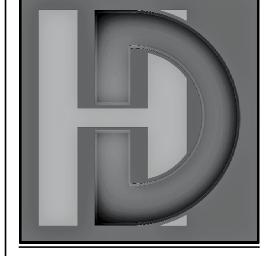
ROOF AND SOFFIT VENTS PER CODE VERIFY WALL BEARING HEIGHTS AND WINDOW HEADERS IN BOTH PLAN AND ELEVATION!

SQUARE FOOTAGE SUMMARY:	
MAIN FLOOR FINISH	1,800 SF
LOWER FINISH AREA	1,100 SF
LOWER FLOOR SLAB	1,760 SF
GARAGE AREA	706 SF
GARAGE SLAB	642 SF
FRONT PORCH	28 SF
REAR DECK	172 SF
Unfinished Basement	660 SF

BRACKET DETAIL

SCALE: 1"=1"-0"

RY:		
	1,800 SF	
	1,100 SF	
	1,760 SF	
	706 SF	
	642 SF	
	28 SF	
	172 SF	
nt	660 SF	



THIS DOCUMENT CONTAINS
COPYRIGHTED MATERIAL AND
CONFIDENTIAL INFORMATION
BELONGINING TO HD ENGINEERING.
UNAUTHORIZED USE, DISCLOSURE,
DISSEMINATION, OR DUPLICATION OF
ANY OF THE INFORMATION
CONTAINED HEREIN MAY RESULT IN
LIABILITY UNDER APPLICABLE LAW.



39707

08/04/2020

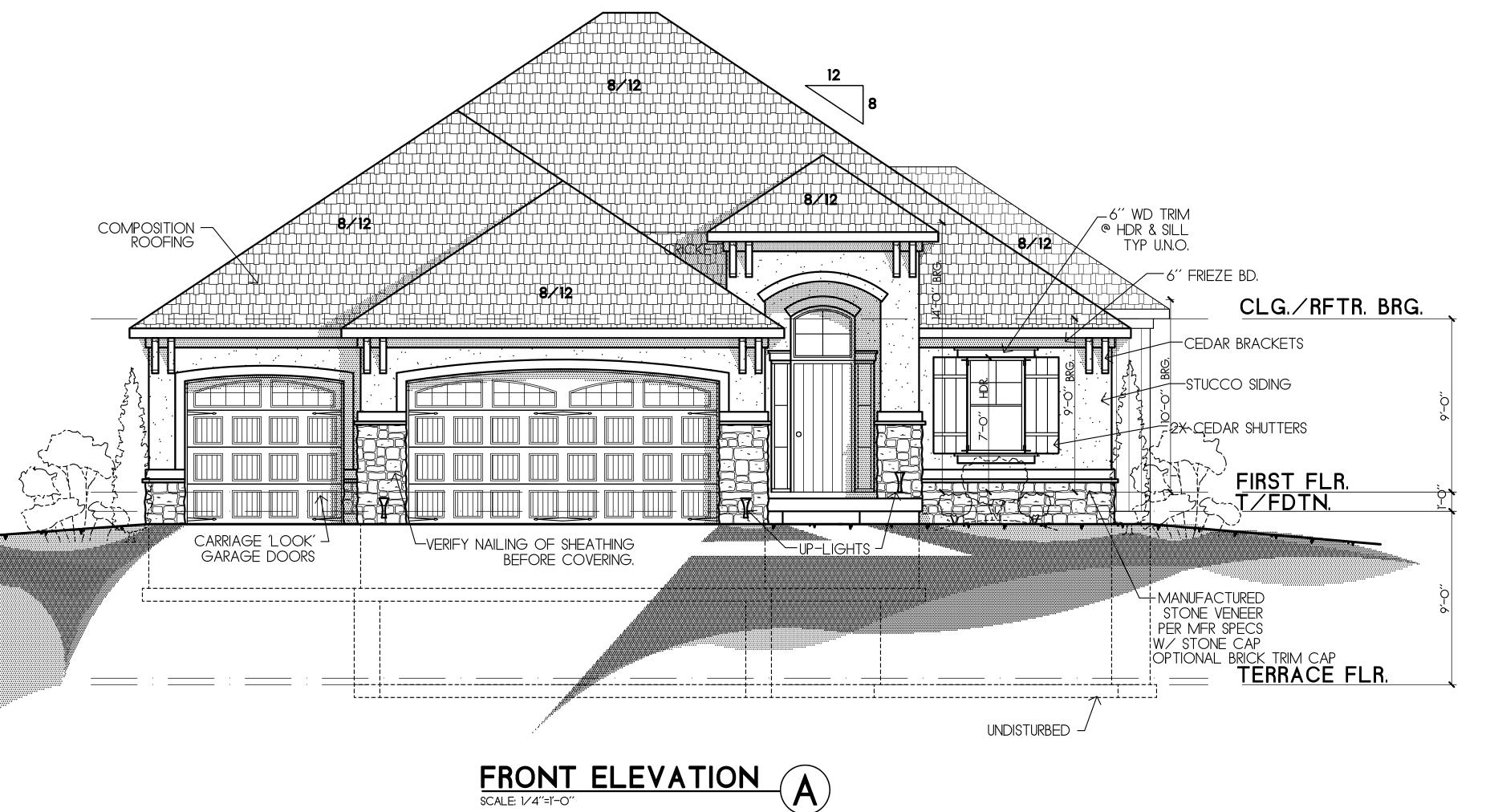
- 1			
		CHECKED BY:	CLS
	NO.	ISSUE/REVISION	Revision Date
- 1			

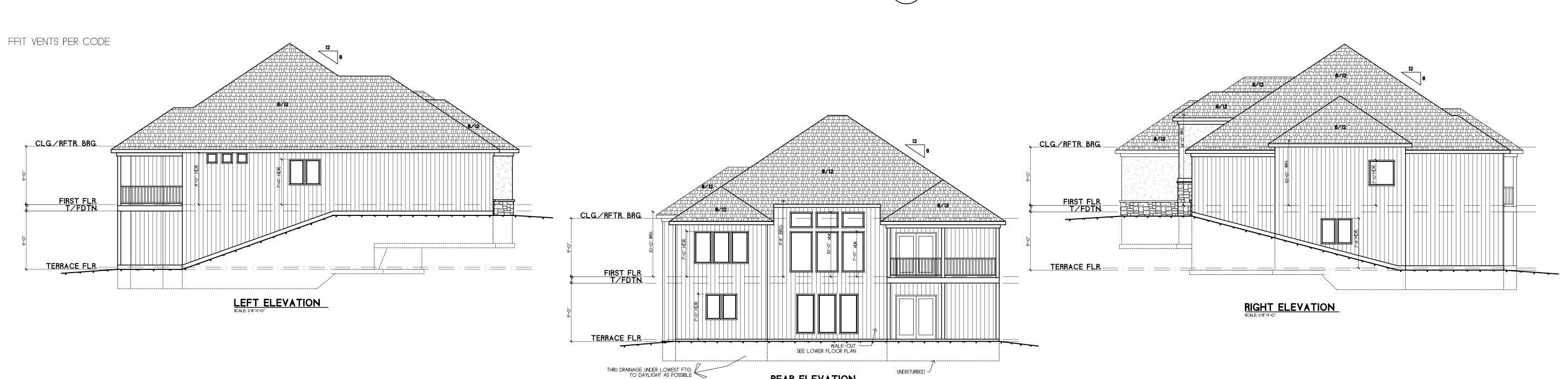
DATE:

PLANS DRAWN BY OTHERS

DRAWN BY: TPM
CHECKED BY: TPM
DATE: 7/30/2020
SCALE: AS NOTED
FILE NAME:
SAB-E716-Ellenbrook-l.aec

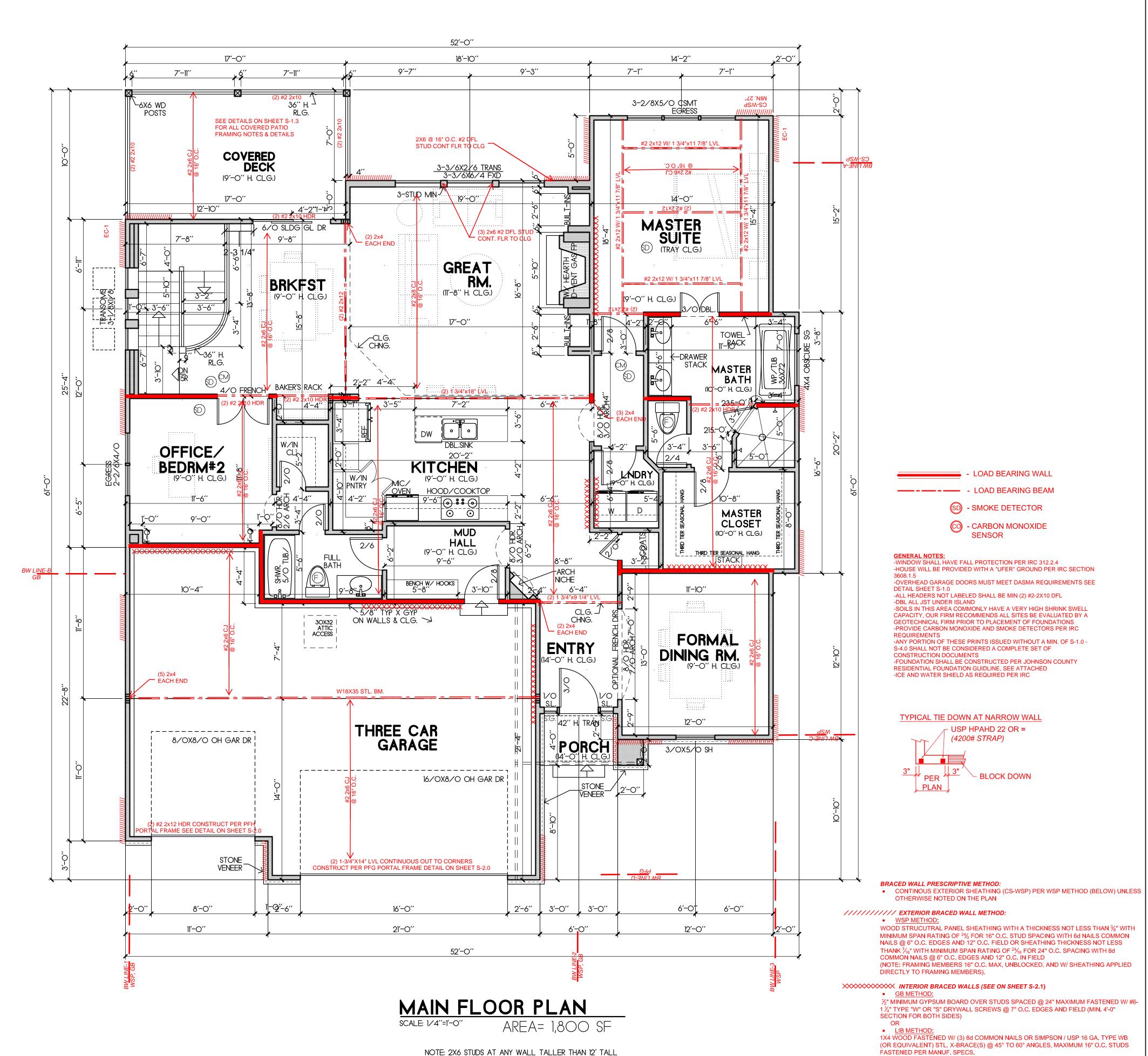
© 2020 HD ENGINEERING & DESIGN





REAR ELEVATION
SCALE 1/8"=1"-O"

UNDISTURBED -



THIS DOCUMENT CONTAINS COPYRIGHTED MATERIAL AND CONFIDENTIAL INFORMATION BELONGINING TO HD ENGINEERING DISSEMINATION, OR DUPLICATION OF ANY OF THE INFORMATION CONTAINED HEREIN MAY RESULT IN LIABILITY UNDER APPLICABLE LAW.







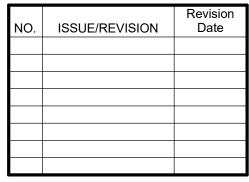
0

SRO SD., I

39707

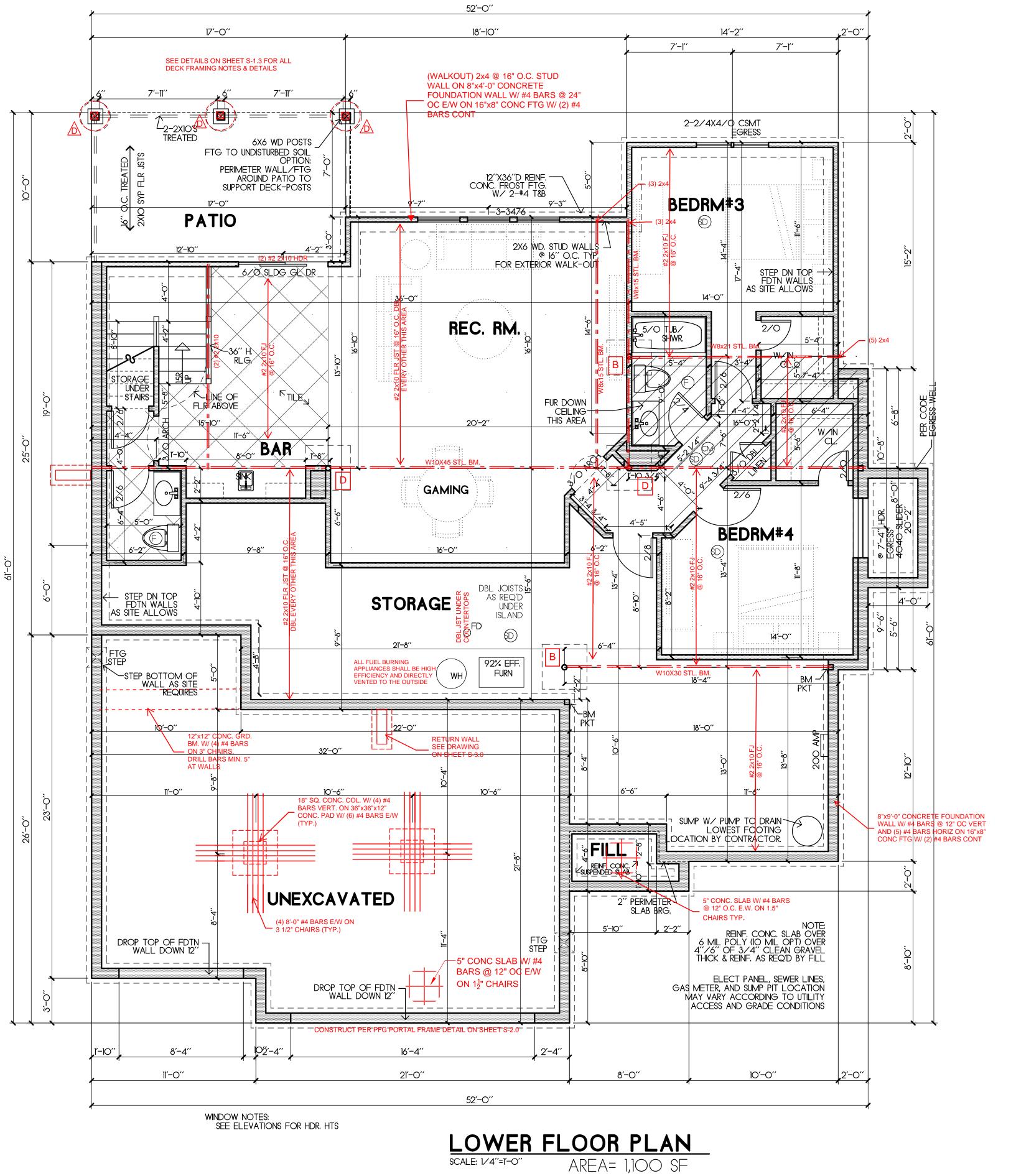
AB

08/04/2020 DATE: CHECKED BY: CLS



PLANS DRAWN BY OTHERS

© 2020 HD ENGINEERING & DESIGN



DECK PIER SCHEDULE

MIN. 6X6 TRTD/CDR POST ON 12" CONC PIER WITH USP PAU 66 BASE OR = (1177# MAX)

MIN. 6X6 TRTD/CDR POST ON 16" CONC PIER WITH USP PAU 66 BASE OR = (2050# MAX)

MIN. 6X6 TRTD/CDR POST ON 18" CONC PIER WITH USP PAU 66 BASE OR = (2649# MAX)

MIN. 6X6 TRTD/CDR POST ON 24" CONC PIER

WITH USP PAU 66 BASE OR =(4710# MAX) PIERS TO TERMINATE ON ORIGINAL SOIL OF 1500

PIERS TO TERMINATE AT A POINT 36" MINIMUM BELOW FINISH GRADE.

-POST ARE NOT TO EXCEED AN UNBRACED LENGTH
OF 12' WITHOUT CONTACTING HD ENGINEERING
FOR GUIDANCE.

COLUMN PAD SCHEDULE

A 3" SCH. 40 STL. COL. ON 30"x30"x12" CONC. PAD W/ (5) #4 BARS E.W. (9.4K MAX.)

B 3" SCH. 40 STL. COL. ON 36"x36"x12" CONC. PAD W/ (6) #4 BARS E.W. (13.5K MAX.)

C 3" SCH. 40 STL. COL. ON 42"x42"x12" CONC. PAD W/ (7) #4 BARS E.W. (18.4K MAX.)

D 3 1/2" SCH. 40 STL. COL. ON 48"x48"x12" CONC. PAD W/ (8) #4 BARS E.W. (24K MAX.)

B 3 1/2" SCH. 40 STL. COL. ON 54"x54"x16" CONC. PAD W/ (9) #4 BARS E.W. (30.4K MAX.)

F 3 1/2" SCH. 40 STL. COL. ON 60"x60"x16" CONC. PAD W/ (10) #4 BARS E.W. (37.5K MAX.)

1. COLUMN AND PIER PAD SIZES SHOWN ARE FOR MAX. 1. COLUMN AND PIER PAD SIZES SHOWN ARE FOR MAX.
COLUMN HEIGHT OF 10'-0" TALL.
2. COLUMN AND PIER PAD SIZES SHOWN ARE BASED ON
AN ASSUMED 1500 PSF. THIS IS THE CAPACITY REQUIRED
BY AHJ, UNDERLINED GENERAL NOTES ON S-1.0 FOR
MORE DETAILS.
3. ALL STEEL COLUMNS SHALL BE ISOLATED FROM SLABS
WITH APPROVED ISSOLATION DEVICE OR JOINT.

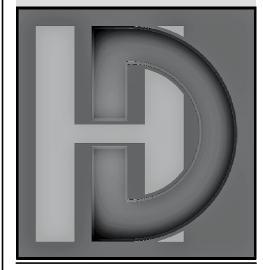
GENERAL NOTES:
-WINDOW SHALL HAVE FALL PROTECTION PER IRC 312.2.4
-HOUSE WILL BE PROVIDED WITH A "UFER" GROUND PER IRC SECTION -OVERHEAD GARAGE DOORS MUST MEET DASMA REQUIREMENTS SEE DETAIL SHEET S-1.0 -ALL HEADERS NOT LABELED SHALL BE MIN (2) #2-2X10 DFL -DBL ALL JST UNDER ISLAND -SOILS IN THIS AREA COMMONLY HAVE A VERY HIGH SHRINK SWELL CAPACITY, OUR FIRM RECOMMENDS ALL SITES BE EVALUATED BY A GEOTECHNICAL FIRM PRIOR TO PLACEMENT OF FOUNDATIONS -PROVIDE CARBON MONOXIDE AND SMOKE DETECTORS PER IRC -ANY PORTION OF THESE PRINTS ISSUED WITHOUT A MIN. OF S-1.0 -S-4.0 SHALL NOT BE CONSIDERED A COMPLETE SET OF CONSTRUCTION DOCUMENTS -FOUNDATION SHALL BE CONSTRUCTED PER JOHNSON COUNTY RESIDENTIAL FOUNDATION GUIDLINE, SEE ATTACHED -ICE AND WATER SHIELD AS REQUIRED PER IRC

> TYPICAL TIE DOWN AT NARROW WALL (4200# STRAP) - BLOCK DOWN

> > PLAN

COPYRIGHTED MATERIAL AND CONFIDENTIAL INFORMATION BELONGINING TO HD ENGINEERIN DISSEMINATION, OR DUPLICATION OF ANY OF THE INFORMATION CONTAINED HEREIN MAY RESULT IN LIABILITY UNDER APPLICABLE LAW.

