

MiTek USA, Inc. 16023 Swingley Ridge Rd Chesterfield, MO 63017 314-434-1200

Re: MN52 SUMMIT HOMES

The truss drawing(s) referenced below have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Mid America MO.

Pages or sheets covered by this seal: I41706564 thru I41706585

My license renewal date for the state of Missouri is December 31, 2021.

Missouri COA: Engineering 001193



June 18,2020

Sevier, Scott

,Engineer

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.





REACTIONS. (size) 4=Mechanical, 2=0-4-9 Max Horz 2=88(LC 9) Max Uplift 4=-24(LC 12), 2=-61(LC 8) Max Grav 4=283(LC 2), 2=404(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Wind: ASCE 7-10; Vult=115mph (3-second gust) V(IRC2012)=91mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope); cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33
- 2) TCLL: ASCE 7-10; Pr=25.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat
- roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp C; Partially Exp.; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 5) Plates checked for a plus or minus 2 degree rotation about its center.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 2.

9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 25 lb down and 36 lb up at 4-1-7, and 25 lb down and 36 lb up at 4-1-7 on top chord, and 10 lb down at 4-1-7, and 10 lb down at 4-1-7 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)
 - Vert: 1-3=-51, 2-4=-20



June 18,2020





Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPIT Quality Criteria**, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

MITEK[®] 16023 Swingley Ridge Rd Chesterfield, MO 63017

Job	Truss	Truss Type	Qty	Ply	SUMMIT HOMES	
						l41706565
MN52	H1	Hip Girder	1	2		
				-	Job Reference (optional)	
Mid America Truss,	Jefferson City, MO - 65101,		8.	410 s May	22 2020 MiTek Industries, Inc. Thu Jun 18 08:25:20 2020	Page 2

ID:iWyqRbhQKfmME38ILt45uHzsXkA-?nYQsFtqtJ?NGqIrmsaiKNxmYY?EpIVda3TgiEz562z

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-3=-51, 3-4=-61, 4-6=-51, 2-5=-20 Concentrated Loads (lb)

Vert: 8=-359(B) 7=-359(B)





	•		4-11-4					
LOADING (psf) TCLL (roof) 25.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2012/TPI2007	CSI. TC 0.45 BC 0.27 WB 0.00 Matrix-P	DEFL. ir Vert(LL) -0.03 Vert(TL) -0.07 Horz(TL) -0.00	n (loc) 2-4 2-4 4	l/defl >999 >798 n/a	L/d 360 240 n/a	PLATES MT20 Weight: 20 lb	GRIP 244/190 FT = 3%

LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.2

BRACING-TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 4-11-4 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 4=Mechanical, 2=0-3-8 Max Horz 2=86(LC 11) Max Uplift 4=-17(LC 12), 2=-14(LC 12) Max Grav 4=201(LC 2), 2=288(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=115mph (3-second gust) V(IRC2012)=91mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed;
- MWFRS (envelope); cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33
- 2) TCLL: ASCE 7-10; Pr=25.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat
- roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp C; Partially Exp.; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 5) Plates checked for a plus or minus 2 degree rotation about its center.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 2.







			2-10-15						
LOADING (psf) TCLL (roof) 25.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2012/TPI2007	CSI. TC 0.11 BC 0.08 WB 0.00 Matrix-P	DEFL. Vert(LL) Vert(TL) Horz(TL)	in -0.00 -0.01 -0.00	(loc) 2-4 2-4 3	l/defl >999 >999 n/a	L/d 360 240 n/a	PLATES MT20 Weight: 11 lb	GRIP 244/190 FT = 3%

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 2-10-15 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 3=Mechanical, 2=0-3-8, 4=Mechanical Max Horz 2=51(LC 12)

Max Uplift 3=-31(LC 12), 2=-8(LC 12)

Max Grav 3=81(LC 2), 2=207(LC 2), 4=54(LC 7)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=115mph (3-second gust) V(IRC2012)=91mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed;
- MWFRS (envelope); cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33
- 2) TCLL: ASCE 7-10; Pr=25.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp C; Partially Exp.; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 5) Plates checked for a plus or minus 2 degree rotation about its center.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) Refer to girder(s) for truss to truss connections.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2.







