



May 29, 2020

Don Julian Builders

Re: 2924 NW Audubon Ln  
Lot 1450 Winterset Valley  
Lee's Summit, MO

Apex Engineers Inc. observed the house under construction at the above referenced address. For the purposes of this report the house will be referred to as facing south. Our firm has been retained to address the following items from the rough-in framing inspection:

1. *Rafters bearing on multiple top plates on wall at east side of single car garage.*
  - The existing rafters, top plates and ceiling joists shall be fastened as shown in detail **2/SX-1**, attached to this report for reference. This detail is also shown on **11/S3.1** on the original plans.
2. *Missing (6)-2x4 studs below (3)-2x10 in wall between entry and garage.*
  - Install a built-up wood column consisting of a minimum of (3)-2x6 studs, in lieu of the (6)-2x4 stud pack shown on the original plans.
  - Each 2x6 ply shall be fastened with 1 row of 10d common nails (0.128"x3') at 9" on-center alternating side to side, 1.4" min edge distance, and starting 2.5" from each end.
  - See detail **1/S3.1** for clarity for fastening of built-up wood column plies.
  - The existing (3) #2-2x10 beam spanning over the garage may be fastened into the broad side of the (3)-2x6 built-up wood column with a Simpson LUS210-3 face-mount joist hanger. Fasten hanger into the column/blocking with (8) 16d common nails (0.162"x3 1/2") and fasten the hanger into the joist with (6) 16d common nails (0.162"x3 1/2").

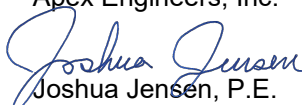
This report does not constitute approval by the enforcing jurisdiction. Please call if our firm can of further assistance.

#### LIMITATIONS

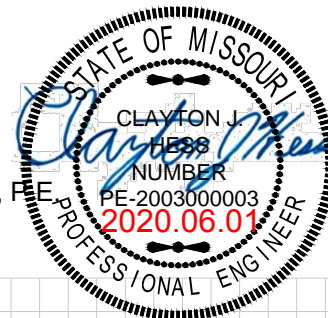
The scope of our services includes only those items specifically addressed herein. All other items are outside the scope of this inspection; including any environmental assessment (such as, but not limited to mold, mildew, or presence of any other toxic substance or environmental risks).

In addition, the scope our services does not include any evaluation of the building or site for job-site safety and/or hazardous conditions. All construction shall be performed in compliance with IRC and OSHA standards at all times. Our firm has not been retained to examine the site or building for any of these conditions. In addition, the contractor shall retain sole responsibility for the quality of work, for adhering to plans, specifications, appropriate codes, and, for repairing defects, deficiencies or omission, regardless of when they are found. By the use of this report it is understood the above conditions are agreed to.

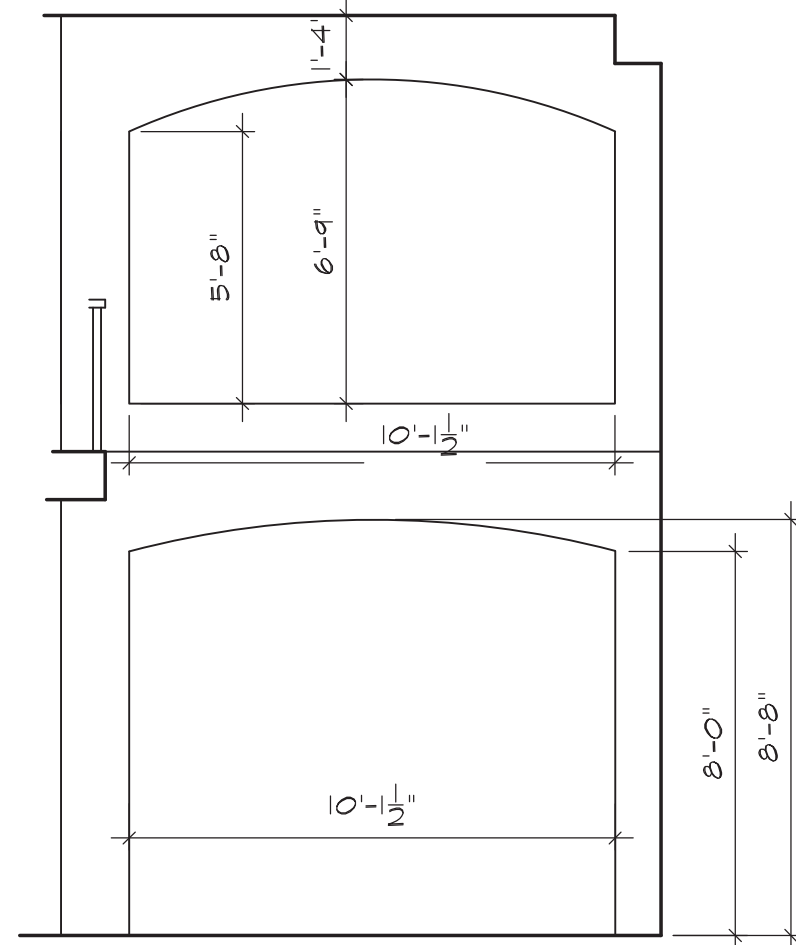
Best Regards,  
Apex Engineers, Inc.

