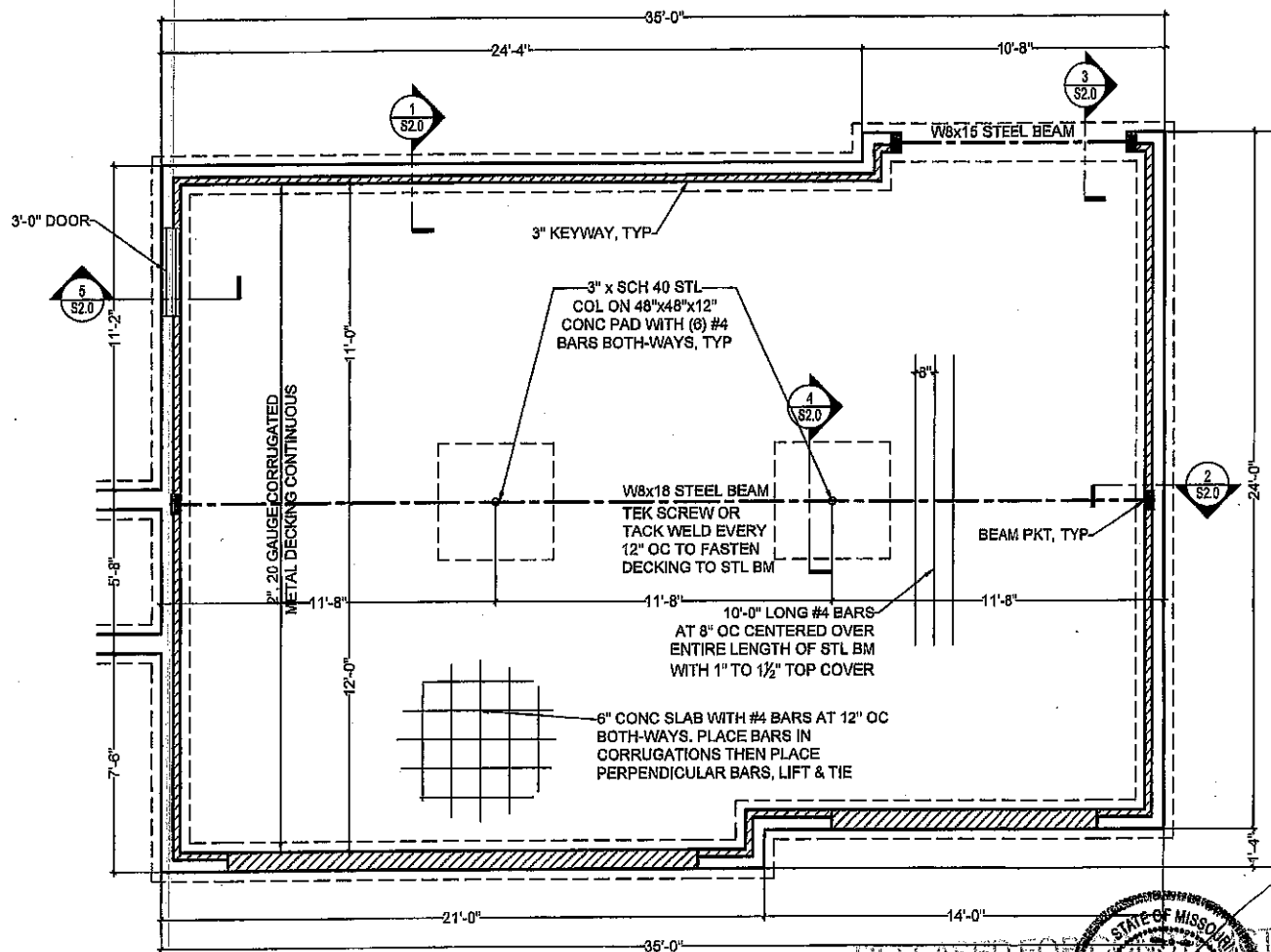


- 3500 PSI CONCRETE
- LAP SPICES 24" MIN
- GRADE 40 REBAR
- GARAGE BEAM ABOVE SHALL CLEARSPAN. NO POINT LOADS ON SUSPENDED SLAB.
- EMBED BARS 24" OR DRILL & EPOXY MIN 5" AT FDTN WALLS
- SHORE DECKING AT MAX 6'-0" DURING CONSTRUCTION
- FDTN WALL DESIGN BY OTHERS

