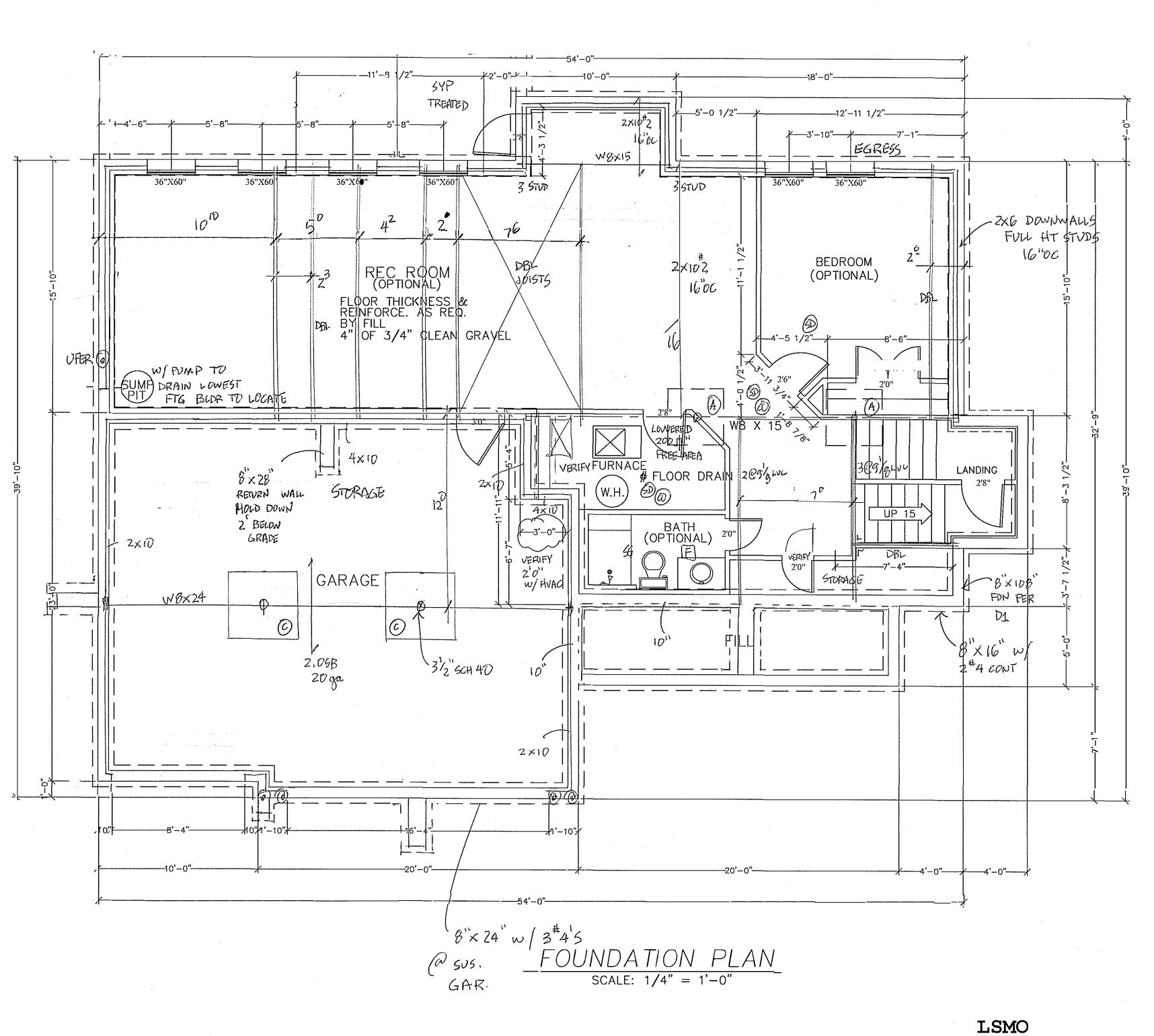


BEAMS
FIELD VERIFY LENGTH
LENGTH SIZE
24'11" W8 X 15
30'0" W18X45

2 POSTS ADJUSTIBLE



(A) 36x36x12 PAD W/ (6) #4'8 E.W. 3" SCH 40 COL, ALL PADS UNO

B 42x42x14 PA0 W/ (7) #4's E.W.

@ 48x48x16 PAD

MODEL:

BRANTLY

DATE:

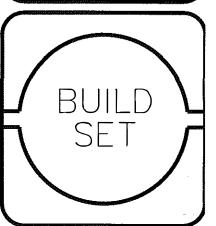
8 /29 /2

FOUNDATION

DESCRIPTION:

ARCHITECT IS NOT RESPONSIBLE FOR THE STRUCTURAL ELEMENTS OF THESE PLANS. A STRUCTURAL ENGINEER MAY NEED TO VERIFY ALL STRUCTURAL ASPECTS OF THESE PRINTS BEFORE CONSTRUCTION BEGINS. FIELD CONSTRUCTION BEGINS. FIELD

ENGINEER MAY NEED TO VERIFY ALL STRUCTURAL ASPECTS OF THESE PRINTS BEFORE CONSTRUCTION BEGINS. FIELD CONDITIONS MAY BE DIFFERENT FROM PLAN. ALL STATE AND LOCAL CODES TAKE PRECIDENCE OVER THESE PLANS. CONTRACTOR WILL BE RESPONSIBLE FOR PLAN INTEGRITY AND CODE COMPLIANCE



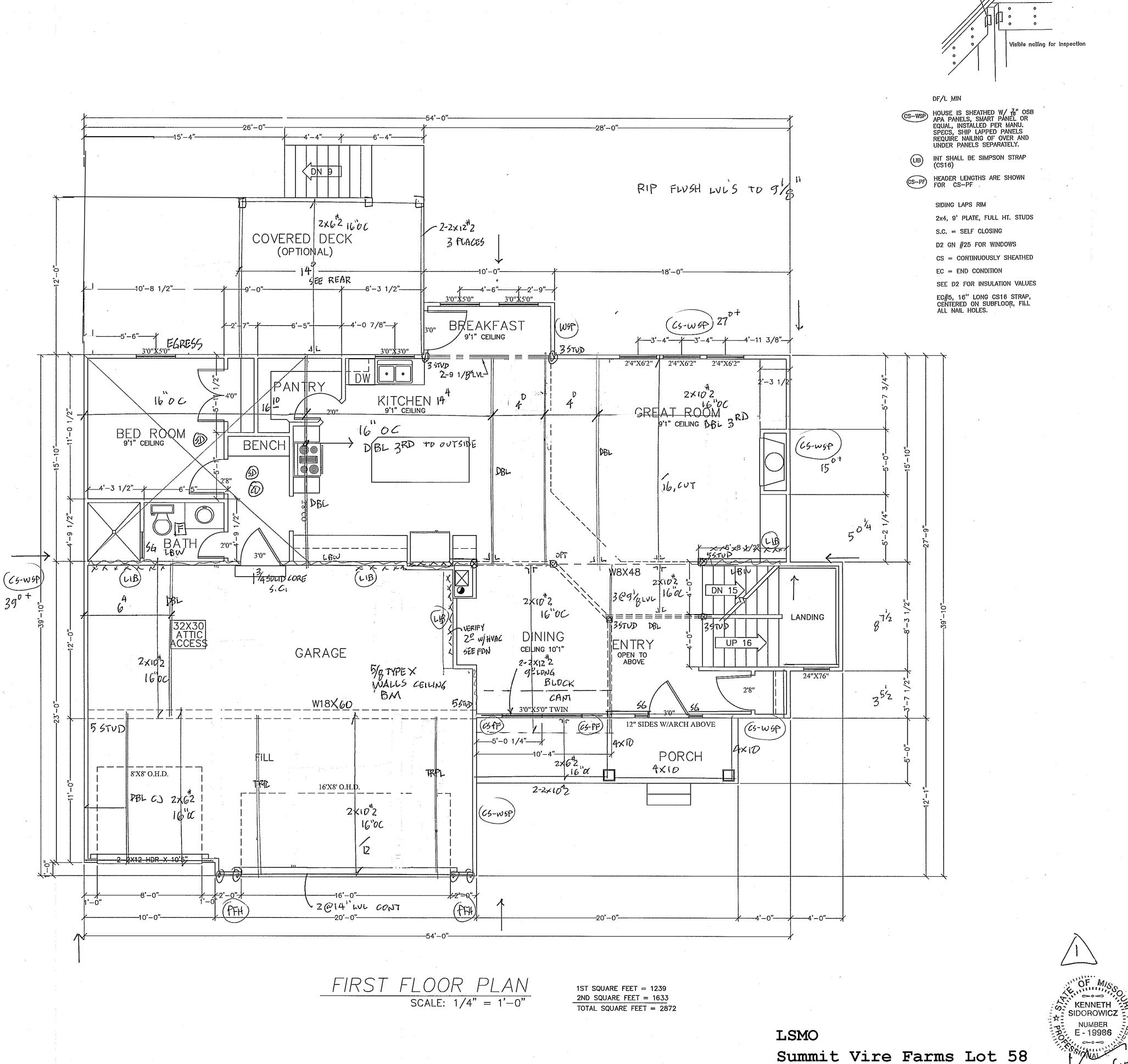
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KENNETH

NUMBER E - 19986

3 of 6

Summit View Farms Lot 58 3133 Blue Ribbon Rd.



