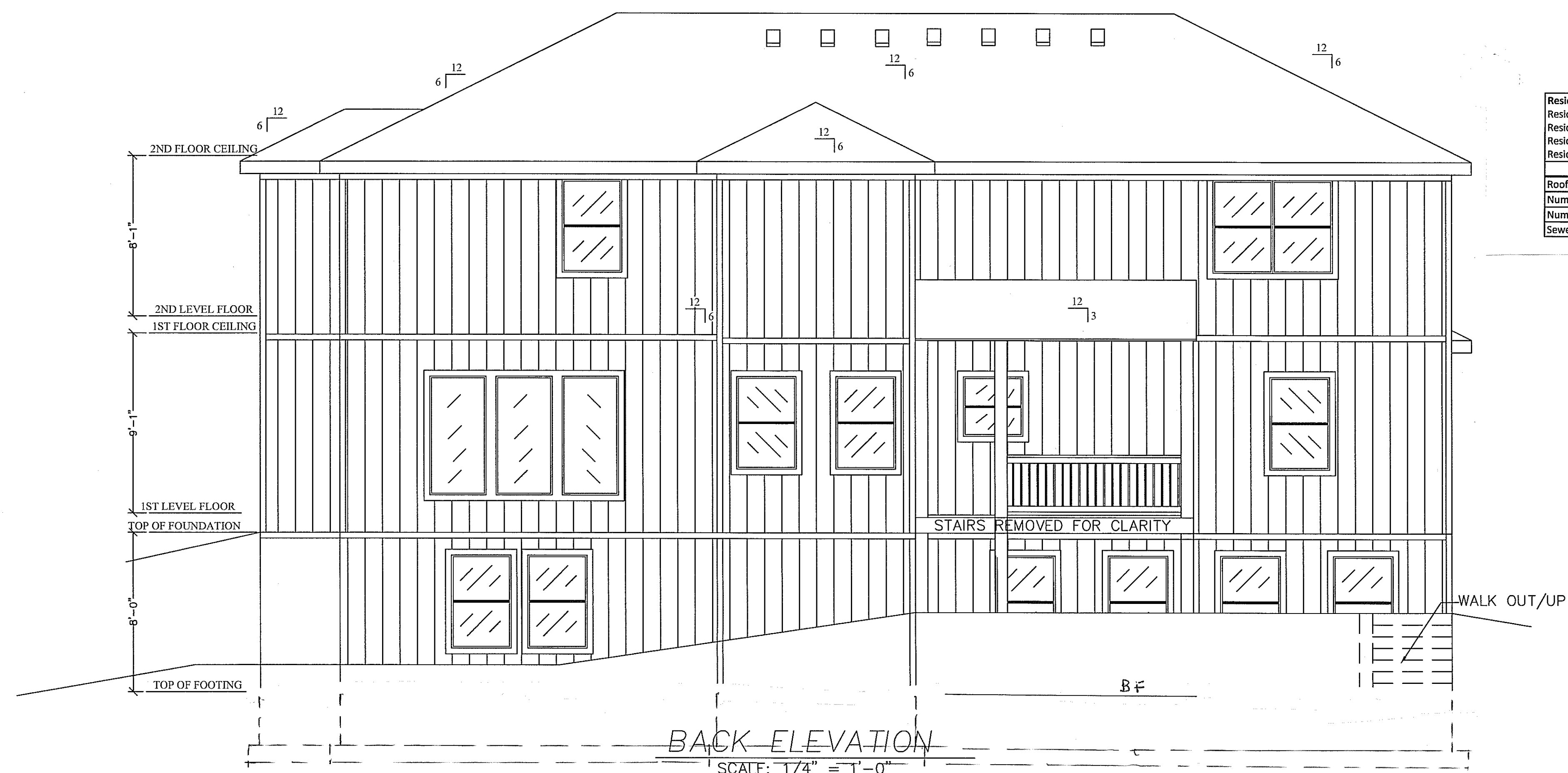


FRONT ELEVATION

SCALE: 1/4" = 1'-0"

COMP ROOF

ROOF & SOFFIT VENTS
PER CODE



BACK ELEVATION

SCALE: 1/4" = 1'-0"

| | | | |
|------------------------------------|----|---------------------|------|
| Residential Area: | | | |
| Residential, Living Area 2 | | 1391 | |
| Residential, Living Area | | 1284 | |
| Residential, Un-Finished basements | | 1284 | |
| Residential, garage | | 660 | |
| | | | |
| Roofing Material | | Number of Bathrooms | 3.5 |
| Number of Bedrooms | 4 | Number of Stories | 2 |
| Number of Living Units | 1 | Total Living Area | 2675 |
| Sewer Connection Fee | 19 | | |

DESCRIPTION:
FRONT/REAR ELEVATIONS

MODEL:

BRANTLY A

DATE:

8/29/20

ARCHITECT IS NOT RESPONSIBLE FOR THE STRUCTURAL ELEMENTS OF THESE PLANS. A STRUCTURAL ENGINEER MAY NEED TO VERIFY ALL STRUCTURAL ASPECTS OF THESE PRINTS BEFORE CONSTRUCTION BEGINS. FIELD CONDITIONS MAY BE DIFFERENT FROM PLAN. ALL STATE AND LOCAL CODES TAKE PRECEDENCE OVER THESE PLANS. CONTRACTOR WILL BE RESPONSIBLE FOR PLAN INTEGRITY AND CODE COMPLIANCE.

BUILD
SET

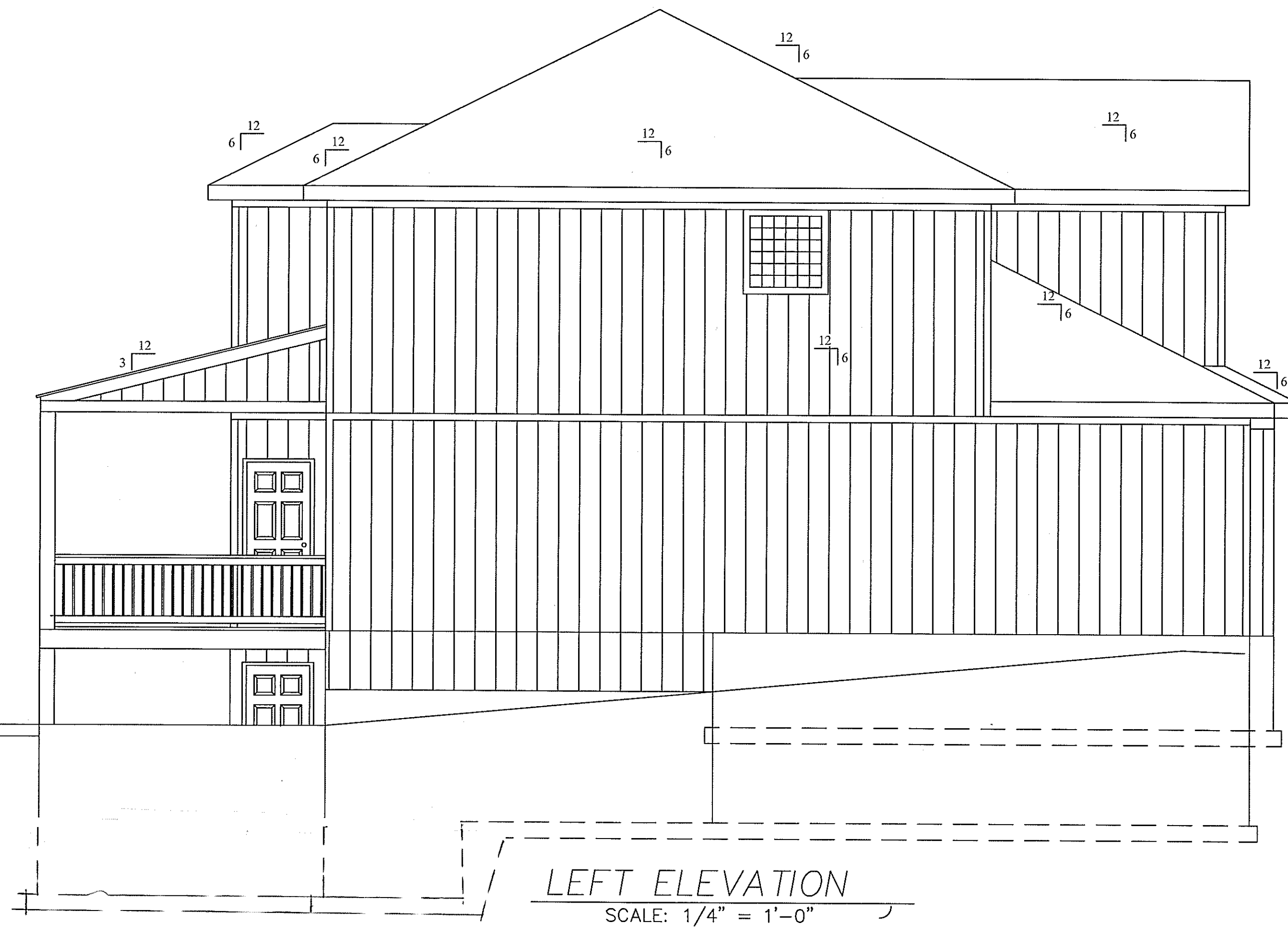
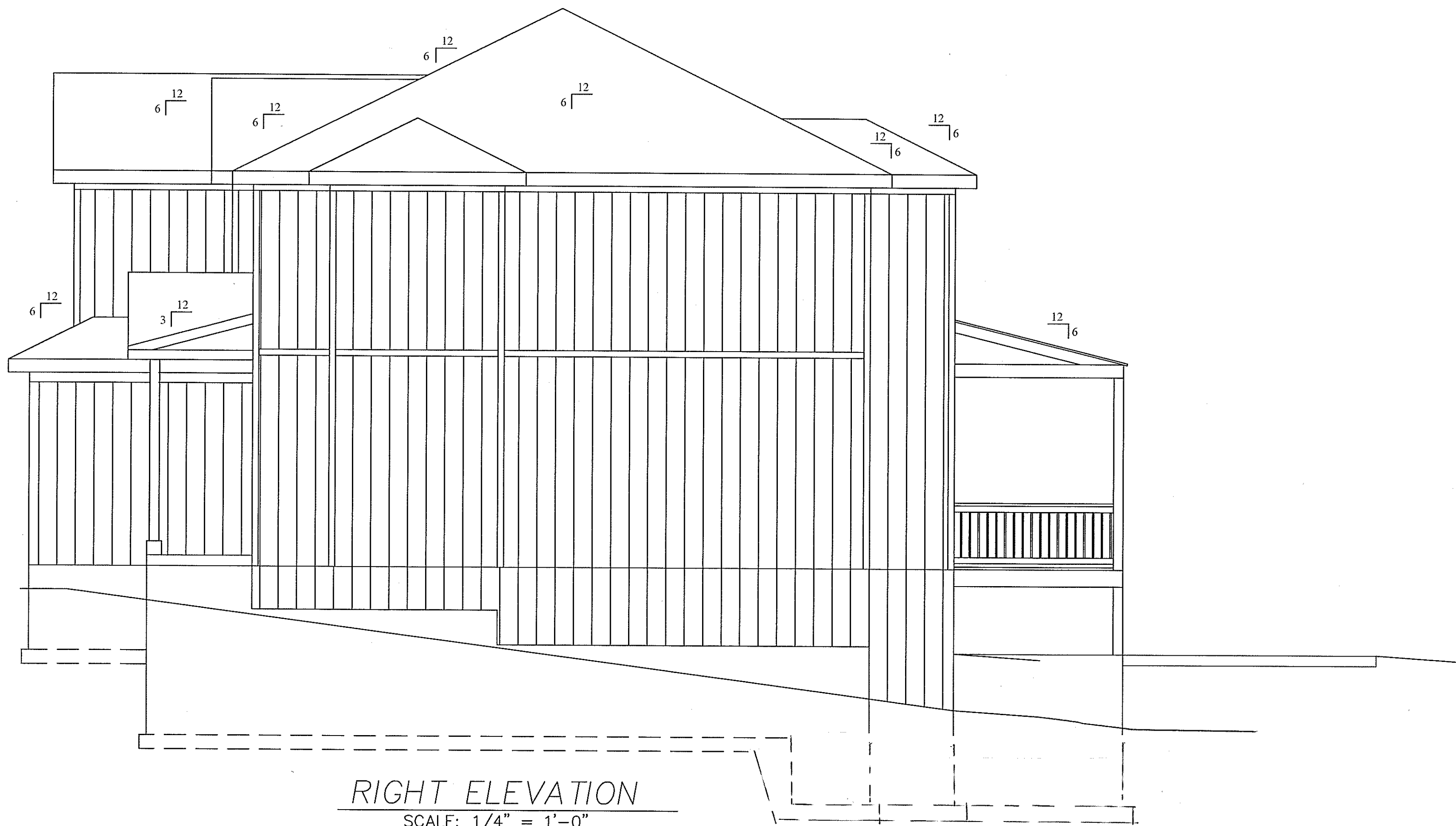
LSMO
Summit View Farms
Lot 58
3133 Blue Ribbon Rd.



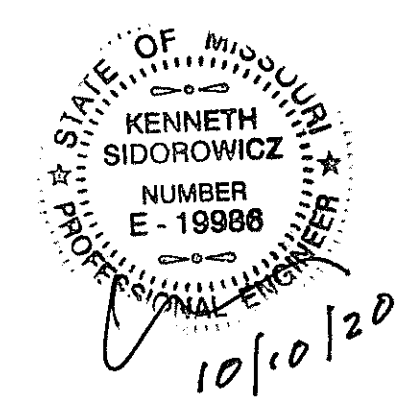
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1 of 6

SHEET NO:



LSMO
Summit View Farms Lot 58
3133 Blue Ribbon Rd.



DESCRIPTION:
LEFT / RIGHT ELEVATIONS

MODEL:
BRANTLY A
DATE:
8/29/20

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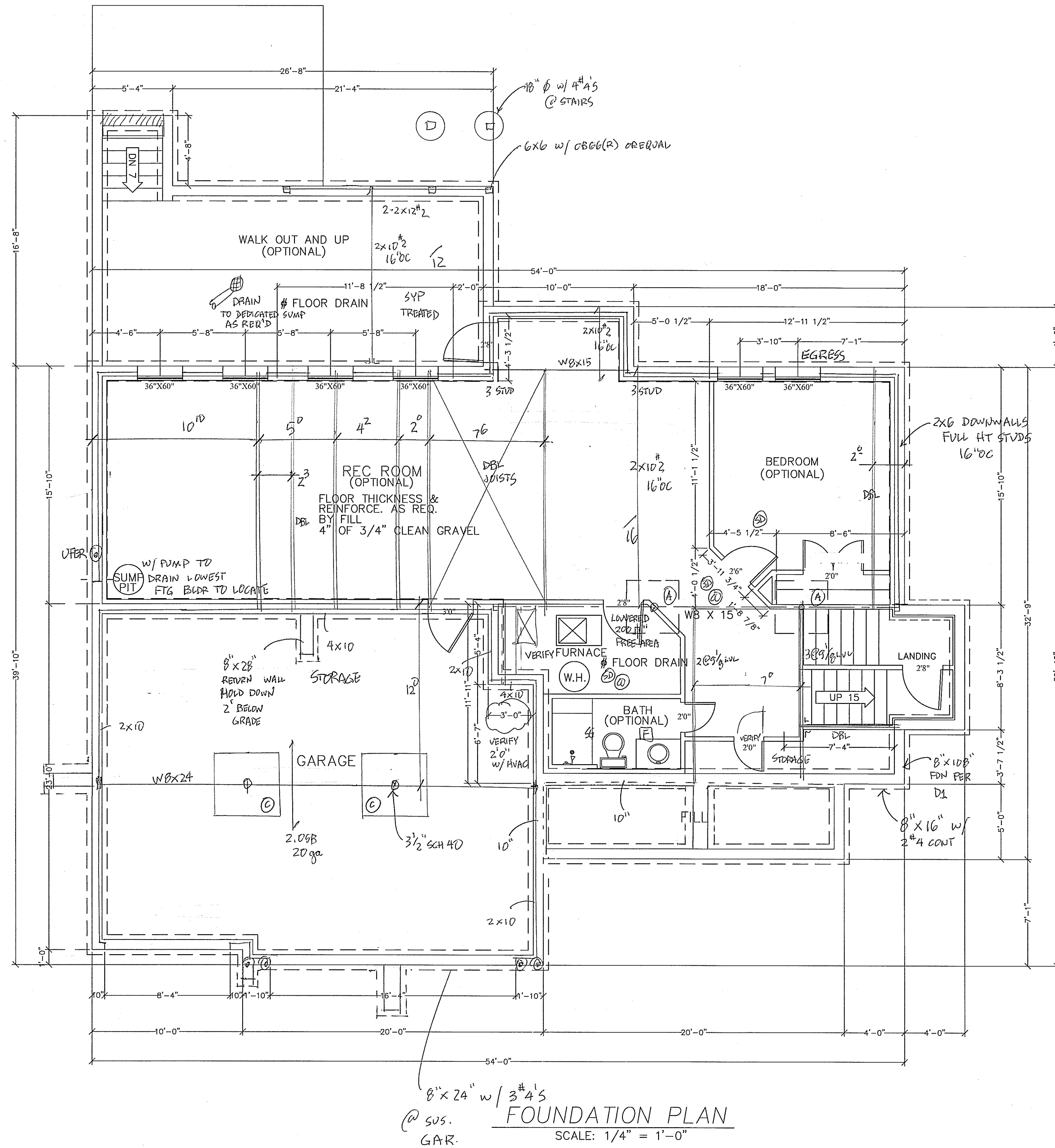
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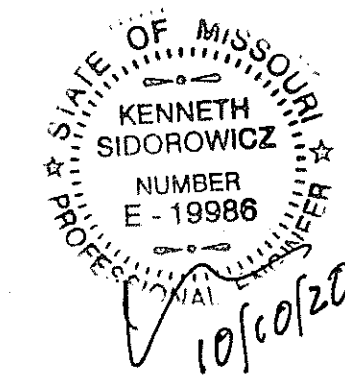
2 of 6

SHEET NO:

| | | |
|---------------------|------------|---------|
| FIELD VERIFY LENGTH | | BEAMS |
| LENGTH | | SIZE |
| 24'11" | | W8 X 15 |
| 30'0" | | W18X45 |
| 2 POSTS | ADJUSTIBLE | |



LSMO
Summit View Farms Lot 58
3133 Blue Ribbon Rd.



