

**RELEASED FOR
CONSTRUCTION**
As Noted on Plans Review

**Development Services Department
Lee's Summit, Missouri
10/29/2025**

The truss designs referenced below have been prepared by me or under my direct supervision based on the truss design criteria and requirements ("design criteria") provided by Quality Line Truss.

These truss designs are intended for the fabrication of individual building components that will perform to the design criteria provided. Any variance from the design criteria will render the affected truss designs inapplicable.

Listed below are the truss designs included in this package and covered by this seal.

Job: QU02699_BUILDING G_REV_10222025 - 1250194
GE01, GE02, T01, T02, T05, T06, T07

Any location identification is for file reference only. No determination of the appropriateness of design criteria for any specific project has been made in preparing the truss designs.

Please refer to individual truss designs for specific design criteria.



STAND STRUCTURAL ENGINEERING
8234 Robinson St
Overland Park, KS 66204
(913) 214-2169

Reviewed Revise and Resubmit
 Reviewed as Noted Rejected
 Not required by the Contract Documents
 For Record Only

Review is only for general conformance with the design concept and the intent of the Contract Documents. Contractor is solely responsible for verifying dimensions, for establishing fabrication processes, means, techniques, sequences and procedures of construction and for coordination of work of all trades. Review action taken and noted to information shown does not authorize deviations from the intent of the contract documents, unless so stated in a separate letter or Change Order. The Structural Engineer and/or Architect-of-Record retained by the Contractor or his Supplier, for the Design / Build portion of the project represented by this submittal, is solely responsible for the correctness, appropriateness and adequacy of the Design / Build system.

By: JFunk
Date: 10/29/2025

Arturo A. Hernandez (MO, 2006000095)
My license expiration date for the state of MO is 12/31/2026.

IMPORTANT NOTE: The responsibility of the engineer sealing this package, as a Truss Engineer, is solely for design of individual trusses as individual building components based upon design criteria provided by others and set forth in the referenced truss drawings. The truss design criteria for the components have not been verified as appropriate for any particular building, project or use. Adequacy and suitability of design criteria and requirements for the truss designs for any specific project are the responsibility of the building designer, not the Truss Engineer, per ANSI/TPI-1, Chapter 2.

DESIGN NOTES

1. The Truss Design Drawing(s) provided with these Design Notes have been prepared under and are subject to ANSI / TPI 1 published by the Truss Plate Institute, www.tpinst.org. Capitalized terms have the meanings provided in ANSI / TPI 1.
2. Copies of each Truss Design Drawing shall be furnished to the installation contractor, Building Designer, Owner and all persons fabricating, handling, installing, bracing, or erecting the trusses.

DESIGN LIMITATIONS

3. The Truss Design Drawing is based upon specifications provided by the Building Designer in accordance with ANSI / TPI 1. Neither the Truss Designer, Eagle, nor an engineer who seals this design (if any) assumes any responsibility for the adequacy or accuracy of specifications provided by the Building Designer.
4. The Building Designer is solely responsible for the suitability based upon the Truss Design Drawing and shall be responsible for reviewing and verifying that the information shown is in general conformance with the design of the Building.
5. Each Truss Design Drawing is for the individual building component (a truss). A seal on the Truss Design Drawing indicates acceptance of professional engineering responsibility solely for the individual truss.
6. Each Truss Design Drawing assumes trusses will be suitably protected from the environment.

HANDLING, INSTALLING, & BRACING

7. Refer to Building Component Safety Information (BCSI) for handling, installing, restraining and bracing trusses. Copies can be obtained from the Structural Building Components Association, www.sbcindustry.com.
8. Bracing shown on each Truss Design Drawing is for lateral support of individual truss components only to reduce buckling lengths. All temporary and permanent bracing, including lateral load and diagonal or cross bracing, are the responsibility, respectively, of the erector and Building Designer.
9. Eagle is not responsible for improper truss fabrication, handling, erection or bracing.
10. Compression chords shall be laterally braced by the roof or floor sheathing, directly attached, or have purlins provided at spacing shown, unless noted otherwise.

11. Bottom chord required bracing shall be at 10ft spacing or less, if no structural rated ceiling is installed, unless noted otherwise.
12. Strongbacking shall be installed on all parallel chord trusses, including flooring systems, to limit deflection and reduce vibration. Refer to BCSI-B7.
13. Never exceed the design loading shown. Never stack building or other materials on inadequately braced truss; refer to BCSI.
14. Concentration of construction loads greater than the design loads shall not be applied to the trusses at any time; refer to BCSI.
15. Trusses shall be handled with care prior to erection to avoid damage. Refer to BCSI for recommended truss handling and erection.