  
Joshua Jensen, P.E.  
Project Engineer

Clayton J. Hess, P.E.  
Principal

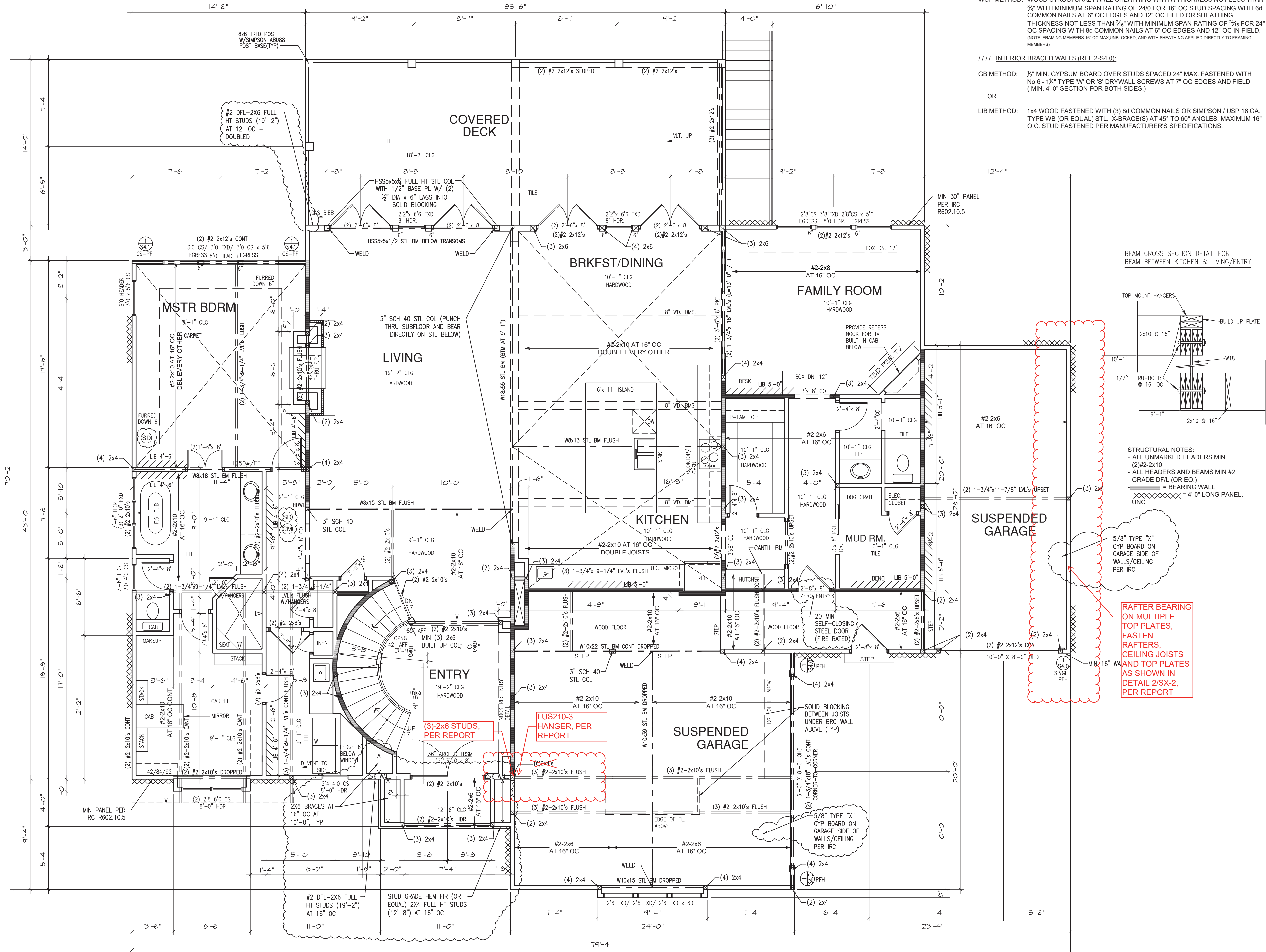


(SD) SMOKE DETECTOR  
(CM) CARBON MONOXIDE DETECTOR



ENTRY DETAIL

SQUARE FOOTAGE:  
Living Area - 1ST Flr: 2552  
Living Area - 2ND Flr: 21822  
Un-Finished basements: 1997  
Deck: 490  
Garage: 720



BRACED WALL METHODOLOGY  
CONTINUOUS EXTERIOR SHEATHING PER WSP METHOD (BELOW)  
UNLESS OTHERWISE NOTED ON THE PLAN

XXXX EXTERIOR BRACED WALLS:

WSP METHOD: WOOD STRUCTURAL PANEL SHEATHING WITH A THICKNESS NOT LESS THAN 3/4" WITH MINIMUM SPAN RATING OF 24/0 FOR 