All design and construction work for this project shall conform to the requirements of the 2018 International Building Code, as amended by the City of Lee's Summit, Missouri. Missou

4. These drawings are for this specific project and no other use is authorized.

A. All concrete for foundations (walls, grade beams, footings) shall develop minimum ultimate compressive design strength of 3500 psi in 28 days, but not less than 500 pounds of cement shall be used per cubic yard of concrete regardless of strengths obtained, not over 6 gallons of water per 100 pounds of cement and not over 4 inches of slump.

B. All concrete for interior flat work shall develop minimum ultimate compressive design strength of 4000 psi in 28 days, but not less than 525 pounds of cement shall be used per cubic yard of concrete regardless of strengths obtained, not over 5.75 gallons of water per 100 pounds of cement and not over 4 inches of slump. Concrete mix shop drawing shall contain testing data proving concrete design mix shrinkage is less than 0.034% at 28 days when tested according to ASTM C157 (air drying method only).

C. All concrete for exterior flatwork shall have a minimum design compressive strength of 4500 psi in 28 days, with not less than 560 pounds of cement per cubic yard of concrete, not over 5 gallons of water per 100 pounds of cement, with 6% +/- 1% air entrainment, and a maximum of 4 inches of slump. D. The preceding minimum mix requirements may have water-reducing admixtures conforming to ASTM C494 added to the mix at manufacturer's

dosage rates for improved workability. E. The preceding minimum mix requirements may have up to 15% maximum of the cement content replaced with an approved ASTM C618 Class C fly ash,

provided the total minimum cementitious content is not reduced. F. Combined aggregate (coarse plus fine) for all concrete shall be well graded from coarsest to finest with no more than 18 percent and not less than 8 percent retained on an individual sieve, except that less than 8 percent may be retained on coarsest sieve and on No. 50 and finer sieves. Submit this

gradation report with the concrete mix design shop drawings. G. All interior concrete slabs on grade shall be placed over 15 mil, Class A Vapor Barrier per ASTM E1745 with less than 0.01 perms, tested after mandatory conditioning. All joints shall be lapped and sealed per manufacturer's recommendations. All penetrations, as well as damaged vapor barrier material shall also be sealed per manufacturer's recommendation prior to concrete placement. Install barrier per manufacturer recommended details at all discontinuous edges (at interior columns, exterior edge of slab, etc.) to ensure terms of warranty are followed. The vapor barrier shall be placed over free-draining granular material as prescribed by

H. All concrete is reinforced concrete unless specifically called out as unreinforced. Reinforce all concrete not otherwise shown with same steel as in similar sections or areas. Any details not shown shall be detailed per ACI 315 and meet requirements of ACI 318, current editions.

I. Control joints in dirt formed slab to be as shown on plans. Where not shown, limit controlled areas to not more than 144 square feet, or 12 feet on any side. Slab panel side ratio shall not exceed 1 1/2 to 1. J. Contractor shall verify that all concrete inserts, reinforcing and embedded

items are correctly located and rigidly secured prior to concrete placement. K. Construction joints in beams, slabs, and grade beams shall occur at midspan (middle third) unless noted otherwise. Provide 2 x 4 horizontal keys at construction joints for shear transfer.

L. No aluminum items shall be embedded in any concrete.

Reinforcing Steel:

A. All reinforcing steel shall conform to the requirements of ASTM A615 or A706 grade 60 steel. Welded plain wire fabric shall be supplied in sheets and conform to the requirements of ASTM A185.

B. Clear minimum coverage of concrete over reinforcing steel shall be as follows: Concrete placed against earth Formed concrete against earth

Beams or Columns All coverage shall be nominal bar diameter minimum. C. All dowels shall be the same size and spacing as adjoining main bars (splice

lap 48 bar diameters or 24" minimum unless noted otherwise). D. At corners of all walls, beams, and grade beams supply corner bars (minimum 2'-0" in each direction or 48 bar diameters) in outside face of wall, matching size and spacing of horizontal bars. Where there are no vertical bars in outside face of wall, supply 3 - #4 vertical support bars for corner bars.

E. Bars marked continuous and all vertical steel shall be lapped 48 bar diameters (2'-0" minimum) at splices and embedments, unless shown otherwise. Splice top bars near midspan and splice bottom bars over supports, unless noted otherwise. F. At all holes in concrete walls and slabs, add 2 - #5 bars (opening dimension

plus 96 diameters long) at each of four sides and add 2 - #5 x 5'-0" diagonally at each of four corners of hole. Openings in 8" thick walls are reinforced similar, but with 1 - #5 instead of 2 - #5, respectively. G. Unless otherwise covered on architectural plans or specifications, vertical control joints in concrete wall shall be spaced at a maximum of 20'-0" on

center and coordinated with the architect. Every other horizontal wall reinforcing bar shall be discontinuous at control joints except heavy top and bottom bars unless noted otherwise. Provide base seal waterstop style number 772 (by Greenstreak Inc. or approved equal) on dirt face side of wall at all walls below grade.

H. Accessories shall be as specified in latest edition of the ACI Detailing Handbook and the concrete Reinforcing Steel Institute Design Handbook. Maximum accessory spacing shall be 4'-0" on center, and all accessories on exposed surfaces are to have plastic coated feet. I. All slabs and stairs not shown otherwise shall be 6" thick with #4 bars at 12"

on center each way. All exterior porches and stoops not otherwise detailed may be constructed in any standard manner, solid or hollow, but must be reinforced with #4 bars at 12" on center each way minimum. Porches shall be doweled to adjacent walls or grade beams with #4 bars at 12" on center, hooked or embedded 48 diameters into both members. Slope porches 1/8" per foot for drainage unless noted otherwise. J. Allow 1/2 ton of reinforcing bars #4 or larger to be used as directed in the

field for special conditions by the engineer of record (labor for placing same to be included).

7. Structural Steel:

A. All structural steel beams and columns shall be ASTM A992, grade 50 steel and all miscellaneous steel shall be ASTM A36 grade steel (except at moment connections where plates shall be ASTM A992, grade 50). Hollow Structural Sections (HSS) shall be ASTM A500, grade B. Fabrication and erection shall be in accordance with AISC 303-05 "Code of Standard Practice for Steel Buildings and Bridges" in the 13th Edition of the AISC Steel Construction Manual.

B. All welding shall conform to the recommendations of the AWS. C. All exterior steel and connections, and brick relief angles shall be hot-dip

D. All bolts not otherwise specified shall be 3/4" diameter high strength (ASTM A325-N). All bolts shall be fully pretensioned. All beam connections shall be designed per the AISC Manual of Steel Construction "Framed Beam Connections" for the indicated reactions or at least 0.4 x beam total shear capacity, Vn/Omega, shown in the Beam Properties of the maximum total uniform load tables, whichever is greater; and, shall account for eccentricity when the bolt line is more than 2" from the center of the support. All connections must be two bolt minimum. Connection design and shop drawing preparation shall be completed under the direct supervision of a professional engineer licensed in the state the project is located and shop drawings and connection calculations shall bear his seal.

E. All anchor bolts shall be 3/4" diameter, ASTM F1554, Grade 36 unless noted F. Allow 1 and 1/2 tons of miscellaneous structural steel to be used as directed in the field for special conditions by the structural engineer of record. Cost for

shop drawings, fabrication, delivery, detailing, and erection to be included.

Post-Installed Anchors:

A. Post-installed anchors shall be used only where specified on the drawings unless approved in writing by the engineer of record. See drawings for anchor diameter, spacing and embedment. Performance values of the anchors shall be obtained for specified products using appropriate design procedures and/or standards as required by the governing building code. Anchors installed in concrete shall have an ICC-ES Evaluation Service Report. Special inspection is required for all post-installed anchors. The contractor shall coordinate an on-site meeting with the post-installed anchor manufacturer field representative to educate the construction team on the anchor installation guidelines and requirements.

B. Mechanical anchors used in cracked and uncracked concrete shall have been tested and qualified for use in accordance with ACI 355.2 and ICC-ES AC193. All anchors shall be installed per the anchor manufacturer's written

C. Adhesive anchors used in cracked and uncracked concrete shall have been tested and qualified for use in accordance with ICC-ES AC308. All anchors shall be installed per the anchor manufacturer's written instructions. D. Mechanical anchors used in solid grouted masonry shall have been tested and qualified for use in accordance with ICC-ES AC01. All anchors shall be

installed per the anchor manufacturer's written instructions. E. Adhesive anchors used in solid grouted masonry shall have been tested and qualified for use in accordance with ICC-ES AC58. All anchors shall be installed per the anchor manufacturer's written instructions.

F. Anchors used in hollow concrete masonry shall have been tested and qualified in accordance with ICC-ES AC106 or ICC-ES AC58 as appropriate. All anchors shall be installed per the anchor manufacturer's written instructions with appropriate screen tubes used for adhesives.

A. The soil investigation was prepared by Olsson, Inc. The report number is 018-1450 and the telephone number is (913) 829-0078. B. Spread footings and grade beams are designed to bear on native soil or

engineered fill (placed in accordance with the recommendations of the geotechnical report) capable of safely sustaining 2,500 psf. C. Contractor shall provide for dewatering at excavations from either surface

D. All foundation excavations shall be inspected by a qualified soil engineer, approved by the architect and/or structural engineer, prior to placement of steel or concrete. This inspection shall be at the owner's expense.

E. All concrete in the structural portion retaining the backfill shall have attained

its design strength prior to being backfilled. F. Moisture content in soils beneath building locations should not be allowed to change after footing excavations and after grading for slabs on grade are completed. If subgrade materials become desiccated or softened by water or other conditions, recompact materials to the density and water content specified for engineered fill. Do not place concrete on frozen ground.

Concrete Block Masonry:

A. Concrete block used in exterior walls or load bearing walls shall meet the requirements of ASTM C90 and have a minimum net compressive strength of 1900 psi and laid up using type N mortar such that f'm equals 1500 psi. Mortar shall be volume proportion based cement lime mortar. Proportioning shall be completed by box measure. Any block in contact with earth shall be normal weight units, laid using type "S" mortar and grouted solid.

B. The contractor shall provide adequate temporary bracing for all masonry walls during construction. C. All concrete block shall have 9 gage (or larger) horizontal joint reinforcing (ladder or truss) per architectural drawings and specifications (16" maximum

vertical spacing). D. Cavity wall construction shall be reinforced as designed for specific concrete block used. The horizontal joint reinforcing shall be of the ladder or truss style per specification and continuous between brick and block, as prescribed

by architectural drawings. E. Concrete block shall be reinforced as follows in 8" walls: 1.) Vertical reinforcing (unless noted otherwise per schedule or details on the drawings) shall be a minimum of 1 - #5 bar in 8" walls at 32" on center, at each corner, at each door and window jamb, each side of control joints and in the end void of each length of wall. Lap splices for masonry vertical reinforcing shall be 48 bar diameters or 24" minimum.

2.) Horizontal reintorcing A.) Horizontal joint reinforcing as noted above. B.) Continuous horizontal bars shall be included per section or detail in bond

beam or optional running bond beam where noted. Where bond beams are continuous at corners of walls, supply corner bars matching size of horizontal bars (minimum 2'-0" or 40 bar diameters in each direction). F. Grout, where noted above, shall have a minimum design ultimate compressive strength of 2500 psi at 28 day test and 3/8" maximum

aggregate size. G. Non-load bearing concrete block walls shall be isolated from adjacent structural elements with vertical 3/8" control joints and at the top of the wall

with 1" air space or compressible material and support per architectural detail. H. Unless otherwise covered on architectural plans or specifications, vertical control joints in masonry construction shall be 3/8" wide, full height of wall. Joints shall be spaced at a maximum of 24'-0" on center and coordinated with the architect. All horizontal joint reinforcing shall be discontinuous at control joints in masonry. All bond beam horizontal reinforcing shall be continuous

I. Lintels over all openings in walls not otherwise covered shall be one 6" x 3-1/2" x 5/16" angle for each 4" of masonry. All exterior lintels to be galvanized.

Walls shall be anchored top and bottom by dowels matching wall vertical reinforcing (unless noted otherwise) from floor slab bottom and bracing angles at the top, per details on the drawings.

12. Timber and Wood Framing:

A. Quality and construction of wood framing members and their fasteners for load supporting purposes not otherwise indicated on the drawings shall be

in accordance with the 2018 International Building Code. B. All studs and top and bottom plates shall be Douglas Fir No. 2 grade visually graded lumber, with an allowable fiber stress in bending of 900 psi minimum and an elastic modulus of 1,600,000 psi unless noted otherwise. All joist, truss members and headers to be No. 2 grade (min.) (unless noted

C. Bridging of stud bearing walls and shear walls shall be solid, matching D. Joist blocking and bridging shall be solid wood or cross bridging of either wood or metal straps. Spacing, in any case, shall not exceed

E. Wood members and sheathing shall be fastened with number and size of fasteners not less than that set forth in Table 2304,10.1 of the 2018 International Building Code. Floor sheathing shall be APA rated tongue and groove Sturd-I-Floor, exposure 1, glued and nailed with 10d nails or # 10 screws at 6" on center to supports at edges and 12" on center field. Sheathing of shear walls or roof diaphragms shall be edge nailed with 8d common nails at 6" on center and nailed to intermediate framing and/or blocking members with 8d common nails at 12" on center unless otherwise

noted on the drawings. F. Sill plates shall be bolted to concrete slabs with Đ" diameter bolts at 32" on center (UNO, Re: shearwall sched). Provide plate washers at sill plate anchors for shearwalls per shearwall sched. Plates in direct contact

with concrete or masonry shall be treated lumber G. All hangers, ties and connections shown are based on Simpson Strong Tie as the basis of design. Provide Simpson Strong Tie or an approved equal. Joist hangers shall be equal to "LUS" for wood application and "LB" for steel weld-on application. Roof truss ties shall be equal to "H2.5A" and tie the roof truss to the top plate (provide (2) "H2.5A" Diagonally across from each other when uplift load shown in truss shop submittal exceeds 600lbs). Roof girder ties shall be equal to a "LGT2", "LGT3" or "LGT4" tie (dependent on number of plies) and tie the truss girder to the top plate. Provide "H4" at the top of each stud to top track when the top track has roof truss attached.

H. Service condition - dry with moisture content at or below 19% in service. I. Laminated strand lumber (LSL) shall have an allowable flexural stress (Fb) of 1,700 psi (reduced by size factor) and an elastic modulus (E) of 1,300,000 psi J. Laminated veneer lumber (LVL) shall have an allowable flexural stress (Fb)

of 2,600 psi (reduced by size factor) and an elastic modulus (E) of 1,900,000 K. Parallel Strand Lumber (PSL) shall have an allowable flexural stress (Fb) of 2,900 psi (reduced by size factor) and an elastic modulus (E) of 2,000,000

psi. ((E) = 2,200,000 psi for members > 18") Pre-engineered wood trusses shall be designed in accordance with the Truss Plate Institute's national design standard for metal-plate connected wood truss construction (ANSI/TPI-1 latest edition). Trusses shall be designed and manufactured by an authorized member of the Wood Truss Council of America (WTCA). Truss design shall conform to specified codes,

allowable stress increases, deflection limitations and other applicable criteria of the governing code. M. Shop drawings showing complete erection and fabrication details and calculations (including connections) shall be submitted to the project architect / engineer for review prior to fabrication and/or erection. Calculations shall bear the seal of a professional engineer, registered in the state of the project location. Shop drawings shall also be submitted to the local government controlling agency when requested by that

N. All trusses shall be securely braced both during erection and permanently, as indicated on the approved truss design drawings and in accordance with TPI's commentary and recommendations for handling, installing and bracing metal-plate connected wood trusses (HIB-91, booklet) and the latest edition of ANSI/TPI-1.

O. The truss manufacturer shall supply all hardware and fasteners for joining truss members together and fastening truss members to their supports. Metal connector plates shall be manufactured by a member of the Wood Truss Council of America (WTCA) and shall be 20 gauge minimum. Connector plates shall meet or exceed ASTM A653, grade 33, with ASTM A924 galvanized coating designation G60.

P. Shipment, handling, and erection of trusses shall be by experienced, qualified persons and shall be performed in a manner so as not to endanger life or property. Apparent truss damage shall be reported to the truss manufacturer for evaluation prior to erection. Cutting or alteration of trusses

Q. Pre-engineered floor truss design load and deflection criteria are as follows: Top Chord Dead Load= 32 psf Top Chord Live Load= 40 psf (private)

100 pst (public Bottom Chord Dead Load= 10psf Allowable Total Load Deflection= L/360 Allowable Live Load Deflection= L/480; 1/2" maximum R. Pre-engineered roof truss design load and deflection criteria are as follows:

Top Chord Dead Load= 15 psf Top Chord Live Load (Typical) = 20 psf plus snow drift Top Chord Live Load (at Recessed Mechanical Wells) = 100 psf to account for mechanical equipment plus snow drift. Refer to roof framing Bottom Chord Dead Load= 10 psf Include mechanical equipment loads as required (coordinate locations and sizes with MEP)

Allowable Live Load Deflection= L/360 S. Wood Shrinkage Considerations:

Allowable Total Load Deflection= L/300

(General Contractor to coordinate with all trades required) 1.) All holes and notches for horizontal plumbing pipes are to be

oversized to compensate for shrinkage. 2.) Swing joints and flexible connections, offsets and expansion/ contraction joints are to be utilized in the fabrication of pipes to allow for shrinkage.

3.) Vents are to be installed with double flashing to permit 4.) Hangers for piping below upper floor are required to be adjusted several months after completion of construction. 5.) Slip joints are required for all sheet metal vertical down-

spouts, vents, etc. to compensate for shrinkage Rigid electrical conduit installed vertically should be provided with flexible joints to permit movement.

7.) All roof drains are to be adjusted to the finished roof surface at the time of occupancy and also every year prior to rainy

8.) Vertical mechanical and sprinkler systems are to be installed to compensate for wood shrinkage. 9.) Plates should be fastened tight to precut studs to reduce compressive space between plate and stud to minimize any potential additional shortening of building walls.