DESCRIPTION:

FOUNDATION

| |
|------------------|
| MODEL: |
| <i>BRANTLY A</i> |
| DATE: |
| <i>8/29/20</i> |

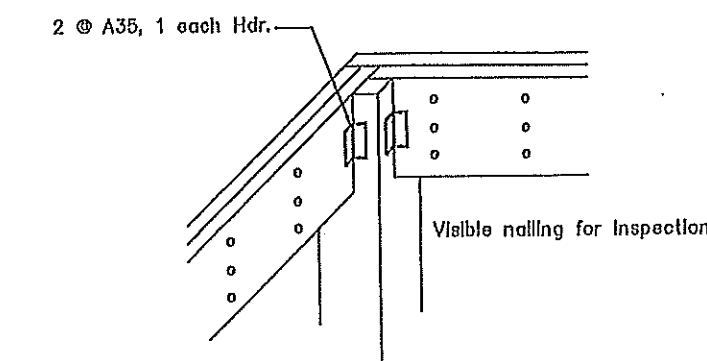
ARCHITECT IS NOT RESPONSIBLE FOR THE STRUCTURAL ELEMENTS OF THESE PLANS. A STRUCTURAL ENGINEER MAY NEED TO VERIFY ALL STRUCTURAL ASPECTS OF THESE PRINTS BEFORE CONSTRUCTION BEGINS. FIELD CONDITIONS MAY BE DIFFERENT FROM PLAN. ALL STATE AND LOCAL CODES TAKE PRECEDENCE OVER THESE PLANS. CONTRACTOR WILL BE RESPONSIBLE FOR PLAN INTEGRITY AND CODE COMPLIANCE

BUILD
SET

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3 of 6

SHEET NO:



(CS-WSP) HOUSE IS SHEATHED W/ 7/8" OSB
APA PANELS, SMART PANEL OR
EQUAL, INSTALLED PER MANU.
SPECS, SHIP LAPPED PANELS
REQUIRE NAILING OF OVER AND
UNDER PANELS SEPARATELY.

(LIB) INT SHALL BE SIMPSON STRAP
(CS16)

(CS-PF) HEADER LENGTHS ARE SHOWN
FOR CS-PF

SIDING LAPS RIM
2x4, 9' PLATE, FULL HT. STUDS
S.C. = SELF CLOSING
D2 GN #25 FOR WINDOWS
CS = CONTINUOUSLY SHEATHED
EC = END CONDITION
SEE D2 FOR INSULATION VALUES
EC/#, 16" LONG CS16 STRAP,
CENTERED ON SUBFLOOR, FILL
ALL NAIL HOLES.

FIRST FLOOR FRAMING

ARCHITECT IS NOT RESPONSIBLE FOR THE STRUCTURAL ELEMENTS OF THESE PLANS. A STRUCTURAL ENGINEER MAY NEED TO VERIFY ALL STRUCTURAL ASPECTS OF THESE PRINTS BEFORE CONSTRUCTION BEGINS. FIELD CONDITIONS MAY BE DIFFERENT FROM PLAN. ALL STATE AND LOCAL CODES TAKE PRECEDENCE OVER THESE PLANS. CONTRACTOR WILL BE RESPONSIBLE FOR PLAN INTEGRITY AND CODE COMPLIANCE

BUILD
SET

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SHEET NO:

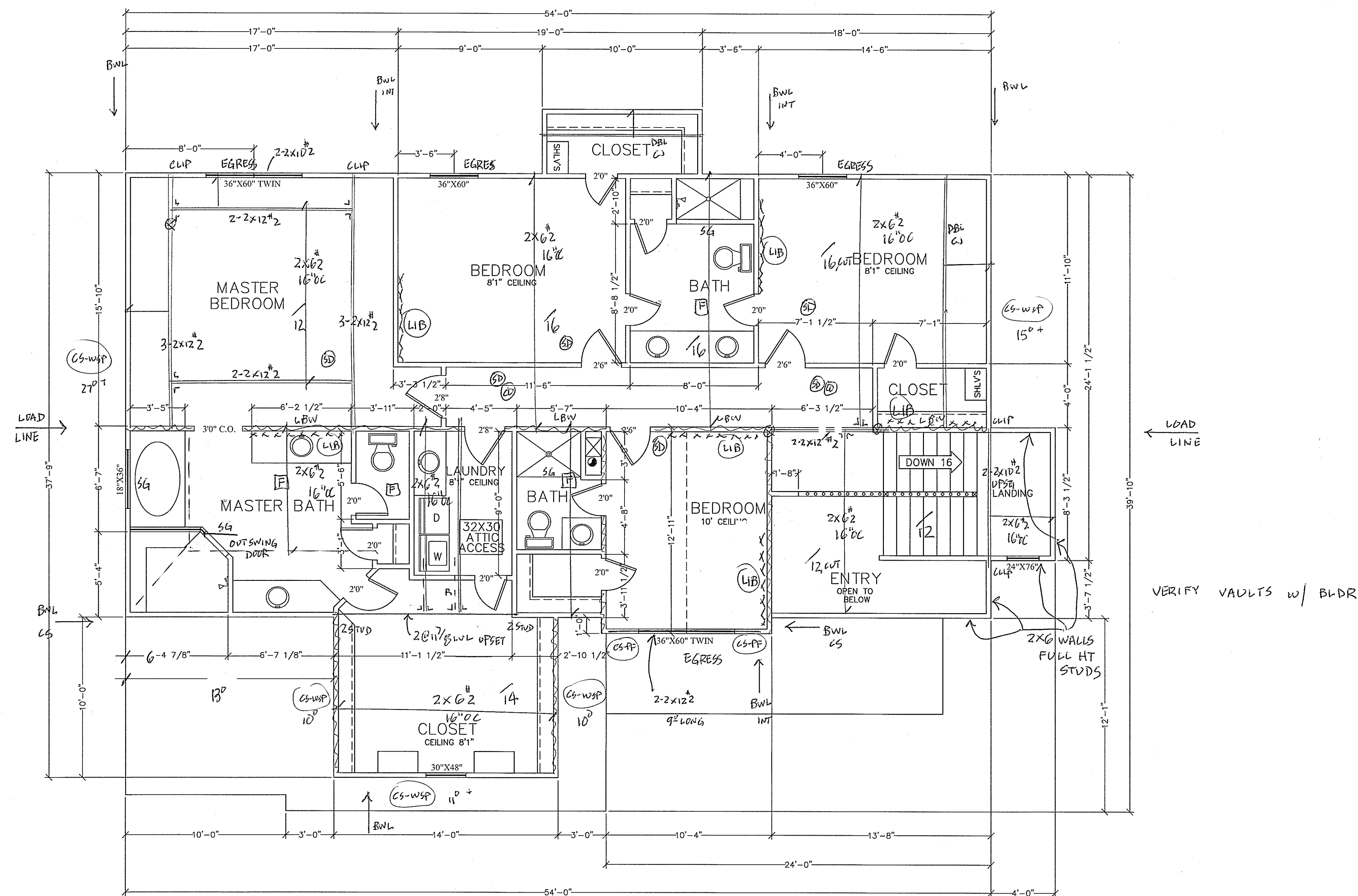
FIRST FLOOR PLAN
SCALE: 1/4" = 1'-0"

$$\begin{array}{r} 1\text{ST SQUARE FEET} = 1239 \\ 2\text{ND SQUARE FEET} = 1633 \\ \hline \text{TOTAL SQUARE FEET} = 2872 \end{array}$$

LSMO
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3133 Blue Ribbon Rd.

STATE OF MISSOURI
KENNETH SIDOROWICZ
NUMBER
E - 19986

10/10/20



SECOND FLOOR PLAN
SCALE: 1/4" = 1'-0"

2ND SQUARE FEET = 1633

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Summit View Farms Lot 58
3133 Blue Ribbon Rd.



DESCRIPTION:
SECOND FLOOR FRAMING
ROOF FRAMING PLAN

MODEL:
BRANTLY A
DATE:
8/29/20

ARCHITECT IS NOT RESPONSIBLE FOR THE STRUCTURAL ELEMENTS OF THESE PLANS. A STRUCTURAL ENGINEER MAY NEED TO VERIFY ALL STRUCTURAL ASPECTS OF THESE PRINTS BEFORE CONSTRUCTION BEGINS. FIELD CONDITIONS MAY BE DIFFERENT FROM PLAN. ALL STATE AND LOCAL CODES TAKE PRECEDENCE OVER THESE PLANS. CONTRACTOR WILL BE RESPONSIBLE FOR PLAN INTEGRITY AND CODE COMPLIANCE.

BUILD
SET

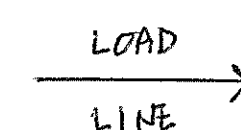
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5 of 6

SHEET NO:



RAFTER TIES SHALL BE PROVIDED
PER 802.3.1 WHEN THE CJ'S ARE
NOT CONNECTED TO THE RAFTERS
AT THE TOP PLATE



ROOF
ASPHALT SHINGLES - 2/2 MIN
WOOD MEMBRANES - 3/4 MIN
CONCRETE TIES - 24/16 MIN
CLAY & COUNTERFLASH ALL ROOF PENETRATIONS
AND INTERSECTIONS

RAPFERS & CEILING JOISTS
CEILING TIES AT UPPER THIRD POINT. 6" OC X 4 X 4 MIN
CEILING JOISTS ARE TYPICAL AS REQUIRED FOR RAPFER TIES

ROOF/RAPFER HANGERS AND SHAPES AS REQD
OUTSUPPORT REQD & GABLE END SORTERS FOR
COMP. ROOF. W SORTERS
OUTSUPPORT REQD & GABLE END SORTERS FOR TILE ROOF

ATTC VENTING
VENT GACH ENCLOSED ATTIC SPACE
NET AREA OPENING = 1/20TH OF VENTED AREA

UNLESS NOTED
RAPFERS ARE 2' X 8' @ 24" OC
MAX SPAN 8' 0"

PROVIDE SUPPORT FOR LOADS SUPPORT AT THE NOTED
LOAD POINTS FOR UPR. VALLIES, PURPOSE & PROPOSE
LET-AN SUPPORT LUGS TO PURLIN
ALL UPR. VALLIES & PROPOSE TO BE SIZED FOR
THE RAPFER DEPTH, RICH & 2X8 AND

| PUBUN | COMP | TILE |
|--------|--------|--------|
| | LEG OC | LEG OC |
| 2 X 0 | 4'-0" | -- |
| 2 X 0 | 0'-4" | 0'-4" |
| 2 X 10 | 0'-8" | -- |
| 2 X 10 | 0'-0" | 0'-0" |

| SUPPORT LEG | COMP | TILE |
|------------------------|-------------|-------------|
| | MAX. LENGTH | MAX. LENGTH |
| 2 X 4 W/ 2 X 4 T-BRACE | 9'-0" | 7'-0" |
| 2 X 6 W/ 2 X 4 T-BRACE | 9'-0" | 6'-0" |
| 2 X 6 W/ 2 X 6 T-BRACE | 7'-0" | 6'-0" |
| 2 X 6 W/ 2 X 4 T-BRACE | 9'-0" | 7'-0" |
| 2 X 6 W/ 2 X 6 T-BRACE | 7'-0" | 6'-0" |

| HEEL JOINT CONNECTION FACTOR | |
|------------------------------|-----|
| H_0/H_n | |
| $1/3$ | 16 |
| $1/4$ | 133 |
| $1/5$ | 125 |
| $1/6$ | 12 |
| $1/10$ OR LESS | 111 |

H_0 = HEIGHT OF CEILING JOISTS OR RAFTER TIES MEASURED VERTICALLY ABOVE TOP OF RAFTER SUPPORT WALL.
 H_n = HEIGHT OF ROOF RIDGE MEASURED VERTICALLY ABOVE THE TOP OF RAFTER SUPPORT WALL.

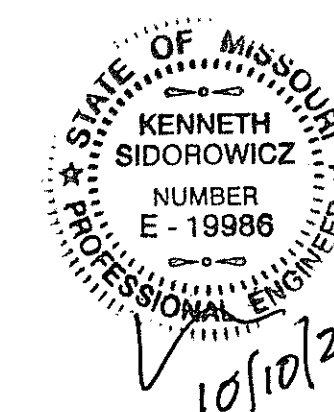
ALL ROOF FRAMING MEMBERS ARE SIZED AS BEAMS AND BRACES TO LBW₆ HEADERS OR OTHER STRUCTURE

2X6 WALLS
FULL HT
STUDS

6/12 VNO

ROOF FRAMING PLAN
SCALE: 1/4" = 1'-0"