16" OC STUD SPACING WITH 6d COMMON NAILS AT 6" OC EDGES AND 12" OC FIELD OR SHEATHING THICKNESS NOT LESS THAN 7/8" WITH MINIMUM SPAN RATING OF 24/0 FOR 24" OC SPACING WITH 8d COMMON NAILS AT 6" OC EDGES AND 12" OC IN FIELD.  
(NOTE: FRAMING MEMBERS 16" OC MAX UNBLOCKED, AND WITH SHEATHING APPLIED DIRECTLY TO FRAMING MEMBERS)

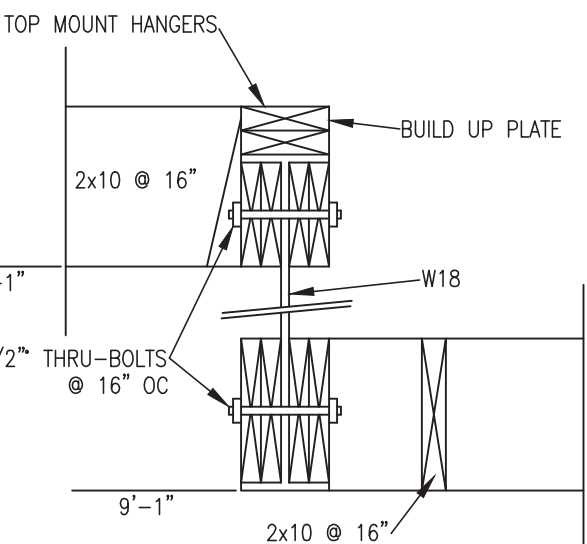
//// INTERIOR BRACED WALLS (REF 2-S4.0):

GB METHOD: 1/2" MIN. GYPSUM BOARD OVER STUDS SPACED 24" MAX. FASTENED WITH NO 6 - 1 1/2" TYPE 'W' OR 'S' DRYWALL SCREWS AT 7" OC EDGES AND FIELD (MIN. 4'-0" SECTION FOR BOTH SIDES.)