10.) All wood structural panels on walls are required to have a 1/2"

relief gap at each floor level to relieve possible bulging. 11.) At stucco construction install horizontal expansion joints, slip joint flashing, etc. 12.) At brick veneer construction provide slip joints for flashing. Refer to architect's plans for flashing and clearances required

between brick and wood structure at horizontal locations to compensate for wood shrinkage. 13.) Delay window and door installation to allow wood framing to reach equilibrium moisture content (EMC). Also, allow 1/2" gap

gap at window sills and a gap around pre-hung doors. 14.) Refer to the "ESTIMATED WOOD SHRINKAGE" table on this sheet. The application of all finish materials and installation of non-structural systems shall account for shrinkage of the wood framing per the estimated values given. T. Construction bracing shall be provided by the contractor as required to keep

the building and studs plumb. U. Structural members shall not be cut for pipes, etc., unless specifically detailed. Notching and boring of studs and top of plates shall conform to the provisions of section 2308.9.10 and 2308.9.11 of the IBC. Where top plates or sole plates are cut for pipes, a metal tension tie with minimum 0.058 inches thick and 1 1/2" inches wide shall be fastened to each plate across and to each side of the opening with not less than (6) 16d nails, in accordance

section 2308.9.8 of the IBC. V. All fasteners for wood to wood connections and wood connectors shall be as indicated in structural drawings or manufacturer literature to achieve full capacity of connector. Alternate fasteners may be submitted as a substitution request. Submittal must show that alternative fasteners will not reduce the capacity of the connection.

13. Shop Drawing Review:

A. Bob D. Campbell and Company, Inc. will review the General Contractor's (GC) shop drawings and related submittals (as indicated below) with respect to the ability of the detailed work, when complete, to be a properly functioning integral element of the overall structural system designed by Bob D. Campbell and Company, Inc.

B. Prior to submittal of a shop drawing or any related material to Bob D. Campbell and Company, Inc., the GC shall: Review each submission for conformance with the means, methods, techniques, sequences and operations of construction and safety precautions and programs incidental thereto, all of which are the sole

Review and approve each submission Stamp each submission as approved. C. Bob D. Campbell and Company, Inc. shall assume that no submission comprises a variation unless the GC advises Bob D. Campbell and Company, Inc. with written documentation.

compounds applied to the concrete after placement.

responsibility of the GC.

D. Shop drawings and related material (if any) required are indicated below. Should Bob D. Campbell and Company, Inc. require more than ten (10) working days to perform the review, Bob D. Campbell and Company, Inc. shall so notify the GC. 1) Concrete mix designs and material certificates including admixtures and

2) Reinforcing steel shop drawings including erection drawings and bending details.Bar list will not be reviewed for correct quantities 3) Elevations of all reinforced concrete masonry walls at a scale no smaller than 3/8" = 1'-0" showing all required reinforcing.

4) Grout mix designs (for CMU).) Construction and control joint plans and/or elevations. 6) Structural steel shop drawings including erection drawings and piece details. Include miscellaneous framing specified on the structural drawings, but do not submit framing specified on non-structural drawings for Bob D. Campbell and Company, Inc. review.

7) Structural steel connection design calculations. 8) Miscellaneous anchors shown on the structural drawings

9) Wood truss design calculations and detailed erection and fabrication drawings. Standard stick framing shop drawings need not be submitted. Bob D. Campbell and Company, Inc. shall review shop drawings and related materials with comments provided that each submission has met the above requirements. Bob D. Campbell and Company, Inc. shall return without

Structural Special Inspection:

A. The structural design for this project is based on completion of special spections during construction in accordance with section 1704 of the 2018

comment unrequired material or submissions without GC approval stamp.

nternational Building Code. The owner shall employ one or more qualified special inspections to provide the required special inspections. B. Special Inspections shall be required for the items indicated below. The General Contractor shall provide notification to the inspector when items requiring inspection are ready to be inspected and provide access for those

1) Placement of Concrete 2) Testing of Concrete

Bolts in Concrete 5) Placement of Reinforcing Steel Verification of Soil Bearing Capacities

High Strength Bolting 8) Post-Installed Anchors 9) Structural Welding

10) Steel Frame Inspection 11) Structural Masonry 12) Shop Fabrication of Structural Steel 13) Wood shear walls and holdowns

14) Wood gravity framing and placement C. The special inspector shall furnish inspection reports to the building official, owner, architect and structural engineer, and any other designated person. D. All discrepancies shall be brought to the immediate attention of the contractor for correction, then, if uncorrected, to the proper design authority, building

official and structural engineer. E. The special inspector shall submit a final signed report stating that the work requiring special inspection was, to the best of the inspector's knowledge, in conformance with the approved plans and specifications and the applicable workmanship provisions of the building code.

15. Copyright and Disclaimer:

A. All drawings in the structural set (S-series drawings) are the copyrighted work of Bob D. Campbell and company, Inc. These drawings may not be photographed, traced, or copies in any manner without the written permission of Bob D. Campbell and Company, Inc. Exception: Original drawings may be printed for distribution to the owner, architect, and general contractor for coordination, bidding, and construction. Subcontractors may not reproduce these drawings for any purpose or in any manner.

B. I, Christopher W. Boos, P.E., registered engineer and a representative of Bob D. Campbell and Company, Inc., do hereby accept professional responsibility as required by the professional registration laws of this state for the structural design drawings consisting of S-series drawings. I hereby disclaim responsibility for all other drawings in the construction document package, they being the responsibility of other design professionals whose seals and signed statements may appear elsewhere in the construction document

PROVIDE CORNER

CONTINUOUS BOND

BEAM REINFORCING

BARS TO MATCH

STRUCTURAL DESIGN CRITERIA: $\sim\sim\sim\sim\sim\sim$ GOVERNING CODE 2018 International Building Code minum of the same 100 psf • Floors (slab on grade) Private Rooms & Corridors Serving Them 40 psf Public Rooms & Corridors Serving Them 100 psf SNOW LOADING: Ground Snow Load Pg = 20 psf Flat Roof Snow Load Pf = 14 psf Snow Exposure Factor Ce = 1.0 Snow Load Importance Factor ls = 1.0 Thermal Factor Ct = 1.0 Drift per ASCE/SEI 7-16 <u>WIND LOADING:</u> Main Wind-force Resisting System (MWFRS): • Ultimate Design Wind Speed Vult = 115 mph Nominal Design Wind Speed Vasd = 89 mph Risk Category Wind Load Importance Factor lw = 1.0 Wind Exposure Category Internal Pressure Coefficient (Enclosed) GCpi = +/-0.18Components & Cladding: Design wind pressures to be used for the design of exterior component and cladding materials on the designated zones of wall and roof surfaces shall be per ASCE/SEI 7-16. Tabulated

SEISMIC DESIGN REQUIREMENTS:

where applicable.

 Risk Category II • Seismic Importance Factor Is = 1.0 Spectral Response Acceleration Parameters: Sds = 0.122g

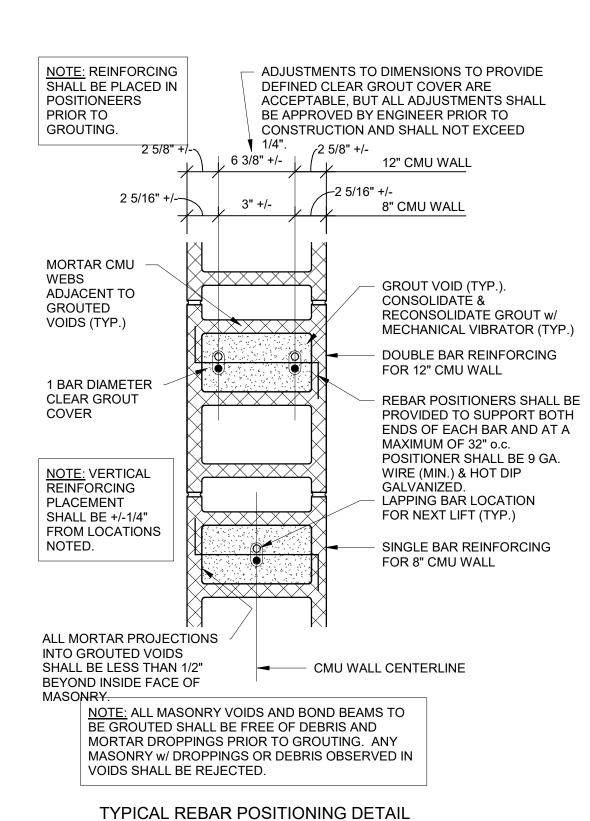
pressures shall be multiplied by effective area reduction

factors, exposure adjustment factors, and topographic factors

Sd1 = 0.107gSite Class Seismic Design Category

ESTIMATED WOOD SHRINKAGE				
LEVEL	SHRINKAGE AT LEVEL	CUMULATIVE SHRINKAGE		
ROOF	0.40"	1.30"		
3RD FLOOR	0.42"	0.90"		
2ND FLOOR	0.48"	0.48"		

1. REFER TO GENERAL NOTES FOR WOOD SHRINKAGE CONSIDERATIONS. THE APPLICATION OF ALL FINISH MATERIALS AND INSTALLATION OF NON-STRUCTURAL SYSTEMS SHALL ACCOUNT FOR SHRINKAGE OF THE WOOD FRAMING PER THE ESTIMATED VALUES GIVEN.



B SECTION

COLUMN DIMENSIONAL RANGE 16" MIN. TO 40" MAX. ALL VOIDS IN (2) TYPICAL VERTICAL **COLUMN SHALL** BARS PER VOID (FULL HEIGHT OF WALL)

LEE'S SUMMIT, MISSOURI HORIZONTAL BOND BEAM VERT. WALL REINFORCING CONTINUOUS CONTROL JT. THROUGH CONTROL JOINTS (FULL HEIGHT) TOP OF WALL ROOF OR FLOOR LINE WHERE INDICATED S0.1 TYPICAL CMU WALL REINFORCING AT OPENINGS

A CMU WALL ELEVATION

1 FULL HEIGHT VERTICAL BARS AS JAMB REINFORCING IN FIRST 2 CELLS ADJACENT TO OPENING. REINFORCE $\overset{ extstyle op}{ o}$ EACH CELL WITH SIZE & QUANTITY OF BAR TO MATCH WALL REINFORCING (1 BAR TYPICAL IN 8" WALLS AND 2

BARS TYPICAL IN 12" WALLS). 2 | LINTEL REINFORCING PER SECTION C. EXTEND 2'-0" PAST EDGE OF OPENING ON EACH SIDE (TYPICAL)

3 2-#5 CONTINUOUS HORIZONTAL BARS AS SILL REINFORCING IN 8" COURSE BELOW OPENING (U.N.O.). EXTEND 2'-0" PAST EDGE OF OPENING ON EACH SIDE (TYPICAL).

 $\lceil_{A}\rceil$ FULL HEIGHT VERTICAL BARS PER MASONRY VERTICAL REINFORCING SCHEDULE LOCATED IN END CELL AT EACH SIDE OF VERTICAL WALL CONTROL JOINTS.

GENERAL CRITERIA: (SECTION A CONTINUED):

1. VERTICAL REINFORCING BARS SHALL BE DOWELED TO FOUNDATION WITH A DOWEL OF MATCHING SIZE

2. CONTRACTOR SHALL COORDINATE AND VERIFY OPENINGS IN MASONRY WALLS. OPENINGS SHALL BE

DETAILED ON REINFORCING STEEL SHOPE DRAWING ELEVATIONS. 3. VERTICAL CONTROL JOINTS IN MASONRY WALLS SHALL BE 3/8" WIDE, FULL HEIGHT OF WALL. JOINTS SHALL BE SPACED AT A MAXIMUM OF 24'-0" ON CENTER AND NOT LESS THAN 2'-0" FROM THE EDGE OF ANY OPENING. ALL HORIZONTAL JOINT REINFORCING SHALL BE DISCONTINUOUS AT CONTROL JOINTS. ALL BOND BEAM HORIZONTAL REINFORCING SHALL BE CONTINUOUS THROUGH CONTROL JOINTS. CONTRACTOR SHALL COORDINATE AND VERIFY ALL CONTROL JOINT LOCATIONS

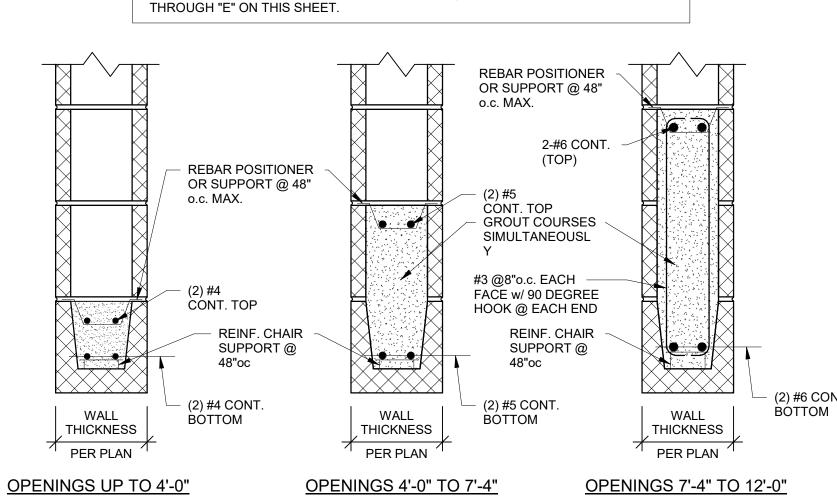
MASONRY VERTICAL REINFORCING SCHEDULE FOR LOAD BEARING MASONRY (CMU) WALLS WALL THICKNESS LOCATION VERTICAL REINF. SPACING
(IN GROUTED CELLS)
8" ELEVATOR 1-#5 32"oc
8" TRASH ENCLOSURE 1-#5 16"oc

1. IN ADDITION TO SPACING SHOWN IN SCHEDULE, VERTICAL REINFORCING SHALL BE PROVIDED IN GROUTED CELLS AT THE FOLLOWING LOCATIONS

A.) IN THE FIRST 2 CELLS ADJACENT TO EACH OPENING B.) IN THE END CELLS ON EACH SIDE OF VERTICAL CONTROL JOINTS C.) IN THE END CELLS OF EACH LENGTH OF WALL D.) AT EACH CORNER OF WALLS

ALL MASONRY VOIDS AND BOND BEAMS TO BE GROUTED SHALL BE FREE OF DEBRIS AND MORTAR DROPPINGS PRIOR TO GROUTING. ANY MASONRY W DROPPINGS OR DEBRIS OBSERVED IN VOIDS SHALL BE REJECTED.

> TYPICAL MASONRY REINFORCING NOTE: ALL INTERIOR & EXTERIOR MASONRY WALLS SHOWN ON ARCHITECTURAL AND STRUCTURAL DRAWINGS ARE TO BE REINFORCED HORIZONTALLY WITH BOND BEAMS (2 - #5 BOTTOM) AT BOTTOM COURSE, TOP COURSE, JOIST BEARING ELEVATION AND AT 8'-0" MAXIMUM O.C. AND VERTICALLY AS INDICATED ON DRAWINGS. THESE WALLS ARE TO BE ANCHORED TOP AND BOTTOM TO THE FOUNDATION, FLOOR, OR ROOF PER TYPICAL DETAILS. THE VERTICAL REINFORCING IS CONTINUOUS (IN 6'-6" MAXIMUM LENGTHS, LAPPED 2'-6" MINIMUM) FILL BLOCK CELLS AND BOND BEAMS WITH 2,500psi GROUT. RE: DETAILS "A"



C SECTION

TYPICAL LINTELS AT ALL CMU WALLS (U.N.O.)

#2 TIES @8"oc THROUGH COLUMN HEIGHT PLUS 2'-0" ABOVE AND BELOW OPENING. TIES SHALL BE FABRICATED TO MAINTAIN A SINGLE LAYER OF TIE REINFORCING WITHIN THE HORIZONTAL MORTAR JOINT. CUT WEBS OF

BLOCK AS REQUIRED TO

RECEIVE TIES WHERE CONFLICTS OCCUR.

E SECTION

COMMUNIT $\overline{\mathcal{C}}$ SUMMIT S

EST 1935

ARCHITECTURAL CORPORATION

MISSOURI CERTIFICATE

OF AUTHORITY NO. 000073

RELEASE FOR CONSTRUCTION AS NOTED ON PLANS REVIEW

> **ENGINEER -CHRISTOPHER W. BOOS** MO LICENSE # PE-2008023332

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STRUCTURAL GENERAL NOTES

ISSUE DATE: REVISIONS: 1 Revision#1 7-26-19

PROJECT NO.:1811

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D DETAIL

TOP BOND BEAM

(REINFORCING NOT

- SAW CUT OR

SPECIAL BLOCK

OR K.O. BLOCK

SHOWN FOR

CLARITY)

"KNOCKOUT " (K.O.) or TROUGH BOND

BEAM BLOCK (TYPICAL UNIT EXCEPT @

STOP (RE: SPECS.) UNDER K.O. BOND

BE REINFORCED AND GROUTED.

DOOR OPENINGS; SOLID BOTTOM BOND

BEAM SHALL BE USED). PROVIDE GROUT

BEAMS OVER CELLS WHICH ARE NOT TO

TYPICAL BOND BEAM DETAIL AT CORNER OF CMU WALL

BE GROUTED TYPICAL MASONRY COLUMN SOLID

MANUFACTURER & OBSERVED BY SPECIAL INSPECTOR. SHEAR WALL HOLDOWN ANCHOR DETAIL RE: FOUNDATION PLAN & SCHEDULE FOR LOCATIONS (AT EACH END OF SHEAR WALL)

QUANTITY OF SHEAR WALL

CHORD STUDS PER SHEAR

FASTENERS PER SIMPSON

- ASTM A36 GALV. THREADED

w/ 16" MIN. EMBEDMENT IN

CONCRETE FOUNDATION.

