

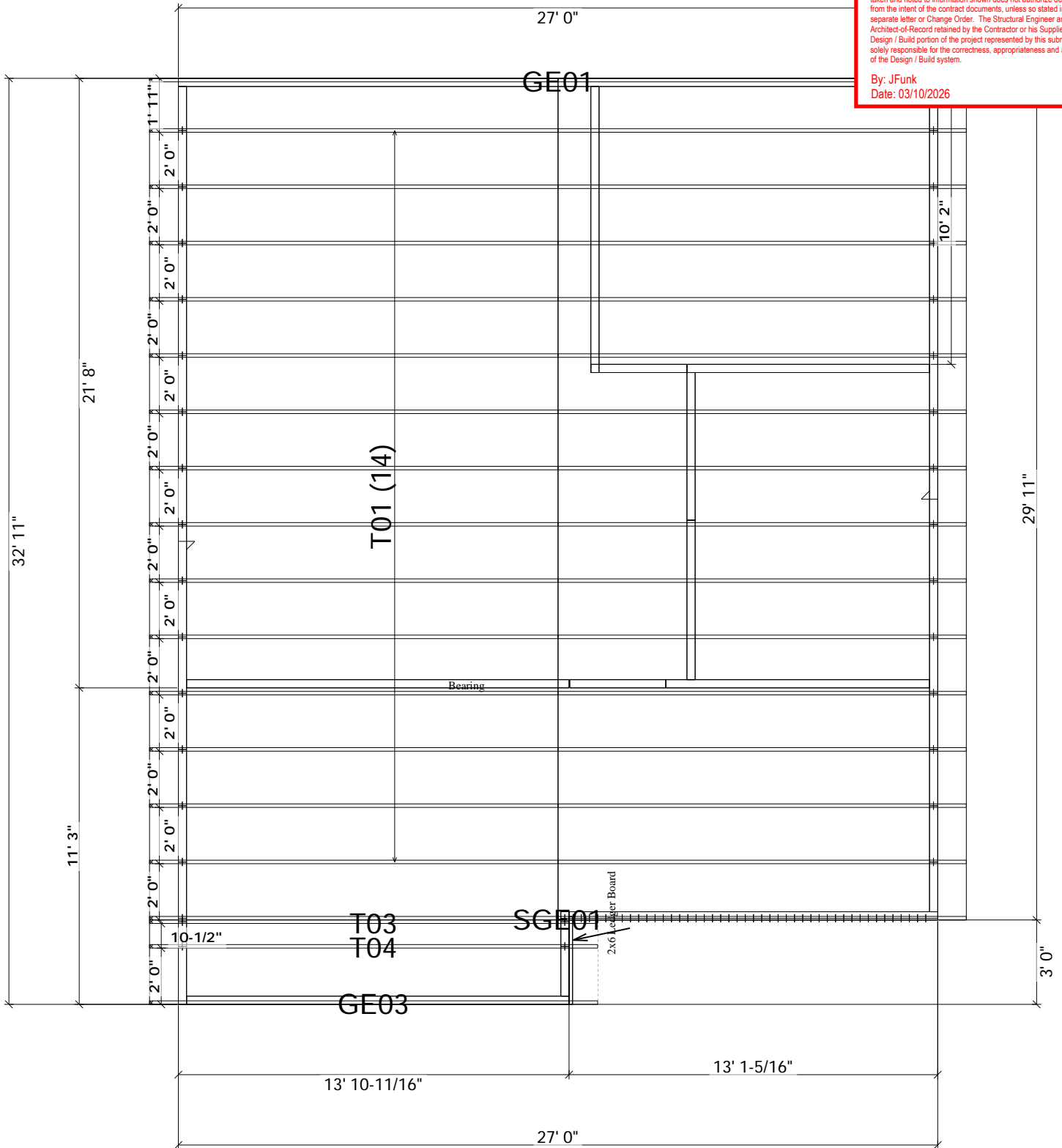
**RELEASE FOR CONSTRUCTION  
 AS NOTED ON PLANS REVIEW  
 DEVELOPMENT SERVICES  
 LEE'S SUMMIT, MISSOURI  
 03/27/2026 2:27:09**

**STAND STRUCTURAL ENGINEERING**  
 8234 Robinson St  
 Overland Park, KS 66204  
 (913) 214-2169

- Reviewed  Revise and Resubmit
- Reviewed as Noted  Rejected
- Not required by the Contract Documents
- For Record Only

Review is only for general conformance with the design concept and the intent of the Contract Documents. Contractor is solely responsible for verifying dimensions, for establishing fabrication processes, means, techniques, sequences and procedures of construction and for coordination of work of all trades. Review action taken and noted on information shown does not authorize deviations from the intent of the contract documents, unless so stated in a separate letter or Change Order. The Structural Engineer and/or Architect-of-Record retained by the Contractor or his Supplier, for the Design / Build portion of the project represented by this submittal, is solely responsible for the correctness, appropriateness and adequacy of the Design / Build system.

By: JFunk  
 Date: 03/10/2026



**RESERVE BUILDING H REVISED  
 LEVEL 2 FRAMING PLAN\_02182026**

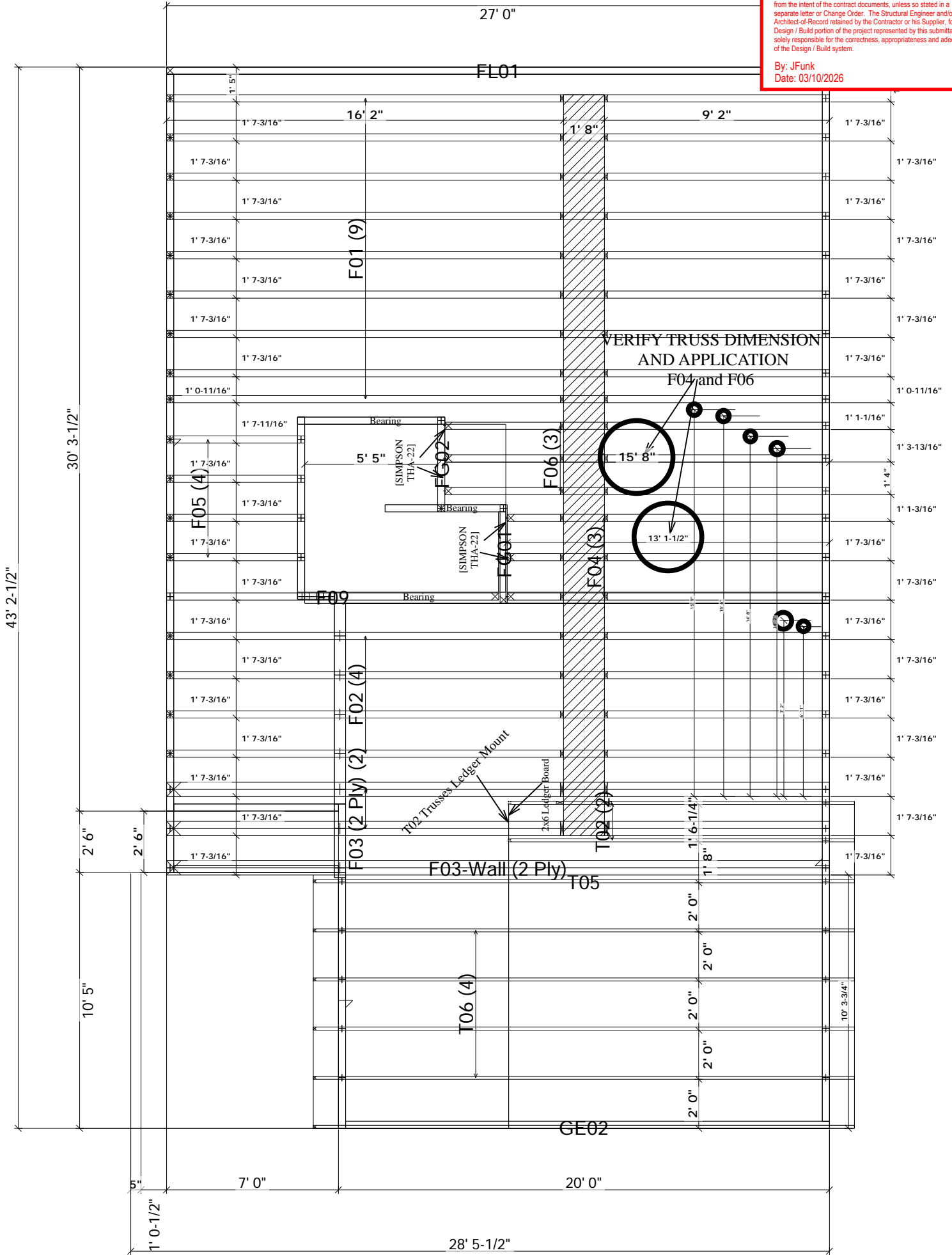
**RELEASE FOR CONSTRUCTION  
AS NOTED ON PLANS REVIEW  
DEVELOPMENT SERVICES  
LEE'S SUMMIT, MISSOURI  
03/27/2026 2:26:51**

**STAND STRUCTURAL ENGINEERING**  
8234 Robinson St  
Overland Park, KS 66204  
(913) 214-2169

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By: JFunk  
Date: 03/10/2026



**BUILDING H - REVISED 02182026  
LEVEL 1 FRAMING PLAN**

The truss designs referenced below have been prepared by me or under my direct supervision based on the truss design criteria and requirements ("design criteria") provided by **Quality Line Truss**.

These truss designs are intended for the fabrication of individual building components that will perform to the design criteria provided. Any variance from the design criteria will render the affected truss designs inapplicable.

Listed below are the truss designs included in this package and covered by this seal.

Job: QU03647\_RESERVE\_BUILDING H\_02042026 - 1258286

GE01, GE02, GE03, SGE01, T01, T02, T03, T04, T06, T05, F01, F02, F03-Wall, F03, F04, F05, F06, F09, FG01, FG02, FL01

Any location identification is for file reference only. No determination of the appropriateness of design criteria for any specific project has been made in preparing the truss designs.

Please refer to individual truss designs for specific design criteria.

02/17/2026



**RELEASE FOR CONSTRUCTION  
AS NOTED ON PLANS REVIEW  
DEVELOPMENT SERVICES  
LEE'S SUMMIT, MISSOURI  
03/27/2026 2:26:43**

**STAND STRUCTURAL ENGINEERING**  
8234 Robinson St  
Overland Park, KS 66204  
(913) 214-2169

- Reviewed
- Reviewed as Noted
- Not required by the Contract Documents
- For Record Only
- Revise and Resubmit
- Rejected

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By: JFunk  
Date: 03/10/2026

Anish Kekre (MO, 2024044263)

My license expiration date for the state of MO is 12/31/2026.

**IMPORTANT NOTE:** The responsibility of the engineer sealing this package, as a Truss Engineer, is solely for design of individual trusses as individual building components based upon design criteria provided by others and set forth in the referenced truss drawings. The truss design criteria for the components have not been verified as appropriate for any particular building, project or use. Adequacy and suitability of design criteria and requirements for the truss designs for any specific project are the responsibility of the building designer, not the Truss Engineer, per ANSI/TPI-1, Chapter 2.

## DESIGN NOTES

1. The Truss Design Drawing(s) provided with these Design Notes have been prepared under and are subject to ANSI / TPI 1 published by the Truss Plate Institute, [www.tpinst.org](http://www.tpinst.org). Capitalized terms have the meanings provided in ANSI / TPI 1.
2. Copies of each Truss Design Drawing shall be furnished to the installation contractor, Building Designer, Owner and all persons fabricating, handling, installing, bracing, or erecting the trusses.
- DESIGN LIMITATIONS**
3. The Truss Design Drawing is based upon specifications provided by the Building Designer in accordance with ANSI / TPI 1. Neither the Truss Designer, Eagle, nor an engineer who seals this design (if any) assumes any responsibility for the adequacy or accuracy of specifications provided by the Building Designer.
4. The Building Designer is solely responsible for the suitability based upon the Truss Design Drawing and shall be responsible for reviewing and verifying that the information shown is in general conformance with the design of the Building.
5. Each Truss Design Drawing is for the individual building component (a truss). A seal on the Truss Design Drawing indicates acceptance of professional engineering responsibility solely for the individual truss.
6. Each Truss Design Drawing assumes trusses will be suitably protected from the environment.

### HANDLING, INSTALLING, & BRACING

7. Refer to Building Component Safety Information (BCSI) for handling, installing, restraining and bracing trusses. Copies can be obtained from the Structural Building Components Association, [www.sbcindustry.com](http://www.sbcindustry.com).
8. Bracing shown on each Truss Design Drawing is for lateral support of individual truss components only to reduce buckling lengths. All temporary and permanent bracing, including lateral load and diagonal or cross bracing, are the responsibility, respectively, of the erector and Building Designer.
9. Eagle is not responsible for improper truss fabrication, handling, erection or bracing.
10. Compression chords shall be laterally braced by the roof or floor sheathing, directly attached, or have purlins provided at spacing shown, unless noted otherwise.

11. Bottom chord required bracing shall be at 10ft spacing or less, if no structural rated ceiling is installed, unless noted otherwise.
12. Strongbacking shall be installed on all parallel chord trusses, including flooring systems, to limit deflection and reduce vibration. Refer to BCSI-B7.
13. Never exceed the design loading shown. Never stack building or other materials on inadequately braced truss; refer to BCSI.
14. Concentration of construction loads greater than the design loads shall not be applied to the trusses at any time; refer to BCSI.
15. Trusses shall be handled with care prior to erection to avoid damage. Refer to BCSI for recommended truss handling and erection.

### MATERIALS & FABRICATION

16. Lumber moisture content shall be 19% or less at the time of fabrication unless noted otherwise.
17. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
18. Unless expressly noted, the truss designs are not applicable for use with fire retardant or preservative treated lumber.
19. Plates shall be applied on both faces of truss at each joint and embedded fully. Knots and wane at joint locations shall be regulated in accordance with ANSI / TPI 1.

