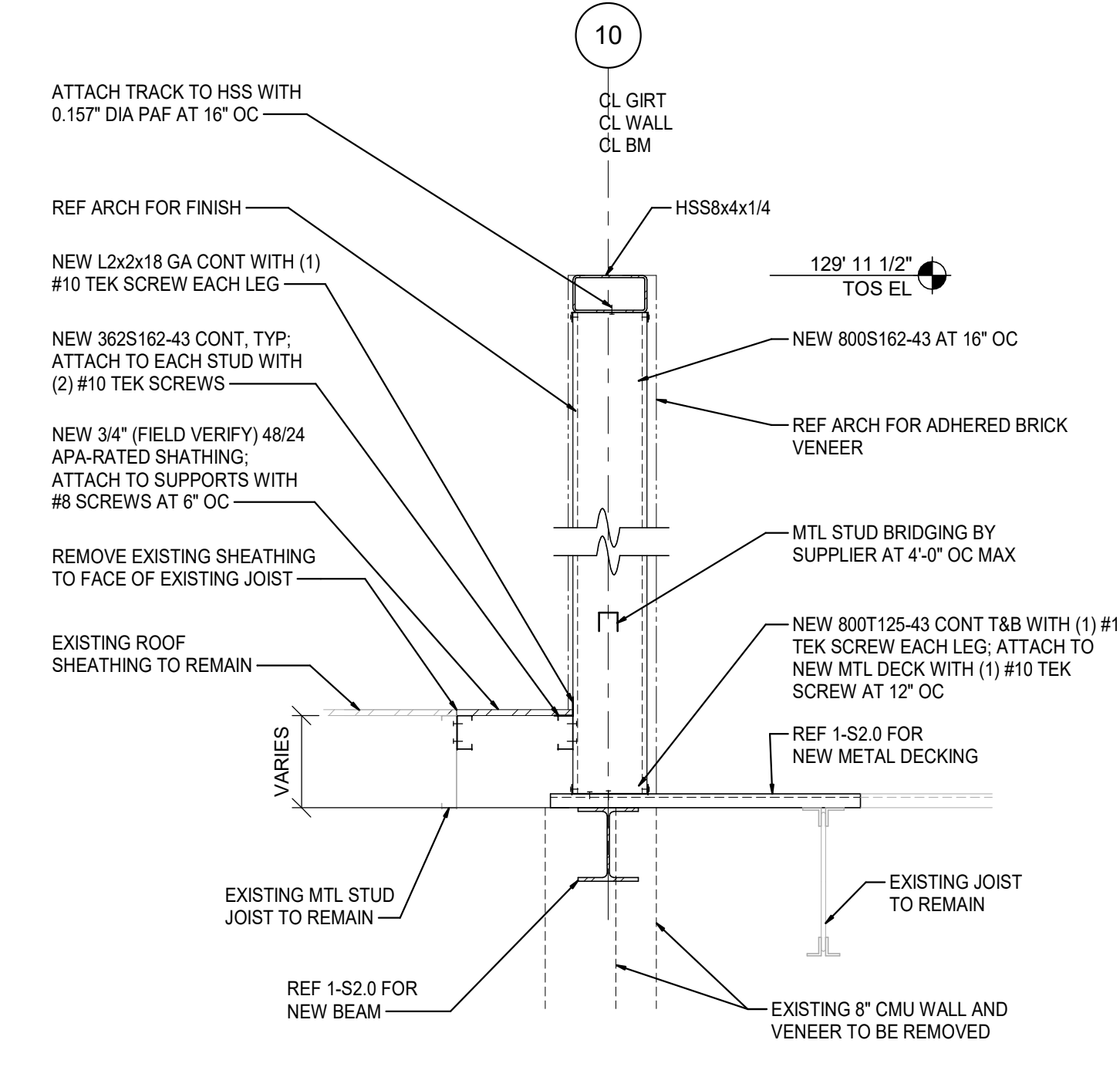
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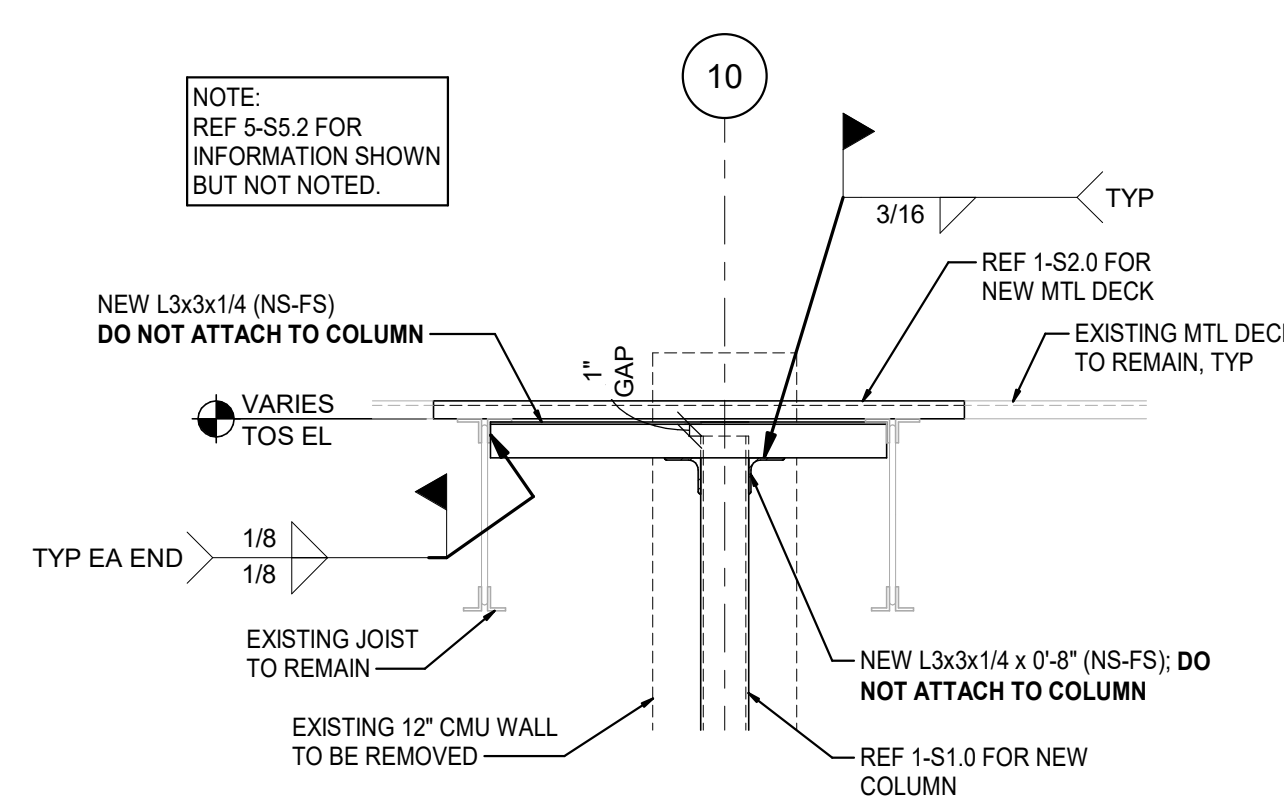
1 NEW SUPPORT FRAMING AT EXIST BEAM/JST GIRDER
SCALE: 3/4" = 1'-0"



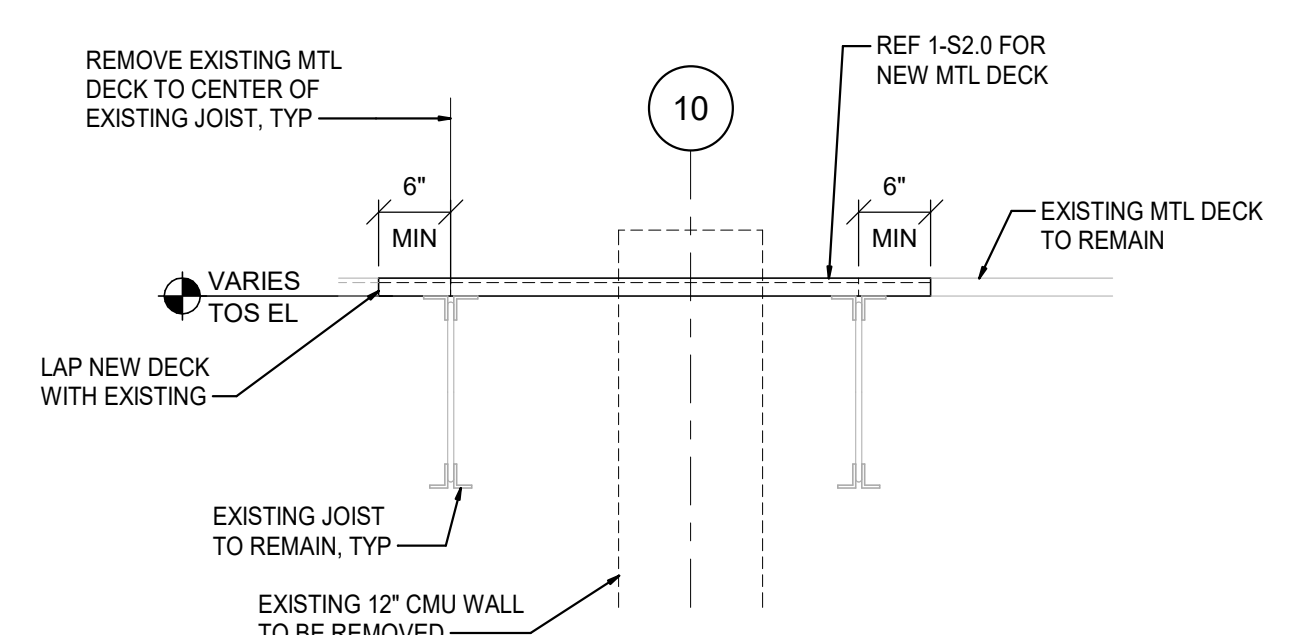
2 NEW COLUMN AT EXISTING ROOF
SCALE: 3/4" = 1'-0"