INSTALLATION SHALL BE IN

ACCORDANCE w/ EPOXY

ROD ANCHOR. DRILL & EPOXY

WALL SCHEDULE

SHEAR WALL SCHEDULE - MEMORY CARE

3rd FLOOR

SHEAR WALL SCHEDULE - ASSISTED LIVING

SHEAR WALL SCHEDULE - INDEPENDENT LIVING

WALL SHEATHING

7/16" OSB ONE SIDE:

8d NAILS @6"oc AT

PANEL EDGES,

INTERMEDIATE

7/16" OSB ONE SIDE: 8d NAILS @6"oc AT

PANEL EDGES,

INTERMEDIATE

INTERMEDIATE

FRAMING

@12"oc TO

FRAMING

@12"oc TO

FRAMING

WALL SHEATHING

2nd FLOOR

SHEATHING & ATTACHMENT

SHEATHING & ATTACHMENT

WALL SHEATHING

7/16" OSB ONE SIDE:

7/16" OSB ONE SIDE:

8d NAILS @6"oc AT

PANEL EDGES,

INTERMEDIATE

5/8" GYP. BD ONE 5/8" GYP. BD ONE 5/8" GYP. BD ONE

7"oc TO INTERMEDIATE

FRAMING

SIDE: 6d NAILS @4"oc | SIDE: 6d NAILS @4"oc | SIDE: 6d NAILS @4"oc

AT PANEL EDGES, @ | AT PANEL EDGES, @ | AT PANEL EDGES, @ |

@12"oc TO

FRAMING

8d NAILS @6"oc AT

PANEL EDGES,

INTERMEDIATE

@12"oc TO

FRAMING

2nd FLOOR

(PER PLAN) | WALL SHEATHING | WALL SHEATHING

7/16" OSB ONE SIDE:

8d NAILS @4"oc AT

7/16" OSB ONE SIDE:

8d NAILS @4"oc AT

5/8" GYP. BD ONE

INTERMEDIATE

SIDE: 6d NAILS @4"oc

AT PANEL EDGES, @

7/16" OSB ONE SIDE:

7/16" OSB ONE SIDE:

8d NAILS @4"oc AT

5/8" GYP. BD ONE

SIDE: 6d NAILS @4"or

AT PANEL EDGES, @

PANEL EDGES,

INTERMEDIATE

INTERMEDIATE

1st FLOOR

@12"oc TO

INTERMEDIATE

WALL SHEATHING

8d NAILS @4"oc AT

7/16" OSB EACH SIDE:

8d NAILS @6"oc AT

PANEL EDGES,

INTERMEDIATE

INTERMEDIATE

@12"oc TO

7"oc TO

SIMPSON HOLDOWN -

ANCHOR PER

SCHEDULE

2x TREATED SILL

PLATE ANCHORED TO

1/2"Ø GALV. ANCHOR

CONCRETE

FOUNDATION

BOLTS @ 32"oc MAX.

CONC. FOUNDATION w/

FRAMING

PANEL EDGES,

INTERMEDIATE

@12"oc TO

FRAMING

@12"oc TO

FRAMING

PANEL EDGES,

INTERMEDIATE

@12"oc TO

PANEL EDGES,

INTERMEDIATE

@12"oc TO

FRAMING

SHEAR

MC-B

SHEAR

(PER PLAN)

AL-C

SHEAR

WALL TYPE

IL-B

IL-C

IL-D

(PER PLAN)

WALL TYPE

WALL TYPE | 1st FLOOR

BASE HOLD

(EACH END)

CHORD STUDS DOWN ANCHOR FLOOR TIE

HDU5-SDS2.5 w/ 5/8" ANCHOR

& 16" EMBED

(RE: SECTION

1/S0.2)

1/S0.2)

ISIMPSON

HDU5-SDS2.5

w/ 5/8" ANCHOR

& 16" EMBED

(RE: SECTION

SIMPSON

HDU5-SDS2.5

w/ 5/8" ANCHOR

& 16" EMBED

(RE: SECTION 1/S0.2)

BASE HOLD

(EACH END)

ISIMPSON

HDU5-SDS2.5

w/ 5/8" ANCHOR

& 16" EMBED

(RE: SECTION

SIMPSON

HDU5-SDS2.5

& 16" EMBED

1/S0.2)

SIMPSON

1/S0.2)

HDU5-SDS2.5

w/ 5/8" ANCHOR

& 16" EMBED

(RE: SECTION

CHORD STUDS DOWN ANCHOR FLOOR TIE

HDU14-SDS2.5

w/ 1" ANCHOR

& 16" EMBED

(RE: SECTION

HDU5-SDS2.5

w/ 5/8" ANCHOR

& 16" EMBED

(RE: SECTION

1/S0.2)

1/S0.2)

BASE HOLD | FLOOR-TO-

(EACH END) (EACH END)

HDU14-SDS2.5 | HDU14-SDS2.5

SECTION 3/S3.3) SECTION 2/S0.2)

HDU5-SDS2.5 HDU5-SDS2.5

(RE: SECTION | (RE: SECTION

1/\$0.2) 1/\$0.2)

w/ 5/8" ANCHOR w/ 5/8" ANCHOR

ROD (RE: ROD (RE:

HDU14-SDS2.5

T&B w/ 1" TIE

SECTION 2/S0.2)

ROD (RE:

SIMPSON

(RE: SECTION

w/ 5/8" ANCHOR

1/S0.2)

DOWN ANCHOR FLOOR TIE

(EACH END)

(EACH END)

(EACH END)

(5) 2x6's

FLOOR-TO-

(EACH END)

FLOOR-TO-

(EACH END)

TYPE

(MC1B)

(MC1C)

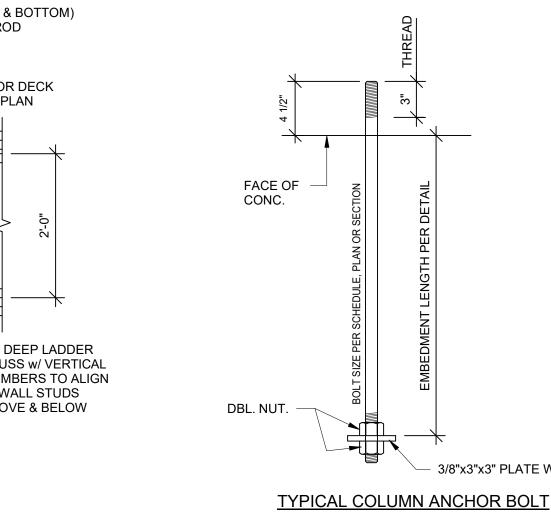
HEADER SIZE

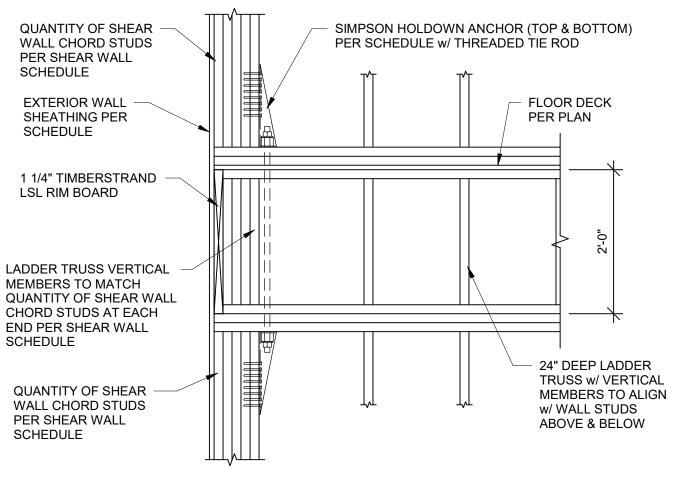
RE: SHEAR WALL SCHEDULE (AT EACH END OF SHEAR WALL)

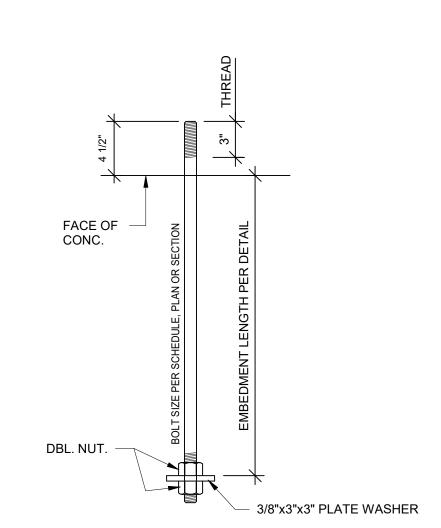
2 **SECTION**3/4" = 1'-0" 3 **SECTION**

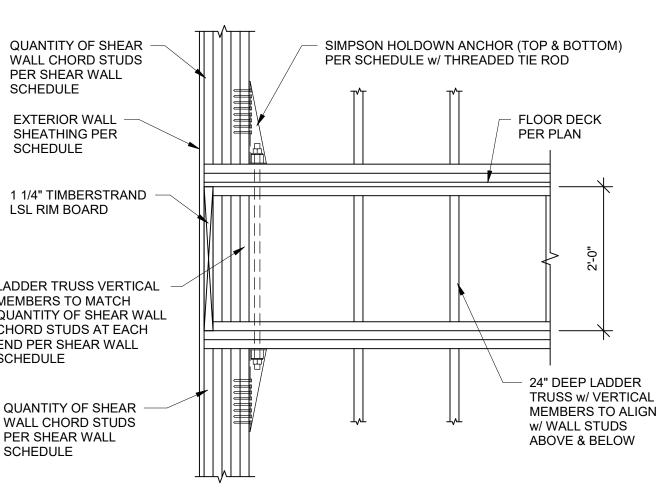
TYPICAL COLUMN ANCHOR BOLT

QUANTITY OF SHEAR — WALL CHORD STUDS PER SHEAR WALL		SIMPSON HOLDOW PER SCHEDULE w/		
SCHEDULE		T ^T	T [*] T	
EXTERIOR WALL SHEATHING PER SCHEDULE				DR DECK PLAN]
				
1 1/4" TIMBERSTRAND LSL RIM BOARD				
LADDER TRUSS VERTICAL MEMBERS TO MATCH QUANTITY OF SHEAR WALL CHORD STUDS AT EACH END PER SHEAR WALL				5-0"
SCHEDULE				
QUANTITY OF SHEAR WALL CHORD STUDS PER SHEAR WALL SCHEDULE			TRI ME w/ v	DEEP LADDER USS w/ VERTICAL MBERS TO ALIGN WALL STUDS OVE & BELOW
FLOOR-	TO-FLOOR SHE	EAR WALL TIE	DETAIL	









4 1/2"	3" THREAD	
FACE OF CONC.	BOLT SIZE PER SCHEDULE, PLAN OR SECTION EMBEDMENT LENGTH PER DETAIL	
DBL. NUT.	3/8"x3"x3" PLATE WASHER	

(RE: FOUNDATION PLAN FO	R LOCATION & ORIENTATION)
HSS-6x6 COLUMN PER PLAN (8) 3/4"Ø ANCHOR BOLTS w/ 1'-6" EMBEDMENT (RE: 3/S0.2) 4 1/2" 3/4"x12"x12" BASE PL ON 1 1/2" GROUT	HSS-5x5COLUMN PER PLAN (4) 3/4"Ø ANCHOR BOLTS w/ 1'-0" EMBEDMENT (RE: 3/S0.2) B 3/4"x11"x11" BASE PL ON 1 1/2" GROUT
HSS-8x8 COLUMN PER PLAN (8) 1"Ø ANCHOR BOLTS w/ 1'-6" EMBEDMENT (RE: 3/S0.2) 1"x15"x15" BASE PL ON 1 1/2" GROUT	HSS-3x3 COLUMN PER PLAN (4) 3/4"Ø ANCHOR BOLTS w/ 1'-6" EMBEDMENT (RE: 3/S0.2) 3/4"x12"x12" BASE PL ON 1 1/2" GROUT

COLUMN BASE PLATE DETAILS

0's w/ (2) 1/2" OOD SPACERS 0's w/ (2) 1/2" OOD SPACERS 8" W/ (2) 1/2" OOD SPACERS 4" DEEP LVL's 4" DEEP LVL's 8" W/ (2) 1/2" OOD SPACERS	(1) 2x6 (1) 2x6 (1) 2x6 (2) 2x6's (2) 2x6's (1) 2x6	(2) 2x6 (1) 2x6 (1) 2x6 (2) 2x6 (1) 2x6	RE: SECTION 1/S0.3 RE: SECTION 1/S0.3 RE: SECTION 1/S0.3 RE: SECTION 1/S0.3
90D SPACERS 8" W/ (2) 1/2" 90D SPACERS 4" DEEP LVL's 4" DEEP LVL's 8" W/ (2) 1/2"	(1) 2x6 (2) 2x6's (2) 2x6's	(1) 2x6 (2) 2x6	RE: SECTION 1/S0.3 RE: SECTION 1/S0.3
ODD SPACERS 4" DEEP LVL's 4" DEEP LVL's 8" W/ (2) 1/2"	(2) 2x6's (2) 2x6's	(2) 2x6	RE: SECTION 1/S0.3
4" DEEP LVL's 8" W/ (2) 1/2"	(2) 2x6's	, ,	
8" W/ (2) 1/2"	· ,	(1) 2x6	RE: SECTION 1/S0.3
	(1) 2v6		
	(1) 200	(1) 2x6	RE: SECTION 1/S0.3
/4" DEEP LVL's	(3) 2x6's	(2) 2x6	RE: SECTION 1/S0.3
/4" DEEP LVL's	(3) 2x6's	(1) 2x6	RE: SECTION 1/S0.3
/4" DEEP LVL's	(4) 2x6's	(1) 2x6	RE: SECTION 1/S0.3
8" W/ (2) 1/2" ODD SPACERS	(1) 2x6	(1) 2x6	RE: SECTION 1/S0.3
	8" W/ (2) 1/2"	8" W/ (2) 1/2" (1) 2×6	8" W/ (2) 1/2"

HEADER SCHEDULE - INDEPENDENT LIVING				
TYPE	HEADER SIZE	BEARING STUDS BELOW EACH END OF HEADER	CONTINUOUS JAMB STUDS AT EACH END	REMARKS
(IL1A)	(3) 2x10's w/ (2) 1/2" PLYWOOD SPACERS	(1) 2x6	(2) 2x6	RE: SECTION 1/S0.3
(IL1B)	(3) 2x10's w/ (2) 1/2" PLYWOOD SPACERS	(1) 2x6	(1) 2x6	RE: SECTION 1/S0.3
(IL1C)	(3) 2X8" W/ (2) 1/2" PLYWOOD SPACERS	(1) 2x6	(1) 2x6	RE: SECTION 1/S0.3
(IL2A)	(3) 9 1/4" DEEP LVL's	(2) 2x6's	(2) 2x6	RE: SECTION 1/S0.3
(IL2B)	(3) 9 1/4" DEEP LVL's	(2) 2x6's	(1) 2x6	RE: SECTION 1/S0.3
(IL2C)	(3) 2X8" W/ (2) 1/2" PLYWOOD SPACERS	(1) 2x6	(1) 2x6	RE: SECTION 1/S0.3
(IL3A)	(3) 11 1/4" DEEP LVL's	(3) 2x6's	(2) 2x6	RE: SECTION 1/S0.3
(IL3B)	(3) 11 1/4" DEEP LVL's	(3) 2x6's	(1) 2x6	RE: SECTION 1/S0.3
(IL3C)	(3) 11 1/4" DEEP LVL's	(4) 2x6's	(1) 2x6	RE: SECTION 1/S0.3
(IL3D)	(3) 2X8" W/ (2) 1/2" PLYWOOD SPACERS	(1) 2x6	(1) 2x6	RE: SECTION 1/S0.3

HEADER SCHEDULE - INDEPENDENT LIVING					
TYPE	HEADER SIZE	BEARING STUDS BELOW EACH END OF HEADER	CONTINUOUS JAMB STUDS AT EACH END	REMARKS	
(IL1A)	(3) 2x10's w/ (2) 1/2" PLYWOOD SPACERS	(1) 2x6	(2) 2x6	RE: SECTION 1/S0.3	
(IL1B)	(3) 2x10's w/ (2) 1/2" PLYWOOD SPACERS	(1) 2x6	(1) 2x6	RE: SECTION 1/S0.3	
(IL1C)	(3) 2X8" W/ (2) 1/2" PLYWOOD SPACERS	(1) 2x6	(1) 2x6	RE: SECTION 1/S0.3	
(IL2A)	(3) 9 1/4" DEEP LVL's	(2) 2x6's	(2) 2x6	RE: SECTION 1/S0.3	
(IL2B)	(3) 9 1/4" DEEP LVL's	(2) 2x6's	(1) 2x6	RE: SECTION 1/S0.3	
(IL2C)	(3) 2X8" W/ (2) 1/2" PLYWOOD SPACERS	(1) 2x6	(1) 2x6	RE: SECTION 1/S0.3	
(IL3A)	(3) 11 1/4" DEEP LVL's	(3) 2x6's	(2) 2x6	RE: SECTION 1/S0.3	
(IL3B)	(3) 11 1/4" DEEP LVL's	(3) 2x6's	(1) 2x6	RE: SECTION 1/S0.3	
(IL3C)	(3) 11 1/4" DEEP LVL's	(4) 2x6's	(1) 2x6	RE: SECTION 1/S0.3	
(IL3D)	(3) 2X8" W/ (2) 1/2" PLYWOOD SPACERS	(1) 2x6	(1) 2x6	RE: SECTION 1/S0.3	

HEADER SCHEDULE - ASSISTED LIVING				
TYPE	HEADER SIZE	BEARING STUDS BELOW EACH END OF HEADER	CONTINUOUS JAMB STUDS AT EACH END	REMARKS
(AL1A)	(3) 2x8's w/ (2) 1/2" PLYWOOD SPACERS	(1) 2x6's	(2) 2x6	RE: SECTION 1/S0.3
(AL1B)	(3) 2x8's w/ (2) 1/2" PLYWOOD SPACERS	(1) 2x6's	(1) 2x6	RE: SECTION 1/S0.3
(AL1C)	(3) 2x12's w/ (2) 1/2" PLYWOOD SPACERS	(2) 2x6's	(2) 2x6	RE: SECTION 1/S0.3

HEADER SCHEDULE - MEMORY CARE

CONTINUOUS JAMB

STUDS AT EACH END

BEARING STUDS BELOW

EACH END OF HEADER

(0) 0 01 / (0) 4/0"				l ,	
(3) 2x8's w/ (2) 1/2" PLYWOOD SPACERS	(1) 2x6's	(2) 2x6	RE: SECTION 1/S0.3		EXTERIOR WALL (EXTERIOR SIDE)
(3) 2x8's w/ (2) 1/2" PLYWOOD SPACERS	(1) 2x6's	(1) 2x6	RE: SECTION 1/S0.3		U.N.O. PER SHEAR WALL SCHEDULE
(3) 2x10's w/ (2) 1/2" PLYWOOD SPACERS	(2) 2x6's	(2) 2x6	RE: SECTION 1/S0.3		EXTERIOR WALL (INTERIOR SIDE)
					U.N.O. PER SHEAR WALL SCHEDULE
					TYP. INTERIOR WALL
					U.N.O. PER SHEAR WALL SCHEDULE
					INDEPENDENT
HEAD	ER SCHEDULE - A		LIVING BUILDING MAIN CORRIDOR INTERIOR WALLS (CORRIDOR SIDE)		
HEADER SIZE	BEARING STUDS BELOW EACH END OF HEADER	CONTINUOUS JAMB STUDS AT EACH END	REMARKS		NOTES:
	LAGITEND OF TILADER	OTODO AT LACITLIND			1. REFER TO SHEA
(3) 2x8's w/ (2) 1/2" PLYWOOD SPACERS	(1) 2x6's	(2) 2x6	RE: SECTION 1/S0.3		SHEAR WALLS.