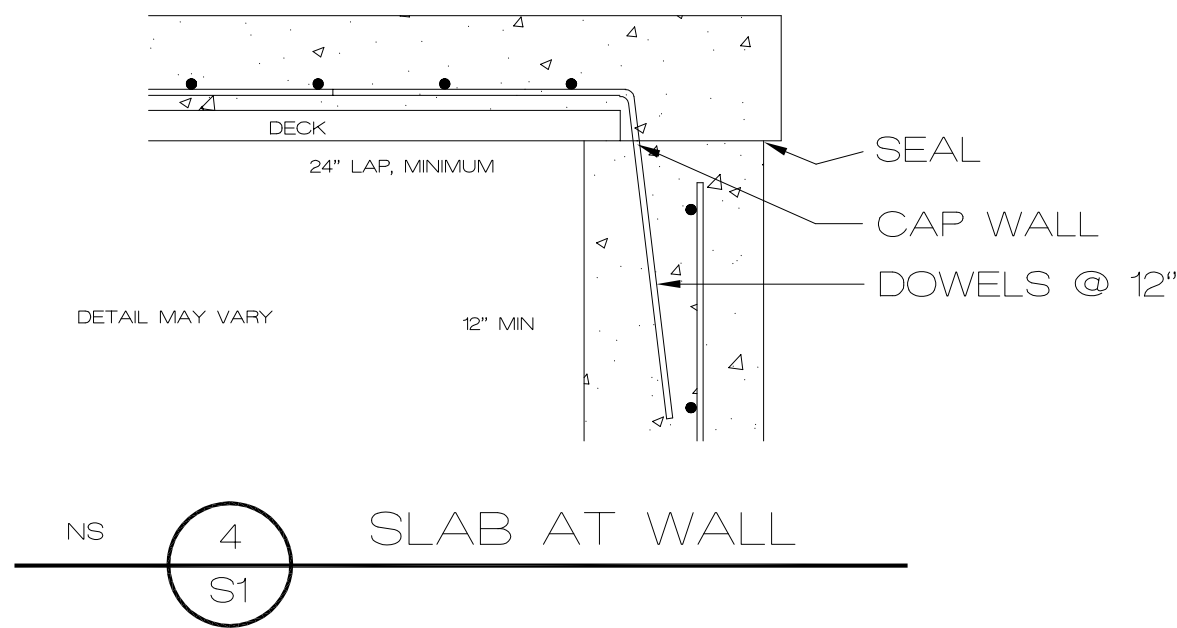
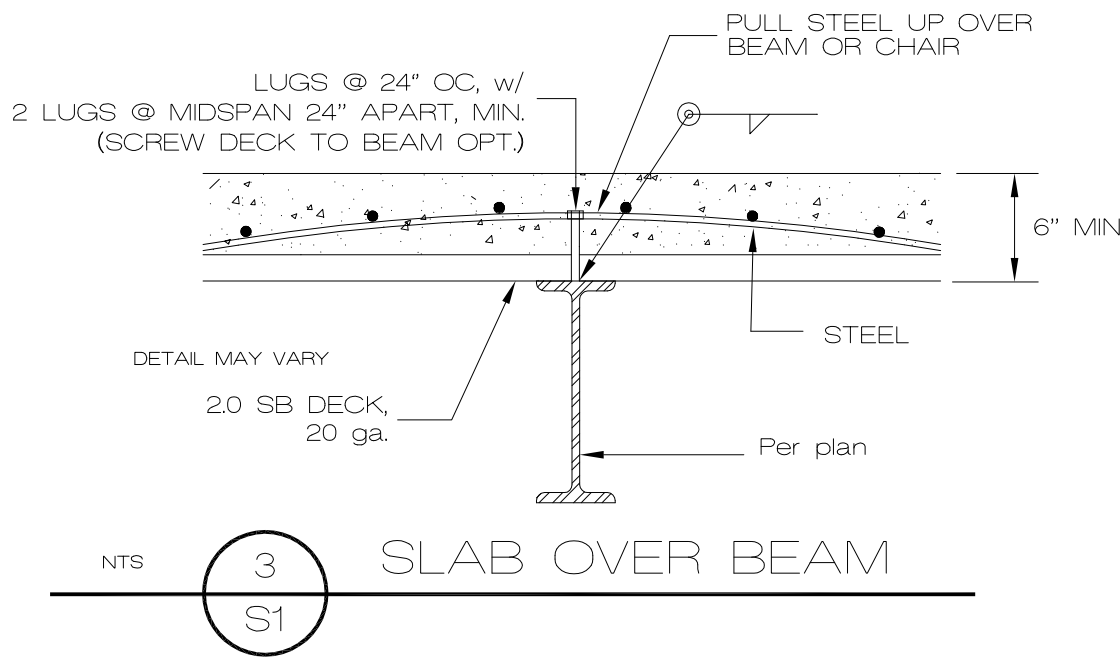
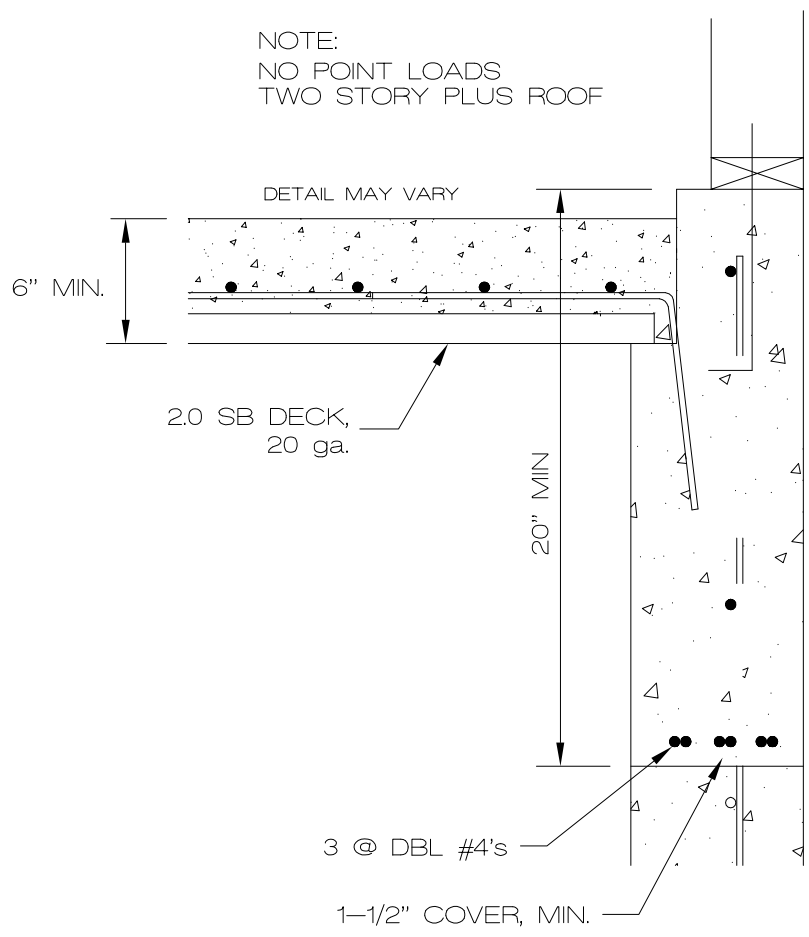
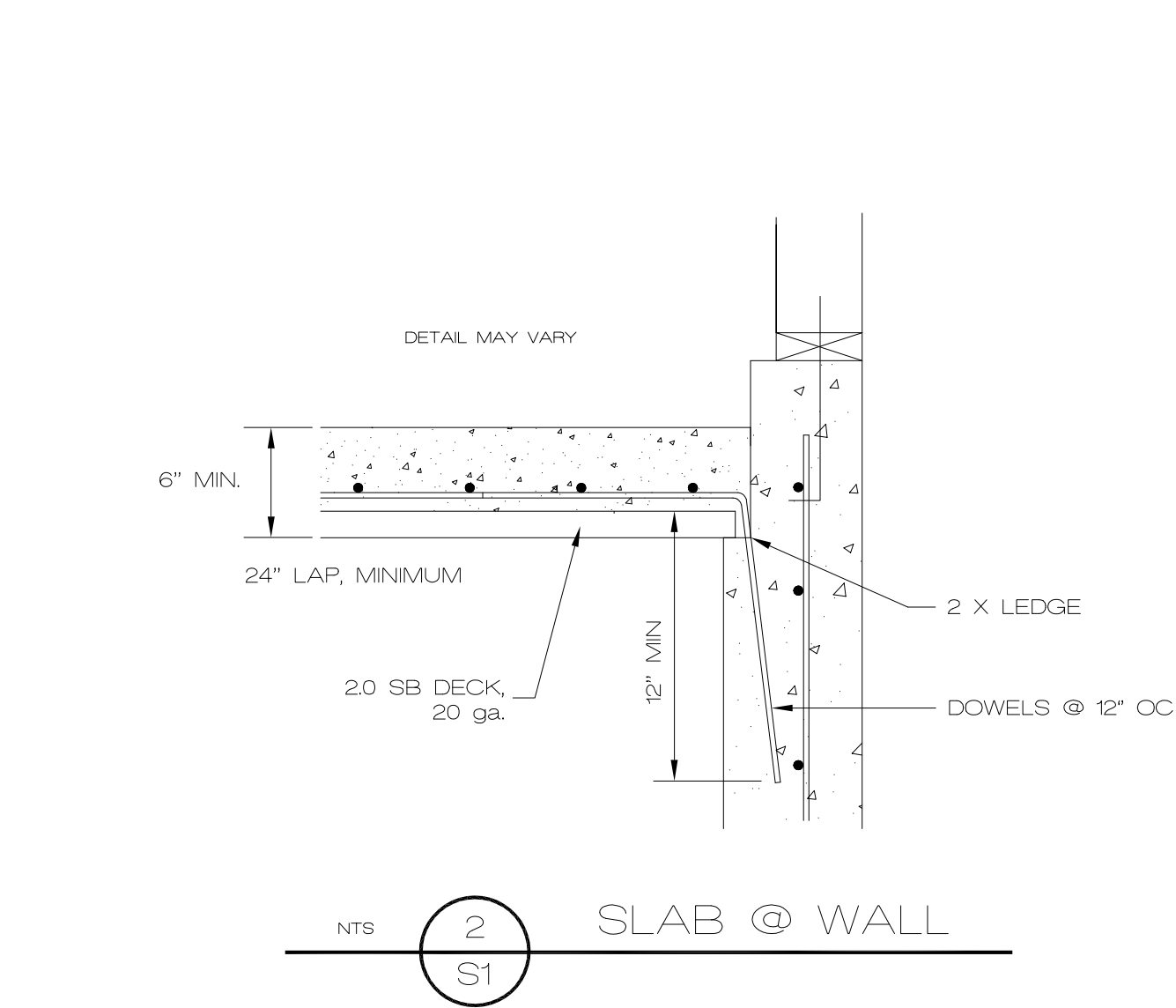
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SHEET NO:



2.0 SB Normal Weight

Gage

t in

Wd psf

Sp in³

Sn in³

Ip in⁴

In in⁴

As in²

Fy ksi

22

0.0295

2.0

0.257

0.258

0.317

0.309

0.472

50

20

0.0358

2.3

0.334

0.337

0.402

0.393

0.573

50

18

0.0474

3.0

0.507

0.517

0.557

0.552

0.759

40

16

0.0600

3.7

0.659

0.663

0.705

0.705

0.961

40

145 pcf Normal Weight Concrete

Total Slab Depth D

Gage

Maximum Unshored Clear Spans

Composite Properties

Superimposed Live Loads — psf: No Studs

Wt. Conc. Area Conc.

Single Span

Double Span

Triple Span

avg in⁴ / ft

Sc in³ / ft

7—0"

7—6"

8—0"

8—6"

9—0"

9—6"

10—0"

10—6"

11—0"

11—6"

12—0"

12—6"

6"

22

6'—2"

7'—11"

8'—2"

12.702

1684

400

400

400

400

366

322

284

252

224

200

179

161

144

60.4 psf

20

7'—2"

9'—1"

9'—5"

13.548

2.010

400

400

400

400

393

348

309

276

247

222

200

181

42.7 in²

18

8'—0"

10'—0"

10'—4"

14.981

2.589

400

400

400

400

400

359

320

285

256

230

207

187

16

9'—3"

11'—4"

11'—9"

16.369

3.184

400

400

400

400

400

359

320

285

256

230

207

187

Metal Decking Details

NOTES:

SET LEDGE
658 #s OF CEMENT PER YD. MINIMUM (7 SACK)
PROVIDE TEMPORARY DECK SUPPORT, READY AT INSPECTION
SLOPE SLAB TO DOORS OR FLOOR DRAIN OPTION
FLOOR DRAINS SHALL NOT INTERFERE WITH SLAB REINFORCING (OPT.)
REBAR SHALL BE GR40 MIN.
TIE STEEL TO PREVENT DISPLACEMENT
SEAL OR WATERSTOP AS REQ'D
HOOK AND TIE STEEL AS POSSIBLE
SET STEEL ON CHAIRS AS REQ'D
SEAL AT PERIMETER AS REQ'D
SEAL ALL PENETRATIONS
LUGS ON BEAMS CAN BE 3/8 BOLTS OR 1/2" REBAR
SEAL OPTIONAL DRAIN AS REQ'D
DO NOT SAW CUT STRUCTURAL SLAB w/o APPROVAL
CONSTRUCTION SHALL MEET ALL APPLICABLE STANDARDS
CONSTRUCTION SHALL COMPLY WITH IRC
BRACE WALLS OR LIMIT BACKFILL UNTIL SLAB IS POURED
ADJUST FDN FOR SITE & SOIL CONDITIONS

Ken Sidorowicz, PC

P.O. Box 12089, Parkville, Missouri 64152
Tel. (816) 741-0852 Fax (816) 741-0858

Spellerberg

SVF 58
LSMO



10/7/20

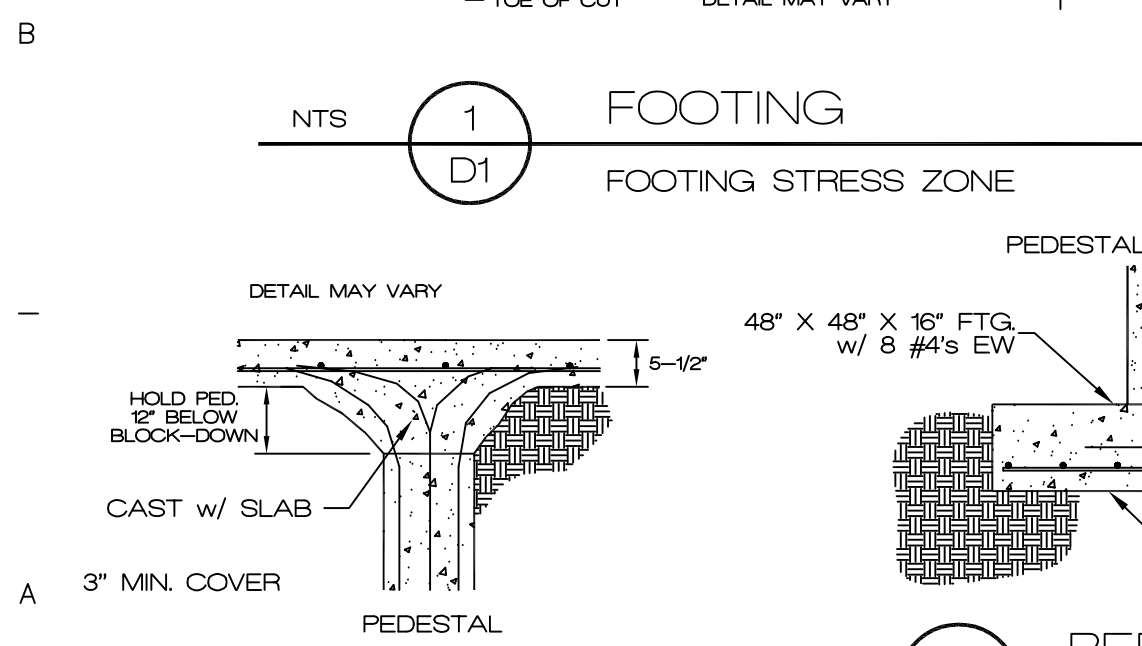
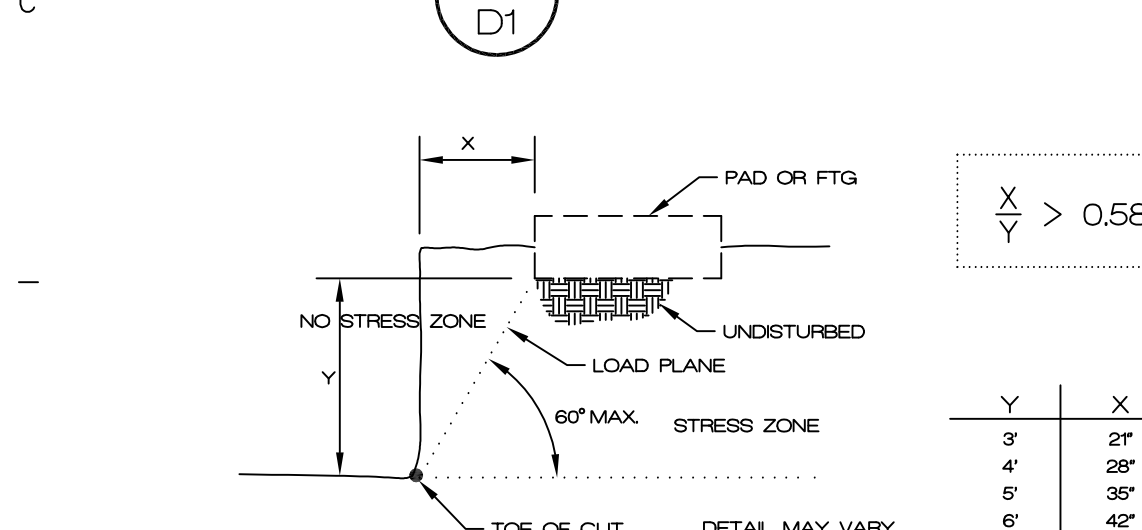
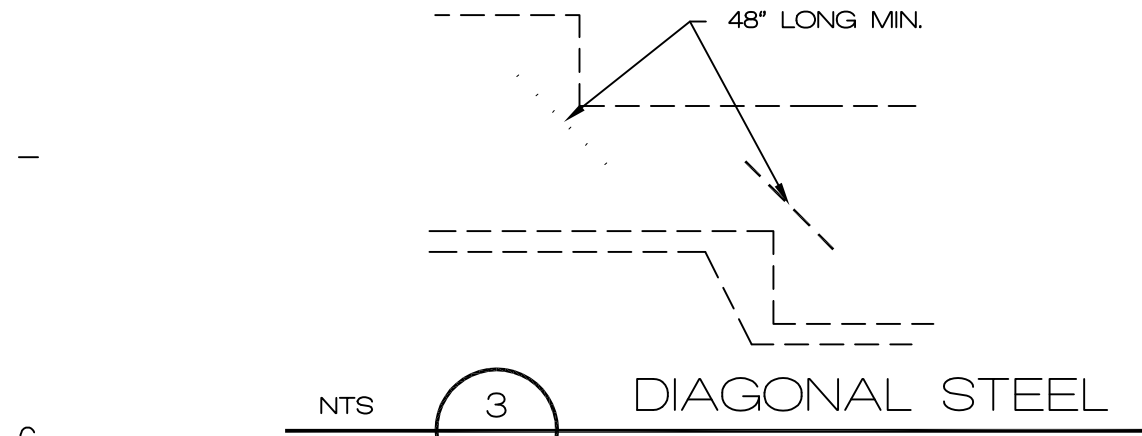
S1