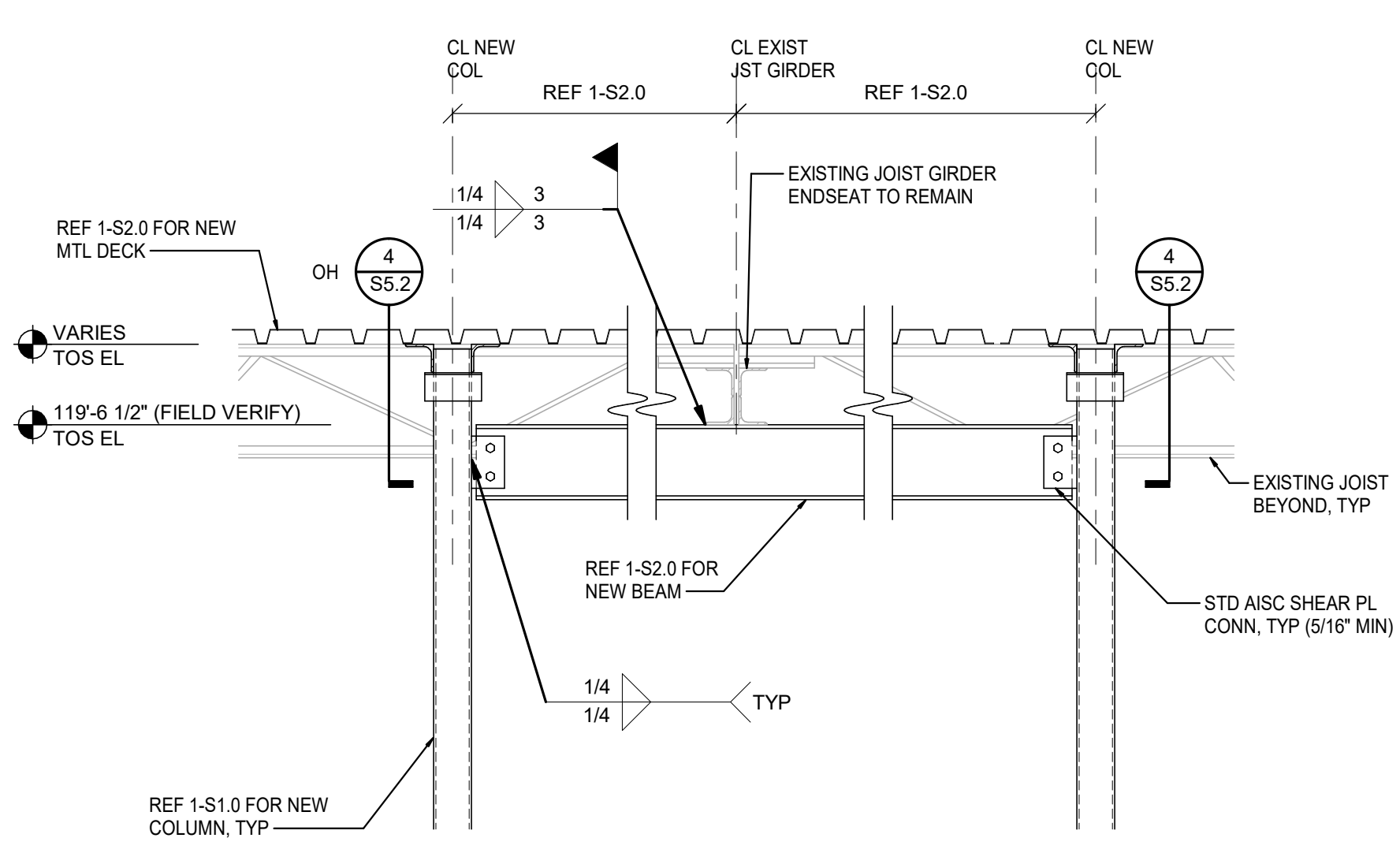
3 NEW PARAPET RETURN WALL
SCALE: 3/4" = 1'-0"



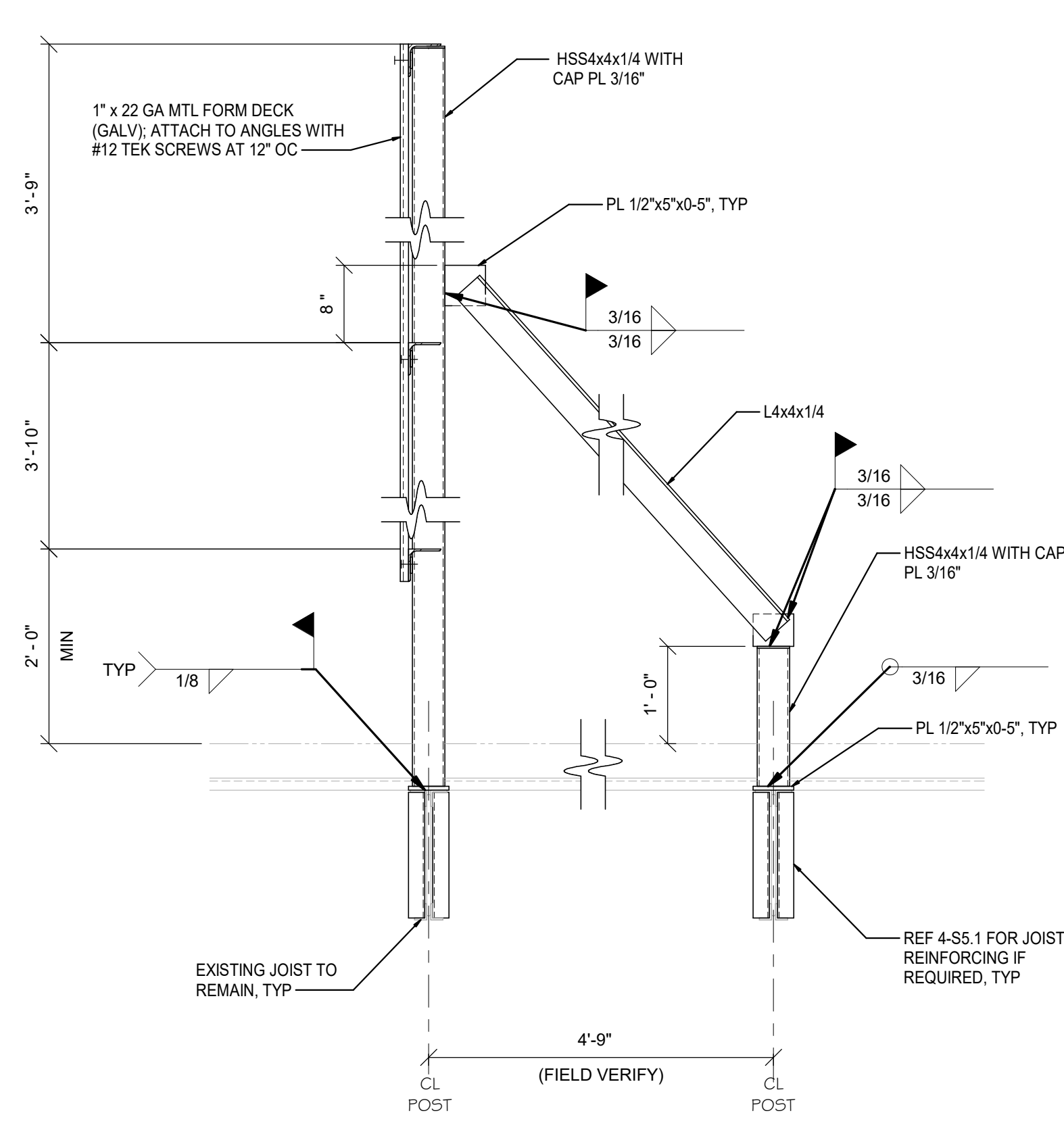
4 NEW COLUMN AT EXISTING ROOF
SCALE: 3/4" = 1'-0"



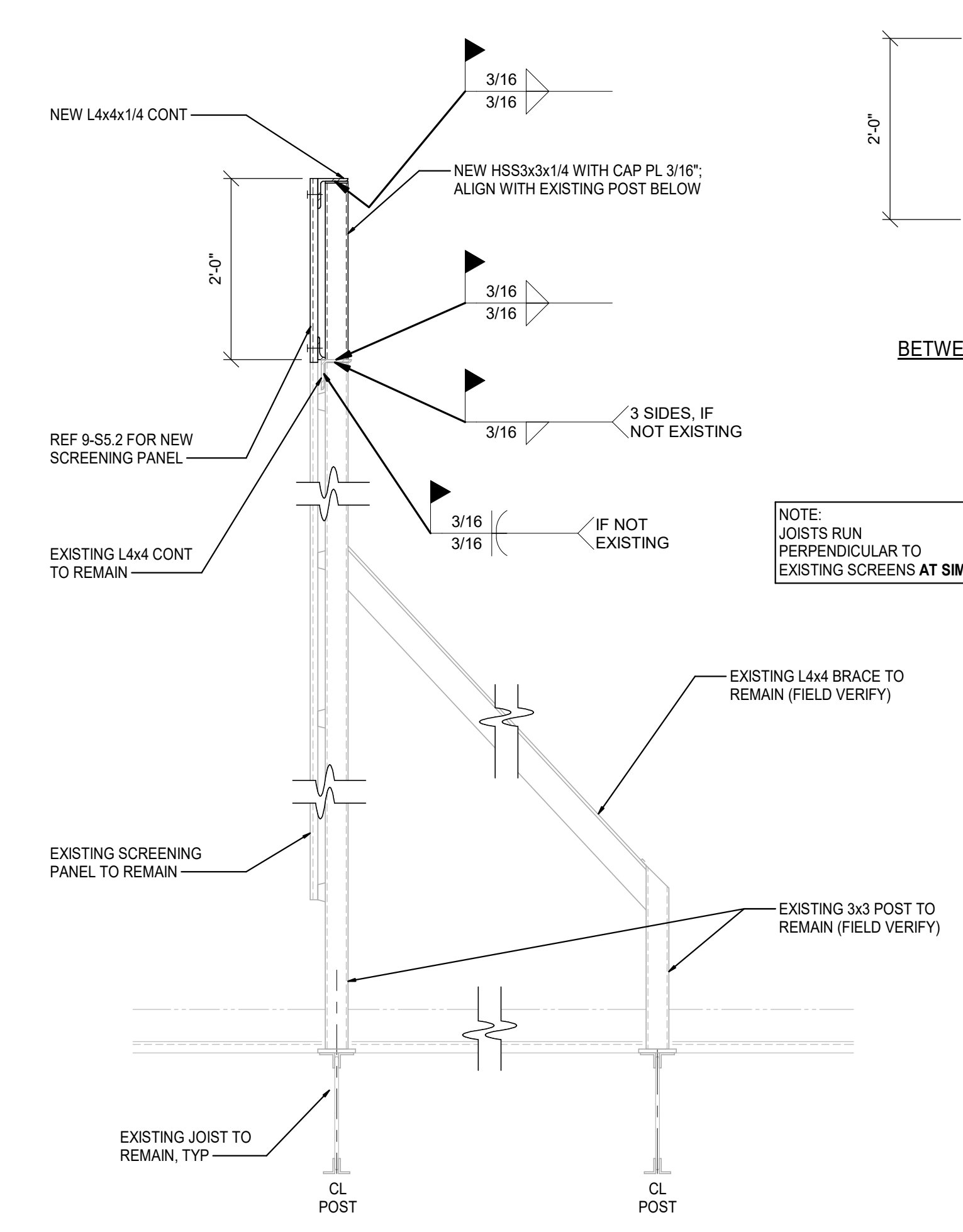
5 NEW DECK BRG AT EXISTING
SCALE: 3/4" = 1'-0"



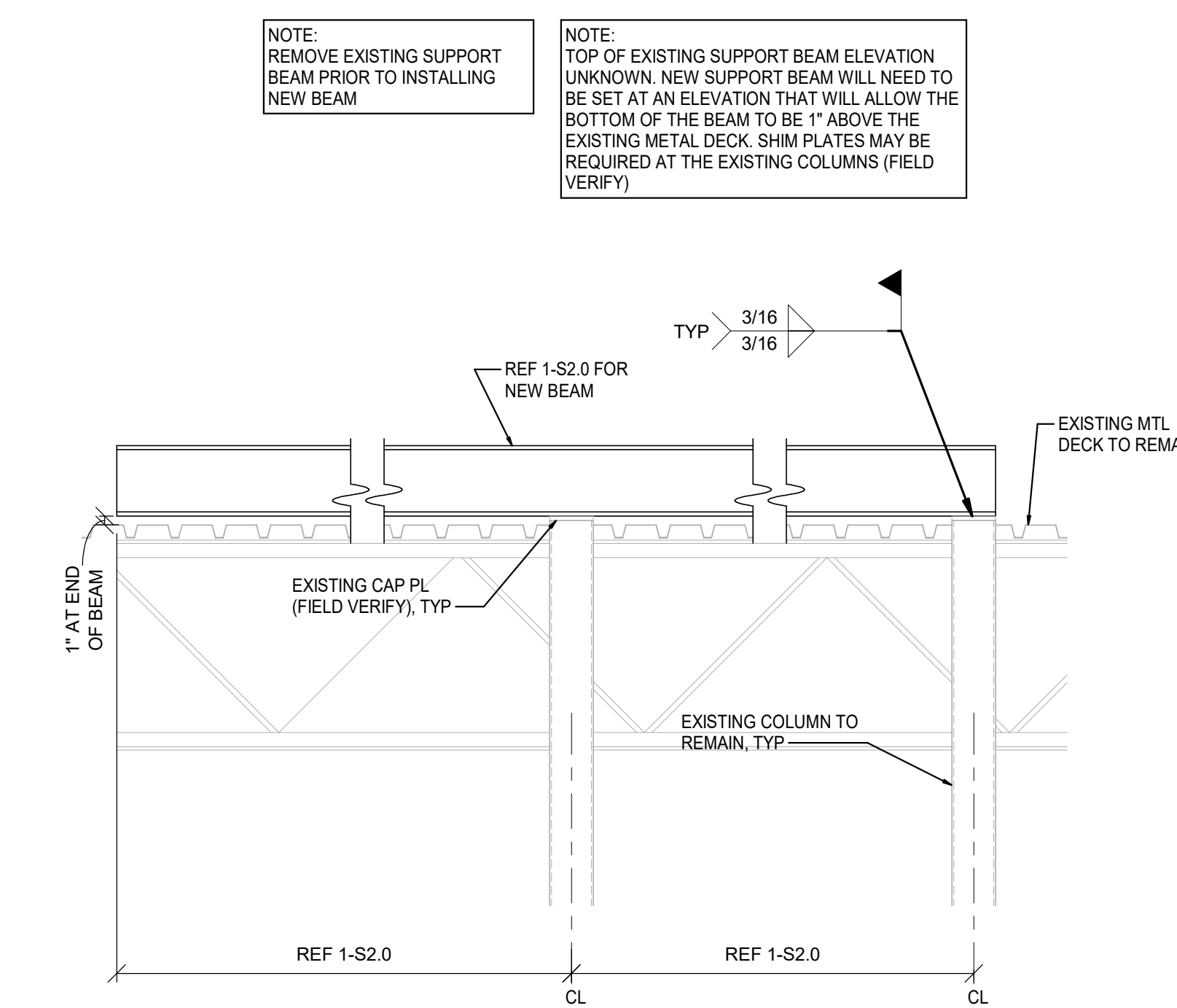
6 NEW SUPPORT FRAMING AT EXISTING JST GIRDER
SCALE: 3/4" = 1'-0"