KENNETH SIDOROWICZ

3313 SW Blue Ribbon Rd.

2 @ A35, 1 each Hdr.—

FRAMING

FLOOR

MODEL:

BRANTLY

DATE:

ARCHITECT IS NOT RESPONSIBLE FOR THE STRUCTURAL ELEMENTS OF

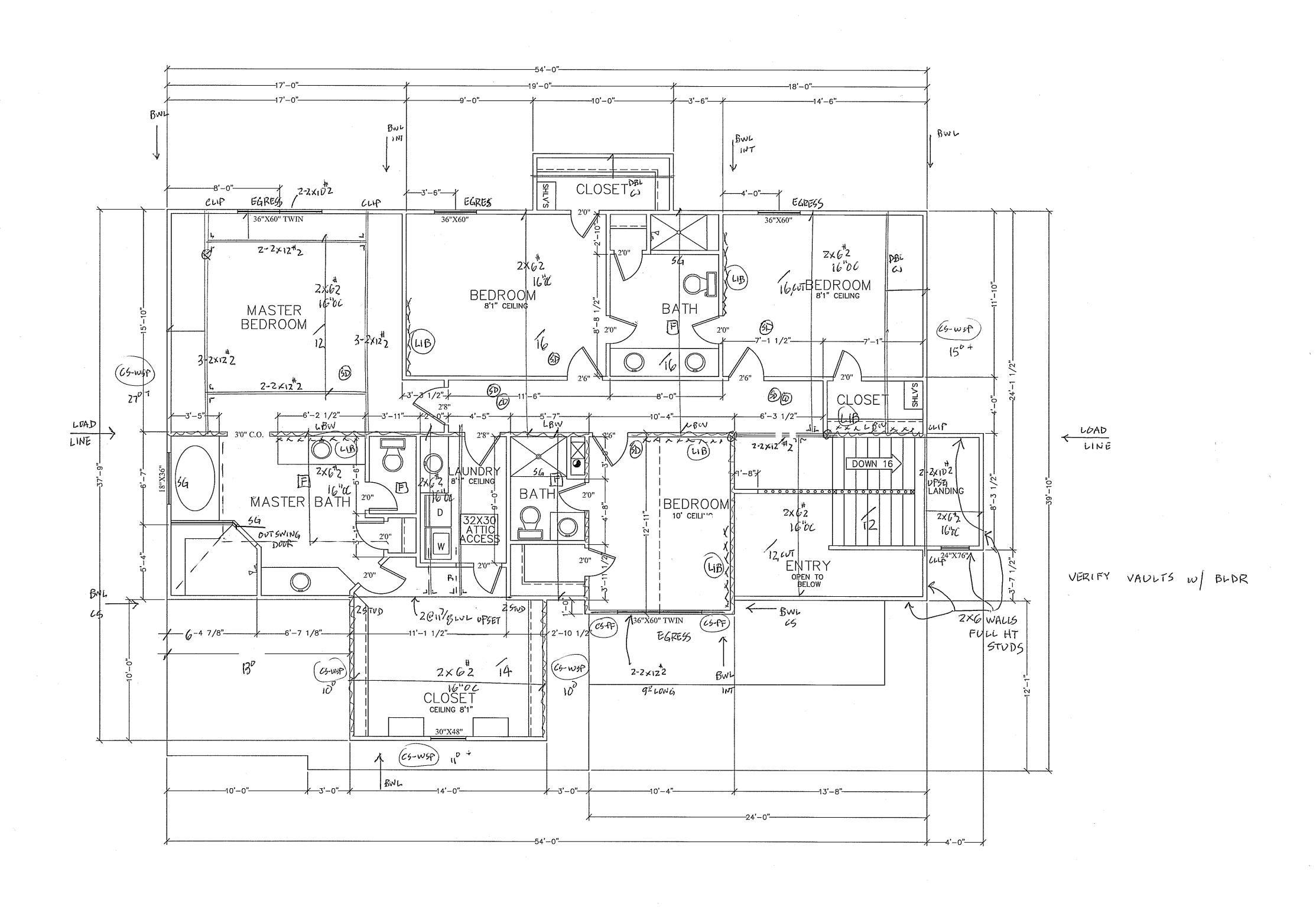
THESE PLANS. A STRUCTURAL

ENGINEER MAY NEED TO VERIFY ALL STRUCTURAL ASPECTS OF THESE PRINTS BEFORE CONSTRUCTION BEGINS. FIELD CONDITIONS MAY BE DIFFERENT FROM PLAN. ALL STATE AND LOCAL CODES TAKE PRECIDENCE OVER THESE PLANS. CONTRACTOR WILL BE

RESPONSIBLE FOR PLAN INTEGRITY AND CODE COMPLIANCE

DESCRIPTION

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2ND SQUARE FEET = 1633

SECOND FLOOR PLAN SCALE: 1/4" = 1'-0"

LSMO
Summit View Farms Lot 58
3133 Blue Ribbon Rd.



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FLOOR FRAMING FRAMING PLAN

SECOND ROOF

ARCHITECT IS NOT RESPONSIBLE FOR THE STRUCTURAL ELEMENTS OF THESE PLANS. A STRUCTURAL

ENGINEER MAY NEED TO VERIFY ALL STRUCTURAL ASPECTS OF THESE PRINTS BEFORE CONSTRUCTION BEGINS. FIELD

CONDITIONS MAY BE DIFFERENT FROM PLAN. ALL STATE AND

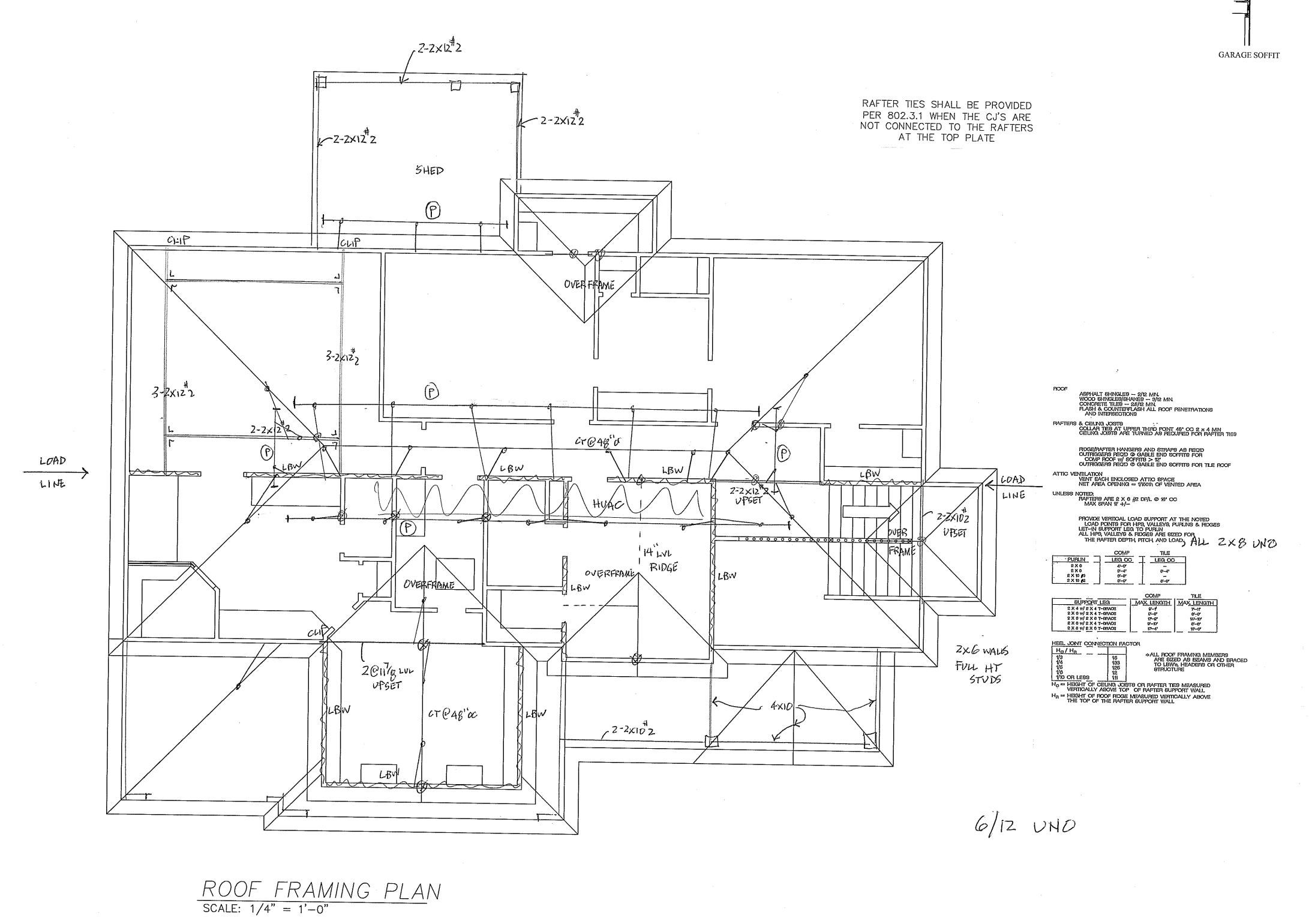
THESE PLANS. CONTRACTOR WILL BE

RESPONSIBLE FOR PLAN INTEGRITY AND CODE COMPLIANCE

5 of 6

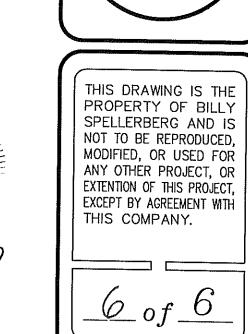
SHEET NO





LSMO Summit View Farms Lot 58 3133 Blue Ribbon Rd.





6 of 6

FRAMING

FLOOR

FIRST

BRANTLY

ARCHITECT IS NOT RESPONSIBLE FOR THE STRUCTURAL ELEMENTS OF THESE PLANS. A STRUCTURAL

ENGINEER MAY NEED TO

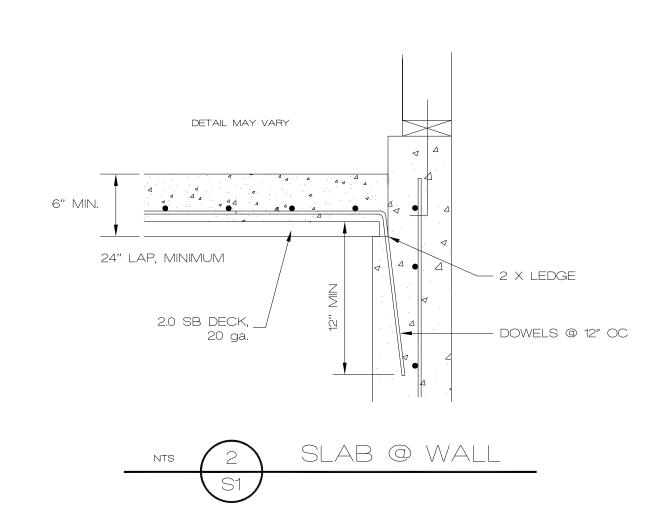
VERIFY ALL STRUCTURAL ASPECTS

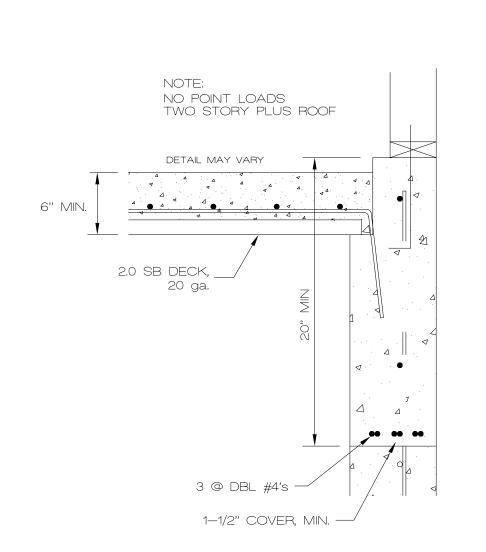
OF THESE PRINTS BEFORE

CONSTRUCTION BEGINS. FIELD CONDITIONS MAY BE DIFFERENT FROM PLAN. ALL STATE AND LOCAL CODES TAKE PRECIDENCE OVER

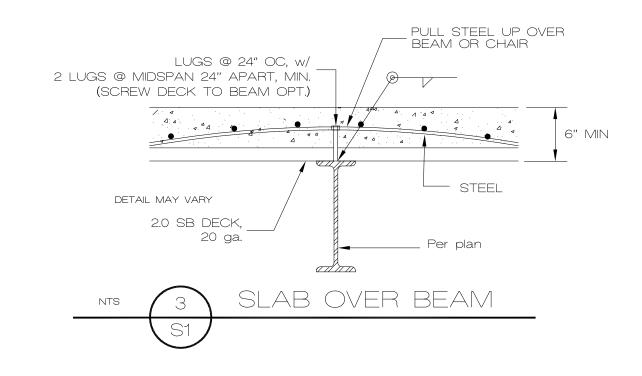
THESE PLANS. CONTRACTOR WILL BE RESPONSIBLE FOR PLAN INTEGRITY AND CODE COMPLIANCE