DIVISION 1 – GENERAL REQUIREMENTS

- DESIGN AND CONSTRUCTION WORK FOR THIS PROJECT SHALL CONFORM TO THE REQUIREMENTS OF THE 2018 IRC.
- FURNISH ALL LABOR, MATERIALS, AND EQUIPMENT NECESSARY TO COMPLETE THE WORK AS SHOWN OR INFERRED BY THE DRAWINGS.
- DESIGN FACTORS:
 - GROUND SNOW LOAD (INCLUDING DRIFTING SNOW) 20 PSF
 - WIND SPEED (EXPOSURE B) 115 MPH
 - SEISMIC CATEGORY (A), GROUND ACCELERATION = NA
- DESIGN LOADS (PSF, UNLESS NOTED OTHERWISE):
 - ROOF (LL/DL) SEE TABLE
 - FLOOR (LL/DL) SEE TABLE
 - CEILING (LL/DL) SEE TABLE, (3/10 TRUSSES)
- DO NOT SCALE DRAWINGS. IF DIMENSIONS ARE IN QUESTION, OBTAIN CLARIFICATION FROM A / E BEFORE CONTINUING CONSTRUCTION.
- THE CONTRACTOR SHALL EXAMINE ACTUAL JOB CONDITIONS AND BE RESPONSIBLE FOR VERIFYING ALL DIMENSIONS AND ELEVATIONS SHOWN ON THE PLANS. IF ERRORS, OMISSIONS, OR DISCREPANCIES ARE FOUND THEY SHALL BE REPORTED TO THE DESIGN PROFESSIONAL BEFORE PROCEEDING WITH THE WORK.
- DIMENSIONS FOR NEW CONSTRUCTION ARE TO FACE OF FINISH OR COLUMNS AND FACE OF CONCRETE, WOOD, OR MASONRY WALLS UNLESS OTHERWISE INDICATED. DIMENSIONS INDICATE NOMINAL DIMENSIONS RATHER THAN ACTUAL DIMENSIONS.
- CONTRACTOR IS RESPONSIBLE FOR COORDINATING ALL TRADES EVEN IF THE TRADE IS UNDER A SEPARATE CONTRACT.
- PROVIDE SUFFICIENT STUDS AND BLOCKING WHERE REQUIRED TO SUPPORT EQUIPMENT AND/OR MISCELLANEOUS ITEMS, IE, LOAD POINTS, TYPICAL CASEWORK, CABINETS, GRAB BARS ETC.
- PRETREAT FOUNDATION FOR TERMITES AS REQUIRED.
- GARAGE DOORS AND FRAMES SHALL BE DESIGNED AND INSTALLED TO MEET THE 90 MPH WIND LOAD RESISTANCE REQUIREMENTS OF DASHMA 108 AND ASTM E 930-96.
- ALL EXTERIOR DOORS, INCLUDING THE DOOR LEADING FROM THE GARAGE TO THE DWELLING UNIT, SHALL INCORPORATE THE PHYSICAL SECURITY PROVISIONS OF THE JURISDICTION IN WHICH THE CONSTRUCTION TAKES PLACE.

DIVISION 2 – EARTHWORK

- ALL PROPERTY MARKERS SHALL BE EXPOSED.
- ALL FOOTINGS ARE DESIGNED TO BEAR ON NATURAL UNDISTURBED SOIL CAPABLE OF ADEQUATELY SUSTAINING A MINIMUM BEARING PRESSURE OF 1500 PSF. IF SUITABLE UNDISTURBED BEARING CAPACITY IS NOT ENCOUNTERED AT THE ELEVATION INDICATED ON THE DRAWINGS, CONTRACTOR SHALL NOTIFY THE ENGINEER OF RECORD IMMEDIATELY.
- ALL TOPSOIL, ORGANIC MATERIAL, AND EXISTING STRUCTURES SHALL BE REMOVED FROM BUILDING AREA AND FROM AREAS TO BE PAVED. STOCKPILE ALL TOPSOIL FOR REUSE.
- REFERENCE THE SOILS REPORT FOR ALL FILL CONDITIONS.
- OVEREXCAVATE BUILDING AREA BELOW SLAB SUBGRADE ELEVATION AND REPLACE WITH MATERIAL PER SOILS REPORT, VERIFY.
- SITE EROSION CONTROL SHALL COMPLY WITH ALL STATE AND LOCAL ORDINANCES.
- IN-SITU SOIL CONDITIONS, SEE SOILS REPORT OR 1500 PSF BEARING & 60 PCF EQUIVALENT FLUID WEIGHT.
- SOIL CONDITIONS AT THE DEPTH OF EXCAVATION FOR THE FOOTING SHALL BE UNIFORM AND CONSISTENT. NOTIFY THE ENGINEER OF RECORD OF ANY INCONSISTENCIES.
- CONTRACTOR SHALL BE RESPONSIBLE FOR REMOVING AND DISPOSING OF ANY EXCESS EXCAVATION MATERIALS AND FOR OBTAINING AND SUPPLYING ADDITIONAL FILL MATERIAL AS REQUIRED.

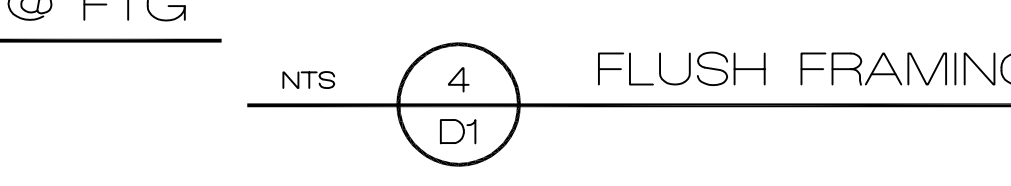
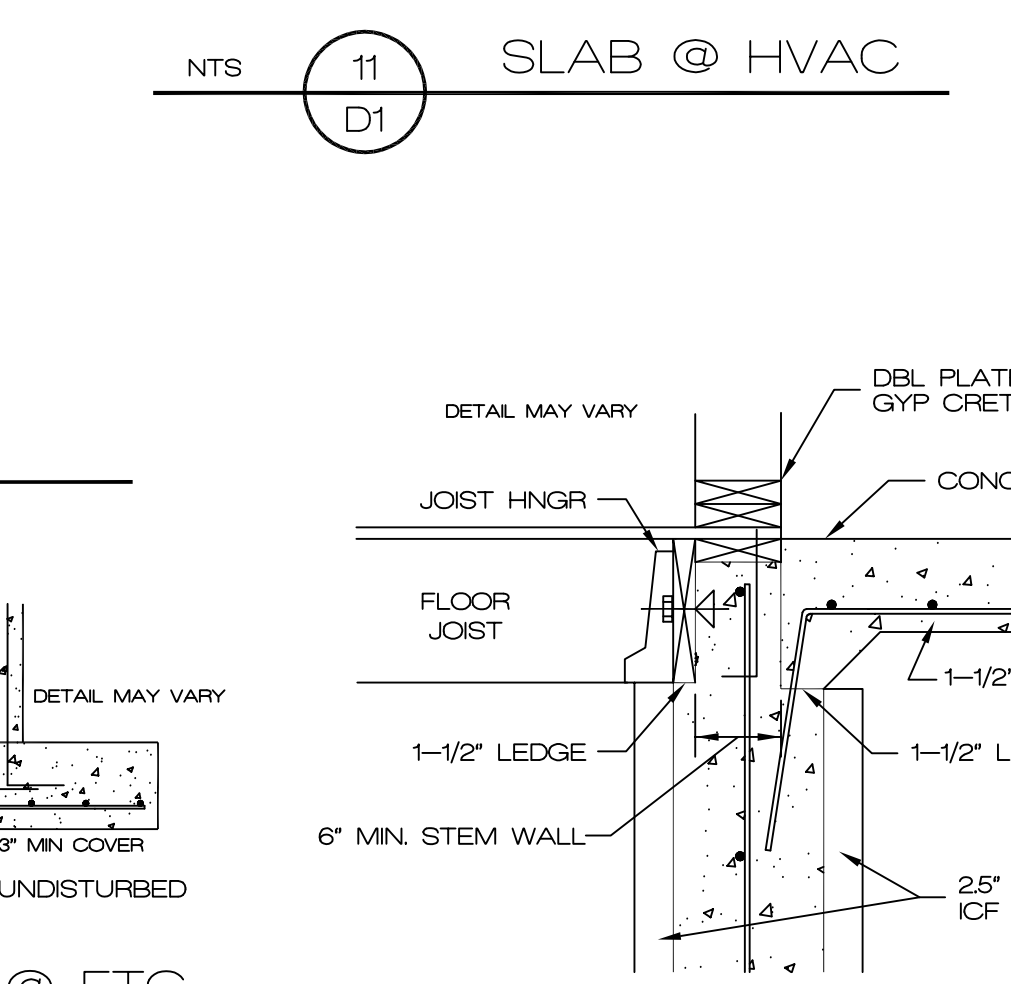
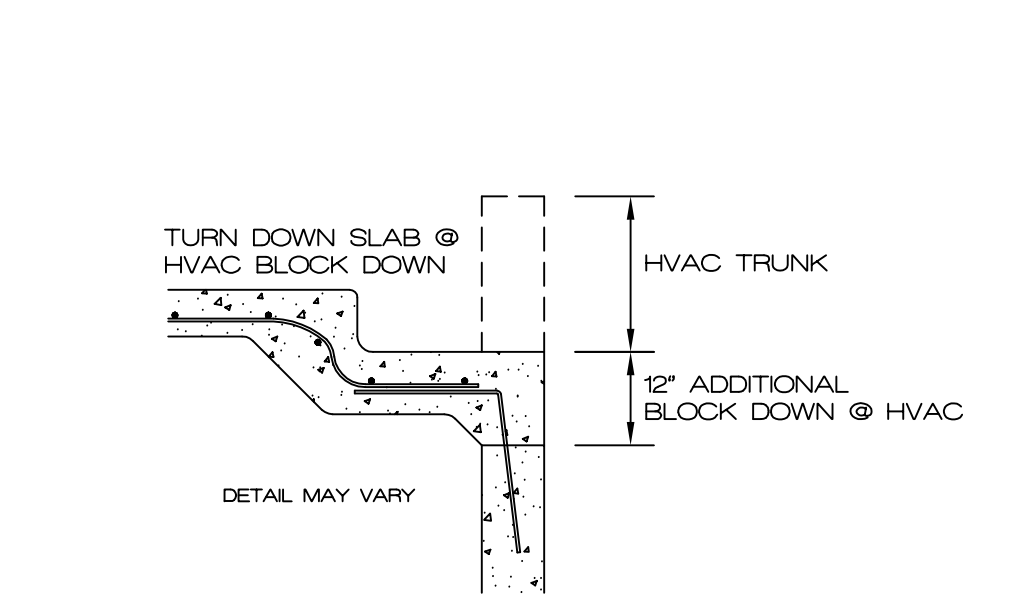
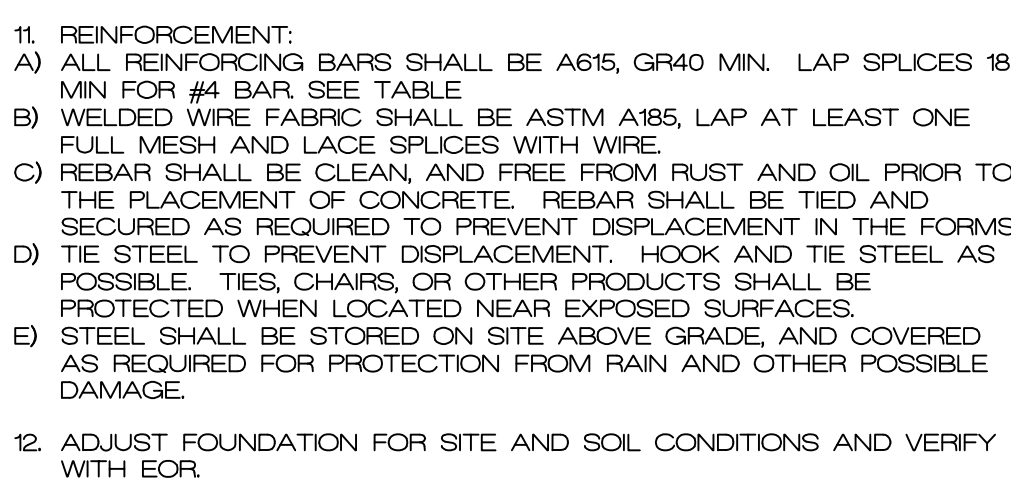
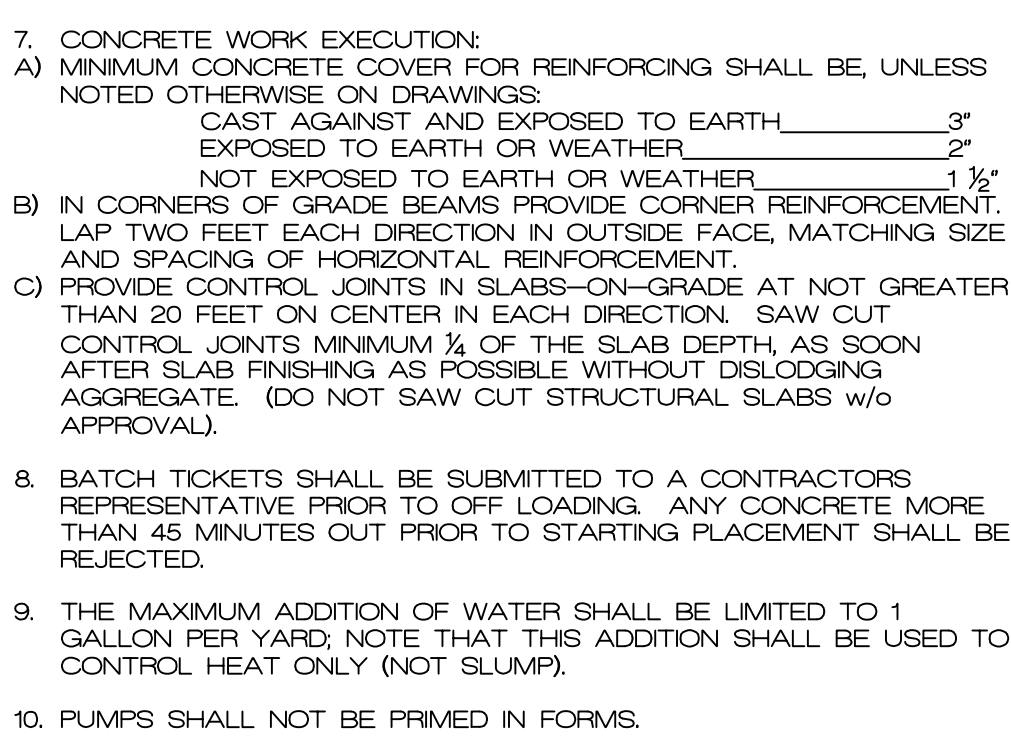