7 SCREEN WALL
SCALE: 3/4" = 1'-0"



8 SCREEN WALL EXTENSION
SCALE: 3/4" = 1'-0"



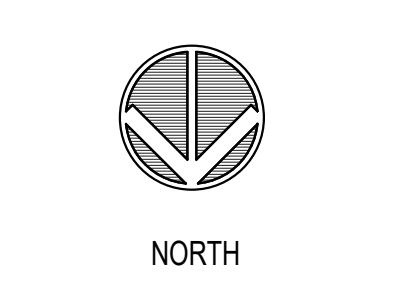
9 RTU-1 SUPPORT BEAM
SCALE: 3/4" = 1'-0"

REVISION	DATE BY

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JBA PROJ # - 20332 - JSH



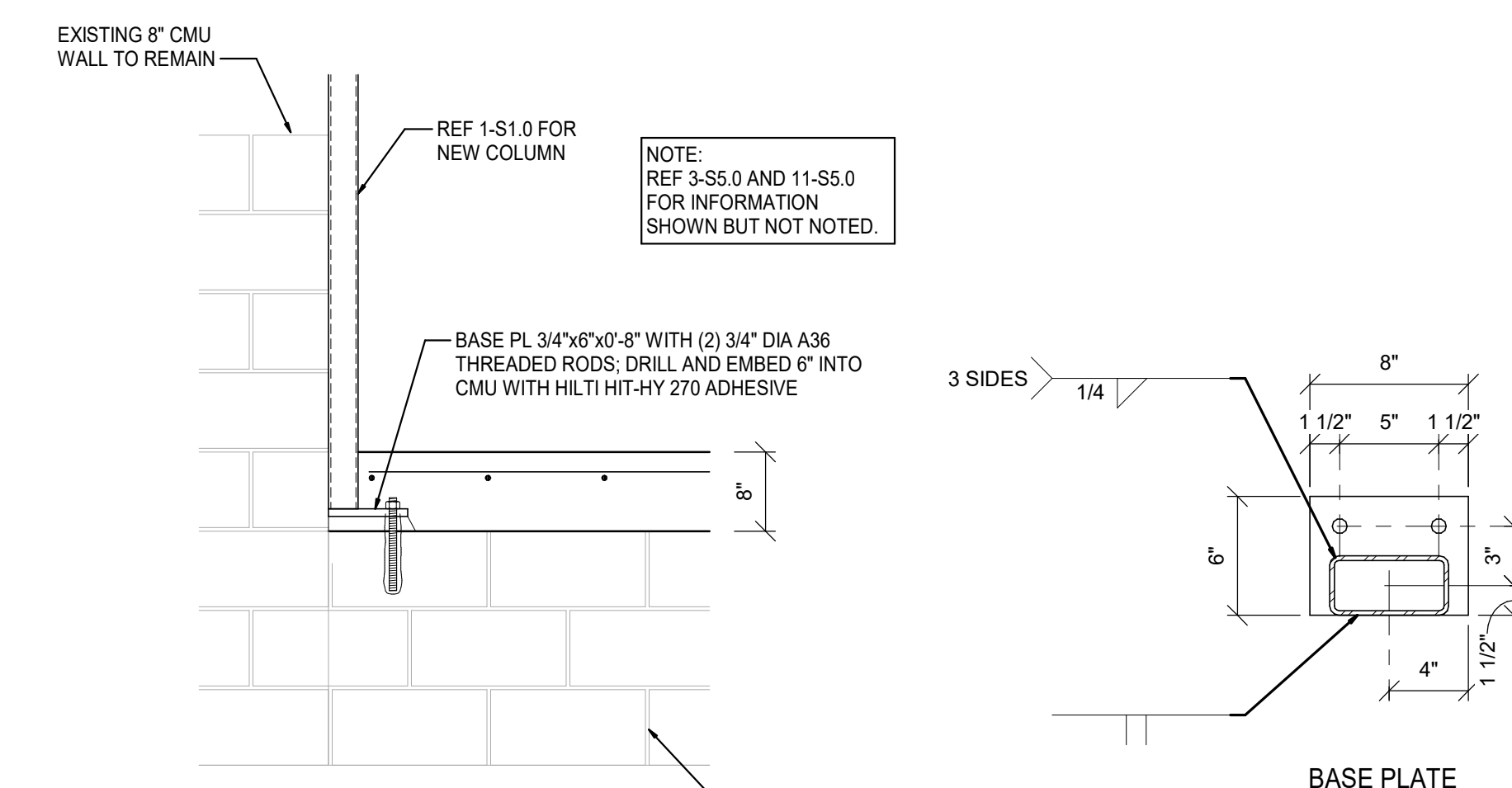
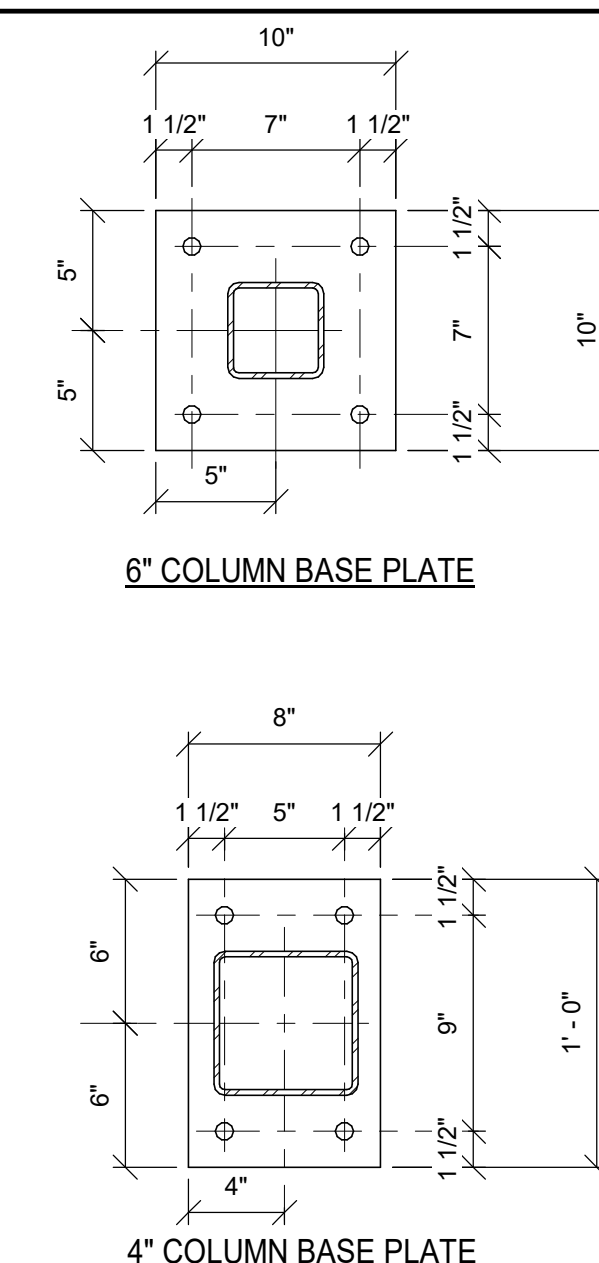
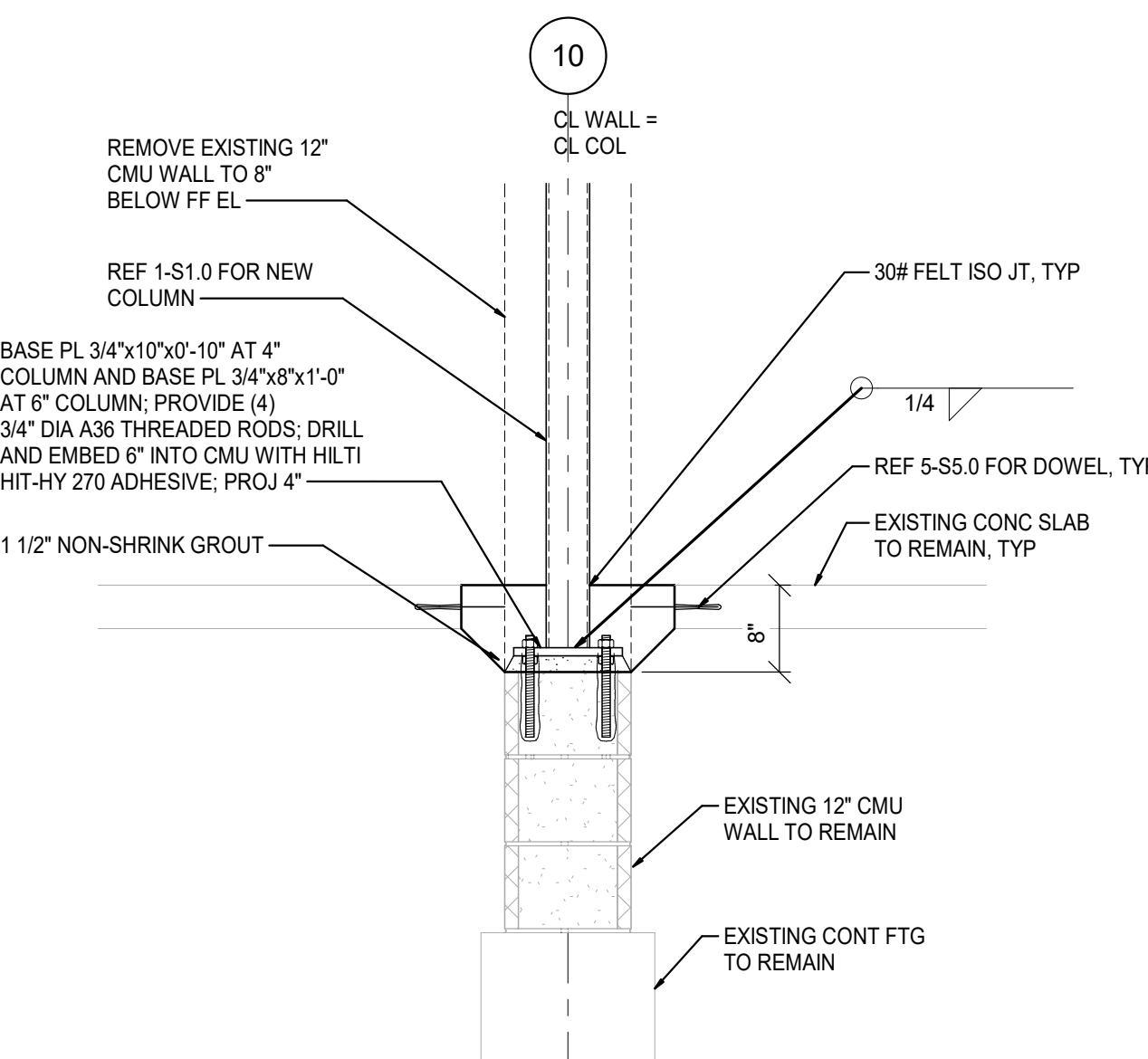
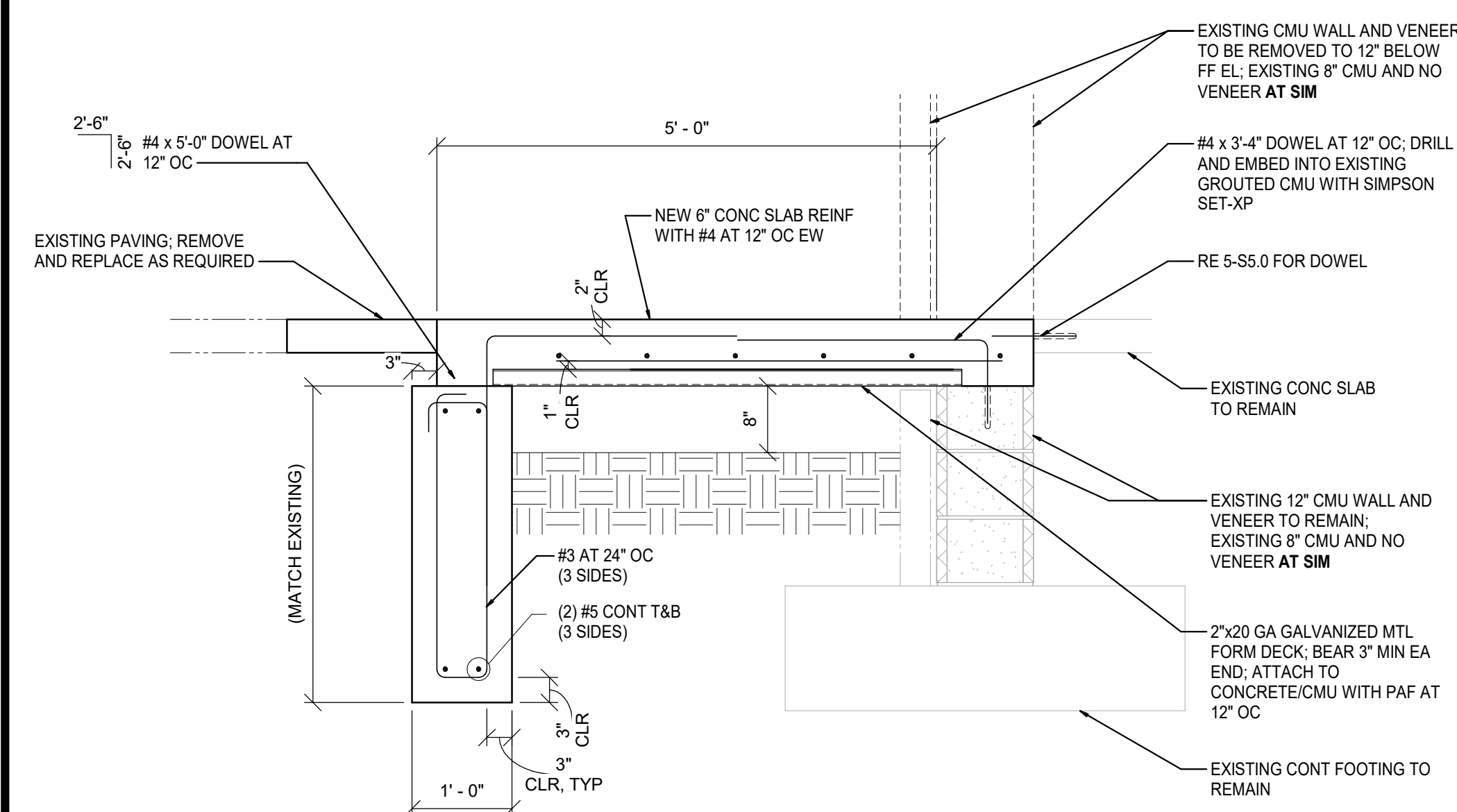
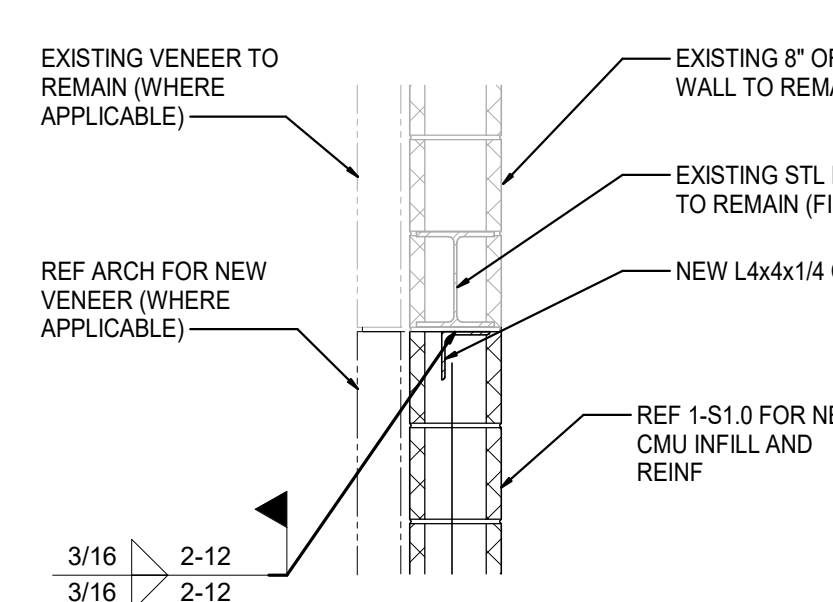
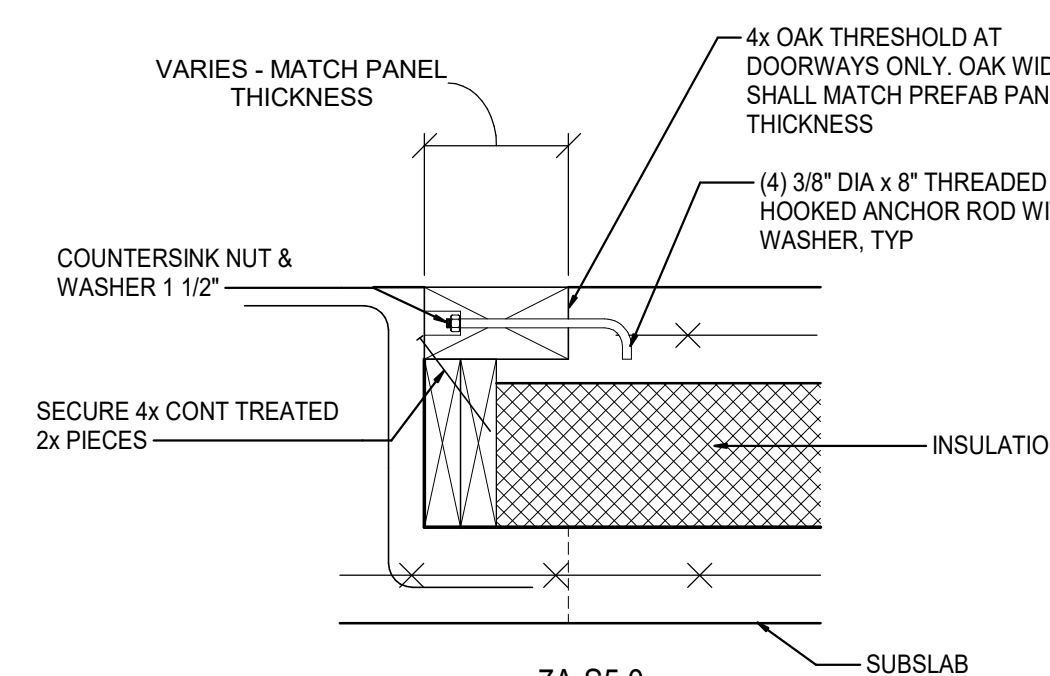
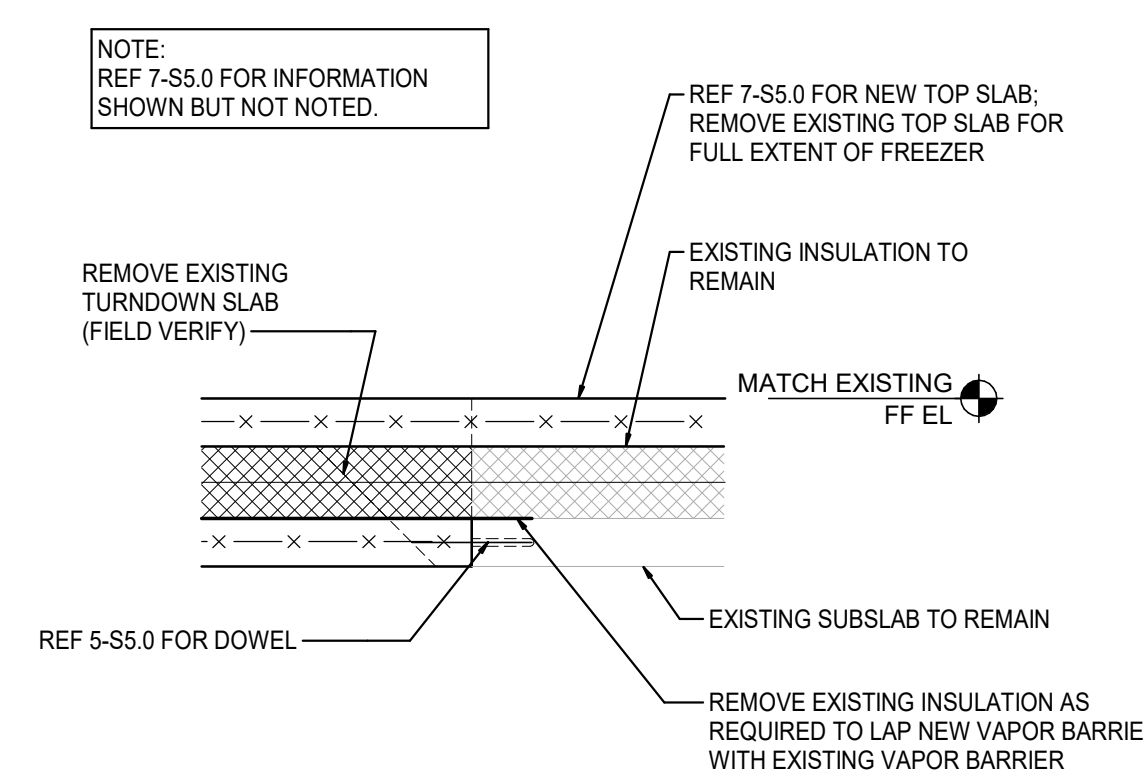