MATERIALS & FABRICATION

16. Lumber moisture content shall be 19% or less at the time of fabrication unless noted otherwise.
17. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
18. Unless expressly noted, the truss designs are not applicable for use with fire retardant or preservative treated lumber.
19. Plates shall be applied on both faces of truss at each joint and embedded fully. Knots and wane at joint locations shall be regulated in accordance with ANSI / TPI 1.
20. For a specified plate gauge and grade, the specified size is a minimum.
21. Connections not shown are the responsibility of others.
22. Adequate support shall be provided to resist gravity, lateral and uplift loads.
23. For 4X2 truss orientation, locate plates 0 - 1/16" from outside the edge of the truss.
24. Fabrication of truss shall be in accordance with ANSI / TPI 1.

OTHER NOTES

25. Gamber is a non-structural consideration and is the responsibility of truss fabricator.
26. Do not cut or alter any truss member or plate without prior approval from a professional engineer.
27. Lumber design values are in accordance with ANSI / TPI 1; lumber design values are by others.
28. Install specified hangers per manufacturer recommendations.

SYMBOLS

PLATE SIZE

3X4 - The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

- / - / Indicates required direction of slots; Reference "Joint Details" for more information.

- 20 Ga Gr40 connectors required
- 3X10 - 20HS** - 20 Ga Gr60 connectors required
- 8X10 - 18HS** - 18 Ga Gr60 connectors required

LATERAL BRACING

When this symbol shown, continuous lateral bracing is required on the member of the truss.



BEARING

Indicates location where bearings (supports) occur.



PLATE LOCATION & ORIENTATION

The plate shall be centered on joint and/or placed in accordance with the design drawing/QC full scale details.



REFERENCES

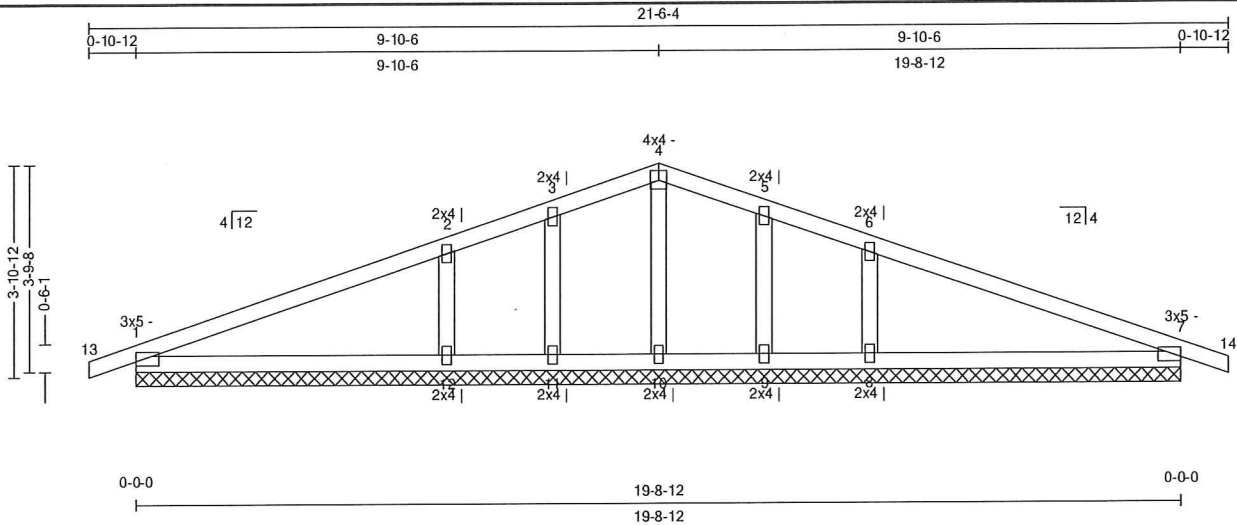
- ANSI / TPI 1: National Design Standard for Metal Plate Connected Wood Trusses
- BCSI: Building Component & Safety Information - Guide to Good Practice for Handling, Installing, Restraining, & Bracing of Metal Plate Connected Wood Trusses.
- NDS: National Design Specification for Wood Construction
- ESR- 1082 published by the International Code Council. www.icc-es.org

Quality Line Truss Co., LLC

34593 S 4350 RD
Address 2
Adair, OK 74330

Truss:GE01
Job: QU02699_BUILDING G_REV_1022200
Date: 10/22/25 12:26:09
Page: 1 of 1