OR

LIB METHOD: 1x4 WOOD FASTENED WITH (3) 8d COMMON NAILS OR SIMPSON / USP 16 GA. TYPE WB (OR EQUAL) STL. X-BRACE(S) AT 45° TO 60° ANGLES, MAXIMUM 16" O.C. STUD FASTENED PER MANUFACTURER'S SPECIFICATIONS.

BEAM CROSS SECTION DETAIL FOR  
BEAM BETWEEN KITCHEN & LIVING/ENTRY



STRUCTURAL NOTES:  
- ALL UNMARKED HEADERS MIN (2) #2-2x10  
- ALL HEADERS AND BEAMS MIN #2 GRADE DFL (OR EQ.)  
- = BEARING WALL  
- = 4'-0" LONG PANEL, UNO

5/8" TYPE 'X' GYP BOARD ON GARAGE SIDE OF WALLS/CeILING PER IRC

FIRST FLOOR PLAN

SCALE: 1/4" = 1'-0"

2,552 SQ FT

LINDSTROM RESIDENCE - MADISON II  
LOT 1450 WINTERSET VALLEY

PROJECT #

18032

DRAWING NAME

18032F1

DATE

8-20-19

DRAWN BY

RRF

CHECKED BY

042616

SHEET #

2

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BUILDERS, INC.  
Custom Homes Since 1974  
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15521 West 110th Street  
Lenexa, Kansas 66219  
(913) 894-6300  
(913) 894-6321 fax

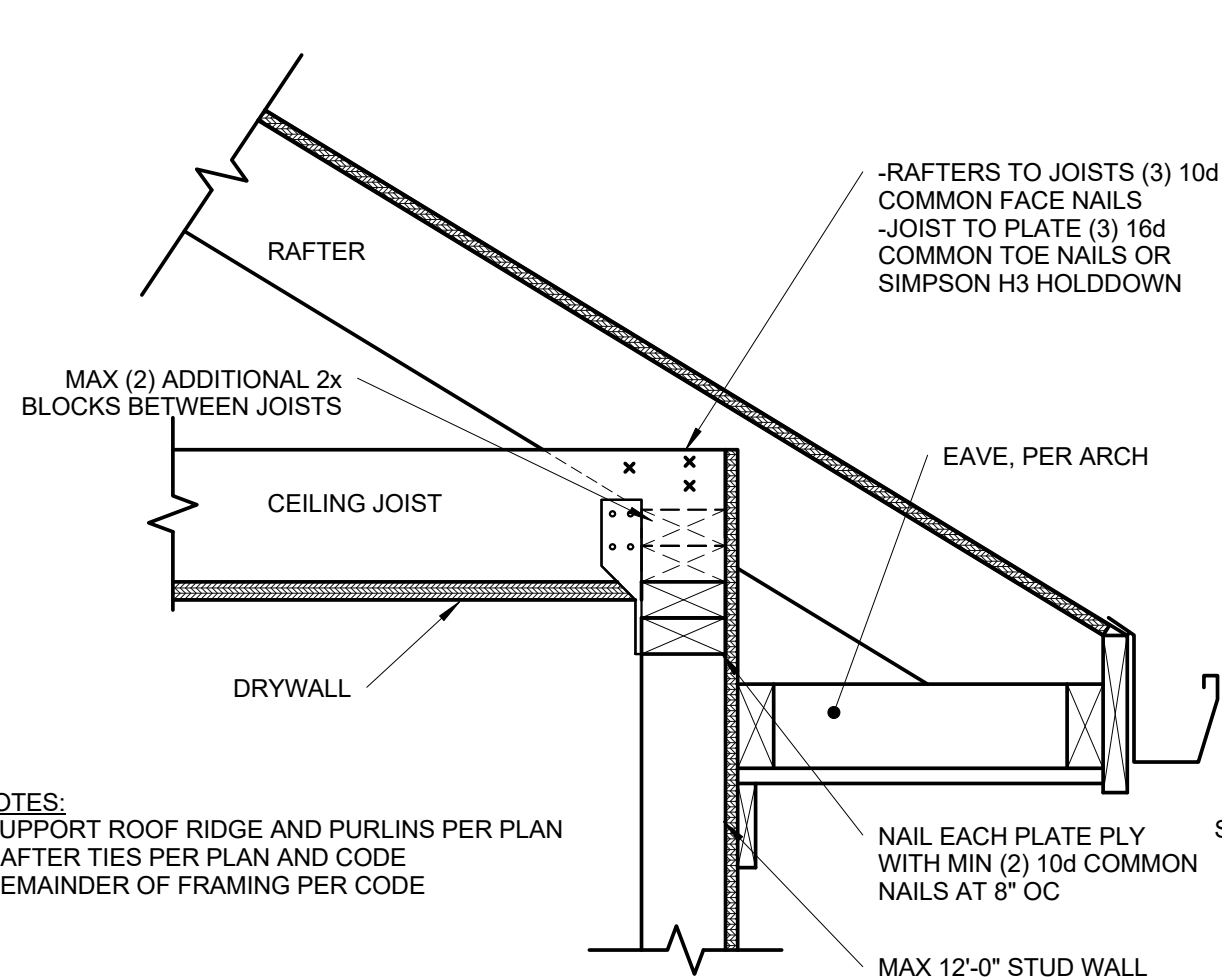
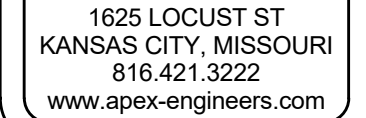