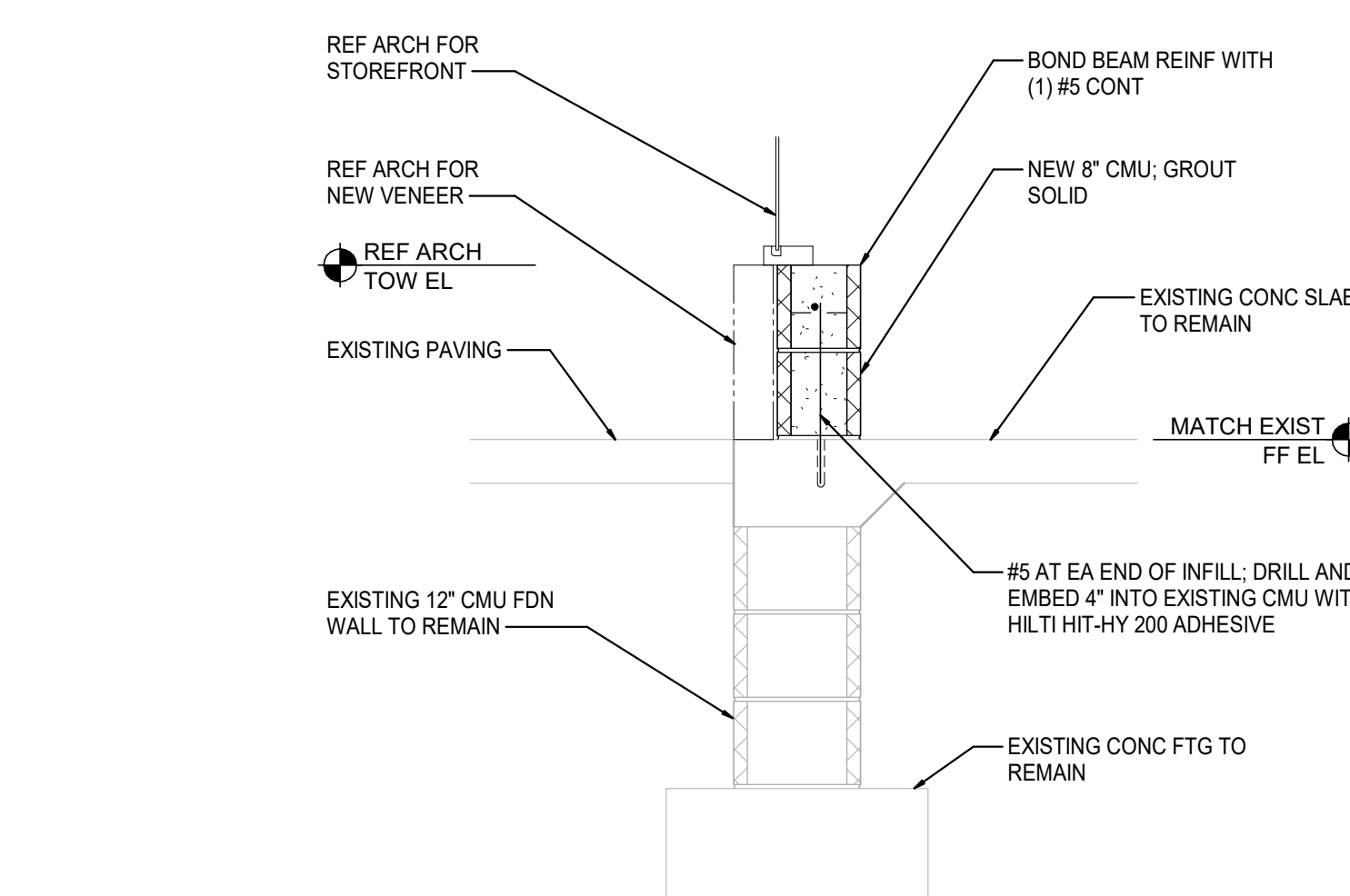
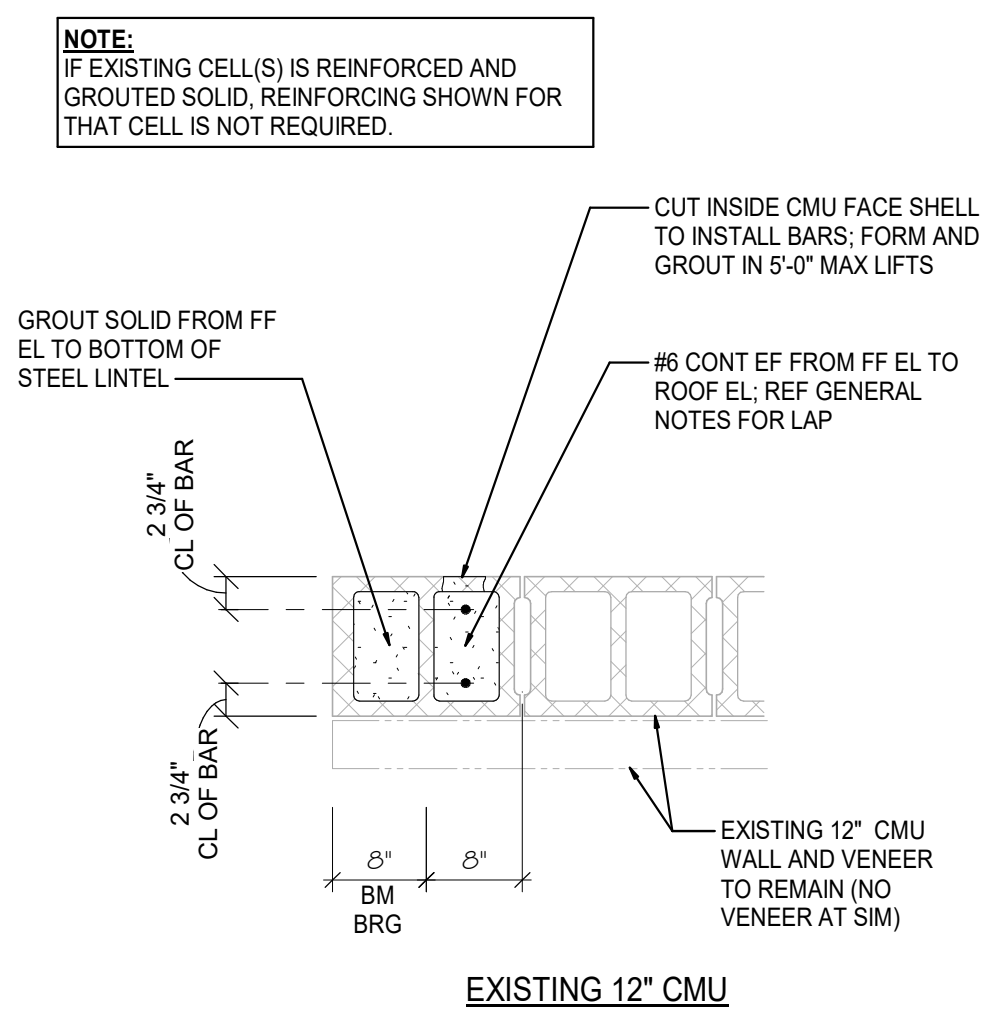
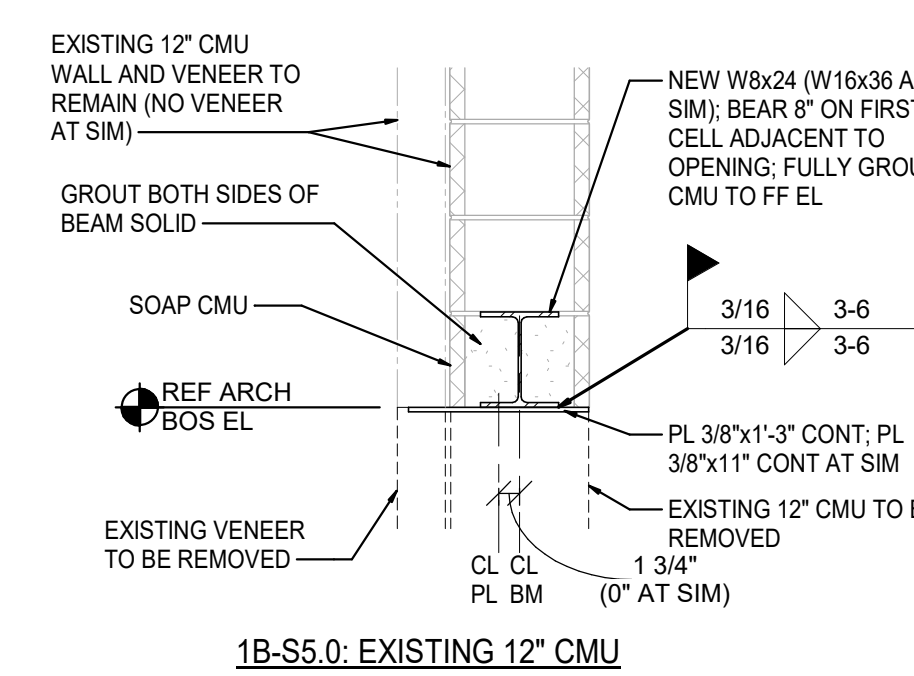
LOCATION
LEE'S SUMMIT, MO
HY-VEE, INC.
5820 WESTOWN PARKWAY
WEST DES MOINES, IOWA 50386
TEL: (515) 267-2600
FAX: (515) 267-2636