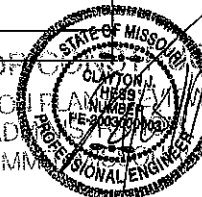


1 SUSPENDED SLAB LAYOUT
S1.0 SCALE X" = 1'-0"

REFUSED FOR CLASSIFICATION

AS NOTED C
CODES AD
LIFE'S SUM

DATE 4-26-47



APEX
ENGINEERS

1600 BALTIMORE AVE, SUITE 102
KANSAS CITY, MO 64108
Ph: 816.421.3222
Fax: 816.421.1050
www.epex-engineers.com

STRUCTURAL DESIGN REVIEW
KANSAS ENGINEERING LICENSE
892
MISSOURI ENGINEERING LICENSE
2003004873

PROJECT: 4725 NE GATEWAY DR
LOT 54 MONTICELLO
LEE'S SUMMIT, MO
CLIENT:

D&D BUILDING

PROJECT #

DRAWING NAME

**SUSPENDED
SLAB LAYOUT**

DATE:	COMMENTS:
-------	-----------

DRAWN BY: APEX
CHECKED BY: —

SHEET #

S1.0



**APEX
ENGINEERS**

1500 BALTIMORE AVE. SUITE 102
KANSAS CITY, MO 64108
Ph: 816.421.3222
Fax: 816.421.1050
www.apex-engineers.com

STRUCTURAL DESIGN REVIEW
KANSAS ENGINEERING LICENSE:
882
MISSOURI ENGINEERING LICENSE:
200304673

PROJECT: 4725 NE GATEWAY DR
LOT 54 MONTICELLO
LEE'S SUMMIT, MO
CLIENT: D&D BUILDING

PROJECT #

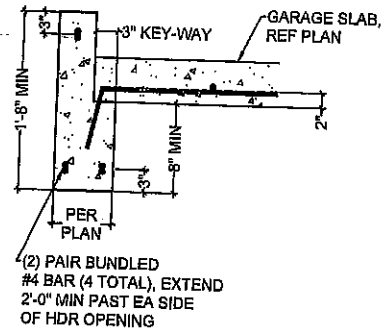
DRAWING NAME

**SUSPENDED
SLAB DETAILS**

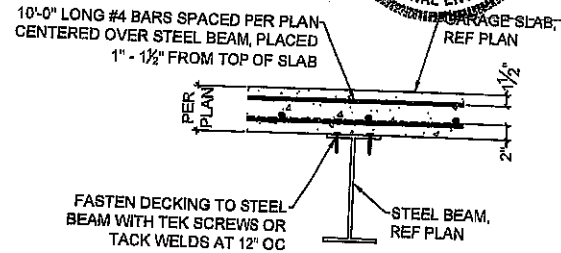
DATE:	COMMENTS:
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DRAWN BY: APEX
CHECKED BY: ---

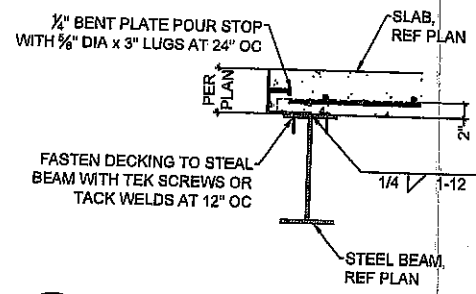
S2.0



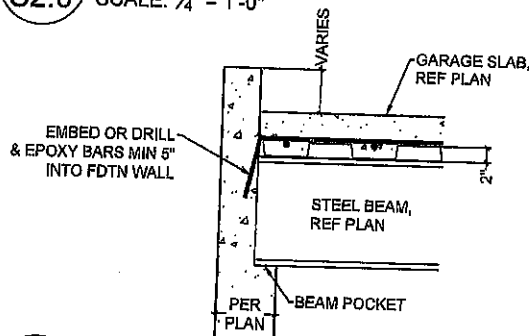
5 CONCRETE HEADER DETAIL
S2.0 SCALE: 3/4" = 1'-0"