DIVISION 3 – CONCRETE

- ALL CONCRETE WORK SHALL CONFORM TO THE REQUIREMENTS OF ACI 318 "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE" AND ACI 332 "REQUIREMENTS FOR RESIDENTIAL CONCRETE CONSTRUCTION".
 - CEMENT – ASTM C 150 TYPE 1
 - AGGREGATE – ASTM C 33, MAXIMUM AGGREGATE SIZE 3/4"
 - WATER – POTABLE, WATER/CEMENT RATIO .5 (MAX)
 - AIR-ENTRAINING ADMIXTURE – ASTM C 260
 - WATER-REDUCING ADMIXTURE – ASTM C 494, INCLUDING SUPERPLASTICIZERS
 - FLY ASH – ASTM C 618, CLASS C
- CONCRETE SHALL DEVELOP THE FOLLOWING MINIMUM 28 DAY DESIGN COMPRESSIVE STRENGTH (f'c):

| TYPE OF CONSTRUCTION | COMP. STRENGTH (f'c) |
|--|----------------------|
| A) FOOTINGS, WALLS, AND SLABS | SEE TABLE |
| B) EXTERIOR SLABS AND CURBS (AIR-ENTRAINED CONCRETE) | SEE TABLE |
- CONCRETE PROPORTIONS SHALL BE ESTABLISHED ON THE BASIS OF FIELD EXPERIENCE AND/OR TRIAL MIXTURES IN ACCORDANCE WITH ACI 318-89 SECTIONS 52 AND 53. WHEN FLY ASH IS UTILIZED IN THE MIX, MIX SHALL CONTAIN A WATER-REDUCER. FLY ASH SHALL BE ADDED AT THE RATE OF NOT MORE THAN 100 POUNDS PER CUBIC YARD AND CEMENT SHALL BE REDUCED BY NOT MORE THAN 15 PERCENT BY WEIGHT.
- PROPORTION AND DESIGN MIXES TO RESULT IN CONCRETE SLUMP AT A POINT OF PLACEMENT OF NOT MORE THAN 4" TO 5".
- USE AIR-ENTRAINING ADMIXTURES IN EXTERIOR EXPOSED CONCRETE TO RESULT IN CONCRETE AT POINT OF PLACEMENT HAVING AIR CONTENT OF 5 TO 7 PERCENT ENTRAINED AIR.
- ALL PLUMBING AND ELECTRICAL ROUGH-INS MUST BE COMPLETE, INSPECTED AND APPROVED BEFORE REQUESTING THE SLAB INSPECTION.
- CONCRETE WORK EXECUTION:
 - MINIMUM CONCRETE COVER FOR REINFORCING SHALL BE, UNLESS NOTED OTHERWISE ON DRAWINGS:

| CAST AGAINST AND EXPOSED TO EARTH | EXPOSED TO EARTH OR WEATHER |
|-----------------------------------|-----------------------------|
| 3" | 2" |
 - IN CORNERS OF GRADE BEAMS PROVIDE CORNER REINFORCEMENT, LAP TWO FEET EACH DIRECTION IN OUTSIDE FACE, MATCHING SIZE AND SPACING OF HORIZONTAL REINFORCEMENT.
 - PROVIDE CONTROL JOINTS IN SLABS-ON-GRADE AT NOT GREATER THAN 20 FEET ON CENTER IN EACH DIRECTION. SAW CUT CONTROL JOINTS MINIMUM 1/4" OF THE SLAB DEPTH, AS SOON AFTER SLAB FINISHING AS POSSIBLE WITHOUT DISLODGING AGGREGATE. (DO NOT SAW CUT STRUCTURAL SLABS W/O APPROVAL).
- BATCH TICKETS SHALL BE SUBMITTED TO A CONTRACTORS REPRESENTATIVE PRIOR TO OFF LOADING. ANY CONCRETE MORE THAN 45 MINUTES OUT PRIOR TO STARTING PLACEMENT SHALL BE REJECTED.
- THE MAXIMUM ADDITION OF WATER SHALL BE LIMITED TO 1 GALLON PER YARD, NOTE THAT THIS ADDITION SHALL BE USED TO CONTROL HEAT ONLY (NOT SLUMP).
- PUMPS SHALL NOT BE PRIMED IN FORMS.
- REINFORCEMENT:
 - ALL REINFORCING BARS SHALL BE A615, GR40 MIN. LAP SPICES 18" MIN FOR #4 BAR SEE TABLE
 - WELDED WIRE FABRIC SHALL BE ASTM A185, LAP AT LEAST ONE FULL MESH AND LACE SPICES WITH WIRE
 - REBAR SHALL BE CLEAN, AND FREE FROM RUST AND OIL PRIOR TO THE PLACEMENT OF CONCRETE. REBAR SHALL BE TIED AND SECURED AS REQUIRED TO PREVENT DISPLACEMENT IN THE FORMS.
 - TIE STEEL TO PREVENT DISPLACEMENT. HOOK AND TIE STEEL AS POSSIBLE. TIES, CHAIRS, OR OTHER PRODUCTS SHALL BE PROTECTED WHEN LOCATED NEAR EXPOSED SURFACES.
 - STEEL SHALL BE STORED ON SITE ABOVE GRADE, AND COVERED AS REQUIRED FOR PROTECTION FROM RAIN AND OTHER POSSIBLE DAMAGE.
- ADJUST FOUNDATION FOR SITE AND SOIL CONDITIONS AND VERIFY WITH EOR.



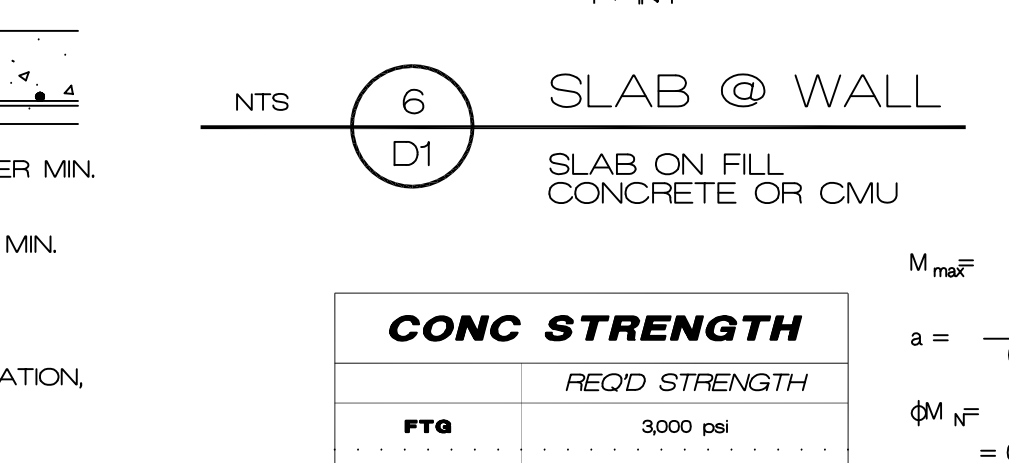
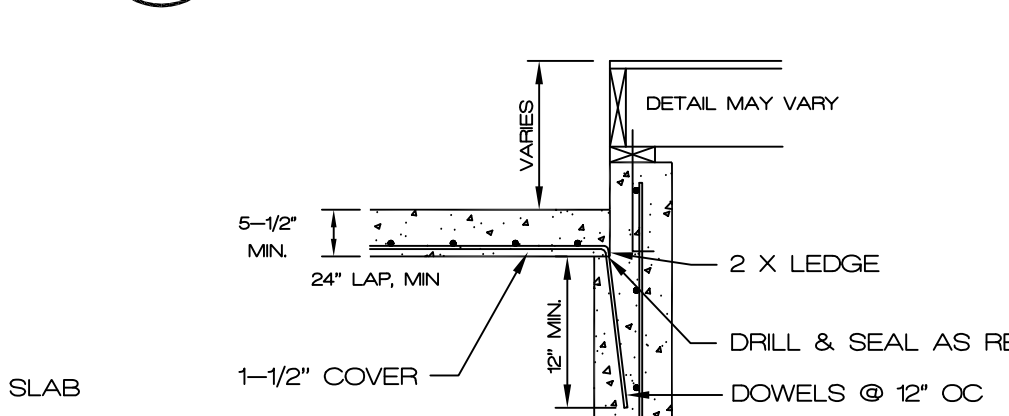
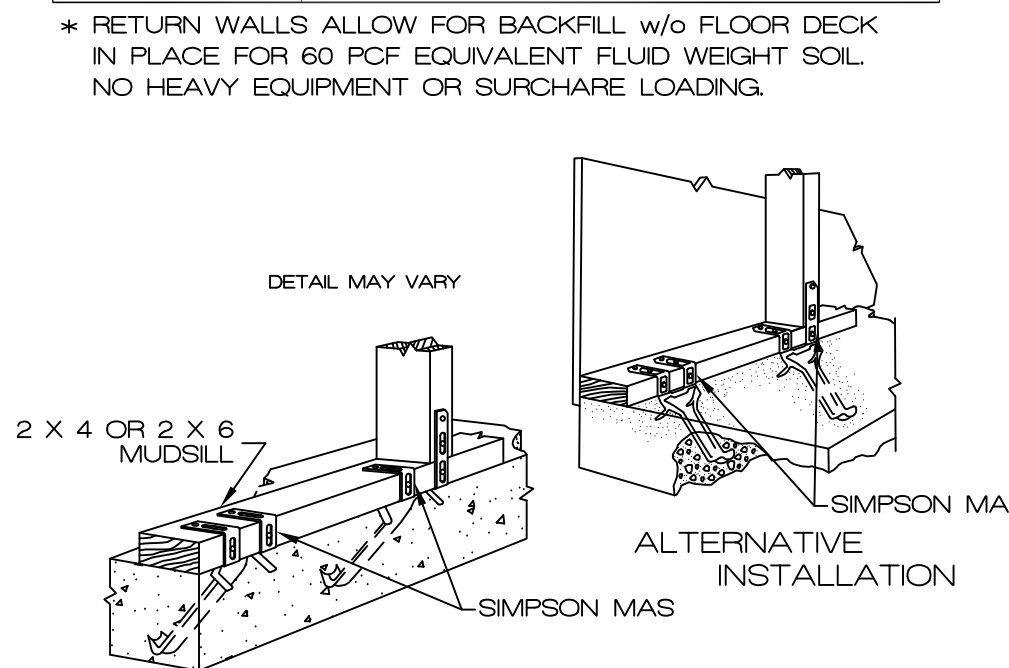
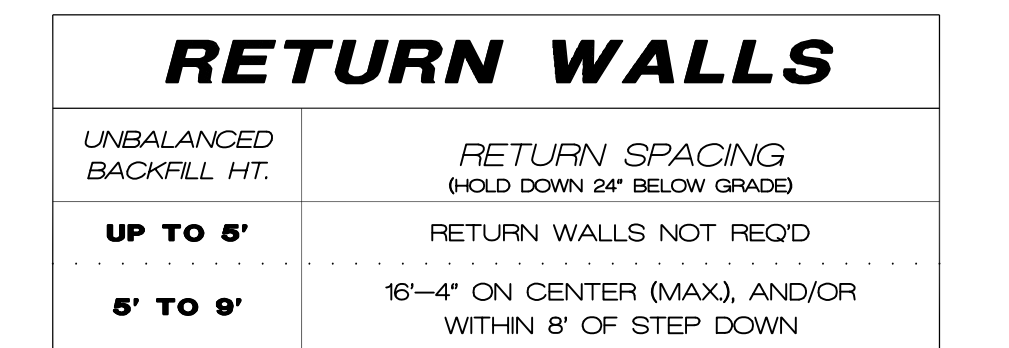
DIVISION 4 – MASONRY

- COMPRESSIVE STRENGTH OF CONCRETE MASONRY CONSTRUCTION (CMU) SHALL BE AS FOLLOWS (PSI): MASONRY STRENGTH NOT SPECIFICALLY NOTED ON PLAN SHALL BE (f'm) 1500 PSI.

| MASONRY STRENGTH (f'm DESIGN) | 1500 |
|-------------------------------|------|
| BLOCK STRENGTH | 1900 |
| MORTAR STRENGTH | 1800 |
| GROUT STRENGTH | 2000 |
- CONCRETE BLOCK SHALL BE HOLLOW LOAD-BEARING CONCRETE MASONRY UNITS CONFORMING TO ASTM C 90, TYPE N-IL. ALL BLOCKS SHALL BE PLACED IN RUNNING BOND CONSTRUCTION (UNLESS OTHERWISE NOTED) WITH ALL VERTICAL CELLS IN ALIGNMENT.
- MORTAR MIX SHALL CONFORM TO THE REQUIREMENTS OF ASTM C 270, TYPE M OR S. TYPE M MORTAR SHALL BE USED WHERE MASONRY IS IN CONTACT WITH SOIL.
- GROUT SHALL CONFORM TO THE REQUIREMENTS OF ASTM C 476. USE SUFFICIENT WATER FOR GROUT TO FLOW INTO ALL JOINTS OF THE MASONRY WITHOUT SEGREGATION. ALL CELLS IN CONCRETE BLOCKS CONTAINING REINFORCING SHALL BE FILLED SOLID WITH GROUT. ALL MASONRY BELOW FINISHED FLOOR OR GRADE SHALL BE GROUTED SOLID. HOLD GROUT DOWN 1-3/4" BELOW TOP OF BLOCK AT GROUT LIFT JOINTS AND AT CONCRETE PLACED OVER MASONRY.
- MINIMUM LINTEL, WHERE NOT ON PLANS, SHALL HAVE A MINIMUM OF 2 #4'S CONTINUOUS HORIZONTAL BARS IN BOTTOM OF BOND BEAM OR LINTEL BLOCK AND SHALL BE GROUTED SOLID TO A MIN. DEPTH OF 24". ALL LINTEL REINFORCING AND GROUT SHALL EXTEND 2" MINIMUM PAST JAMBS UNLESS NOTED OTHERWISE ON PLANS OR DETAILS.
- LAP REINFORCING 48 BAR DIAMETERS. STAGGER LAP SPICES A MINIMUM OF ONE LAP LENGTH.
- MASONRY VENEER SHALL BE ATTACHED TO SUPPORT WALL FRAMING WITH 3/8" DIAMETER WALL TIES OR DOVETAIL-TYPE METAL TIES OF EQUIVALENT STIFFNESS EMBEDDED INTO HORIZONTAL MORTAR JOINTS. MAXIMUM VERTICAL SPACING OF TIES SHALL BE 16". MAXIMUM HORIZONTAL SPACING SHALL BE 24". TIES IN ALTERNATE COURSES SHALL BE STAGGERED. PROVIDE #9 WIRE REINFORCING IN HORIZONTAL MORTAR JOINTS AT 16" OC. ENGAGE #9 WIRE WITH WALL ANCHOR TIES. CONSTRUCTION JOINTS IN MASONRY VENEER WALLS SHALL BE LOCATED PER THE DRAWINGS.
- WATERPROOFING, DRAINAGE PLANE, AND INSTALLATION PER ADOPTED BUILDING CODE.