STRUCTURAL SECTIONS & DETAILS

PROJECT MANAGER JSH	CHECKED BY Approver
DRAWN BY: Author	DATE: 10/20/2020
SCALE: 3/4" = 1'-0"	JOB NUMBER: 62930547

SHEET:
S5.2



JBA JOHNSTON
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930 CENTRAL - KANSAS CITY, MO 64105
816.421.4200 - WWW.JBAENG.COM
JBA PROJ #: 20332 - JSH

PROFESSIONAL SEAL



LOCATION
LEE'S SUMMIT, MO

Hy-Vee®
EMPLOYEE OWNED



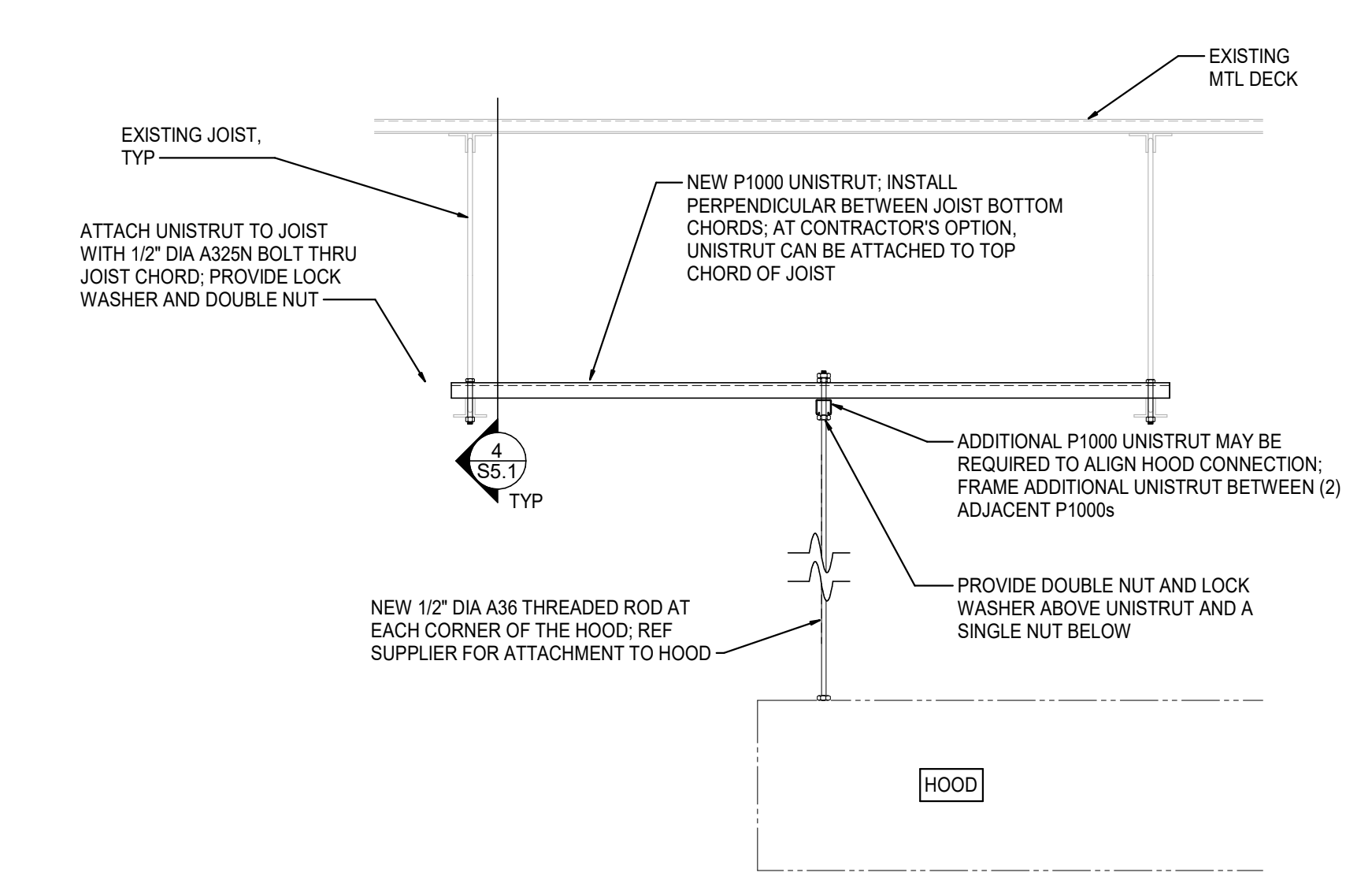
NORTH

STRUCTURAL SECTIONS & DETAILS

PROJECT MANAGER JSH	CHECKED BY: BJH
DRAWN BY: JBA'	DATE: 10/20/2020
SCALE: As indicated	JOB NUMBER: 62930547

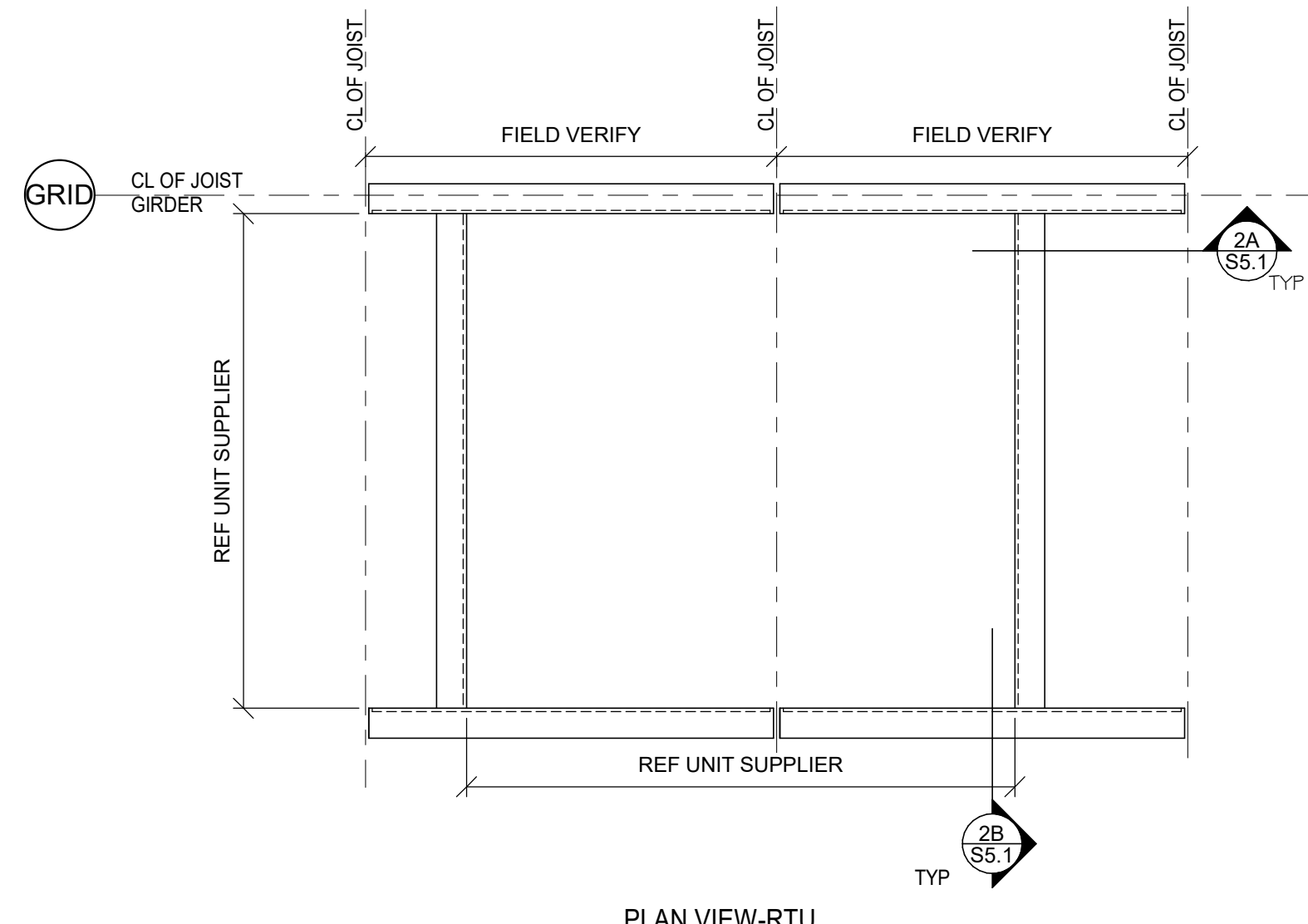
S5.0

10/22/2020 1:40:44 PM



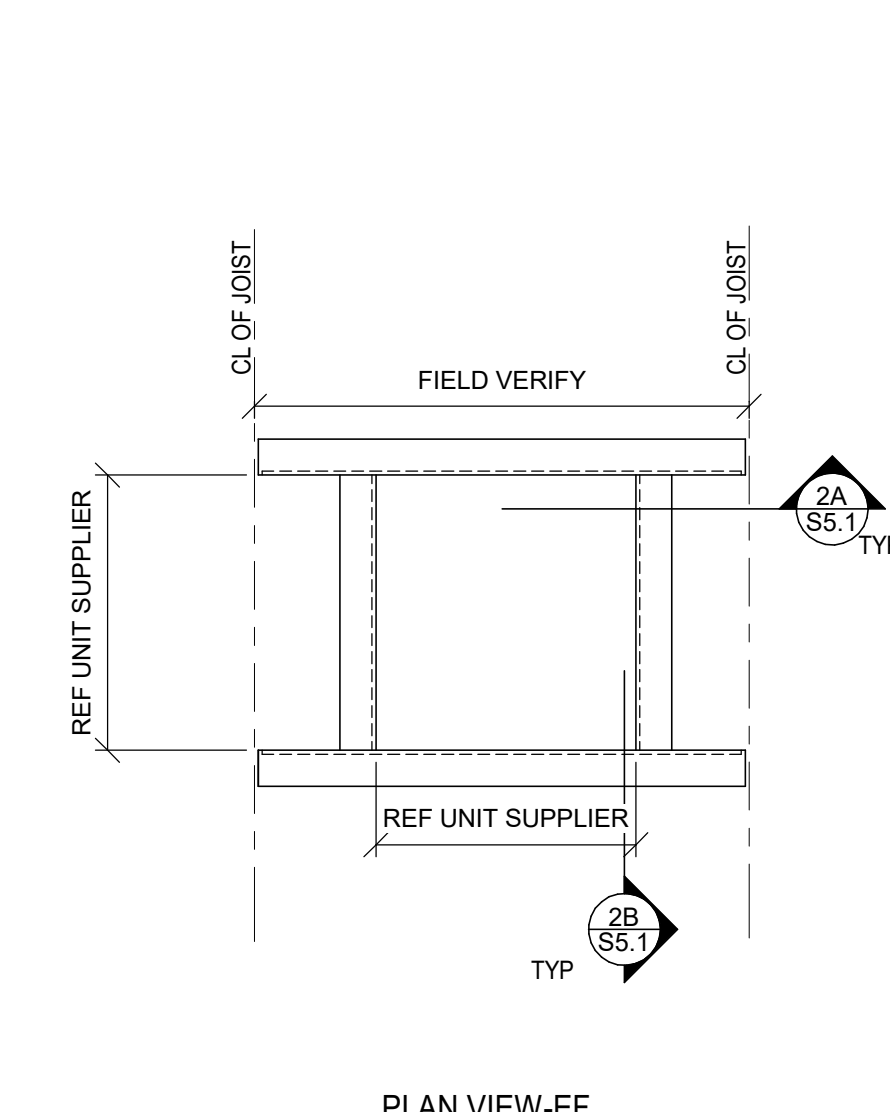
1 SUSPENDED HOOD SUPPORT

SCALE: 3/4" = 1'-0"