THIS DOCUMENT CONTAINS





RD., I

AB

39707 08/04/2020 DATE: CHECKED BY: CLS

ISSUE/REVISION

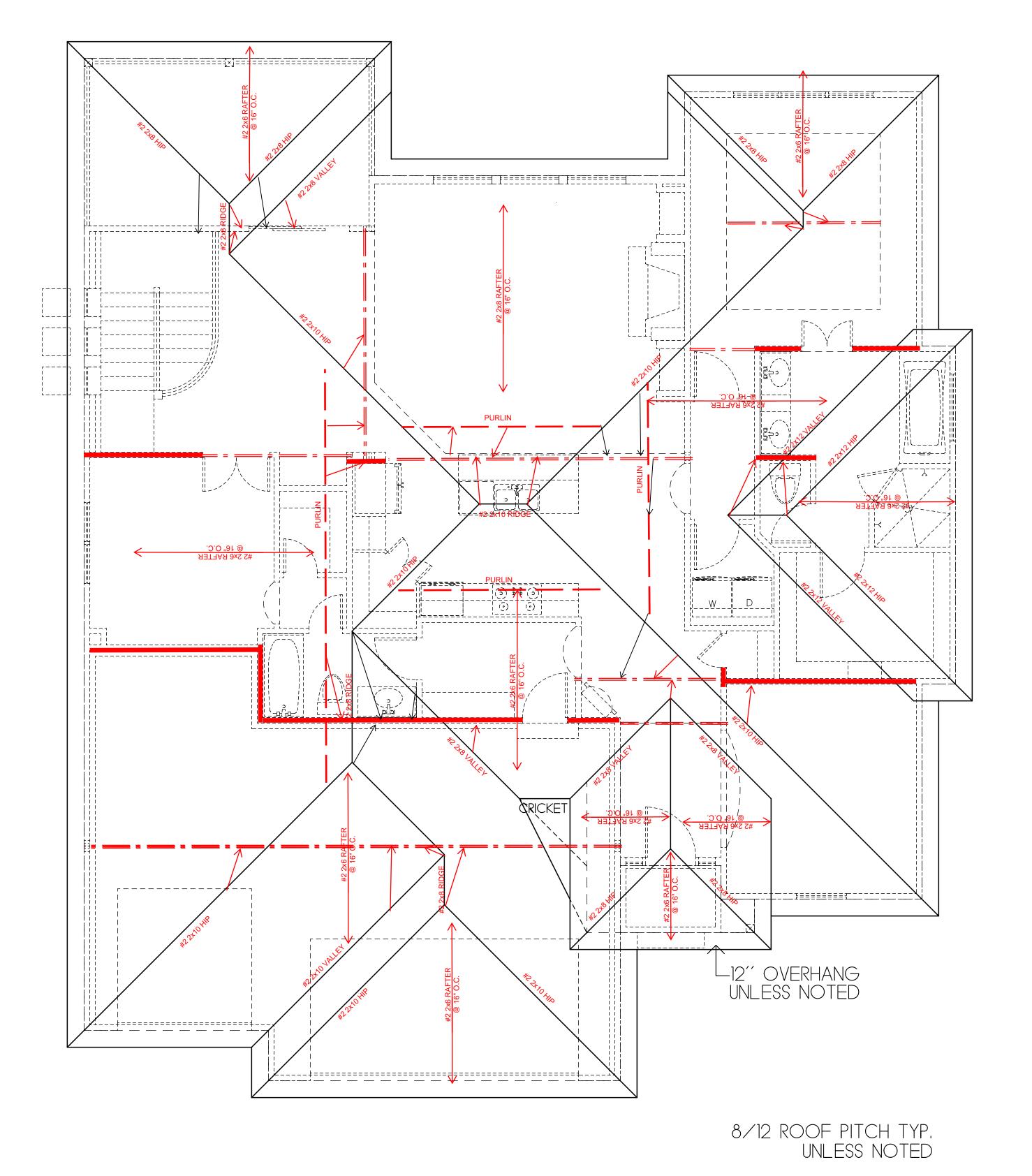
PLANS DRAWN BY OTHERS

DRAWN BY: TPM CHECKED BY: TPM

DATE: 7/30/2020 SCALE: AS NOTED

FILE NAME: SAB-E716-Ellenbrook-1.aec

© 2020 HD ENGINEERING & DESIGN



NOTES

RAFTERS (DOUG-FIR, OR EQUAL):

CODE IVITATIVIONI		
RAFTERS	SPACING	MAX HORIZONTAL CLEARSPAN
#2-2x6	@24" O.C.	11'-11"
#2-2x6	@16" O.C.	14'-1"
#2-2x8	@24" O.C.	15'-1"
#2-2x8	@16" O.C.	18'-5"
#2-2x10	@24" O.C.	18'-5"
#2-2x10	@16" O.C.	22'-6"

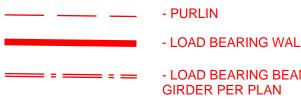
GREATER THAN CODE

ONLATER HARR	ODL	
RAFTERS	SPACING	MAX HORIZONTAL CLEARSP
#2 - 2x6	@24" O.C.	8'-6"
#2 - 2x6	@16" O.C.	9'-9"
#2-2x8	@24" O.C.	11'-3"
#2-2x8	@16" O.C.	12'-9"
#2-2x10	@24" O.C.	14'-3"
#2-2x10	@16" O.C.	16'-3"

DEFLECTION = L/360 LIVE LOAD, L/240 TOTAL LOAD

PURLIN STRUTS ARE AT 4'-0" O.C. PURLIN STRUTS SHALL BE INSTALLED AT NOT LESS THAN A 45 DEGREE ANGLE WITH THE HORIZONTAL ALL PURLINS STRUTS SHALL HAVE A MAXIMUM UNBRACED LENGTH OF 8'-0" PURLINS STRUTS SHALL BE CONSTRUCTED IN A "T"

PURLIN STRUT	MAX PURLIN STRUT LENGTH
(2) 2x4	8'-0"
(1) 2x4 & (1) 2x6	12'-0"
(1) 2x6 & (1) 2x8	20'-0"
(2) 2x6 & (1) 2x8	30'-0"
CONSULT ARCH./ENGR.	>30'-0"



ROOF DESIGNED FOR LIGHT ROOF COVERING 30PSF TOTAL LOAD [10PSF DL, 20PSF LL (SL)]

SEE SPAN CHARTS BELOW

CODE MINIMUM

RAFTERS	SPACING	MAX HORIZONTAL CLEARSPAI
#2-2x6	@24" O.C.	11'-11"
#2-2x6	@16" O.C.	14'-1"
#2-2x8	@24" O.C.	15'-1"
#2-2x8	@16" O.C.	18'-5"
#2-2x10	@24" O.C.	18'-5"
#2-2x10	@16" O.C.	22'-6"
NOTE: CODE MININ	MUM L/240 DEFLECT	ION

#2-2X10 @16" O.C.

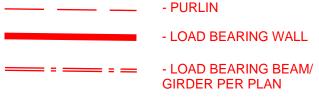
VAULTS TO BE 2x10 DEPTH

ALL RIDGES, HIPS, AND VALLEYS NOT MARKED SHALL BE (1) NOMINAL SIZE LARGER THAN THE INTERSECTING RAFTERS

PURLINS ARE 2x6 MIN.

CONFIGURATION AND PER THE FOLLOWING CHART

SEE DETAILS 1, 5, 6, 7, 11, 12, 13, & 14 ON S-1.2 FOR ROOF FRAMING AND INSULATION OPTIONS



39707

08/04/2020 DATE: CHECKED BY: CLS

COPYRIGHTED MATERIAL AND CONFIDENTIAL INFORMATION BELONGINING TO HD ENGINEERING. UNAUTHORIZED USE, DISCLOSURE, DIS

DISSEMINATION, OR DUPLICATION OF ANY OF THE INFORMATION CONTAINED HEREIN MAY RESULT IN LIABILITY UNDER APPLICABLE LAW.

NO.	ISSUE/REVISION	Revision Date

PLANS DRAWN BY OTHERS

© 2020 HD ENGINEERING & DESIGN

ROOF FRAMING PLAN
SCALE: 1/4"=1'-0"

DRAWN BY: TPM
CHECKED BY: TPM
DATE: 7/30/2020
SCALE: AS NOTED
FILE NAME:
SAB-E716-Ellenbrook-1.aec

ALLOWABLE LOADS FOR PNEUMATIC OR **MECHANICALLY DRIVEN NAILS AND STAPLES**

	NAIL GUN		PENETRATION	AL	LOWABLE LO	E LOADS (IN POUNDS)		
FASTENER DESCRIPTION	NAILS/	WIRE GA.	REQUIRED INTO MAIN MEMBER FOR LATERAL	LATERAL STRENGTH		WITHDRAWAL STRENGT		
	WIRE DIA.		STRENGTH (IN.)	SP	DF/L	SP	DF/L	
16 GA. STAPLE	.063	16	1	51		36	32	
15 GA. STAPLE	.072	15	1	64		42	37	
14 GA. STAPLE	.080	14	1	75		46	41	
6d COOLER NAIL	.092	13	1	46		27	23	
6d SINKER NAIL	.002	10	'	40		21	25	
6d BOX NAIL								
6d CASING NAIL	.099	12-1/2	1-1/8	61	55	31	24	
7d COOLER NAIL								
6d COMMON NAIL								
8d COOLER NAIL								
8d SINKER NAIL	.113	11-1/2	1-1/4	79	72	35	28	
8d BOX NAIL								
8d CASING NAIL								
6d RING SHANK NAIL								
6d SCREW SHANK NAIL	.120	11	1-3/8	89	81	41	32	
8d RING SHANK NAIL			1 0/0					
8d SCREW SHANK NAIL								
10d Cooler Nail								
10d Sinker Nail	.128	10-1/2	1-1/2	89	81	36	31	
12d Short								
10d Box Nails								
12d Box Nails	.128	10-1/2	1-1/2	101	93	40	31	
10d Casing Nails								
8d Common Nails	131	.131 10-1/4 1-	1-1/2	1/2 106	97	41	32	
16d Short	.101	10 1/4	1 1/2	100		71	02	
12d Sinkers	.135	10	1-1/2	113	103	42	33	
16d Box Nails	.100	10	1-1/2	110	100	72	33	
10d Ring Shank Nails								
10d Screw Shank Nails	.135	10	1-5/8	113	103	46	36	
12d Ring Shank Nails			1-5/6	110		40		
12d Screw Shank Nails								
10d Common Nails								
12d Common Nails								
16d Sinker Nails	.148	9	1-5/8	128	118	46	36	
20d Box Nails								
30d Box Nails								
16d Ring Shank Nails	.148	9	1-3/4	128	118	50	40	
16d Screw Shank Nails	.170		. 5/7	120	110			
16d Common Nails	.162	8	1-3/4	154	141	50	40	
40d Box Nails			. 57.		141	30	40	
20d Ring Shank Nails	.177	7	2-1/8	178	163	59	47	
20d Screw Shank Nails								
20d Sinker Nails	.177	7	2-1/8	178	163	54	43	
20d Common Nails	.148	9	2-1/8	170	166	59	47	
30d Sinker Nails		9	2 1/0	170	100] 38	41	

SHEATHING SCHEDULE

ALL SHEATHING MATERIALS TO BE APPLIED PERPENDICULAR TO JOISTS AND ENDS STAGGERED

BUILDING COMPONENT	MATERIAL	FASTENING
ROOF SHEATHING	7/16" PLYWOOD	16 GA X 1 3/4" STAPLES @ 6" OC EDGES & 12" OC IN FIELD
NOOF OFFERTING	1x 4 #3 FURRING	1/2" CROWN STAPLES
FLOOD SUFATUING	### THING	
FLOOR SHEATHING	PINE PLYWOOD	
WALL COVERING		
CEILING COVERING		HEAD; $0.098~\varnothing$, $1~1/4$ " LONG, ANG-RINGED; 5D COOLER NAIL, $0.086~\varnothing$, $1~5/8$ " LONG, $15/64$ " HEAD; OR GYP BD NAIL,
EXTERIOR WALL	7/16" APA RATED SHEATHING	8D COMMON NAILS @ 6" OC EDGES & 12" OC IN THE FIELD
SHEATHING	RATED PANEL SIDING, RATED 16" O.C. 7/16" THICK	8D BOX OR SINKER NAILS @ 6" OC EDGES & 12" OC IN THE FIELD

FRAME FASTENING SCHEDULE

BUILDING COMPONENT	FASTEN TO	FASTEN WITH
	RIDGE / VALLEY / HIP	TOENAIL W/ (4) 16D, FACENAIL W/ (3)
DAETEDS	PLATE	TOENAIL W/ (3) 10D
RAFTERS	LEDGER STRIPS SUPPORTING JOISTS OR RAFTERS	FACENAIL W/ (3) 16D
	COLLAR TIE TO RAFTERS	FACENAIL W/ (3) 10D
	TOP PLATE	TOENAIL W/ (3) 8D @ EACH END
CEILING JOISTS	WHERE CLG JST RUN PARALLEL TO RAFTERS FAC	ENAIL TO RAFTERS W/ (3) 10D MINIMUM
CEILING JOIS 13	LAPS OVER PARTITIONS	FACENAIL W/ (3) 10D
	BLOCKING BETWEEN JOISTS OR RAFTERS TO TOP PLATE	TOENAIL W/ (3) 8D
	BUILT-UP BEAMS, 2" LUMBER LAYERS, FACENAIL OPPOSITE SIDES, (2) @ EACH END PLUS	10D @ 32" OC STAGGERED, TOP & BOTTOM, OPPOSITE SIDES
BEAMS	BUILT-UP BEAMS OF ENGINEERED LUMBER, FACE NAIL OPPOSITE SIDES	(2) ROWS @ 12" OC
	BUILT-UP HEADER, TWO PIECES W/ 1/2" SPACER	16D @16" OC ALONG EDGES
	BUILT-UP HEADER, TWO PIECES, NO 1/2" SPACER	3" x 0.131" NAILS @ 12" OC ALONG EDO
	BEARING	TOENAIL W/ (2) 18D @ EACH END
	RIM JOIST TO SILL OR TOP PLATE	TOENAIL W/ 8D COMMON OR 10D BC NAILS @ 6" OC
FLOOR JOISTS	JOIST TO SILL OR GIRDER	TOENAIL W/ (3) 8D
	JOIST TO RIM JOIST	FACENAIL W/ (3) 16D
	BRIDGING TO JOIST	TOENAIL W/ (2) 8D
	I-JOIST TO BEARING PLATE	TOENAIL W/ (2) 8D - ONE INTO EACH SIE LEAST 1 1/2" FROM THE END
	RIM JOIST TO I-JOIST	FACENAIL W/ (2) 10D BOX NAILS - ONE EACH FLANGE
	SOLE PLATE TO LSL RIM BOARD	16D BOX NAILS @ 12" OC
	SINGLE JOIST HANGERS *	10D FACENAILS AND TOENAILS
	DOUBLE JOIST HANGERS *	16D FACENAILS AND TOENAILS
	TOP & SOLE PLATE TO STUD	END NAIL W/ (2) 16D
	STUD TO SOLE AND TOP PLATE	TOENAIL W/ (4) 8D
	DOUBLE TOP PLATES	FACENAIL W/ 16D @ 16" OC
	DOUBLE TOP PLATE LAP SPLICE	FACENAIL W/ (8) 16D
	TOP PLATE LAPS & INTERSECTIONS	FACENAIL W/ (2) 16D
	DOUBLE STUDS	FACENAIL W/ 16D @ 24" OC
	BUILT-UP CORNER STUDS	FACENAIL W/ 16D - 2 ROWS @ 24" O
	STEEL "X" BRACING	FACENAIL W/ (2) 16D IN EACH TOP 8 BOTTOM PLATE & (1) 8D PER STUD
WALLS	SOLE PLATE TO JOIST OR BLOCKING	FACENAIL W/ 16D @ 16" OC
	SOLE PLATES TO JOIST OR BLOCKING AT BRACED WALL LINES, PERPENDICULAR TO FRAMING	FACENAIL W/ (3) 16D @ 16" OC ALON BRACED WALL PANEL
	TOP PLATE TO JOIST OR BLOCKING AT BW LINES, PERPENDICULAR TO FRAMING	TOENAIL W/ 8D @ 6" OC ALONG BRACED WALL PANEL
	SOLE PLATES TO JOIST OR BLOCKING AT BW LINES PARALLEL TO FRAMING, BLOCKING @ 16" OC	FACENAIL W/ (3) 16D @ 16" OC ALONG PANEL & AT EACH BLOCK
	TOP PLATE TO JOIST OR BLOCKING AT BW LINES, PARALLEL TO FRAMING, BLOCKING @ 16" OC	TOENAIL W/ 8D @ 6" OC ALONG BW PANEL & AT EACH BLOCK
	NON-STRUCT. SIDING OVER STRUCT. SHEATHING	(1) 6D BOX NAIL IN EACH STUD
	FIBER CEMENT PLANK SIDING	(1) 6D GALVANIZED NAIL IN EACH STU
	WINDOW INSTALLATION NAILING	1 3/4" - 2" ROOFING NAILS @ 12" OC M

* JOIST HANGER NOTES: 1) NO JOIST HANGER NAILS ALLOWED FOR TOENAILS, 2) NO GUN NAILS OR SCREWS ALLOWED IN

CONNECTORS, 3) TOENAILS SHALL ALWAYS BE A FULL 3" OR 3.5" NAIL

COLUMN CONNECTION TO STEEL BEAMS SHALL BE WITH A CLIP POST CAP WITH ALL FOUR TAB EARS BENT AROUND THE BOTTOM FLANGE OF THE BEAM. FOR A BEARING PLATE, FOUR HOLES SHALL BE DRILLED IN THE BOTTOM FLANGE OF THE STEEL BEAM TO MATCH THE HOLE PATTERN OF THE PLATE. 1/2"x2" BOLTS SHOULD THEN BE INSTALLED WITH A FLAT WASHER, LOCK WASHER, AND A NUT IN EACH OF THE HOLES. THE POST CAP MAY BE WELDED TO THE STEEL BEAM IN ACCORDANCE WITH AWS D1.1-92 AS AN ALTERNATIVE, AND WOULD NEED TO BE INSPECTED BY AN AWS-CERTIFIED

DUCT SEALING METHOD, PER IRC2018 W1103.3.2

N1103.2.2 (R403.2.2) SEALING (MANDATORY) DUCTS, AIR HANDLERS, AND FILTER BOXES SHALL BE SEALED. JOINTS AND SEAMS SHALL COMPLY WITH SECTION M1601.4.1 OF THIS CODE.

1. AIR-IMPERMEABLE SPRAY FOAM PRODUCTS SHALL BE PERMITTED TO BE APPLIED WITHOUT ADDITIONAL JOINT

ADDITIONAL CLOSURE SYSTEMS.

