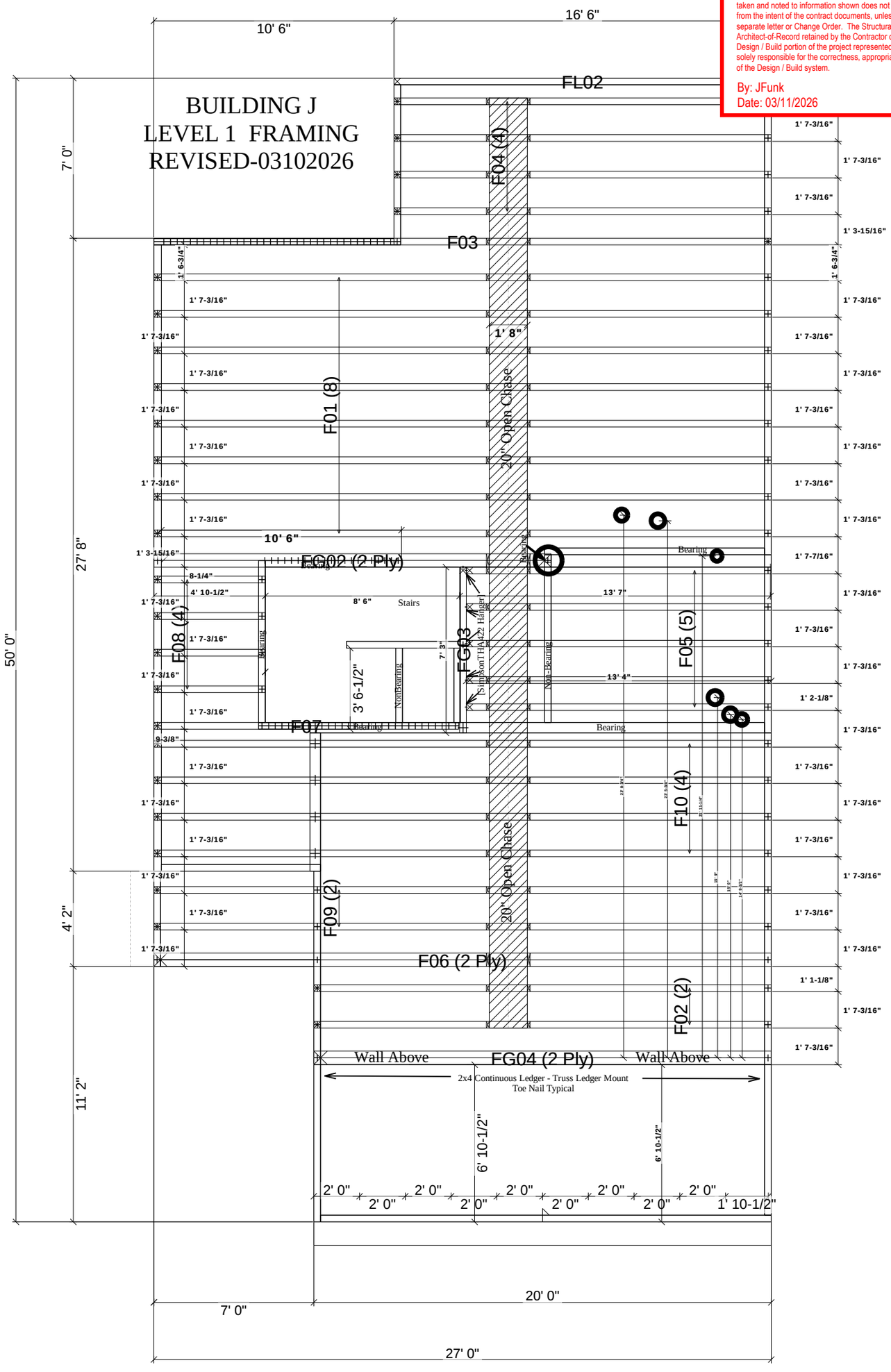
STAND STRUCTURAL ENGINEERING
8234 Robinson St
Overland Park, KS 66204
(913) 214-2169