DIVISION 5 – MISC. STRUCTURAL STEEL

- ALL MISCELLANEOUS STRUCTURAL STEEL WORK SHALL CONFORM TO THE REQUIREMENTS OF AISC "SPECIFICATIONS FOR DESIGN, FABRICATION, AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS".
- MISCELLANEOUS STRUCTURAL STEEL MATERIAL SHALL COMPLY WITH:
 - STRUCTURAL STEEL – ASTM A992
 - STEEL PIPE COLUMNS – ASTM A53 GRADE B(Sch 40 TYP)
 - ANCHOR BOLTS – ASTM A307 GRADE A, NON-HEADED TYPE UNLESS OTHERWISE NOTED.
- FLITCH PLATES SHALL HAVE 3/4" DIA. BOLTS @ 16" OC, STAGGERED TOP AND BOTTOM BETWEEN JOIST LAYOUT.



| CONC STRENGTH | |
|---------------|------------|
| FTG | 3000 psi |
| WALL | 3500 psi |
| SLAB | 3500 psi |
| SUB-SLAB | 7 SACK MIX |

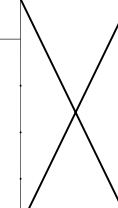

DIVISION 6 – ROUGH CARPENTRY

- ALL ROUGH CARPENTRY WORK SHALL CONFORM TO THE REQUIREMENTS OF NFPA "NATIONAL DESIGN SPECIFICATION OF WOOD CONSTRUCTION", TPI "DESIGN SPECIFICATIONS FOR LIGHT METAL PLATE CONNECTED WOOD TRUSSES", APA "PLYWOOD DESIGN SPECIFICATIONS", DOC PS 1 "PRODUCT STANDARD FOR CONSTRUCTION AND INDUSTRIAL PLYWOOD", DOC PS 56 "STRUCTURAL GLUED LAMINATED TIMBER", AND APPLICABLE SECTIONS OF THE INTERNATIONAL BUILDING CODE.
- ROUGH CARPENTRY MATERIALS SHALL COMPLY WITH:
 - LUMBER – S4S, S-DRY, KD, OR S-GRN GRADE MARKED, COMPLYING WITH PS 20, GRADED UNDER WWPA OR SPB RULES:

| STUDS: | STUD GRADE |
|----------|----------------------------|
| HEADER | #2 DOUGLAS FIR MIN TYPICAL |
| RAFTER | #2 DOUGLAS FIR |
| PLATES | #2 DOUGLAS FIR |
| BLOCKING | #2 DOUGLAS FIR |
 - METAL FRAMING FASTENERS – ASTM A 153, HOT-DIP GALVANIZED FASTENERS, EQUAL TO SIMPSON STRONG-TIE CONNECTORS COMPLYING WITH APPLICABLE ICC-ES REPORTS.
 - PLYWOOD – APA RATED SHEATHING, COMPLYING TO PS 1
 - LVL – LAMINATED VENEER LUMBER SHALL BE GRADE 2800 F-20E AND SHALL MEET THE REQUIREMENTS OF APPLICABLE ICC-ES REPORTS.
 - GLULAM BEAMS – COMBINATION 24F-V3 IN ACCORDANCE WITH AITC A19.01
- EXTERIOR WALL AND ROOF SHEATHING SHALL BE 3/4" APA RATED SHEATHING 24/0 EXTERIOR GLUED (MIN) FOR 16" OC STUD SPACING. NAIL SHEATHING TO SUPPORT MEMBERS WITH 8D COMMON NAILS AT 6" ON CENTER ALONG EDGE SUPPORTS AND 12" ON CENTER ALONG FIELD SUPPORTS UNLESS NOTED OTHERWISE. PROVIDE SOLID BLOCKING AT ALL UNSUPPORTED PANEL EDGES, 4/8 GUN NAILS.
- NOTE: ROOF SHEATHING SHALL BE 3/4" APA RATED SHEATHING FOR TILE ROOF, OR AS REQUIRED BY MANUFACTURER.
- INTERIOR SHEAR WALL SHEATHING WHERE NOTED SHALL BE 3/4" APA RATED SHEATHING 24/0 EXTERIOR GLUED (MIN) FOR 16" OC STUD SPACING. NAIL SHEATHING TO SUPPORT MEMBERS WITH 8D COMMON NAILS AT 4" ON CENTER ALONG EDGE SUPPORTS AND 6" ON CENTER ALONG FIELD SUPPORTS UNLESS NOTED OTHERWISE. PROVIDE SOLID BLOCKING AT ALL UNSUPPORTED PANEL EDGES.
- ATTACH METAL FRAMING FASTENERS TO FRAMING MEMBERS WITH MINIMUM NUMBER AND SIZE OF NAILS LISTED IN THE APPLICABLE ICC-ES REPORTS.
- WOOD TRUSS SYSTEM, TRUSS JOIST SYSTEM AND GLULAM SYSTEM FOR ROOFS:
 - DESIGN, FABRICATE, AND ERECT IN ACCORDANCE WITH BCSI STANDARDS AND NDS SPECIFICATIONS.
 - DESIGN LOADS:

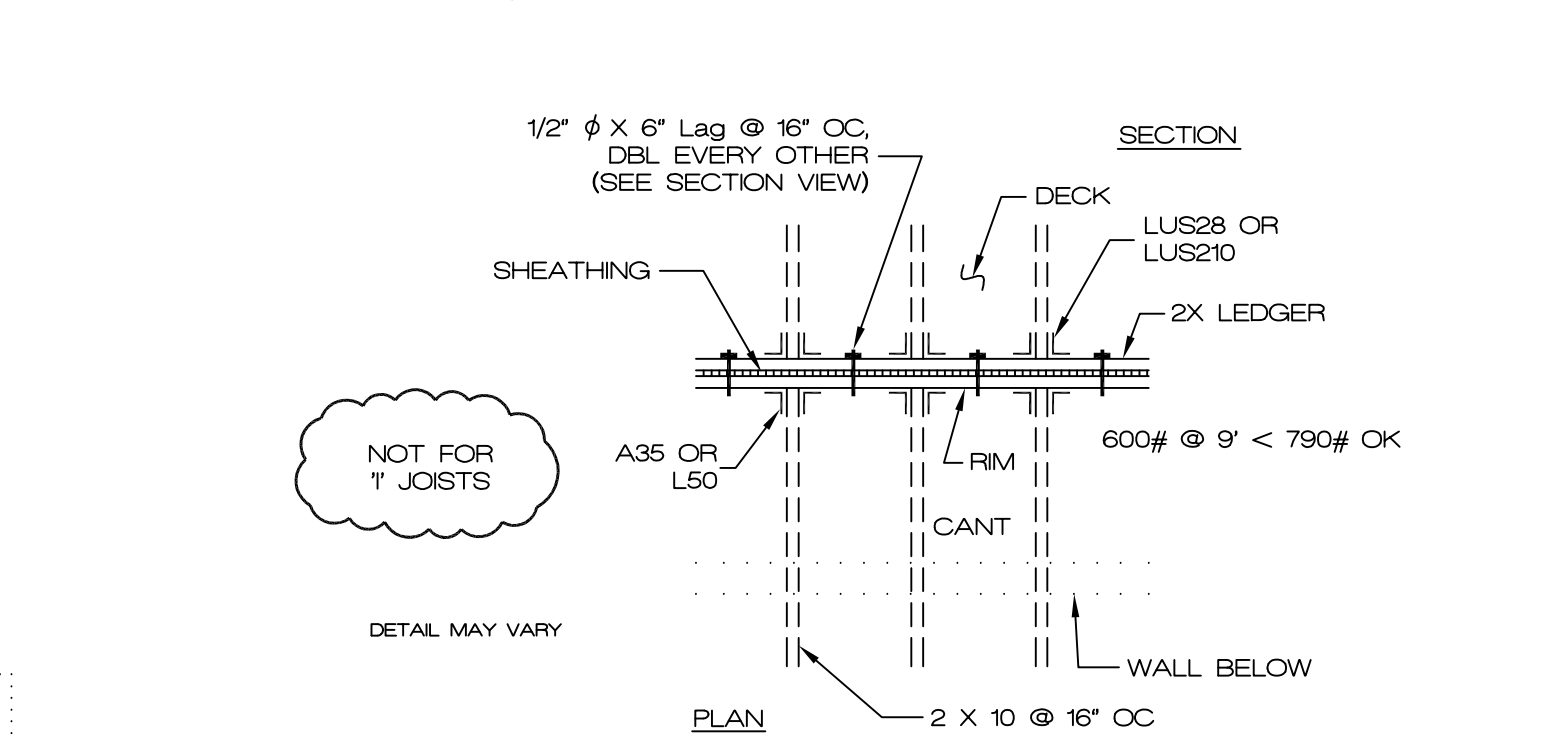
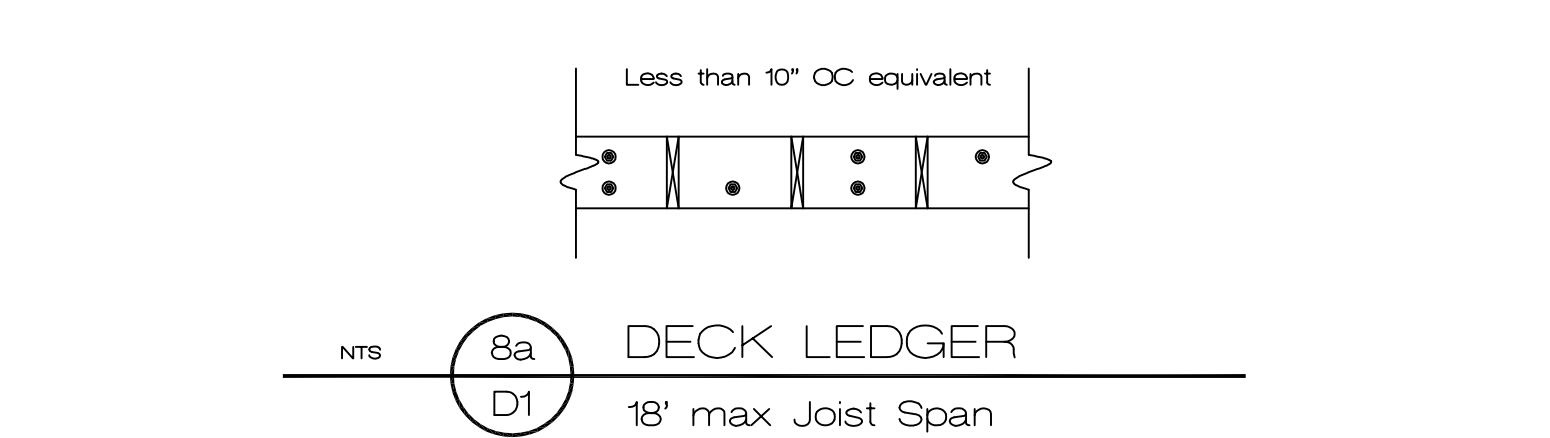
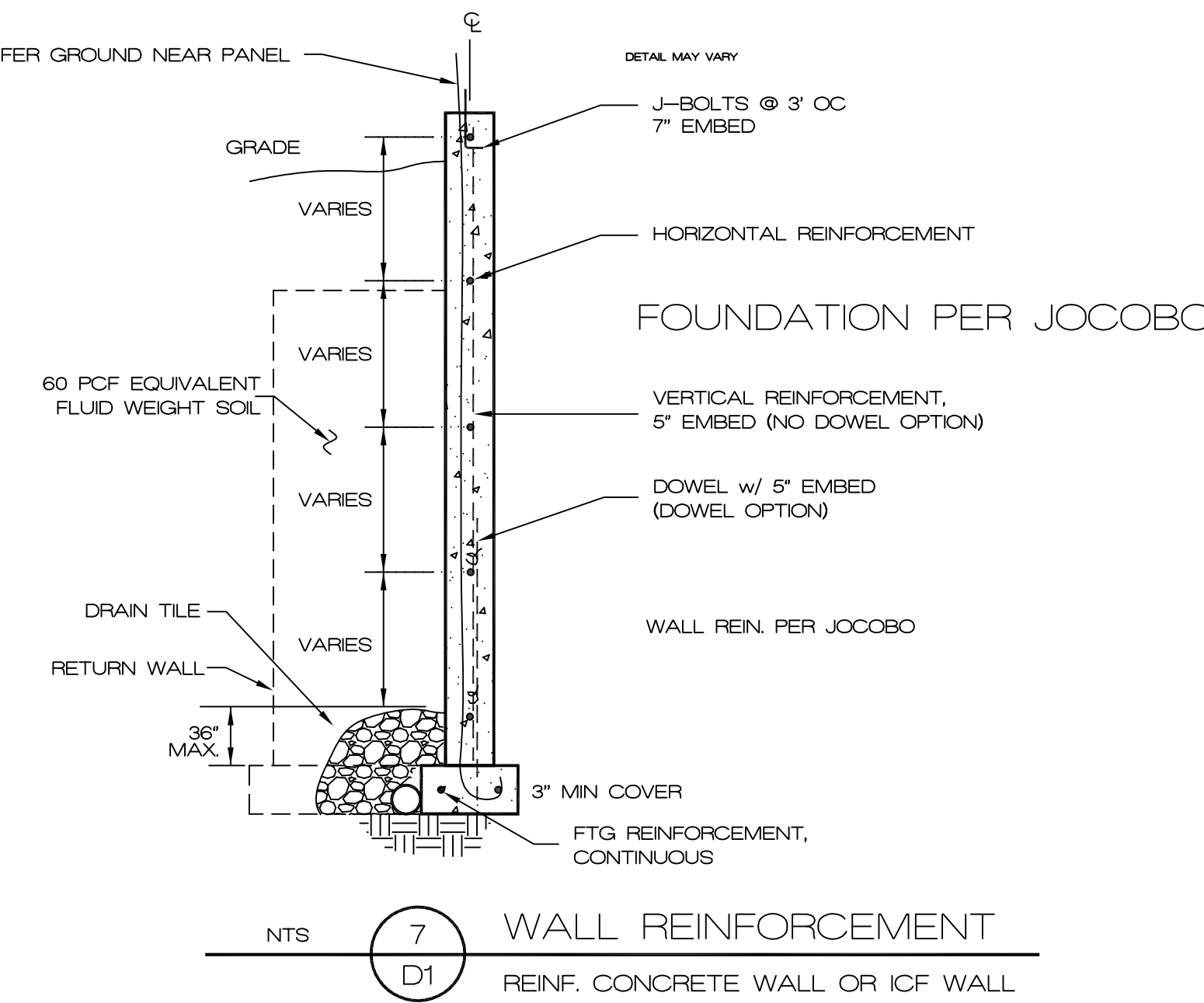
| 25 PSF SNOW LIVE LOAD |
|-------------------------------------|
| 10 PSF DEAD LOAD TOP CHORD (20 TIE) |
| 10 PSF DEAD LOAD BOTTOM CHORD |
 - SUBMIT SHOP DRAWINGS, INCLUDING DESIGN CALCULATIONS, MATERIAL STRESSES, GRADE AND SPECIES OF WOOD, AND PLACEMENT DRAWING.
- DEFAULT HEADER SIZE NOT SPECIFIED SPANNING 8'-0" MAX SHALL BE 2 – 2 X 10 #2, WITH 2 STUD SUPPORT.
- ALL HEADERS OVER 4'-0" SHALL HAVE DOUBLE TRIMMER @ EACH SUPPORT, OR AS SPECIFIED, UNO.
- SOLID BLOCKING BETWEEN JOISTS @ 36" OC FOR JOISTS PARALLEL TO THE EXTERIOR FOUNDATION WALL, MIN. 48" OR 3 JOIST SPACES.
- ALL FLUSH FRAMING @ HEADERS OR GIRDERS SHALL BE HANGERED.
- BLOCK BETWEEN JOISTS @ SUPPORTS OR OVER BEAMS.
- RATED CONSTRUCTION FOR PROJECTIONS INTO SETBACKS AS REQD.
- DOUBLE JOIST BELOW PARALLEL NONBEARING WALLS ON LAYOUT, SINGLE JOIST OFF LAYOUT. STRUCTURE BELOW LOAD-BEARING WALLS AS NOTED ON PLANS.

FOUNDATION PER JOCOBO RESIDENTIAL FOUNDATION GUIDELINE

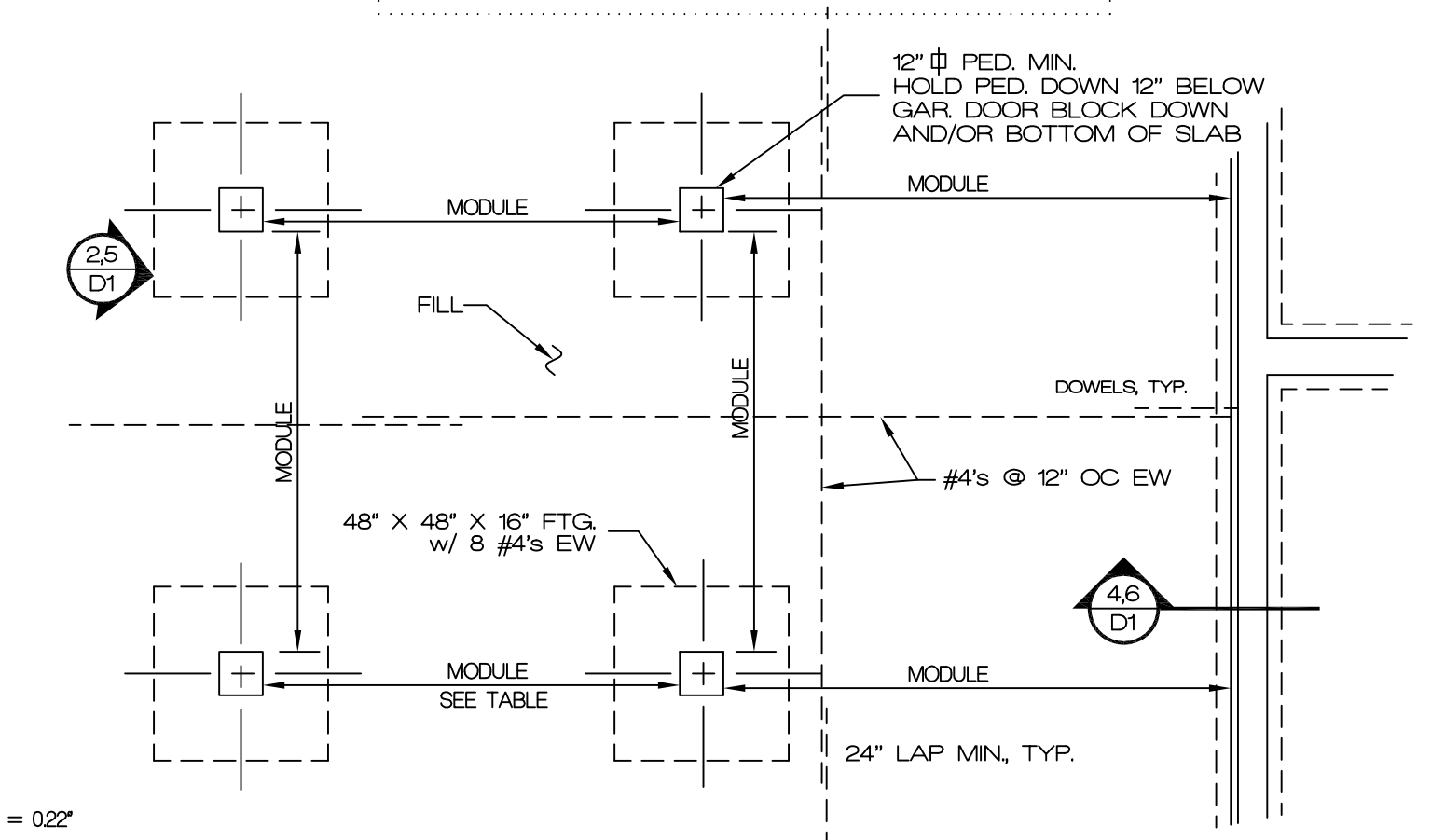
| WALL REINFORCING | | | | | | |
|-----------------------------------|------|------|---|-----------|------|------|
| 8" THICK | | | | 10" THICK | | |
| | 8' | 9' |  | 8' | 9' | 10' |
| 3000, GR40 | 16 | 12 | | 24 | 16 | 12 |
| 3500, GR40 | 16 | 12 | | 24 | 24 | 12 |
| 3000, GR60 | 24 | 16 | | 24 | 20 | 16 |
| 3500, GR60 | 24 | 16 | | 24 | 24 | 16 |
| HOR. REIN. MIN. GR40 #4 | | | | | | |
| One bar 12" from top & 24" oc max | 4 #4 | 5 #4 |  | 4 #4 | 5 #4 | 6 #4 |