2. WHERE A DUCT CONNECTION IS MADE THAT IS PARTIALLY INACCESSIBLE, THREE SCREWS OR RIVETS SHALL BE

EQUALLY SPACED ON THE EXPOSED PORTION OF THE JOINT SO AS TO PREVENT A HINGE EFFECT. 3. CONTINUOUSLY WELDED AND LOCKING-TYPE LONGITUDINAL JOINTS AND SEAMS IN DUCTS OPERATING AT STATIC PRESSURE LESS THAN 2 INCHES OF WATER COLUMN (500 Pa) PRESSURE CLASSIFICATION SHALL NOT REQUIRE

DUCT TIGHTNESS SHALL BE VERIFIED BY EITHER OF THE FOLLOWING: 1. POST CONSTRUCTION TEST: TOTAL LEAKAGE SHALL NOT BE LESS THAN OR EQUAL TO 4 CFM (113.3 L/MIN) PER

100FT² (9.29m²) OF CONDITIONED FLOOR AREA WHEN TESTED AT A PRESSURE DIFFERENTIAL OF 0.1 INCHES W.G. (25 Pa) ACROSS THE ENTIRE SYSTEM, INCLUDING THE MANUFACTURER'S AIR HANDLER ENCLOSURE. ALL REGISTER BOOTS SHALL BE TAPED OR OTHERWISE SEALED DURING THE TEST. 2. ROUGH-IN TEST: TOTAL AIR LEAKAGE SHALL BE LESS THAN OR EQUAL TO 4 CFM (113.3 L/MIN) PER 100FT2 (9.29m2) OF

CONDITIONED FLOOR AREA WHEN TESTED AT A PRESSURE DIFFERENTIAL OF 0.1 INCHES W.G. (25 Pa) ACROSS THE ENTIRE SYSTEM, INCLUDING THE MANUFACTURER'S AIR HANDLER ENCLOSURE. ALL REGISTERS SHALL BE TAPED OR OTHERWISE SEALED DURING THE TEST. IF THE AIR HANDLER IS NOT INSTALLED AT THE TIME OF THE TEST, TOTAL AIR LEAKAGE SHALL BE LESS THAN OR EQUAL TO 3 CFM (85 L/MIN) PER 100FT² (9.29m²) OF CONDITIONED FLOOR AREA. EXCEPTION: THE TOTAL LEAKAGE IS NOT REQUIRED FOR DUCTS AND AIR HANDLERS LOCATED ENTIRELY WITHIN THE BUILDING THERMAL ENVELOPE.

1. PLANS SHALL COMPLY WITH THE 2018 INTERNATIONAL RESIDENTIAL CODE, 2018 IECC, AND ALL AMENDMENTS AS ADOPTED BY THE AHJ. IF ANY CHANGES OR DEVIATIONS ARE MADE FROM THESE PLANS THE CONTRACTOR SHALL NOTIFY THE APPROPRIATE AUTHORITY AND THE ENGINEER TO EVALUATE THE CHANGES AND MAKE ANY APPROPRIATE MODIFICATIONS TO THE PLANS 2. WHERE DISCREPANCIES EXIST BETWEEN THE STANDARD COMMENTS, NOTES FOR THE DESIGN PROFESSIONAL OR THE CODE, THE MOST RESTRICTIVE SHALL APPLY.

3. THE CONTRACTUAL OBLIGATION OF THESE PLANS IS TO PROVIDE THE OWNER/BUILDER AND THE AHJ WITH A SET OF PLANS THAT MEET AHJ AND CODE REQUIREMENTS FOR A SINGLE SITE CONSTRUCTION PROJECT. UNLESS REQUESTED BY OUR CLIENT, CODE/AHJ MINIMUM DESIGNS WILL BE UTILIZED. ALSO, UNLESS REQUESTED BY THE OWNER, OUR FIRM CAN NOT AND WILL NOT BE AUTHORIZED TO VISIT THE SITE TO EVALUATE THE SITE OR ANY CONSTRUCTION FOR THIS PROJECT. IMPLEMENTATION OF ALTERNATES TO THE DESIGNS INCLUDING BUT NOT LIMITED TO PIER DESIGNS, FOUNDATION ALTERATIONS, OR ANY STRUCTURAL CHANGES NOT PROVIDED BY HD ENGINEERING OR A PROFESSIONAL REFERRED BY HD ENGINEERING SHALL RELEASE HD ENGINEERING FROM ALL LIABILITY ASSOCIATED WITH THIS DESIGN. 4. OUR FIRM HIGHLY RECOMMENDS THAT ANY SITE WITH GREATER THAN A 15% GRADE, ANY SITE WHERE A PREVIOUS STRUCTURE WAS LOCATED, OR ANY SITE WITH POTENTIAL FILL MATERIAL OR A POTENTIAL SOIL BEARING CAPACITY BELOW 1500 PSF SHOULD BE EVALUATED BY OUR FIRM OR AN HD ENGINEERING REFERRED

GEOTECHNICAL FIRM PRIOR TO PLACING FOOTINGS. THE ATTACHED PLANS HAVE BEEN DESIGNED WITH THE UNDERSTANDING THAT OUR FIRM HAS NOT AND CAN NOT VISIT OR INSPECT THE SITE WITHOUT WRITTEN CONSENT/REQUEST OF THE OWNER/BUILDER. DUE TO THIS FACT OUR FIRM CAN ONLY DESIGN THE ATTACHED PLANS TO CERTAIN CODE REQUIREMENTS WHICH ARE DETAILED THROUGHOUT THE PLAN AND ATTACHED DETAIL SHEETS, IF THE OWNER DESIRES GREATER THAN CODE DESIGNS THAT REQUEST MUST BE MADE CLEARLY AND IN WRITING PRIOR TO ENGINEERING OF THE PLAN.

5. DUE TO THE WIDE VARIETY OF SOIL CONDITIONS IN OUR AREA AND THE WIDE VARIETY OF PLASTICITY INDEX AND SOIL BEARING CAPACITIES OUR FIRM RECOMMENDS ALL SITES BE EVALUATED BY HD ENGINEERING OR AN HD ENGINEERING REFERRED GEOTECHNICAL FIRM PRIOR TO PLACEMENT OF ANY "STANDARD" FOUNDATIONS .

. THE FOUNDATION DESIGN SHALL COMPLY WITH THE ENFORCING JURISDICTION RESIDENTIAL FOUNDATION STANDARD IN LIEU OF ENGINEERING REPORT

REQUIREMENTS BASED ON ACTUAL SITE CONDITIONS 2. FOUNDATION WALLS SHALL BE DAMP-PROOFED PER IRC SECTION R406.

3. PROVIDE A MINIMUM 4" PERFORATED DRAIN AROUND USABLE SPACE BELOW GRADE OR OTHER EQUIVALENT MATERIALS PER IRC SECTION 405.1. THE PIPE SHALL BE COVERED WITH NOT LESS THAN 6" OF WASHED GRAVEL OR CRUSHED ROCK. THE DRAIN SHALL DAYLIGHT TO THE EXTERIOR BELOW THE FLOOR LEVEL OR TERMINATE IN A MINIMUM 20 GALLON SUMP PIT.

4. FOUNDATION DESIGN SHALL BE BASED ON A MINIMUM SOIL BEARING CAPACITY OF 1500 PSF. 5. FOOTINGS SHALL BE A MIN. OF 16" WIDE AND 8" DEEP W/ (2) #4 BARS CONTINUOUS, LOCATED A MIN. OF 3" CLEAR FROM BOTTOM. FOOTINGS SHALL BE A MINIMUM OF 36" BELOW GRADE FOR FROST PROTECTION.

6. COLUMN PADS SHALL BE A MINIMUM OF 24"X24"X8" WITH (3) #4 BARS EACH WAY. 7. FOUNDATION WALLS SHALL BE A MINIMUM 8" THICK W/ MINIMUM #4 BARS @ 24" O.C. HORIZONTAL AND VERTICAL W/ THE TOP BAR WITHIN 8" OF THE TOP OF THE WALL

UNLESS NOTED OTHERWISE ON PLAN. 8. REINFORCEMENT SHALL LAP A MINIMUM OF 24"

9. INTERIOR BEARING WALLS AND COLUMNS SHALL BE ISOLATED FROM THE BASEMENT FLOOR SLAB.

10. INTERIOR NON-BEARING WALLS, OTHER THAN THOSE RESTING DIRECTLY ON THE FOOTING, SHALL BE ISOLATED FROM THE FLOOR FRAMING ABOVE BY A SEPARATION

11. CONCRETE FLOOR SLABS ON GRADE, SHALL BE A MINIMUM 4" THICK OVER A MINIMUM 4" BASE OF SAND, GRAVEL, OR CRUSHED STONE. BASEMENT SLABS SHALL HAVE A MIN. 6 MIL POLYETHYLENE OR APPROVED VAPOR RETARDER WITH JOINTS LAPPED NOT LESS THAN 6" SHALL BE PLACED BETWEEN THE FLOOR SLAB AND THE BASE

12. FLOOR SLABS SUPPORTED BY FILL CONSISTING OF MORE THAN 24" OF GRANULAR FILL OR 8" OF EARTH SHALL BE REINFORCED PER A SEPARATE ENGINEERING

13. BASEMENT FOUNDATION SILL PLATES SHALL BE BOLTED TO THE FOUNDATION W/ A MINIMUM OF 1/2" ANCHOR BOLTS EMBEDDED AT LEAST 7" INTO THE CONCRETE AND SPACED NOT MORE THAN 3' ON CENTER AND WITHIN 12" OF EACH END PIECE PER IRC SECTION R403.1.6. 14. FOUNDATION WINDOW WELLS FOR SECONDARY MEANS OF EGRESS SHALL PROVIDE A MINIMUM 3'X3' HORIZONTAL AREA. 15. THE BASE OF ALL FOOTING EXCAVATIONS SHOULD BE FREE OF ALL WATER AND LOOSE MATERIAL PRIOR TO PLACING CONCRETE. CONCRETE SHOULD BE PLACED AS

SOON AS POSSIBLE AFTER EXCAVATING SO THAT EXCESSIVE DRYING OR DISTURBANCE OF BEARING MATERIALS DOES NOT OCCUR. SHOULD THE MATERIALS AT BEARING LEVEL BECOME EXCESSIVELY DRY OR SATURATED. WE RECOMMEND THAT THE AFFECTED MATERIAL BE REMOVED PRIOR TO PLACING CONCRETE 16. IT IS RECOMMENDED THAT ALL FOOTING EXCAVATIONS BE EVALUATED AND TESTED BY A GEOTECHNICAL ENGINEER IMMEDIATELY PRIOR TO PLACEMENT OF FOUNDATION CONCRETE. UNSUITABLE AREAS IDENTIFIED AT THIS TIME SHOULD BE CORRECTED. CORRECTIVE PROCEDURES WOULD BE DEPENDENT UPON CONDITIONS ENCOUNTERED AND MAY INCLUDE DEEPENING OF FOUNDATION ELEMENTS, OR UNDERCUTTING OF UNSUITABLE MATERIALS AND REPLACEMENT WITH ENGINEERED FILL.

. STAIRWAYS SHALL PROVIDE A MAXIMUM 7 3/4" RISE AND MIN. 10" RUN. 2. PROVIDE MINIMUM 36" GUARDRAILS ON THE OPEN SIDES OF RAISED FLOORS, PORCHES AND BALCONIES. MINIMUM 34" GUARDRAILS ON THE OPEN SIDES OF STAIRWAYS LOCATED MORE THAN 30" ABOVE THE FLOOR OR GRADE BELOW. GUARDRAIL ENCLOSURES SHALL HAVE INTERMEDIATE RAILS OR ORNAMENTAL PATTERNS THAT DO NOT

3. EACH STAIRWAY OF 3 OR MORE RISERS SHALL PROVIDE A CONTINUOUS HANDRAIL ON AT LEAST ONE SIDE BETWEEN 34" AND 38" ABOVE THE NOSING OF THE THREADS. 4. HANDRAILS SHALL HAVE A CIRCULAR CROSS-SECTION OF 1 1/4" MINIMUM TO 2" MAXIMUM OR OTHER APPROVED GRASPABLE SHAPE PER IRC SECTION R311.7.8.5 5. PROVIDE A MINIMUM 6'-8" OF HEADROOM CLEARANCE IN STAIRWAYS.

6. ENCLOSED ACCESSIBLE SPACE UNDER STAIRWAYS SHALL HAVE WALLS AND THE UNDERSIDE OF THE STAIR AND LANDING PROTECTED WITH 1/2" GYPSUM BOARD ON 7. WINDERS SHALL PROVIDE A MINIMUM TREAD OF AT LEAST 6" AT ANY POINT WITHIN CLEAR WIDTH OF STAIRS. WINDER TREAD PROPORTION TO COMPLY WITH

IRCR311.7.5.2.1.

1. GLAZING IN HAZARDOUS LOCATIONS AS IDENTIFIED IN IRC SECTION R308.4 SHALL BE OF APPROVED SAFETY GLAZING MATERIALS. GLASS IN STORM DOORS. INDIVIDUAL FIXED OR OPERABLE PANELS ADJACENT TO A DOOR WHERE THE NEAREST VERTICAL EDGE IS WITHIN A 24" ARCH OF THE DOOR IN A CLOSED POSITION AND WHOSE BOTTOM EDGE IS WITHIN 60" OF THE FLOOR, WALLS ENCLOSING STAIRWAYS AND LANDINGS WHERE THE GLAZING IS WITHIN 60" OF THE TOP OR BOTTOM OF THE STAIR, ENCLOSURES FOR SPAS, TUBS, SHOWERS AND WHIRLPOOLS, GLAZING IN FIXED OR OPERABLE PANELS EXCEEDING 9 S.F. AND WHOSE BOTTOM EDGE IS LESS THAN 18" ABOVE THE FLOOR OR WALKING SURFACE WITHIN 36"

2. IN DWELLING UNITS, WHERE THE OPENING OF AN OPERABLE WINDOW IS LOCATED MORE THAN 72 INCHES ABOVE THE FINISHED GRADE OR SURFACE BELOW, THE LOWEST PART OF THE CLEAR OPENING OF THE WINDOW SHALL BE A MINIMUM OF 24 INCHES ABOVE THE FINISHED FLOOR OF THE ROOM IN WHICH THE WINDOW IS LOCATED. OPERABLE SECTIONS OF WINDOWS SHALL NOT PERMIT OPENINGS THAT ALLOW PASSAGE OF A 4 INCH DIAMETER SPHERE WHERE SUCH OPENINGS ARE LOCATED WITHIN 24 INCHES OF THE FINISHED FLOOR.

FRAMING NOTES:

1. ALL LUMBER SIZES ARE FOR DOUGLAS FIR-LARCH UNLESS OTHERWISE NOTED.

2. ALL HEADERS TO BE A MINIMUM OF (2) #2-2X10'S UNLESS OTHERWISE NOTED. 3. BLOCK CANTILEVERS, DOOR JAMBS, AND OVER BEAMS.

4. ALL HEADERS/BEAMS TO BEAR ON A MINIMUM OF (2) 2X4 POSTS UNLESS NOTED OTHERWISE.

5. INTERIOR NON-BEARING WALLS, OTHER THAN THOSE RESTING DIRECTLY ON THE FOOTING SHALL BE ISOLATED FROM THE FLOOR FRAMING ABOVE 6. WHERE JOISTS RUN PARALLEL TO FOUNDATION WALLS, SOLID BLOCKING FOR A MINIMUM OF (2) JOIST SPACES SHALL BE PROVIDED AT A MAXIMUM OF 4' CENTERS TO TRANSFER LATERAL LOADS ON THE WALL TO THE FLOOR DIAPHRAGM. THE BLOCKING SHALL BE SECURELY NAILED TO THE JOISTS AND FLOORING. NAIL JOISTS AND

BLOCKING TO SILL PLATE WITH (4) 10D NAILS. 7. IF DUCTS ARE INSTALLED IN THE FIRST JOIST SPACE(S), NAIL 2X4'S FLAT AT 4' CENTERS WITHIN THE JOIST SPACE(S) AND THEN PROVIDE SOLID BLOCKING, INSTALLED UPRIGHT, IN THE NEXT TWO JOIST SPACES. SECURE THE 2X4'S TO THE SILL PLATE WITH (4) 10D NAILS.

8. ALL SILLS AND SLEEPERS SUPPORTED ON CONCRETE OR MASONRY AND FURRING ATTACHED TO CONCRETE OR MASONRY SHALL BE OF DECAY RESISTANT 9. JOISTS UNDER BEARING PARTITIONS SHALL BE SIZED TO CARRY THE DESIGN LOAD IN ACCORDANCE WITH IRC SECTION R502.4.

10. JOISTS FRAMING FROM OPPOSITE SIDES OVER BEARING SUPPORTS SHALL LAP A MINIMUM OF 3" AND SHALL BE NAILED TOGETHER WITH A MINIMUM 10D FACE NAILS. 11. JOISTS FRAMING INTO A WOOD GIRDER OR BEAM SHALL BE SUPPORTED BY APPROVED FRAMING ANCHORS OR ON MINIMUM 2"X2" LEDGER STRIPS. 12. HEADER AND TRIMMERS SHALL BE OF SUFFICIENT CROSS SECTION TO SUPPORT THE FLOOR FRAMING. TRIMMER JOISTS SHALL BE DOUBLED WHEN THE HEADER IS

SUPPORTED MORE THAN 3' FROM THE TRIMMER JOIST BEARING. WHEN THE HEADER SPAN EXCEEDS 4', THE HEADER AND TRIMMER SHALL BE DOUBLED. 13. JOISTS AT SUPPORTS SHALL BE SUPPORTED LATERALLY AT THE ENDS BY FULL-DEPTH SOLID BLOCKING NOT LESS THAN 2" NOMINAL THICKNESS OR BY ATTACHMENT TO A HEADER, BAND OR RIM JOIST OR TO AN ADJOINING STUD OR OTHERWISE PROVIDED WITH LATERAL SUPPORT TO PREVENT ROTATION.

14. ALL WALL COVERINGS TO COMPLY WITH IRC SECTION 702 AND 703 15. ALL RAFTER / COLLAR TIES TO COMPLY WITH IRC SECTIONS 804

16. ALL RAFTERS TO HAVE 2x4 COLLAR TIES @ 48" OC IN UPPER 1/3 OF DISTANCE BETWEEN CEILING AND ROOF

17. BLOCKING BETWEEN JOISTS UNDER A PERPENDICULAR LOAD-BEARING WALL IS NOT REQUIRED 18. BOTTOM OF ALL FLOOR ASSEMBLIES SHALL BE PROVIDED WITH A 1/2" GYPSUM WALLBOARD MEMBRANE (IF REQUIRED BY LOCAL CODE)

19. I-JOIST AND FLOOR TRUSS SYSTEMS SHALL BE FIRE PROTECTED PER IRC AS ADOPTED BY AHJ 20. STUDS SHALL BE CONTINUOUS FROM THE FLOOR TO THE ROOF/ CEILING DIAPHRAGM PER IRC 602.3

CONCRETE SHALL BE AIR-ENTRAINED (5%-7%) WITH A MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS OF 2500 PSI FOR BASEMENT AND INTERIOR FLOOR SLABS, 3000 PSI FOR BASEMENT AND FOUNDATION WALLS AND 3500 PSI FOR PORCHES, CARPORTS AND GARAGE FLOOR SLABS.