BUILDING J
LEVEL 1 FRAMING
REVISED-03102026



Joe Frogge
Digitally signed by Joe Frogge
DN: CN=Joe Frogge, OU=Development, OU=LS Users, DC=COLS, DC=LOC
Date: 2026.03.16 14:14:25-05'00'

RELEASE FOR CONSTRUCTION
AS NOTED ON PLANS REVIEW
DEVELOPMENT SERVICES
LEE'S SUMMIT, MISSOURI
03/16/2026 2:13:39

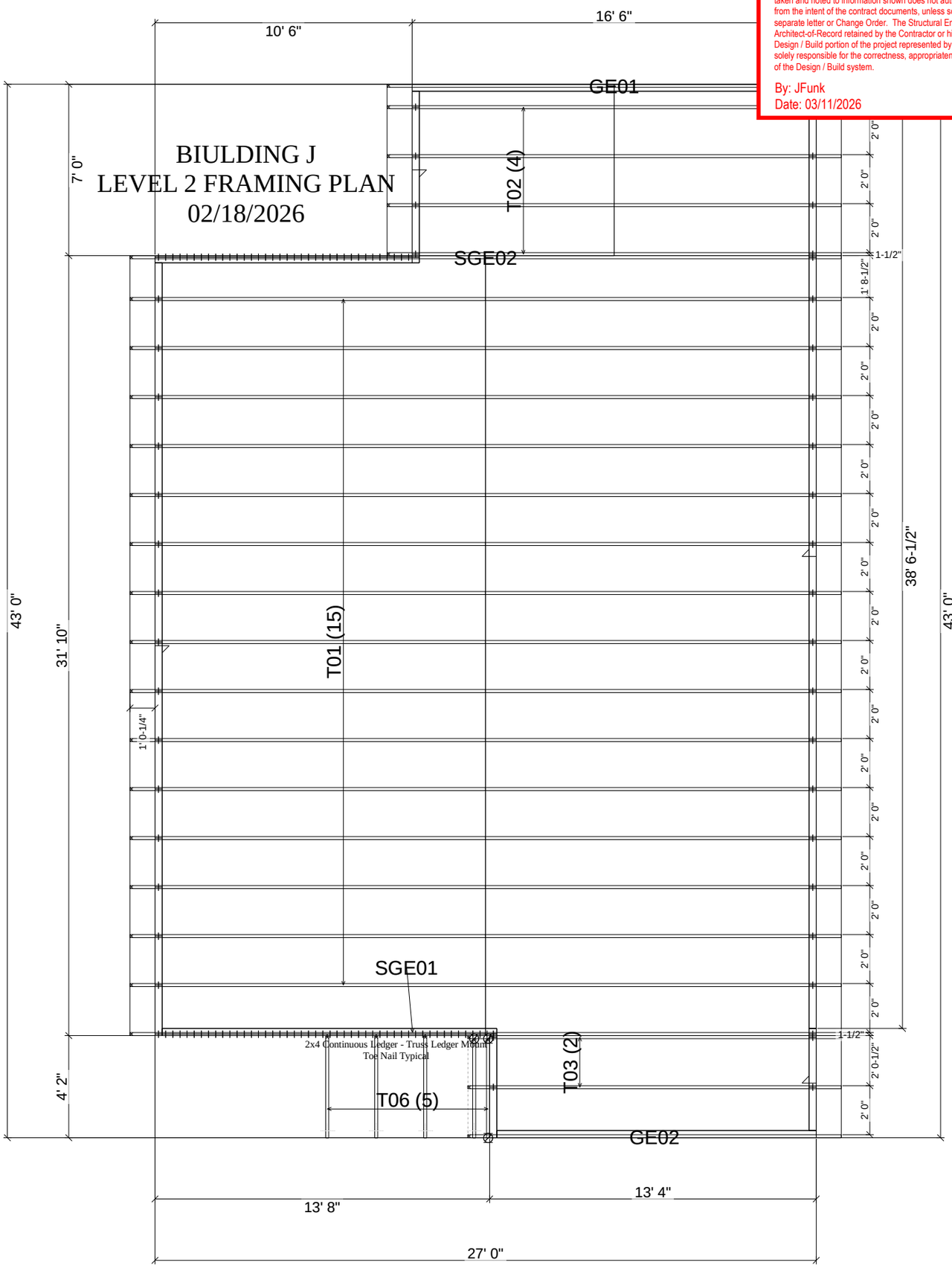
STAND STRUCTURAL ENGINEERING
8234 Robinson St
Overland Park, KS 66204
(913) 214-2169