REMARKS

WALL SHEATHING SCHEDULE				
LOCATION	SHEATHING	FASTENER SPACING		
LOCATION	SHEATHING	PANEL EDGE FIELD		
EXTERIOR WALL (EXTERIOR SIDE) U.N.O. PER SHEAR WALL SCHEDULE	7/16" OSB	8d COMMON NAILS @ 4"oc	8d COMMON NAILS @ 12"oc	
EXTERIOR WALL (INTERIOR SIDE) U.N.O. PER SHEAR WALL SCHEDULE	5/8" GYPSUM BOARD	6d COOLER NAILS @ 4"oc	6d COOLER NAILS @ 7"oc	
TYP. INTERIOR WALL U.N.O. PER SHEAR WALL SCHEDULE	5/8" GYPSUM BOARD	6d COOLER NAILS @ 4"oc	6d COOLER NAILS @ 7"oc	
INDEPENDENT LIVING BUILDING MAIN CORRIDOR INTERIOR WALLS (CORRIDOR SIDE)	7/16" OSB	8d COMMON NAILS @ 4"oc	8d COMMON NAILS @ 12"oc	

NO	TES:
1.	REFER TO SHEAR WALL SCHEDULE FOR SHEATHING OF SHEAR WALLS
2.	ALL PANEL JOINTS ARE TO BE FULLY BLOCKED.

STUD BEARING WALL SCHEDULE		
LOCATION	HEIGHT OF WALL	
LOCATION	0'-0" TO 10'-1 5/8"	10'-2" TO 13'-2 3/4"
1st FLOOR EXTERIOR WALLS AT 3-STORY STRUCTURE	(2) 2x6 @16"oc	
1st FLOOR INTERIOR WALLS AT 3-STORY STRUCTURE	(2) 2x6 @16"oc	
2nd FLOOR EXTERIOR WALLS AT 3-STORY STRUCTURE	2x6 @16"oc	
2nd FLOOR INTERIOR WALLS AT 3-STORY STRUCTURE	2x6 @16"oc	
3rd FLOOR EXTERIOR WALLS AT 3-STORY STRUCTURE	2x6 @16"oc	
3rd FLOOR INTERIOR WALLS AT 3-STORY STRUCTURE	2x6 @16"oc	
1st FLOOR EXTERIOR WALLS AT 1-STORY STRUCTURE	2x6 @16"oc	2x6 @12"oc
1st FLOOR INTERIOR WALLS AT 1-STORY STRUCTURE	2x6 @16"oc	2x6 @12"oc

	0'-0" TO 10'-1 5/8"	10'-2" TO 13'-2 3/4"
1st FLOOR EXTERIOR WALLS AT 3-STORY STRUCTURE	(2) 2x6 @16"oc	
1st FLOOR INTERIOR WALLS AT 3-STORY STRUCTURE	(2) 2x6 @16"oc	
2nd FLOOR EXTERIOR WALLS AT 3-STORY STRUCTURE	2x6 @16"oc	
2nd FLOOR INTERIOR WALLS AT 3-STORY STRUCTURE	2x6 @16"oc	
3rd FLOOR EXTERIOR WALLS AT 3-STORY STRUCTURE	2x6 @16"oc	
3rd FLOOR INTERIOR WALLS AT 3-STORY STRUCTURE	2x6 @16"oc	
1st FLOOR EXTERIOR WALLS AT 1-STORY STRUCTURE	2x6 @16"oc	2x6 @12"oc
1st FLOOR INTERIOR WALLS AT 1-STORY STRUCTURE	2x6 @16"oc	2x6 @12"oc
NOTES: 1. UNLESS NOTED OT	HERWISE, PROVIDE	STUD PACKS AT

ALL GIRDER TRUSS BEARING LOCATIONS. QUANTITY OF STUDS SHALL BE 3 STUDS MINIMUM PLUS ONE ADDITIONAL STUD FOR EACH PLY OF GIRDER TRUSS. REFER TO SECTION 6/S0.3. STUD PACKS SHALL BE ALIGNED & PROVIDED AT EACH LEVEL OF 3-STORY STRUCTURE TO TRANSFER LOAD TO THE FOUNDATION. 2. PROVIDE SQUASH BLOCKS (ALIGNED WITH WALL STUDS ABOVE & BELOW) WITHIN THE DEPTH OF THE FLOOR FRAMING TO TRANSFER STUD LOADS TO THE WALL

BELOW, WHERE SUPPORT IS NOT OTHERWISE PROVIDED 3. WALL STUDS AT DOUBLE-HEIGHT SPACES THAT ARE NOT

BRACED BY THE FLOOR OR BY A HORIZONTAL GIRT AT THE FLOOR LEVEL SHALL BE 1 1/2" x 7 1/4" TIMBERSTRAND LSL STUDS UNLESS NOTED OTHERWISE.
FLOOR & ROOF DECK SCHEDULE

FLOOR & ROOF DECK SCHEDULE			
LOGATION	FASTENER SPACING		
LOCATION	DECKING	PANEL EDGE	FIELD
TYPICAL ROOF DECK	19/32" OSB	10d COMMON NAILS @6"oc	10d COMMON NAILS @12"oc
MECHANICAL WELL ROOF DECK	3/4" PLYWOOD	10d COMMON NAILS @6"oc	10d COMMON NAILS @12"oc
FLOOR DECK	3/4" T&G PLYWOOD	10d RING SHANK NAILS @6"oc (GLUED & NAILED)	10d RING SHANK NAILS @12"oc (GLUED & NAILED

TES:				
SPRE	EAD FOOTINGS SH	ALL BE P	OURED	
MON	OLITHIC w/ GRADE	BEAMS	& CONTINUOUS	
WALI	L FOOTINGS. REIN	IFORCIN	G FOR GRADE	
BEAN	IS & CONTINUOUS	WALL FO	OOTINGS SHALL	BE
001	TINILIOLIO TUDOLIO	LLCDDEA	D FOOTINGS	

FOOTING SCHEDULE		
FOOTING TYPE	FOOTING SIZE (FT.) x THICKNESS (IN.)	REINFORCING (EACH WAY)
3.0	3'-0"x3'-0"x18" Dp	#4@6"oc BOTTOM
4.0	4'-0"x4'-0"x32" Dp	#4@6"oc (TOP & BOT)
⟨4.0a⟩	4'-0"x4'-0"x32" Dp	#4@6"oc (TOP & BOT)
(5.0)	5'-0"x5'-0"x24" Dp	#4@6"oc BOTTOM
⟨5.0a⟩	5'-0"x5'-0"x36" Dp	#5@6"oc (TOP & BOT)
⟨6.0a⟩	6'-0"x6'-0"x36" Dp	#5@6"oc (TOP & BOT)
⟨8.0a⟩	8'-0"x8'-0"x32" Dp	#5@6"oc (TOP & BOT)

NAILING SCHEDULE (REFER TO NOTES #1 and #2)

3- 3" x 0.131" NAILS-TOENAIL

3" x 0.131" NAILS AT 8"o.c.-

4-3" x 0.131" NAILS AT 6"o.c.

3- 3" x 0.131" NAILS-END NAIL

3- 3" x 0.131" NAILS-END NAIL

3-3" x 0.131" NAILS -TOENAIL

3- 3" x 0.131" NAILS-FACE NAIL

5- 3" x 0.131" NAILS-TOENAIL

4- 3" x 0.131" NAILS-TOENAIL

4- 3" x 0.131" NAILS-FACE NAIL

3- 3" x 0.131" NAILS-TOENAIL

3" x 0.131" NAILS AT 16"o.c.

OPPOSITE SIDES

BOTTOM ALONG EDGE

4.) ALL NAILS NOTED AS 8d, 10d, 16d, ETC. SHALL BE COMMON NAILS UNLESS NOTED BOX.

2-3" x 0.131" NAILS-FACE NAIL

TOP AND BOTTOM STAGGERED ON

3" x 0.131" NAILS AT 6"o.c. TOP AND

1.) ALL NAILS SHALL BE AS NOTED UNLESS OTHERWISE SPECIFIED ON STRUCTURAL DRAWINGS OR

2.) CONDITIONS NOT SPECIFIED SHALL BE IN ACCORDANCE WITH CURRENT INTERNATIONAL BUILDING CODE.

3" x 0.131" NAILS AT 6"o.c.-TOENAIL

3" x 0.131" NAILS AT 10"o.c. ALONG

4- 3" x 0.131" NAILS-TOENAIL OR

3" x 0.131" NAILS AT 8"o.c.-FACE NAIL

3" x 0.131" NAILS AT 12"o.c.-FACE NAIL

BRACED WALL PANELS

TYPICAL FACE NAIL

12-3" x 0.131" NAILS

EACH EDGE

CEILING JOISTS, LAPS OVER 4- 3" x 0.131" NAILS-FACE NAIL

ALTERNATE PROVIDED BY ENGINEER IN WRITING.

———— NAIL LENGTH

—— QUANITY

DIAMETER IN INCHES

CONNECTION

JOIST TO SILL OR GIRDER

SOLE PLATE TO JOIST OR

BRIDGING TO JOIST

TOP PLATE TO STUD

STUD TO SOLE PLATE

DOUBLED TOP PLATES

AND INTERSECTIONS

DOUBLE TOP PLATE LAPS

BLOCKING BETWEEN JOISTS

OR RAFTERS TO TOP PLATE

RIM JOIST TO TOP PLATE TOP PLATE LAPS AND

CONTINUOUS HEADER,

CEILING JOISTS TO PLATE

INTERSECTIONS

TWO PIECES

CONTINUOUS

PARTITIONS

HEADER TO STUD

CEILING JOISTS TO

RAFTER TO PLATE

MULTIPLE STUDS

AND PLATE

BEAMS

PARALLEL RAFTERS

1" BRACE TO EACH STUD

BUILT-UP CORNER AND

BUILT-UP GIRDER AND

BUILT-UP LAMINATED

2" PLANKING

VENEER LUMBER BEAMS

3.) NAILING DESIGNATION:

4 - 3" x 0.131" NAILS

DOUBLE STUDS

BLOCKING

ATTACHMENTS (REF NOTE #3 and #4)

2- 3" x 0.131" NAILS-TOENAIL EACH END 2-8d NAILS-TOENAIL EACH END

3-8d NAILS-TOENAIL

MAX. FACE NAILING

16d BOX NAILS AT 16"o.c.

3-16d BOX NAILS AT 16"o.c.

4-8d NAILS-TOENAIL OR 2-16d

16d BOX NAILS AT 24"o.c. MAX.

16d BOX NAILS AT 16"o.c. MAX.

8d NAILS AT 6"o.c. MAX.-TOENAIL

16d NAILS AT 16"o.c. MAX. ALONG

BRACED WALL PANEL

2-16d NAILS-END NAIL

NAILS-END NAIL

FACE NAIL

FACE NAIL

8-16d NAILS

3-8d NAILS-TOENAIL

2-16d NAILS-FACE NAIL

EACH EDGE-TOENAIL

3-8d NAILS-TOENAIL

4-8d NAILS-TOENAIL

3-16d NAILS-FACE NAIL

3-16d NAILS-FACE NAIL

3-8d NAILS-TOENAIL

2-8d NAILS-FACE NAIL

OPPSITE SIDES.

3" x 0.131" NAILS AT 24"o.c. FACE NAILED | 20d NAILS AT 32"o.c. MAX. TOP

3- 3" x 0.131" NAILS AT ENDS AND EACH 2-20d NAILS AT ENDS AND EACH

4- 3" x 0.131" NAILS AT EACH SUPPORT 16d NAILS AT EACH SUPPORT

16d NAILS AT 24"o.c. MAX.

AND BOTTOM, STAGGERED ON

16d NAILS AT 12"o.c. TOP AND

BOTTOM ALONG EDGE

- CMU WALL

- BEAM PER

PLAN

3/8" BEARING PLATE

LESS THAN NOMINAL

BELOW PLATE AS

REINFORCING IN

FIRST 2 CELLS

ADJACENT TO

BEARING.

OPENING

(WIDTH OF PLATE TO BE 1"

WIDTH OF WALL). GROUT

REQUIRED FOR UNIFORM

- A706 (WELDABLE) REBAR

DOWELS WELDED ALL

AROUND TO BEAM TOP

SPACING OF WALL

FLANGE TO MATCH SIZE &

VERTICAL REINFORCING

CONTINUOUS THROUGH SPREAD FOOTINGS. SEAL

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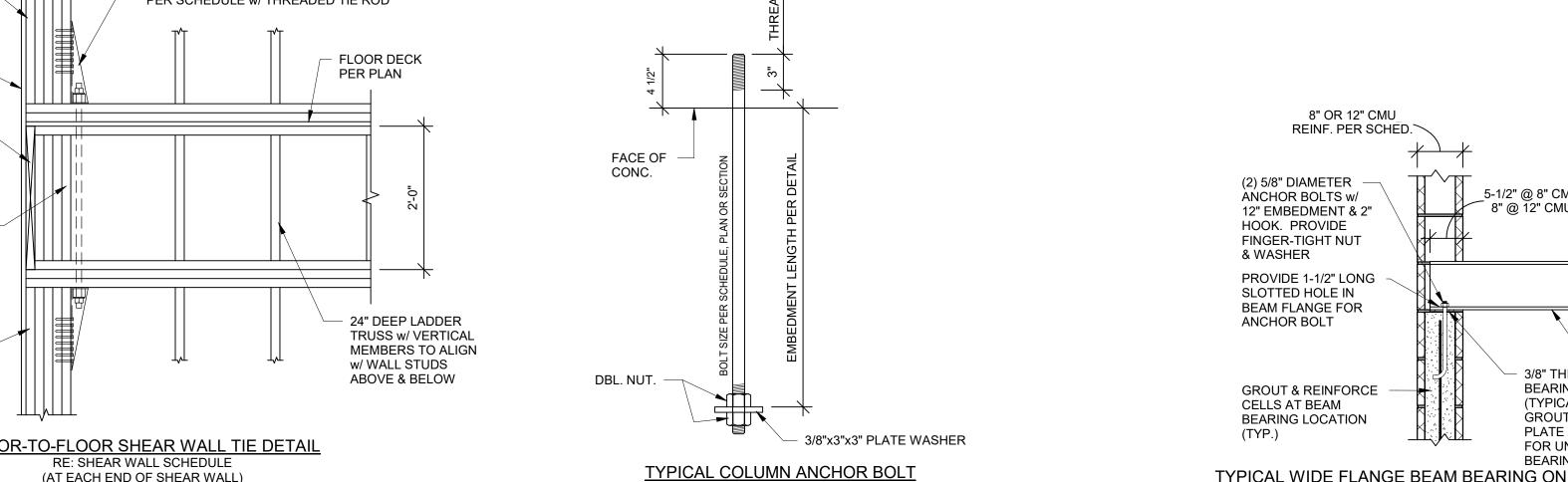
SCHEDULES

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TYPICAL STEEL LINTEL DETAIL AT CMU WALL 5 **SECTION**3/4" = 1'-0"



BEAM PER 3/8" THICK x 8" WIDE **BEARING PLATE** (TYPICAL U.N.O.). **GROUT BELOW** PLATE AS REQUIRED FOR UNIFORM BEARING. TYPICAL WIDE FLANGE BEAM BEARING ON CMU (U.N.O.)