DESCRIP







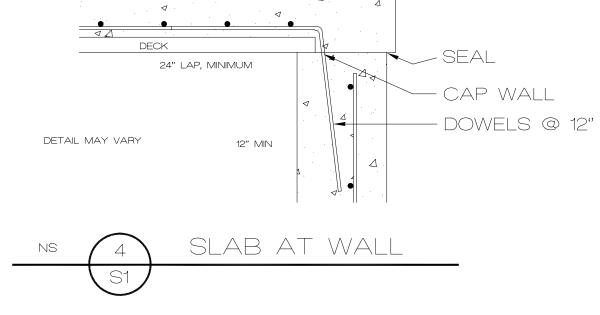


Section	Properties	(per	ft.	of	width)

		Section Properties (per 11, or width)												
	2.0 SB Normal Weight	Gage	t in	Wd psf	Sp in	Sn in	lp in ⁴	In in ⁴). S %	Fy ksi				
		22	0.0295	2.0	0.257	0.258	0.317	0.309	0.472	50				
		20	0.0358	2.3	0.334	0.337	0.402	0.393	0.573	50				
		18	0.0474	3.0	0.507	0.517	0.557	0.552	0.759	40				
	145 pcf Normal Weight Concrete	16	0.0600	3.7	0.659	0.663	0.705	0.705	0.961	40				
	140 poi Normai Weight Concrete						•							

Total Slab		Maximum Unshored			Composite													
Depth D		Clear Spans		Properties		Superimposed Live Loads — psf: No Studs												
Wt. Conc.	Gage	Single	Double	Triple	lavg	Sc Span - Fee						eet ar	and Inches					
Area Conc.		Span	Span	Span	in⁴/ ft	in³/ ft	7'-0"	7-6"	8'-0"	8'-6"	9'-0"	9'—6"	10'-0"	10'—6"	11'—0"	11'—6"	12'-0"	12'-6"
	22	6'-2"	7'—11"	8'-2"	12.702	1.684	400	400	400	366	322	284	252	224	200	179	161	144
6"	20	7'-2"	9'—1"	9'-5"	13.548	2.010	400	400	400	400	393	348	309	276	247	222	200	181
60.4 psf	18	8'-0"	10'-0"	10'—4"	14.981	2.589	400	400	400	400	400	359	320	285	256	230	207	187
42.7 in²	16	9'-3"	11'—4"	11'—9"	16.369	3.184	400	400	400	400	400	359	320	285	256	230	207	187

Metal Decking Details



ISSUE DATE 10/10/20
REVISIONS

SVF 58

KENNETH SIDOROWICZ

NUMBER
E-19986

NOTES:

SET LEDGE
658 #'S OF CEMENT PER YD. MINIMUM (7 SACK)
PROVIDE TEMPORARY DECK SUPPORT, READY AT INSPECTION
SLOPE SLAB TO DOORS OR FLOOR DRAIN OPTION
FLOOR DRAINS SHALL NOT INTERFERE WITH SLAB REINFORCING (OPT.)
REBAR SHALL BE GR40 MIN.
TIE STEEL TO PREVENT DISPLACEMENT
SEAL OR WATERSTOP AS REQ'D
HOOK AND TIE STEEL AS POSSIBLE
SET STEEL ON CHAIRS AS REQ'D
SEAL AT PERIMETER AS REQ'D
SEAL ALL PENETRATIONS
LUGS ON BEAMS CAN BE 3/8 BOLTS OR 1/2" REBAR
SEAL OPTIONAL DRAIN AS REQ'D
DO NOT SAW CUT STRUCTURAL SLAB W/O APPROVAL
CONSTRUCTION SHALL MEET ALL APPLICABLE STANDARDS
CONSTRUCTION SHALL COMPLY WITH IRC
BRACE WALLS OR LIMIT BACKFILL UNTIL SLAB IS POURED
ADJUST FDN FOR SITE & SOIL CONDITIONS

DIVISION 1 — GENERAL REQUIREMENTS

1. DESIGN AND CONSTRUCTION WORK FOR THIS PROJECT SHALL CONFORM TO THE REQUIREMENTS OF THE 2018 IRC.

2. FURNISH ALL LABOR, MATERIALS, AND EQUIPMENT NECESSARY TO COMPLETE THE WORK AS SHOWN OR INFERRED BY THE DRAWINGS.

A) GROUND SNOW LOAD (INCLUDING DRIFTING SNOW)__20 PSF B) WIND SPEED (EXPOSURE B)_ C) SEISMIC CATEGORY (A), GROUND ACCELERATION = NA

4. DESIGN LOADS (PSF, UNLESS NOTED OTHERWISE): A) ROOF (LL/DL)_ _SEE TABLE B) FLOOR (LL/DL)_ _SEE TABLE __SEE TABLE, (0/10 TRUSSES) C) CEILING (LL/DL)_

5. DO NOT SCALE DRAWINGS. IF DIMENSIONS ARE IN QUESTION, OBTAIN CLARIFICATION FROM A / E BEFORE CONTINUING CONSTRUCTION.

6. THE CONTRACTOR SHALL EXAMINE ACTUAL JOB CONDITIONS AND BE RESPONSIBLE FOR VERIFYING ALL DIMENSIONS AND ELEVATIONS SHOWN ON THE PLANS. IF ERRORS, OMISSIONS, OR DISCREPANCIES ARE FOUND THEY SHALL BE REPORTED TO THE DESIGN PROFESSIONAL BEFORE PROCEEDING WITH THE WORK.

7. DIMENSIONS FOR NEW CONSTRUCTION ARE TO FACE OF FINISH OR COLUMNS AND FACE OF CONCRETE, WOOD, OR MASONRY WALLS UNLESS OTHERWISE INDICATED. DIMENSIONS INDICATE NOMINAL DIMENSIONS RATHER THAN ACTUAL DIMENSIONS.

8. CONTRACTOR IS RESPONSIBLE FOR COORDINATING ALL TRADES EVEN IF THE TRADE IS UNDER A SEPARATE CONTRACT.

9. PROVIDE SUFFICIENT STUDS AND BLOCKING WHERE REQUIRED TO SUPPORT EQUIPMENT AND/OR MISCELLANEOUS ITEMS, I.E., LOAD POINTS, TYPICAL CASEWORK, CABINETS, GRAB BARS ETC.

10. PRETREAT FOUNDATION FOR TERMITES AS REQUIRED.

11. GARAGE DOORS AND FRAMES SHALL BE DESIGNED AND INSTALLED TO MEET THE 90 MPH WIND LOAD RESISTANCE REQUIREMENTS OF DASMA 108 AND ASTM E 330-96.

12. ALL EXTERIOR DOORS, INCLUDING THE DOOR LEADING FROM THE GARAGE TO THE DWELLING UNIT, SHALL INCORPORATE THE PHYSICAL SECURITY PROVISIONS OF THE JURISDICTION IN WHICH THE CONSTRUCTION TAKES PLACE.

<u>DIVISION 2</u> — EARTHWORK

1. ALL PROPERTY MARKERS SHALL BE EXPOSED.

2. ALL FOOTINGS ARE DESIGNED TO BEAR ON NATURAL UNDISTURBED SOIL CAPABLE OF ADEQUATELY SUSTAINING A MINIMUM BEARING PRESSURE OF 1,500 PSF. IF SUITABLE UNDISTURBED BEARING CAPACITY IS NOT ENCOUNTERED AT THE ELEVATION INDICATED ON THE DRAWINGS, CONTRACTOR SHALL NOTIFY THE ENGINEER OF RECORD IMMEDIATELY.

3. ALL TOPSOIL, ORGANIC MATERIAL, AND EXISTING STRUCTURES SHALL BE REMOVED FROM BUILDING AREA AND FROM AREAS TO BE PAVED. STOCKPILE ALL TOPSOIL FOR REUSE.

4. REFERENCE THE SOILS REPORT FOR ALL FILL CONDITIONS.

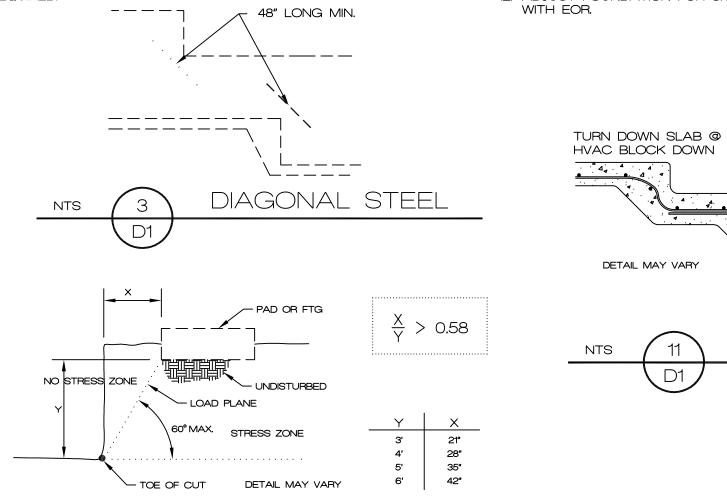
5. OVEREXCAVATE BUILDING AREA BELOW SLAB SUBGRADE ELEVATION AND REPLACE WITH MATERIAL PER SOILS REPORT,

6. SITE EROSION CONTROL SHALL COMPLY WITH ALL STATE AND LOCAL ORDINANCES.

7. IN-SITU SOIL CONDITIONS, SEE SOILS REPORT OR 1,500 PSF BEARING & 60 PCF EQUIVALENT FLUID WEIGHT.

8. SOIL CONDITIONS AT THE DEPTH OF EXCAVATION FOR THE FOOTING SHALL BE UNIFORM AND CONSISTENT. NOTIFY THE ENGINEER OF RECORD OF ANY INCONSISTENCIES.