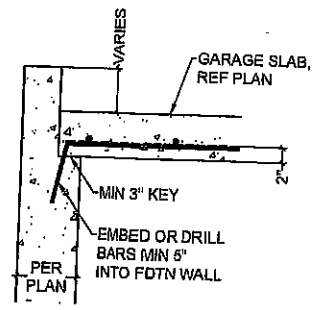
4 GARAGE SLAB BEAM BEARING
S2.0 SCALE: 3/4" = 1'-0"



3 POUR STOP DETAIL
S2.0 SCALE: 3/4" = 1'-0"



2 GARAGE SLAB BEAM BEARING
S2.0 SCALE: 3/4" = 1'-0"



1 GARAGE SLAB BEARING
S2.0 SCALE: 3/4" = 1'-0"

- STEEL DECKING NOTES:
- MINIMUM 1 1/2" BEARING
 - FASTEN TO SUPPORT STEEL WITH 3/8" VISIBLE PUDDLE WELDS AT EDGE RIBS & 12" CTRS. ALONG END BEARING
 - FASTEN SIDE LAPS & PERIMETER EDGES AT 36" CTRS. WITH #10 TEK SCREWS OR 3/8" PUDDLE WELDS
 - MAX UNSUPPORTED CONSTRUCTION SPAN 6'-0"

RELEASED FOR CONSTRUCTION
AS NOTED ON PLANS REVIEW
CODES ADMINISTRATION
LEE'S SUMMIT, MISSOURI

BY _____
DATE _____



Apex Engineers, Inc.
1600 Baltimore Ave, Suite 102
Kansas City, MO 64108
www.apex-engineers.com

Project Title: 4725 NE Gateway Dr
Engineer: Lee's Summit, MO
Project Descr: Lot 54 Monticello

Project ID:

3 of 5

Concrete Beam

Lic. #: KW-06005244

Description: Susp garage

File: G:\Projects\ID & D Building\The Meiner\03 - Eng\Calculations\beam calcs.ccb

ENERCALC, INC. 1993-2017, Build: 17.2.28, Ver: 17.2.28

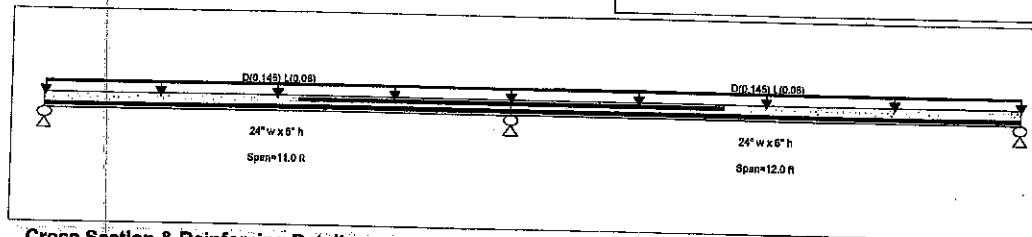
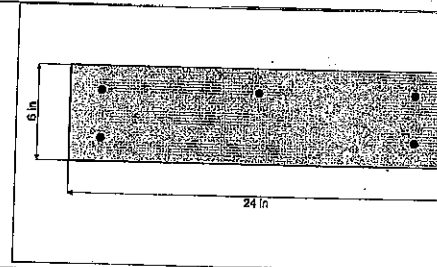
Licensee: APEX ENGINEERS INC

CODE REFERENCES

Calculations per ACI 318-11, IBC 2012, ASCE 7-10
Load Combination Set: IBC 2015

Material Properties

f_c = 3.50 ksi
 $f_r = f_c^{1/2} \cdot 7.50$ = 443.706 psi
 ψ Density = 145.0 pcf
 λ LWt Factor = 1.0
Elastic Modulus = 3,122.0 ksi
 f_y - Main Rebar = 40.0 ksi
E - Main Rebar = 29,000.0 ksi
 ϕ Phi Values
Flexure: 0.90
Shear: 0.750
 β_1 = 0.850
Fy - Stirrups = 40.0 ksi
E - Stirrups = 29,000.0 ksi
Stirrup Bar Size # = 3
Number of Resisting Legs Per Stirrup = 2



Cross Section & Reinforcing Details

Rectangular Section, Width = 24.0 in, Height = 6.0 in

Span #1 Reinforcing....