20. For a specified plate gauge and grade, the specified size is a minimum.

21. Connections not shown are the responsibility of others.

22. Adequate support shall be provided to resist gravity, lateral and uplift loads.

23. For 4X2 truss orientation, locate plates 0 - 1/16" from outside the edge of the truss.

24. Fabrication of truss shall be in accordance with ANSI / TPI 1.

### OTHER NOTES

25. Camber is a non-structural consideration and is the responsibility of truss fabricator.
26. Do not cut or alter any truss member or plate without prior approval from a professional engineer.
27. Lumber design values are in accordance with ANSI / TPI 1; lumber design values are by others.
28. Install specified hangers per manufacturer recommendations.

## SYMBOLS

### PLATE SIZE

**3X4** - The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

**-, /, |** Indicates required direction of slots; Reference "Joint Details" for more information.

20 Ga Gr40 connectors required

**3X10-20HS** - 20 Ga Gr60 connectors required

**8X10-18HS** - 18 Ga Gr60 connectors required

### LATERAL BRACING

When this symbol shown, continuous lateral bracing is required on the member of the truss.



### BEARING

Indicates location where bearings (supports) occur.



### PLATE LOCATION & ORIENTATION

The plate shall be centered on joint and/or placed in accordance with the design drawing/QC full scale details.



## REFERENCES

- **ANSI / TPI 1:** National Design Standard for Metal Plate Connected Wood Trusses
- **BCSI:** Building Component & Safety Information - Guide to Good Practice for Handling, Installing, Restraining, & Bracing of Metal Plate Connected Wood Trusses.
- **NDS:** National Design Specification for Wood Construction
- **ESR:** 1082 published by the International Code Council. [www.icc-es.org](http://www.icc-es.org)

**Quality Line Truss Co., LLC**

34593 S 4350 RD

Address 2

Adair, OK 74330

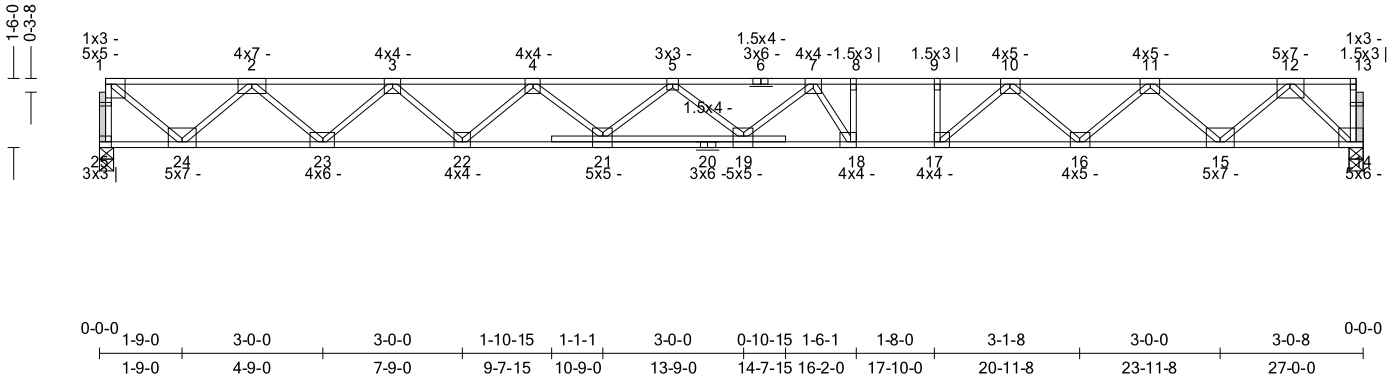
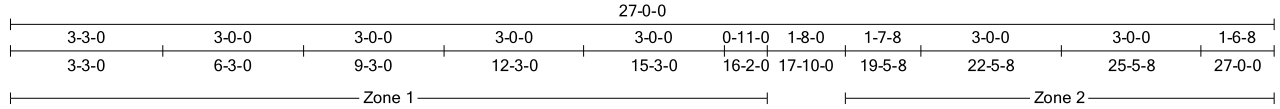
Truss:F01

Job: QU03647\_RESERVE\_BUILDING\_H\_02

Date: 02/17/26 14:47:51

Page: 1 of 1

SPAN 27-0-0	PITCH 0/12	QTY 9	OHL 0-0-0	OHR 0-0-0	PLY(S) 1	SPACING 19.19 in	WGT/PLY 145 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 40	Bldg Code : IBC 2018/	TC : 0.77 (8-9)	Vert TL: 1.19 in	L/267	(18-19)	L/240
TCDL : 10	TPI 1-2014	BC : 0.97 (18-19)	Vert LL: 0.68 in	L/466	(18-19)	L/360
BCLL : 0	Rep Mbr : Yes	Web : 0.35 (1-24)	Horz TL: 0.16 in		14	
BCDL : 10	Lumber D.O.L. : 100 %					

**Reaction**

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
25	1	3.5 in	1.50 in	1,511 lbs					
14	1	3.5 in	1.50 in	1,511 lbs					

**Material**

TC: SYP2400/1.8 4 x 2  
 BC: SYP2400/1.8 4 x 2  
 Web: SYP#1 4 x 2

**Loads**

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

**Member Forces**

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

Member	Member ID	Max CSI	Max Tension Force	Max Compression Force
TC	1-2	0.224	(-1,572 lbs)	4.5
	2-3	0.365	(-4,112 lbs)	5.7
	3-4	0.523	(-5,886 lbs)	7.8
BC	14-15	0.189	1,416 lbs	17-18
	15-16	0.402	4,001 lbs	18-19
	16-17	0.806	5,818 lbs	19-21
Web	1-25	0.159	(-1,488 lbs)	4-22
	1-24	0.347	2,094 lbs	4-21
	2-24	0.234	(-1,928 lbs)	5-21
	2-23	0.252	1,519 lbs	7-19
	3-23	0.167	(-1,376 lbs)	7-18
	3-22	0.171	1,030 lbs	8-18

**Notes**

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Unless otherwise specified by the Building Designer, two strongbacks recommended at one third points of the truss span. Strongback spacing or strongback to support should not exceed 10'-0".
- 3) The fabrication tolerance for this floor truss is 10% (Cq=0.90).
- 4) A creep factor of 2.00 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 6) □ Indicates non-structural members.



WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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Address 2

Adair, OK 74330

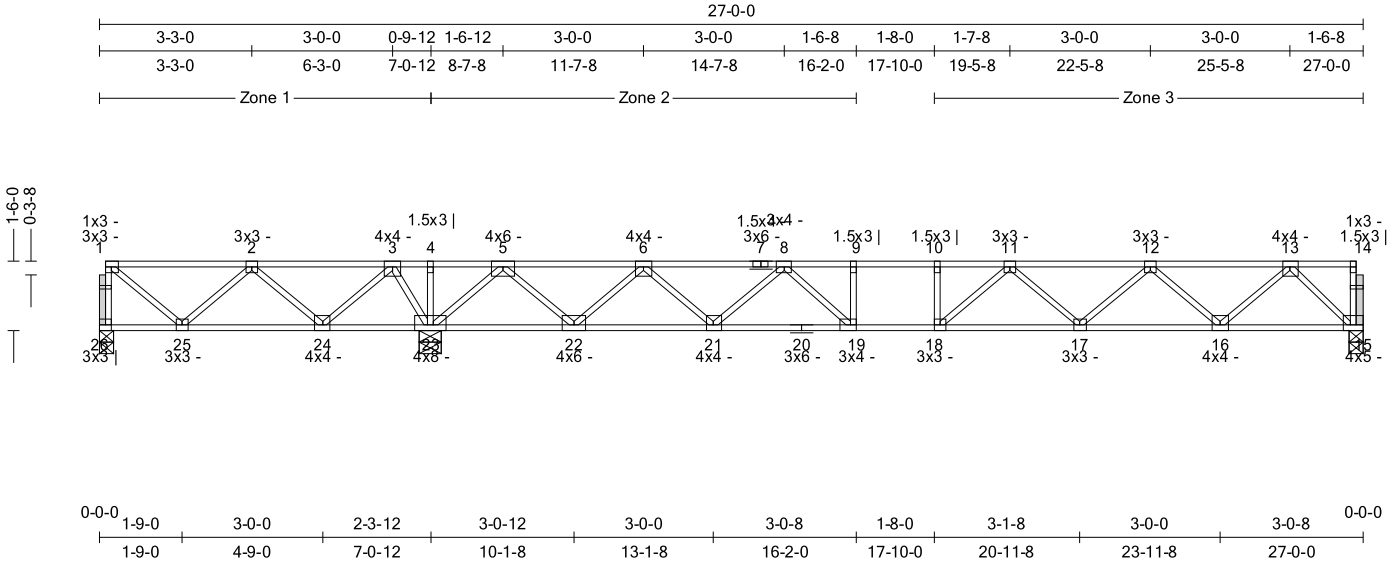
Truss:F02

Job: QU03647\_RESERVE\_BUILDING\_H\_02

Date: 02/17/26 14:47:52

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SPAN 27-0-0	PITCH 0/12	QTY 4	OHL 0-0-0	OHR 0-0-0	PLY(S) 1	SPACING 19.19 in	WGT/PLY 137 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 40	Bldg Code : IBC 2018/	TC : 0.59 (2-3)	Vert TL: 0.33 in	L/707	(17-18)	L/240
TCDL : 10	TPI 1-2014	BC : 0.71 (18-19)	Vert LL: 0.18 in	L/999	(17-18)	L/360
BCLL : 0	Rep Mbr : Yes	Web : 0.25 (5-22)	Horz TL: 0.03 in		15	
BCDL : 10	Lumber D.O.L. : 100 %					

02/17/2026

**Reaction**

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
23	1	5.5 in	1.50 in	2,178 lbs	.	.	.	.	.
26	1	3.5 in	1.50 in	187 lbs	-304 lbs	.	.	-304 lbs	.
15	1	3.5 in	1.50 in	949 lbs	.	.	.	.	.



**Material**

TC: SYP#1 4 x 2  
BC: SYP#1 4 x 2  
Web: SYP#1 4 x 2

**Loads**

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

**Member Forces**

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

Member	Force 1	Force 2	Force 3	Force 4	Force 5	Force 6	Force 7	Force 8	Force 9	Force 10
TC	1-2 0.439 378 lbs	4-5 0.582 2,484 lbs	9-10 0.524 (-2,728 lbs)	12-13 0.272 (-1,598 lbs)	2-3 0.585 1,260 lbs	6-8 0.312 (-1,630 lbs)	10-11 0.455 (-2,728 lbs)	3-4 0.575 2,484 lbs	8-9 0.531 (-2,728 lbs)	11-12 0.317 (-2,588 lbs)
BC	15-16 0.227 863 lbs	18-19 0.713 2,728 lbs	22-23 0.130 (-1,127 lbs)	16-17 0.486 2,229 lbs	19-21 0.636 2,243 lbs	23-24 0.204 (-1,980 lbs)	17-18 0.701 2,805 lbs	21-22 0.258 943 lbs	24-25 0.099 (-787 lbs)	
Web	1-26 0.060 312 lbs	5-23 0.219 (-1,808 lbs)	9-19 0.037 (-307 lbs)	1-25 0.068 (-503 lbs)	5-22 0.245 1,481 lbs	12-17 0.081 487 lbs	2-25 0.106 556 lbs	6-22 0.161 (-1,327 lbs)	12-16 0.104 (-856 lbs)	
	2-24 0.107 (-880 lbs)	6-21 0.154 931 lbs	13-16 0.165 997 lbs	3-24 0.166 1,002 lbs	8-21 0.101 (-833 lbs)	13-15 0.144 (-1,230 lbs)	3-23 0.109 (-990 lbs)	8-19 0.129 677 lbs		

**Notes**

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
- 3) The fabrication tolerance for this floor truss is 10% (Cq = 0.90).
- 4) A creep factor of 2.00 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 6)  Indicates non-structural members.
- 7) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 26 may need to be considered.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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Eagle Metal Products

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Address 2

Adair, OK 74330

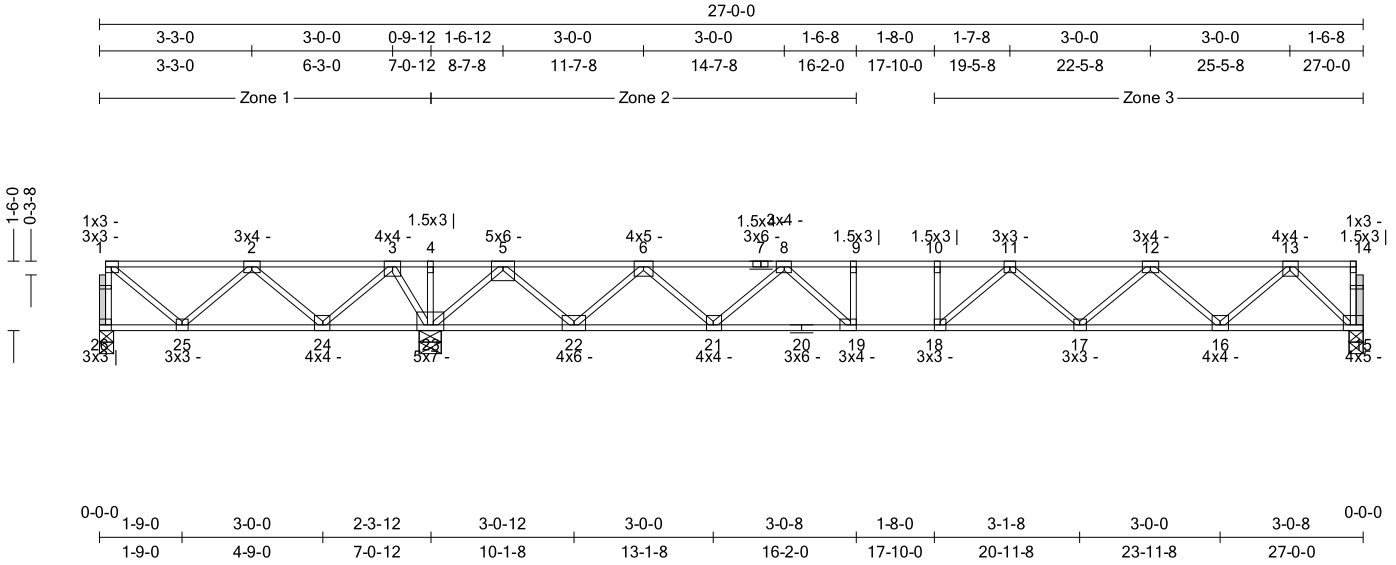
Truss:F03

Job: QU03647\_RESERVE\_BUILDING\_H\_02

Date: 02/17/26 14:47:54

Page: 1 of 2

SPAN 27-0-0	PITCH 0/12	QTY 2	OHL 0-0-0	OHR 0-0-0	PLY(S) 2	SPACING 19.19 in	WGT/PLY 138 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 40	Bldg Code : IBC 2018/	TC : 0.77 (2-3)	Vert TL: 0.32 in	L/723	(17-18)	L/240
TCDL : 10	TPI 1-2014	BC : 0.84 (17-18)	Vert LL: 0.09 in	L/999	(17-18)	L/360
BCLL : 0	Rep Mbr : No	Web : 0.25 (5-22)	Horz TL: 0.03 in		15	
BCDL : 10	Lumber D.O.L. : 100 %					

02/17/2026

**Reaction**

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
23	1	5.5 in	1.50 in	4,324 lbs	.	.	.	.	.
26	1	3.5 in	1.50 in	79 lbs	-411 lbs	.	.	-411 lbs	.
15	1	3.5 in	1.50 in	1,879 lbs	.	.	.	.	.