9. CONTRACTOR SHALL BE RESPONSIBLE FOR REMOVING AND DISPOSING OF ANY EXCESS EXCAVATION MATERIALS AND FOR OBTAINING AND SUPPLYING ADDITIONAL FILL MATERIAL AS



FOOTING FOOTING STRESS ZONE PEDESTAL DETAIL MAY VARY w/ 8 #4's EW DETAIL MAY VAF

CAST w/ SLAB

3" MIN. COVER

PEDESTAL PED @ FTG LAB @ SLAB ON FILL

DIVISION 3 — CONCRETE

1. ALL CONCRETE WORK SHALL CONFORM TO THE REQUIREMENTS OF ACI 318 "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE" AND ACI 332 "REQUIREMENTS FOR RESIDENTIAL CONCRETE CONSTRUCTION."

CONCRETE MATERIALS SHALL COMPLY WITH: A) CEMENT - ASTM C 150 TYPE 1

B) AGGREGATE - ASTM C 33, MAXIMUM AGGREGATE SIZE 3/4" C) WATER - POTABLE, WATER/CEMENT RATIO 5 (MAX.) D) AIR-ENTRAINING ADMIXTURE - ASTM C 260

E) WATER-REDUCING ADMIXTURE - ASTM C 494, INCLUDING SUPERPLASTICIZERS. F) FLY ASH - ASTM C 618, CLASS C

3. CONCRETE SHALL DEVELOP THE FOLLOWING MINIMUM 28 DAY DESIGN COMPRESSIVE STRENGTH (f'c):

TYPE OF CONSTRUCTION COMP. STRENGTH (f'c) A) FOOTINGS, WALLS, AND SLABS SEE TABLE SEE TABLE B) EXTERIOR SLABS AND CURBS (AIR-ENTRAINED CONCRETE)

CONCRETE PROPORTIONS SHALL BE ESTABLISHED ON THE BASIS OF FIELD EXPERIENCE AND/OR TRIAL MIXTURES IN ACCORDANCE WITH ACI 318-89 SECTIONS 5.2 AND 5.3. WHEN FLY ASH IS UTILIZED IN THE MIX, MIX SHALL CONTAIN A WATER-REDUCER. FLY ASH SHALL BE ADDED AT THE RATE OF NOT MORE THAN 100 POUNDS PER CUBIC YARD AND CEMENT SHALL BE REDUCED BY NOT MORE THAN 15 PERCENT BY WEIGHT.

4. PROPORTION AND DESIGN MIXES TO RESULT IN CONCRETE SLUMP AT A POINT OF PLACEMENT OF NOT MORE THAN 4" TO 5".

5. USE AIR-ENTRAINING ADMIXTURES IN EXTERIOR EXPOSED CONCRETE TO RESULT IN CONCRETE AT POINT OF PLACEMENT HAVING AIR CONTENT OF 5 TO 7 PERCENT ENTRAINED AIR.

6. ALL PLUMBING AND ELECTRICAL ROUGH-INS MUST BE COMPLETE. INSPECTED AND APPROVED BEFORE REQUESTING THE SLAB

7. CONCRETE WORK EXECUTION: A) MINIMUM CONCRETE COVER FOR REINFORCING SHALL BE, UNLESS

NOTED OTHERWISE ON DRAWINGS: CAST AGAINST AND EXPOSED TO EARTH_ EXPOSED TO EARTH OR WEATHER_ NOT EXPOSED TO EARTH OR WEATHER

B) IN CORNERS OF GRADE BEAMS PROVIDE CORNER REINFORCEMENT LAP TWO FEET EACH DIRECTION IN OUTSIDE FACE, MATCHING SIZE AND SPACING OF HORIZONTAL REINFORCEMENT. C) PROVIDE CONTROL JOINTS IN SLABS-ON-GRADE AT NOT GREATER

THAN 20 FEET ON CENTER IN EACH DIRECTION. SAW CUT CONTROL JOINTS MINIMUM 1/4 OF THE SLAB DEPTH, AS SOON AFTER SLAB FINISHING AS POSSIBLE WITHOUT DISLODGING AGGREGATE. (DO NOT SAW CUT STRUCTURAL SLABS w/o APPROVAL).

8. BATCH TICKETS SHALL BE SUBMITTED TO A CONTRACTORS REPRESENTATIVE PRIOR TO OFF LOADING. ANY CONCRETE MORE THAN 45 MINUTES OUT PRIOR TO STARTING PLACEMENT SHALL BE REJECTED.

9. THE MAXIMUM ADDITION OF WATER SHALL BE LIMITED TO 1 GALLON PER YARD; NOTE THAT THIS ADDITION SHALL BE USED TO CONTROL HEAT ONLY (NOT SLUMP).

10. PUMPS SHALL NOT BE PRIMED IN FORMS.

A) ALL REINFORCING BARS SHALL BE A615, GR40 MIN. LAP SPLICES 18" MIN FOR #4 BAR. SEE TABLE B) WELDED WIRE FABRIC SHALL BE ASTM A185, LAP AT LEAST ONE

FULL MESH AND LACE SPLICES WITH WIRE. C) REBAR SHALL BE CLEAN, AND FREE FROM RUST AND OIL PRIOR TO THE PLACEMENT OF CONCRETE. REBAR SHALL BE TIED AND

SECURED AS REQUIRED TO PREVENT DISPLACEMENT IN THE FORMS. D) TIE STEEL TO PREVENT DISPLACEMENT. HOOK AND TIE STEEL AS POSSIBLE. TIES, CHAIRS, OR OTHER PRODUCTS SHALL BE PROTECTED WHEN LOCATED NEAR EXPOSED SURFACES.

HVAC TRUNK

12" ADDITIONAL

SLAB @ HVAC

DETAIL MAY VARY

JOIST HNGR -

1-1/2" LEDGE -

FLOOR

JOIST

6" MIN. STEM WALL-

3" MIN COVER

UNDISTURBED

BLOCK DOWN @ HVAC

DBL PLATE FOR GYP CRETE

CONCRETE SLAB

, — 1—1/2" COVER MIN.

2.5" INSULATION,

1-1/2" LEDGE MIN.

ICF WALL

FLUSH FRAMING @ FDN

AS REQUIRED FOR PROTECTION FROM RAIN AND OTHER POSSIBLE 12. ADJUST FOUNDATION FOR SITE AND SOIL CONDITIONS AND VERIFY

E) STEEL SHALL BE STORED ON SITE ABOVE GRADE, AND COVERED

DIVISION 4 - MASONRY

COMPRESSIVE STRENGTH OF CONCRETE MASONRY CONSTRUCTION (CMU) SHALL BE AS FOLLOWS (PSI). MASONRY STRENGTH NOT SPECIFICALLY NOTED ON PLAN SHALL BE (f'm) 1500 PSI.

MASONRY STRENGTH (F'm DESIGN)_ BLOCK STRENGTH_ MORTAR STRENGTH 1800 GROUT STRENGTH_

2. CONCRETE BLOCK SHALL BE HOLLOW LOAD-BEARING CONCRETE MASONRY UNITS CONFORMING TO ASTM C 90, TYPE N-II. ALL BLOCKS SHALL BE PLACED IN RUNNING BOND CONSTRUCTION (UNLESS OTHERWISE NOTED) WITH ALL VERTICAL CELLS IN

3. MORTAR MIX SHALL CONFORM TO THE REQUIREMENTS OF ASTM C 270, TYPE M OR S. TYPE M MORTAR SHALL BE USED WHERE MASONRY IS IN CONTACT WITH SOIL.

4. GROUT SHALL CONFORM TO THE REQUIREMENTS OF ASTM C 476. USE SUFFICIENT WATER FOR GROUT TO FLOW INTO ALL JOINTS OF THE MASONRY WITHOUT SEGREGATION. ALL CELLS IN CONCRETE BLOCKS CONTAINING REINFORCING SHALL BE FILLED SOLID WITH GROUT. ALL MASONRY BELOW FINISHED FLOOR OR GRADE SHALL BE GROUTED SOLID. HOLD GROUT DOWN 1-1" BELOW TOP OF BLOCK AT GROUT LIFT JOINTS AND AT CONCRETE PLACED OVER

5. MINIMUM LINTEL, WHERE NOT ON PLANS, SHALL HAVE A MINIMUM OF 2 — #5's CONTINOUS HORIZONTAL BARS IN BOTTOM OF BOND BEAM OR LINTEL BLOCK AND SHALL BE GROUTED SOLID TO A MIN. DEPTH OF 24." ALL LINTEL REINFORCING AND GROUT SHALL EXTEND 2' MINIMUM PAST JAMBS UNLESS NOTED OTHERWISE ON PLANS OR DETAILS.

6. LAP REINFORCING 48 BAR DIAMETERS. STAGGER LAP SPLICES A MINIMUM OF ONE LAP LENGTH.

7. MASONRY VENEER SHALL BE ATTACHED TO SUPPORT WALL FRAMING WITH &" DIAMETER WALL TIES OR DOVETAIL-TYPE METAL TIES OF EQUIVALENT STIFFNESS EMBEDDED INTO HORIZONTAL MORTAR JOINTS. MAXIMUM VERTICAL SPACING OF TIES SHALL BE 16." MAXIMUM HORIZONTAL SPACING SHALL BE 24." TIES IN ALTERNATE COURSES SHALL BE STAGGERED. PROVIDE #9 WIRE REINFORCING IN HORIZONTAL MORTAR JOINTS AT 16" OC. ENGAGE #9 WIRE WITH WALL ANCHOR TIES. CONSTRUCTION JOINTS IN MASONRY VENEER WALLS SHALL BE LOCATED PER THE DRAWINGS.

8. WATERPROOFING, DRAINAGE PLANE, AND INSTALLATION PER ADOPTED BUILDING CODE.

<u>DIVISION 5.5</u> — MISC. STRUCTURAL STEEL

UNLESS OTHERWISE NOTED.