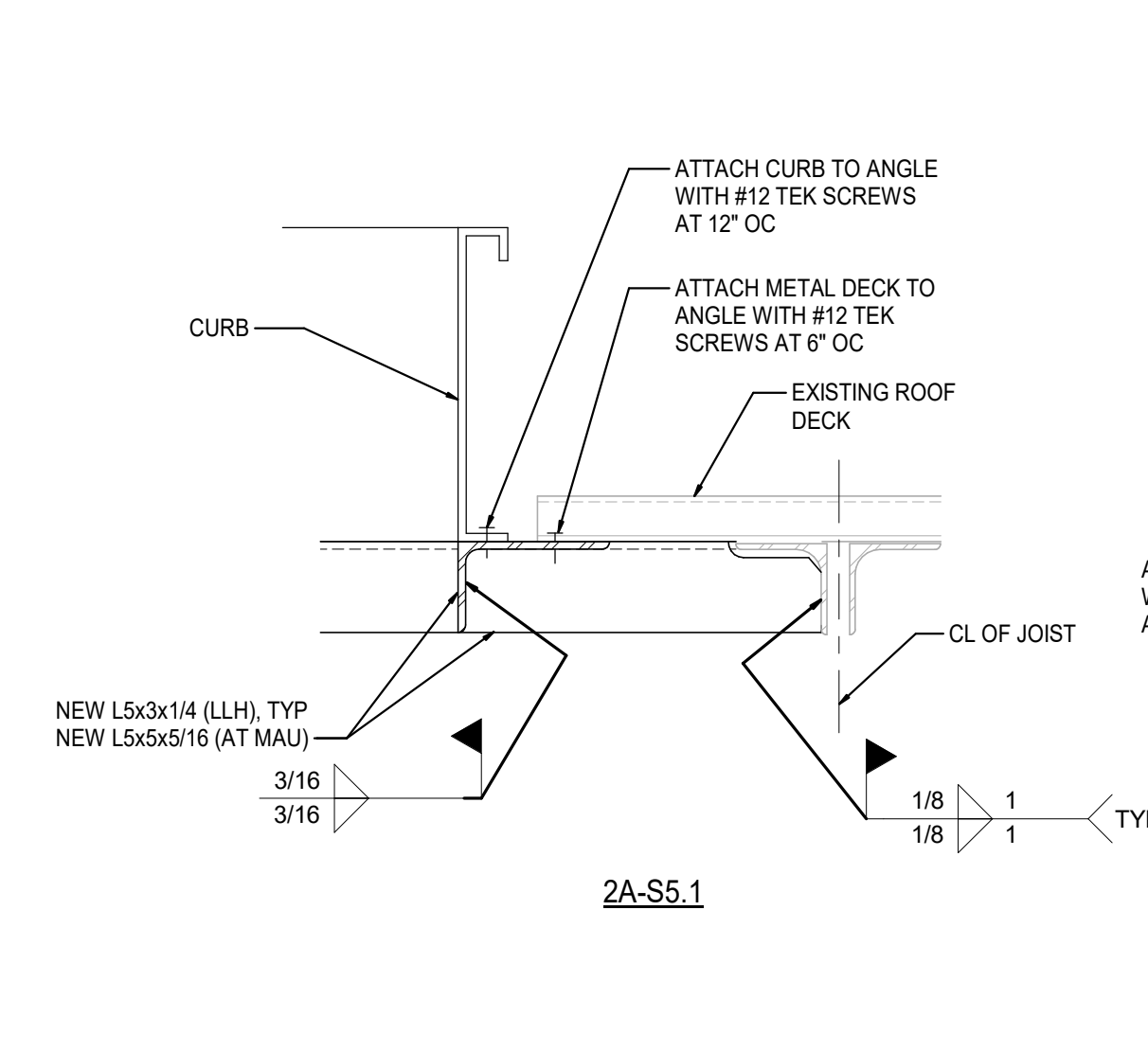


2 RTU/EXHAUST FAN SUPPORT FRAMING

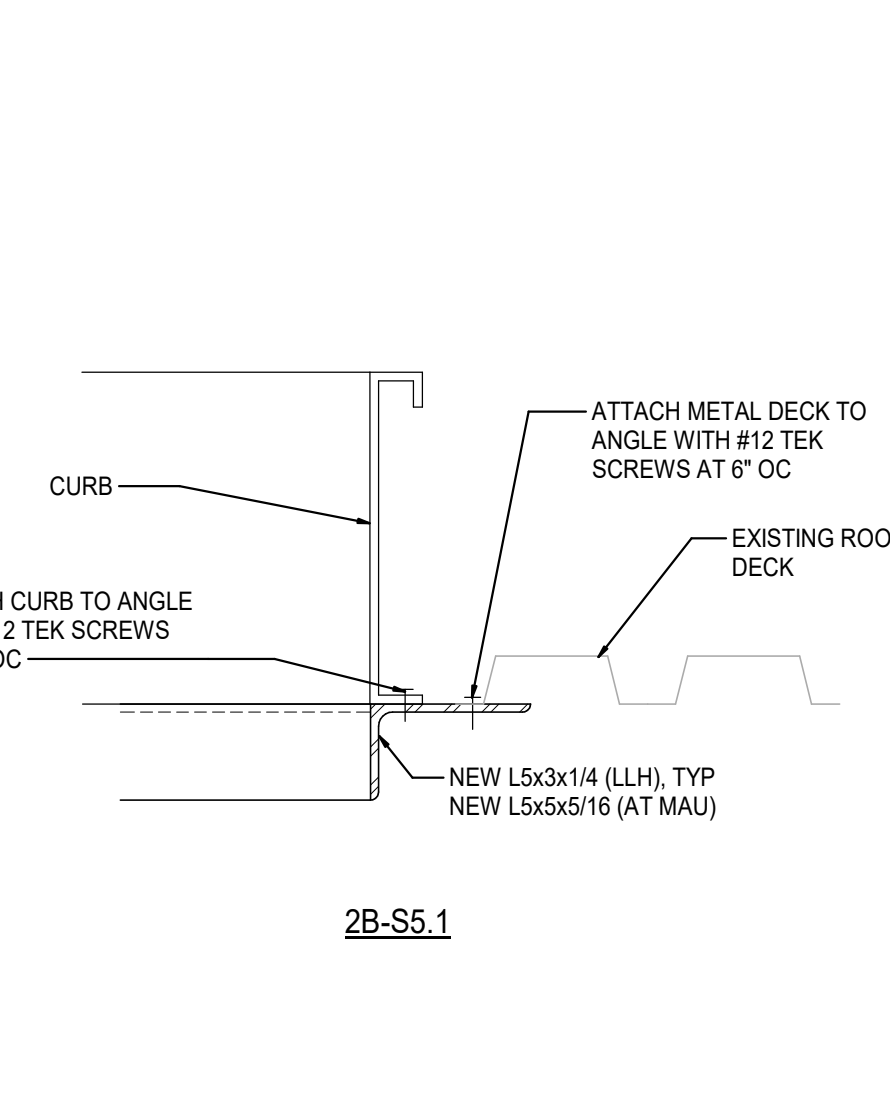
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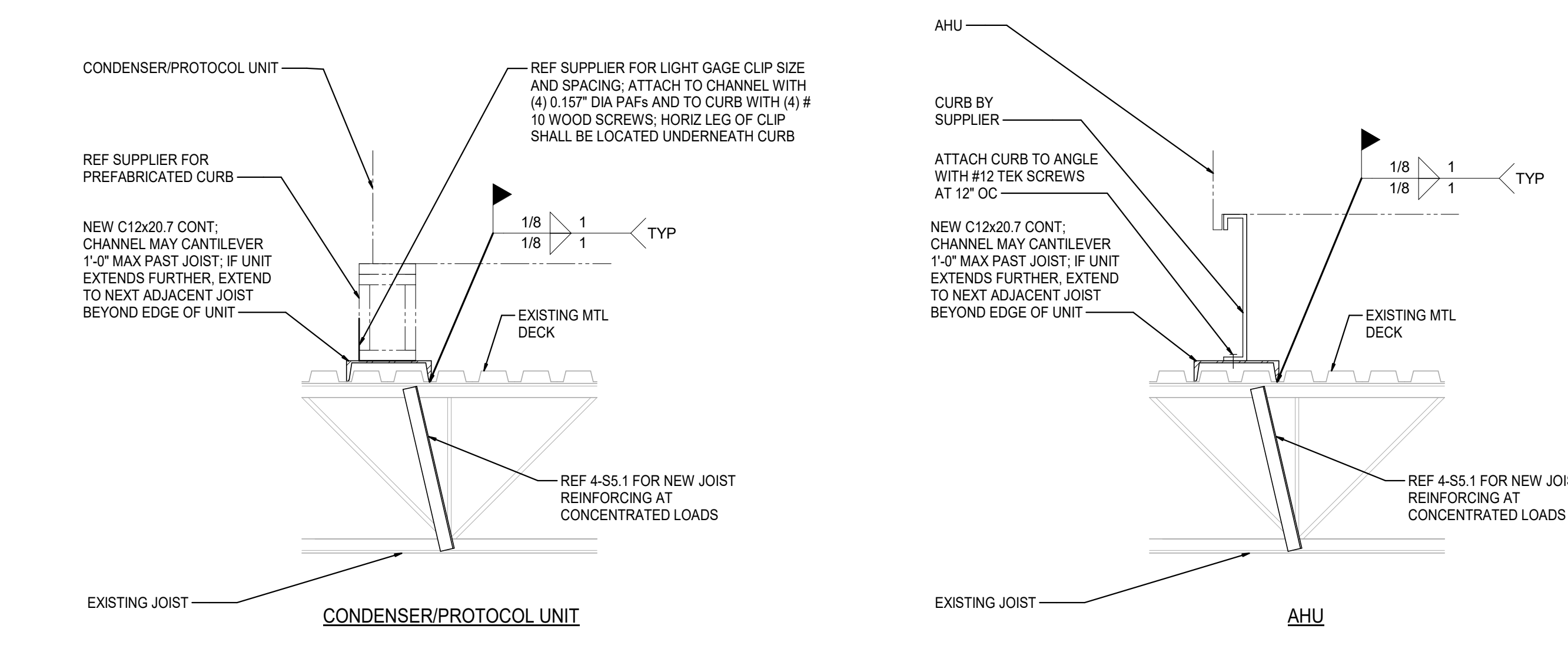
PLAN VIEW-EF



2A-S5.1

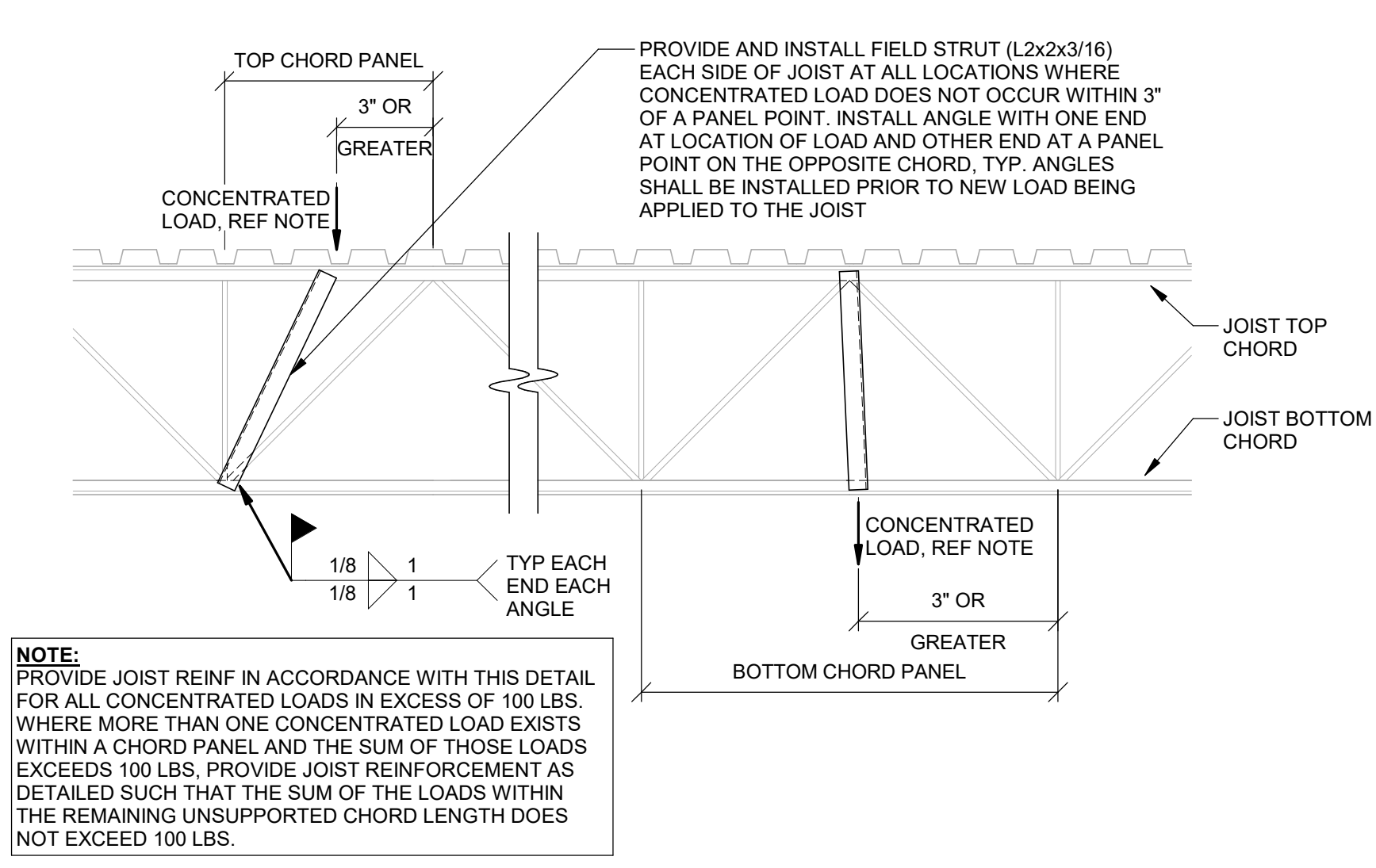


2B-S5.1



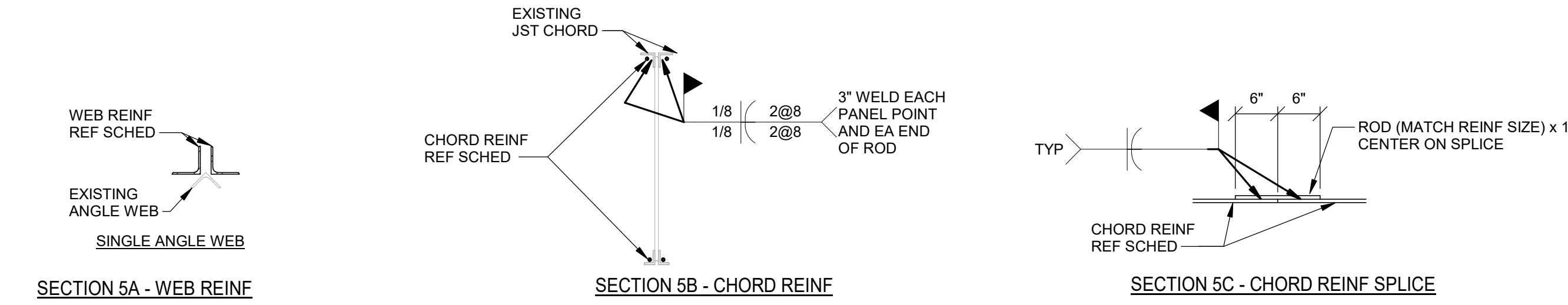
3 CONDENSER/PROTOCOL UNIT/AHU SUPPORT FRAMING

SCALE: 3/4" = 1'-0"