SPAN 19-8-12	PITCH 4/12	QTY 1	OHL 0-10-12	OHR 0-10-12	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 77 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.33 (6-7)	Vert TL: 0.02 in	L/999	(7-8)	L/240
TCDL: 10	TP1 1-2014	BC: 0.12 (7-8)	Vert LL: 0 in	L/999	7	L/360
BCLL: 0	Rep Mbr: No	Web: 0.04 (6-8)	Horz TL: 0 in			
BCDL: 10	Lumber D.O.L.: 115 %					

10/22/2025

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1		1,144 lbs	205 plf	-822 lbs	-161 lbs	-460 lbs	-822 lbs	622 lbs

Material

TC: SYP#1 2 x 4
BC: SYP#1 2 x 4
Web: SYP#2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- 1) This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 2) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.329	957 lbs	(-417 lbs)
	6-7	0.329	957 lbs	(-417 lbs)
BC				
Web				

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Gable requires continuous bottom chord bearing.
- 3) Gable webs placed at 24" OC, U.N.O.
- 4) Attach gable webs with 2x4 20ga plates, U.N.O.
- 5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCEA.
- 6) The fabrication tolerance for this roof truss is 20% (Cq = 0.80).
- 7) A creep factor of 2.00 has been applied for this truss analysis.
- 8) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 9) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 7, 1 may need to be considered.
- 10) Listed wind uplift reactions based on MWFRS & C&C loading.



WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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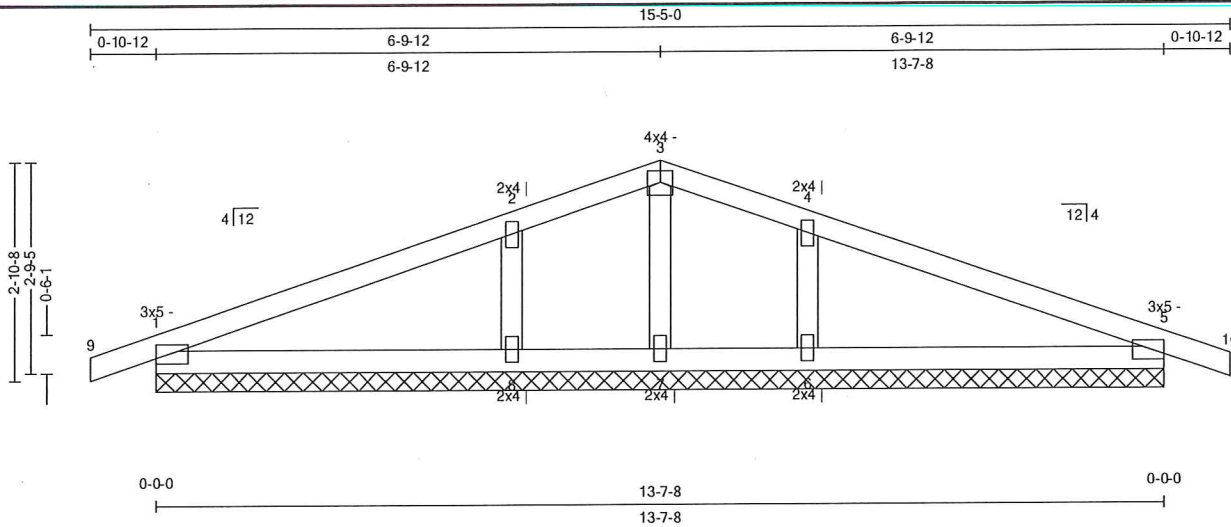
Quality Line Truss Co., LLC

34593 S 4350 RD
Address 2
Adair, OK 74330

Truss:GE02

Job: QU02699_BUILDING G_REV_102220
Date: 10/22/25 12:26:11
Page: 1 of 1

SPAN 13-7-8	PITCH 4/12	QTY 1	OHL 0-10-12	OHR 0-10-12	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24in	WGT/PLY 50lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.24 (4-5)	Vert TL: 0.01 in	L/999	(5-6)	L/240
TCDL: 10	TPI 1-2014	BC: 0.08 (5-6)	Vert LL: 0 in	L/999	5	L/360
BCLL: 0	Rep Mbr: No	Web: 0.04 (4-6)	Horz TL: 0 in			
BCDL: 10	Lumber D.O.L.: 115 %					

10/22/2025

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1		840 lbs	214 plf	-570 lbs	-101 lbs	-433 lbs	-570 lbs	443 lbs

Material

TC: SYP#1 2 x 4
BC: SYP#1 2 x 4
Web: SYP#2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.