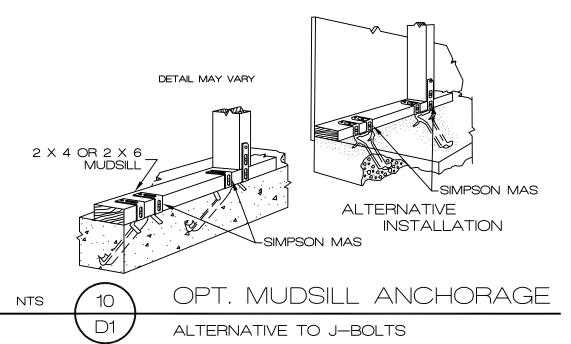
1. ALL MISCELLANEOUS STRUCTURAL STEEL WORK SHALL CONFORM TO THE REQUIREMENTS OF AISC "SPECIFICATIONS FOR DESIGN, FABRICATION, AND ERECTION OF STRUCTURAL STEEL FOR

2. MISCELLANEOUS STRUCTURAL STEEL MATERIAL SHALL COMPLY A) STRUCTURAL STEEL - ASTM A992 B) STEEL PIPE COLUMNS - ASTM A53 GRADE B(Sch 40 TYP) C) ANCHOR BOLTS - ASTM A307 GRADE A, NON-HEADED TYPE

3. FLITCH PLATES SHALL HAVE 1/4" DIA. BOLTS @ 16" OC, STAGGERED TOP AND BOTTOM BETWEEN JOIST LAYOUT.

RETURN WALLS UNBALANCED RETURN SPACING BACKFILL HT. (HOLD DOWN 24" BELOW GRADE) RETURN WALLS NOT REQ'D 16'-4" ON CENTER (MAX.), AND/OR 5' TO 9' WITHIN 8' OF STEP DOWN

* RETURN WALLS ALLOW FOR BACKFILL W/O FLOOR DECK IN PLACE FOR 60 PCF EQUIVALENT FLUID WEIGHT SOIL. NO HEAVY EQUIPMENT OR SURCHARE LOADING.



4 4 4 5—1/2" - 2 X LEDGE 24" LAP, MIN - DRILL & SEAL AS REQ'D 1-1/2" COVER -- DOWELS @ 12" OC



SLAB

SUS-SLAB

REQ'D STRENGTH $\phi M_N = *\phi A * f(d - a/2)$ 3,000 psi = 0.9(0.2)(40000)(4-0.22/2)3,500 psi = 28,008 # in > 27,206 (OKAY)3,500 psi ∴ Use #4 @ 12" OC EW 7 SACK MIX 12'-6" (+/-) MODULE

GARAGE SLAB:

0.85 * 3,500 * 12

= 0.9(0.2)(40000)(4-0.22/2)= 28,008 #-in > 25,951 (OKAY)

∴ Use #4 @ 12" OC EW 15'-6" (+/-) MODULE

<u>DIVISION 6</u> — ROUGH CARPENTRY

1. ALL ROUGH CARPENTRY WORK SHALL CONFORM TO THE REQUIREMENTS OF NFPA "NATIONAL DESIGN SPECIFICATION OF WOOD CONSTRUCTION", TPI "DESIGN SPECIFICATIONS FOR LIGHT METAL PLATE CONNECTED WOOD TRUSSES", APA "PLYWOOD DESIGN SPECIFICATIONS", DOC PS 1 "PRODUCT STANDARD FOR CONSTRUCTION AND INDUSTRIAL PLYWOOD", DOC PS 56 "STRUCTURAL GLUED LAMINATED TIMBER", AND APPLICABLE SECTIONS OF THE INTERNATIONAL BUILDING CODE.

2. ROUGH CARPENTRY MATERIALS SHALL COMPLY WITH: A) LUMBER - S4S, S-DRY, KD, OR S-GRN GRADE MARKED. COMPLYING WITH PS 20, GRADED UNDER WWPA OR SPIB RULES: STUD GRADE #2 DOUGLAS FIR MIN TYPICAL

RAFTER: #2 DOUGLAS FIR PLATES: #2 DOUGLAS FIR BLOCKING: #2 DOUGLAS FIR

B) METAL FRAMING FASTENERS - ASTM A 153, HOT-DIP GALVANIZED FASTENERS; EQUAL TO SIMPSON STRONG-TIE CONNECTORS COMPLYING WITH APPLICABLE ICC-ES REPORTS. C) PLYWOOD - APA RATED SHEATHING, COMPLYING TO PS 1. D) LVL - LAMINATED VENEER LUMBER SHALL BE GRADE 2800 F-2.0E AND SHALL MEET THE REQUIREMENTS OF APPLICABLE ICC-ES REPORTS.

E) GLULAM BEAMS - COMBINATION 24F-V3 IN ACCORDANCE WITH AITC A190.1 3. EXTERIOR WALL AND ROOF SHEATHING SHALL BE 4" APA RATED

SHEATHING 24/0 EXTERIOR GLUED (MIN) FOR 16" OC STUD SPACING. NAIL SHEATHING TO SUPPORT MEMBERS WITH 8D COMMON NAILS AT 6" ON CENTER ALONG EDGE SUPPORTS AND 12" ON CENTER ALONG FIELD SUPPORTS UNLESS NOTED OTHERWIDE, PROVIDE SOLID BLOCKING AT ALL UNSUPPORTED PANEL EDGES; 4/8 GUN NAILS.

NOTE: ROOF SHEATHING SHALL BE §" APA RATED SHEATHING FOR TILE ROOF, OR AS REQUIRED BY MANUFACTURER.

4. INTERIOR SHEAR WALL SHEATHING WHERE NOTED SHALL BE 76" APA RATED SHEATHING 24/0 EXTERIOR GLUED (MIN) FOR 16" OC STUD SPACING, NAIL SHEATHING TO SUPPORT MEMBERS WITH 8D COMMON NAILS AT 4" ON CENTER ALONG EDGE SUPPORTS AND 6" ON CENTER ALONG FIELD SUPPORTS UNLESS NOTED OTHERWISE, PROVIDE SOLID BLOCKING AT ALL UNSUPPORTED PANEL EDGES.

5. ATTACH METAL FRAMING FASTENERS TO FRAMING MEMBERS WITH MINIMUM NUMBER AND SIZE OF NAILS LISTED IN THE APPLICABLE ICC-ES REPORTS.

6. WOOD TRUSS SYSTEM; TRUSS JOIST SYSTEM AND GLULAM SYSTEM FOR ROOFS: A) DESIGN, FABRICATE, AND ERECT IN ACCORDANCE WITH BCSI STANDARDS AND NDS SPECIFICATIONS.

B) DESIGN LOADS: 25 PSF SNOW LIVE LOAD

WALLS AS NOTED ON PLANS.

10 PSF DEAD LOAD TOP CHORD (20 TILE) 10 PSF DEAD LOAD BOTTOM CHORD

C) SUBMIT SHOP DRAWINGS, INCLUDING DESIGN CALCULATIONS, MATERIAL STRESSES, GRADE AND SPECIES OF WOOD, AND PLACEMENT DRAWING.

7. DEFAULT HEADER SIZE NOT SPECIFIED SPANNING 8'-0" MAX SHALL BE 2 - 2 X 10 #2, WITH 2 STUD SUPPORT

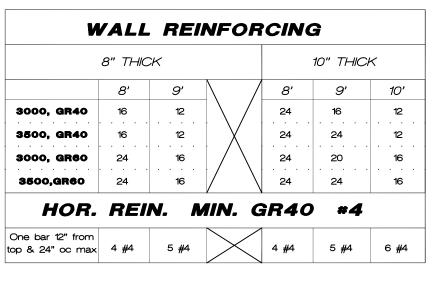
8. ALL HEADERS OVER 4'-0" SHALL HAVE DOUBLE TRIMMER @ EACH SUPPORT, OR AS SPECIFIED, UNO.

9. SOLID BLOCKING BETWEEN JOISTS @ 36" OC FOR JOISTS PARALLEL TO THE EXTERIOR FOUNDATION WALL, MIN. 48" OR 3 JOIST SPACES.

10. ALL FLUSH FRAMING @ HEADERS OR GIRDERS SHALL BE HANGERED. 11. BLOCK BETWEEN JOISTS @ SUPPORTS OR OVER BEAMS.

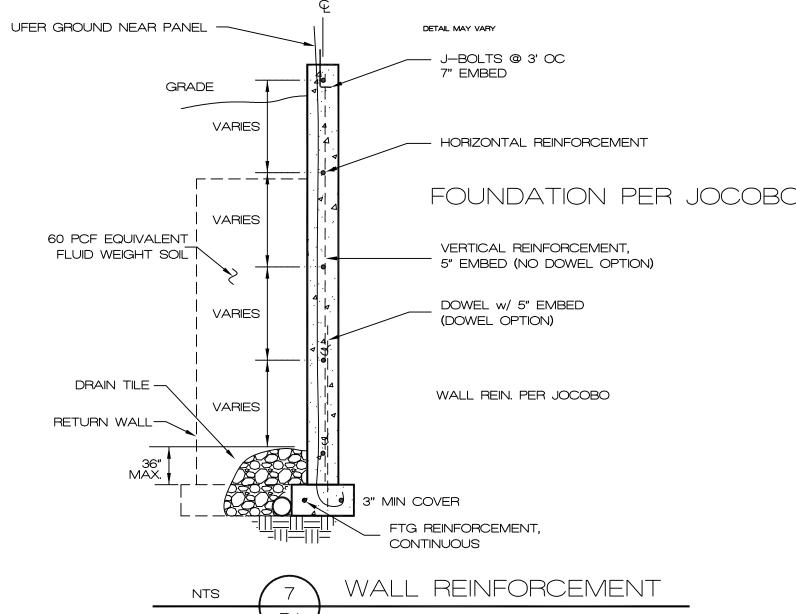
12. RATED CONSTRUCTION FOR PROJECTIONS INTO SETBACKS AS REQ'D. 13. DOUBLE JOIST BELOW PARALLEL NONBEARING WALLS ON LAYOUT SINGLE JOIST OFF LAYOUT. STRUCTURE BELOW LOAD-BEARING

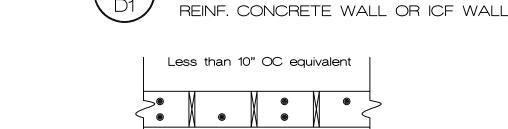
FOUNDATION PER JOCOBO RESIDENTIAL **FOUNDATION GUIDELINE**



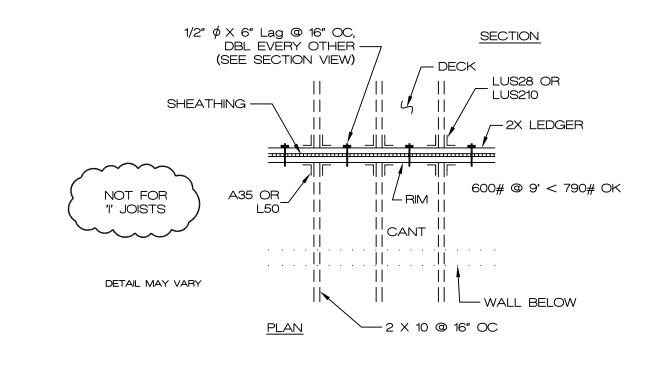
~ 100 # /中' (LL) ~ 40 # /中' (LL) ¯ 67 # /中'(DL) 67 # /中'(DL) $w_u = 12(DL) + 1.6(LL)$ w = 1.2(DL) + 1.6(LL)BASEMENT SLAB: = 240 # / ф' (TL) = 144 # / 丗(TL)