**Material**

TC: SYP#1 4 x 2  
 BC: SYP#1 4 x 2  
 Web: SYP#1 4 x 2

**Loads**

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	27-0-0	Down	Proj	110 plf	110 plf	

**Member Forces**

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	BC	Web	Member ID	Max CSI	Max Tension Force	Max Compression Force	Other Values
1-2	0.593	329 lbs	4-5	0.728	2,473 lbs		
2-3	0.766	1,253 lbs	6-8	0.430		(-1,603 lbs)	10-11 0.487 (-2,704 lbs)
3-4	0.716	2,473 lbs	8-9	0.591		(-2,704 lbs)	11-12 0.450 (-2,555 lbs)
15-16	0.208	870 lbs	18-19	0.811	2,704 lbs		22-23 0.143 (-1,107 lbs)
16-17	0.483	2,226 lbs	19-21	0.718	2,238 lbs		23-24 0.235 (-1,960 lbs)
17-18	0.840	2,797 lbs	21-22	0.245	949 lbs		24-25 0.093 (-669 lbs)
1-25	0.059	(-438 lbs)	5-22	0.251	1,435 lbs		11-17 0.044 (-327 lbs)
2-25	0.088	461 lbs	6-22	0.172		(-1,354 lbs)	12-17 0.076 446 lbs
2-24	0.117	(-913 lbs)	6-21	0.154	887 lbs		12-16 0.114 (-888 lbs)
3-24	0.170	971 lbs	8-21	0.110		(-861 lbs)	13-16 0.166 952 lbs
3-23	0.118	(-1,008 lbs)	8-19	0.123	644 lbs		13-15 0.152 (-1,240 lbs)
5-23	0.230	(-1,820 lbs)	9-19	0.038		(-316 lbs)	

**Notes**

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
- 3) The fabrication tolerance for this floor truss is 10% (Cq=0.90).
- 4) A creep factor of 2.00 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 6) □ Indicates non-structural members.



WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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Address 2

Adair, OK 74330

Truss:F03

Job: QU03647\_RESERVE\_BUILDING\_H\_02

Date: 02/17/26 14:47:54

Page: 2 of 2

SPAN  
27-0-0

PITCH  
0/12

QTY  
2

OHL  
0-0-0

OHR  
0-0-0

PLY(S)  
2

SPACING  
19.19 in

WGT/PLY  
138 lbs

- 7) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 26 may need to be considered.
- 8) The forces shown for this multi-ply truss are per ply and the reactions are for all plies. Two identical trusses shall be built and attached as follows: SDS Simpson 0.250"x6" Screws TC - 1 row @ 2-0-0 oc, BC - 1 row @ 2-0-0 oc, Webs - 1 @ 2-0-0 oc, minimum one fastener per web.
- 9) Screws shall be installed in the same truss ply that the hangers are attached to. If both plies are loaded, the screws shall be divided between the two plies, with the spacing on each side twice the minimum indicated.
- 10) Strongbacks shall be attached to each ply.
- 11) Center screw vertically on the 1-1/2" dimension of chords and webs. If splitting occurs, it may be necessary to pre-drill the holes in accordance with the NDS.
- 12) Install screws per manufacturer recommendations.

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Eagle Metal Products

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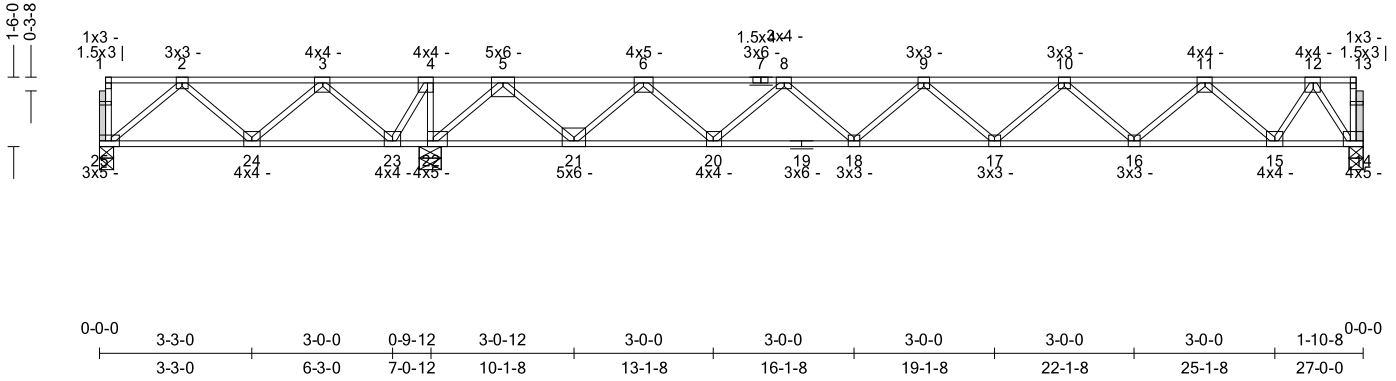
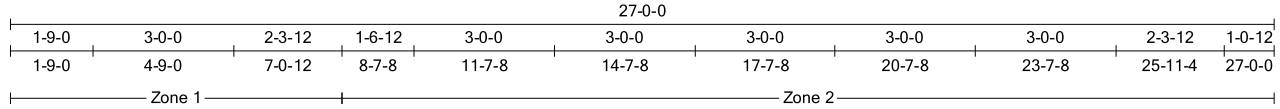
Truss:F03-Wall

Job: QU03647\_RESERVE\_BUILDING\_H\_02

Date: 02/17/26 14:47:56

Page: 1 of 2

SPAN 27-0-0	PITCH 0/12	QTY 1	OHL 0-0-0	OHR 0-0-0	PLY(S) 2	SPACING 19.19 in	WGT/PLY 139 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 40	Bldg Code : IBC 2018/	TC : 0.45 (3-4)	Vert TL: 0.2 in	L / 999	(17-18)	L / 240
TCDL : 10	TPI 1-2014	BC : 0.28 (17-18)	Vert LL: 0.06 in	L / 999	(17-18)	L / 360
BCLL : 0	Rep Mbr : No	Web : 0.26 (5-21)	Horz TL: 0.02 in		14	
BCDL : 10	Lumber D.O.L. : 100 %					

**Reaction**

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
22	1	5.5 in	1.50 in	4,504 lbs	.	.	.	.	.
25	1	3.5 in	N/A	0 lbs	-534 lbs	.	.	-534 lbs	.
14	1	3.5 in	1.50 in	1,833 lbs	.	.	.	.	.

**Material**

TC: SYP2400/1.8 4 x 2  
 BC: SYP2400/1.8 4 x 2  
 Web: SYP#1 4 x 2

**Loads**

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	27-0-0	Down	Proj	110 plf	110 plf	

**Member Forces**

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	2-3	0.311	866 lbs	5-6	0.293	337 lbs	9-10	0.269	(-2,645 lbs)
	3-4	0.448	2,299 lbs	6-8	0.229	(-1,393 lbs)	10-11	0.244	(-2,173 lbs)
	4-5	0.442	2,790 lbs	8-9	0.245	(-2,382 lbs)	11-12	0.167	(-987 lbs)
BC	14-15	0.070	563 lbs	17-18	0.275	2,670 lbs	21-22	0.144	(-1,413 lbs)
	15-16	0.183	1,725 lbs	18-20	0.214	2,043 lbs	22-23	0.252	(-2,790 lbs)
	16-17	0.264	2,568 lbs	20-21	0.091	690 lbs	23-24	0.155	(-1,414 lbs)
Web	2-25	0.088	464 lbs	5-22	0.231	(-1,833 lbs)	9-18	0.051	(-390 lbs)
	2-24	0.095	(-726 lbs)	5-21	0.255	1,460 lbs	10-16	0.070	(-536 lbs)
	3-24	0.142	811 lbs	6-21	0.177	(-1,394 lbs)	11-16	0.105	608 lbs
	3-23	0.154	(-1,215 lbs)	6-20	0.166	953 lbs	11-15	0.128	(-1,001 lbs)
	4-23	0.170	963 lbs	8-20	0.113	(-882 lbs)	12-15	0.146	834 lbs
	4-22	0.112	(-986 lbs)	8-18	0.079	460 lbs	12-14	0.122	(-1,048 lbs)

**Notes**

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
- 3) The fabrication tolerance for this floor truss is 10% (Cq = 0.90).
- 4) A creep factor of 2.00 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 6) □ Indicates non-structural members.



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34593 S 4350 RD

Address 2

Adair, OK 74330

Truss:F03-Wall

Job: QU03647\_RESERVE\_BUILDING\_H\_02

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SPAN  
27-0-0

PITCH  
0/12

QTY  
1

OHL  
0-0-0

OHR  
0-0-0

PLY(S)  
2

SPACING  
19.19 in

WGT/PLY  
139 lbs

- 7) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 25 may need to be considered.
- 8) The forces shown for this multi-ply truss are per ply and the reactions are for all plies. Two identical trusses shall be built and attached as follows: SDS Simpson 0.250"x6" Screws TC - 1 row @ 2-0-0 oc, BC - 1 row @ 2-0-0 oc, Webs - 1 @ 2-0-0 oc, minimum one fastener per web.
- 9) Screws shall be installed in the same truss ply that the hangers are attached to. If both plies are loaded, the screws shall be divided between the two plies, with the spacing on each side twice the minimum indicated.
- 10) Strongbacks shall be attached to each ply.
- 11) Center screw vertically on the 1-1/2" dimension of chords and webs. If splitting occurs, it may be necessary to pre-drill the holes in accordance with the NDS.
- 12) Install screws per manufacturer recommendations.

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Address 2

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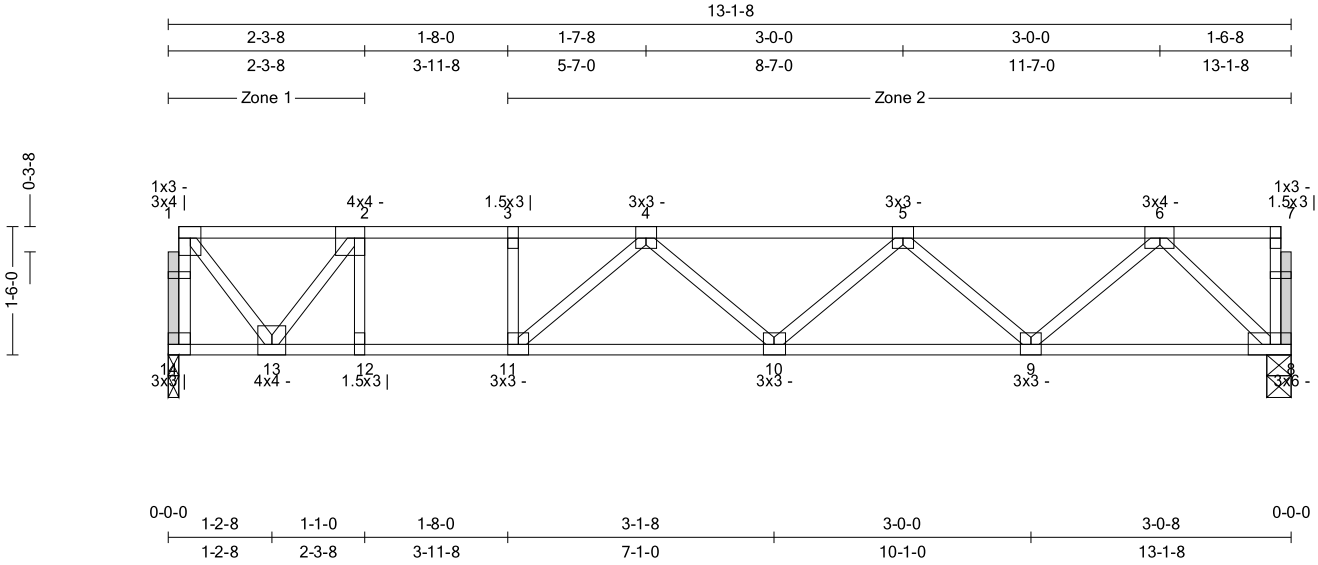
Truss:F04

Job: QU03647\_RESERVE\_BUILDING\_H\_02

Date: 02/17/26 14:47:57

Page: 1 of 1

SPAN 13-1-8	PITCH 0/12	QTY 3	OHL 0-0-0	OHR 0-0-0	PLY(S) 1	SPACING 19.19 in	WGT/PLY 69 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 40	Bldg Code : IBC 2018/	TC : 0.41 (1-2)	Vert TL: 0.3 in	L/510	(10-11)	L/240
TCDL : 10	TPI 1-2014	BC : 0.64 (12-13)	Vert LL: 0.17 in	L/889	(10-11)	L/360
BCLL : 0	Rep Mbr : Yes	Web : 0.14 (1-13)	Horz TL: 0.02 in		8	
BCDL : 10	Lumber D.O.L. : 100 %					

**Reaction**

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
14	1	1.5 in	1.50 in	735 lbs	.	.	.	.	.
8	1	3.5 in	1.50 in	735 lbs	.	.	.	.	.