(2) 5/8" DIAMETER

HOOK. PROVIDE

& WASHER

PROVIDE 1-1/2" LONG

SLOTTED HOLE IN

BEAM FLANGE FOR

ANCHOR BOLT

ADD VERTICAL

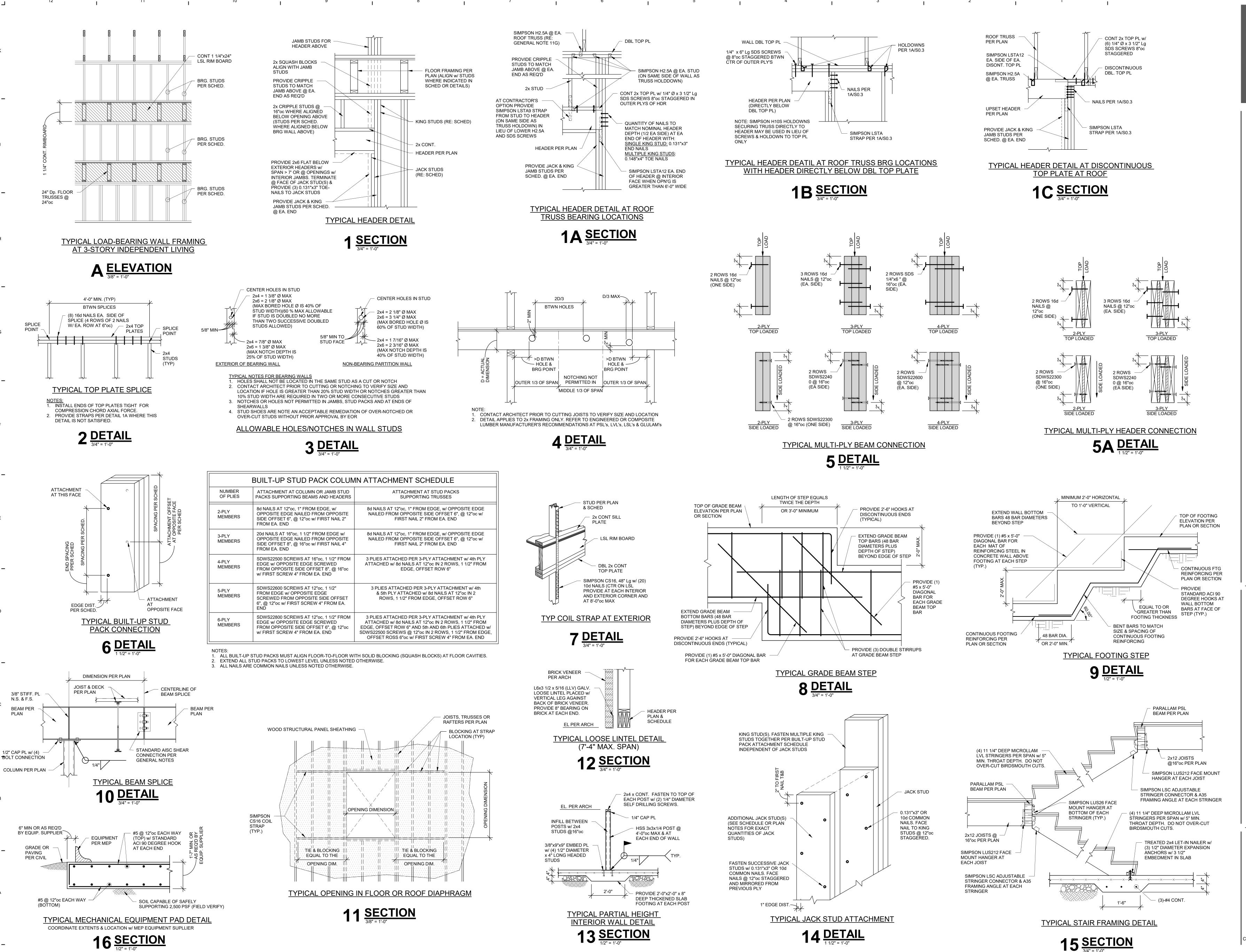
CELL AT BEAM

BAR IN ADJACENT

BEARING OFFSET/

FINGER-TIGHT NUT

ÀNCHOR BOLTS w/ 12" EMBEDMENT & 2"



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> 6-14-19 **TYPICAL**

W BOO

DETAILS

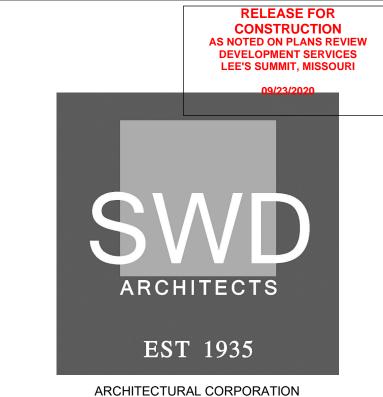
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1 ISOMETRIC VIEW

THIS ISOMETRIC VIEW IS PROVIDED FOR ILLUSTRATIVE PURPOSES ONLY. THIS VIEW IS NOT INTENDED TO SHOW ALL STRUCTURAL MEMBERS & COMPONENTS ASSOCIATED WITH THE WORK. REFER TO PLANS, SECTIONS & DETAILS FOR ADDITIONAL INFORMATION.

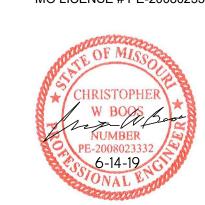


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T, JACKSON COUNTY, MISSOURI

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PARTIAL 1ST FLOOR FOUNDATION PLAN - IL

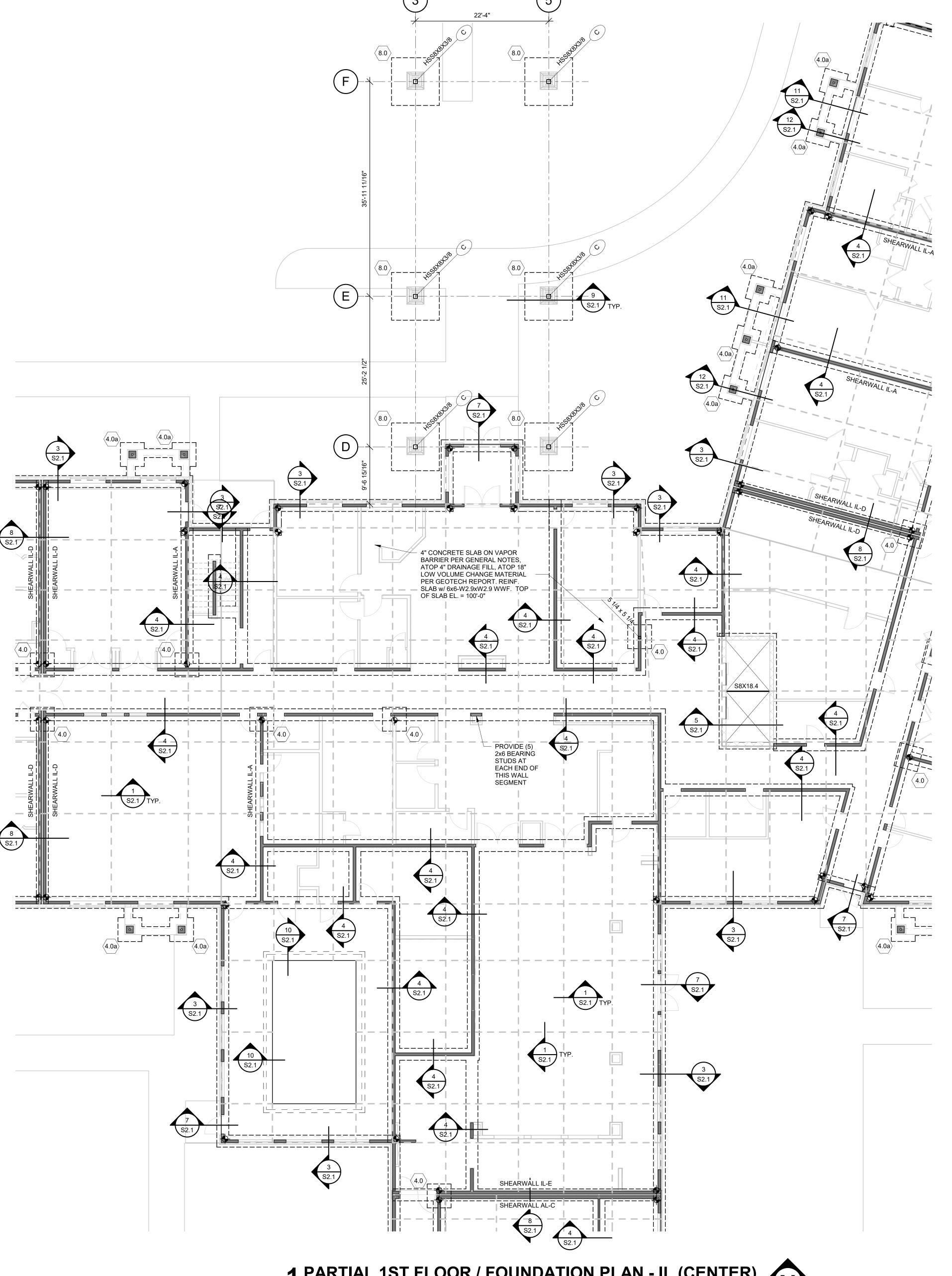
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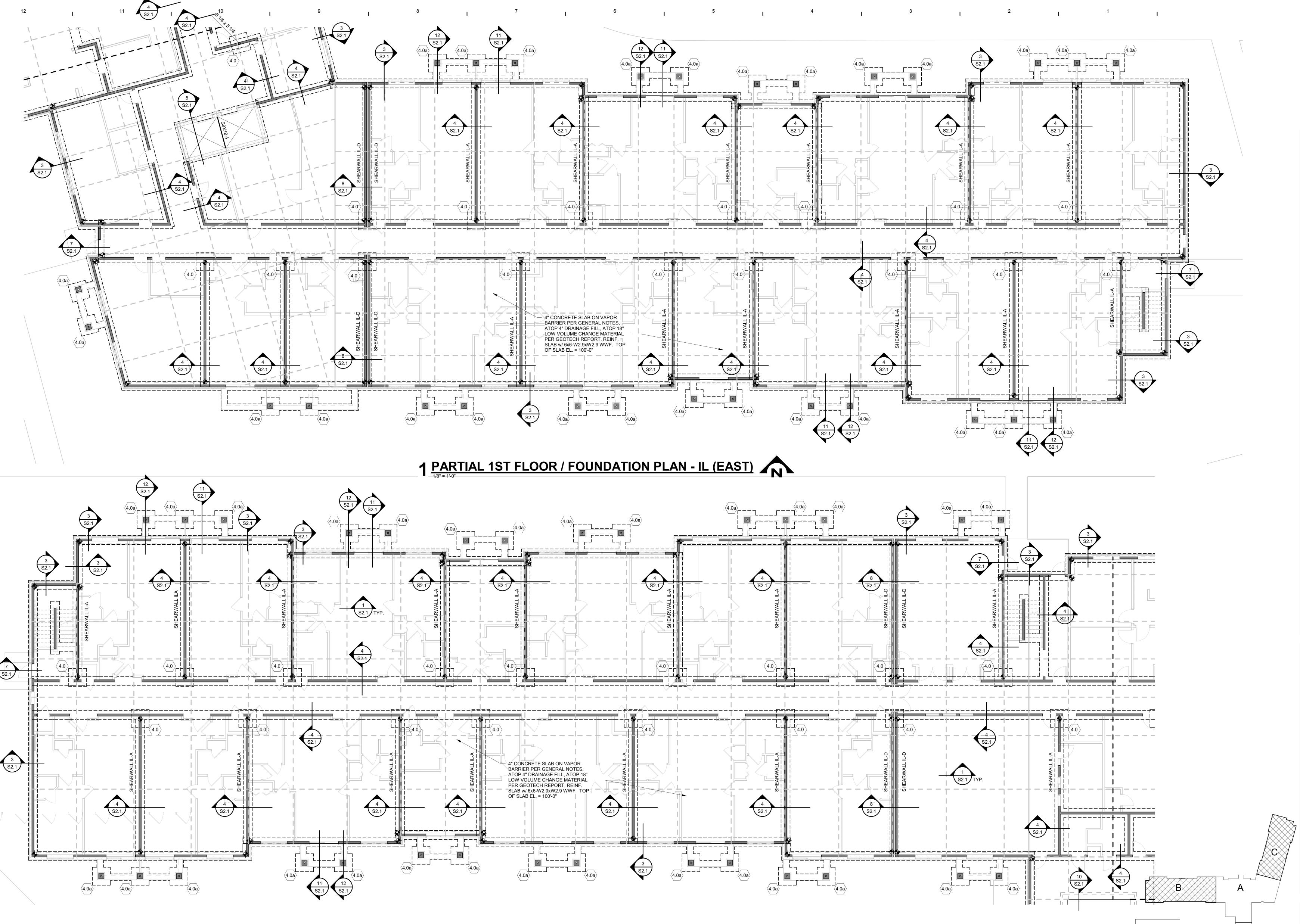
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1 PARTIAL 1ST FLOOR / FOUNDATION PLAN - IL (CENTER)

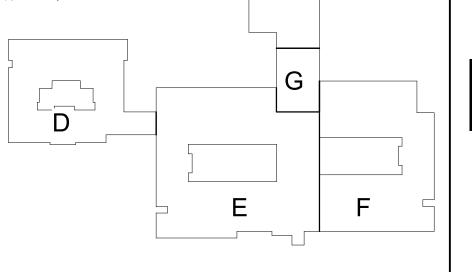
NOTES:
1. REFER TO GENERAL NOTES ON SHEET S0.1.
2. VERIFY ALL DIMENSIONS & ELEVATIONS w/ ARCHITECTURAL DRAWINGS.
3. REFER TO SCHEDULES ON SHEET S0.2.
4. REFER TO TYPICAL DETAILS ON SHEET S0.3.



2 PARTIAL 1ST FLOOR / FOUNDATION PLAN - IL (WEST)

NOTES:

1. REFER TO GENERAL NOTES ON SHEET S0.1.
2. VERIFY ALL DIMENSIONS & ELEVATIONS w/ ARCHITECTURAL DRAWINGS.
3. REFER TO SCHEDULES ON SHEET S0.2.
4. REFER TO TYPICAL DETAILS ON SHEET S0.3.



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PARTIAL 1ST FLOOR FOUNDATION PLAN - IL

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1 PARTIAL 2ND FLOOR FRAMING PLAN - IL (CENTER)

NOTES:

1. REFER TO GENERAL NOTES ON SHEET S0.1.
2. VERIFY ALL DIMENSIONS & ELEVATIONS w/ ARCHITECTURAL DRAWINGS.
3. REFER TO SCHEDULES ON SHEET S0.2

FOR EACH PLY OF GIRDER TRUSS.

REFER TO SCHEDULES ON SHEET S0.2.
 REFER TO TYPICAL DETAILS ON SHEET S0.3.
 UNLESS NOTED OTHERWISE, PROVIDE STUD PACKS CONSISTING OF (4) BEARING STUDS MINIMUM AT ALL PARALLAM PSL BEAM BEARING LOCATIONS. STUD PACKS SHALL BE CONTINUOUS FROM BEARING ELEVATION DOWN TO FOUNDATION w/ MATCHING SQUASH BLOCKS AT ALL FLOOR LEVELS. REFER TO SECTION 6/S0.3 FOR FASTENING OF STUD PACKS.

6. UNLESS NOTED OTHERWISE, PROVIDE STUD PACKS AT ALL GIRDER TRUSS BEARING LOCATIONS. QUANTITY OF STUDS SHALL BE 3 STUDS MINIMUM PLUS ONE ADDITIONAL STUD

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PARTIAL 2ND FLOOR FRAMING PLAN - IL

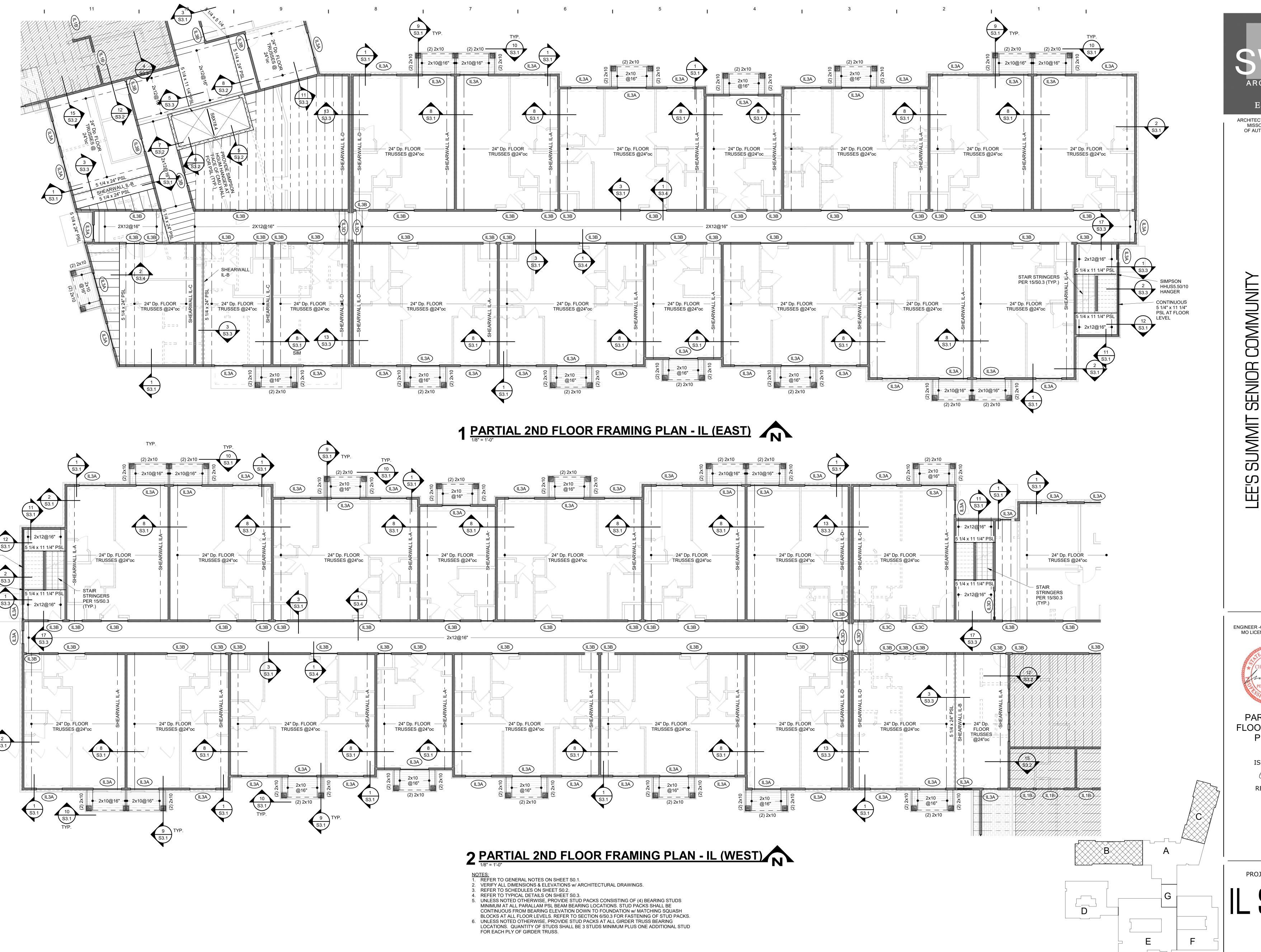
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IL S1.7

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CONTINUOUS 5 1/4" x 11 1/4" PSL AT FLOOR LEVEL