Garage Slab: $M_{max} = \frac{W_u \cdot L^2}{14} \rightarrow 27,206 \text{ #-in}$
 $a = \frac{A_s \cdot f_y}{0.85 \cdot f'_c \cdot b} \rightarrow \frac{40,000 \cdot 0.2}{0.85 \cdot 3500 \cdot 12} = 0.22"$
 $\phi M_n = \phi A_s \cdot f_y \cdot (d - \frac{a}{2}) = 0.9(0.2)(40,000)(4 - 0.22/2) = 28,008 \text{ #-in} > 27,206 \text{ (OKAY)}$
 $\therefore \text{Use } \#4 \text{ @ } 12" \text{ OC EW } 15'-6" (+/-) \text{ MODULE}$

Basement Slab: $M_{max} = \frac{W_u \cdot L^2}{14} \rightarrow 25,951 \text{ #-in}$
 $a = \frac{A_s \cdot f_y}{0.85 \cdot f'_c \cdot b} \rightarrow \frac{40,000 \cdot 0.2}{0.85 \cdot 3500 \cdot 12} = 0.22"$
 $\phi M_n = \phi A_s \cdot f_y \cdot (d - \frac{a}{2}) = 0.9(0.2)(40,000)(4 - 0.22/2) = 28,008 \text{ #-in} > 25,951 \text{ (OKAY)}$
 $\therefore \text{Use } \#4 \text{ @ } 12" \text{ OC EW } 15'-6" (+/-) \text{ MODULE}$



| STRUCT. SLAB MODULE SPACING | |
|---------------------------------|----------------|
| SLAB TYPE | MODULE SPACING |
| BASEMENT | 15'-6" |
| GARAGE | 12'-6" |
| (MODULE ALSO APPLIES @ OVERDIG) | |

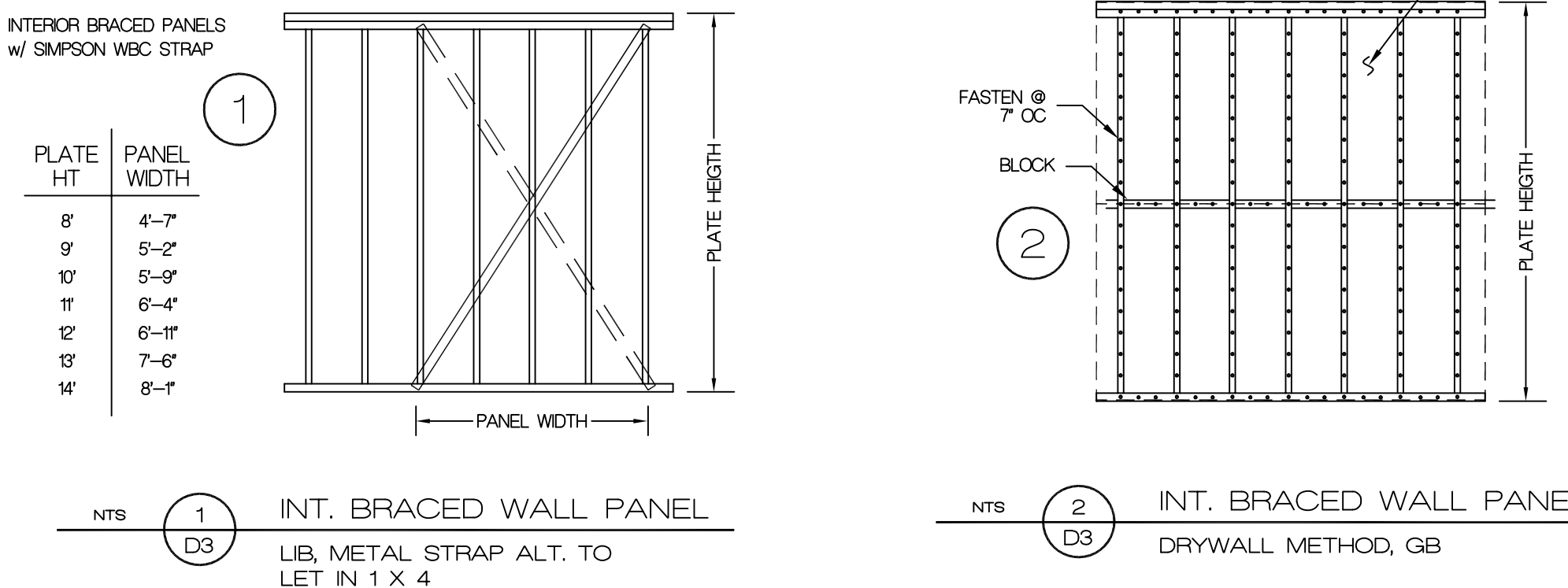
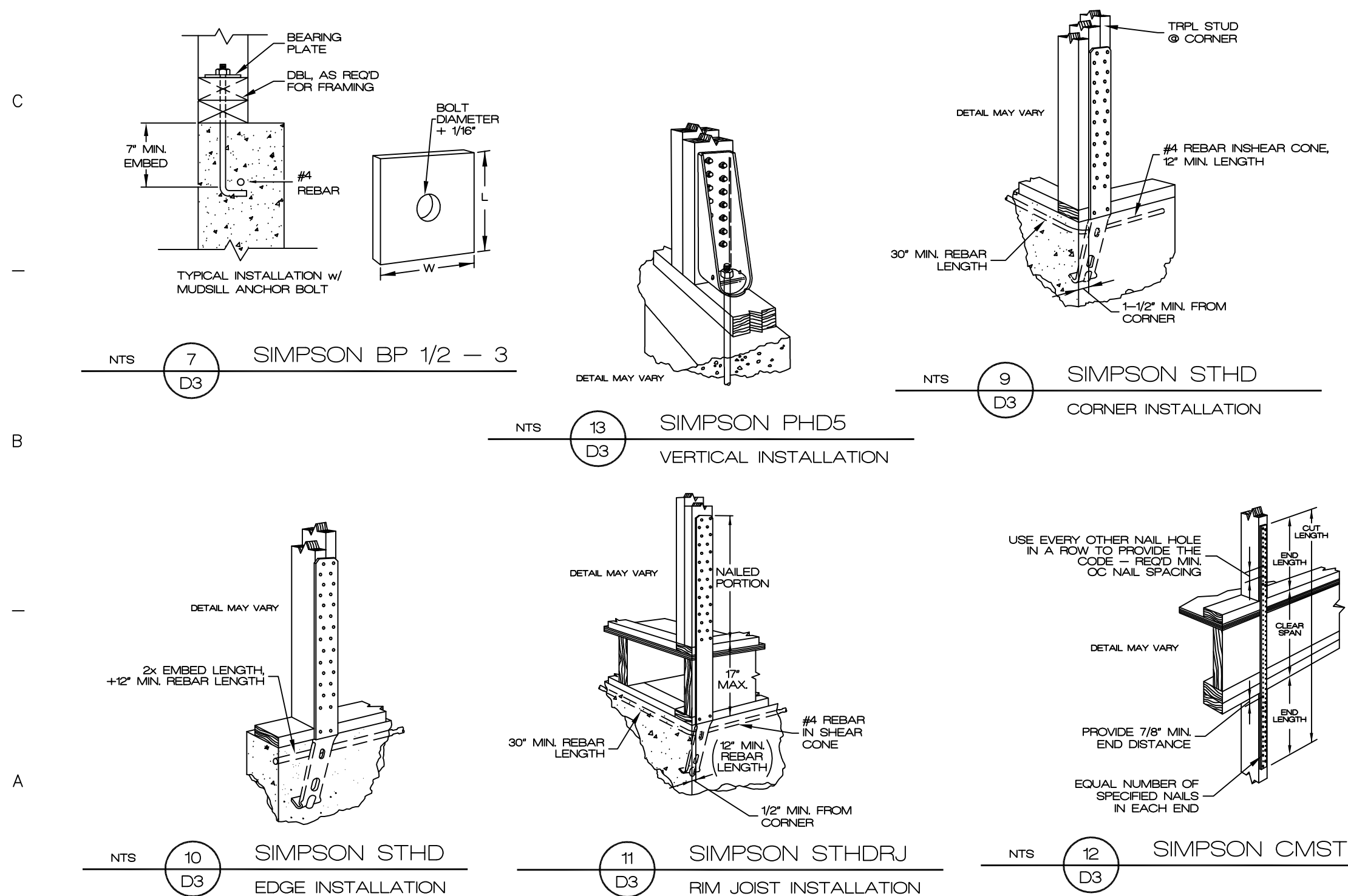


DO NOT SAW CUT STRUCTURAL SLABS W/O APPROVAL
VERIFY ALL STRUCTURAL SLAB DETAILS W/ ENGINEER
DO NOT ISOLATE COLUMNS FROM STRUCTURAL SLABS

FASTENER SCHEDULE FOR STRUCTURAL MEMBERS

| Item | Description of building elements | Number & type of fastener (notes: a, b, c) | Spacing of fasteners |
|---|---|--|---|
| Roof | | | |
| 1 | Blocking between joists or rafters to top plate, toe nail | 3-8d (2-1/2" x 0.137) | |
| 2 | Ceiling joists to plate, toe nail | 3-8d (2-1/2" x 0.137) | |
| 3 | Ceiling joists not attached to parallel rafter, laps over partitions, face nail | 3-10d | |
| 4 | Collar tie rafter, face nail or 1-1/4" x 20 ga. ridge strap | 3-10d (3" x 0.128) | |
| 5 | Rafter to plate, toe nail, note trusses use STC clips at NLB walls and speed holdowns | 3-16d or 3-10d (3-1/2" x 0.135, 0.148) | 2 toe nails side 1 1 toe nail side 2 (note j) |
| 6 | Roof rafters to ridge, valley or hip rafters: | | |
| Toe nail | | 4-16d (3-1/2" x 0.135) | |
| Face nail | | 3-16d (3-1/2" x 0.135) | |
| Wall | | | |
| 7 | Built-up studs-face nail | 10d (3" x 0.128) | 24" o.c. |
| 8 | Abutting studs at intersecting wall corners, face nail | 16d (3-1/2" x 0.135) | 12" o.c. |
| 9 | Built-up header, two pieces w/ 1/2" spacer | 16d (3-1/2" x 0.135) | 16" o.c. along each edge |
| 10 | Continued header, two pieces | 16d (3-1/2" x 0.135) | 16" o.c. along each edge |
| 11 | Continuous header to stud, toe nail | 4-8d (2-1/2" x 0.137) | |
| 12 | Double studs, face nail | 10d (3" x 0.128) | 24" o.c. |
| 13 | Double top plates, face nail | 10d (3" x 0.128) | 24" o.c. |
| 14 | Double top plates, min. 48" offset of end joints, face nail in lapped area | 8-16d (3-1/2" x 0.135) | |
| 15 | Sole plate to joist or blocking, face nail | 16d (3-1/2" x 0.135) | 16" o.c. |
| 16 | Sole plate to joist or blocking at braced wall panels | 3-16d (3-1/2" x 0.135) | |
| 17 | Stud to sole plate, toe nail | 3-8d (2-1/2" x 0.137) or 2-8d (2-1/2" x 0.137) | |
| 18 | Top or sole plate to stud, end nail | 2-16d (3-1/2" x 0.135) | |
| 19 | Top plates, laps at corners and intersections, face nail | 2-10d (3" x 0.128) | |
| 20 | 1" brace to each stud and plate, face nail | 2-8d (2-1/2" x 0.137) | |
| 21 | 1" x 6" sheathing to each bearing, face nail | 2-8d (2-1/2" x 0.137) | |
| 22 | 1" x 6" sheathing to each bearing, face nail | 2-8d (2-1/2" x 0.137) | |
| 23 | Wider than 1" x 6" sheathing to each bearing, face nail | 3-8d (2-1/2" x 0.137) | |
| Floor | | | |
| 24 | Joist to sill or girder, toe nail | 3-8d (2-1/2" x 0.137) | |
| 25 | Rm joist to top plate, toe nail (roof applications also) | 8d (2-1/2" x 0.137) | 6" o.c. |
| 26 | Rm joist or blocking to all plates, toe nail | 8d (2-1/2" x 0.137) | 6" o.c. |
| 27 | 1" x 6" subfloor or less to each joist, face nail | 2-8d (2-1/2" x 0.137) | |
| 28 | 2" subfloor to joist or girder, blind and face nail | 2 staples 1-3/4" | |
| 29 | 2" planks (plank & beam - floor and roof) | 2-16d (3-1/2" x 0.135) | |
| 30 | Built-up girders and beams, 2" lumber layers | 10d (3" x 0.128) | |
| 31 | Ledger strip supporting joists or rafters | 3-16d (3-1/2" x 0.135) | |
| Spacing of Fasteners | | | |
| Description of building materials | | Edges (inches) | Intermediate supports (inches) |
| Description of fastener (notes: b, c, e) | | | |
| Wood structural panels, subfloor, roof and interior wall sheathing to framing and particleboard wall sheathing to framing | | | |
| 32 | 3/8" to 1/2" | 8d common (2" x 0.113) nail (subfloor, wall) (note j) | 6 |
| 33 | 19/32" to 1" | 8d common (2-1/2" x 0.137) nail (roof) | 12 (note g) |
| 34 | 1-1/8" to 1-1/4" | 8d common (2" x 0.148) nail or 8d deformed (2-1/2" x 0.137) nail | 12 (note g) |
| Other wall sheathing (note h) | | | |
| 35 | 1/2" structural cellulose fiberboard sheathing | 1-1/2" galv. roofing nail, 7/16" crown or 1" crown staple 16 ga., 1-1/4" long | 6 |
| 36 | 25/32" structural cellulose fiberboard sheathing | 1-3/4" galv. roofing nail, 7/16" crown or 1" crown staple 16 ga., 1-1/2" long | 6 |
| 37 | 1/2" gypsum sheathing (note d) | 1-1/2" galvanized roofing nail, staple galv., 1-1/2" long 1-1/4" screws, Type W or S | 7 |
| 38 | 5/8" gypsum sheathing (note d) | 1-5/8" galvanized roofing nail, staple galv., 1-5/8" long 1-5/8" screws, Type W or S | 7 |
| Wood structural panels, combination subfloor underlayment to framing | | | |
| 39 | 3/4" and less | 8d deformed (2" x 0.120) nail or 8d common (2-1/2" x 0.137) nail | 6 |
| 40 | 7/8" to 1" | 8d common (2-1/2" x 0.137) nail or 8d deformed (2-1/2" x 0.137) nail | 12 |
| 41 | 1-1/8" to 1-1/4" | 10d common (2" x 0.148) nail or 8d deformed (2-1/2" x 0.137) nail | 12 |