2-#4 at 1.50 in from Bottom, from 0.0 to 11.0 ft in this span

Span #2 Reinforcing....

2-#4 at 1.50 in from Bottom, from 0.0 to 12.50 ft in this span

3-#4 at 1.50 in from Top, from 6.0 to 11.0 ft in this span

3-#4 at 1.50 in from Top, from 0.0 to 5.0 ft in this span

Service loads entered. Load Factors will be applied for calculations.

Applied Loads

Load for Span Number 1

Uniform Load: D = 0.07250, L = 0.040 ksf, Tributary Width = 2.0 ft

Load for Span Number 2

Uniform Load: D = 0.07250, L = 0.040 ksf, Tributary Width = 2.0 ft

DESIGN SUMMARY

Maximum Bending Stress Ratio =		0.680 : 1	Maximum Deflection		Design OK
Section used for this span		Typical Section	Max Downward Transient Deflection		0.019 in Ratio = 7549 >= 60
Mu : Applied		3.581 k-ft	Max Upward Transient Deflection		-0.007 in Ratio = 17688 >= 60
Mn * Phi : Allowable		5.286 k-ft	Max Downward Total Deflection		0.042 in Ratio = 3432 >= 60
Location of maximum on span		7.151 ft	Max Upward Total Deflection		0.002 in Ratio = 16705 >= 60
Span # where maximum occurs		Span # 2			

Vertical Reactions

Load Combination	Support 1	Support 2	Support 3
Overall MAXIMUM	0.988	3.239	1.087
Overall MINIMUM	-0.088	0.541	-0.048
+D+H	0.678	2.098	0.669
+D+L+H, LL Comb Run (*L)	0.510	2.698	1.087
+D+L+H, LL Comb Run (L*)	0.966	2.628	0.621
+D+L+H, LL Comb Run (LL)	0.897	3.239	1.038
+D+L+H, LL Comb Run (*L)	0.578	2.088	0.669
+D+L+H, LL Comb Run (L*)	0.578	2.088	0.669
+D+L+H, LL Comb Run (LL)	0.578	2.088	0.669
+D+S+H	0.578	2.088	0.669

Support notation: Far left is #1



RELEASED FOR CONSTRUCTION
AS NOTED ON PLANS REVIEW
CODES ADMINISTRATION
LEE'S SUMMIT, MISSOURI

BY

DATE



Apex Engineers, Inc.
1600 Baltimore Ave, Suite 102
Kansas City, MO 64108
www.apex-engineers.com

Project Title: 4725 NE Gateway Dr
Engineer: Lee's Summit, MO
Project Descr: Lot 54 Monticello

Project ID: 4 of 8

Steel Beam

Lic. #: KW-06005244

Description: Continuous steel beam

Printed: 25 APR 2017, 1:34PM

File: G:\Projects\4725 NE Gateway Dr\The Molder\03 - Eng\Calculations\beam calcs.ecb

ENGERCALC, INC. 1983-2017, Build 17.2.26, Ver 6.17.2.28

Licensee: APEX ENGINEERS INC

CODE REFERENCES

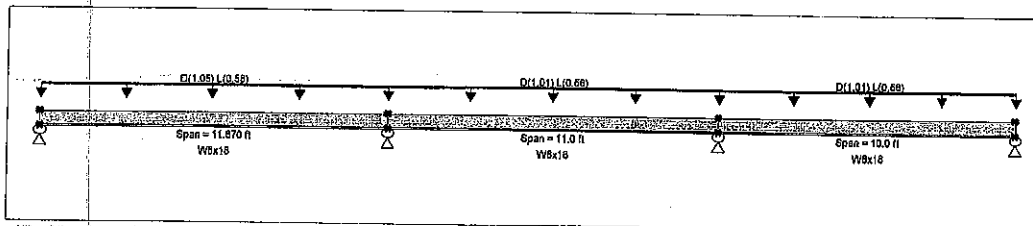
Calculations per AISC 360-10, IBC 2012, ASCE 7-10

Load Combination Set: IBC 2015

Material Properties

Analysis Method: Allowable Strength Design
Beam Bracing: Completely Unbraced
Bending Axis: Major Axis Bending

Fy: Steel Yield: 36.0 ksi
E: Modulus: 29,000.0 ksi



Applied Loads

Service loads entered. Load Factors will be applied for calculations.