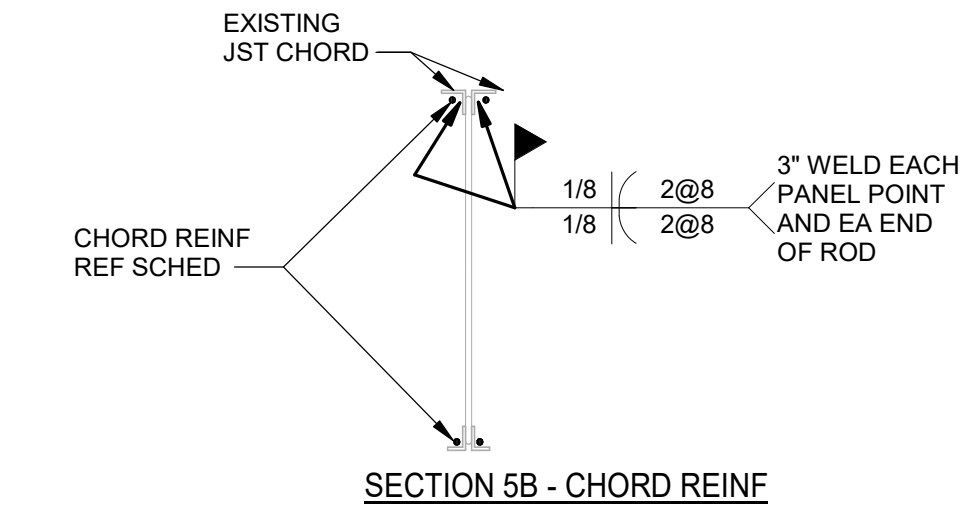


4 JOIST REINFORCING AT CONCENTRATED LOAD

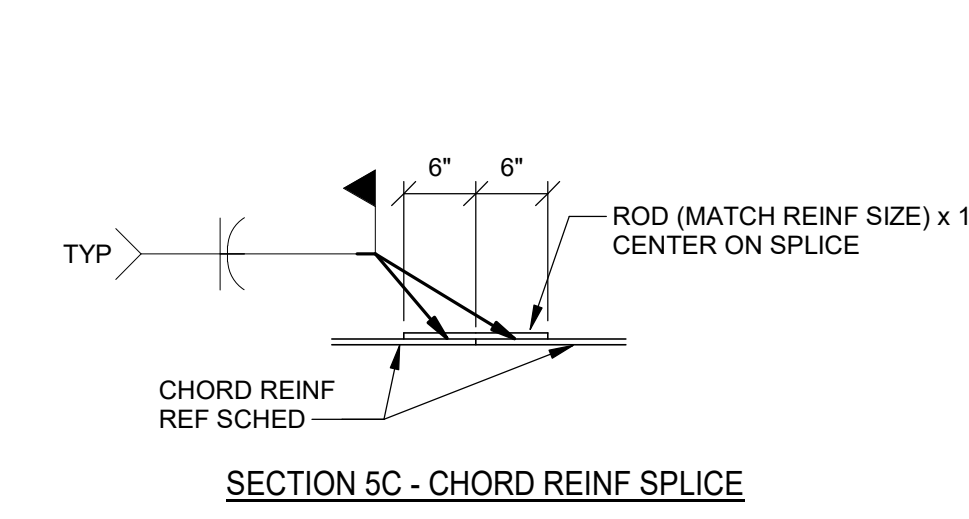
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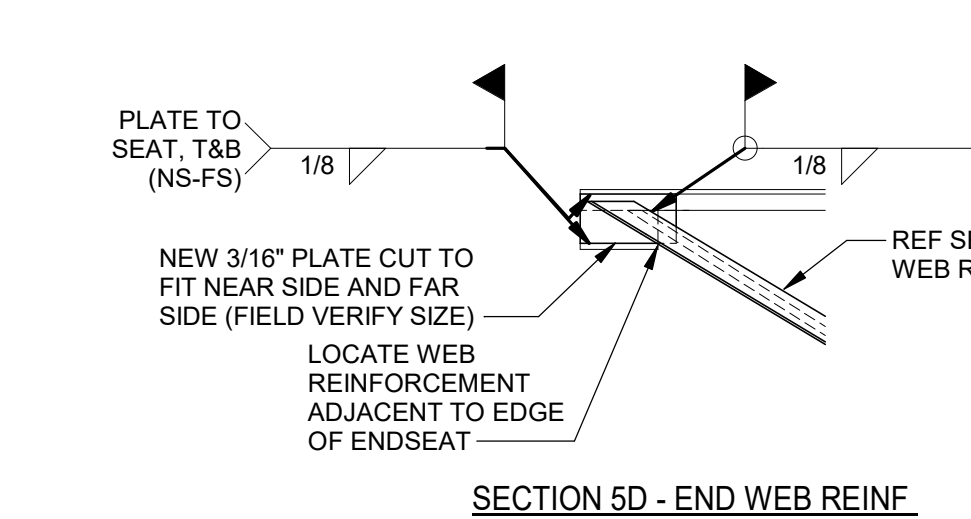
SECTION 5A - WEB REINF



SECTION 5B - CHORD REINF



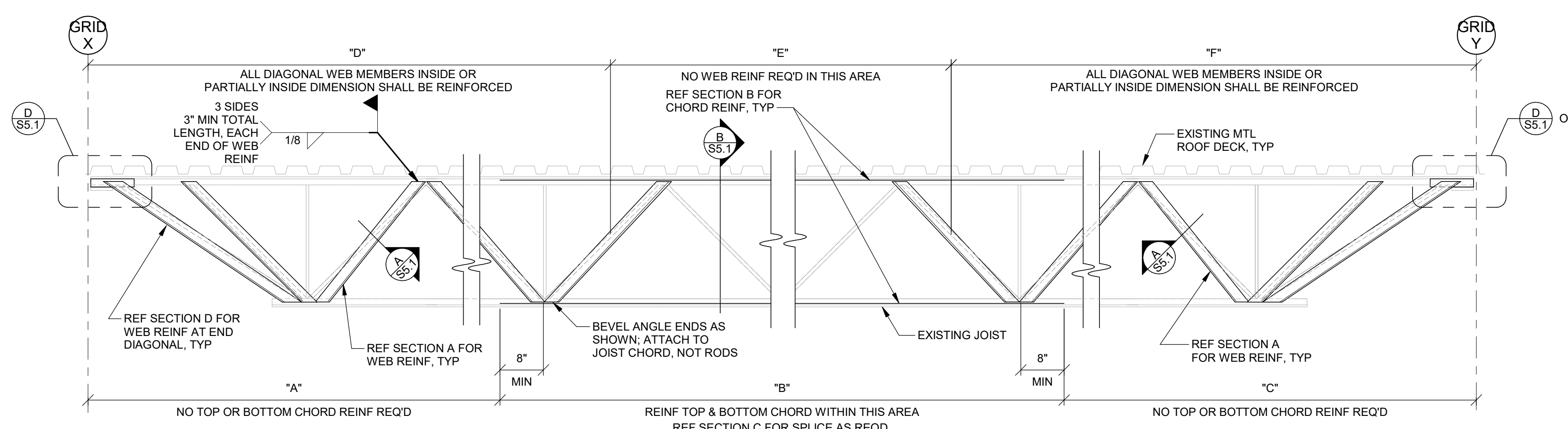
SECTION 5C - CHORD REINF SPLICE



SECTION 5D - END WEB REINF

- JOIST REINFORCEMENT NOTES**
1. JOIST WEB CONFIGURATION IS SHOWN FOR REFERENCE ONLY. FIELD VERIFY ACTUAL CONFIGURATION AND WEB TYPE.
 2. ALL PAINT AND DEBRIS SHALL BE CLEANED FROM JOIST BEFORE WELDING REINFORCEMENT.
 3. STEEL ERECTOR SHALL FIELD CUT REINFORCING TO FIT FROM STOCK LENGTHS.
 4. EXTEND TOP AND BOTTOM CHORD REINFORCING A MINIMUM OF 8" BEYOND PANEL POINTS.
 5. INSTALL WEB REINFORCING PRIOR TO CHORD REINFORCING. COPE OUTSTANDING LEG OF WEB REINFORCING AS REQUIRED FOR PLACEMENT OF CHORD REINFORCING.
 6. IF EXISTING JOIST BRIDGING INTERFERES WITH INSTALLATION OF REINFORCING, REMOVE BRIDGING AND REPLACE IMMEDIATELY UPON COMPLETION OF REINFORCING INSTALLATION. IF BRIDGING TABS FOR BOLTED BRIDGING INTERFERE WITH INSTALLATION OF REINFORCING, REMOVE TABS AND REPLACE IMMEDIATELY UPON COMPLETION OF REINFORCING INSTALLATION. NEW CONNECTIONS SHALL MATCH EXISTING.
 7. JOISTS SHALL BE REINFORCED PRIOR TO INSTALLATION OF MECHANICAL UNITS OR HANGING LOADS. ROOF SNOW LOAD SHALL BE REMOVED PRIOR TO JOIST REINFORCEMENT. REF 4-S5.1 FOR ADDITIONAL REINFORCEMENT REQUIRED AT CONCENTRATED LOADS.

JOIST REINFORCING SCHEDULE									
JOIST REINF MARK	CHORD REINF	WEB REINF	"A"	"B"	"C"	"D"	"E"	"F"	NOTES
JR1	-	(2) L2x2x3/16	0'-0"	0'-0"	0'-0"	10'-0"	0'-0"	0'-0"	1,2,3,6,7
JR2	-	(2) L2x2x3/16	0'-0"	0'-0"	0'-0"	10'-0"	0'-0"	0'-0"	1,2,3,6,7
JR3	-	(2) L2x2x3/16	0'-0"	0'-0"	0'-0"	10'-0"	0'-0"	0'-0"	1,2,3,6,7
JR4	-	(2) L2x2x3/16	0'-0"	0'-0"	0'-0"	13'-0"	0'-0"	0'-0"	1,2,3,6,7
JR5	(2) 1/2" DIA RODS	(2) L2x2x3/16	6'-0"	32'-9"	6'-0"	19'-0"	6'-9"	20'-0"	1,2,3,4,5,6,7



5 JOIST REINFORCEMENT

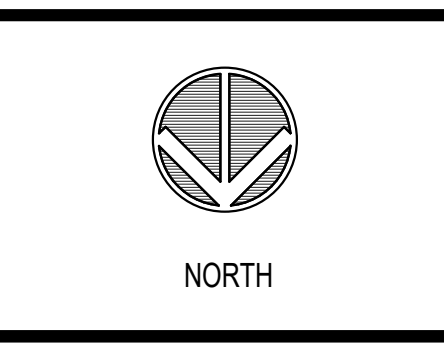
SCALE: 3/4" = 1'-0"

REVISION	DATE BY

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816-421-4200 - www.jbausa.com
JBA PROJ # : 20332 - JSM

PROFESSIONAL SEAL
STATE OF MISSOURI
BART HALVORSON
REGISTERED PROFESSIONAL ENGINEER
NO. 000000000
10/22/20

LOCATION
LEE'S SUMMIT, MO
HY-VEE INC.
5820 WEST TOWN PARKWAY
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TEL: (515) 267-2600
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