- For S: 1 inch = 25.4 mm, 1 foot = 304.8 mm, 1 mile per hour = 0.447 m/s, 1 ksi = 68.95 MPa
- a. All nails are smooth-common, box or deformed shanks except where otherwise stated. Nails used for framing and sheathing connections shall have minimum average bending yield strengths as shown: 80 ksi (551 MPa) for shank diameter of 0.162 inch (20d common nail, 90 ksi (620 MPa) for shank diameters larger than 0.162 inch but not larger than 0.177 inch, and 100 ksi (689 MPa) for shank diameters of 0.162 inch or less.
- b. Staples are 16 gauge wire and have a minimum 7/16-inch on diameter crown width.
- c. Nails shall be spaced at not more than 6 inches on center at all supports where spans are 48 inches or greater.
- d. Four-foot-by-8-foot or 4-foot-by-9-foot panels shall be applied vertically.
- e. Spacing of fasteners not included in this table shall be verified w/ ECR.
- f. For regions having basic wind speed of 110 mph or greater, 8d deformed nails shall be used for attaching plywood and wood structural panel roof sheathing to framing within minimum 48-inch distance from gable and walls, if mean roof height is more than 25 feet, up to 35 feet maximum.
- g. For regions having basic wind speed of 100 mph or less, nails for attaching wood structural panel roof sheathing to gable and wall framing shall be spaced 6 inches on center. When basic wind speed is greater than 100 mph, nails for attaching panel roof sheathing to intermediate supports shall be spaced 6 inches on center for minimum 48-inch distance from ridge, eaves and gable and walls, and 4 inches on center to gable and wall framing.
- h. Gypsum sheathing shall conform to ASTM 1396 and shall be installed in accordance with GA 263. Fiberboard sheathing shall conform to ASTM C 208.
- i. Spacing of fasteners on floor sheathing panel edges applies to panel edges supported by framing members and at all floor perimeters only. Spacing of fasteners on roof sheathing panel edges applies to panel edges supported by framing members and at all roof plane perimeters. Blocking of roof or floor sheathing panel edges perpendicular to the framing members shall not be required except at intersection of adjacent roof planes. Floor and roof perimeter shall be supported by framing members or solid blocking.
- j. Where a rafter is fastened to an adjacent parallel ceiling joist in accordance with this schedule, provide two toe nails on one side of the rafter and toe nails from the ceiling joist to top plate in accordance with this schedule. The toe nail on the opposite side of the rafter shall not be required.



SHEAR WALL DESCRIPTION CONSTRUCTION

1 LIB METAL STRAP METHOD

SIMPSON CS16 STRAP NAILED TO STUDS SPACED AT 16" OC MAXIMUM. STRAPS SHALL BE INSTALLED IN 'V' OR 'X' PATTERN AT THE BRACE LOCATION AND FOR THE SPECIFIED LENGTH, ALTERNATIVE TO LET IN 1 X 4.

2 GB DRYWALL METHOD

1/2" MIN. GYPSUM BOARD OVER STUDS SPACED 24" OC MAXIMUM AND FASTENED AT 7" OC WITH 5d COOLER OR #6 BUGLE HEAD. HORIZONTAL JOINTS SHALL BE BLOCKED FOR ANCHORAGE.

3 WSP/CS-WSP SHEATHING METHOD

7/16" STRUCTURAL SHEATHING OVER STUDS SPACED 16" OC W/ 8d COMMON NAILS AT 4" OC EDGE AND 12" FIELD. HORIZONTAL JOINTS SHALL BE BLOCKED FOR ANCHORAGE.

4 PFH GARAGE DOOR PORTAL

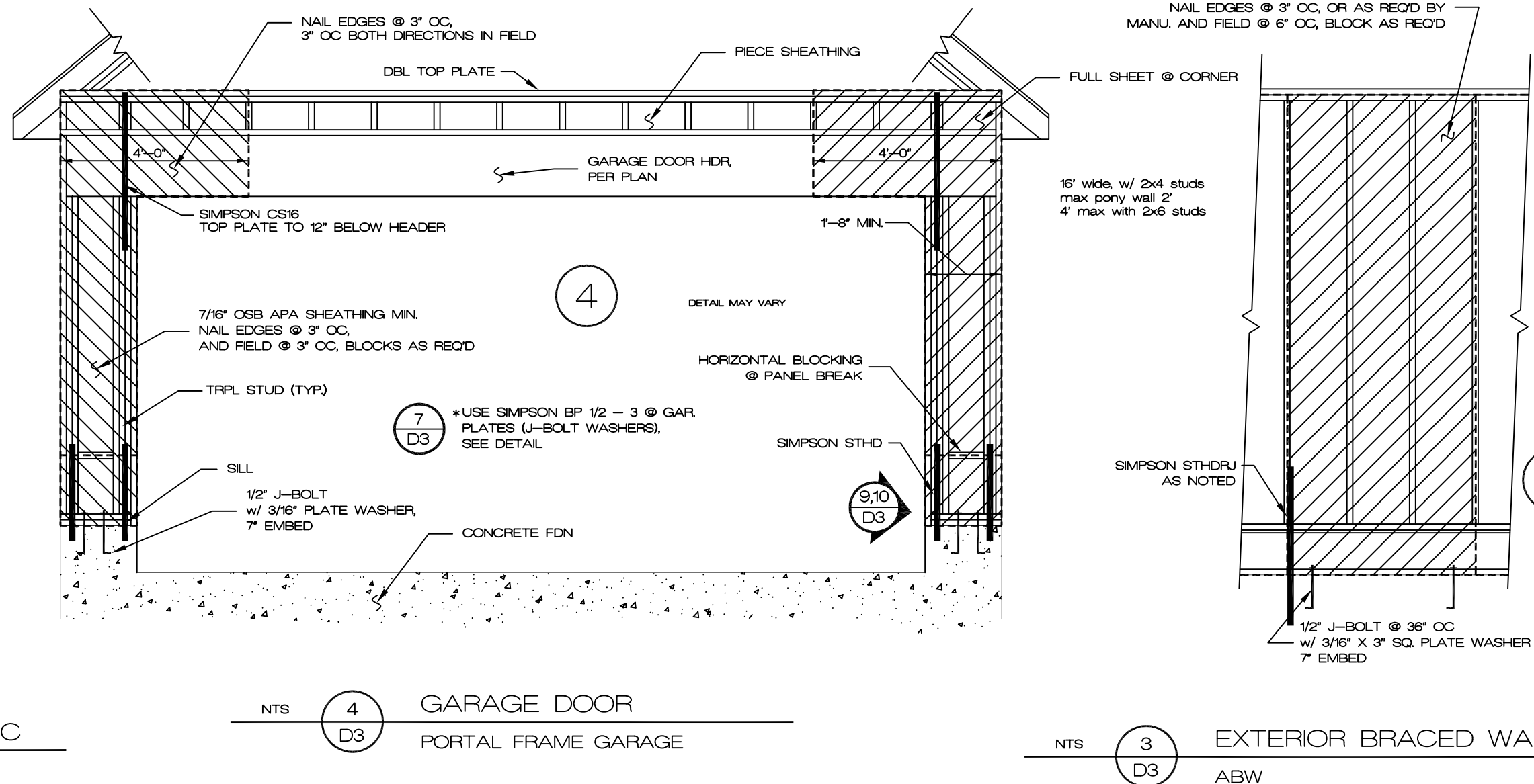
6 TO 1 ASPECT RATIO, HEADER LENGTH AS SPECIFIED WITH FULL PANEL SHEATHING AT UPPER CORNERS CUTOUT FOR THE OPENING. BLOCKING AT HORIZONTAL JOINTS. NOTE FULL 4" WIDTH CUTOUT PANELS REQ'D AT CORNERS. STHD10 & LSTA STRAPS

5 SINGLE STORY PORTAL

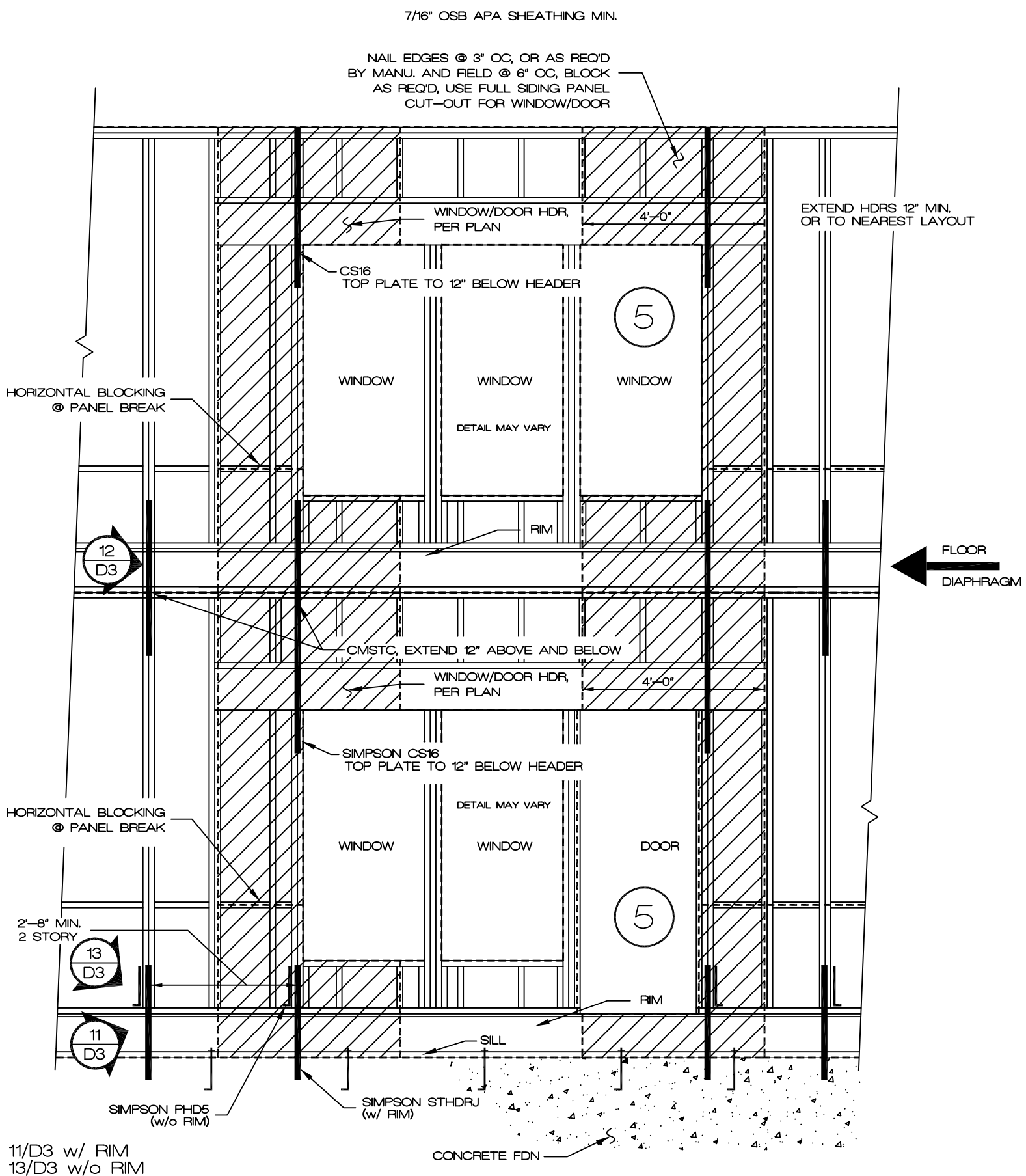
HEADER LENGTH AS SPECIFIED EXTENDED TO NEXT LAYOUT STUD, 18" MINIMUM WIDTH, 9" FULL PANEL SHEATHING REQ'D WITH CUTOUTS FOR OPENINGS. HORIZONTAL BLOCKING AT EDGES.

1' BOLT SPACING FOR SHEAR WALLS IS 3' OC WITH STRAPS AS NOTED.

8 NTS D3 SHEAR WALL SCHEDULE

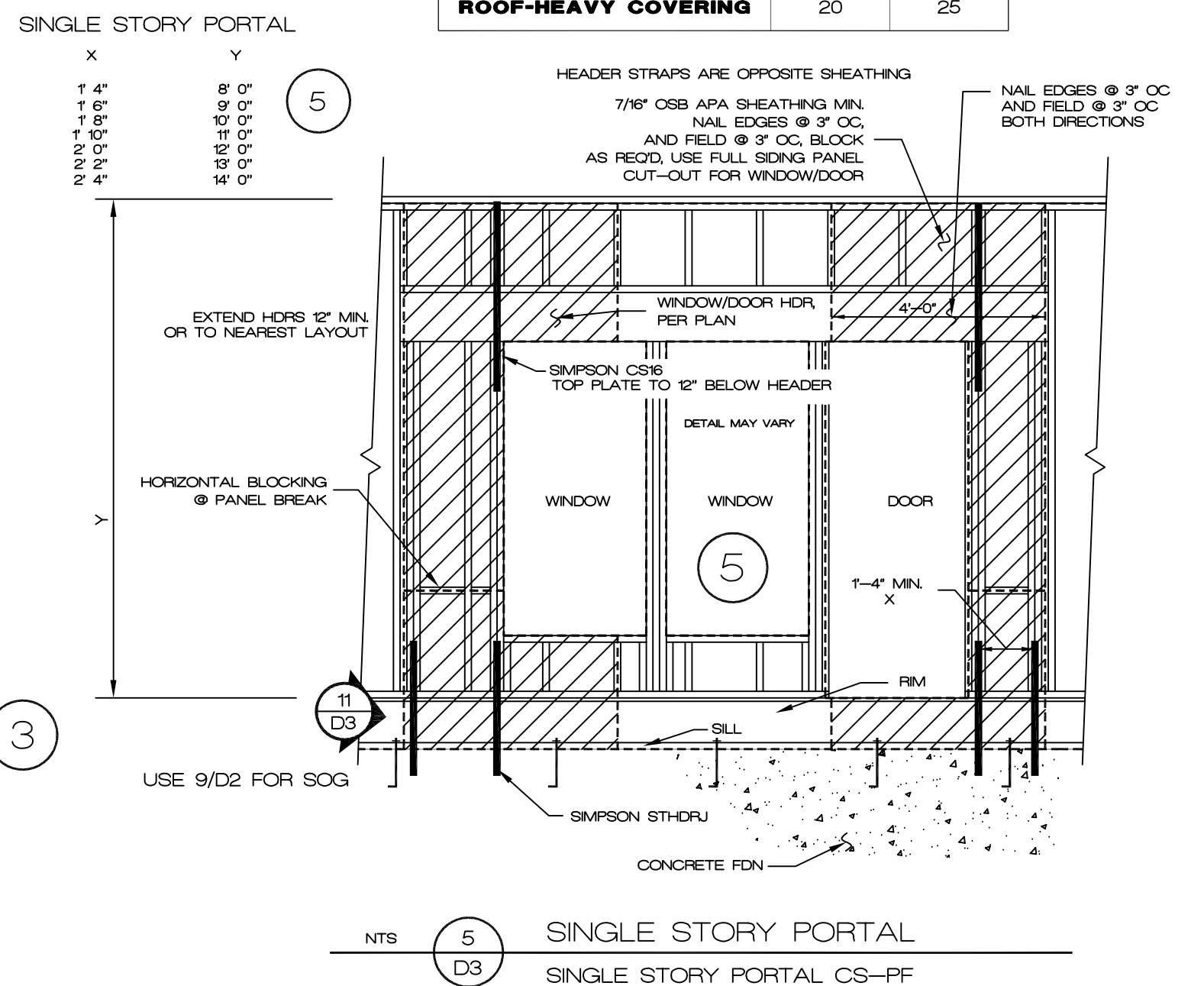


6 NTS D3 EXTERIOR BRACED WALL PANEL MULTI STORY PORTAL CS-PF



LOAD TABLE

| LOCATION | MIN. DL (PSF) | MIN. LL (PSF) |
|---------------------|---------------|---------------|
| EXTERIOR BALCONIES | 10 | 60 |
| DECKS | 10 | 40 |
| CEILING w/o STORAGE | 5 | 10 |
| CEILING w/ STORAGE | 10 | 20 |
| NON-SLEEPING ROOMS | 10 | 40 |
| SLEEPING ROOMS | 10 | 30 |
| ROOF-LIGHT COVERING | 10 | 25 |
| ROOF-HEAVY COVERING | 20 | 25 |



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2018 DETAIL SHEET



D3