SIMPSON — HHUS5.50/10 HANGER CONSTRUCTION
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PARTIAL 3RD FLOOR FRAMING PLAN - IL

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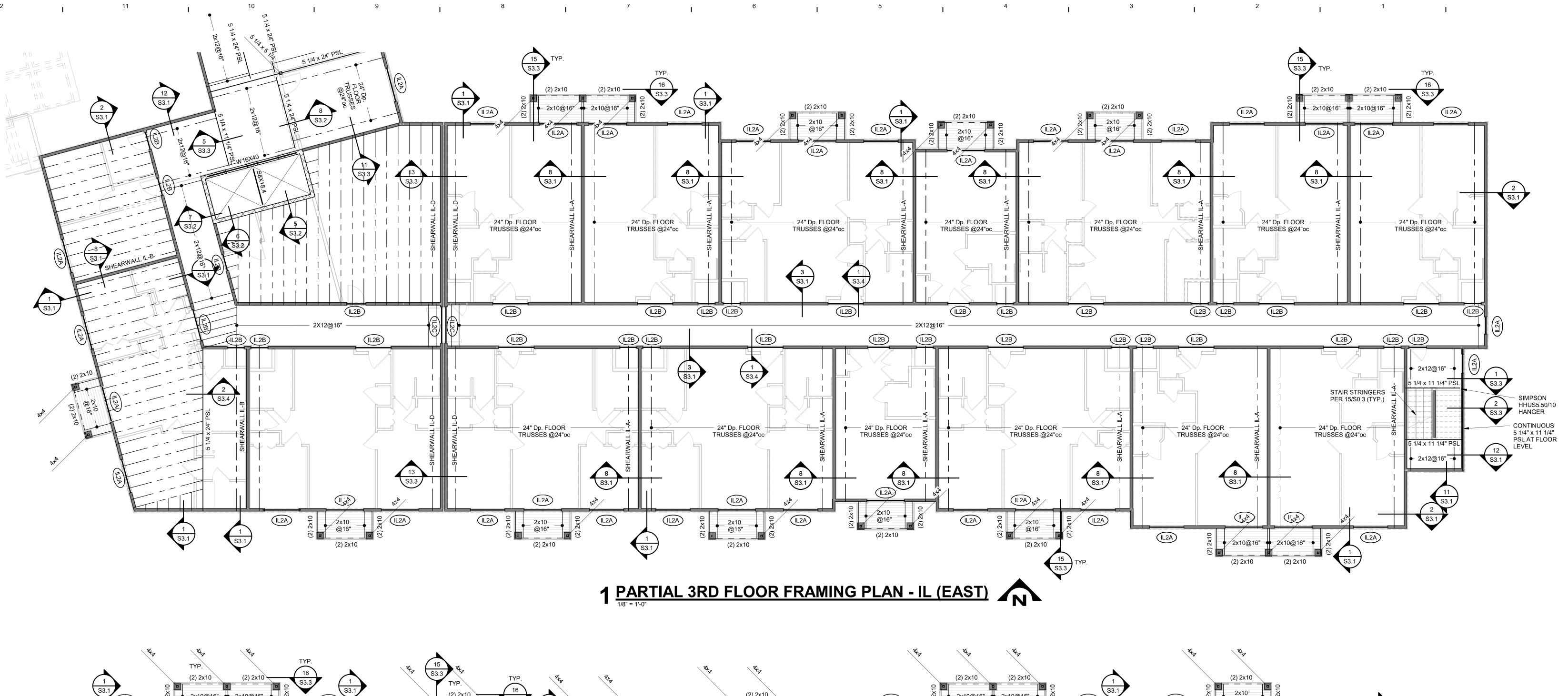
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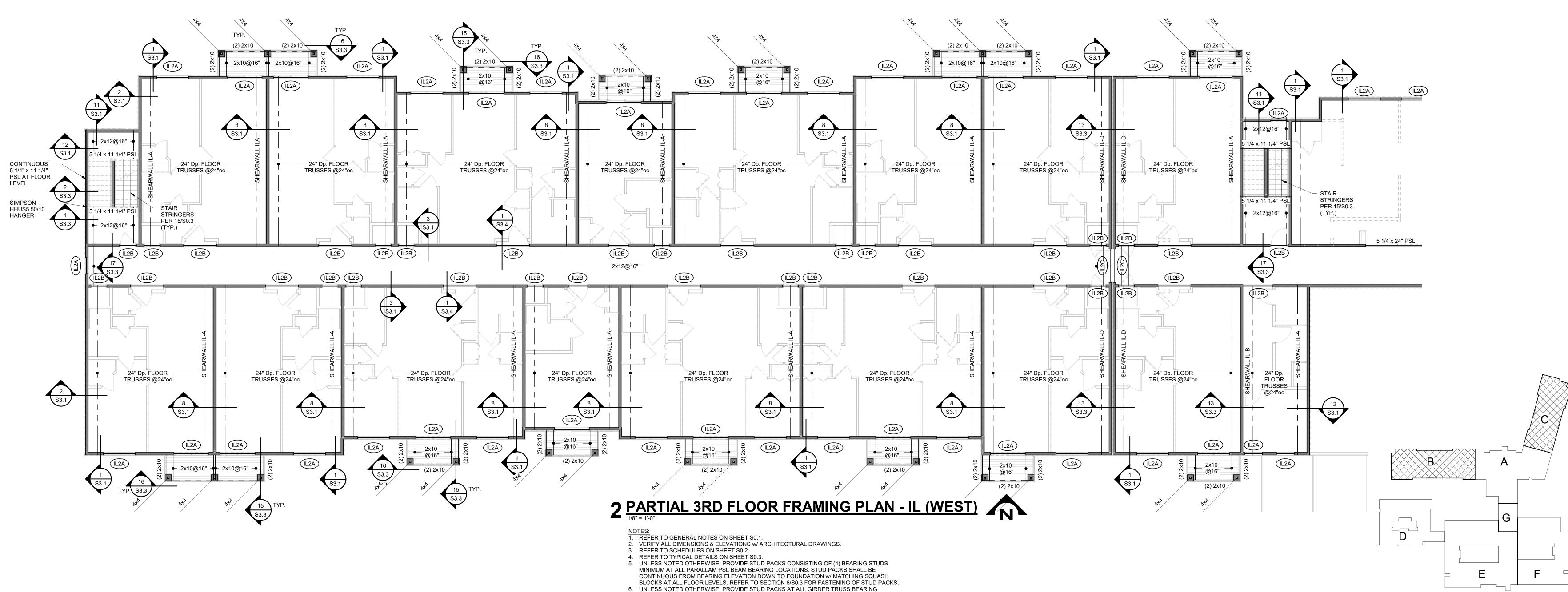
1. REFER TO GENERAL NOTES ON SHEET S0.1.
2. VERIFY ALL DIMENSIONS & ELEVATIONS w/ ARCHITECTURAL DRAWINGS.
3. REFER TO SCHEDULES ON SHEET S0.2.
4. REFER TO TYPICAL DETAILS ON SHEET S0.3.

REFER TO TYPICAL DETAILS ON SHEET S0.3.
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BLOCKS AT ALL FLOOR LEVELS. REFER TO SECTION 6/S0.3 FOR FASTENING OF STUD PACKS.

6. UNLESS NOTED OTHERWISE, PROVIDE STUD PACKS AT ALL GIRDER TRUSS BEARING LOCATIONS. QUANTITY OF STUDS SHALL BE 3 STUDS MINIMUM PLUS ONE ADDITIONAL STUD FOR EACH PLY OF GIRDER TRUSS.





LOCATIONS. QUANTITY OF STUDS SHALL BE 3 STUDS MINIMUM PLUS ONE ADDITIONAL STUD FOR EACH PLY OF GIRDER TRUSS.

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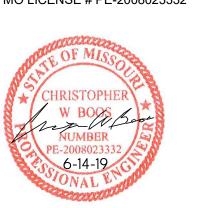
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1 PARTIAL ROOF FRAMING PLAN - IL (CENTER) 1/8" = 1'-0"

- NOTES:

 1. REFER TO GENERAL NOTES ON SHEET S0.1.

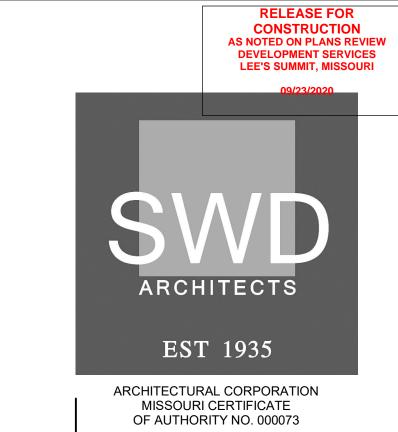
 2. VERIFY ALL DIMENSIONS & ELEVATIONS w/ ARCHITECTURAL DRAWINGS.

 3. REFER TO SCHEDULES ON SHEET S0.2.

 4. REFER TO TYPICAL DETAILS ON SHEET S0.3.

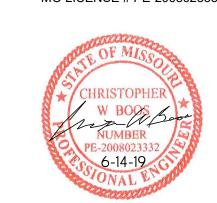
 5. THE TRUSS LAYOUT DEPICTED ON THE FRAMING PLAN IS SHOWN FOR SCHEMATIC PURPOSES.
- THE TRUSS SUPPLIER SHALL BE RESPONSIBLE FOR THE FINAL LAYOUT WHILE COMPLYING W/THE STRUCTURAL DETAILS & UTILIZING THE LOAD BEARING ELEMENTS INDICATED ON THE
- DRAWINGS.

 6. UNLESS NOTED OTHERWISE, PROVIDE STUD PACKS AT ALL GIRDER TRUSS BEARING LOCATIONS. QUANTITY OF STUDS SHALL BE 3 STUDS MINIMUM PLUS ONE ADDITIONAL STUD FOR EACH PLY OF GIRDER TRUSS.



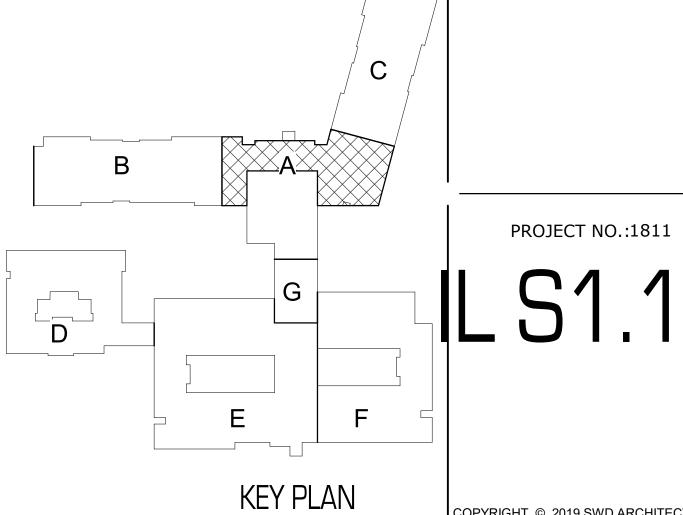
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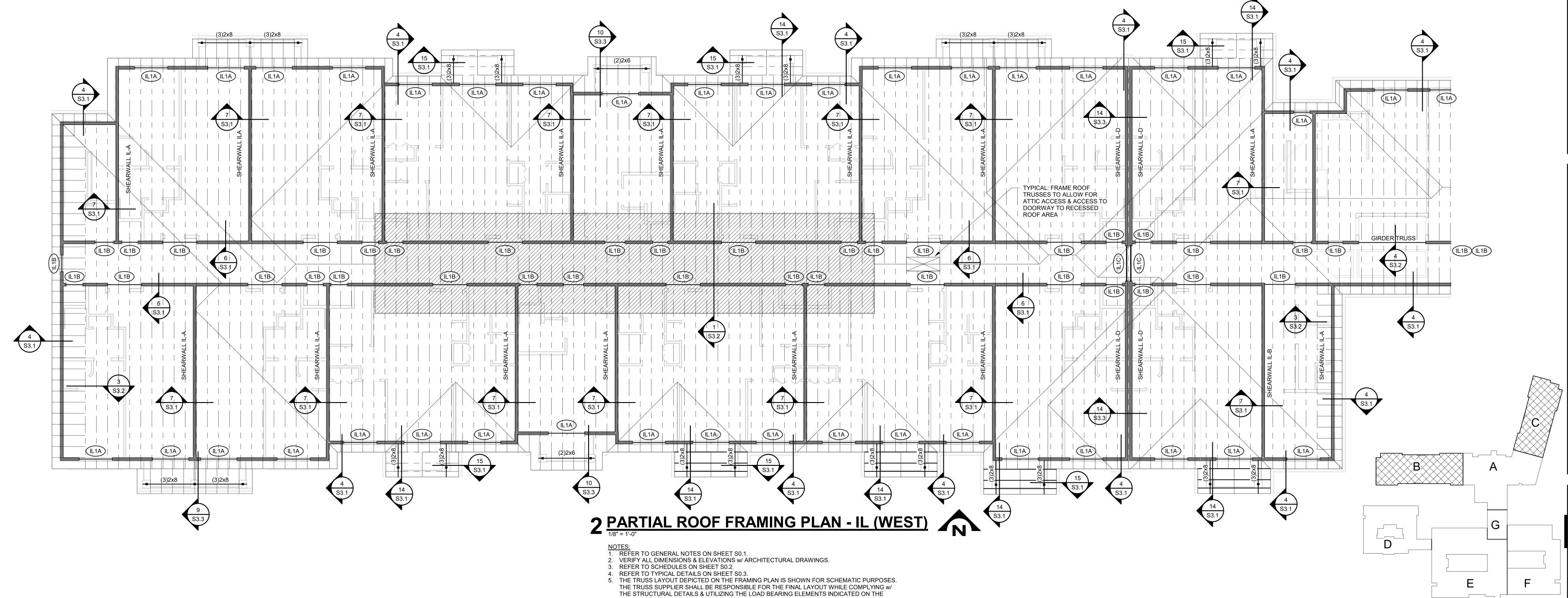
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PARTIAL ROOF FRAMING PLAN -

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6. UNLESS NOTED OTHERWISE, PROVIDE STUD PACKS AT ALL GIRDER TRUSS BEARING LOCATIONS. QUANTITY OF STUDS SHALL BE 3 STUDS MINIMUM PLUS ONE ADDITIONAL STUD

FOR EACH PLY OF GIRDER TRUSS.

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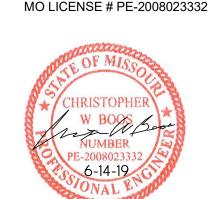
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PARTIAL ROOF FRAMING PLAN -

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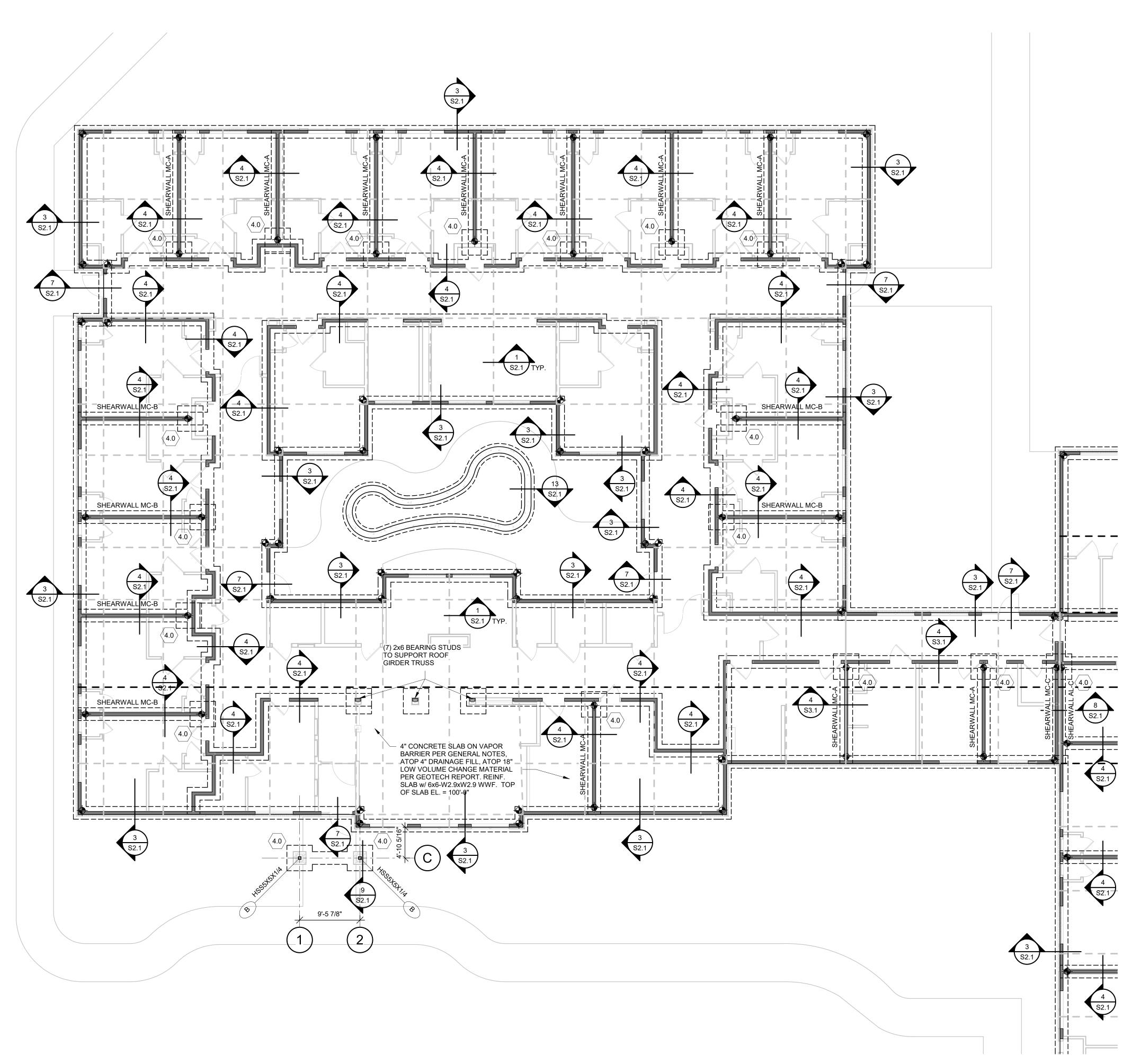
FOUNDATION PLAN - MC ISSUE DATE: 6-14-19

REVISIONS:

PROJECT NO.:1811

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KEY PLAN



1 PARTIAL FIRST FLOOR / FOUNDATION PLAN - MC

NOTES:

1. REFER TO GENERAL NOTES ON SHEET S0.1.

2. VERIFY ALL DIMENSIONS & ELEVATIONS w/ ARCHITECTURAL DRAWINGS.

3. REFER TO SCHEDULES ON SHEET S0.2. 4. REFER TO TYPICAL DETAILS ON SHEET S0.3.

NOTES:

1. REFER TO GENERAL NOTES ON SHEET S0.1.
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4. REFER TO TYPICAL DETAILS ON SHEET S0.3.