**Material**

TC: SYP2400/1.8 4 x 2  
 BC: SYP2400/1.8 4 x 2  
 Web: SYP#1 4 x 2

**Loads**

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

**Member Forces**

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.412	(-503 lbs)	3-4	0.384	(-1,192 lbs)	5-6	0.154	(-1,147 lbs)
	2-3	0.402	(-1,192 lbs)	4-5	0.182	(-1,688 lbs)			
BC	8-9	0.116	652 lbs	10-11	0.485	1,639 lbs	12-13	0.645	1,192 lbs
	9-10	0.274	1,550 lbs	11-12	0.645	1,192 lbs			
Web	1-14	0.072	(-672 lbs)	2-12	0.079	473 lbs	6-9	0.111	672 lbs
	1-13	0.140	844 lbs	4-11	0.074	(-595 lbs)	6-8	0.108	(-929 lbs)
	2-13	0.129	(-1,155 lbs)	5-9	0.067	(-547 lbs)			

**Notes**

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
- 3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
- 4) A creep factor of 2.00 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 6)  Indicates non-structural members.



WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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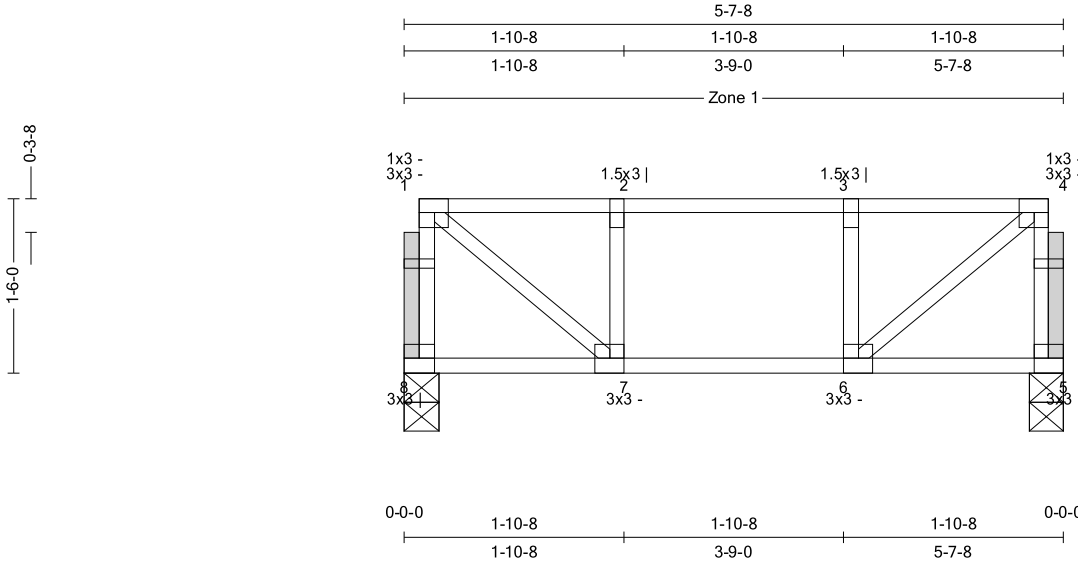
Truss:F05

Job: QU03647\_RESERVE\_BUILDING\_H\_02

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SPAN 5-7-8	PITCH 0/12	QTY 4	OHL 0-0-0	OHR 0-0-0	PLY(S) 1	SPACING 19.19 in	WGT/PLY 32 lbs
---------------	---------------	----------	--------------	--------------	-------------	---------------------	-------------------



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 40	Bldg Code : IBC 2018/	TC : 0.10 (1-2)	Vert TL: 0.01 in	L / 999	(5-6)	L / 240
TCDL : 10	TPI 1-2014	BC : 0.11 (6-7)	Vert LL: 0.01 in	L / 999	(5-6)	L / 360
BCLL : 0	Rep Mbr : Yes	Web : 0.06 (1-7)	Horz TL: 0 in		5	
BCDL : 10	Lumber D.O.L. : 100 %					

**Reaction**

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
8	1	3.5 in	1.50 in	315 lbs	.	.	.	.	.
5	1	3.5 in	1.50 in	315 lbs	.	.	.	.	.

**Material**

TC: SYP#1 4 x 2  
 BC: SYP#1 4 x 2  
 Web: SYP#1 4 x 2

**Loads**

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

**Member Forces**

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	BC	Web
		1-7 0.059 357 lbs
		4-6 0.059 357 lbs

**Notes**

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
- 3) A creep factor of 2.00 has been applied for this truss analysis.
- 4) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 5)  Indicates non-structural members.



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Address 2

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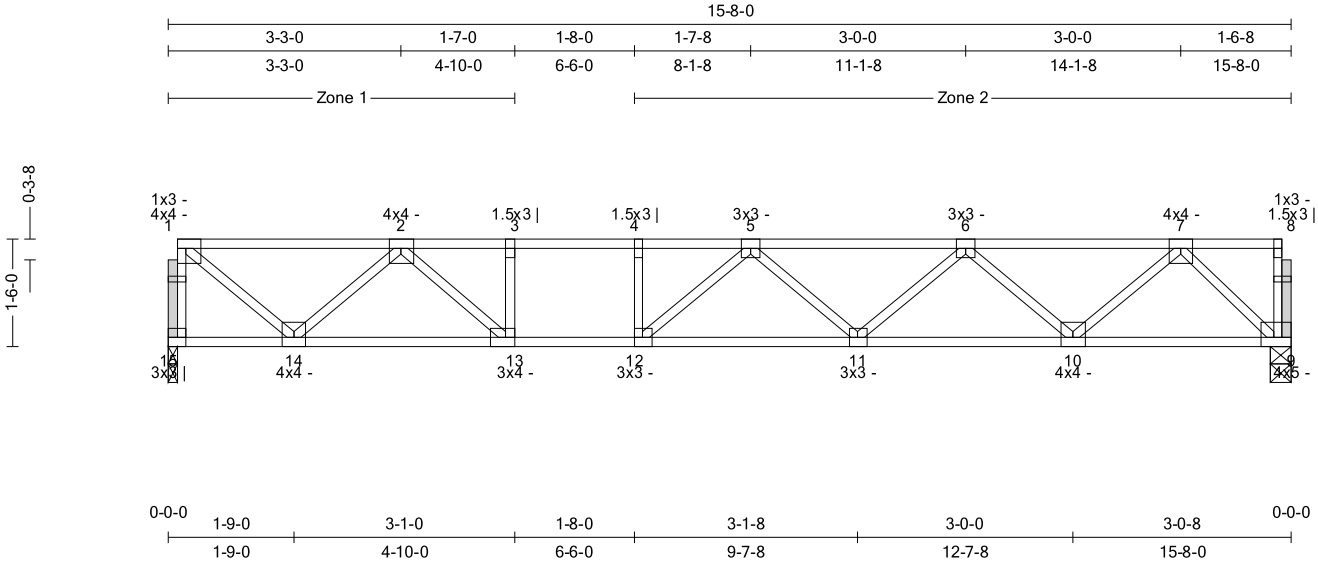
Truss:F06

Job: QU03647\_RESERVE\_BUILDING\_H\_02

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SPAN 15-8-0	PITCH 0/12	QTY 3	OHL 0-0-0	OHR 0-0-0	PLY(S) 1	SPACING 19.19 in	WGT/PLY 80 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 40	Bldg Code : IBC 2018/	TC : 0.61 (2-3)	Vert TL: 0.3 in	L/603	(11-12)	L/240
TCDL : 10	TPI 1-2014	BC : 0.77 (12-13)	Vert LL: 0.18 in	L/999	(11-12)	L/360
BCLL : 0	Rep Mbr : Yes	Web : 0.18 (1-14)	Horz TL: 0.03 in		9	
BCDL : 10	Lumber D.O.L. : 100 %					

02/17/2026

**Reaction**

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
15	1	1.5 in	1.50 in	877 lbs	.	.	.	.	.
9	1	3.5 in	1.50 in	877 lbs	.	.	.	.	.

**Material**

TC: SYP#1 4 x 2  
BC: SYP#1 4 x 2  
Web: SYP#1 4 x 2

**Loads**

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

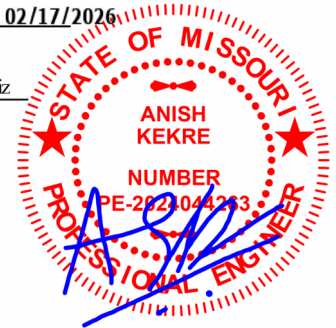
**Member Forces**

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.393	(-823 lbs)	3-4	0.587	(-2,196 lbs)	5-6	0.297	(-2,283 lbs)
	2-3	0.607	(-2,196 lbs)	4-5	0.525	(-2,196 lbs)	6-7	0.264	(-1,445 lbs)
BC	9-10	0.219	792 lbs	11-12	0.735	2,409 lbs	13-14	0.677	1,582 lbs
	10-11	0.486	2,000 lbs	12-13	0.769	2,196 lbs			
Web	1-15	0.089	(-839 lbs)	3-13	0.041	(-339 lbs)	7-10	0.147	887 lbs
	1-14	0.181	1,096 lbs	5-12	0.049	(-360 lbs)	7-9	0.132	(-1,129 lbs)
	2-14	0.125	(-1,030 lbs)	6-11	0.064	385 lbs			
	2-13	0.148	828 lbs	6-10	0.091	(-752 lbs)			

**Notes**

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
- 3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
- 4) A creep factor of 2.00 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 6)  Indicates non-structural members.



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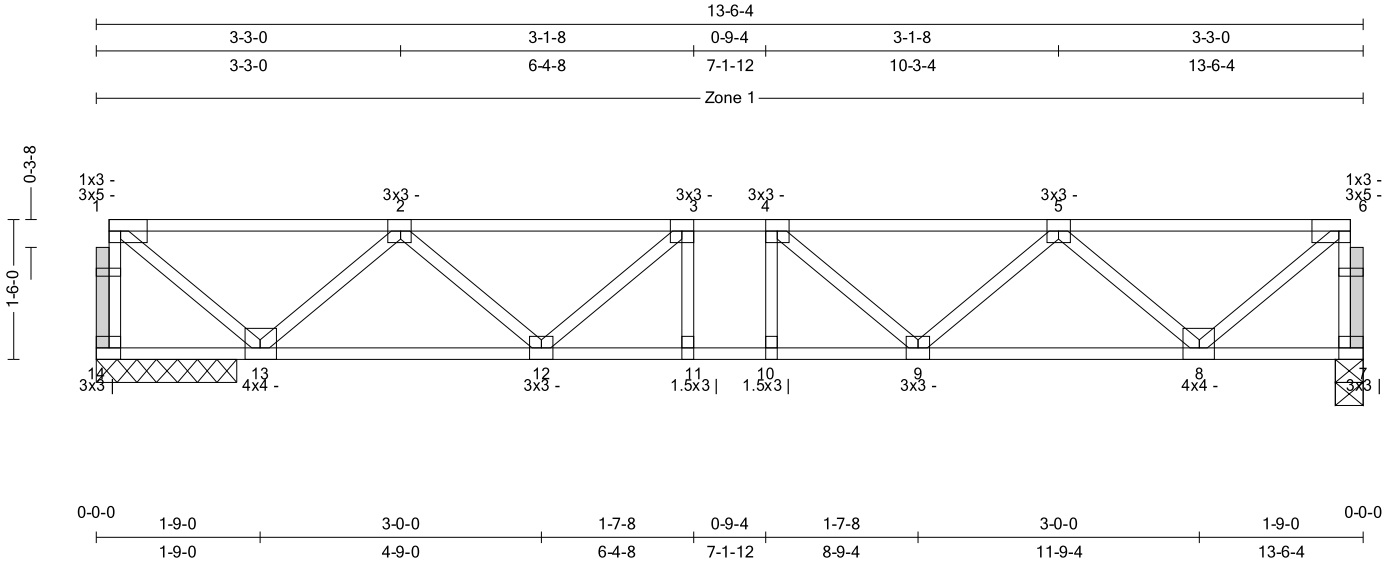
Truss:F09

Job: QU03647\_RESERVE\_BUILDING\_H\_02

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SPAN 13-6-4	PITCH 0/12	QTY 1	OHL 0-0-0	OHR 0-0-0	PLY(S) 1	SPACING 19.19 in	WGT/PLY 71 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 40	Bldg Code : IBC 2018/	TC : 0.42 (4-5)	Vert TL: 0.07 in	L / 999	(9-10)	L / 240
TCDL : 10	Rep Mbr : No	BC : 0.53 (13-14)	Vert LL: 0.04 in	L / 999	(9-10)	L / 360
BCLL : 0	Lumber D.O.L. : 100 %	Web : 0.14 (6-8)	Horz TL: 0.02 in		7	
BCDL : 10						

02/17/2026

**Reaction**

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
7	1	3.5 in	1.50 in	692 lbs	.	.	.	.	.
14	1	18 in	N/A	173 lbs	.	.	.	.	.
14	1	18 in	N/A	648 lbs	.	.	.	.	.

**Material**

TC: SYP#1 4 x 2  
 BC: SYP#1 4 x 2  
 Web: SYP#1 4 x 2

**Loads**

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

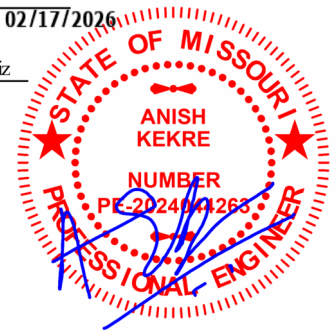
**Member Forces**

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

Member	Force	Member	Force	Member	Force	Member	Force
TC	2-3 0.399 (-1,210 lbs)	4-5 0.421 (-1,386 lbs)					
	3-4 0.220 (-1,487 lbs)	5-6 0.411 (-647 lbs)					
BC	8-9 0.294 1,179 lbs	10-11 0.441 1,487 lbs	12-13 0.327 833 lbs				
	9-10 0.441 1,487 lbs	11-12 0.412 1,487 lbs					
Web	2-13 0.114 (-936 lbs)	5-8 0.088 (-721 lbs)					
	2-12 0.085 511 lbs	6-8 0.143 862 lbs					
	3-12 0.047 (-369 lbs)	6-7 0.072 (-671 lbs)					

**Notes**

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
- 3) The fabrication tolerance for this floor truss is 10% (Cq=0.90).
- 4) A creep factor of 2.00 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 6) Indicates non-structural members.