Beam self weight calculated and added to loading
Load for Span Number 1
Uniform Load: D = 1.050, L = 0.580 k/ft, Tributary Width = 1.0 ft
Load for Span Number 2
Uniform Load: D = 1.010, L = 0.580 k/ft, Tributary Width = 1.0 ft
Load for Span Number 3
Uniform Load: D = 1.010, L = 0.580 k/ft, Tributary Width = 1.0 ft

DESIGN SUMMARY

Design OK			
Maximum Bending Stress Ratio =	0.754 : 1	Maximum Shear Stress Ratio =	0.430 : 1
Section used for this span	W8x18	Section used for this span	W8x18
Ma: Applied	23.022 k-ft	Va: Applied	11.589 k
Mn / Omega: Allowable	30.539 k-ft	Vn / Omega: Allowable	28.960 k
Load Combination	+D+L+H, LL Comb Run (LL*)	Load Combination	+D+L+H, LL Comb Run (LL*)
Location of maximum on span	11.670 ft	Location of maximum on span	11.670 ft
Span # where maximum occurs	Span # 1	Span # where maximum occurs	Span # 1
Maximum Deflection			
Max Downward Transient Deflection	0.099 in Ratio = 1,416 >= 600.		
Max Upward Transient Deflection	-0.050 in Ratio = 2,856 >= 600.		
Max Downward Total Deflection	0.233 in Ratio = 601 >= 600.		
Max Upward Total Deflection	-0.046 in Ratio = 2865 >= 600.		

Maximum Forces & Stresses for Load Combinations

Load Combination	Segment Length	Span #	Max Stress Ratios		Summary of Moment Values						Summary of Shear Values			
			M	V	Mmax +	Mmax -	Ma Max	Mnx	Mnx/Omega Cb	Rm	Vmax	Vnx	Vnx/Omega	
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CLAYTON J. HESS														
REGISTERED PROFESSIONAL ENGINEER														
No. 2003-00008														
Exp. 12/31/2006														
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RELEASED FOR CONSTRUCTION
AS NOTED ON PLANS REVIEW
CODES ADMINISTRATION
LEE'S SUMMIT, MISSOURI

BY

DATE



Apex Engineers, Inc.
1600 Baltimore Ave, Suite 102
Kansas City, MO 64108
www.apex-engineers.com

Project Title: 4725 NE Gateway Dr
Engineer: Lee's Summit, MO
Project Descr: Lot 54 Monticello

Project ID: 6 of 6

Printed: 25 APR 2017, 1:47PM

Steel Beam

File = G:\Projects\10 & D Building\The Maine\103 - Eng\Calculations\beam calcs.rvt
ENERCALC, INC.: 1993-2017, Build 6.17.2.28, Ver 6.17.2.28

Lic. #: KW-06005244

Licensee: APEX ENGINEERS INC

Description: Rear steel beam

CODE REFERENCES

Calculations per AISC 360-10, IBC 2012, ASCE 7-10

Load Combination Set: IBC 2015

Material Properties

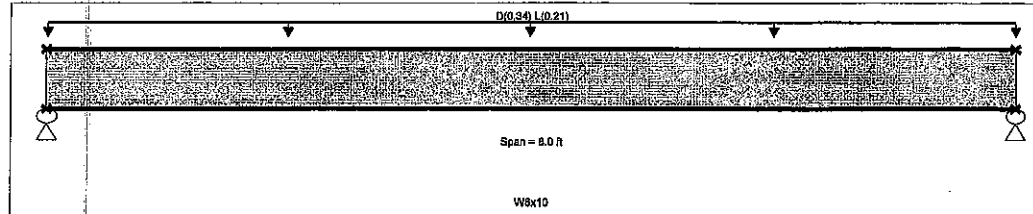
Analysis Method: Allowable Strength Design

Fy: Steel Yield: 36.0 ksi

Beam Bracing: Beam is Fully Braced against lateral-torsional buckling

E: Modulus: 29,000.0 ksi

Bending Axis: Major Axis Bending



Applied Loads

Service loads entered. Load Factors will be applied for calculations.