1. PROVIDE ONE WINDOW FOR EACH BEDROOM THAT HAS A MINIMUM OPENABLE AREA OF 5.7 S.F. WITH A MINIMUM OPENABLE HEIGHT OF 24" AND WIDTH OF 21". IN ADDITION, THE OPENABLE PORTION OF EGRESS WINDOWS SHALL NOT EXCEED 44" ABOVE THE ADJOINING FLOOR OR PERMANENT STEP. 2. PROVIDE SMOKE ALARMS IN EACH SLEEPING ROOM, OUTSIDE OF EACH SLEEPING AREA AND ON EACH FLOOR INCLUDING BASEMENTS. ALARMS SHALL BE INTERCONNECTED IN SUCH A MANNER THAT THE ACTIVATION OF ONE ALARM WILL ACTIVATE ALL OF THE ALARMS IN THE DWELLING.

3. PROVIDE CARBON MONOXIDE ALARMS AS REQUIRED PER IRC. CARBON MONOXIDE ALARMS SHALL BE INSTALLED OUTSIDE OF EACH SEPARATE SLEEPING AREA. WHERE FUEL-BURNING APPLIANCES ARE LOCATED WITHIN A BEDROOM OR ITS ATTACHED BATHROOM, A CARBON MONOXIDE ALARM SHALL BE INSTALLED IN THE BEDROOM.

1. THE GARAGE FLOOR SHALL SLOPE TOWARDS THE GARAGE DOORWAYS OR SLOPE TO A TRENCH OR UNTRAPPED DRAIN THAT DISCHARGES DIRECTLY TO THE EXTERIOR ABOVE GRADE. 2. DOORS BETWEEN THE GARAGE AND DWELLING - MINIMUM 1 3/8" SOLID WOOD, SOLID OR HONEY-COMBED CORE STEEL DOOR NOT LESS THAN 1 3/8" THICK, OR 20 -

MINUTE FIRE - RATED EQUIPPED WITH SELF CLOSING DEVICE PER IRC2018 R302.5.1.. 3. GARAGE VEHICLE DOORS AND FRAMES SHALL BE DESIGNED AND INSTALLED TO MEET THE 115-MPH 3-SECOND GUST LOADING PER DASMA 108 AND ASTM E 330-96 PER IRC2018 R301.2.1 4. THE GARAGE SHALL BE SEPARATED FROM THE DWELLING AND ITS ATTIC AREAS BY MINIMUM 5/8" GYPSUM BOARD APPLIED TO THE GARAGE SIDE. WHERE HABITABLE

SPACE OCCURS ABOVE THE GARAGE, THE FLOOR CEILING ASSEMBLY SHALL BE PROTECTED WITH MINIMUM 5/8" TYPE X GYPSUM BOARD ON THE GARAGE CEILING. WHERE A FLOOR/CEILING SPACE IS PROVIDED ABOVE THE GARAGE COLUMNS AND BEAMS SUPPORTING THE SEPARATION SHALL ALSO BE PROTECTED WITH 5/8" GYPSUM BOARD 5. GARAGE DOOR H-FRAME FOR THE ATTACHMENT OF THE TRACK AND COUNTER BALANCE SHALL CONSIST OF THE FOLLOWING: 2x6 VERTICAL JAMBS RUNNING FROM

FLOOR TO CEILING ATTACHED WITH 1 3/4"X.120" NAILS AT 7" CENTERS STAGGERED WITH (7) 3 1/4"X.120" NAILS THRU THE JAMB INTO THE HEADER, MINIMUM 2X8 HEADER FOR ATTACHMENT OF COUNTER BALANCE SYSTEM. 6. ANY ATTACHED GARAGE TO THE MAIN HOUSE SHALL BE PROVIDED WITH A SINGLE HEAT DETECTOR. HEAT DETECTOR SHALL BE HARDWIRED AND INTERCONNECTED

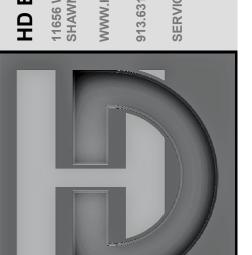
WITH THE HOUSEHOLD SMOKE ALARM SYSTEM. HEAT DETECTOR SHALL BE LISTED FOR THE AMBIENT ENVIRONMENT AND INSTALLED PER MANF. INSTRUCTIONS.

1. BUILDING ENVELOPE INSULATION SHALL COMPLY WITH IRC TABLE N1102.1.1 OR THE 2018 IECC. (SEE S-6.0 FOR MORE DETAILS)

1. ENCLOSED ATTICS SHALL HAVE CROSS VENTILATION FOR EACH SEPARATE SPACE BY VENTILATING OPENINGS PROTECTED AGAINST THE ENTRANCE OF RAIN OR SNOW. VENTILATING OPENINGS SHALL BE PROVIDED WITH CORROSION-RESISTANT WIRE MESH, WITH 1/8" TO 1/4" OPENINGS. THE TOTAL FREE VENTILATING AREA SHALL NOT BE LESS THAN 1/150 OF THE AREA OF SPACE VENTILATED, EXCEPT WHERE THE VENTILATORS AREA LOCATED IN THE UPPER PORTION OF THE SPACE TO BE VENTILATED THE REQUIRED AREA MAY BE REDUCED TO 1/300.

THIS DOCUMENT CONTAINS CONFIDENTIAL INFORMATION LONGINING TO HD ENGINEER INAUTHORIZED USE, DISCLOSURI SEMINATION, OR DUPLICATION O

ANY OF THE INFORMATION AINED HEREIN MAY RESULT IN IABILITY UNDER APPLICABLE LAW





39707

08/04/2020 DATE: CHECKED BY: CLS

ISSUE/REVISION

GENERAL NOTES

TABLE R602.3(1) FASTENER SCHEDULE FOR STRUCTURAL MEMBERS

TEM	DESCRIPTION OF BUILDING ELEMENTS	NUMBER AND TYPE OF ^{a,b,c} FASTENER	SPACING OF FASTENERS	ITEM	
		ROOF 4-8D BOX (2 1/2" X 0.113")			
2	BLOCKING BETWEEN JOISTS OR RAFTERS TO TOP PLATE, TOE NAIL CEILING JOISTS TO PLATE, TOE NAIL	3-8D (2 1/2" X 0.113") 3-10D (3"X0.128")	TOE NAIL PER JOIST, TOE NAIL		\top
	CEILING JOISTS NOT ATTACHED TO PARALLEL RAFTER, LAPS OVER	3-3"X 0.131" NAILS 4-10D BOX (3"X 0.128")	· · · · · · · · · · · · · · · · · · ·	30	
3	PARTITIONS (SEE SECTION R802.5.2 AND TABLE R802.52	3-16D COMMON (3 1/2"X 0.162") 4-3"X 0.131"NAILS	FACE NAIL	31	
4	CEILING JOIST ATTACHED TO PARALLEL RAFTER (HEEL JOINT) SEE SECTION R802.5.2 AND TABLE R802.5.2)	TABLE R802.5.2	FACE NAIL		+
5	COLLAR TIE TO RAFTER, FACE NAIL OR 1 1/4" X 20GA. RIDGE STRAP TO RAFTER	4-10D BOX (3" X 0.128") 3-10D COMMON (3" X 0.148")	FACE NAILS EACH RAFTER	32	╧
	STRAF TO RAFTER	4-3" X 0.131" NAILS 3-16D BOX NAILS (3 1/2" X0.135")	2 TOE MAIL CON ONE CIDE AND 4 TOE		₹
6	RAFTER OR ROOF TRUSS TO PLATE	3-10D COMMON NAILS (3" X 0.148" 4-10D BOX (3" X 0.128" 4-3" X0.131" NAILS	2 TOE NAILS ON ONE SIDE AND 1 TOE NAIL ON OPPOSITE SIDE OF EACH RAFTER OR TRUSS ¹	33	
	POOF DAFTERS TO DIDGE WALLEY OR HID DAFTERS OF DOOF	4-16D(3 1/2" X 0.135"); OR 3-10D COMMON (3" X 0.148") 4-10D BOX (3" X 0.128"); OR 4-3" X 0.131" NAILS		34	
7	ROOF RAFTERS TO RIDGE, VALLEY OR HIP RAFTERS OR ROOF RAFTER TO MINIMUM 2" RIDGE BEAM	3-16D(3 1/2" X0.135"); OR 2-16D COMMON (3 1/2" X0.162") 3-10D BOX (3" X 0.128"); OR 3-3" X 0.131" NAILS	TOE NAIL	35	
		WALL		36	T
0	OTUD TO OTUD (NOT BRACED WALL BANK)	16D (3 1/2" X 0.162")	24" OC FACE NAIL		\perp
8	STUD TO STUD (NOT BRACED WALL PANELS)	10D BOX (3" X 0.128"); OR 3" X 0.131" NAILS	16" OC FACE NAIL		—
	STUD TO STUD AND ABUTTING STUDS AT INTERSECTING WALL	16D BOX (3 1/2" X 0.135"); OR 3" X 0.131" NAILS	12" OC FACE NAIL	37	
9	CORNERS (AT BRACED WALL PANELS)	16D COMMON (3 1/2" X 0.162")	16" OC FACE NAIL	<u> </u>	+
	BUILT-UP HEADER (2" TO 2" HEADER WITH 1/2" SPACER)	16D COMMON (3 1/2" X 0.162")	16" OC EACH EDGE FACE NAIL	38	
10	BOILT OF THE ROLL (2 TO 2 THE ROLL WITH 1/2 OF NOLLY)	16D BOX (3 1/2" X 0.135")	12" OC EACH EDGE FACE NAIL		1
11	CONTINUOUS HEADER TO STUD	5-8D BOX (2 1/2" X 0.113") or 4-8D COMMON (2 1/2" X 0.131") 4-10D BOX (3" X 0.128")	TOE NAIL	39	
		16D COMMON (3 1/2" X 0.162")	16" OC FACE NAIL	For SI:	1 i
12	TOP PLATE TO TOP PLATE	10D BOX (3" X 0.128") OR 3" X 0.131" NAILS	12" OC FACE NAIL		
13	DOUBLE TOP PLATE SPLICE	8-16D COMMON (3 1/2" X 0.162"); or 12-16D BOX (3 1/2" X 0.135"); or 12-10D BOX (3" X 0.128"); or 12-3" X 0.131" NAILS	FACE NAIL ON EACH SIDE OF END JOINT (MINIMUM 24" LAP SPLICE LENGTH EACH SIDE OF END JOINT)		
	BOTTOM PLATE TO JOIST, RIM JOIST, BAND JOIST OR BLOCKING	16D COMMON (3 1/2" X 0.162")	16" OC FACE NAIL		
14	(NOT AT BRACED WALL PANELS	16D BOX (3 1/2" X 0.135"); OR 3" X 0.131" NAILS	12" OC FACE NAIL STUD		ים פ
15	BOTTOM PLATE TO JOIST, RIM JOIST, BAND JOIST OR BLOCKING (NOT AT BRACED WALL PANELS	3-16D BOX (3 1/2" X 0.135"); or 2-16D COMMON (3 1/2" X0.162"); or 4-3" X 0.131" NAILS	3, 2, OR 4 EACH 16" OC FACE NAIL		(IN
		4-8D BOX (2 1/2" X 0.113"); or 3-16D BOX (3 1/2" X0.135"); or 4-8D COMMON (2 1/2" X0.131");or 4-10D BOX (3" X0.128"); or 3-3" X 0.131" NAILS	TOE NAIL		
16	TOP OR BOTTOM PLATE TO STUD	3-16D BOX (3 1/2" X 0.135"); or 2-16D COMMON (3 1/2" X0.162"); or 3-10D BOX (3" X0.128");or 3-3" X 0.131" NAILS	END NAIL		
17	TOP PLATES, LAPS AT CORNERS AND INTERSECTIONS	3-10D BOX (3" X 0.128"); or 2-16D COMMON (3 1/2" X0.162"); or 3-3" X 0.131" NAILS	FACE NAIL	2	2x3
18	1" BRAVE TO EACH STUD AND PLATE	3-8D BOX (2 1/2" X 0.113"); or 2-8D COMMON (2 1/2"	FACE NAIL		2x4
		X0.131") or 2-10D BOX (3" X 0.128"); or 2 STAPLES 1 3/4" 3-8D BOX (2 1/2" X 0.113"); or 2-8D COMMON (2 1/2"			3x4 2x5
19	1" X 6" SHEATHING TO EACH BEARING	X0.131") or 2-10D BOX (3" X 0.128"); or 2 STAPLES 1" CROWN, 16GA., 1 3/4" LONG	FACE NAIL		2x6
		3-8D BOX (2 1/2" X 0.113"); or 3-8D COMMON (2 1/2" X0.131") or 3-10D BOX (3" X 0.128"); or 3 STAPLES, 1" CROWN, 16GA., 1 3/4" LONG		FOR SI: a. LIST ON NOT	E
20	1" X 8" AND WIDER SHEATHING TO EACH BEARING	WIDER THAN 1" X 8" 4-8D BOX (2 1/2" X 0.113"); or 3-8D COMMON (2 1/2" X0.131") or 3-10D BOX (3" X 0.128"); or 4	FACE NAIL	UNSUPF PRACTION SHA	PO ICE
		STAPLES, 1" CROWN, 16GA., 1 3/4" LONG FLOOR		c. A HA INCREA	
21	JOIST TO SILL, TOP PLATE OR GIRDER	4-8D BOX (2 1/2" X 0.113"); or 3-8D COMMON (2 1/2" X0.131") or 3-10D BOX (3" X 0.128"); or 3-3" X 0.131: NAILS	TOE NAIL	MI	١
22	RIM JOIST, BAND JOIST OR BLOCKING TO SILL OR TOP PLATE	8D BOX (2 1/2" X 0.113")	4" OC TOE NAIL	VA	
	(ROOF APPLICATIONS ALSO)	8D COMMON (2 1/2" X 0.131"); or 10D BOX(3" X0.128") or 3-3" X 0.131" NAILS 3-8D BOX (2 1/2" X 0.113"); or 2-8D COMMON (2 1/2"	6" OC TOE NAIL		
23	1" X 6" SUBFLOOR OR LESS TO EACH JOIST	X0.131") or 3-10D BOX (3" X 0.128"); or 2 STAPLES, 1" CROWN, 16GA., 1 3/4" LONG	FACE NAIL		
24	2" SUBFLOOR TO JOIST OR GIRDER	3-16D BOX (3 1/2" X 0.135"); or 2-16D COMMON (3 1/2" X0.162")	BLIND AND FACE NAIL		
25	2" PLANKS (PLANK & BEAM-FLOOR AND ROOF)	3-16D BOX (3 1/2" X 0.135"); or 2-16D COMMON (3 1/2" X0.162")	AT EACH BEARING, FACE NAIL		
26	BAND OR RIM JOIST TO JOIST	3-16D COMMON (3 1/2" X 0.162"); or 4-10D BOX (3" X0.128") or 4-3" X 0.131" NAILS; or 4-3" X 14GA. STAPLES, 7/16" CROWN	END NAIL		
27	DI III T LID CIDDEDO AND DEAMO O INCLUENZACIO INCLUENZACIO	20D COMMON (4" X 0.192"); or	NAIL EACH LAYER AS FOLLOWS: 32" OC AT TIP AND BOTTOM AND STAGGERED 24" OC FACE NAIL AT TOP AND BOTTOM	<u>N</u>	<u>/</u>
27	BUILT-UP GIRDERS AND BEAMS, 2-INCH LUMBER LAYERS	10D BOX (3" X 0.128"); or 3" X 0.131" NAILS AND: 2-20D COMMON (4" X 0.192"); or	STAGGERED ON OPPOSITE SIDES		
	LEDGER STRIP SUPPORTING JOISTS OR RAFTERS	3-10D BOX (3" X 0.128; or 3-3" X 0.131" NAILS 4-16D BOX (3 1/2" X 0.135"): or 3-26D COMMON (3 1/2" X 0.162"); or	AT EACH JOIST OR RAFTER, FACE NAIL	CLIMAT	
28	· · · · - · · - · · - · · · -	4-10D BOX (3" X 0.128"); or 4-3" X 0.131" NAILS	,	4 EXCEP	T M
28		2-10D BOX (3" X 0.128"): or 2-8D COMMON		NOTES: 1	1\ r

e. SPACING OF FASTENERS NOT INCLUDED IN THIS TABLE SHALL BE BASED ON TABLE R602.3(2).
f. FOR REGIONS HAVING BASIC WIND SPEED OF 110 MPH OR GREATER, 8D DEFORMED (2 1/2" X 0.120) NAILS SHALL BE USED FOR ATTACHING PLYWOOD AND WOOD STRUCTURAL PANEL ROOF SHEATHING TO FRAMING WITHIN MINIMUM 48-INCHES DISTANCE FROM GABLE END WALLS, IF MEAN ROOF

SHEATHING TO INTERMEDIATE SUPPORTS SHALL BE SPACED 6 INCHES ON CENTER FOR MINIMUM 48-INCH DISTANCE FROM RIDGES, EAVES AND GABLE END WALLS; AND 4 INCHES ON CENTER TO GABLE END WALL FRAMING.

h. GYPSUM SHEATHING SHALL CONFORM TO ASTM C 1396 AND SHALL BE INSTALLED IN ACCORDANCE WITH GA 253. FIBERBOARD SHEATHING SHALL CONFORM TO ASTM C 208.

HEIGHT IS MORE THAN 25 FEET, UP TO 35 FEET MAXIMUM.

g. FOR REGIONS HAVING BASIC WIND SPEED OF 100 MPH OR LESS, NAILS FOR ATTACHING WOOD STRUCTURAL PANEL ROOF SHEATHING TO GABLE END WALL FRAMING SHALL BE SPACED 6 INCHES ON CENTER. WHEN BASIC WIND SPEED IS GREATER THAN 100 MPH, NAILS FOR ATTACHING PANEL ROOF

i. SPACING OF FASTENERS ON FLOOR SHEATHING PANEL EDGES SUPPORTED BY FRAMING MEMBERS AND REQUIRE BLOCKING AND AT ALL FLOOR PERIMETERS ONLY. SPACING OF FASTENERS ON FLOOR SHEATHING PANEL EDGES SUPPORTED BY FRAMING MEMBERS AND REQUIRED BLOCKING, BLOCKING OF ROOF OR FLOOR SHEATHING PANEL EDGES SUPPORTED BY FRAMING MEMBERS AND REQUIRED BLOCKING, BLOCKING OF ROOF OR FLOOR SHEATHING PANEL EDGES PERPENDICULAR TO THE FRAMING MEMBERS NEED NOT BE PROVIDED EXCEPT AS REQUIRED BY OTHER PROVISIONS OF THIS CODE. FLOOR PERIMETER SHALL BE SUPPORTED BY FRAMING MEMBERS OR SOLID BLOCKING.