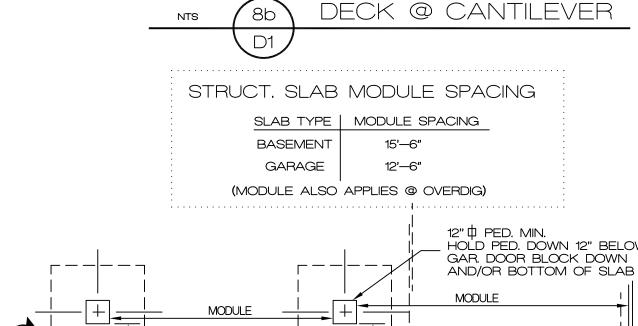
> $\frac{1}{0.85 * 3,500 * 12} = 0.22^{\circ}$ $\phi M_N = *\phi A * f(d - a/2)$

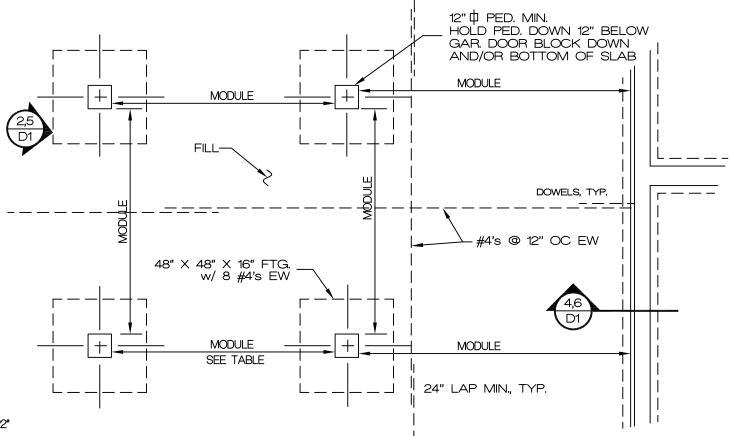












STRUCTURAL SLAB ON FILL DO NOT SAW CUT STRUCTURAL SLABS W/o APPROVAL

VERIFY ALL STRUCTURAL SLAB DETAILS W/ ENGINEER

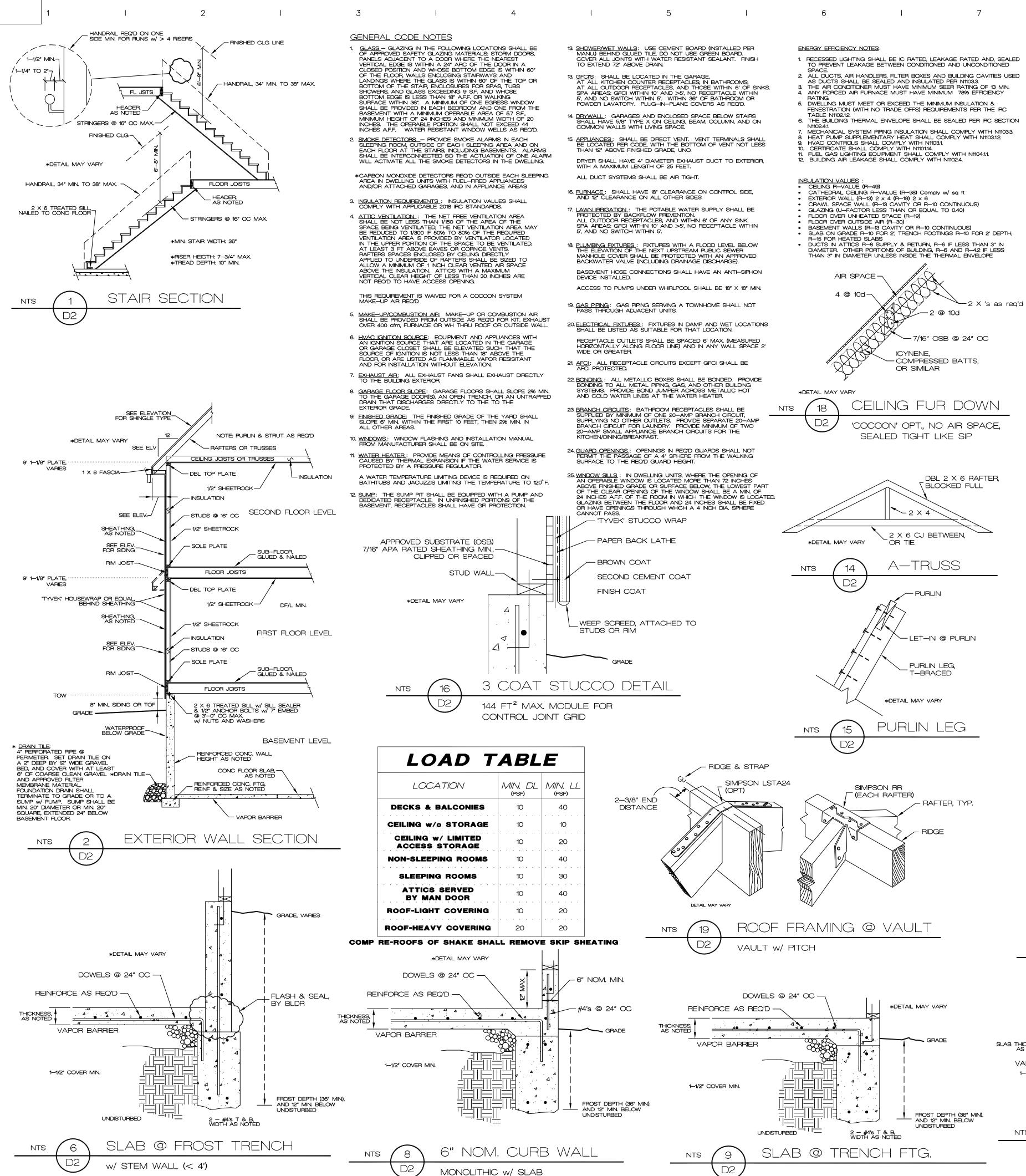
DO NOT ISOLATE COLUMNS FROM STRUCTURAL SLABS

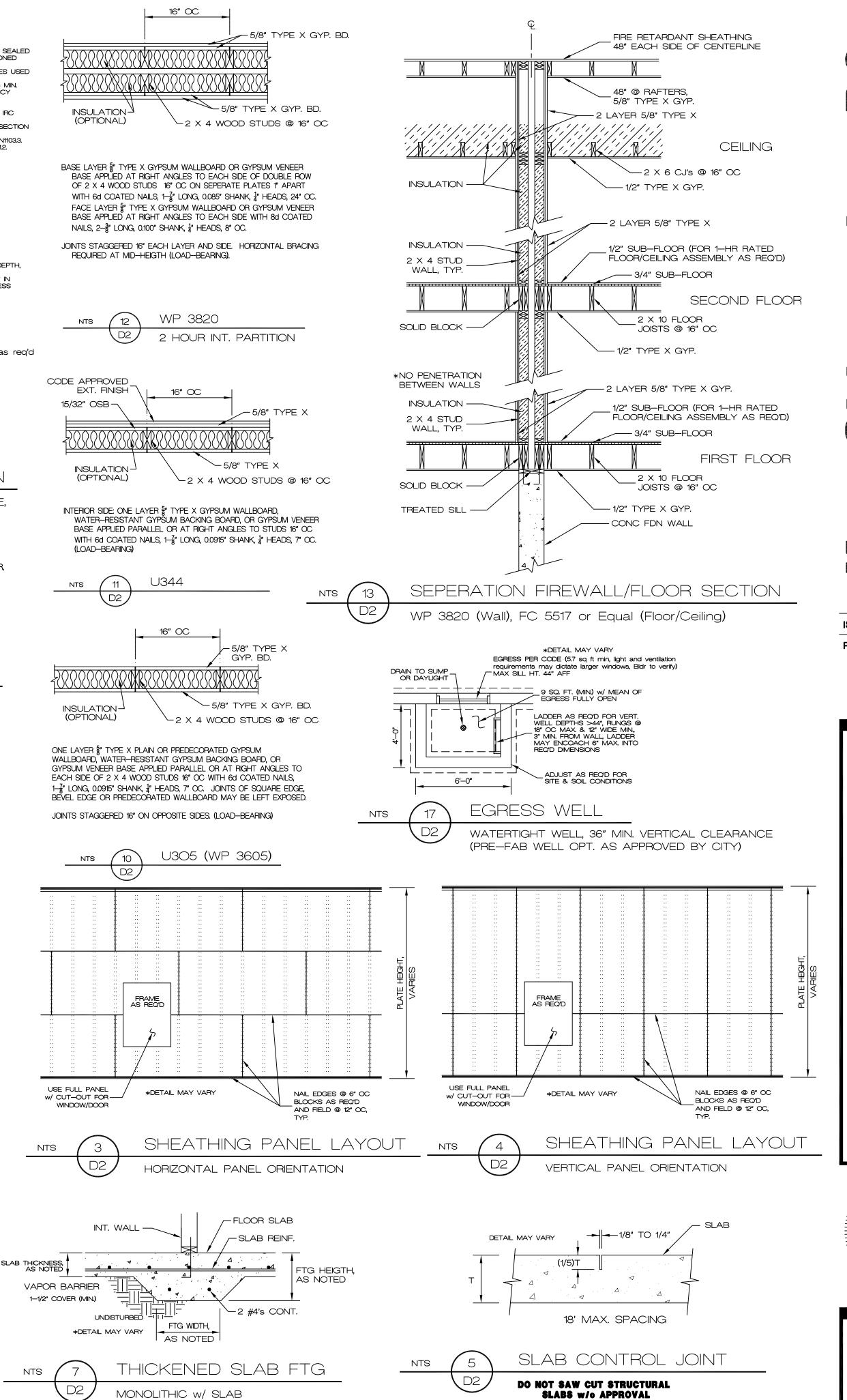
SIDOROWICZ NUMBER / E-19986 10/10/20

ISSUE DATE

REVISIONS

KENNETH ^{*}





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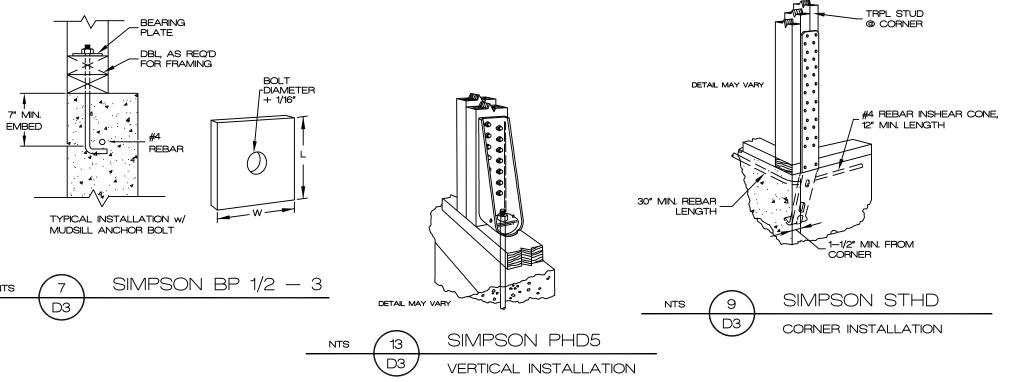
2018 DETAIL SHEET