REVISION DATE

2924 NW AUDOBON LANE  
LEE'S SUMMIT, MISSOURI 64081



THIS IS AN ENGINEERED ROOF STRUCTURE  
DESIGNED FOR COMPLIANCE WITH IRC 802.3, BUILD  
AS SHOWN WITH NO DEVIATIONS



**NOTES:**  
 -SUPPORT ROOF RIDGE AND PURLINS PER PLAN  
 -RAFTER TIES PER PLAN AND CODE  
 -REMAINDER OF FRAMING PER CODE

0d

RAFTER

RAFTER TO PLATE (3) 16d COMMON TOE NAILS

EAVE, PER ARCH

CUT OUT 2x6 CONT PLATE AT STRAP LOCATIONS (IF NECESSARY)

CEILING JOIST

DRYWALL

MIN #6 SCREWS AT 8" OC

7 1/2"

SIMPSON CS22 STRAP AT 32" OC STARTING AT THE TOP AND EXTENDING 7 1/2" BELOW BOTTOM PLATE. FILL ALL HOLES WITH 8d COMMON NAILS

NAIL EACH PLATE PLY WITH MIN (2) 10d COMMON NAILS AT 8" OC (MAX (5) PLATES)

MAX 12'-0" STUD WALL

NOTES:

- SUPPORT ROOF RIDGE AND PURLINS PER PLAN
- RAFTER TIES PER PLAN AND CODE
- REMAINDER OF FRAMING PER CODE

NOTES:  
-SUPPORT ROOF RIDGE AND PURLINS PER PLAN  
-RAFTER TIES PER PLAN AND CODE  
-REMAINDER OF FRAMING PER CODE

## 2 | OPTIONAL RAFTER BEARING

<b>SX-1</b>	1 1/2" = 1'-0"
-------------	----------------

## 1 | OPTIONAL RAFTER BEARING

<b>SX-1</b>	1 1/2" = 1'-0"
-------------	----------------

PROJECT:	
X	
X	
X	
CLIENT:	
X	

PROJECT #:	31634
DRAWN BY:	APEX
CHECKED BY:	APEX

SHEET #:  
**SX-1**



**APEX**  
ENGINEERS, INC.

1625 LOCUST ST  
KANSAS CITY, MO 64108  
816.421.3222  
www.apex-engineers.com



STRUCTURAL DESIGN REVIEW

KANSAS ENGINEERING LICENSE:

E-992

MISSOURI ENGINEERING LICENSE:

2003004673

PROJECT:

2924 NW Audubon Ln  
Lot 1450 Winterset Valley  
Lee's Summit, MO

CLIENT:

Don Julian Builders

PROJECT #: XXXXX

DRAWN BY: XXX

CHECKED BY: BDC

SUBMITTAL DATE: 2018.01.08

COMMENTS

DATE

#

SHEET:

FRAMING DETAILS

S3.1

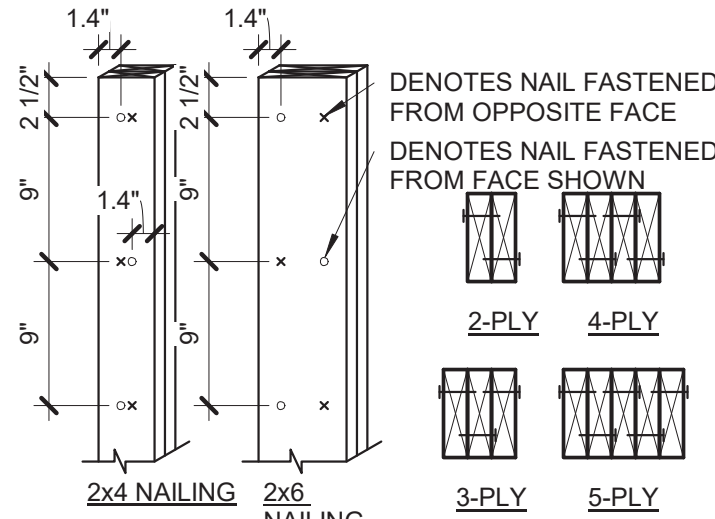
TOP VIEW OF BUILT-UP WOOD COLUMN

BLOCKIN

(3)-2x6 BUILT-UP  
WOOD COLUMN

SIMPSON  
LUS210-3 FACE-  
MOUNT HANGER,  
PER REPORT

(3) #2-2x10 BEAM



- NOTES:
1. EACH 2x PLY SHALL BE FASTENED WITH (1) ROW OF 10d NAILS AT 9" OC, ALTERNATING SIDE TO SIDE 1.4" MIN EDGE DISTANCE, AND STARTING 2 1/2" FROM EACH END.
  2. EXTEND FULL AREA OF COLUMN AS SOLID BLOCKING THROUGH JOIST BAYS AND WALLS TO LOAD-BEARING BEAM/WALL BELOW

1 | BUILT-UP STUD COLUMN

S3.1 | 1" = 1'-0"



## Wood Column

Lic. #: KW-06005244

019\2924 NW Audubon Lane\_Lot 1450 Winterset Valley\_LSMO\2020.05.29\_Struct\_Don Julian\beam calcs.ec6  
Software copyright ENERCALC, INC. 1983-2020, Build:12.20.2.28

APEX ENGINEERS INC

DESCRIPTION: Column Calc

### Code References

Calculations per NDS 2018, IBC 2018, CBC 2019, ASCE 7-16  
Load Combinations Used : ASCE 7-10

### General Information

Analysis Method :		Allowable Stress Design		Wood Section Name		3-2x6	
End Fixities		Top & Bottom Pinned		Wood Grading/Manuf.		Graded Lumber	
Overall Column Height		10 ft		Wood Member Type		Sawn	
( Used for non-slender calculations )							
Wood Species		Douglas Fir - Larch		Exact Width		4.50 in	
Wood Grade		Stud		Exact Depth		5.50 in	
Fb +		700.0 psi	Fv	180.0 psi	Area		24.750 in^2
Fb -		700.0 psi	Ft	450.0 psi	Ix		62.391 in^4
Fc - Prll		850.0 psi	Density	31.210 pcf	Iy		41.766 in^4
Fc - Perp		625.0 psi					
E : Modulus of Elasticity . . .		x-x Bending	y-y Bending	Axial			
Basic		1,400.0	1,400.0	1,400.0 ksi			
Minimum		510.0	510.0				
Allow Stress Modification Factors							
Cf or Cv for Bending 1.0							
Cf or Cv for Compression 1.0							
Cf or Cv for Tension 1.0							
Cm : Wet Use Factor 1.0							
Ct : Temperature Factor 1.0							
Cfu : Flat Use Factor 1.0							
Kf : Built-up columns 0.60 NDS 15.3.2							
Use Cr : Repetitive ? No							
Brace condition for deflection (buckling) along columns :							
X-X (width) axis : Unbraced Length for buckling ABOUT Y-Y Axis = 10 ft, K = 1.0							
Y-Y (depth) axis : Unbraced Length for buckling ABOUT X-X Axis = 10 ft, K = 1.0							