. WHERE A RAFTER IS FASTENED TO AN ADJACENT PARALLEL CEILING JOIST IN ACCORDANCE WITH THIS SCHEDULE, PROVIDE TWO TOE NAILS ON ONE SIDE OF THE RAFTER AND TOE NAILS FROM CEILING JOIST TO TOP PLATE IN ACCORDANCE WITH THIS SCHEDULE. THE TOE NAIL ON THE OPPOSITE

CONTINUED TABLE R602.3(1)

ITEM	DESCRIPTION OF BUILDING ELEMENTS	SPACING OF FASTENERS EDGES (INCHES) UNDER THE COMMON COM										
	WOOD STRUCTURAL PANELS, SUBFLOOR, ROOF AND INTERIOR WALL SHEATHING TO FRAMING AND PARTICLEBOARD WALL SHEATHING TO FRAMING [SEE TABLE R602.3(3) FOR WOOD STRUCTURAL PANEL EXTERIOR WALL SHEATHING TO WALL FRAMING]											
30	3/8"- 1/2"	6D COMMON (2"X 0.113" NAIL (SUBFLOOR, WALL) i 8D COMMON (2 1/2" X 0.131 NAIL (ROOF); or RSRS-01 (2 3/8" X 0.113" NAIL (ROOF) j	6	12 f								
31	19/32" - 1"	8D COMMON NAIL (2 1/2" X 0.131; or RSRS-01; 2 3/8" X 0.113) NAIL ROOF j	6	12 f								
32	1 1/8" - 1 1/4"	10D COMMON NAIL (3" X 0.148) NAIL; or 8D (2 1/2" X 0.131") DEFORMED NAIL	6	12								
	ОТ	THER WALL SHEATHING ⁹										
33	1/2" STRUCTURAL CELLULOSE FIBERBOARD SHEATHING	1 1/2" GALVANIZED ROOF NAIL, 7/16" HEAD DIAMETER, OR 1 1/4" LONG 16GA. STAPLE WITH 7/16" OR 1" CROWN	3	6								
34	25/32" STRUCTURAL CELLULOSE FIBERBOARD SHEATHING	1 3/4" GALVANIZED ROOF NAIL, 7/16" HEAD DIAMETER, OR 1 1/2" LONG 16GA. STAPLE WITH 7/16" OR 1" CROWN	3	6								
35	1/2" GYPSUM SHEATHING ^d	1 1/2" GALVANIZED ROOF NAIL, STAPLE GALVANIZED, 11/2" LONG; 1 1/4" SCREWS, TYPE W or S	7	7								
36	5/8" GYPSUM SHEATHING ^d	1 3/4" GALVANIZED ROOF NAIL; STAPLE GALVANIZED, 1 5/8" LONG; 1 5/8" SCREWS, TYPE W or S	7	7								
	WOOD STRUCTURAL PANELS, CO	MBINATION SUBFLOOR UNDERLAYMENT TO FRAMING										
37	3/4" AND LESS	6D DEFORMED (2" X 0.120") NAIL OR 8D COMMON (2 1/2" X 0.131") NAIL	6	12								
38	7/8" - 1"	8D COMMON (2 1/2" X 0.131") NAIL OR 8D DEFORMED (2 1/2" X 0.120") NAIL	6	12								
39	1 1/8" - 1 1/4"	10D COMMON (3" X 0.148") NAIL OR 8D DEFORMED (2 1/2" X 0.120") NAIL	6	12								

For SI: 1 inch = 25.4mm, 1 foot = 304.8 mm, 1 mile per hour = 0.447 m/s; 1 ksi = 6.895 MPa.

TABLE R 602.3(5) SIZE, HEIGHT, AND SPACING OF WOOD STUDS

	BEARING WALLS					NON-BEARING WALLS	
STUD SIZE (IN)	LATERALLY UNSUPPORTED STUD HEIGHT a (feet)	MAXIMUM SPACING WHERE SUPPORTING A ROOF-CEILING ASSEMBLY OR A HABITABLE ATTIC ASSEMBLY, ONLY (inches)	MAXIMUM SPACING WHERE SUPPORTING ONE FLOOR, PLUS A ROOF-CEILING ASSEMBLY OR A HABITABLE ATTIC ASSEMBLY (inches)	MAXIMUM SPACING WHERE SUPPORTING TWO FLOORS, PLUS A ROOF-CEILING ASSEMBLY OR A HABITABLE ATTIC ASSEMBLY (inches)	MAXIMUM SPACING WHERE SUPPORTING ONE FLOOR HEIGHT _a (inches)	LATERALLY UNSUPPORTED STUD HEIGHT a (feet)	LATERALLY UNSUPPORTED STUD HEIGHT (feet)
2x3 ^b						10	16
2x4	10	24 _c	16 c		24	14	24
3x4	10	24	24	16	24	14	24
2x5	10	24	24		24	16	24
2x6	10	24	24	16	24	20	24

FOR SI: 1 INCH = 25.4mm, 1 FOOT = 304.8mm

a. LISTED HEIGHTS ARE DISTANCES BETWEEN POINTS OF LATERAL SUPPORT PLACED PERPENDICULAR TO THE PLANE OF THE WALL. BEARING WALL SHALL BE SHEATHED ON NOT LESS THAN ONE SIDE OR BRIDGING SHALL BE INSTALLED NOT GREATER THAN 4 FEET APART MEASURED VERTICALLY FROM EITHER END OF THE STUD. INCREASES IN UNSUPPORTED HEIGHT ARE PERMITTED WHERE IN COMPLIANCE WITH EXCEPTION 2 OF SECTION R602.3.1 OR DESIGNED IN ACCORDANCE WITH ACCEPTED ENGINEERING

c. A HABITABLE ATTIC ASSEMBLY SUPPORTED BY 2X4 STUDS IS LIMITED TO A ROOF SPAN OF 32 FEET. WHERE THE ROOF SPAN EXCEEDS 32 FEET, THE WALL STUDS SHALL BE INCREASED TO 2X6 OR THE STUDS SHALL BE DESIGNED IN ACCORDANCE WITH ACCEPTED ENGINEERING PRACTICE.

FASTENER SCHEDULE FOR STRUCTURAL MEMBERS

THE DWELLING SHALL COMPLY WITH THE FOLLOWING LOAD CONDITIONS

AREA	MIN DEAD LOAD	MIN LIVE LOAD
EXTERIOR BALCONIES	10	60
DECKS, STAIRS	10	40
CEILING JOISTS / ATTICS NO STORAGE - SCUTTLE ACCESS ONLY ROOF SLOPE 3:12 OR LESS	10	10
CEILING JOISTS / ATTICS NO STORAGE - SCUTTLE ACCESS ONLY ROOF SLOPE OVER 3:12	10	10
CEILING JOISTS / ATTICS WITH STORAGE - DOOR PULL DOWN LADDER ACCESS	10	20
ROOMS: NON-SLEEPING	10	40
ROOMS: SLEEPING	10	30
ROOF: LIGHT ROOF COVERING	10	20
ROOF: HEAVY ROOF COVERING / CONCRETE / TILE / SLATE	20	20
GUARDRAILS, HANDRAILS	200# LL	NORMAL

HEAVY ROOF COVERING MATERIAL (TILE, CONCRETE, SLATE, ETC.) SHALL NOT BE USED UNLESS 20 PSF DEAD LOAD AND HEAVY ROOF IS NOTED ON THE ROOF PLAN. IF HEAVY ROOFING IS TO BE USED AND NOT NOTED ON THE ROOF PLAN NOTIFY ENGINEER PRIOR TO ANY CONSTRUCTION, INCLUDING FOUNDATION AND SITE WORK. IF THE PLAN HAS BEEN DESIGNED FOR HEAVY ROOF LOADS IT WILL BE NOTED IN THE ROOF NOTES ON THE ROOF PLAN.

COLUMN SCHEDULE

BASED ON FOOTING SIZE (ASSUME 1500 PSF SOIL)

PAD SIZE	REINFORCEMENT	COL. MIN.	COL. TYPE	MAX. LOAD
24x24x12	(4) #4 BARS E/W	3"	SCH40	6K
30x30x12	(5) #4 BARS E/W	3"	SCH40	9.4K
36x36x12	(6) #4 BARS E/W	3"	SCH40	13.5K
42x42x14	(7) #4 BARS E/W	3 1/2"	SCH40	18.4K
48x48x16	(8) #4 BARS E/W	3 1/2"	SCH40	24.0K
54x54x16	(9) #4 BARS E/W	3 1/2"	SCH40	30.4K
60x60x18	(10) #4 BARS E/W	3 1/2"	SCH40	37.5K

ALL FOUR TAB EARS BENT AROUND THE BOTTOM FLANGE OF THE BEAM. FOR A STEEL BEAM TO MATCH THE HOLE PATTERN OF THE PLATE. 1/2" X 2" BOLTS SHOULD THEN BE INSTALLED WITH A FLAT WASHER, LOCK WASHER, AND A NUT IN EACH OF THE HOLES. THE POST CAP MAY BE WELDED TO THE STEEL BEAM IN ACCORDANCE WITH AWS D1.1-92 AS AN ALTERNATIVE, AND WOULD NEED TO BE INSPECTED BY AN AWS-CERTIFIED INSPECTOR.

ENGINEERED LUMBER

MIN. DESIGN REQUIREMENTS

	F _b (psi)	E (psi)	F _v (psi)
LVL	2600	1.8x10	285
GLULAM	2400	1.8x10	190
PARALAM	2600	2.0x10	290

MINIMUM MECHANICAL EQUIPMENT EFFICIENCY VALUES BY COMPONENT, PER IRC2018 N1103.6.1

FAN LOCATION	AIR FLOW RATE MINIMUM (CFM)	MINIMUM EFFICACY CFM/WATT	AIR FLOW RATE MAXIMUM (CFM)
HRV OR ERV	ANY	1.2 CFM/WATT	ANY
RANGE HOOD	ANY	2.8 CFM/WATT	ANY
IN-LINE FAN	ANY	2.8 CFM/WATT	ANY
BATHROOM UTILITY FAN	10	1.4 CFM/WATT	<90
BATHROOM UTILITY FAN	90	2.8 CFM/WATT	ANY

CATHEDRAL / VAULTED CEILING **FRAMING AND INSULATION**

MINIMUM R-38 INSULATION REQUIRED, SEE DETAIL 14/S-1.2

WHERE THE CEILING IS APPLIED DIRECTLY TO THE BOTTOM OF THE RAFTERS, A MINIMUM 1" AIR SPACE SHALL BE PROVIDED BETWEEN THE TOP OF THE INSULATION AND THE SHEATHING FOR VENTILATION (R806.3) NOTE: RAFTER SIZES SPECIFIED ON PLANS ARE THE MINIMUM REQUIRED FOR STRUCTURAL PURPOSES ONLY.

IF FULL RAFTER DEPTH IS NOT ADEQUATE FOR MINIMUM INSULATION VALUE, RAFTER SIZES WILL NEED TO BE INCREASED OR ADEQUATE FURRING SHALL BE USED TO OBTAIN THE MINIMUM JOIST DEPTH FOR THE REQUIRED INSULATION. IN ADDITION, IF THE RAFTER SIZE IS INCREASED IT SHALL BE VERIFIED THAT THE RIDGE BE A MINIMUM OF ONE NOMINAL SIZE LARGER THAN THE RAFTERS BEING RECEIVED. (SEE CHART BELOW)

MAXIMUM INSULATION VALUE	2x6	2x8	2x10	2x12
1" AIR SPACE (FIBERGLASS)	R-13, 3 1/2"	R-19, 6 1/4"	CONDENSED R-38, 8 1/4"	R-38, 10 1/4"

MINIMUM INSULATION & FENSTRATION VALUES BY COMPONENT, PER IRC2018 N1102.1.2

CLIMATE ZONE	FENSTRATION U-FACTOR	SKYLIGHT U-FACTOR	GLAZED SHGC FENSTRATION	INSULATED METAL DOOR U-VALUE	INSULATED WOOD DOOR U-VALUE	CEILING R-VALUE	WOOD FRAMED WALL R-VALUE					DUCTWORK (ALL OTHER) R-VALUE
4 EXCEPT MARINE	0.32	0.55	0.40	0.60	0.50	49	15	19	10 CONTINUOUS OR 13 CAVITY	10 CONTINUOUS OR 13 CAVITY	8	6

NOTES: 1) BUILDING THERMAL ENVELOPE IS REQUIRED TO BE SEALED WITH AN AIR BARRIER AS PER N1102.4.1 OF THE 2018 IRC 2) RECESSED LIGHTING SHALL BE SEALED TO PREVENT LEAKAGE BETWEEN THE CONDITIONED SPACE AND UNCONDITIONED SPACE

PLANS SHALL RELIEVE HD ENGINEERING & DESIGN OF ALL RESPONSIBILITIES OF THE CONSEQUENCES.

3) ALL DUCTS, AIR HANDLERS, FILTER BOXES, AND BUILDING CAVITIES USED AS DUCTS SHALL BE SEALED AS PER N1103.2 OF THE 2018 IRC

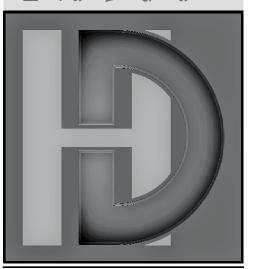
RESIDENTIAL CONSTRUCTION AND A THOROUGH UNDERSTANDING OF THE INTERNATIONAL RESIDENTIAL CODE (IRC). THE CONTRACTOR WARRANTS TO HD ENGINEERING & DESIGN THAT HE POSSESSES THE PARTICULAR DESIGN HAVE PERFORMED THEIR SERVICES WITH DUE CARE AND DILIGENCE. WE CANNOT GUARANTEE PERFECTION. ANY AMBIGUITY OR DISCREPANCY DISCOVERED BY THE USE OF THESE PLANS SHALL BE REPORTED IMMEDIATELY TO HD ENGINEERING. CONSTRUCTION MAY REQUIRE THAT THE CONTRACTOR ADAPT THE "BUILDER'S PLANS" TO THE FIELD CONDITIONS ENCOUNTERED AND MAKE LOGICAL ADJUSTMENTS IN FIT, FORM, DIMENSION AND QUANTITY. CHANGES MADE FROM THE PLANS WITHOUT THE CONSENT OF HD ENGINEERING & DESIGN ARE UNAUTHORIZED. IT IS ALSO UNDERSTOOD THAT THE CONTRACTOR WILL BE RESPONSIBLE FOR MEETING ALL APPLICABLE BUILDING CODES INCLUDING BUT NOT LIMITED TO MECHANICAL, ELECTRICAL, AND PLUMBING CODE REQUIREMENTS (WHICH IS EXCLUDED FROM THESE PLANS). IN THE EVENT ADDITIONAL DETAIL OR GUIDANCE IS NEEDED BY THE CONTRACTOR OR HOMEOWNER FOR CONSTRUCTION OF ANY ASPECT OF THE PROJECT, HD ENGINEERING & DESIGN OR A QUALIFIED ENGINEER SHALL IMMEDIATELY BE RETAINED. FAILURE TO NOTIFY US OF THESE NEEDS OR OF CHANGES TO THE

GENERAL NOTES

DESIGN LOADS (PSF)

COPYRIGHTED MATERIAL AND CONFIDENTIAL INFORMATION

ANY OF THE INFORMATION

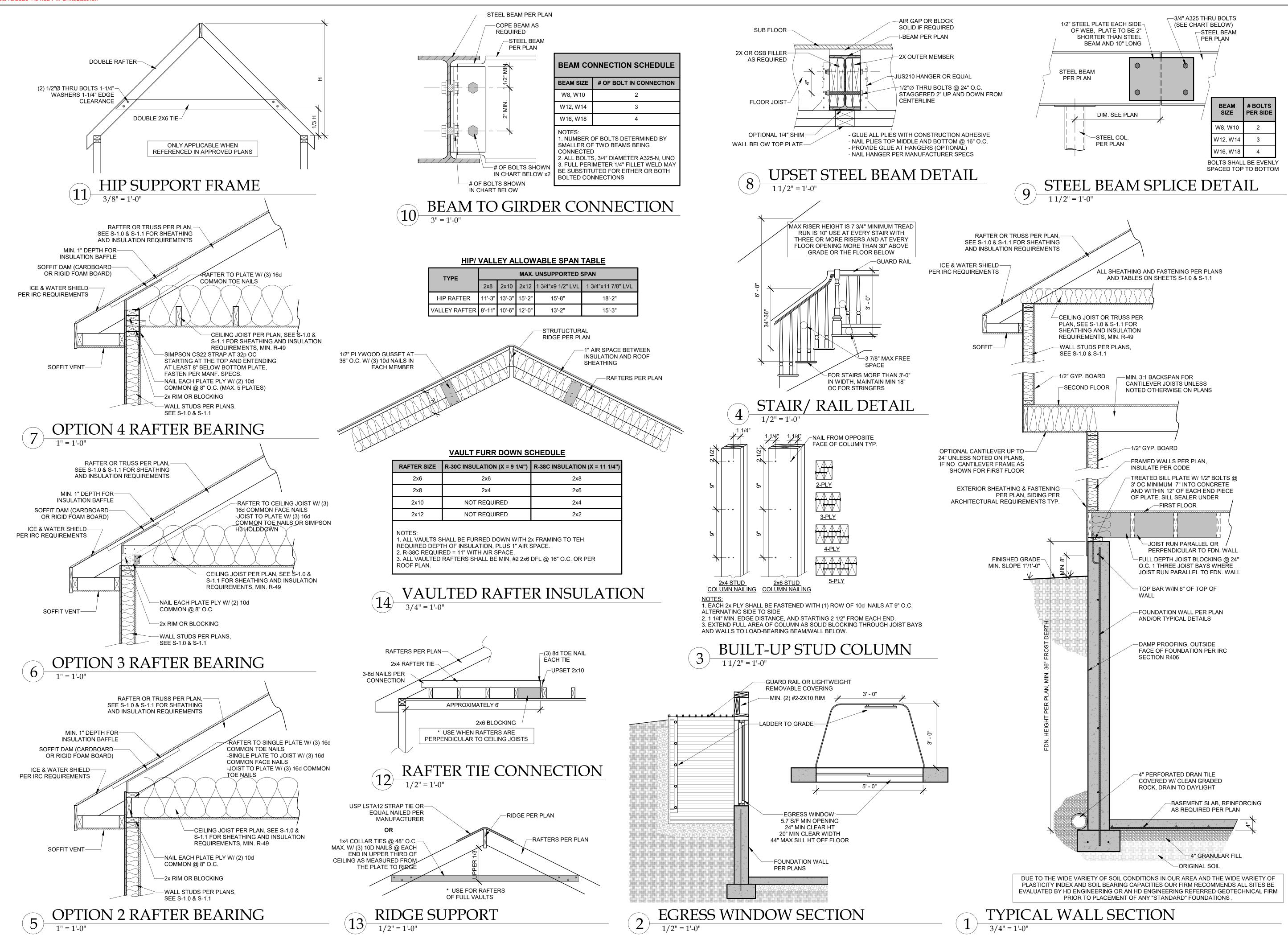




39707 08/04/2020 DATE:

ISSUE/REVISION

CHECKED BY: CLS



THIS DOCUMENT CONTAINS
COPYRIGHTED MATERIAL AND
CONFIDENTIAL INFORMATION
BELONGINING TO HD ENGINEERING
UNAUTHORIZED USE, DISCLOSURE,
DISSEMINATION, OR DUPLICATION O
ANY OF THE INFORMATION
CONTAINED HEREIN MAY RESULT IN
LIABILITY UNDER APPLICABLE LAW.