Loads

- This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

Member	Force	Member	Force
TC 1-2	0.239 674 lbs (373 lbs)		
TC 4-5	0.239 674 lbs (373 lbs)		
BC			
Web			

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- Gable requires continuous bottom chord bearing.
- Gable webs placed at 24" OC, U.N.O.
- Attach gable webs with 2x4 20ga plates, U.N.O.
- Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- The fabrication tolerance for this roof truss is 20% (Cq = 0.80).
- A creep factor of 2.00 has been applied for this truss analysis.
- The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 5, 1 may need to be considered.
- Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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Quality Line Truss Co., LLC

34593 S 4350 RD

Address 2

Adair, OK 74330

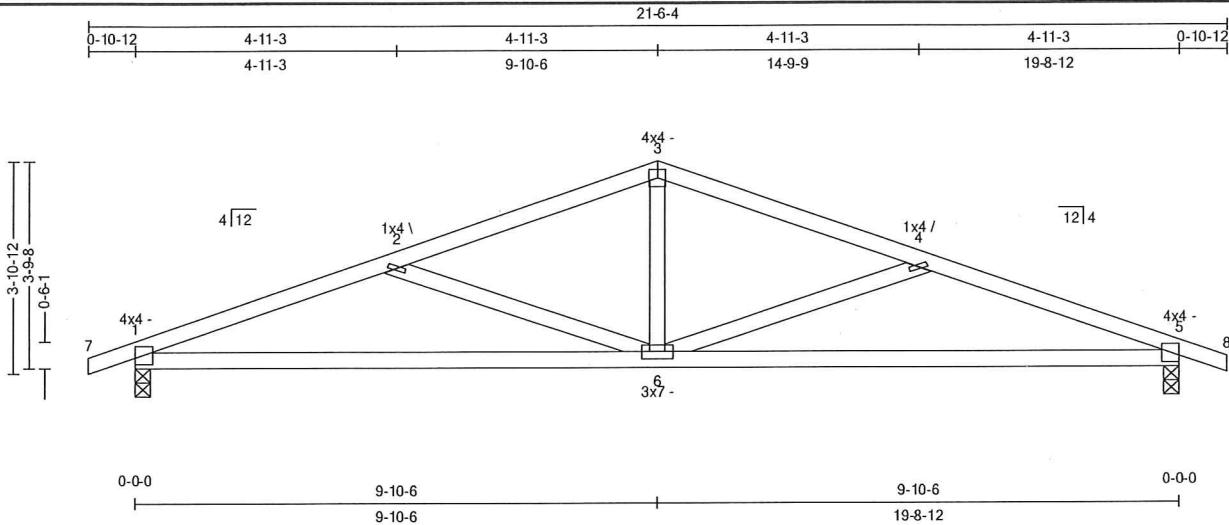
Truss:T01

Job: QU02699_BUILDING G_REV_1022201

Date: 10/22/25 12:26:12

Page: 1 of 1

SPAN 19-8-12	PITCH 4/12	QTY 8	OHL 0-10-12	OHR 0-10-12	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 78 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	I/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.26 (3-4)	Vert TL: 0.31 in	L/735	(5-6)	L/240
TCDL: 10	TPI 1-2014	BC: 0.89 (6-1)	Vert LL: 0.13 in	L/999	(5-6)	L/360
BCLL: 0	Rep Mbr: Yes	Web: 0.21 (2-6)	Horz TL: 0.05 in		5	
BCDL: 10	Lumber D.O.L.: 115 %					

10/22/2025

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	3.5 in	1.50 in	1,040 lbs		-80 lbs	-332 lbs	-332 lbs	-4 lbs
5	1	3.5 in	1.50 in	1,040 lbs		-80 lbs	-332 lbs	-332 lbs	

Material

TC: SYP#1 2 x 4
BC: SYP#1 2 x 4
Web: SYP#1 2 x 4

Bracing

TC: Sheathed or Purlins at 4-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 8-10-0, Purlin design by Others.

Loads

- This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

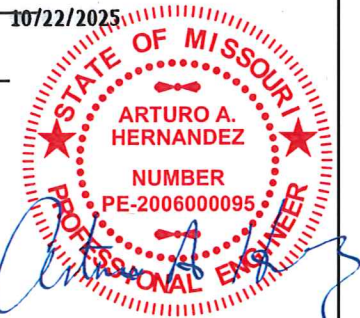
Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

Member	CSI	Tension (lbs)	Compression (lbs)
TC 1-2	0.252	645	(2,077)
TC 2-3	0.257	467	(1,644)
TC 3-4	0.257	645	(2,077)
TC 4-5	0.252	645	(2,077)
BC 5-6	0.895	1,931	(518)
BC 6-1	0.895	1,931	(518)
Web 2-6	0.214		(490)
Web 3-6	0.112	675	
Web 4-6	0.214		(490)

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20% (Cq = 0.80).
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- A creep factor of 2.00 has been applied for this truss analysis.
- The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- Listed wind uplift reactions based on MWFRS & C&C loading.



WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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Quality Line Truss Co., LLC

34593 S 4350 RD

Address 2

Adair, OK 74330

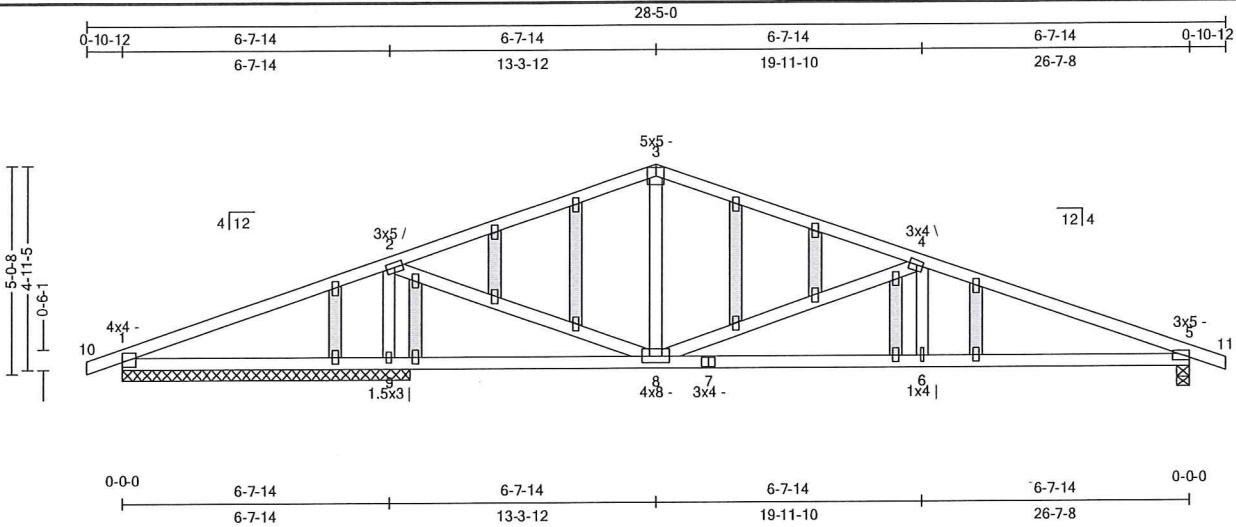
Truss:T02

Job: QU02699_BUILDING G_REV_102220

Date: 10/22/25 12:26:14

Page: 1 of 1

SPAN 26-7-8	PITCH 4/12	QTY 1	OHL 0-10-12	OHR 0-10-12	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24in	WGT/PLY 136lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.63 (1-2)	Vert TL: 0.17 in	L/999	(6-7)	L/240
TCDL: 10	TPI 1-2014	BC: 0.62 (6-8)	Vert LL: 0.05 in	L/999	(6-7)	L/360
BCLL: 0	Rep Mbr: No	Web: 0.75 (4-8)	Horz TL: 0.03 in		5	
BCDL: 10	Lumber D.O.L.: 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
5	1	3.5 in	1.50 in	977 lbs	-	-78 lbs	-245 lbs	-245 lbs	-
9	1	86.25 in	N/A	1,567 lbs	-	-93 lbs	-274 lbs	-274 lbs	491 lbs
1	1	86.25 in	N/A	265 lbs	-1,819 lbs	-109 lbs	-	-1,819 lbs	-1,189 lbs
1	1	86.25 in	N/A	2,044 lbs	-	-177 lbs	-379 lbs	-379 lbs	720 lbs

Material

TC: SYP#1 2 x 4
BC: SYP#1 2 x 4
Web: SYP#1 2 x 4

Bracing

TC: Sheathed or Purlins at 4-2-0, Purlin design by Others.
BC: Sheathed or Purlins at 8-6-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.

2) This truss has been designed to account for the effects of ice dams forming at the eaves.