☒ Reviewed ☐ Revise and Resubmit
☐ Reviewed as Noted ☐ Rejected
☐ Not required by the Contract Documents
☐ For Record Only

Review is only for general conformance with the design concept and the intent of the Contract Documents. Contractor is solely responsible for verifying dimensions, for establishing fabrication processes, means, techniques, sequences and procedures of construction and for coordination of work of all trades. Review action taken and noted to information shown does not authorize deviations from the intent of the contract documents, unless so stated in a separate letter or Change Order. The Structural Engineer and/or Architect-of-Record retained by the Contractor or his Supplier, for the Design / Build portion of the project represented by this submittal, is solely responsible for the correctness, appropriateness and adequacy of the Design / Build system.

By: JFunk
Date: 03/11/2026

BIULDING J
LEVEL 2 FRAMING PLAN
02/18/2026



EAGLE METAL

RELEASE FOR CONSTRUCTION
AS NOTED ON PLANS REVIEW
DEVELOPMENT SERVICES
LEE'S SUMMIT, MISSOURI
03/16/2026 2:13:43

The truss designs referenced below have been prepared by me or under my direct supervision based on the truss design criteria and requirements ("design criteria") provided by **Quality Line Truss**.

These truss designs are intended for the fabrication of individual building components that will perform to the design criteria provided. Any variance from the design criteria will render the affected truss designs inapplicable.

Listed below are the truss designs included in this package and covered by this seal.

Job: **QU03648_RESERVE BUILDING J_REVISED_02042026** - 1260641
F05

Any location identification is for file reference only. No determination of the appropriateness of design criteria for any specific project has been made in preparing the truss designs.

Please refer to individual truss designs for specific design criteria.

03/11/2026
STATE OF MISSOURI
ARTURO A. HERNANDEZ
NUMBER PE-2006000095
PROFESSIONAL ENGINEER

STAND STRUCTURAL ENGINEERING
8234 Robinson St
Overland Park, KS 66204
(913) 214-2169

☒ Reviewed ☐ Revise and Resubmit
☐ Reviewed as Noted ☐ Rejected
☐ Not required by the Contract Documents
☐ For Record Only

Review is only for general conformance with the design concept and the intent of the Contract Documents. Contractor is solely responsible for verifying dimensions, for establishing fabrication processes, means, techniques, sequences and procedures of construction and for coordination of work of all trades. Review action taken and noted to information shown does not authorize deviations from the intent of the contract documents, unless so stated in a separate letter or Change Order. The Structural Engineer and/or Architect-of-Record retained by the Contractor or his Supplier, for the Design / Build portion of the project represented by this submittal, is solely responsible for the correctness, appropriateness and adequacy of the Design / Build system.

By: JFunk
Date: 03/11/2026

Arturo A. Hernandez (MO, 2006000095)
My license expiration date for the state of MO is 12/31/2026.

IMPORTANT NOTE: The responsibility of the engineer sealing this package, as a Truss Engineer, is solely for design of individual trusses as individual building components based upon design criteria provided by others and set forth in the referenced truss drawings. The truss design criteria for the components have not been verified as appropriate for any particular building, project or use. Adequacy and suitability of design criteria and requirements for the truss designs for any specific project are the responsibility of the building designer, not the Truss Engineer, per ANSI/TPI-1, Chapter 2.

DESIGN NOTES

1. The Truss Design Drawing(s) provided with these Design Notes have been prepared under and are subject to ANSI / TPI 1 published by the Truss Plate Institute, www.tpinst.org. Capitalized terms have the meanings provided in ANSI / TPI 1.
2. Copies of each Truss Design Drawing shall be furnished to the installation contractor, Building Designer, Owner and all persons fabricating, handling, installing, bracing, or erecting the trusses.