11656 W. 75TH STREET
SHAWNEE, KS 66214
WWW.HDENGINEERS.COM
913.631.2222
SERVICE@HDENGINEERS.COM





S SUMMIT, MO

E/16 - ELLENBROOK PL/ RIVER TRAIL RD., LEE'S S

D#: 39707

CHECKED BY: CLS

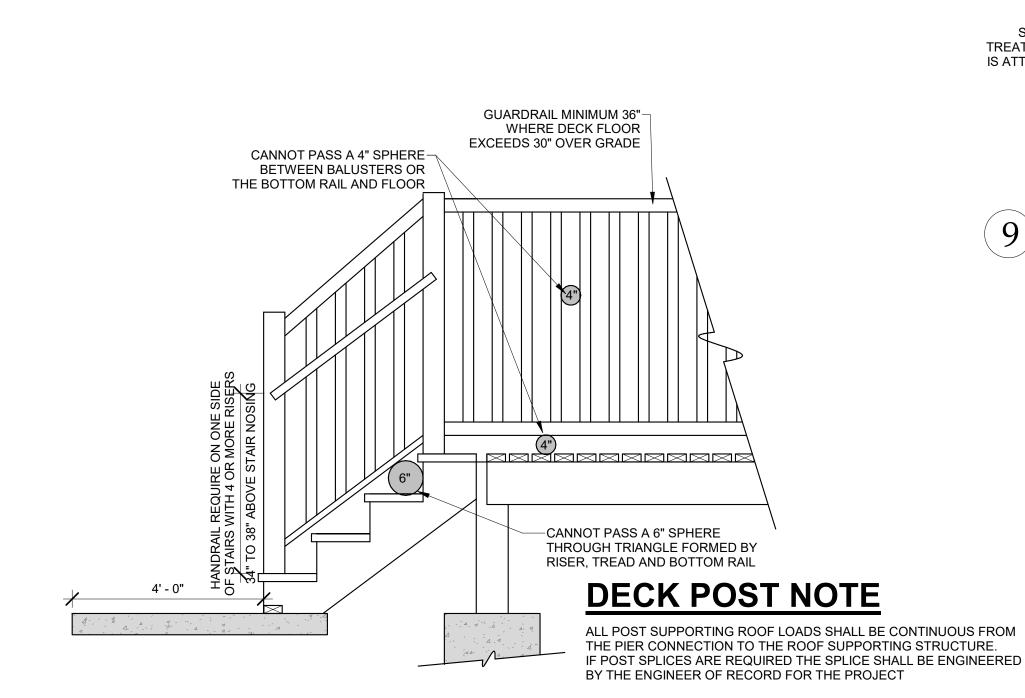
DATE:

08/04/2020

NO. ISSUE/REVISION Date

FRAMING SECTIONS

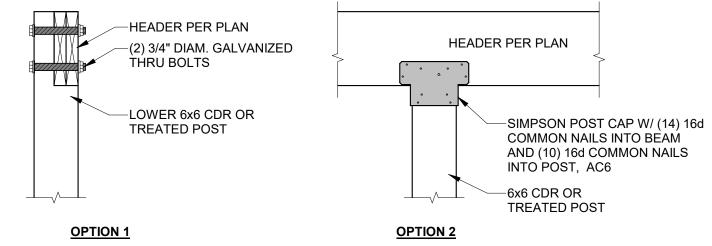
S-1.2



TOP OF EACH STRINGER IS-TOE-NAILED (TYP) AND SUPPORTED BY SIMPSON LS70 GUSSET ANGLE OR EQUIVALENT ON ONE SIDE, OR SLOPED -MIN (2) STRINGERS 24" OC TREÀÓS OVERHANGE 5" EACH SIDE (3) STRINGERS 36" OC THE THIRD CENTERED BETWEEN MAX SS = 5' FOR STRINGERS MAX SS = 9' FOR 3 STRINGERS MIN #2-2X12 TREATED SP STRINGER- COLUMN ATTACHED TO STRINGERS MAY BE PART OF HANDRAIL SYSTEM STRINGERS NOTCHED OVER-TREATED #2-2X4 SLEEPER WHICH IS ATTACHED TO LANDING LOCKS IN BOTTOM OF STRINGERS STRINGER SPAN (SS) CONCRETE LANDING IS RECOMMECDED IT SHALL SUPPORT THE HEEL CUT OF THE STRINGERS

STAIR STRINGER DETAIL

1/2" = 1'-0"



ROOF LEVEL INTERIOR BEAM TO COLUMN

TABLE IRC2018 R507.9.1.3(1) DECK LEDGER CONNECTION TO BAND JOIST (DECK LIVE LOAD = 40 PSF, DECK HEAD LOAD = 10 PSF, SNOW LOAD < 40 PSF)

				· · · /					
JOIST SPAN	6' AND LESS	6'-1" TO 8'	8'-1" TO 10'	10'-1" TO 12'	12'-1" TO 14'	14'-1" TO 16'	16'-1" TO 18'		
CONNECTION DETAILS		ON-CENTER SPACING OF FASTENERS ^{d, e}							
1/2" LAG SCREW WITH 15/32" MAX. SHEATHING ^{c,d}	30	23	18	15	13	11	10		
1/2" DIAM. BOLT WITH 15/32" MAX. SHEATHING ^d	36	36	34	29	24	21	19		
1/2" DIAM. BOLT WITH 15/32" MAX. SHEATHING & 1/2" STACKED WASHERS ^e	36	36	29	24	21	18	16		

For SI: 1 inch = 25.4mm, 1 foot = 304.8mm, 1 pound per square foot = 0.0479 kPa

a. Ledges shall be flashed in accordance with Section R703.4 to prevent water from contacting the house band joist.

b. Snow load shall not be assumed to act concurrently with live load. c. The tip of the lag screw shall fully extend beyond the inside face of the band joist.

d. Sheathing shall be wood structural panel or solid sawn lumber.

e. Sheathing shall be permitted to be wood structural panel, gypsum board, fiberboard lumber or foam sheathing. Up to 1/2" thinckness of stacked washers shall be permitted to substitute for you to 1/2" of allowable sheathing thickness where combined with wood structural panel or lumbers sheathing.

TABLE IRC2018 R507.9.1.3(2) PLACEMENT OF LAG SCEWS AND BOLT IN **DECK LEDGERS AND BAND JOISTS**

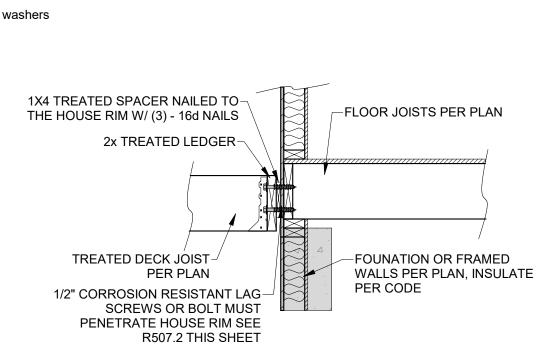
MINIMUM END AND EDGE DISTANCES AND SPACING BETWEEN ROWS											
	TOP EDGE BOTTOM EDGE		ENDS	ROW SPACING							
LEDGER ^a	2 inches ^d	3/4 inches	2 inches ^b	1 5/8 inches ^b							
BAND JOIST °	3/4 inches	2 inches	2 inches	1 5/8 inches ^b							

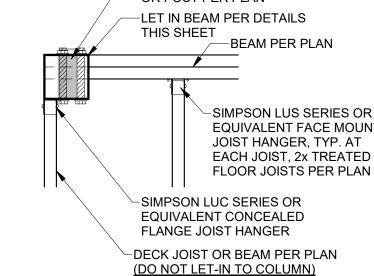
For SI: 1 inch = 25.4mm. a. Lag screws of bolts shal lbe staggered from the top to the bottom along the horizontal run of the deck

ledger in accordance with Figure R507.9.1.3(1) b. Maximum 5 inces

c. For engineered rim joists, the manufacturer's recommendations shall govern. d. The minimum distances from bottom row of lag screws or bolts to the top of the ledger shall be in accordance with Figure R507.9.1.3(1)

DECK LEDGER ATTACHMENT

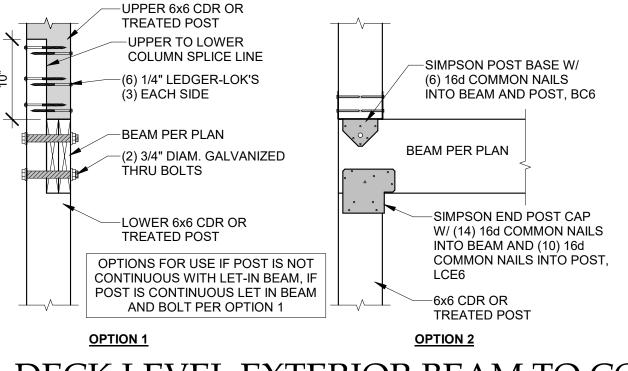




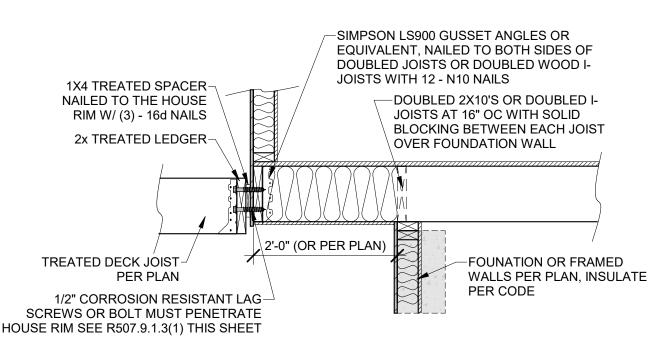


TREATED POST -UPPER 6x6 CDR OR -UPPER TO LOWER TREATED POST COLUMN SPLICE LINE SIMPSON POST BASE W/ (6) 16d COMMON NAILS -(6) 1/4" LEDGER-LOK'S **INTO BEAM AND POST, BC6** (3) EACH SIDE -BEAM PER PLAN BEAM PER PLAN -(2) 3/4" DIAM. GALVANIZED THRU BOLTS -LOWER 6x6 CDR OR -SIMPSON POST CAP W/ (14) 16d TREATED POST COMMON NAILS INTO BÈAM OPTIONS FOR USE IF POST IS NOT AND (10) 16d COMMON NAILS CONTINUOUS WITH LET-IN BEAM, IF POST INTO PÓST, AC6 IS CONTINUOUS LET IN BEAM AND BOLT PER OPTION 1 TREATED POST

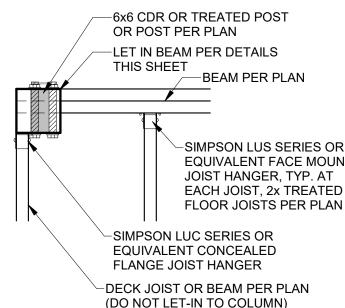
DECK LEVEL INTERIOR BEAM TO COLUMN



DECK LEVEL EXTERIOR BEAM TO COLUMN



DECK LEDGER TO CANTILEVER



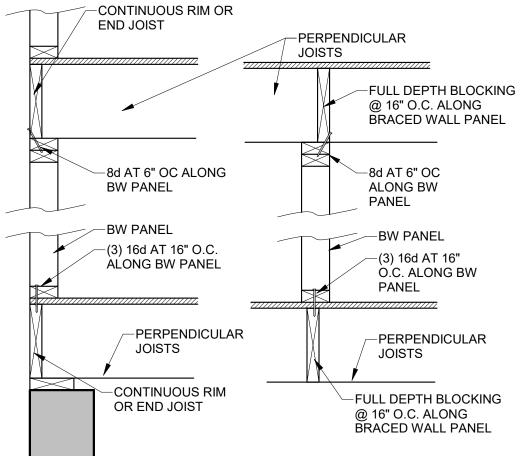
-SIMPSON LUS SERIES OR **EQUIVALENT FACE MOUNT**

39707

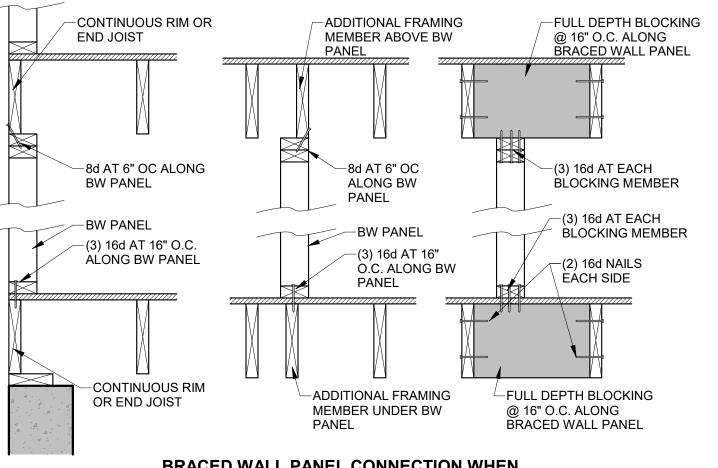
08/04/2020 DATE: CHECKED BY: CLS ISSUE/REVISION

COPYRIGHTED MATERIAL AND CONFIDENTIAL INFORMATION ELONGINING TO HD ENGINEERING UNAUTHORIZED USE, DISCLOSURE, DISSEMINATION, OR DUPLICATION OF ANY OF THE INFORMATION CONTAINED HEREIN MAY RESULT IN IABILITY UNDER APPLICABLE LAW

DECK DETAILS



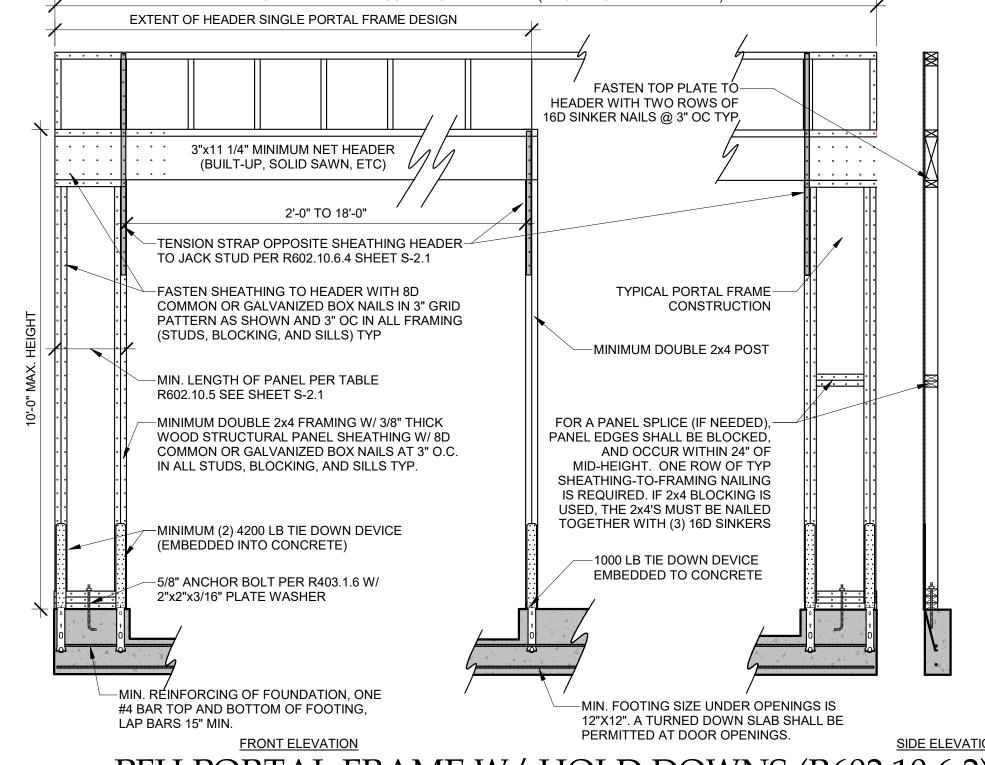
BRACED WALL PANEL CONNECTION WHEN PERPENDICULAR TO FLOOR/CEILING JOISTS



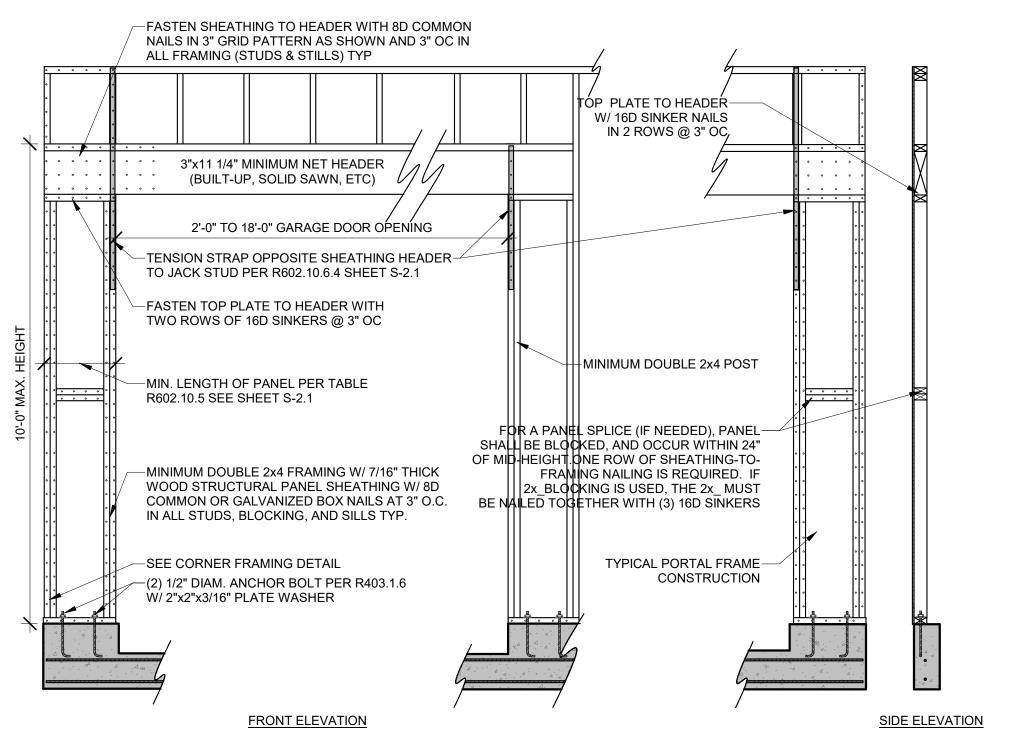
BRACED WALL PANEL CONNECTION WHEN PARALLEL TO FLOOR/CEILING JOISTS

BRACED WALL PANEL CONNECTIONS

1" = 1'-0"



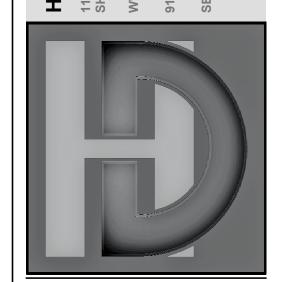
SIDE ELEVATION PFH PORTAL FRAME W/ HOLD DOWNS (R602.10.6.2)



PFG PORTAL FRAME W/OUT HOLD DOWNS (R602.10.6.3)

1/2" = 1'-0"

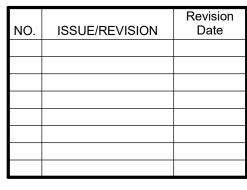
COPYRIGHTED MATERIAL AND CONFIDENTIAL INFORMATION BELONGINING TO HD ENGINEERING. DISSEMINATION, OR DUPLICATION OF CONTAINED HEREIN MAY RESULT IN





39707

08/04/2020 DATE: CHECKED BY: CLS



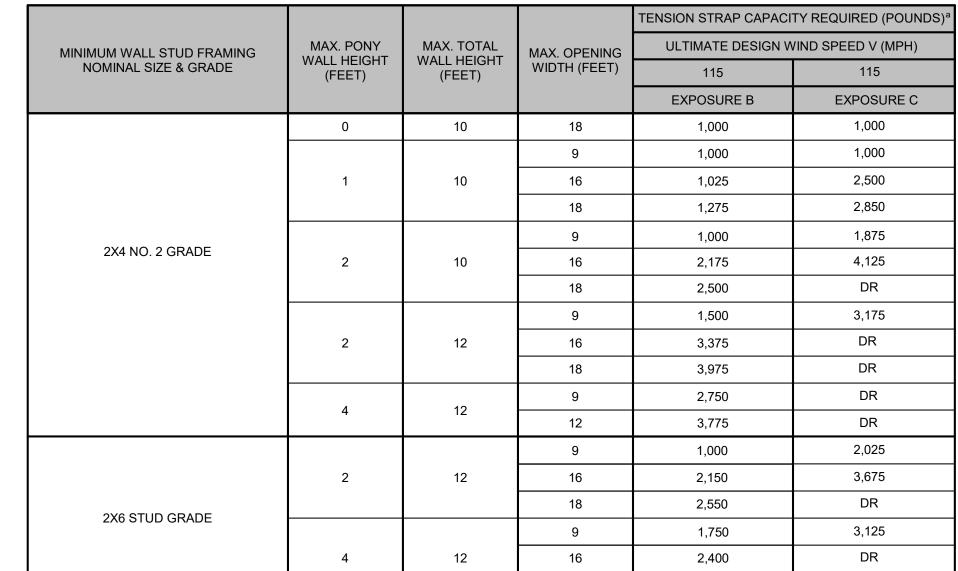
BRACED WALL NOTES & DETAILS

TENSION STRAP CAPACITY REQUIRED FOR RESISTING WIND PRESSURES PERPENDICULAR TO METHOD PFH, PFG AND CS-PF BRACED WALL PANELS IRC2018 TABLE R602.10.6.4 (2) 8D NAILS @ EACH-