3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60

4) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

Member	ID	Force 1	Force 2	Force 3	Force 4	Force 5	Force 6
TC	1-2	0.634	2,010 lbs	(364 lbs)	3-4	0.443	(917 lbs)
	2-3	0.458		(919 lbs)	4-5	0.402	368 lbs (-1,896 lbs)
	5-6	0.562	1,741 lbs		6-8	0.616	1,741 lbs
BC	5-6	0.562	1,741 lbs		6-8	0.616	1,741 lbs
	8-9	0.288					(491 lbs)
Web	2-9	0.165	351 lbs	(-1,295 lbs)	4-8	0.749	(989 lbs)
	2-8	0.226	1,363 lbs				

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Gable webs placed at 24" OC, U.N.O.
- 3) Attach structural gable blocks with 2x4 20ga plates, U.N.O.
- 4) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- 5) The fabrication tolerance for this roof truss is 20% (Cq = 0.80).
- 6) A creep factor of 2.00 has been applied for this truss analysis.
- 7) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 8) Indicates non-structural members.
- 9) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 1 may need to be considered.
- 10) Listed wind uplift reactions based on MWFRS & C&C loading.



WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

TrueBuild® Truss Software v5.8.11
Eagle Metal Products

Quality Line Truss Co., LLC

34593 S 4350 RD

Address 2

Adair, OK 74330

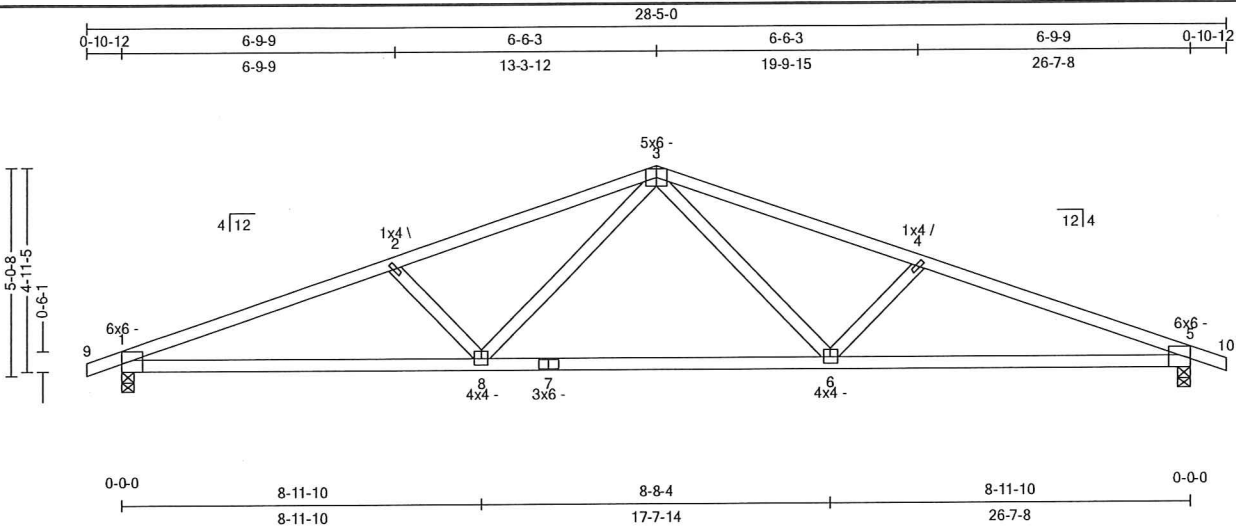
Truss:T05

Job: QU02699_BUILDING G_REV_102220

Date: 10/22/25 12:26:15

Page: 1 of 1

SPAN 26-7-8	PITCH 4/12	QTY 26	OHL 0-10-12	OHR 0-10-12	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24in	WGT/PLY 106lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.41 (1-2)	Vert TL: 0.66 in	L/477	(6-7)	L/240
TCDL: 15	TP1 1-2014	BC: 0.91 (8-1)	Vert LL: 0.34 in	L/907	(6-7)	L/360
BCLL: 20	Rep Mbr: Yes	Web: 0.23 (3-8)	Horz TL: 0.12 in		5	
BCDL: 10	Lumber D.O.L.: 115 %					

10/22/2025

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	3.5 in	1.71 in	2,060 lbs	-	-17 lbs	-232 lbs	-232 lbs	-3 lbs
5	1	3.5 in	1.71 in	2,060 lbs	-	-17 lbs	-232 lbs	-232 lbs	-

Material

TC: SYP2400/1.8 2 x 4
 BC: SYP2400/1.8 2 x 4
 Web: SYP#1 2 x 4

Bracing

TC: Sheathed or Purlins at 3-6-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- 1) This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 2) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 4) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