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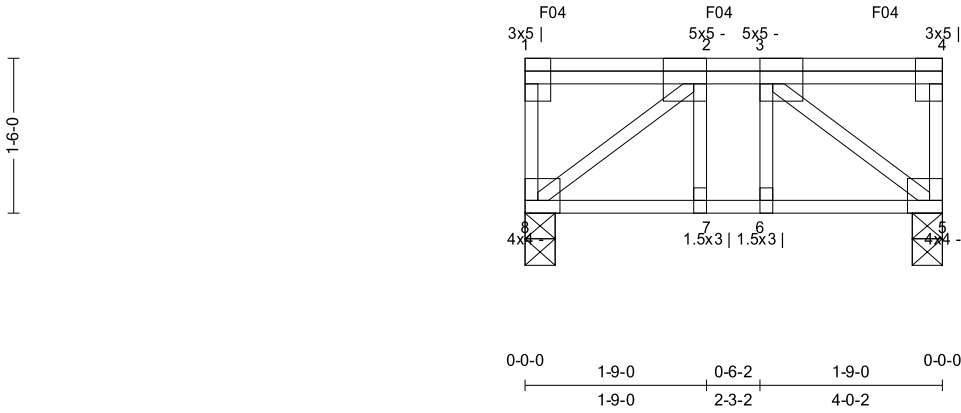
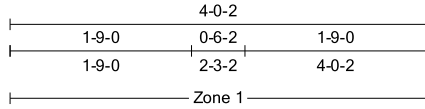
Truss:FG01

Job: QU03647\_RESERVE\_BUILDING\_H\_02

Date: 02/17/26 14:48:02

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SPAN 4-0-2	PITCH 0/12	QTY 1	OHL 0-0-0	OHR 0-0-0	PLY(S) 1	SPACING 49.5 in	WGT/PLY 30 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code: IBC 2018/	TC: 0.29 (3-4)	Vert TL: 0.01 in	L/999	(7-8)	L/240
TCLL: 40	Rep Mbr: No	BC: 0.14 (7-8)	Vert LL: 0.01 in	L/999	(6-7)	L/360
TCDL: 10	Lumber D.O.L.: 100 %	Web: 0.17 (2-8)	Horz TL: 0 in		5	
BCLL: 0						
BCDL: 10						

02/17/2026

**Reaction**

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
8	1	3.5 in	1.50 in	1,505 lbs					
5	1	3.5 in	1.50 in	1,327 lbs					

**Material**

TC: SYP2400/1.8 4 x 2  
 BC: SYP2400/1.8 4 x 2  
 Web: SYP#1 4 x 2

Load Case L1: Std Live Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	3-8-10	Down	Proj	165 plf	165 plf	
Top	3-8-10	4-0-2	Down	Proj	50.83 plf	50.83 plf	

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	3-8-10	Down	Proj	41.25 plf	41.25 plf	
Top	3-8-10	4-0-2	Down	Proj	12.71 plf	12.71 plf	
Bot	0-0-0	3-8-10	Down	Proj	41.25 plf	41.25 plf	
Bot	3-8-10	4-0-2	Down	Proj	12.71 plf	12.71 plf	

**Member Forces**

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	2-3	0.108	(-970 lbs)				
BC	5-6	0.137	970 lbs	6-7	0.106	970 lbs	7-8 0.141 970 lbs
Web	1-8	0.082	(-686 lbs)	3-5	0.166	(-1,247 lbs)	
	2-8	0.166	(-1,247 lbs)	4-5	0.062	(-517 lbs)	

**Truss to Truss Connection Summary**

Carried Truss	Carrying Chord	Carrying Offset
F04	TC	0-3-5
F04	TC	1-10-8
F04	TC	3-5-11

**Notes**

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).



WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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**Quality Line Truss Co., LLC**

34593 S 4350 RD

Address 2

Adair, OK 74330

Truss:FG01

Job: QU03647\_RESERVE\_BUILDING\_H\_02

Date: 02/17/26 14:48:02

Page: 2 of 2

SPAN  
4-0-2

PITCH  
0/12

QTY  
1

OHL  
0-0-0

OHR  
0-0-0

PLY(S)  
1

SPACING  
49.5 in

WGT/PLY  
30 lbs

3) A creep factor of 2.00 has been applied for this truss analysis.

4) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes.

This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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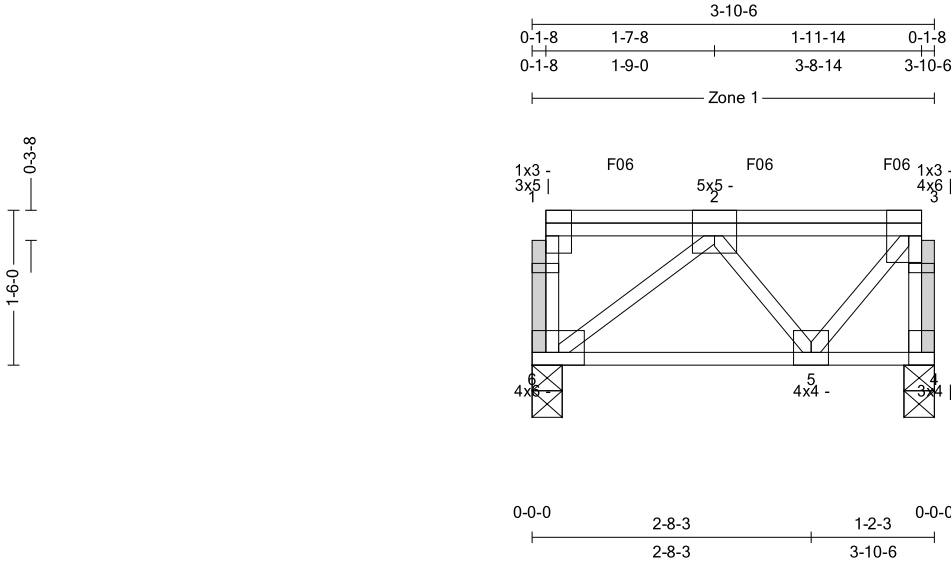
Truss:FG02

Job: QU03647\_RESERVE\_BUILDING\_H\_02

Date: 02/17/26 14:48:04

Page: 1 of 2

SPAN 3-10-6	PITCH 0/12	QTY 1	OHL 0-0-0	OHR 0-0-0	PLY(S) 1	SPACING 34.25 in	WGT/PLY 31 lbs
----------------	---------------	----------	--------------	--------------	-------------	---------------------	-------------------



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code: IBC 2018/	TC: 0.52 (1-2)	Vert TL: 0.02 in	L/999	(5-6)	L/240
TCLL: 40	TPI 1-2014	BC: 0.33 (5-6)	Vert LL: 0.01 in	L/999	(5-6)	L/360
TCDL: 10	Rep Mbr: No	Web: 0.19 (3-4)	Horz TL: 0 in		4	
BCLL: 0	Lumber D.O.L.: 100 %					
BCDL: 10						

02/17/2026

**Reaction**

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
6	1	3.5 in	1.50 in	1,353 lbs	.	.	.	.	.
4	1	3.5 in	1.50 in	1,674 lbs	.	.	.	.	.

**Material**

TC: SYP#1 4 x 2  
BC: SYP#1 4 x 2  
Web: SYP#1 4 x 2

**Loads**

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case L1: Std Live Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	3-10-6	Down	Proj	114.17 plf	114.17 plf	

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	3-10-6	Down	Proj	28.54 plf	28.54 plf	
Bot	0-0-0	3-10-6	Down	Proj	28.54 plf	28.54 plf	

**Member Forces**

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
TC	2-3	0.404	(-594 lbs)				
BC	5-6	0.330	1,056 lbs				
Web	1-6	0.047	(-393 lbs)	2-5	0.102	(-824 lbs)	3-4
	2-6	0.179	(-1,379 lbs)	3-5	0.175	984 lbs	

**Truss to Truss Connection Summary**

Carried Truss	Carrying Chord	Carrying Offset
F06	TC	0-10-3
F06	TC	2-2-3
F06	TC	3-6-1

**Notes**

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).



WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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Truss:FG02

Job: QU03647\_RESERVE\_BUILDING\_H\_02

Date: 02/17/26 14:48:04

Page: 2 of 2

SPAN  
3-10-6

PITCH  
0/12

QTY  
1

OHL  
0-0-0

OHR  
0-0-0

PLY(S)  
1

SPACING  
34.25 in

WGT/PLY  
31 lbs

3) A creep factor of 2.00 has been applied for this truss analysis.

4) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.

5)  Indicates non-structural members.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes.

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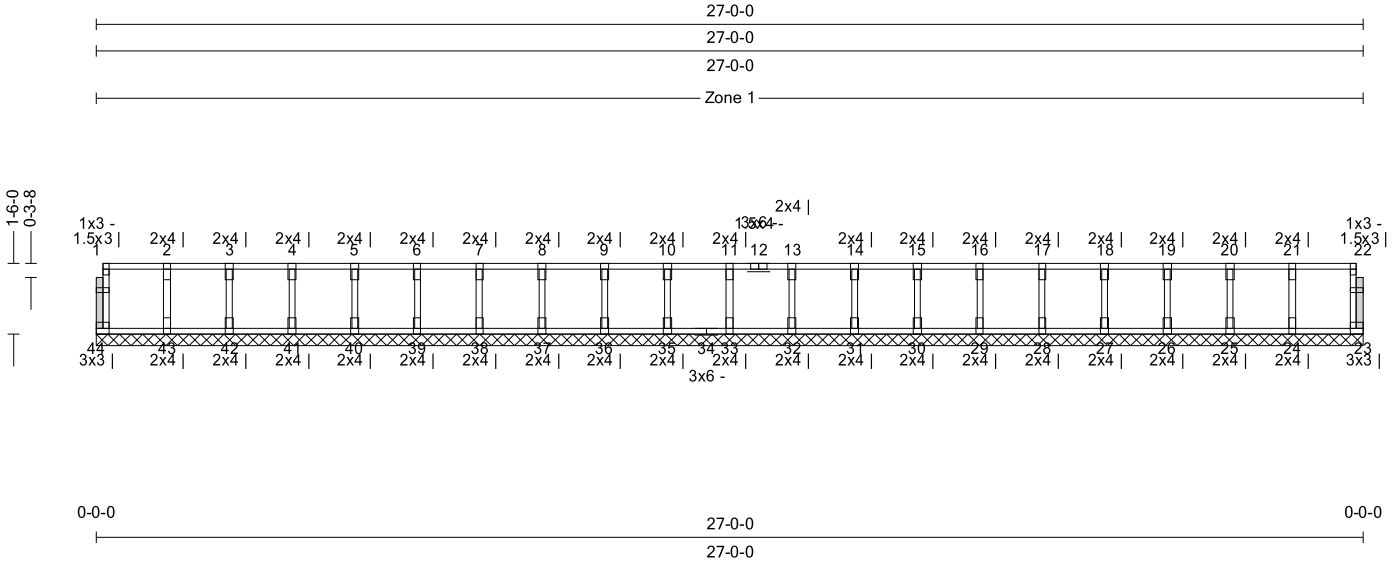
Truss:FL01

Job: QU03647\_RESERVE\_BUILDING\_H\_02

Date: 02/17/26 14:48:05

Page: 1 of 1

SPAN 27-0-0	PITCH 0/12	QTY 1	OHL 0-0-0	OHR 0-0-0	PLY(S) 1	SPACING 19.19 in	WGT/PLY 121 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 40	Bldg Code : IBC 2018/	TC : 0.06 (21-22)	Vert TL: 0 in UP	L / 999	23	L / 240
TCDL : 10	TPI 1-2014	BC : 0.02 (23-24)	Vert LL: 0 in	L / 999	23	L / 360
BCLL : 0	Rep Mbr : No	Web : 0.03 (1-44)	Horz TL: 0 in			
BCDL : 10	Lumber D.O.L. : 100 %					

02/17/2026

**Reaction**

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1		148 lbs	101 plf					-4 lbs

**Material**

TC: SYP#1 4 x 2  
 BC: SYP#1 4 x 2  
 Web: SYP#1 4 x 2

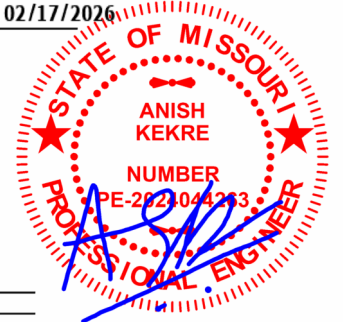
**Member Forces**

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	BC	Web

**Notes**

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
- 3) Gable requires continuous bottom chord bearing.
- 4) Continuous bearing knee-wall/ladder floor trusses are not designed for any loads from levels above. Additional blocking, by others, may be required in order to transfer loads.
- 5) Gable webs placed at 16" OC, U.N.O.
- 6) Attach gable webs with 2x4 20ga plates, U.N.O.
- 7) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- 8) The fabrication tolerance for this floor truss is 10% (Cq = 0.90).
- 9) A creep factor of 2.00 has been applied for this truss analysis.
- 10) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 11)  Indicates non-structural members.



WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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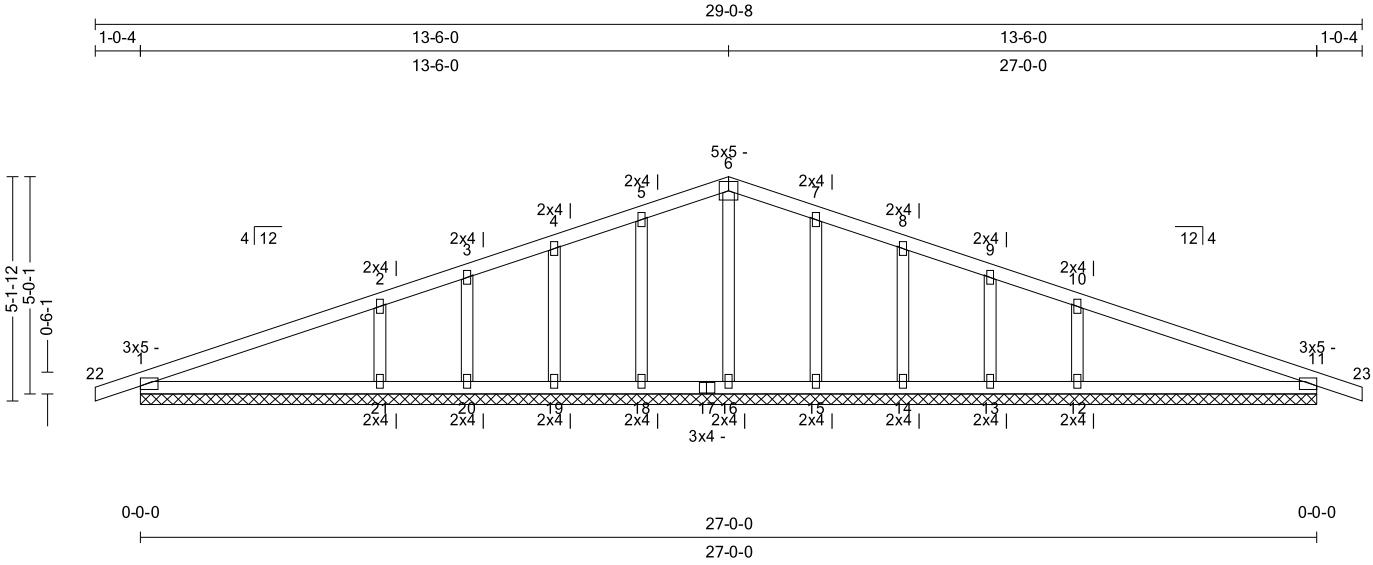
Truss:GE01

Job: QU03647\_RESERVE\_BUILDING\_H\_02

Date: 02/17/26 14:48:07

Page: 1 of 1

SPAN 27-0-0	PITCH 4/12	QTY 1	OHL 1-0-4	OHR 1-0-4	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 120 lbs
----------------	---------------	----------	--------------	--------------	-----------------	-----------------	-------------	------------------	--------------------



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 20	Bldg Code : IBC 2018/	TC : 0.29 (10-11)	Vert TL: 0.01 in	L / 999	(11-12)	L / 240
TCDL : 10	Rep Mbr : No	BC : 0.11 (11-12)	Vert LL: 0 in	L / 999	11	L / 360
BCLL : 0	Lumber D.O.L. : 115 %	Web : 0.04 (5-18)	Horz TL: 0 in			
BCDL : 10						

**Reaction**

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1		978 lbs	149 plf	-668 lbs	-150 lbs	-269 lbs	-668 lbs	-529 lbs

**Material**

TC: SYP#1 2 x 4  
BC: SYP#1 2 x 4  
Web: SYP#2 2 x 4

**Bracing**

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.  
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

**Loads**

- This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60

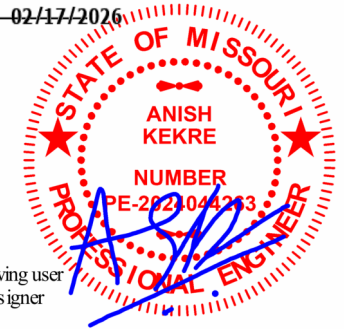
**Member Forces**

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

Member	Member ID	max CSI	max tension force	max compression force
TC	1-2	0.287	809 lbs	
	10-11	0.287	809 lbs	
BC				
Web				

**Notes**

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- Gable requires continuous bottom chord bearing.
- Gable webs placed at 24" OC, U.N.O.
- Attach gable webs with 2x4 20ga plates, U.N.O.
- Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- The fabrication tolerance for this roof truss is 20% (Cq = 0.80).
- A creep factor of 2.00 has been applied for this truss analysis.
- The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 11, 1 may need to be considered.
- Listed wind uplift reactions based on MWFRS & C&C loading.



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34593 S 4350 RD

Address 2

Adair, OK 74330

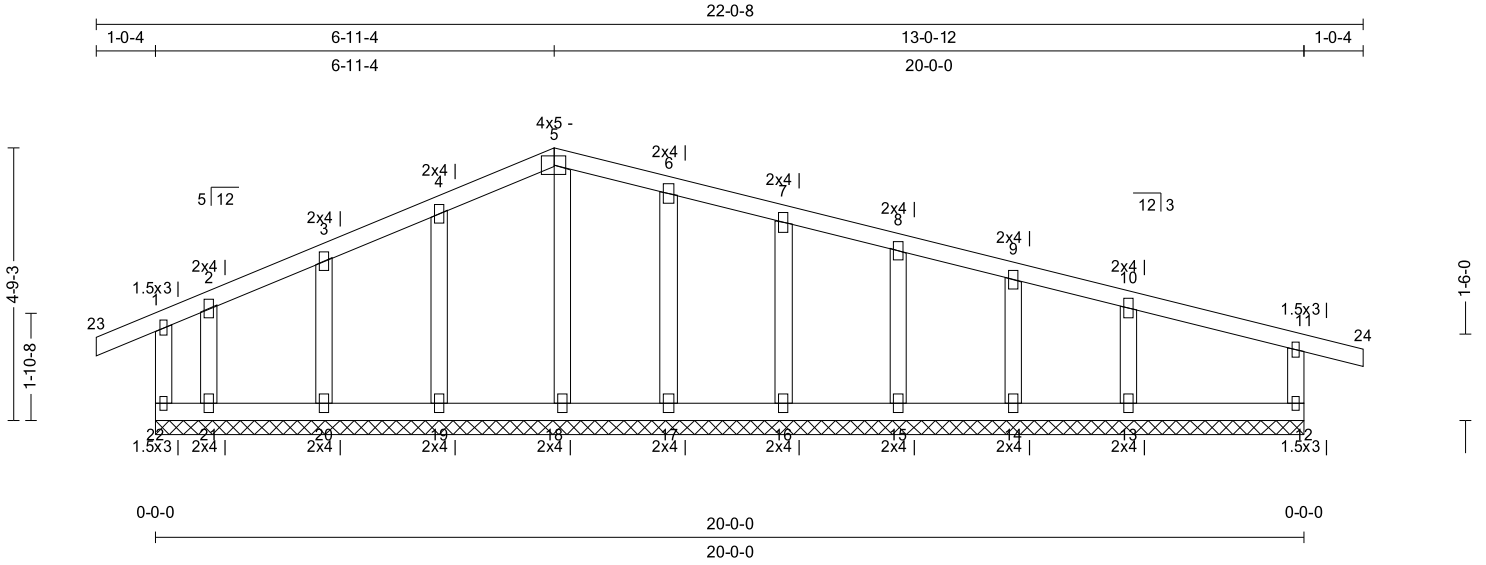
Truss:GE02

Job: QU03647\_RESERVE\_BUILDING\_H\_02

Date: 02/17/26 14:48:08

Page: 1 of 1

SPAN 20-0-0	PITCH 5/12	QTY 1	OHL 1-0-4	OHR 1-0-4	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 101 lbs
----------------	---------------	----------	--------------	--------------	-----------------	-----------------	-------------	------------------	--------------------



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 20	Bldg Code : IBC 2018/	TC : 0.12 (23-1)	Vert TL: 0 in	L / 999	(12-13)	L / 240
TCDL : 10	Rep Mbr : No	BC : 0.04 (13-14)	Vert LL: 0 in	L / 999	12	L / 360
BCLL : 0	Lumber D.O.L. : 115 %	Web : 0.08 (1-22)	Horz TL: 0 in			
BCDL : 10						

02/17/2026

**Reaction**

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1		216 lbs	92 plf		-65 lbs	-138 lbs	-138 lbs	44 lbs

**Material**

TC: SYP#1 2 x 4  
BC: SYP#1 2 x 4  
Web: SYP#2 2 x 4

**Bracing**

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.  
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

**Loads**

- This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60

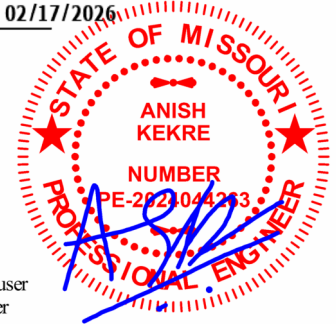
**Member Forces**

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	BC	Web

**Notes**

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- Gable requires continuous bottom chord bearing.
- Gable webs placed at 24" OC, U.N.O.
- Attach gable webs with 2x4 20g a plates, U.N.O.
- Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- The fabrication tolerance for this roof truss is 20% (Cq=0.80).
- A creep factor of 2.00 has been applied for this truss analysis.
- The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- Listed wind uplift reactions based on MWFRS & C&C loading.



WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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Address 2

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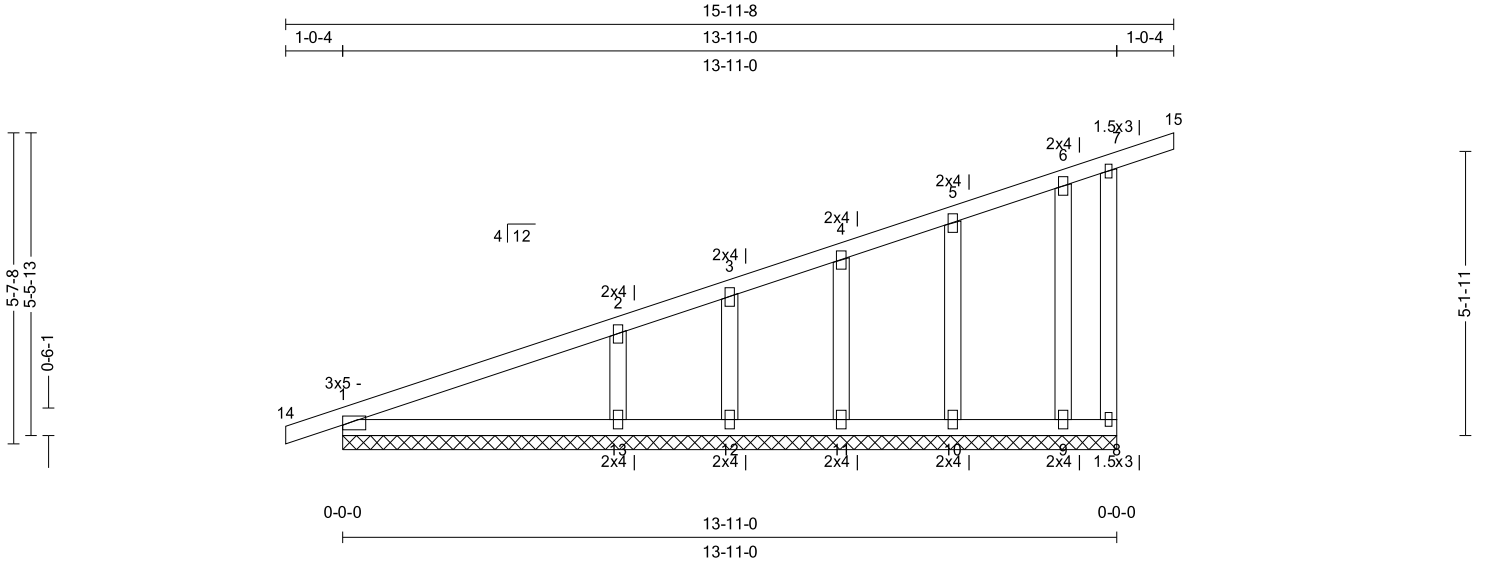
Truss:GE03

Job: QU03647\_RESERVE\_BUILDING\_H\_02

Date: 02/17/26 14:48:09

Page: 1 of 1

SPAN 13-11-0	PITCH 4/12	QTY 1	OHL 1-0-4	OHR 1-0-4	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 71 lbs
-----------------	---------------	----------	--------------	--------------	-----------------	-----------------	-------------	------------------	-------------------



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 20	Bldg Code : IBC 2018/	TC : 0.25 (1-2)	Vert TL: 0.01 in	L / 999	(13-1)	L / 240
TCDL : 10	TPI 1-2014	BC : 0.09 (13-1)	Vert LL: 0 in	L / 999	8	L / 360
BCLL : 0	Rep Mbr : No	Web : 0.22 (7-8)	Horz TL: 0 in			
BCDL : 10	Lumber D.O.L. : 115 %					

**Reaction**

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1		893 lbs	165 plf	-625 lbs	-197 lbs	-525 lbs	-625 lbs	-455 lbs

**Material**

TC: SYP#1 2 x 4  
 BC: SYP#1 2 x 4  
 Web: SYP#2 2 x 4

**Bracing**

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.  
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

**Loads**

- This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60

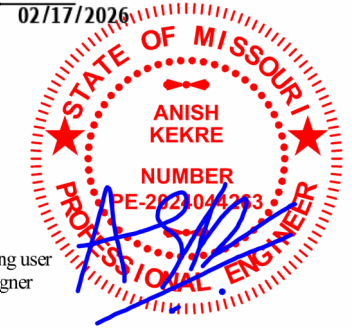
**Member Forces**

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.253	743 lbs	(-530 lbs)
BC				
Web				

**Notes**

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- Gable requires continuous bottom chord bearing.
- Gable webs placed at 24" OC, U.N.O.
- Attach gable webs with 2x4 20g plates, U.N.O.
- Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- The fabrication tolerance for this roof truss is 20% (Cq=0.80).
- A creep factor of 2.00 has been applied for this truss analysis.
- The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 1 may need to be considered.
- Listed wind uplift reactions based on MWFRS & C&C loading.



WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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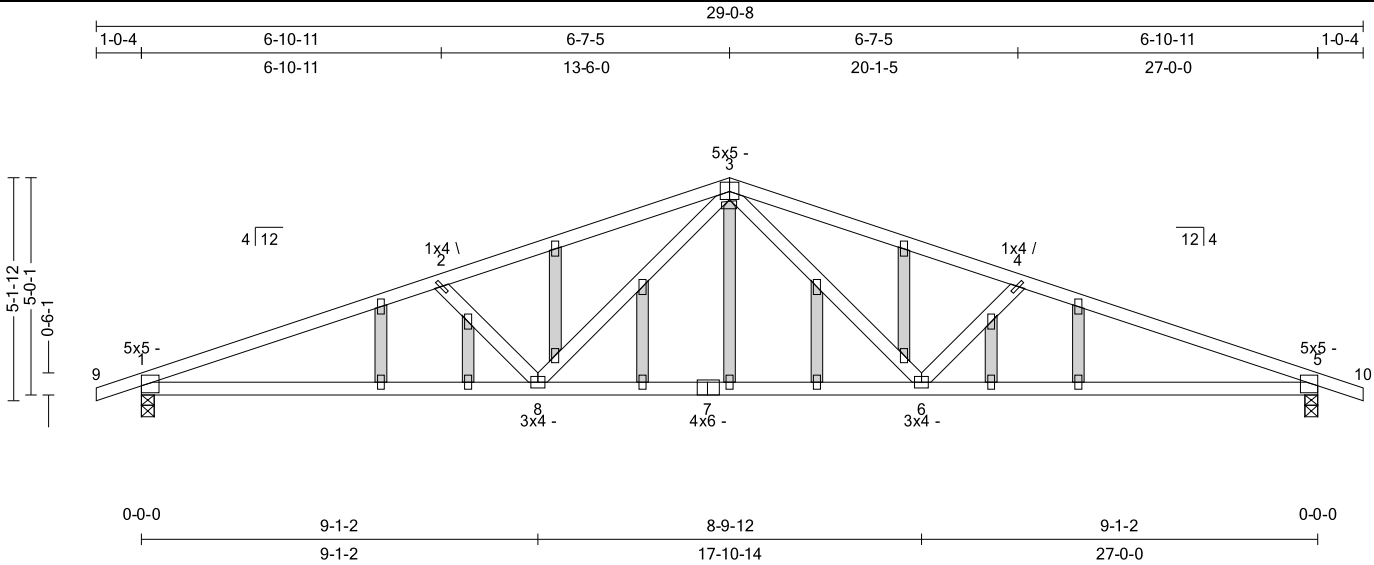
Truss:SGE01

Job: QU03647\_RESERVE\_BUILDING\_H\_02

Date: 02/17/26 14:48:11

Page: 1 of 1

SPAN 27-0-0	PITCH 4/12	QTY 1	OHL 1-0-4	OHR 1-0-4	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 139 lbs
----------------	---------------	----------	--------------	--------------	-----------------	-----------------	-------------	------------------	--------------------



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 20	Bldg Code : IBC 2018/	TC : 0.62 (2-3)	Vert TL: 0.48 in	L / 661	(6-7)	L / 240
TCDL : 10	TPI 1-2014	BC : 1.00 (8-1)	Vert LL: 0.16 in	L / 999	7	L / 360
BCLL : 0	Rep Mbr : No	Web : 0.14 (3-8)	Horz TL: 0.1 in		5	
BCDL : 10	Lumber D.O.L. : 115 %					

02/17/2026

**Reaction**

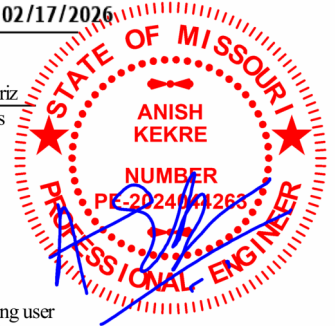
JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	3.5 in	1.67 in	1,411 lbs	.	-106 lbs	-319 lbs	-319 lbs	-4 lbs
5	1	3.5 in	1.67 in	1,411 lbs	.	-106 lbs	-319 lbs	-319 lbs	.

**Material**

TC: SYP#1 2 x 4  
BC: SYP#1 2 x 4  
Web: SYP#1 2 x 4

**Bracing**

TC: Sheathed or Purlins at 3-1-0, Purlin design by Others.  
BC: Sheathed



**Loads**

- This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL = 1.15. If the roof configuration differs from hip/gable Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

**Member Forces**

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.544	583 lbs	(-3,032 lbs)	3-4	0.618	531 lbs	(-2,743 lbs)
	2-3	0.618	531 lbs	(-2,743 lbs)	4-5	0.544	583 lbs	(-3,032 lbs)
BC	5-6	1.002	2,814 lbs	(-448 lbs)	6-8	0.859	1,951 lbs	
Web	2-8	0.081		(-421 lbs)	3-8	0.144	870 lbs	
					3-6	0.144	870 lbs	
					4-6	0.081		(-421 lbs)

**Notes**

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- Gable webs placed at 24" OC, U.N.O.
- Attach structural gable blocks with 2x4 20g plates, U.N.O.
- Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- A creep factor of 2.00 has been applied for this truss analysis.
- The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- ☐ Indicates non-structural members.
- Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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Address 2

Adair, OK 74330

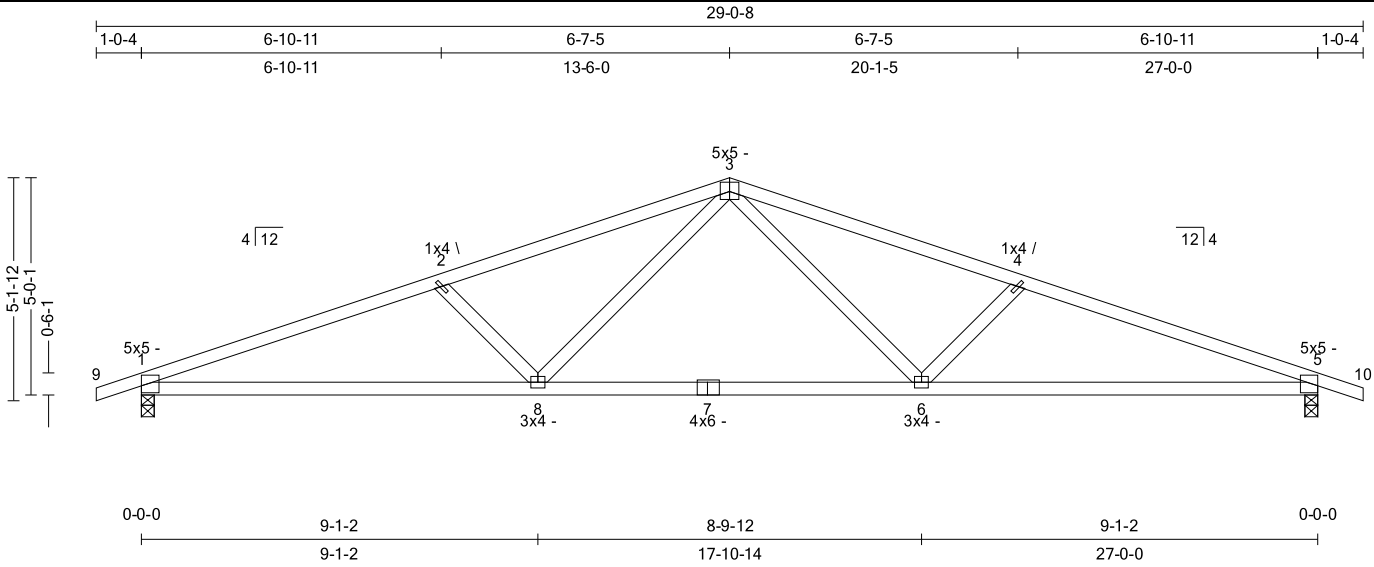
Truss:T01

Job: QU03647\_RESERVE\_BUILDING\_H\_02

Date: 02/17/26 14:48:12

Page: 1 of 1

SPAN 27-0-0	PITCH 4/12	QTY 14	OHL 1-0-4	OHR 1-0-4	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 107 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 20	Bldg Code : IBC 2018/	TC : 0.52 (2-3)	Vert TL: 0.48 in	L / 661	(6-7)	L / 240
TCDL : 10	TPI 1-2014	BC : 0.89 (8-1)	Vert LL: 0.16 in	L / 999	7	L / 360
BCLL : 0	Rep Mbr : Yes	Web : 0.14 (3-8)	Horz TL: 0.1 in		5	
BCDL : 10	Lumber D.O.L. : 115 %					

**Reaction**

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	3.5 in	1.67 in	1,411 lbs	.	-106 lbs	-319 lbs	-319 lbs	-4 lbs
5	1	3.5 in	1.67 in	1,411 lbs	.	-106 lbs	-319 lbs	-319 lbs	.

**Material**

TC: SYP#1 2 x 4  
BC: SYP#1 2 x 4  
Web: SYP#1 2 x 4

**Bracing**

TC: Sheathed or Purlins at 3-4-0, Purlin design by Others.  
BC: Sheathed or Purlins at 9-6-0, Purlin design by Others.

**Loads**

- This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

**Member Forces**

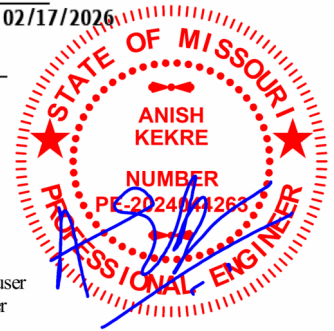
Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.469	583 lbs	(-3,032 lbs)	3-4	0.522	531 lbs	(-2,743 lbs)			
	2-3	0.522	531 lbs	(-2,743 lbs)	4-5	0.469	583 lbs	(-3,032 lbs)			
BC	5-6	0.890	2,814 lbs	(-448 lbs)	6-8	0.760	1,951 lbs		8-1	0.890	2,814 lbs (-448 lbs)
Web	2-8	0.081		(-421 lbs)	3-8	0.144	870 lbs		3-6	0.144	870 lbs
									4-6	0.081	(-421 lbs)

**Notes**

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq=0.80).
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- A creep factor of 2.00 has been applied for this truss analysis.
- The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- Listed wind uplift reactions based on MWFRS & C&C loading.

02/17/2026



WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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Address 2

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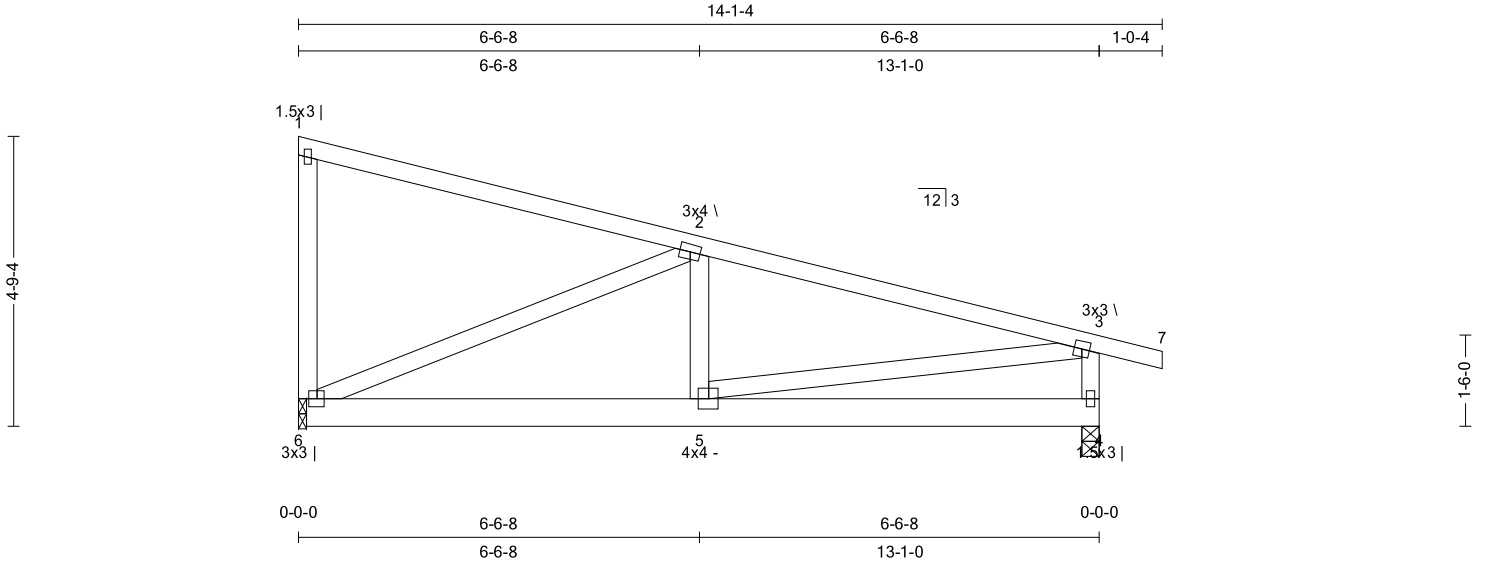
Truss:T02

Job: QU03647\_RESERVE\_BUILDING\_H\_02

Date: 02/17/26 14:48:14

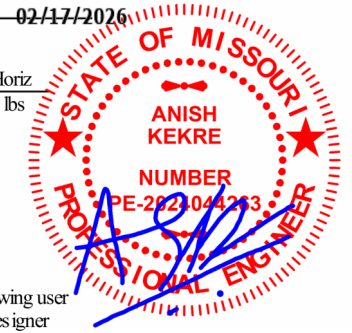
Page: 1 of 1

SPAN 13-1-0	PITCH -3 /12	QTY 2	OHL 0-0-0	OHR 1-0-4	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 76 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 20	Bldg Code : IBC 2018/	TC : 0.66 (2-3)	Vert TL: 0.05 in	L / 999	(5-6)	L / 240
TCDL : 10	Rep Mbr : No	BC : 0.26 (5-6)	Vert LL: 0.02 in	L / 999	(5-6)	L / 360
BCLL : 0	Lumber D.O.L. : 115 %	Web : 0.70 (2-6)	Horz TL: 0.01 in		4	
BCDL : 10						



**Reaction**

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
6	1	1.5 in	1.50 in	648 lbs	.	-64 lbs	-287 lbs	-287 lbs	-165 lbs
4	1	3.5 in	1.50 in	776 lbs	.	-40 lbs	-328 lbs	-328 lbs	.