8'-0"

9'-0"

10'-0"



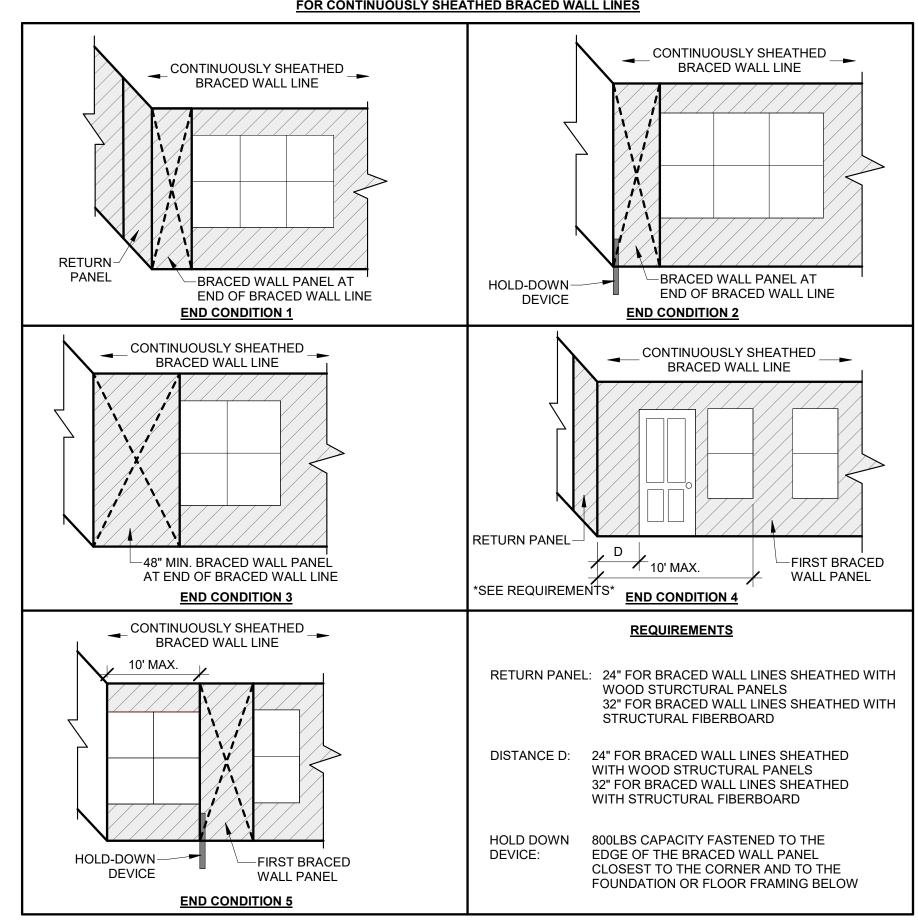
a. DR = DESIGN REQUIRED b. STRAP SHALL BE INSTALLED IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS.

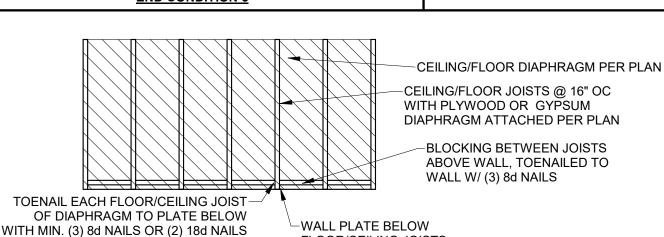
END WALL CONDITIONS

FOR CONTINUOUSLY SHEATHED BRACED WALL LINES

3,800

DR



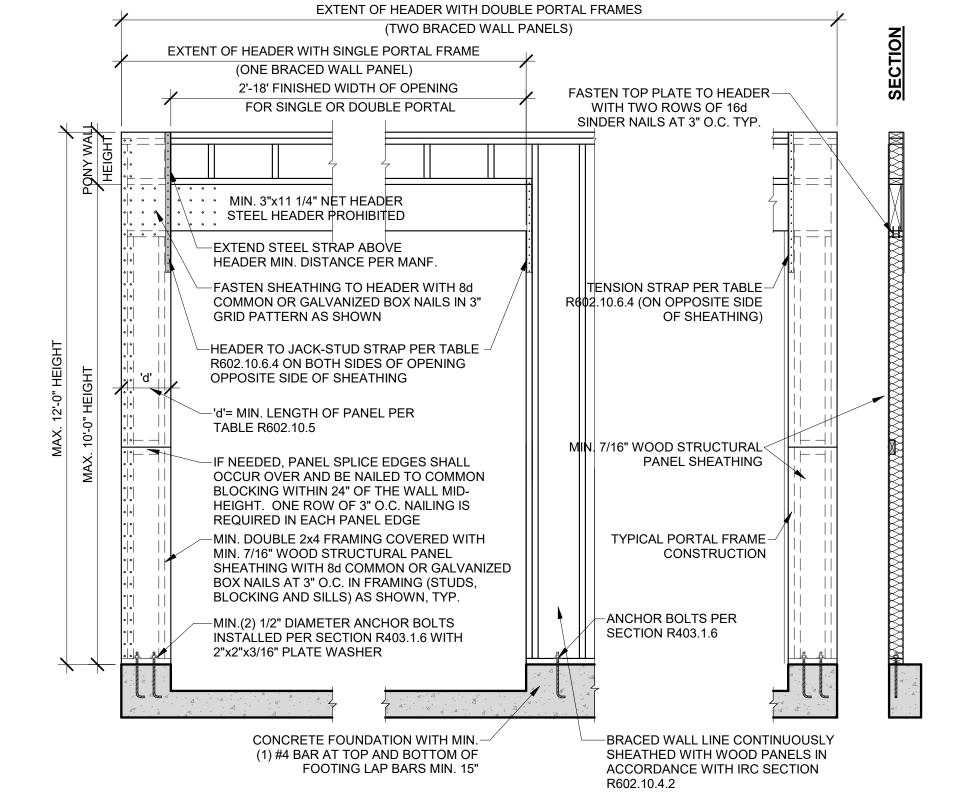


FLOOR/CEILING JOISTS DIAPHRAGM CONNECTION TO INTERIOR WALL

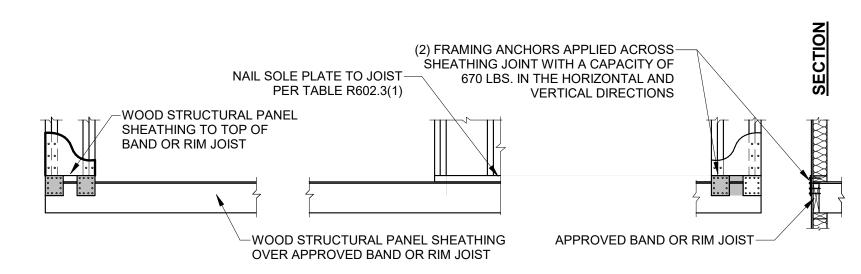
·1/**‡†** GYPSU**M** BOARD **W**/ NO6 - 1 1/4 TYPE "W" OR "S" \$CREWS @ MIN. 410 GYP BOARD BOTH SIDES

GB BRACING

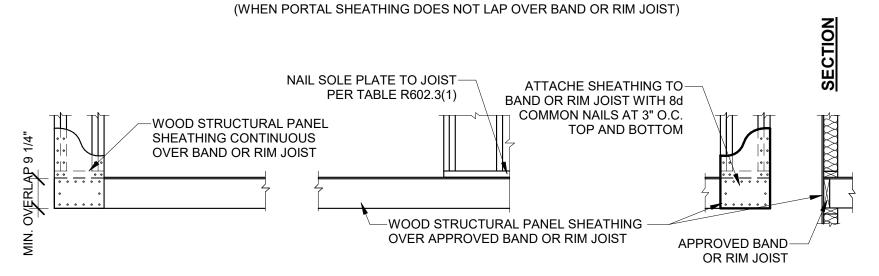
FRONT ELEVATION



OVER CONCRETE OR MASONRY BLOCK FOUNDATION



OVER RAISED WOOD FLOOR - FRAMING ANCHOR OPTION



OVER RAISED WOOD FLOOR - OVERLAP OPTION

(WHEN PORTAL SHEATHING LAPS OVER BAND OR RIM JOIST)

CS-PF

INAUTHORIZED USE, DISCLOSURI SEMINATION, OR DUPLICATION O ANY OF THE INFORMATION NTAINED HEREIN MAY RESULT IN IABILITY UNDER APPLICABLE LAW

THIS DOCUMENT CONTAIN

COPYRIGHTED MATERIAL AND CONFIDENTIAL INFORMATION

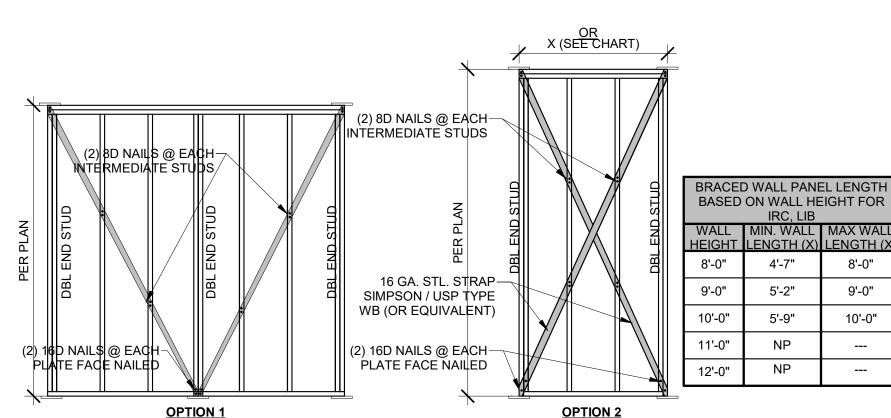




39707

08/04/2020 DATE: CHECKED BY: CLS ISSUE/REVISION

BRACED WALLS NOTES & DETAILS



LIB BRACING

FOR IRC CODE PRESCRIPTIVE METHOD

TABLE R602.10.5 MINIMUM LENGTH OF BRACED WALL PANELS

			MINIMUM	LENGTH	(INCHES)	a		
	METHOD (SEE TABLE R602.10.4)		W	ALL HEIGI	нт		CONTRIBUTING LENGTH (INCHES)	
	(0 //,0 //,0	8 FEET	9 FEET	10 FEET	T 11 FEET 12 FEET		(
DWB,	WSP,SFB,PBS,PCP,HPS,BV-WSP	48	48	48	53	58	ACTUAL ^b	
	GB	48	48	48	53	58	DOUBLE SIDED = ACTUAL SINGLE SIDED=.5xACTUAL	
	LIB	55	62	69	NP	NP	ACTUAL ^b	
A D) A /	SDC A, B, AND C ULTIMATE DESIGN WIND SPEED<140	28	32	34	38	42	40	
ABW	SDC D_0,D_1,D_2 ULTIMATE DESIGN WIND SPEED<140	32	32	34	NP	NP	48	
PFH	SUPPORTING ROOF ONLY	16	16	16	NOTE C	NOTE C	48	
РГП	SPTNG. ONE STORY & ROOF	24	24	24	NOTE C	NOTE C	48	
	PFG	24	27	30	NOTE D	NOTE D	1.5 x ACTUAL ^b	
	CS-G	24	27	30	33	36	ACTUAL ^b	
	CS-PF	16	18	20	NOTE E	NOTE E	ACTUAL ^b	
	ADJACENT CLEAR OPENING HEIGHT (INCHES)							
	≤64	24	27	30	33	36		
	68	26	27	30	33	36		
	72	27	27	30	33	36		
	76	30	29	30	33	36		
	80	32	30	30	33	36		
	84	35	32	32	33	36		
	88	38	35	33	33	36		
	92	43	37	35	35	36		
CS-WSP,	96	48	41	38	36	36	ACTUAL ^b	
CS-SFB	100	-	44	40	38	38	TOTOTE	
	104	-	49	43	40	39		
	108	-	54	46	43	41		
	112	-	-	50	45	43		
-	116	-	-	55	48	45		
	120	-	-	60	52	48		
	124	-	-	-	56	51		
	128	-	-	-	61	54		
	132	-	-	-	66	58		
	136	-	-	-	-	62		
	140	-	-	-	-	66		
	144	-	_	_	-	72		

a. LINEAR INTERPOLATION SHALL BE PERMITTED b. USE THE ACTUAL LENGTH WHEN IT IS GREATER THAN OR EQUAL TO THE MINIMUM LENGTH

 d. MAX. HEADER HEIGHT FOR PFH IS 10' IN ACCORDANCE WITH R602.10.6.2, WALL HEIGHT MAY BE INCREASED TO 12' WITH PONY WALL.
 d. MAX. OPENING HEIGHT FOR PFG IS 10' IN ACCORDANCE WITH R602.10.6.3, WALL HEIGHT MAY BE INCREASED TO 12' WITH PONY WALL. e. MAX. OPENING HEIGHT FOR CS-PF IS 10' IN ACCORDANCE WITH R602.10.6.4, WALL HEIGHT MAY BE INCREASED TO 12' WITH PONY WALL.

BRACED WALL PRESCRIPTIVE METHOD:

CONTINOUS EXTERIOR SHEATHING (CS-WSP) PER WSP METHOD (BELOW) UNLESS OTHERWISE

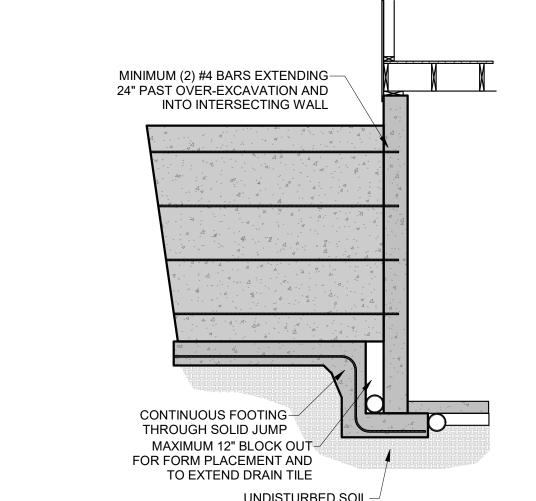
EXTERIOR BRACED WALL METHOD: (SEE ON THIS SHEET)

WOOD STRUCUTRAL PANEL SHEATHING WITH A THICKNESS NOT LESS THAN 3/8" WITH MINIMUM SPAN RATING OF 24/0 FOR 16" O.C. STUD SPACING WITH 6d NAILS COMMON NAILS @ 6" O.C. EDGES AND 12" O.C. FIELD OR SHEATHING THICKNESS NOT LESS THANK 7/16" WITH MINIMUM SPAN RATING OF 24/16 FOR 24" O.C. SPACING WITH 8d COMMON NAILS @ 6" O.C. EDGES AND 12" O.C. IN FIELD (NOTE: FRAMING MEMBERS 16" O.C. MAX, UNBLOCKED, AND W/ SHEATHING APPLIED DIRECTLY TO FRAMING MEMBERS).

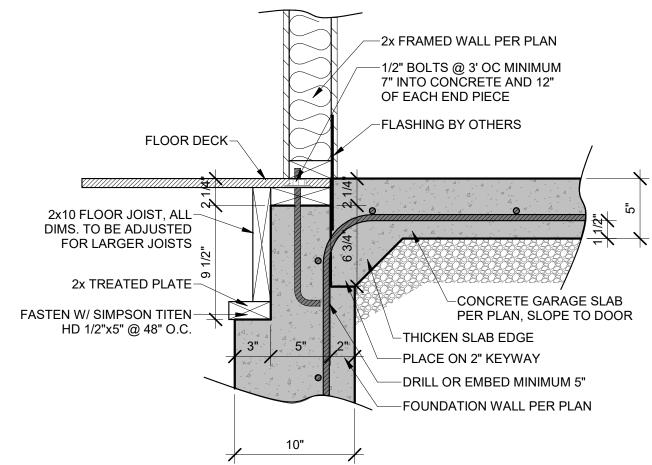
INTERIOR BRACED WALLS (SEE ON THIS SHEET)

1/2" MINIMUM GYPSUM BOARD OVER STUDS SPACED @ 24" MAXIMUM FASTENED W/ #6- 1 1/4" TYPE "W" OR "S" DRYWALL SCREWS @ 7" O.C. EDGES AND FIELD (MIN. 4'-0" SECTION FOR BOTH SIDES)

1X4 WOOD FASTENED W/ (3) 8d COMMON NAILS OR SIMPSON / USP 16 GA. TYPE WB (OR EQUIVALENT) STL. X-BRACE(S) @ 45° TO 60° ANGLES, MAXIMUM 16" O.C. STUDS FASTENED PER MANUF. SPECS.