DESIGN LIMITATIONS

3. The Truss Design Drawing is based upon specifications provided by the Building Designer in accordance with ANS1 / TPI 1. Neither the Truss Designer, Eagle, nor an engineer who seals this design (if any) assumes any responsibility for the adequacy or accuracy of specifications provided by the Building Designer.
4. The Building Designer is solely responsible for the suitability based upon the Truss Design Drawing and shall be responsible for reviewing and verifying that the information shown is in general conformance with the design of the Building.
5. Each Truss Design Drawing is for the individual building component (a truss). A seal on the Truss Design Drawing indicates acceptance of professional engineering responsibility solely for the individual truss.
6. Each Truss Design Drawing assumes trusses will be suitably protected from the environment.

HANDLING, INSTALLING, & BRACING

7. Refer to Building Component Safety Information (BCSI) for handling, installing, restraining and bracing trusses. Copies can be obtained from the Structural Building Components Association, www.sbcindustry.com.
8. Bracing shown on each Truss Design Drawing is for lateral support of individual truss components only to reduce buckling lengths. All temporary and permanent bracing, including lateral load and diagonal or cross bracing, are the responsibility, respectively, of the erector and Building Designer.
9. Eagle is not responsible for improper truss fabrication, handling, erection or bracing.
10. Compression chords shall be laterally braced by the roof or floor sheathing, directly attached, or have purlins provided at spacing shown, unless noted otherwise.
11. Bottom chord required bracing shall be at 1Oft spacing or less, if no structural rated ceiling is installed, unless noted otherwise.
12. Strongbacking shall be installed on all parallel chord trusses, including flooring systems, to limit deflection and reduce vibration. Refer to BCSI-B7.
13. Never exceed the design loading shown. Never stack building or other materials on inadequately braced truss; refer to BCSI.
14. Concentration of construction loads greater than the design loads shall not be applied to the trusses at any time; refer to BCSI.
15. Trusses shall be handled with care prior to erection to avoid damage. Refer to BCSI for recommended truss handling and erection.

MATERIALS & FABRICATION

16. Lumber moisture content shall be 19% or less at the time of fabrication unless noted otherwise.
17. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
18. Unless expressly noted, the truss designs are not applicable for use with fire retardant or preservative treated lumber.
19. Plates shall be applied on both faces of truss at each joint and embedded fully. Knots and wane at joint locations shall be regulated in accordance with ANSI / TPI 1.
20. For a specified plate gauge and grade, the specified size is a minimum.
21. Connections not shown are the responsibility of others.
22. Adequate support shall be provided to resist gravity, lateral and uplift loads.
23. For 4X2 truss orientation, locate plates 0 - 1/16" from outside the edge of the truss.
24. Fabrication of truss shall be in accordance with ANSI / TPI 1.

OTHER NOTES

25. Camber is a non-structural consideration and is the responsibility of truss fabricator.
26. Do not cut or alter any truss member or plate without prior approval from a professional engineer.
27. Lumber design values are in accordance with ANSI / TPI 1; lumber design values are by others.
28. Install specified hangers per manufacturer recommendations.

SYMBOLS

PLATE SIZE

3X4 - The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

-, /, I, Indicates required direction of slots; Reference "Joint Details" for more information.

20 Ga Gr40 connectors required
3X10-20HS - 20 Ga Gr60 connectors required
8X10-18HS - 18 Ga Gr60 connectors required

LATERAL BRACING

When this symbol shown, continuous lateral bracing is required on the member of the truss.

BEARING

Indicates location where bearings (supports) occur.