**Material**

TC: SYP#1 2 x 4  
BC: SYP#1 2 x 6  
Web: SYP#1 2 x 4

**Bracing**

TC: Sheathed or Purlins at 5-0-0, Purlin design by Others.  
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

**Loads**

- 1) This truss has been designed for the effects of balanced (20 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (C<sub>e</sub> = 1.0), Thermal (C<sub>t</sub> = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 2) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 4) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

**Member Forces**

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	BC	Web	Member ID	max CSI	max tension force	max compression force
2-3	5-6	2-6	0.656	0.260	0.704	355 lbs (-959 lbs)
		3-5	0.151	0.068	0.151	894 lbs (-974 lbs)

**Notes**

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (C<sub>q</sub> = 0.80).
- 3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 4) A creep factor of 2.00 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 6) Please refer to Eagle Metals Engineering Details sheet titled, Girder Ledger Detail.
- 7) Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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Address 2

Adair, OK 74330

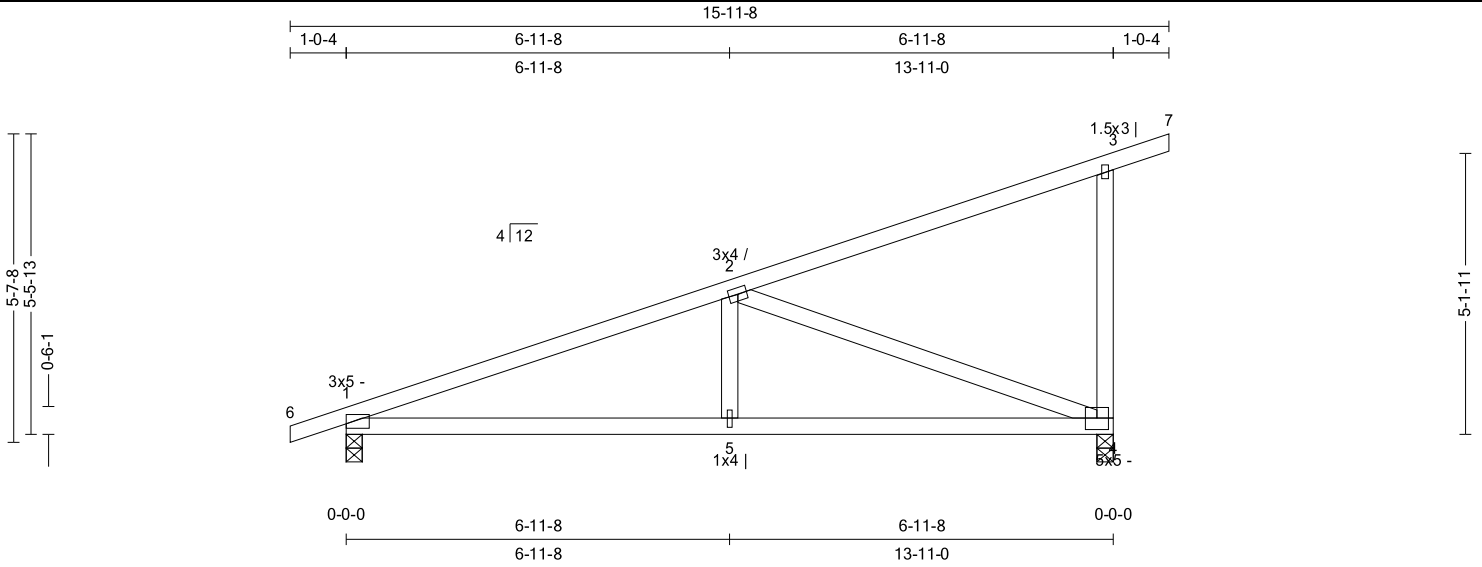
Truss:T03

Job: QU03647\_RESERVE\_BUILDING\_H\_02

Date: 02/17/26 14:48:15

Page: 1 of 1

SPAN 13-11-0	PITCH 4/12	QTY 1	OHL 1-0-4	OHR 1-0-4	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 62 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 20	Bldg Code : IBC 2018/	TC : 0.54 (2-3)	Vert TL: 0.15 in	L / 999	(4-5)	L / 240
TCDL : 10	TPI 1-2014	BC : 0.66 (4-5)	Vert LL: 0.06 in	L / 999	(4-5)	L / 360
BCLL : 0	Rep Mbr : No	Web : 0.95 (2-4)	Horz TL: 0.02 in		4	
BCDL : 10	Lumber D.O.L. : 115 %					

**Reaction**

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	3.5 in	1.50 in	749 lbs	.	-34 lbs	-305 lbs	-305 lbs	186 lbs
4	1	3.5 in	1.50 in	765 lbs	.	-94 lbs	-370 lbs	-370 lbs	.

**Material**

TC: SYP#1 2 x 4  
BC: SYP#1 2 x 4  
Web: SYP#1 2 x 4

**Bracing**

TC: Sheathed or Purlins at 5-0-0, Purlin design by Others.  
BC: Sheathed or Purlins at 9-11-0, Purlin design by Others.

**Loads**

- This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

**Member Forces**

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.511	354 lbs	(-1,233 lbs)
BC	4-5	0.660	1,115 lbs	(-373 lbs)
Web	2-5	0.056	335 lbs	
	2-4	0.949	502 lbs	(-1,190 lbs)

**Notes**

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- A creep factor of 2.00 has been applied for this truss analysis.
- The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- Listed wind uplift reactions based on MWFRS & C&C loading.



WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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Address 2

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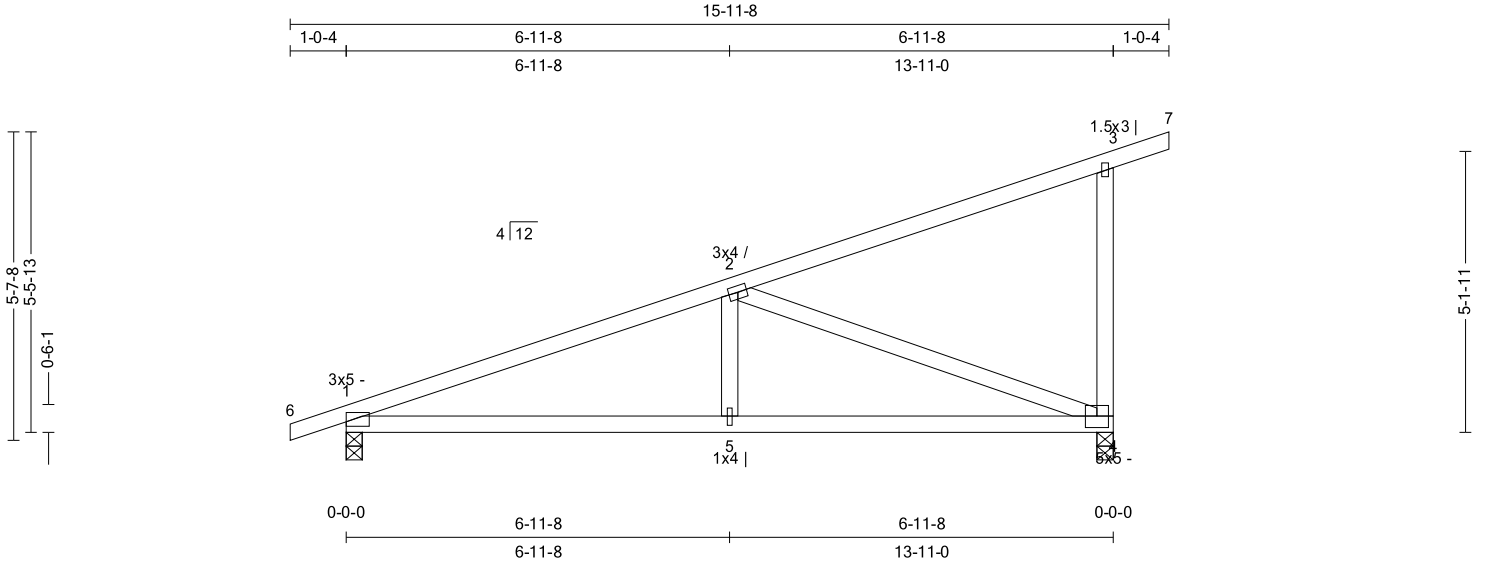
Truss:T04

Job: QU03647\_RESERVE\_BUILDING\_H\_02

Date: 02/17/26 14:48:17

Page: 1 of 1

SPAN 13-11-0	PITCH 4/12	QTY 1	OHL 1-0-4	OHR 1-0-4	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 62 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 20	Bldg Code : IBC 2018/	TC : 0.54 (2-3)	Vert TL: 0.15 in	L / 999	(4-5)	L / 240
TCDL : 10	TPI 1-2014	BC : 0.66 (4-5)	Vert LL: 0.06 in	L / 999	(4-5)	L / 360
BCLL : 0	Rep Mbr : No	Web : 0.95 (2-4)	Horz TL: 0.02 in		4	
BCDL : 10	Lumber D.O.L. : 115 %					

02/17/2026

**Reaction**

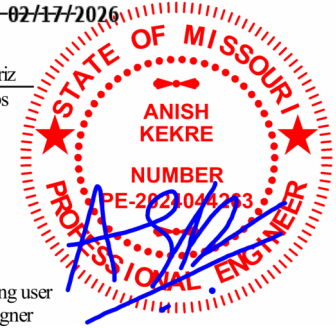
JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	3.5 in	1.50 in	749 lbs	.	-34 lbs	-305 lbs	-305 lbs	186 lbs
4	1	3.5 in	1.50 in	765 lbs	.	-94 lbs	-370 lbs	-370 lbs	.

**Material**

TC: SYP#1 2 x 4  
BC: SYP#1 2 x 4  
Web: SYP#1 2 x 4

**Bracing**

TC: Sheathed or Purlins at 5-0-0, Purlin design by Others.  
BC: Sheathed or Purlins at 9-11-0, Purlin design by Others.



**Loads**

- 1) This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 2) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 4) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

**Member Forces**

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.511	354 lbs	(-1,233 lbs)
BC	4-5	0.660	1,115 lbs	(-373 lbs)
Web	2-5	0.056	335 lbs	
	2-4	0.949	502 lbs	(-1,190 lbs)

**Notes**

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 4) A creep factor of 2.00 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 6) Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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Address 2

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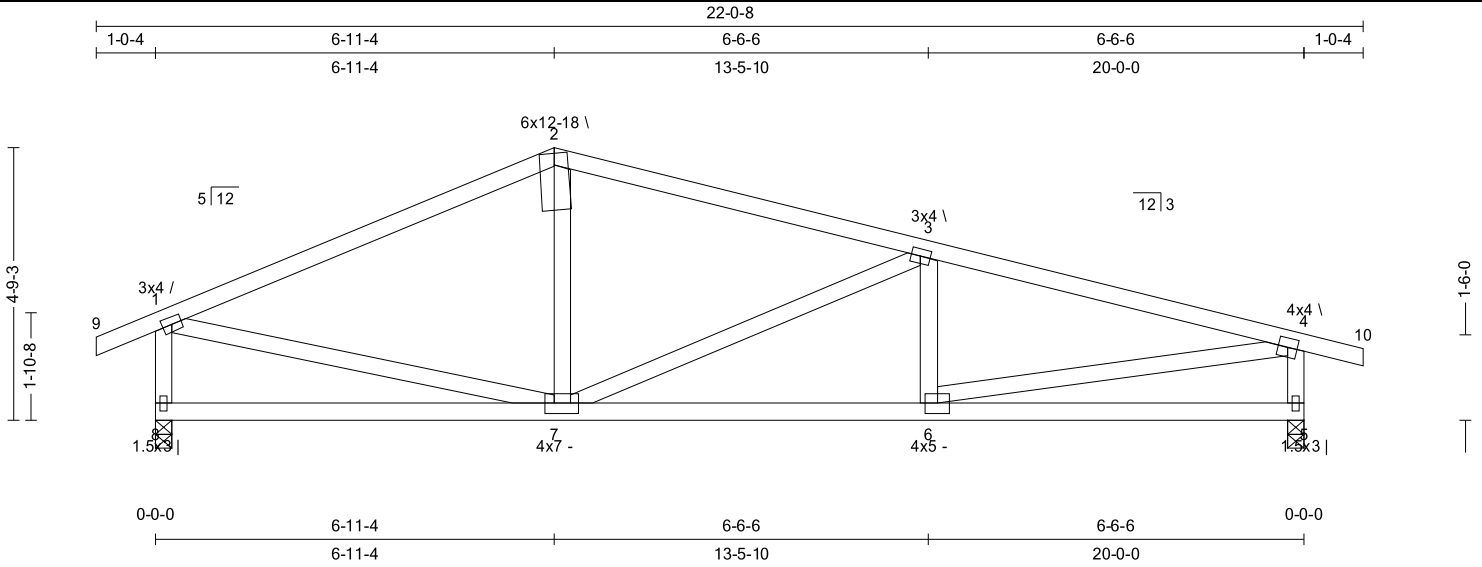
Truss:T05

Job: QU03647\_RESERVE\_BUILDING\_H\_02

Date: 02/17/26 14:48:18

Page: 1 of 1

SPAN 20-0-0	PITCH 5/12	QTY 1	OHL 1-0-4	OHR 1-0-4	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 103 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 20	Bldg Code : IBC 2018/	TC : 0.72 (1-2)	Vert TL: 0.17 in	L / 999	(7-8)	L / 240
TCDL : 10	Rep Mbr : No	BC : 0.68 (6-7)	Vert LL: 0.08 in	L / 999	(7-8)	L / 360
BCLL : 0	Lumber D.O.L. : 115 %	Web : 0.44 (3-7)	Horz TL: 0.01 in		5	
BCDL : 10						

**Reaction**

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
8	1	3.5 in	1.50 in	1,061 lbs	.	-84 lbs	-338 lbs	-338 lbs	-63 lbs
5	1	3.5 in	1.50 in	1,061 lbs	.	-83 lbs	-329 lbs	-329 lbs	.

**Material**

TC: SYP#1 2 x 4  
BC: SYP#1 2 x 4  
Web: SYP#1 2 x 4

**Bracing**

TC: Sheathed or Purlins at 4-1-0, Purlin design by Others.  
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

**Loads**

- 1) This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 2) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 4) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

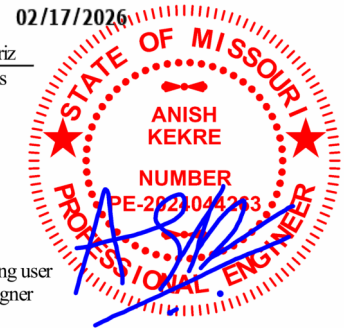
**Member Forces**

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	2-3	3-4	4-6	4-5
1-2	0.717	383 lbs (-1,232 lbs)	0.466	465 lbs (-1,698 lbs)
2-3	0.476	395 lbs (-1,164 lbs)		
BC	6-7	0.679	1,608 lbs (-332 lbs)	
Web	1-8	0.102	373 lbs (-943 lbs)	2-7
	1-7	0.183	1,106 lbs	3-7
				4-6
				4-5

**Notes**

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 4) A creep factor of 2.00 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 6) Listed wind uplift reactions based on MWFRS & C&C loading.



WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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