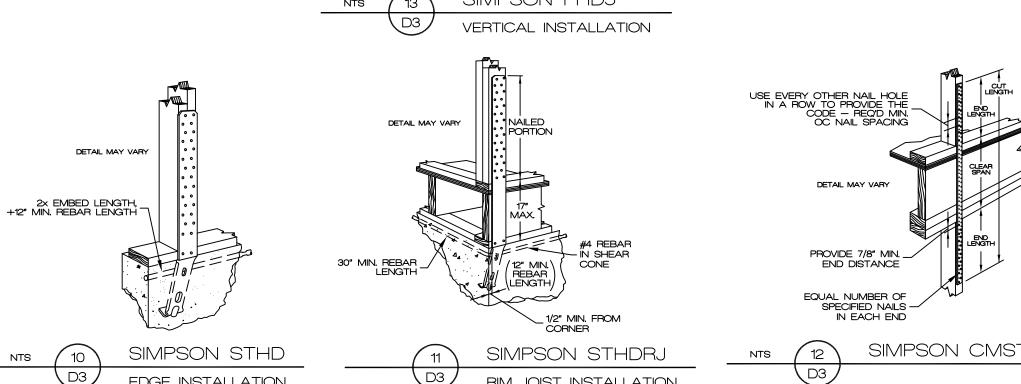
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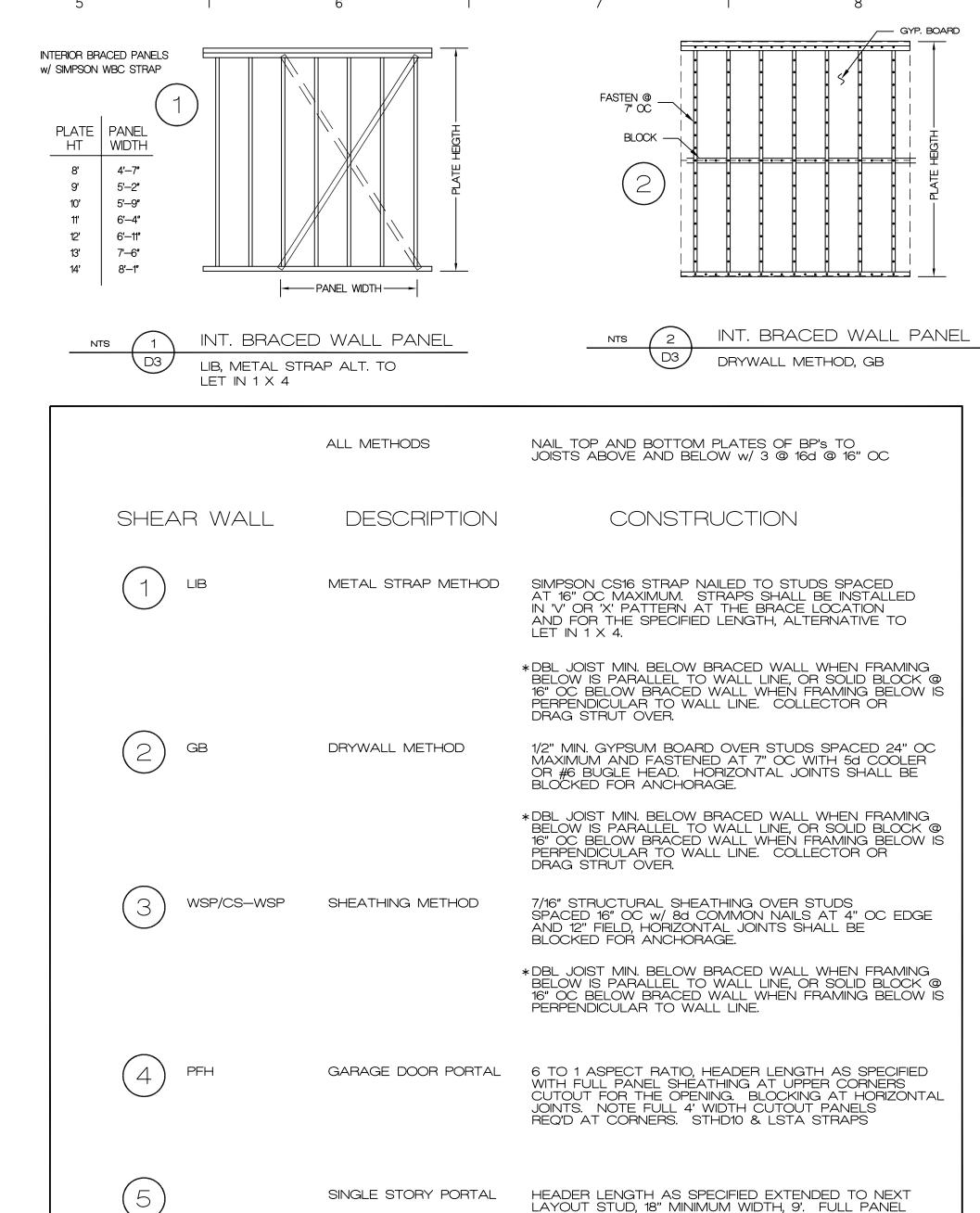
D2

FASTENER SCHEDULE FOR STRUCTURAL MEMBERS Item Description of building elements Number & type of fastener Spacing of fasteners (notes: a, b, c) Blocking between joists or rafters to top plate, toe nail 3-8d (2-1/2" × 0.113") $3-8d (2-1/2" \times 0.113")$ Ceiling joists not attached to parallel rafter, laps over partitions, face nail 3-10d Collar tie rafter, face nail or 1-1/4" x 20 ga. ridge strap 3-10d (3" x 0,128") Rafter to plate, toe nail, note trusses use STC clips at NLB walls and spec'd holdowns 3-16d or 3-10d (3-1/2" × 0.135", 0.148") 2 toe nails side 1, 1 toe nail side 2 (note j) Roof rafters to ridge, valley or hip rafters: 4-16d (3-1/2" x 0.135") Toe nail: 3-16d (3-1/2" × 0.135") Face nail: 10d (3" × 0.128") Built-up studs-face nail 12" o.c. 16d (3-1/2" x 0.135") Abutting studs at intersecting wall corners, face nail Built-up header, two pieces w/ 1/2" spacer 16d (3-1/2" × 0.135") 16" o.c. along each edge 16d (3-1/2" × 0.135") Continued header, two pieces 16" o.c. along each edge 4-8d (2-1/2" × 0.113") Continuous header to stud, toe nail Double studs, face nail 10d (3" × 0.128") 10d (3" × 0.128") Double top plates, face nail Double top plates, min. 48" offset of end joints, face nail in lapped area $8-16d (3-1/2" \times 0.135")$ 16" o.c. 16d (3-1/2" x 0.135") Sole plate to joist or blocking, face nail Sole plate to joist or blocking at braced wall panels 3-16d (3-1/2" × 0.135") 3-8d ($2-1/2" \times 0.113"$) or Stud to sole plate, toe nail 2-16d (3-1/2" × 0.135") 2-16d (3-1/2" × 0.135") Top or sole plate to stud, end nail Top plates, laps at corners and intersections, face nail 2-10d (3" x 0.128") 2-8d (2-1/2" × 0.113") brace to each stud and plate, face nail 2 staples 1-3/4" x 6" sheathing to each bearing, face nail 2-8d (2-1/2" × 0.113") 2 staples 1-3/4" x 8" sheathing to each bearing, face nail 2-8d (2-1/2" x 0.113") 3 staples 1-3/4" Wider than 1" \times 8" sheathing to each bearing, face nail 3-86s1(2plot/2°1-x3(04113°) 3-8d (2-1/2" \times 0.113") Joist to sill or girder, toe nail Rim joist to top plate, toe nail (roof applications also) 8d (2-1/2" × 0.113") 8d (2-1/2" × 0.113") Rim joist or blocking to sill plate, toe nail 6" o.c. 2-8d (2-1/2" x 0.113") 1" x 6" subfloor or less to each joist, face nail 2 staples 1-3/4" 2" subfloor to joist of girder, blind and face nail 2-16d (3-1/2" × 0.135") 2" planks (plank & beam - floor and roof) 2-16d (3-1/2" × 0.135") @ each bearing Built-up girders and beams, 2" lumber layers 10d (3" × 0.128") Nail each layer as follows: 32" o.c. at top and bottom and staggered. Two nails at ends and at each splice 3-16d (3-1/2" × 0.135") Ledger strip supporting joists or rafters @ each joist or rafter Spacing of Fasteners Edges (inches) Description of building Description of fastener Intermediate supports (inches) materials (notes: b, c, e) Wood structural panels, subfloor, roof and interior wall sheathing to framing and particleboard wall sheathing to framing 6d common (2" \times 0.113") nail (subfloor, wall) (note j) 12 (note: g) 8d common (2-1/2" x 0.131") nail (roof) 19/32" to 1" 8d common nail (2-1/2" x 0.131") 12 (note: g) 10d common (3" x 0.148") nail or 1-1/8" to 1-1/4" 8d deformed (2-1/2" x 0.131") nail Other wall sheathing (note h) 1/2" structural cellulosic fiberboard 1-1/2" galv. roofing nail, 7/16" crown or sheathing crown staple 16 ga., 1-1/4" long 25/32" structural cellulosic 1-3/4" galv. roofing nail, 7/16" crown or crown staple 16 ga., 1-1/2" long fiberboard sheathing 1/2" gypsum sheathing (note d) 1-1/2" galvanized roofing nail; staple galv., 1-1/2" long; 1-1/4" screws, Type W or S 5/8" gypsum sheathing (note d) 1-3/4" galvanized roofing nail; staple galv., 1-5/8" long; 1-5/8" screws, Type W or S Wood structural panels, combination subfloor underlayment to framing 8d common (2-1/2" x 0.131") nail 8d common (2-1/2" x 0.131") nail or 8d deformed (2-1/2" x 0.120") nail 1-1/8" to 1-1/4" 10d common (3" x 0.148") nail or 8d deformed (2-1/2" x 0.120") nail For St 1 inch = 254 mm 1 foot = 3048 mm 1 mile per hour = 0.447 m/s: 1 ksi = 6.895 MPs