PLATE LOCATION & ORIENTATION

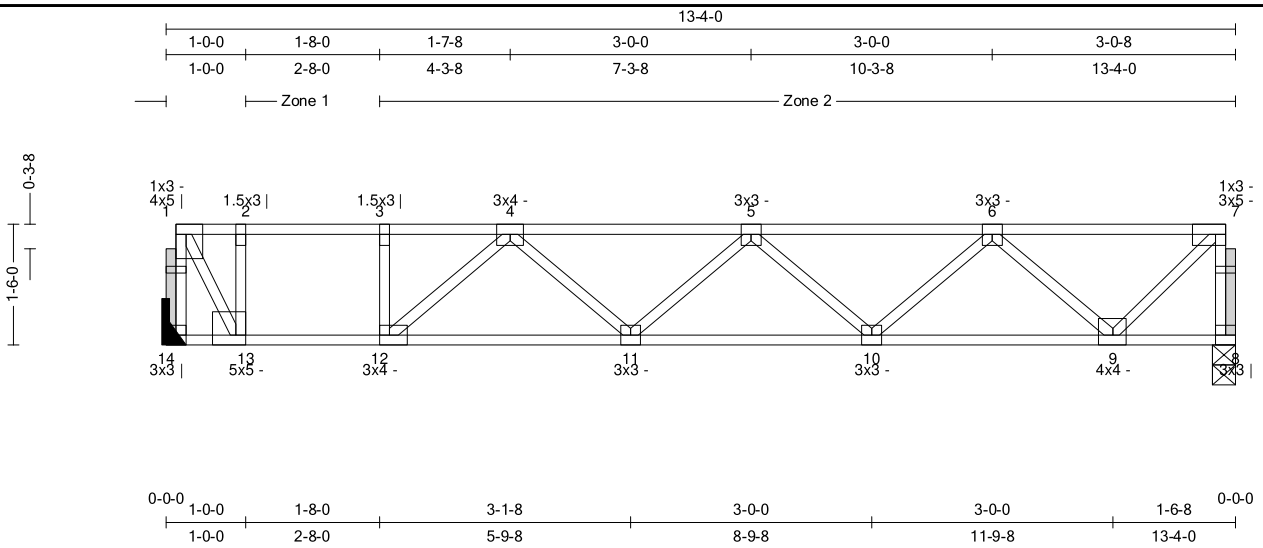
The plate shall be centered on joint and/or placed in accordance with the design drawing/QC full scale details.

REFERENCES

- **ANSI / TPI 1:** National Design Standard for Metal Plate Connected Wood Trusses
- **BCSI:** Building Component & Safety Information - Guide to Good Practice for Handling, Installing, Restraining, & Bracing of Metal Plate Connected Wood Trusses.
- **NDS:** National Design Specification for Wood Construction
- **ESR:** 1082 published by the International Code Council. www.icc-es.org

	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss:F05 Job: QU03648_RESERVE BUILDING J_RE Date: 03/11/26 07:25:37 Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	PLY(S)	SPACING	WGT/PLY
13-4-0	0/12	5	0-0-0	0-0-0	1	19.19 in	70 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL :	40	Bldg Code :	IBC 2018/	TC :	0.64 (2-3)	Vert TL:	0.39 in	L / 395	(11-12)	L / 240
TCDL :	10		TPI 1-2014	BC :	0.77 (12-13)	Vert LL:	0.21 in	L / 718	(11-12)	L / 360
BCLL :	0	Rep Mbr :	Yes	Web :	0.28 (1-13)	Horz TL:	0.02 in		8	
BCDL :	10	Lumber D.O.L. :	100 %							

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
14	1	3.5 in	---	746 lbs	·	·	·	·	·
8	1	3.5 in	1.50 in	746 lbs	·	·	·	·	·

03/11/2026

STATE OF MISSOURI
ARTURO A. HERNANDEZ
NUMBER PE-2006000095
PROFESSIONAL ENGINEER

Material

TC: SYP 2400/1.8 4 x 2
BC: SYP 2400/1.8 4 x 2
Web: SYP #1 4 x 2

Loads

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.630		(-800 lbs)	3-4	0.511		(-800 lbs)	5-6	0.186		(-1,528 lbs)					
	2-3	0.636		(-800 lbs)	4-5	0.211		(-1,731 lbs)	6-7	0.179		(-627 lbs)					
BC	9-10	0.153	1,225 lbs		11-12	0.555	1,501 lbs										
	10-11	0.317	1,752 lbs		12-13	0.774	800 lbs										
Web	1-14	0.116		(-1,090 lbs)	3-12	0.062	376 lbs		5-10	0.039		(-304 lbs)	7-9	0.148	893 lbs		
	1-13	0.277	1,672 lbs		4-12	0.113		(-933 lbs)	6-10	0.068	411 lbs		7-8	0.078		(-733 lbs)	
	2-13	0.088		(-823 lbs)	4-11	0.052	313 lbs		6-9	0.099		(-812 lbs)					

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
4) Hanger is for graphical interpretation only. Install hanger per manufacturer's recommendation.
5) A creep factor of 2.00 has been applied for this truss analysis.
6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
7) ▭ Indicates non-structural members.