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PARTIAL 1ST FLOOR FOUNDATION PLAN - AL WEST

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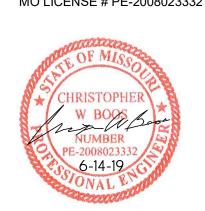
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KEY PLAN



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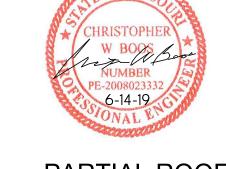
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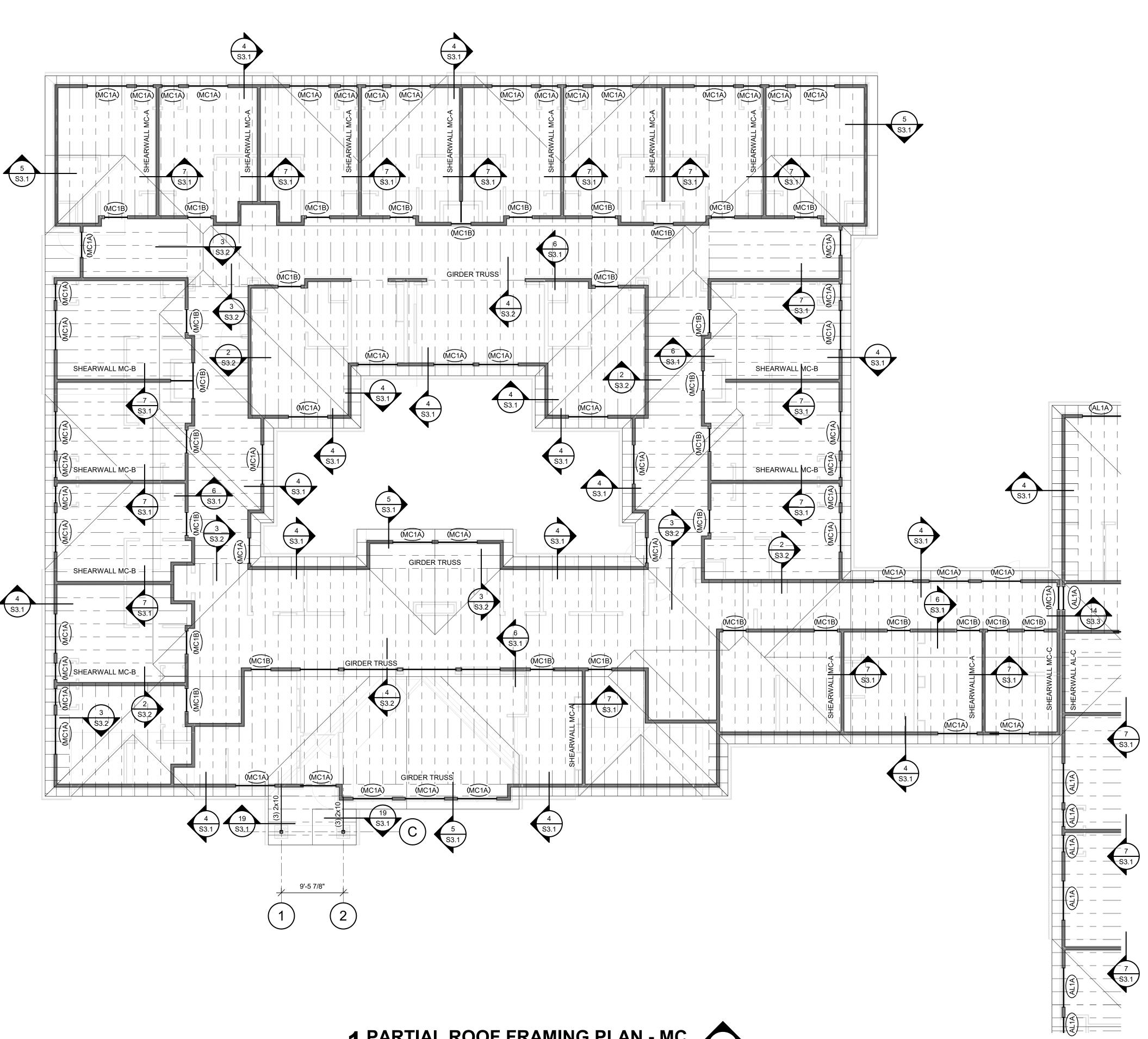
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1 PARTIAL ROOF FRAMING PLAN - MC

- NOTES:

 1. REFER TO GENERAL NOTES ON SHEET S0.1.

 2. VERIFY ALL DIMENSIONS & ELEVATIONS w/ ARCHITECTURAL DRAWINGS.

 3. REFER TO SCHEDULES ON SHEET S0.2.

 4. REFER TO TYPICAL DETAILS ON SHEET S0.3.

 5. THE TRUSS LAYOUT DEPICTED ON THE FRAMING PLAN IS SHOWN FOR SCHEMATIC PURPOSES. THE TRUSS SUPPLIER SHALL BE RESPONSIBLE FOR THE FINAL LAYOUT WHILE COMPLYING w/ THE STRUCTURAL DETAILS & UTILIZING THE LOAD BEARING ELEMENTS INDICATED ON THE
- UNLESS NOTED OTHERWISE, PROVIDE STUD PACKS AT ALL GIRDER TRUSS BEARING LOCATIONS. QUANTITY OF STUDS SHALL BE 3 STUDS MINIMUM PLUS ONE ADDITIONAL STUD FOR EACH PLY OF GIRDER TRUSS.



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PARTIAL ROOF FRAMING PLAN -AL (WEST)

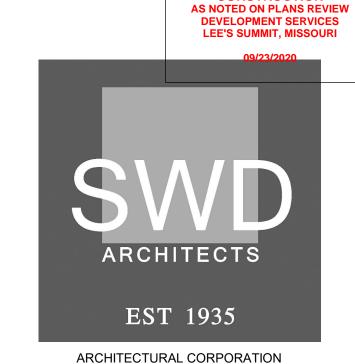
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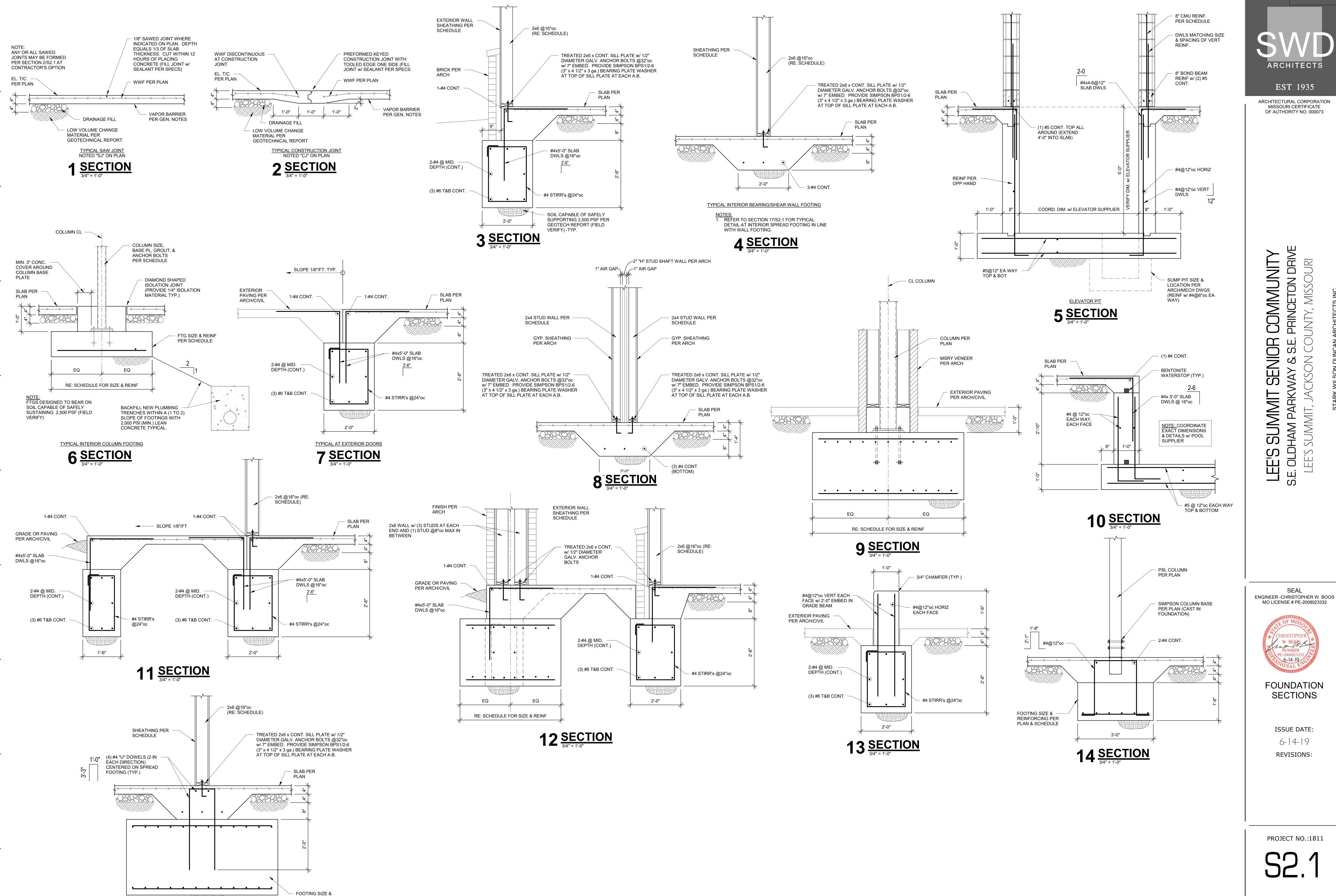
PARTIAL ROOF FRAMING PLAN -AL (EAST)

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REVISIONS: 1 Revision#1 7-26-19

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KEY PLAN



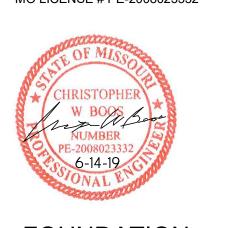
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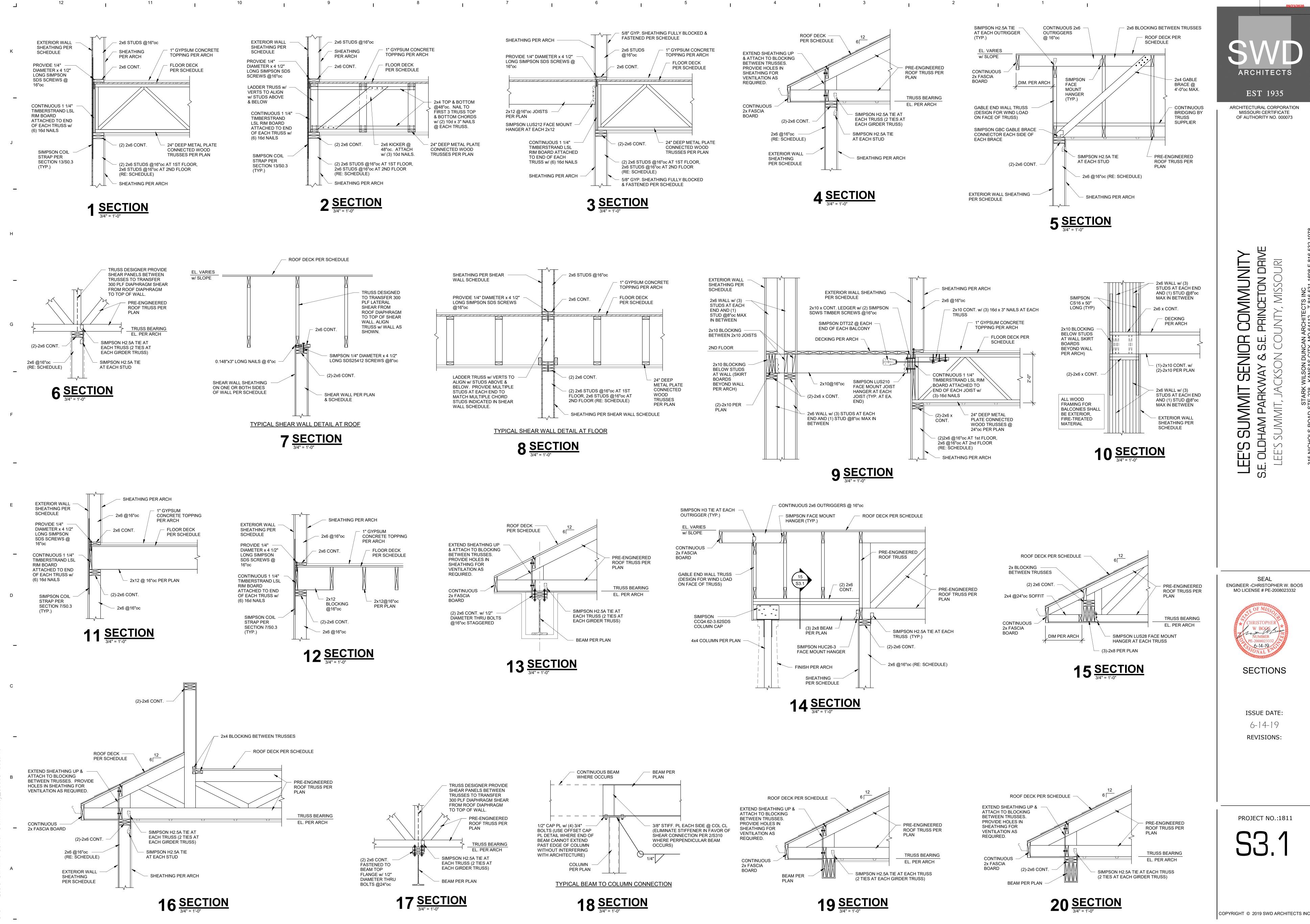
TYPICAL INTERIOR SPREAD FOOTING AT END OF SHEAR WALL

15 <u>SECTION</u>

DEVELOPMENT SERVICES LEE'S SUMMIT, MISSOURI

RELEASE FOR CONSTRUCTION AS NOTED ON PLANS REVIEW



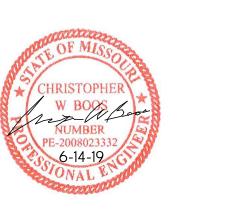


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RELEASE FOR

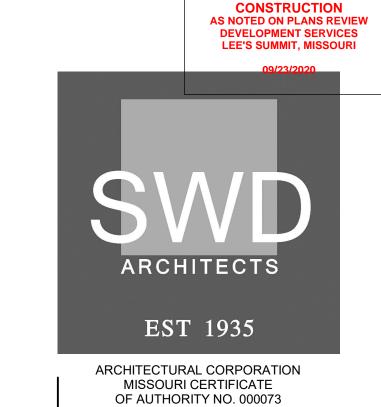
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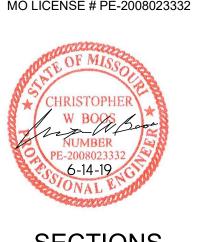
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RELEASE FOR

LEE'S SUMMIT, JACKSON COUNTY, MISSOURI

SEAL
ENGINEER -CHRISTOPHER W. BOOS
MO LICENSE # PE-2008023332

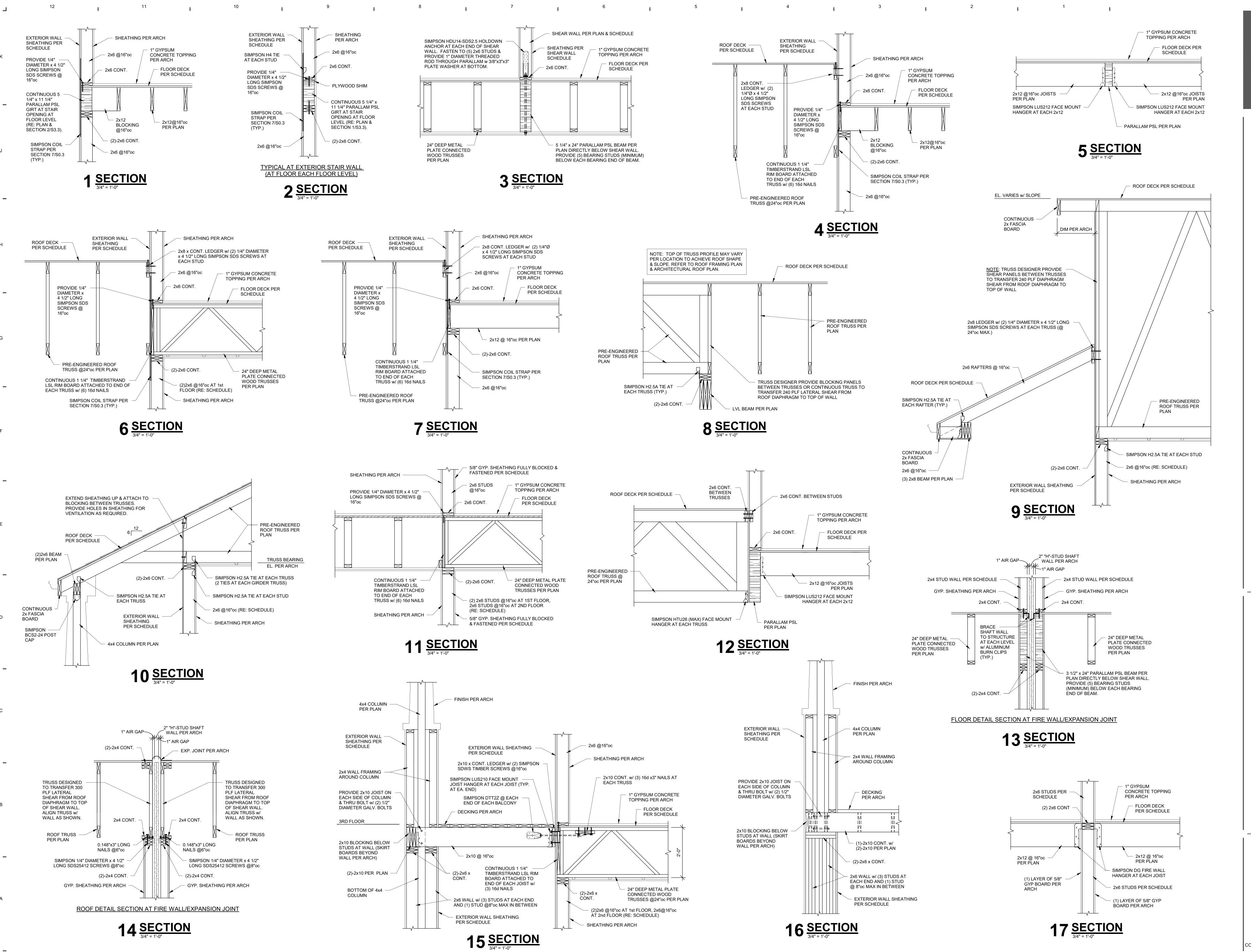


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6-14-19 REVISIONS:

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S3.2

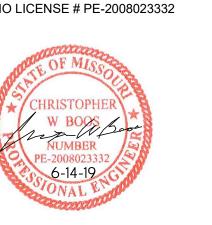




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COMMUNIT SENIOR SUMMIT SHAM PARKW က

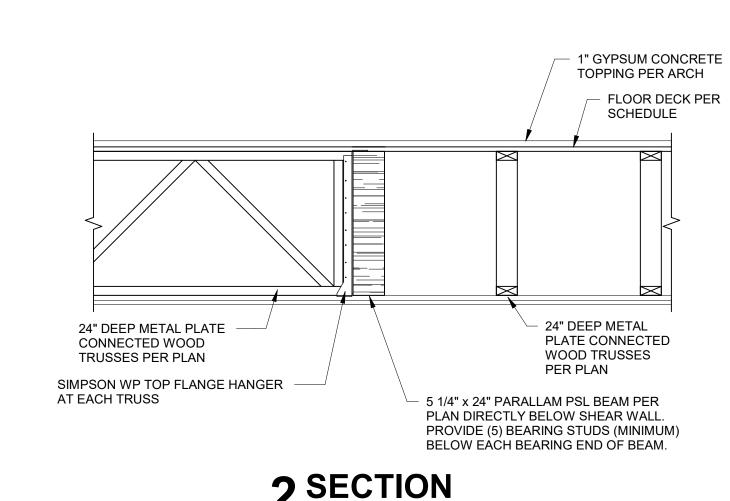
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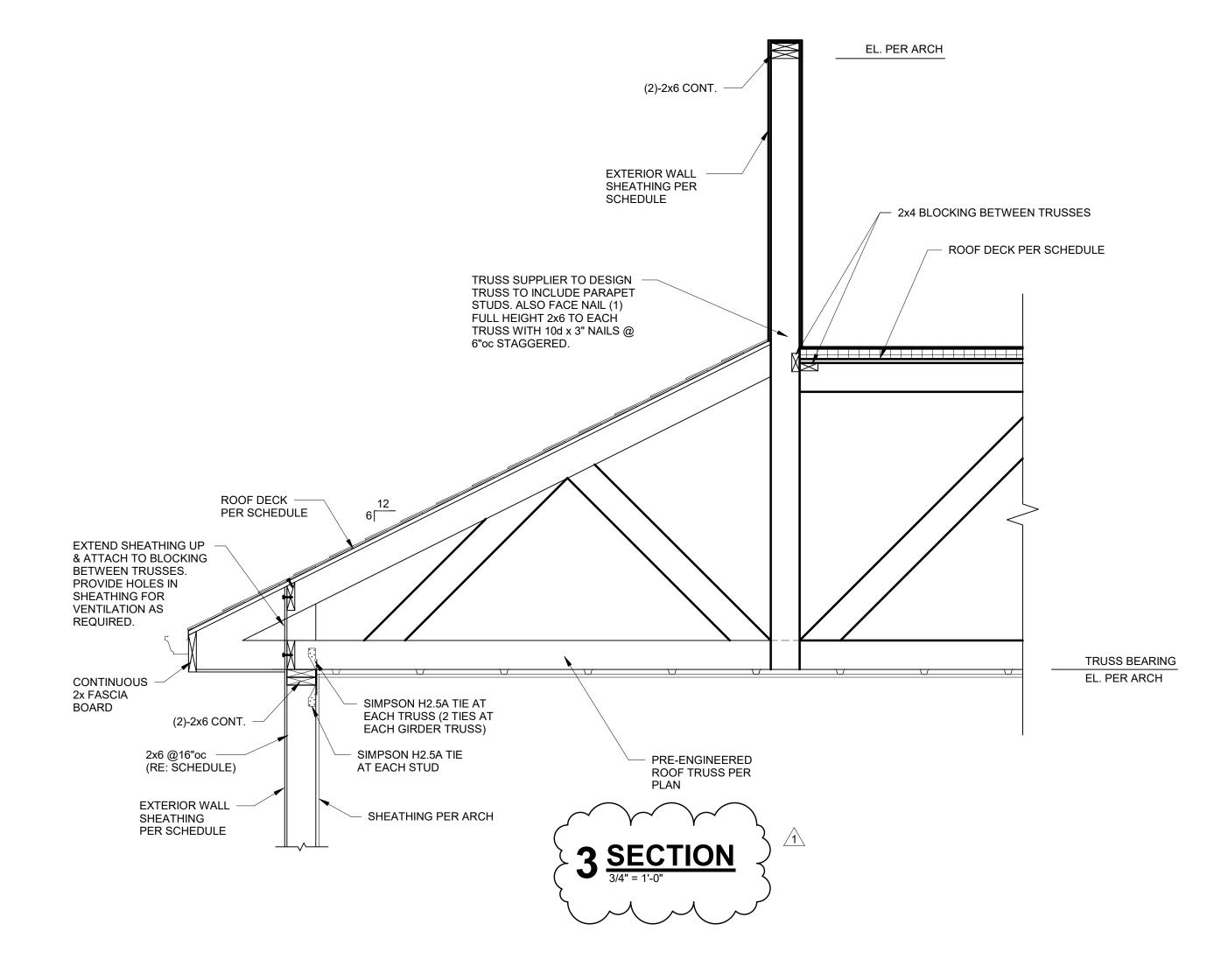


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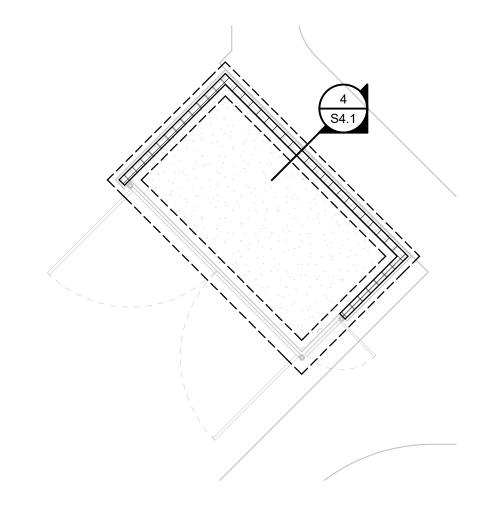


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REVISIONS:
1 Revision#1 7-26-19

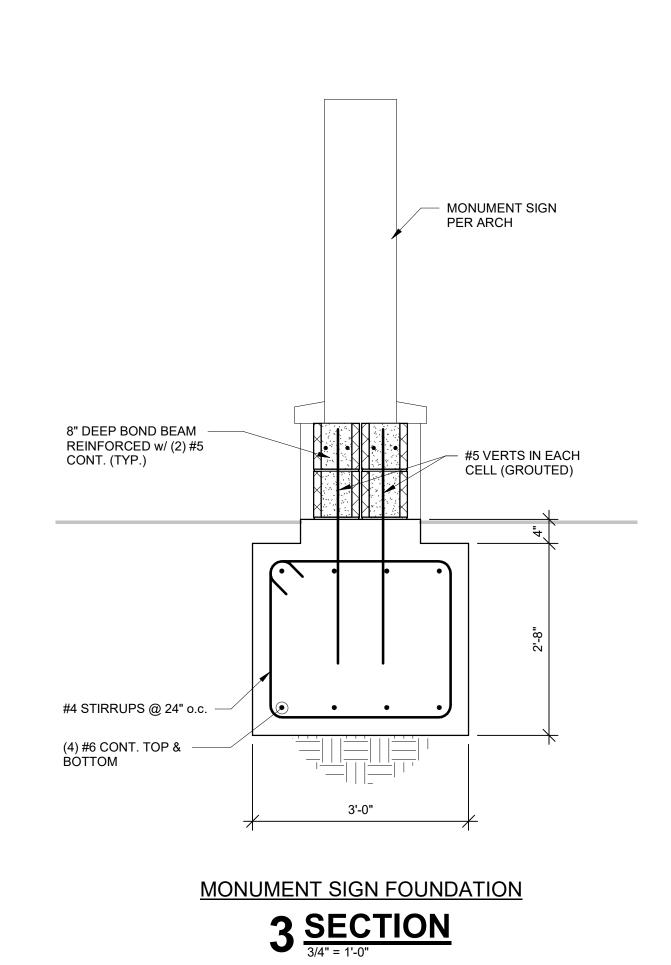
PROJECT NO.:1811

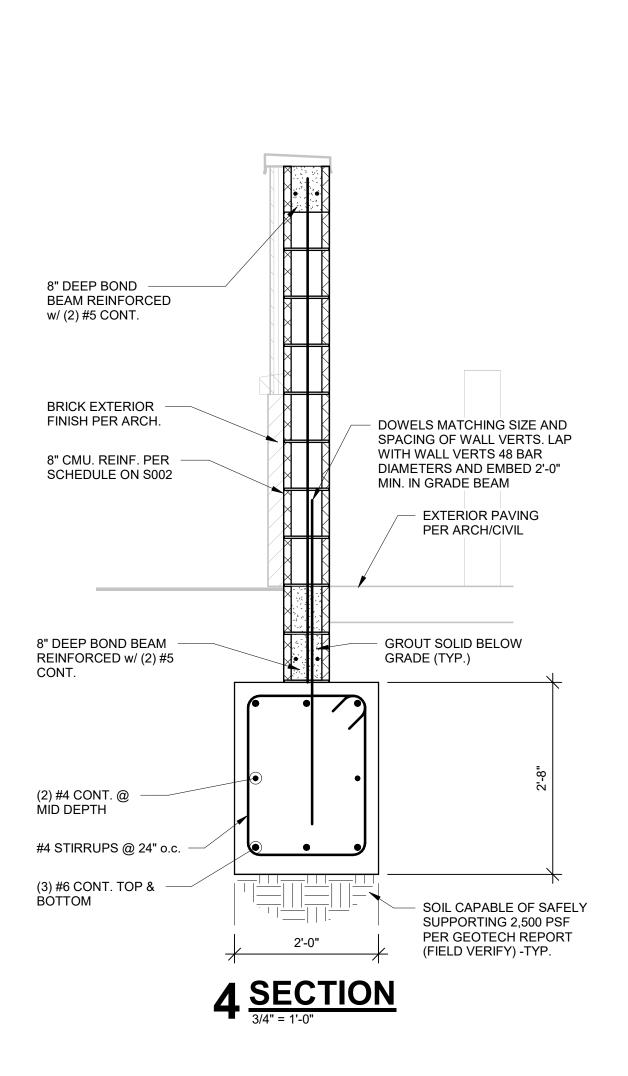
S3.4

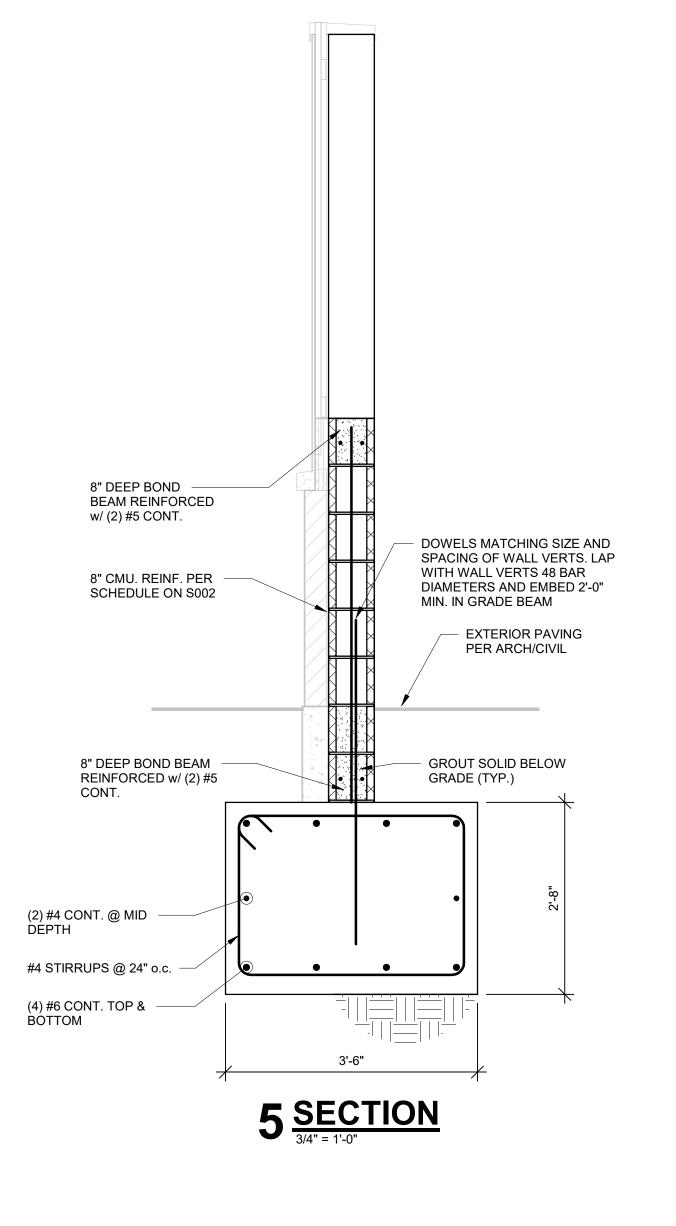


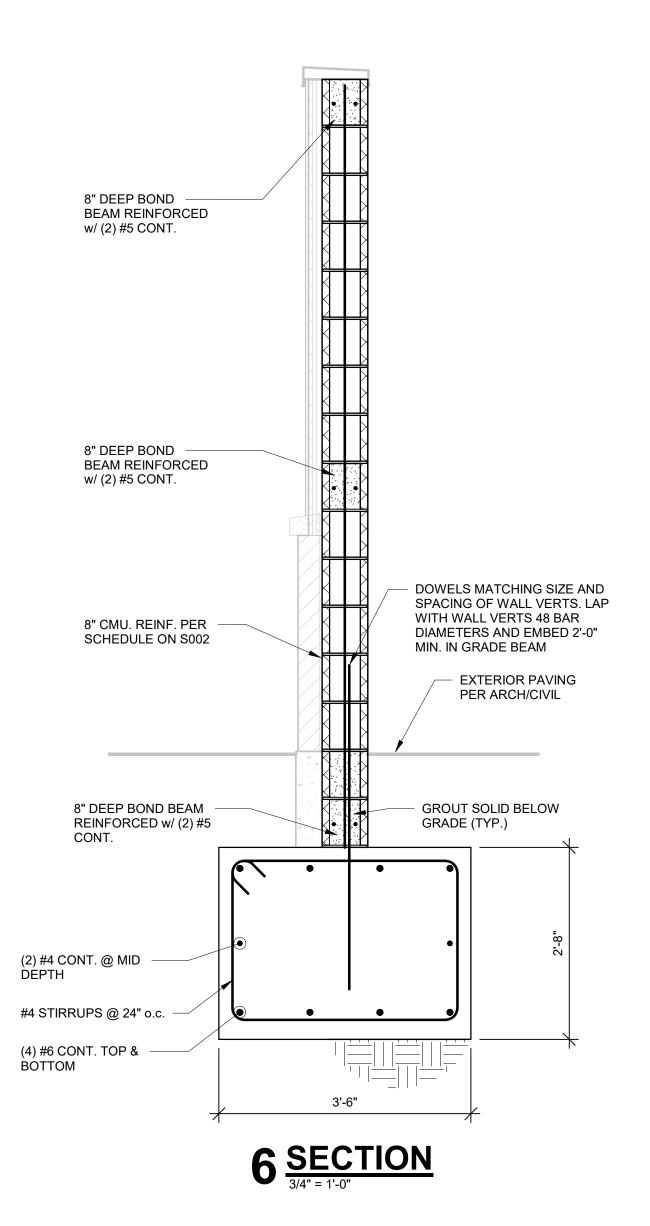
2 WEST TRASH ENCLOSURE PLAN 1/8" = 1'-0"

1 GENERATOR ENCLOSURE & EAST TRASH ENCLOSURE PLAN







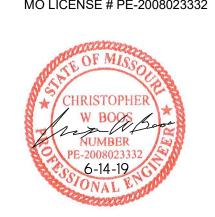




ARCHITECTURAL CORPORATION MISSOURI CERTIFICATE OF AUTHORITY NO. 000073

IMMIT SENIOR COMMUNITY A PARKWAY & S.E. PRINCETON DRIVE AMIT JACKSON COUNTY MISSOURI

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ENGINEER -CHRISTOPHER W. BOOS
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TRASH ENCLOSURE & GENERATOR ENCLOSURE

ISSUE DATE:
6-14-19
REVISIONS:

PROJECT NO.:1811

S4.1