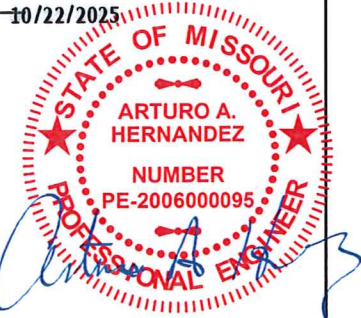
Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.409	421 lbs	(-4,428 lbs)	3-4	0.384	396 lbs	(-4,094 lbs)			
	2-3	0.384	396 lbs	(-4,094 lbs)	4-5	0.409	421 lbs	(-4,428 lbs)			
BC	5-6	0.912	4,130 lbs		6-8	0.812	2,858 lbs		8-1	0.912	4,130 lbs
Web	2-8	0.090		(-478 lbs)	3-8	0.233	1,406 lbs		3-6	0.233	1,406 lbs
									4-6	0.090	(-478 lbs)

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 4) A creep factor of 2.00 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 6) Listed wind uplift reactions based on MWFRS & C&C loading.



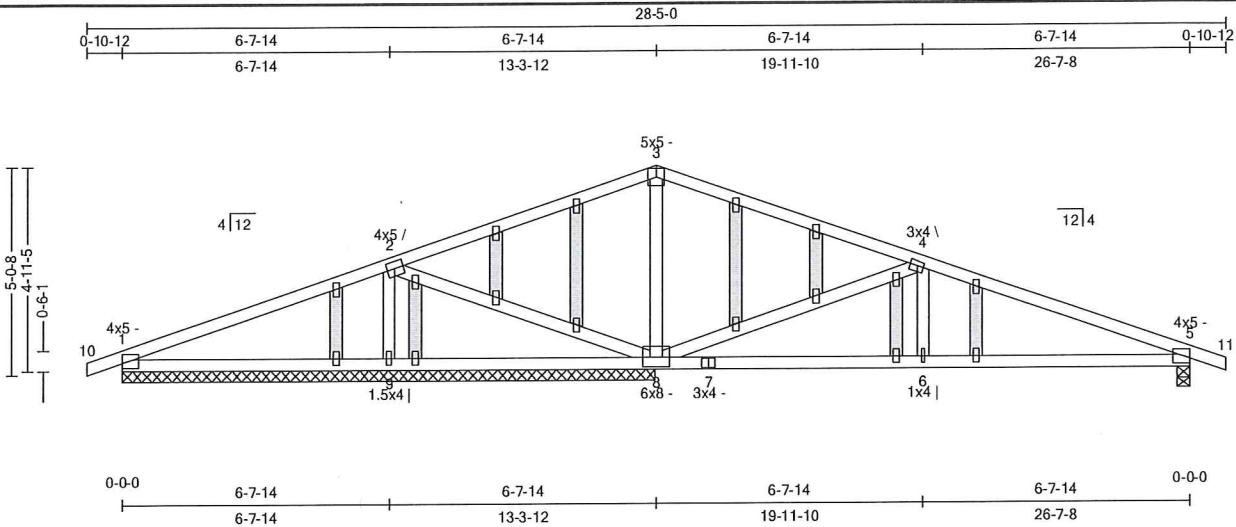
WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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 Eagle Metal Products

Quality Line Truss Co., LLC
 34593 S 4350 RD
 Address 2
 Adair, OK 74330

Truss:T06
 Job: QU02699_BUILDING G_REV_1022200
 Date: 10/22/25 12:26:16
 Page: 1 of 1

SPAN 26-7-8	PITCH 4/12	QTY 1	OHL 0-10-12	OHR 0-10-12	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24in	WGT/PLY 137 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.44 (1-2)	Vert TL: 0.2 in	L/768	(6-7)	L/240
TCDL: 15	TPI 1-2014	BC: 0.51 (6-8)	Vert LL: 0.09 in	L/999	(6-7)	L/360
BCLL: 10	Rep Mbr: No	Web: 0.97 (4-8)	Horz TL: 0.03 in		5	
BCDL: 10	Lumber D.O.L.: 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
5	1	3.5 in	1.50 in	1,269 lbs	-	-17 lbs	-185 lbs	-185 lbs	-
9	1	159.5 in	N/A	2,015 lbs	-	-	-178 lbs	-178 lbs	559 lbs
1	1	159.5 in	N/A	156 lbs	-2,320 lbs	-205 lbs	-	-2,320 lbs	-1,537 lbs
1	1	159.5 in	N/A	2,624 lbs	-	-60 lbs	-256 lbs	-256 lbs	978 lbs



Material

TC: SYP2400/1.8 2 x 4
 BC: SYP2400/1.8 2 x 4
 Web: SYP#1 2 x 4

Bracing

TC: Sheathed or Purlins at 4-7-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- 1) This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 2) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 4) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.443	2,595 lbs	3-4	0.311	(-1,203 lbs)
	2-3	0.327	(-1,206 lbs)	4-5	0.292	(-2,474 lbs)
BC	5-6	0.457	2,280 lbs	6-8	0.505	2,280 lbs
				8-9	0.267	(-559 lbs)
Web	2-9	0.205	(-1,607 lbs)	4-8	0.973	(-1,284 lbs)
	2-8	0.288	1,737 lbs	4-6	0.064	388 lbs