						6-0-0						
Plate Offsets (X,Y) [2:0-0-12	,0-2-0], [4:Edge,0-1-14]										
LOADING (ps TCLL (roof) Snow (Pf/Pg) TCDL	sf) 25.0 15.4/20.0 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.15 1.15 YES	CSI. TC BC WB	0.73 0.13 0.00	DEFL. Vert(LL) Vert(TL) Horz(TL)	in -0.01 -0.04 -0.00	(loc) 2-4 2-4 4	l/defl >999 >999 n/a	L/d 360 240 n/a	PLATES MT20	GRIP 244/190
BCDL	10.0	Code IRC2012/TF	912007	Matri	x-P						Weight: 28 lb	FT = 3%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x6 SP No.1WEBS2x4 SP No.2

REACTIONS. (size) 2=0-3-0, 4=0-1-8

Max Horz 2=91(LC 9) Max Uplift 2=-16(LC 12), 4=-20(LC 12) Max Grav 2=334(LC 2), 4=252(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Wind: ASCE 7-10; Vult=115mph (3-second gust) V(IRC2012)=91mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed;

- MWFRS (envelope); cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33 2) TCLL: ASCE 7-10; Pr=25.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat
- roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp C; Partially Exp.; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 5) Plates checked for a plus or minus 2 degree rotation about its center.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.



Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing

except end verticals.