Beam self weight calculated and added to loading

Uniform Load: D = 0.340, L = 0.210 k/ft, Tributary Width = 1.0 ft

DESIGN SUMMARY

Design OK

Maximum Bending Stress Ratio =	0.281 : 1	Maximum Shear Stress Ratio =	0.116 : 1
Section used for this span	W8x10	Section used for this span	W8x10
Ma: Applied	4.480 k-ft	Va: Applied	2.240 k
Mn / Omega: Allowable	15.934 k-ft	Vn / Omega: Allowable	19.315 k
Load Combination	+D+L+H	Load Combination	+D+L+H
Location of maximum on span	4.000 ft	Location of maximum on span	0.000 ft
Span # where maximum occurs	Span # 1	Span # where maximum occurs	Span # 1
Maximum Deflection			
Max Downward Transient Deflection	0.022 in Ratio = 4.410 >= 600.		
Max Upward Transient Deflection	0.000 in Ratio = 0 < 600.0		
Max Downward Total Deflection	0.058 in Ratio = 1654 >= 600.		
Max Upward Total Deflection	0.000 in Ratio = 0 < 600.0		

Maximum Forces & Stresses for Load Combinations

Load Combination	Segment Length	Span #	Max Stress Ratios		Summary of Moment Values							Summary of Shear Values		
			M	V	Mmax +	Mmax -	Ma Max	Mnx	Mnx/Omega	Cb	Rm	Va Max	Vnx	Vnx/Omega
+D+H	Dsgn. L = 8.00 ft	1	0.176	0.072	2.80		2.80	26.61	15.93	1.00	1.00	1.40	28.97	19.31
+D+L+H	Dsgn. L = 8.00 ft	1	0.281	0.116	4.48		4.48	26.61	15.93	1.00	1.00	2.03	28.97	19.31
+D+L+H	Dsgn. L = 8.00 ft	1	0.176	0.072	2.80		2.80	26.61	15.93	1.00	1.00	1.40	28.97	19.31
+D+S+H	Dsgn. L = 8.00 ft	1	0.176	0.072	2.80		2.80	26.61	15.93	1.00	1.00	1.40	28.97	19.31
+D+0.750L+0.750L+H	Dsgn. L = 8.00 ft	1	0.255	0.105	4.06		4.06	26.61	15.93	1.00	1.00	2.03	28.97	19.31
+D+0.750L+0.750S+H	Dsgn. L = 8.00 ft	1	0.255	0.105	4.06		4.06	26.61	15.93	1.00	1.00	2.03	28.97	19.31
+D+0.60W+H	Dsgn. L = 8.00 ft	1	0.176	0.072	2.80		2.80	26.61	15.93	1.00	1.00	1.40	28.97	19.31
+D+0.70E+H	Dsgn. L = 8.00 ft	1	0.176	0.072	2.80		2.80	26.61	15.93	1.00	1.00	1.40	28.97	19.31
+D+0.750L+0.750L+0.450W+H	Dsgn. L = 8.00 ft	1	0.255	0.105	4.06		4.06	26.61	15.93	1.00	1.00	2.03	28.97	19.31
+D+0.750L+0.750S+0.450W+H	Dsgn. L = 8.00 ft	1	0.255	0.105	4.06		4.06	26.61	15.93	1.00	1.00	2.03	28.97	19.31
+D+0.750L+0.750S+0.5250E+H	Dsgn. L = 8.00 ft	1	0.255	0.105	4.06		4.06	26.61	15.93	1.00	1.00	2.03	28.97	19.31
+0.60D+0.60W+0.60H	Dsgn. L = 8.00 ft	1	0.105	0.043	1.68		1.68	26.61	15.93	1.00	1.00	0.84	28.97	19.31
+0.60D+0.70E+0.60H														