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Gable webs placed at 24" OC, U.N.O.
- 3) Attach structural gable blocks with 2x4 20ga plates, U.N.O.
- 4) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- 5) The fabrication tolerance for this roof truss is 20% (Cq = 0.80).
- 6) A creep factor of 2.00 has been applied for this truss analysis.
- 7) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 8) Indicates non-structural members.
- 9) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 1 may need to be considered.
- 10) Listed wind uplift reactions based on MWFRS & C&C loading.

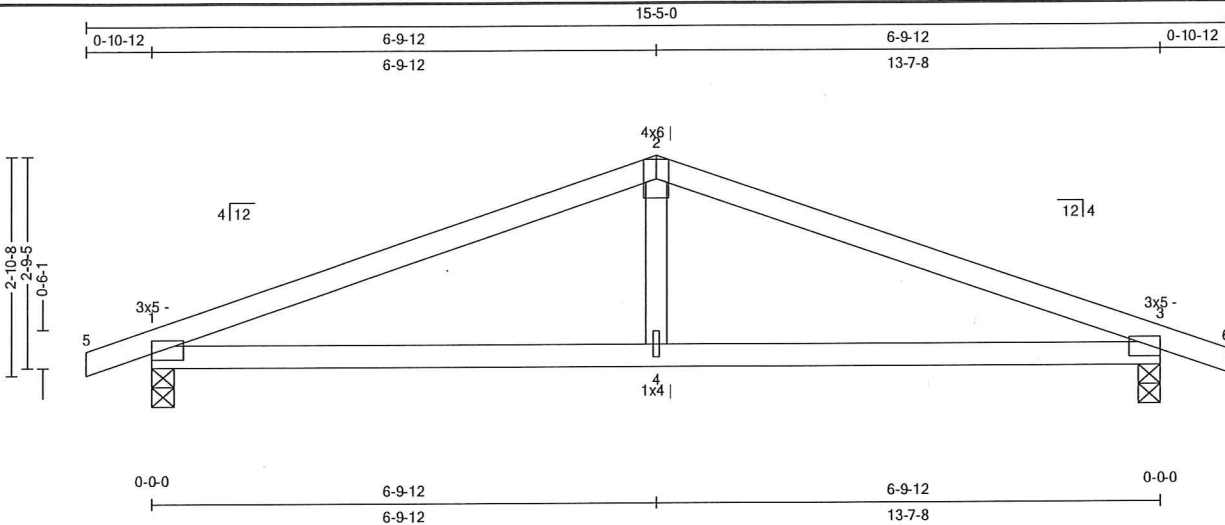
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 Eagle Metal Products

Quality Line Truss Co., LLC
34593 S 4350 RD
Address 2
Adair, OK 74330

Truss:T07
Job: QU02699_BUILDING G_REV_1022200
Date: 10/22/25 12:26:18
Page: 1 of 1

SPAN 13-7-8	PITCH 4/12	QTY 3	OHL 0-10-12	OHR 0-10-12	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24in	WGT/PLY 45 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.47 (1-2)	Vert TL: 0.14 in	L/999	(3-4)	L/240
TCDL: 15	TPI 1-2014	BC: 0.73 (4-1)	Vert LL: 0.06 in	L/999	(3-4)	L/360
BCLL: 10	Rep Mbr: Yes	Web: 0.07 (2-4)	Horz TL: 0.03 in		3	
BCDL: 10	Lumber D.O.L.: 115 %					

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	3.5 in	1.50 in	948 lbs		-14 lbs	-310 lbs	-310 lbs	5 lbs
3	1	3.5 in	1.50 in	948 lbs		-14 lbs	-310 lbs	-310 lbs	

Material

TC: SYP#1 2 x 4
BC: SYP#1 2 x 4
Web: SYP#1 2 x 4

Bracing

TC: Sheathed or Purlins at 4-9-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.470	456 lbs	(-1,576 lbs)	2-3	0.470	456 lbs	(-1,576 lbs)
BC	3-4	0.729	1,433 lbs		4-1	0.729	1,433 lbs	
Web	2-4	0.074	445 lbs					

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- A creep factor of 2.00 has been applied for this truss analysis.
- The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- Listed wind uplift reactions based on MWFRS & C&C loading.



WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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