### Applied Loads

Service loads entered. Load Factors will be applied for calculations.

Column self weight included : 53.642 lbs \* Dead Load Factor

AXIAL LOADS . . .

Axial Load at 10.0 ft, Yecc = 2.250 in, D = 1.50 k

### DESIGN SUMMARY

#### Bending & Shear Check Results

PASS	Max. Axial+Bending Stress Ratio =	0.3736 : 1	Maximum SERVICE Lateral Load Reactions . .			
	Load Combination	+D+H	Top along Y-Y	0.0 k	Bottom along Y-Y	0.0 k
	Governing NDS Formula	Comp + Myy, NDS Eq. 3.9-3	Top along X-X	0.02813 k	Bottom along X-X	0.02813 k
	Location of max.above base	9.933 ft	Maximum SERVICE Load Lateral Deflections . . .			
	At maximum location values are . . .		Along Y-Y	0.0 in	at	0.0 ft above base
	Applied Axial	1.554 k	for load combination : n/a			
	Applied Mx	0.0 k-ft	Along X-X	-0.05378 in	at	5.839 ft above base
	Applied My	-0.2794 k-ft	for load combination : +D+H			
	Fc : Allowable	273.286 psi	Other Factors used to calculate allowable stresses . . .			
				<u>Bending</u>	<u>Compression</u>	<u>Tension</u>
PASS	Maximum Shear Stress Ratio =	0.01052 : 1				
	Load Combination	+D+H				
	Location of max.above base	10.0 ft				
	Applied Design Shear	1.705 psi				
	Allowable Shear	162.0 psi				

### Load Combination Results

Load Combination	C <sub>D</sub>	C <sub>P</sub>	Maximum Axial + Bending Stress Ratios			Maximum Shear Ratios		
			Stress Ratio	Status	Location	Stress Ratio	Status	Location
+D+H	0.900	0.357	0.3736	PASS	9.933 ft	0.01052	PASS	10.0 ft
+D+L+H	1.000	0.333	0.3379	PASS	9.933 ft	0.009470	PASS	10.0 ft
+D+Lr+H	1.250	0.283	0.2747	PASS	9.933 ft	0.007576	PASS	10.0 ft
+D+S+H	1.150	0.301	0.2966	PASS	9.933 ft	0.008235	PASS	10.0 ft
+D+0.750Lr+0.750L+H	1.250	0.283	0.2747	PASS	9.933 ft	0.007576	PASS	10.0 ft
+D+0.750L+0.750S+H	1.150	0.301	0.2966	PASS	9.933 ft	0.008235	PASS	10.0 ft
+D+0.60W+H	1.600	0.231	0.2203	PASS	9.933 ft	0.005919	PASS	10.0 ft
+D+0.70E+H	1.600	0.231	0.2203	PASS	9.933 ft	0.005919	PASS	10.0 ft
+D+0.750Lr+0.750L+0.450W+H	1.600	0.231	0.2203	PASS	9.933 ft	0.005919	PASS	10.0 ft
+D+0.750L+0.750S+0.450W+H	1.600	0.231	0.2203	PASS	9.933 ft	0.005919	PASS	10.0 ft
+D+0.750L+0.750S+0.5250E+H	1.600	0.231	0.2203	PASS	9.933 ft	0.005919	PASS	10.0 ft
+0.60D+0.60W+0.60H	1.600	0.231	0.1198	PASS	0.0 ft	0.003551	PASS	10.0 ft