MS2 M2 Monoplich 4 1 DB Reference (opticnal) H1706569 Mid America Truss, Jefferson City, M0 - 65101, B. 41 Monoplich B. 41 Monoplich B. 41 Monoplich B. 41 Monoplich B. 42 Monoplich B. 44 Monoplich B	Job	Truss	Truss Type		Qty	Ply	SUMM	IT HOMES	S		
Mid America Truss. Jefferson City, MO - 65101. 8.410 S May 22020 MTR Houstries, Inc. Thu Jun 18 08:25:23 2020 Page 1 ID:WyGRbnCKmMESBIL45UH25XLA-PLDYVHwiAENy7IUGR78Py72ML11d0G3G1bKLY:2562w 1.6-0 ID:WyGRbnCKmMESBIL45UH25XLA-PLDYVHwiAENy7IUGR78Py72ML11d0G3G1bKLY:2562w ID:WyGRbnCKmMESBIL45UH25XLA-PLDYVHwiAENy7IUGR78Py72ML11d0G3G1bKLY:2562w ID:WyGRbnCKmMESBIL45UH25XLA-PLDYVHwiAENy7IUGR78Py72ML11d0G3G1bKLY:2562w ID:WyGRbnCKmMESBIL45UH25XLA-PLDYVHwiAENy7IUGR78Py72ML11d0G3G1bKLY:2562w ID:WyGRbnCKmMESBIL45UH25XLA-PLDYVHwiAENy7IUGR78Py72ML11d0G3G1bKLY:2562w ID:WyGRbnCKmMESBIL45UH25XLA-PLDYVHwiAENy7IUGR78Py72ML11d0G3G1bKLY:2562w ID:WyGRbnCKmMESBIL45UH25XLA-PLDYVHwiAENy7IUGR78Py72ML11d0G3G1bKLY:2562w ID:WyGRbnCKmMESBIL45UH25XLA-PLDYVHwiAENy7IUGR78Py72ML11d0G3G1bKLY:2562w ID:WyGRbnCKmMESBIL45UH25XLA-PLDYVHwiAENy7IUGR78Py72ML11d0G3G1bKLY:2562w ID:WyGRbnCKmMESBIL45UH25WL 5.000 T2 ID:WyGRbnCKmMESBIL45UH25WL 4 ID:WyGRbnCKmMESBIL45UH25WL 4 ID:WyGRbnCKmMESBIL45UH25WL 4 ID:WyGRbnCKmMESBIL45UH25WL 4 ID:WyGRbnCKmMESBIL45UH25WL 5 ID:WyGRbnCKmMESBIL45UH25WL 5 ID:WyGRbnCKmMESBIL45UH25WL 4 ID:WyGRbnCKmMESBIL45UH25WL 5 ID:WyGRbnCKmMESBIL45UH25WL 5<	MN52	M2	Monopitch		4	1					141706569
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State - 1.00 State - 1.00 State - 1.01 State - 1.01			0-4-8	<u>1-6-0</u> 1-6-0							
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Long (psf) TCLL (roo) SPACING- 16-0 2-0-0 16-0 CSI. DEFL. in (loc) l/deft L/d PLATES GRIP MI20 TCLL (roo) 25.0 Snow (Pf/Pg) SPACING- 1.15 Rep Stress Incr 2-0-0 1.15 Rep Stress Incr CSI. DEFL. in (loc) l/deft L/d MI20 244/190 UMBER- LUMBER- LUMBER- TOP CHORD 2x4 SP No.2 Structural wood sheathing directly applied or 1-6-0 oc purlins,				/			\square				
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3x4 = 2x4 1-6-0 1-6-0 LOADING (psf) TCLL (roof) SPACING- 25.0 Snow (Pf/Pg) 2-0-0 Plate Grip DOL CSI. DEFL. in (loc) I/deft L/d Now (Pf/Pg) 15.4/20.0 TCDL Plate Grip DOL 1.15 Lumber DOL TC 0.03 BCDL Vert(LL) -0.00 2 >999 360 Vert(TL) MT20 244/190 MT20 244/190 Vert(TL) -0.00 2 >999 240 Horz(TL) MT20 244/190 BCLL 0.0 BCDL 10.0 Code IRC2012/TPI2007 WB 0.00 Matrix-P BRACING- TOP CHORD Structural wood sheathing directly applied or 1-6-0 oc purlins,											
LOADING (psf) SPACING- 2-0-0 CSI. DEFL. in (loc) l/defl L/d PLATES GRIP TCLL (roof) 25.0 Plate Grip DOL 1.15 TC 0.03 Vert(LL) -0.00 2 >999 360 MT20 244/190 TCDL 10.0 BCL 0.0 BC 0.02 Vert(TL) -0.00 2 >999 240 BCLL 0.0 Code IRC2012/TPI2007 WB 0.00 Matrix-P Weight: 6 lb FT = 3% LUMBER- TOP CHORD 2x4 SP No.2 TOP CHORD Structural wood sheathing directly applied or 1-6-0 oc purlins,			3x4 =				2x4				
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SPACING 25.0 SPACING- 2-0-0 CSI. DEFL. in (loc) //defl L/d PLATES GRIP TCLL (roof) 25.0 Plate Grip DOL 1.15 TC 0.03 Vert(LL) -0.00 2 >999 360 MT20 244/190 TCDL 10.0 Rep Stress Incr YES WB 0.00 Vert(TL) -0.00 2 >999 240 MT20 244/190 BCLL 0.0 Rep Stress Incr YES WB 0.00 Matrix-P Weight: 6 lb FT = 3% LUMBER- TOP CHORD 2x4 SP No.2 TOP CHORD Structural wood sheathing directly applied or 1-6-0 oc purlins,				1-0-0							
Snow (Pf/Pg) 15.4/20.0 TCDL Plate Grp DOL 1.15 Lumber DOL IC 0.03 BCL Vert(LL) -0.00 2 >999 360 M120 244/190 TCDL 10.0 BCLL 0.0 BCDL 10.0 Rep Stress Incr YES Code IRC2012/TPI2007 WB 0.00 Matrix-P Vert(TL) -0.00 2 >999 240 LUMBER- TOP CHORD 2x4 SP No.2 BRACING- TOP CHORD Structural wood sheathing directly applied or 1-6-0 oc purlins, Weight: 6 -0 oc purlins,	TCLL (roof) 25.0	SPACING-	2-0-0 CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCDL 10.0 Rep Stress Incr YES WB 0.00 Horz(TL) -0.00 4 n/a BCDL 10.0 Code IRC2012/TPI2007 WB 0.00 Matrix-P Horz(TL) -0.00 4 n/a n/a LUMBER- TOP CHORD 2x4 SP No.2 Structural wood sheathing directly applied or 1-6-0 oc purlins, BRACING- TOP CHORD Structural wood sheathing directly applied or 1-6-0 oc purlins,	Snow (Pf/Pg) 15.4/20.0	Plate Grip DOL	1.15 IC 1.15 BC	0.03	Vert(LL)	-0.00	2	>999	360 240	MT20	244/190
BCDL 0.0 Code IRC2012/TPI2007 Matrix-P Weight: 6 lb FT = 3% LUMBER- TOP CHORD 2x4 SP No.2 BRACING- TOP CHORD Structural wood sheathing directly applied or 1-6-0 oc purlins,	TCDL 10.0	Rep Stress Incr	YES WB	0.00	Horz(TL)	-0.00	4	n/a	n/a		
LUMBER- BRACING- TOP CHORD 2x4 SP No.2 TOP CHORD Structural wood sheathing directly applied or 1-6-0 oc purlins,	BCDL 10.0	Code IRC2012	TPI2007 Matrix-	P						Weight: 6 lb	FT = 3%
TOP CHORD 2x4 SP No.2 TOP CHORD Structural wood sheathing directly applied or 1-6-0 oc purlins,	LUMBER-			BRACIN	3-						
	TOP CHORD 2x4 SP N	0.2		TOP CHO	ORD	Structura	al wood	sheathing	g directly ap	plied or 1-6-0 oc purli	ns,
BUT CHURD 2X4 SP No.2 except end verticals. WEBS 2x4 SP No.2 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing	WFBS 2x4 SP N	0.2 o 2		BOT CHO)RD	except e Rigid ce	end vertionalized	cais. ectiv appli	ed or 10-0-0) oc bracing	
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REACTIONS. (size) 4=Mechanical, 2=0-4-0 Max Horz 2=27(LC 9) Max Uplift 4=-5(LC 12), 2=-5(LC 12) Max Grav 4=55(LC 2), 2=92(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=115mph (3-second gust) V(IRC2012)=91mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed;
- MWFRS (envelope); cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33
- 2) TCLL: ASCE 7-10; Pr=25.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat
- roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp C; Partially Exp.; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 5) Plates checked for a plus or minus 2 degree rotation about its center.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 2.







roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp C; Partially Exp.; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.

6) Plates checked for a plus or minus 2 degree rotation about its center.

- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building designe. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.



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- 10) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 28, 23, 24, 25, 26, 27, 21, 20, 18, 17, 16, 14.



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6) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.

7) Plates checked for a plus or minus 2 degree rotation about its center.

8) Gable requires continuous bottom chord bearing.

9) Gable studs spaced at 2-0-0 oc.

10) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 25, 26, 27, 28, 29, 23, 22, 20, 19, 18.



June 18,2020





5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.

6) Plates checked for a plus or minus 2 degree rotation about its center.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8.







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3) TCLL: ASCE 7-10; Pr=25.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat

roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp C; Partially Exp.; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.

6) Plates checked for a plus or minus 2 degree rotation about its center.

- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8.







June 18,2020





June 18,2020





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MiTek

Job	Truss	Truss Type	Qty	Ply	SUMMIT HOMES	
						141706579
MN52	T3GR	Roof Special Girder	1	ົ		
				2	Job Reference (optional)	
Mid America Truss, Jef	ferson City, MO - 65101,		8.	410 s May	22 2020 MiTek Industries, Inc. Thu Jun 18 08:25:36 2020	Page 2
		ID:iWyql	RbhQKfmN	/E38ILt45	uHzsXkA-XsWTDj4s6E06BIzwiEsS kbWZ?NsZM5 FYLW0	GIz562j

LOAD CASE(S) Standard

Uniform Loads (plf)

Vert: 1-3=-51, 3-5=-51, 1-5=-20

Concentrated Loads (lb)

Vert: 6=-900(B) 11=-900(B) 12=-900(B) 13=-900(B) 14=-900(B) 15=-900(B) 16=-900(B) 17=-900(B) 18=-900(B) 19=-901(B)





- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 7) Plates checked for a plus or minus 2 degree rotation about its center.
- 8) Gable requires continuous bottom chord bearing.
- 9) Gable studs spaced at 2-0-0 oc.
- 10) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 16, 17, 18, 14, 13, 12.



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6) Gable requires continuous bottom chord bearing.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8, 6.



🙏 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPIT Quality Criteria**, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

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1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=115mph (3-second gust) V(IRC2012)=91mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope); cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33
3) TCLL: ASCE 7-10; Pr=25.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat

roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp C; Partially Exp.; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) Plates checked for a plus or minus 2 degree rotation about its center.

6) Gable requires continuous bottom chord bearing.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.







REACTIONS. (size) 1=6-8-2, 3=6-8-2, 4=6-8-2 Max Horz 1=-35(LC 8) Max Uplift 1=-12(LC 12), 3=-16(LC 13) Max Grav 1=134(LC 2), 3=134(LC 2), 4=242(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=115mph (3-second gust) V(IRC2012)=91mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (envelope); cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33

- 3) TCLL: ASCE 7-10; Pr=25.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=20.0 psf (ground snow); Pf=15.4 psf (flat
- roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp C; Partially Exp.; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) Plates checked for a plus or minus 2 degree rotation about its center.

6) Gable requires continuous bottom chord bearing.

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- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.