- a. All nails are smooth-common, box or deformed shanks except where otherwise stated. Nails used for framing and sheathing connections shall have minimum average bending yield strengths as shown: 80 ksi (551 MPa) for shank diameter of 0.192 inch (20d common nail), 90 ksi (620 MPa) for shank diameters larger than 0.142 inch but not larger than 0.177 inch, and 100 ksi (689 MPa) for shank diameters of 0.142 inch or less. Staples are 16 gage wire and have a minimum 7/16-inch on diameter crown width.
- Nails shall be spaced at not more than 6 inches on center at all supports where spans are 48 inches or greate
- d. Four-foot-by-8-foot or 4-foot-by-9-foot panels shall be applied vertically. e. Spacing of fasteners not included in this table shall be verified w/ EOR.
- f. For regions having basic wind speed of 110 mph or greater, 8d deformed nails shall be used for attaching plywood and wood structural panel roof sheathing to framing within minimum 48-inch distance from gable end walls, if mean roof height
- is more than 25 feet, up to 35 feet maximum. g. For regions having basic wind speed of 100 mph or less, nails for attaching wood structural panel roof sheathing to gable endwall framing shall be spaced 6 inches on center. When basic wind speed is greater than 100 mph, nails for attaching panel roof sheathing to intermediate supports shall be spaced 6 inches on center for minimum 48-inch distance from ridges, eaves and gable end walls; and 4 inches on center to gable end wall framing.
- h. Gypsum sheathing shall conform to ASTMC 1396 and shall be installed in accordance with GA 253. Fiberboard sheathing shall conform to ASTM C 208. i. Spacing of fasteners on floor sheathing panel edges applies to panel edges supported by framing members and at all floor perimeters only. Spacing of fasteners on roof sheathing panel edges applies to panel edges supported by framing members and at all roof plane perimeters. Blocking of roof or floor sheathing panel edges perpendicular to the framing members shall not be required except at intersection of adjacent roof planes. Floor and roof perimeter shall be supported by
- Where a rafter is fastened to an adjacent parallel ceiling joist in accordance with this schedule, provide two toe nails on one side of the rafter and toe nails from the ceiling joist to top plate in accordance with this schedule. The toe nail on the







SHEAR WALL SCHEDULE

DETAIL MAY VARY

HORIZONTAL BLOCKING @ PANEL BREAK

GARAGE DOOR HDR.

*USE SIMPSON BP 1/2 - 3 @ GAR.

PLATES (J-BOLT WASHERS),

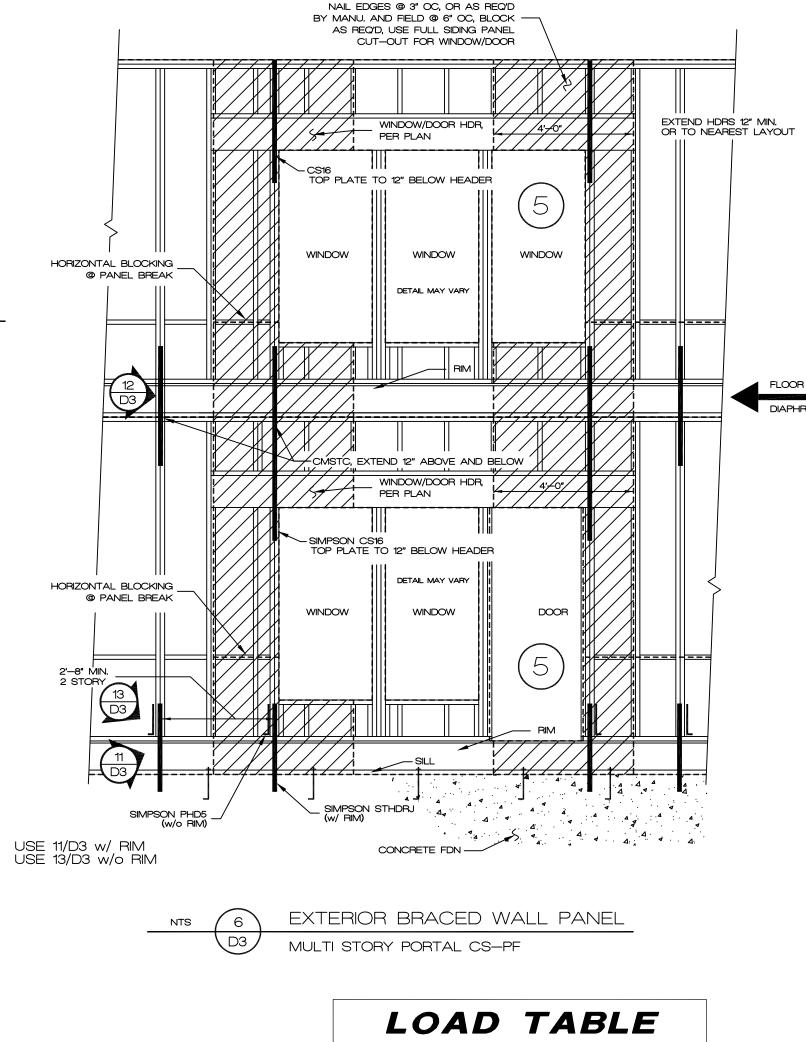
- CONCRETE FDN

GARAGE DOOR

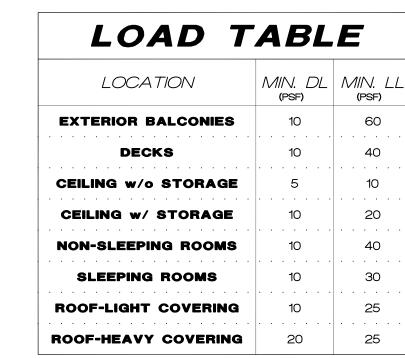
PIECE SHEATHING

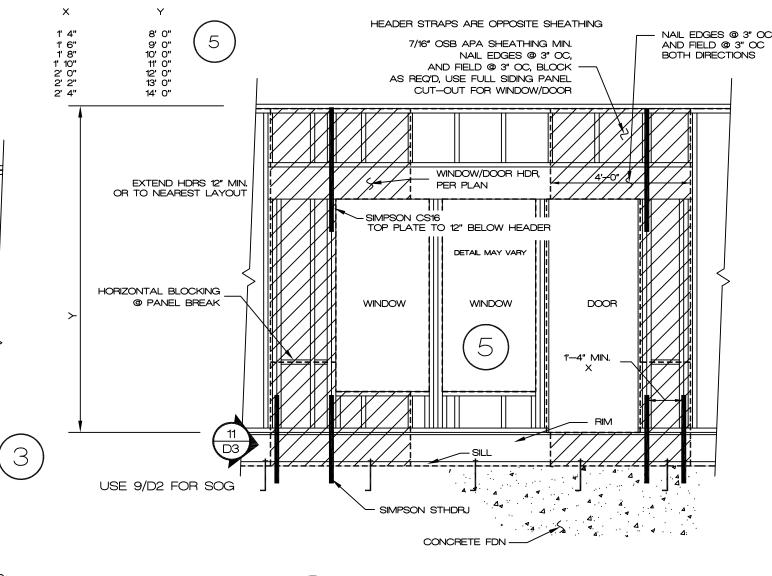
1'-8" MIN.-

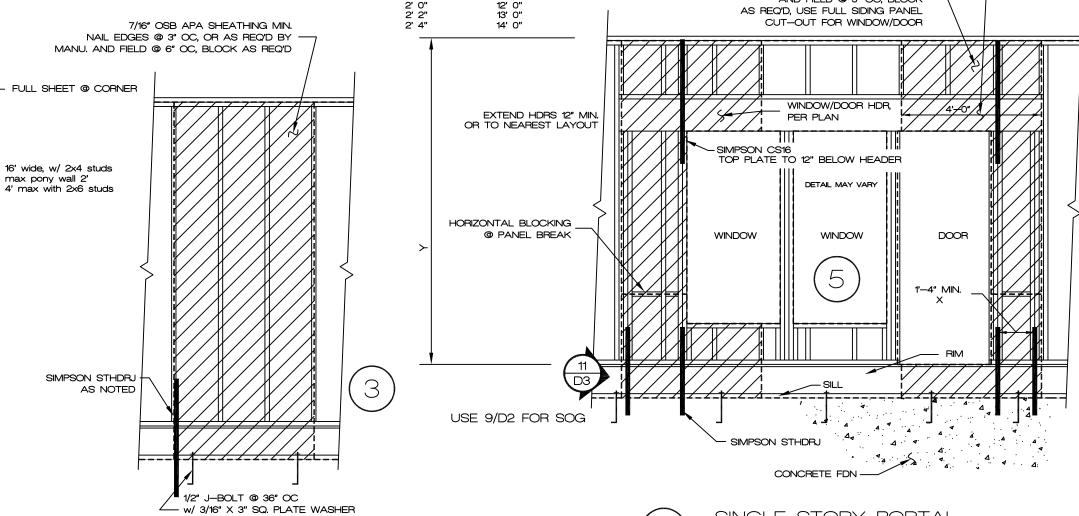
SIMPSON STHD -



7/16" OSB APA SHEATHING MIN.







SINGLE STORY PORTAL

SINGLE STORY PORTAL SINGLE STORY PORTAL CS-PF

KENNETH

SIDOROWICZ

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EXTERIOR BRACED WALL PANEL

SHEATHING RÉQ'D WITH CUTOUTS FOR OPENINGS.

max pony wall 2'

HORIZONTAL BLOCKING AT EDGES.

EDGE INSTALLATION

RIM JOIST INSTALLATION

SIMPSON CMSTC

PORTAL FRAME GARAGE

'J' BOLT SPACING FOR SHEAR WALLS IS

3' OC WITH STRAPS AS NOTED.

NAIL EDGES @ 3" OC, 3" OC BOTH DIRECTIONS IN FIELD

SIMPSON CS16 TOP PLATE TO 12" BELOW HEADER

7/16" OSB APA SHEATHING MIN.

AND FIELD @ 3" OC, BLOCKS AS REQ'D

NAIL EDGES @ 3" OC,

-TRPL STUD (TYP.)

1/2" J-BOLT ____ w/ 3/16" PLATE WASHER,

7° EMBED

DBL TOP PLATE -