## Wood Column

Lic. #: KW-06005244

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APEX ENGINEERS INC

DESCRIPTION: Column Calc

### Load Combination Results

Load Combination	C <sub>D</sub>	C <sub>P</sub>	Maximum Axial + Bending Stress Ratios			Maximum Shear Ratios		
			Stress Ratio	Status	Location	Stress Ratio	Status	Location
+0.60D+0.70E+0.60H	1.600	0.231	0.1198	PASS	0.0 ft	0.003551	PASS	10.0 ft

### Maximum Reactions

Note: Only non-zero reactions are listed.

Load Combination	X-X Axis Reaction		k	Y-Y Axis Reaction		Axial Reaction	My - End Moments		k-ft	Mx - End Moments	
	@ Base	@ Top		@ Base	@ Top	@ Base	@ Base	@ Top		@ Base	@ Top
+D+H	-0.028	0.028				1.554					
+D+L+H	-0.028	0.028				1.554					
+D+Lr+H	-0.028	0.028				1.554					
+D+S+H	-0.028	0.028				1.554					
+D+0.750Lr+0.750L+H	-0.028	0.028				1.554					
+D+0.750L+0.750S+H	-0.028	0.028				1.554					
+D+0.60W+H	-0.028	0.028				1.554					
+D+0.70E+H	-0.028	0.028				1.554					
+D+0.750Lr+0.750L+0.450W+H	-0.028	0.028				1.554					
+D+0.750L+0.750S+0.450W+H	-0.028	0.028				1.554					
+D+0.750L+0.750S+0.5250E+H	-0.028	0.028				1.554					
+0.60D+0.60W+0.60H	-0.017	0.017				0.932					
+0.60D+0.70E+0.60H	-0.017	0.017				0.932					
D Only	-0.028	0.028				1.554					
Lr Only											
L Only											
S Only											
W Only											
E Only											
H Only											

### Maximum Deflections for Load Combinations

Load Combination	Max. X-X Deflection	Distance	Max. Y-Y Deflection	Distance
+D+H	-0.0538 in	5.839 ft	0.000 in	0.000 ft
+D+L+H	-0.0538 in	5.839 ft	0.000 in	0.000 ft
+D+Lr+H	-0.0538 in	5.839 ft	0.000 in	0.000 ft
+D+S+H	-0.0538 in	5.839 ft	0.000 in	0.000 ft
+D+0.750Lr+0.750L+H	-0.0538 in	5.839 ft	0.000 in	0.000 ft
+D+0.750L+0.750S+H	-0.0538 in	5.839 ft	0.000 in	0.000 ft
+D+0.60W+H	-0.0538 in	5.839 ft	0.000 in	0.000 ft
+D+0.70E+H	-0.0538 in	5.839 ft	0.000 in	0.000 ft
+D+0.750Lr+0.750L+0.450W+H	-0.0538 in	5.839 ft	0.000 in	0.000 ft
+D+0.750L+0.750S+0.450W+H	-0.0538 in	5.839 ft	0.000 in	0.000 ft
+D+0.750L+0.750S+0.5250E+H	-0.0538 in	5.839 ft	0.000 in	0.000 ft
+0.60D+0.60W+0.60H	-0.0323 in	5.839 ft	0.000 in	0.000 ft
+0.60D+0.70E+0.60H	-0.0323 in	5.839 ft	0.000 in	0.000 ft
D Only	-0.0538 in	5.839 ft	0.000 in	0.000 ft
Lr Only	0.0000 in	0.000 ft	0.000 in	0.000 ft
L Only	0.0000 in	0.000 ft	0.000 in	0.000 ft
S Only	0.0000 in	0.000 ft	0.000 in	0.000 ft
W Only	0.0000 in	0.000 ft	0.000 in	0.000 ft
E Only	0.0000 in	0.000 ft	0.000 in	0.000 ft
H Only	0.0000 in	0.000 ft	0.000 in	0.000 ft



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Sketches