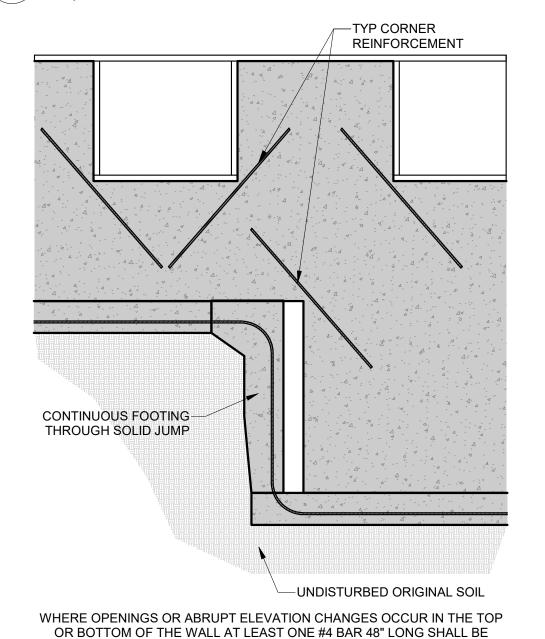


16'-0" MAX. 16'-0" MAX. -RETURN WALL NOT **REQUIRED FOR 5'** TALL OR LESS ☐ IF THE WALL IS NOT SUPPORTED AT THE TOP, PLACE THE FIRST RETURN NOT MORE THAN 8' FROM THE LOW END OF THE STEP. RETURN WALL PLACEMENT



ZERO ENTRY GARAGE DETAIL

UNDISTURBED SOIL-SOLID FOOTING JUMP DETAIL 3/8" = 1'-0"



DIAGONALLY AS CLOSE A PRACTICAL TO THE CORNER

·3500 psi CONCRETE MINIMUM

DRILL/EMBED BARS MIN. 5" INTO

GRADE 40 STEEL MINIMUM ·LAP SPLICES 24" MINIMUM

FOUNDATION WALLS

SLOPE SLAB TO DOORS

REINFORCEMENT AT CORNERS AND STEPS

-FDN. WALL IF 2 CAR OPTION

—5" STRUCTURAL SLAB MINIMUM 3500 PSI CONCRETE #4 BARS @ 12" OC ON 11/2" CHAIRS E/W DRILL AND EMBED

MINIMUM 5" @ FOUNDATION WALLS

-18"x18" SQ. CONC. COL. W/

(4) #4 BARS ON 36"X36"X12"

CONC. PAD W/ (6) #4 BARS

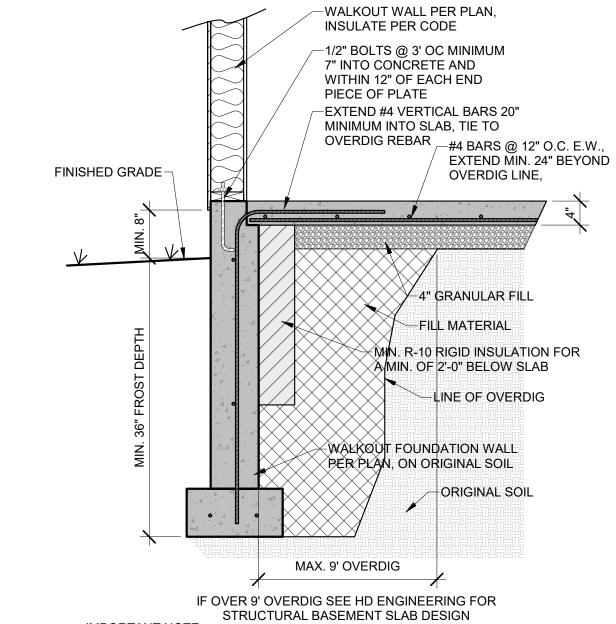
OTHÈRWISE ON THE PLANS

E/W (TYP) UNLESS NOTED

(4) 8'-0" LONG #4 BARS È.W. 1 1/2" CLEAR FROM TOP -5" STRUCTURAL CONCRETE SLAB W/ #4 BARS @ 12" O.C. E.W. ON 1 1/2" CHAIRS -MIN. 4" OF 1/2"-3/4" GRADED ROCK FILL SOIL-18"x18" SQ. CONC. COL. W/ (4) #4 BARS ON 36"X36"X12" CONC. PAD W/ (6) #4 BARS E/W (TYP) UNLESS NOTED OTHERWISE ON THE PLANS

3 GARAGE SLAB COLUMN DETAIL

1" = 1'-0"



ANY SLAB WITH GREATER THAT 2' OF GRADED ROCK OR 8" OF FILL SOIL BELOW SHALL BE DESIGNED AS STRUCTURAL PER PLAN. OUR FIRM SHOULD BE CONTACTED IMMEDIATELY FOR DESIGN RECOMMENDATIONS. DESIGN MUST BE COMPLETED PRIOR TO PLACEMENT OF PIERS OR FOOTINGS.

WALKOUT DETAIL 3/4" = 1'-0"

	CONC
	3000 P
TURAL CONCRETE ER PLAN	3500 P
/	3000 P
	3500 P
	HORIZ
14" OF 1/2"-3/4" DED ROCK	ONE BA
IIN. 5" ION	* CONCRE*
/ALL PER PLAN	CENTER (A

STRUCTURAL SLAB/ WALL

SERUCTURAL CONCRETE SLAB PER PLAN
MIN. 4" OF 1/2"-3/4" GRADED ROCK
─DRILL/EMBED MIN. 5" INTO FOUNDATION —FOUNDATION WALL PER PLAN

VERTICAL REINFORCEMENT SPACING* 60 PSF SOIL; 40 & 60 KSI STEEL 8" THICK WALL 10" THICK WALL CRETE STRENGTH PSI/ 40 KSI 12 PSI/ 40 KSI 12 PSI/ 60 KSI 24 16 24 20

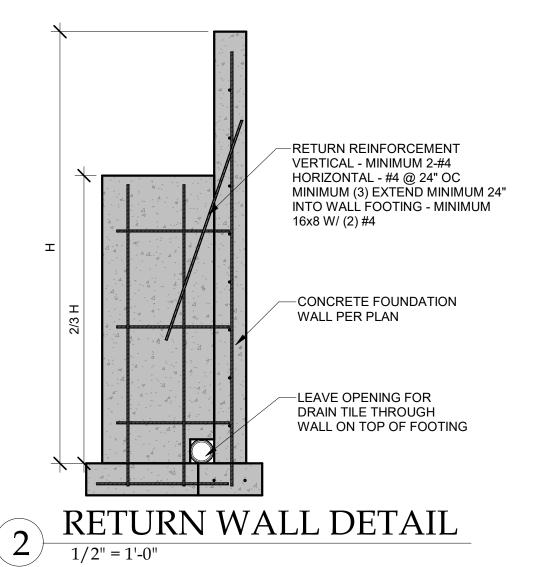
3500 PSI/ 60 KSI	24	16	24	24	16
HORIZONTAL REINFORCEMENT**					
ONE BAR 12" FROM TOP OF WALL; MAX. SPACING 24" O.C.	4- #4	5- #4	4- #4	5- #4	6- #4

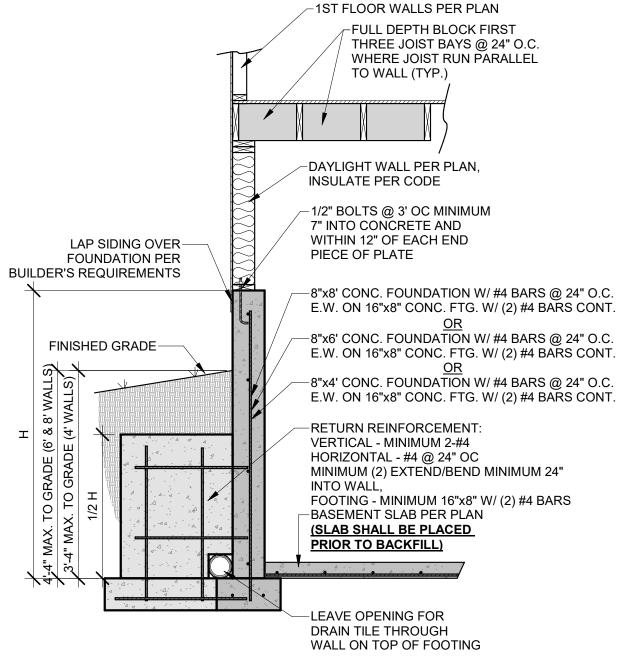
ETE SHALL HAVE AIR ENTRAINMENT OF 5-7%. M REQUIREMENT FOR VERTICAL REBAR IN PLAIN CONCRETE WALLS IS #4 @ 36" ON * VERTICAL BARS SHALL BE CONTINUED UP TO WITHIN 8" OF THE TOP OF THE WALL. * REBAR SHALL BE POSITIONED AT THE TENSION FACE OF THE WALL (2" FROM THE INSIDE

* REINFORCEMENT SHALL LAP A MINIMUM OF 24 INCHES AT ENDS, SPLICES, AND AROUND CORNERS.

** #4 BARS @ 24" ON CENTER. ** #4 BAR WITHIN 12 OF TOP AND BOTTOM OF WALL ** MINIMUM GRADE 40 (40ksi) STEEL (PER ACI 332). ** HORIZONTAL REINFÒRCEMENT SHALL BE INSTALLED ON THE COMPRESSION SIDE (SOIL

SIDE) OF THE VERTICAL REINFORCEMENT

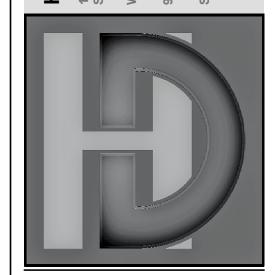




8"x4', 8"x6', AND 8"x8' DAYLIGHT FOUNDATION IF SLAB IS NOT PLACED PRIOR TO BACKFILL CONTRACTOR IS RESPONSIBLE FOR BRACING THE FOUNDATION AS REQUIRED

UNRESTRAINED FOUNDATION WALL

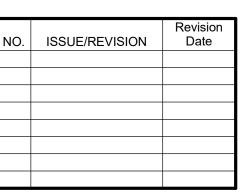
THIS DOCUMENT CONTAINS COPYRIGHTED MATERIAL AND CONFIDENTIAL INFORMATION BELONGINING TO HD ENGINEERIN INAUTHORIZED USE, DISCLOSURE ISSEMINATION OR DUPLICATION O ANY OF THE INFORMATION CONTAINED HEREIN MAY RESULT IN IABILITY UNDER APPLICABLE LAW





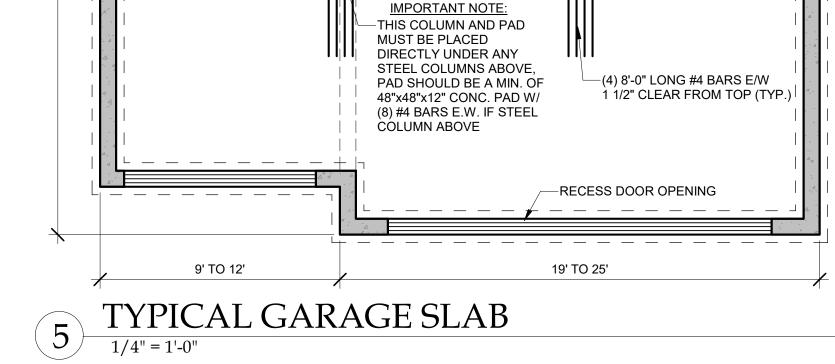
39707

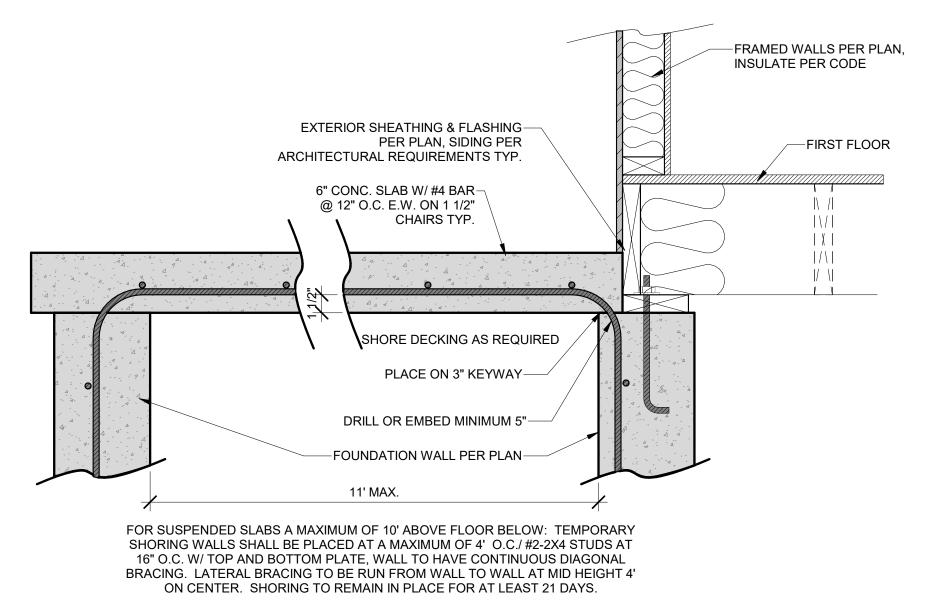
08/04/2020 DATE: CHECKED BY: CLS



CONCRETE DETAILS

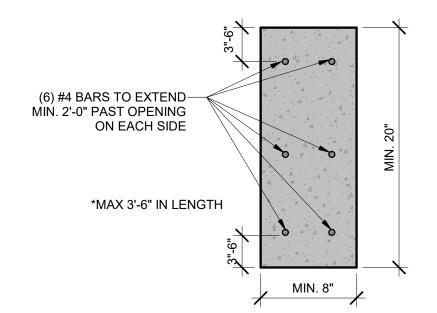
© 2020 HD ENGINEERING & DESIGN





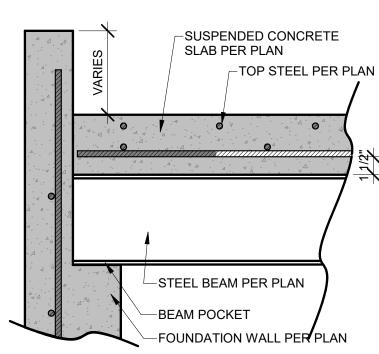
6 SUSPENDED PORCH STOOP SLAB

1 1/2" = 1'-0"



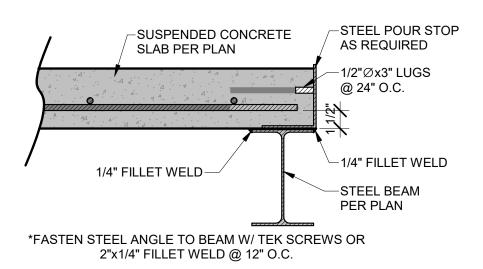
5 CONCRETE HEADER DETAIL

1 1/2" = 1'-0"



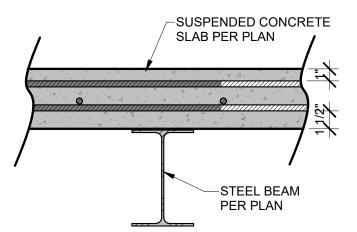
SUSPENDED SLAB BEAM/WALL CONNECTION

1 1/2" = 1'-0"



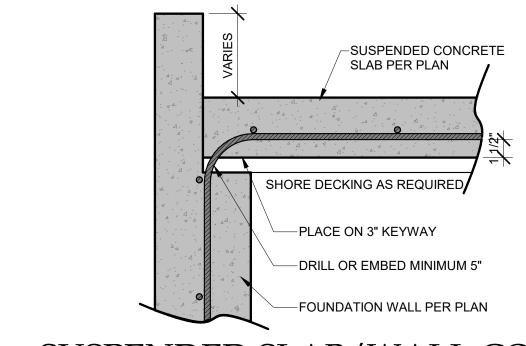
2 SUSPENDED SLAB POUR STOP

1 1/2" = 1'-0"



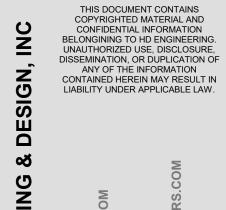
3 SUSPENDED SLAB/STEELBEAM CROSS SECTION

1 1/2" = 1'-0"



SUSPENDED SLAB/WALL CONNECTION

1 1/2" = 1'-0"



HU ENGINEERING & 11656 W. 75TH STREET SHAWNEE, KS 66214 WWW.HDENGINEERS.COM 913.631.2222 SERVICE@HDENGINEERS.COM





STION, LLC
SOK PLAN
LEE'S SUMMIT, MO

SAB CONSTRUCTION E716 - ELLENBROOK PA

HD#: 39707

DATE: 08/04/2020

CHECKED BY: CLS

NO.	ISSUE/REVISION	Revision Date
140.	IOOOL/INE VIOIOIN	Date

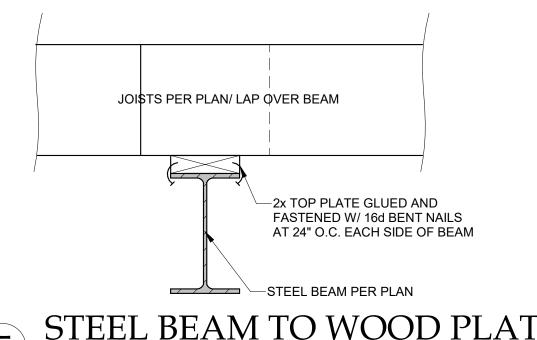
SUSPENDED SLAB DETAILS

S-3.1

© 2020 HD ENGINEERING & DESIGN

IMPORTANT NOTE:

-FOR SUSPENDED SLABS A MAXIMUM OF 10' ABOVE FLOOR BELOW: TEMPORARY SHORING WALLS SHALL BE PLACED AT A MAXIMUM OF 4'
O.C./#2-2X4 STUDS AT 16" O.C. W/ TOP AND BOTTOM PLATE, WALL TO HAVE CONTINUOUS DIAGONAL BRACING. LATERAL BRACING TO BE
RUN FROM WALL TO WALL AT MID HEIGHT 4' ON CENTER. SHORING TO REMAIN IN PLACE FOR AT LEAST 21 DAYS.
-ANY CAST IN PLACE SLABS FORMED MORE THAN 10' ABOVE THE FLOOR BELOW SHALL HAVE A SITE SPECIFIC SHORING DESIGN DONE. OUR
FIRM SHOULD BE CONSULTED FOR THIS DESIGN ONCE FOUNDATION WALLS ARE IN PLACE TO EVALUATE ALL FIELD CONDITIONS. IT SHOULD
BE NOTED THAT FAILURE TO HAVE AN ADEQUATE SHORING DESIGN CAN RESULT IN FORM COLAPSE AND/OR CATASTROPHIC FAILURE.

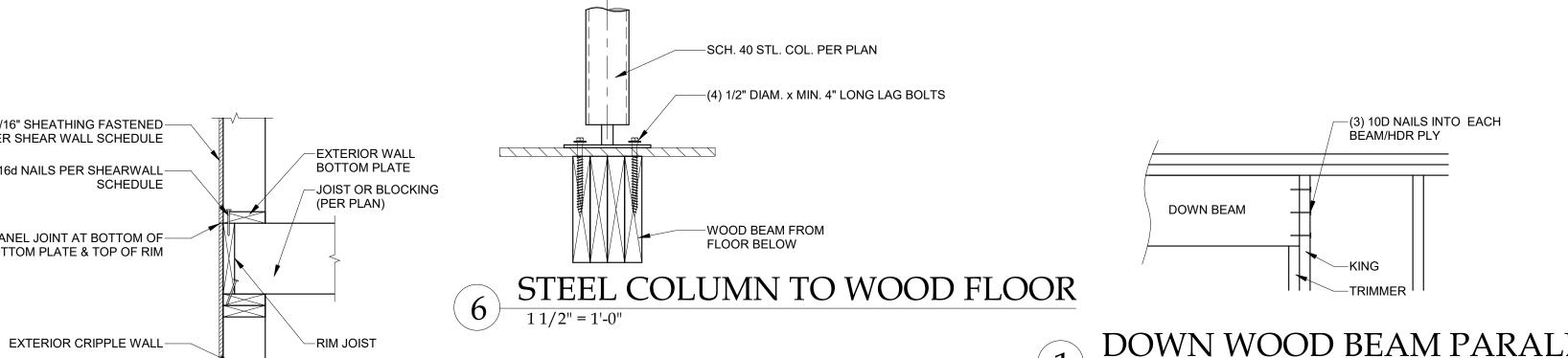


STEEL BEAM TO WOOD PLATE

FLUSH WOOD BEAM CONNECTION

EXTERIOR WALL STEEL BEAM BEARING

DOWN WOOD BEAM PERPENDICULAR



SHEATHING JOINT LOCATION

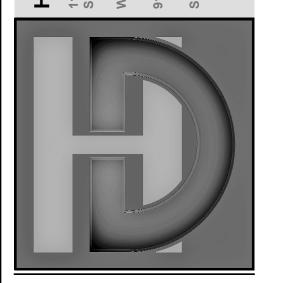
1" = 1'-0"

DOWN WOOD BEAM PARALLEL

1" = 1'-0"

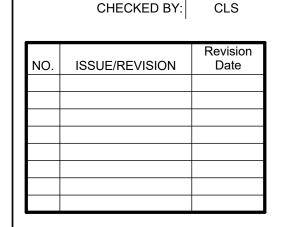
COPYRIGHTED MATERIAL AND CONFIDENTIAL INFORMATION BELONGINING TO HD ENGINEERING. UNAUTHORIZED USE, DISCLOSURE, DISSEMINATION, OR DUPLICATION OF ANY OF THE INFORMATION CONTAINED HEREIN MAY RESULT IN LIABILITY UNDER APPLICABLE LAW.

THIS DOCUMENT CONTAINS





39707 08/04/2020 DATE:



GENERAL DETAILS