TrueBuild®Truss Software v5.9.7
Eagle Metal Products

EAGLE METAL

RELEASE FOR CONSTRUCTION
AS NOTED ON PLANS REVIEW
DEVELOPMENT SERVICES
LEE'S SUMMIT, MISSOURI
03/16/2026 2:13:45

The truss designs referenced below have been prepared by me or under my direct supervision based on the truss design criteria and requirements ("design criteria") provided by **Quality Line Truss**.

These truss designs are intended for the fabrication of individual building components that will perform to the design criteria provided. Any variance from the design criteria will render the affected truss designs inapplicable.

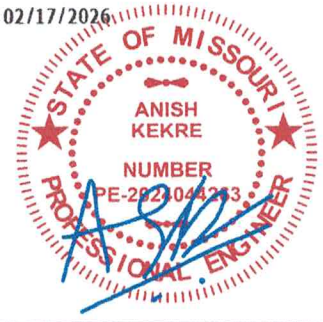
Listed below are the truss designs included in this package and covered by this seal.

Job: **QU03648_RESERVE BUILDING J_REVISED_02042026** - 1258287
F01, F02, F03, F04, F05, F06, F07, F08, F09, F10, FG02, FG03, FG04, FL02, GE01, GE02, GE03, SGE01, SGE02, T01, T02, T03, T04, T06

Any location identification is for file reference only. No determination of the appropriateness of design criteria for any specific project has been made in preparing the truss designs.

Please refer to individual truss designs for specific design criteria.

02/17/2026



Anish Kekre (MO, 2024044263)
My license expiration date for the state of MO is 12/31/2026.

IMPORTANT NOTE: The responsibility of the engineer sealing this package, as a Truss Engineer, is solely for design of individual trusses as individual building components based upon design criteria provided by others and set forth in the referenced truss drawings. The truss design criteria for the components have not been verified as appropriate for any particular building, project or use. Adequacy and suitability of design criteria and requirements for the truss designs for any specific project are the responsibility of the building designer, not the Truss Engineer, per ANSI/TPI-1, Chapter 2.

DESIGN NOTES

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2. Copies of each Truss Design Drawing shall be furnished to the installation contractor, Building Designer, Owner and all persons fabricating, handling, installing, bracing, or erecting the trusses.

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5. Each Truss Design Drawing is for the individual building component (a truss). A seal on the Truss Design Drawing indicates acceptance of professional engineering responsibility solely for the individual truss.
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HANDLING, INSTALLING, & BRACING

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8. Bracing shown on each Truss Design Drawing is for lateral support of individual truss components only to reduce buckling lengths. All temporary and permanent bracing, including lateral load and diagonal or cross bracing, are the responsibility, respectively, of the erector and Building Designer.
9. Eagle is not responsible for improper truss fabrication, handling, erection or bracing.
10. Compression chords shall be laterally braced by the roof or floor sheathing, directly attached, or have purlins provided at spacing shown, unless noted otherwise.
11. Bottom chord required bracing shall be at 10ft spacing or less, if no structural rated ceiling is installed, unless noted otherwise.
12. Strongbacking shall be installed on all parallel chord trusses, including flooring systems, to limit deflection and reduce vibration. Refer to BCSI-B7.
13. Never exceed the design loading shown. Never stack building or other materials on inadequately braced truss; refer to BCSI.
14. Concentration of construction loads greater than the design loads shall not be applied to the trusses at any time; refer to BCSI.
15. Trusses shall be handled with care prior to erection to avoid damage. Refer to BCSI for recommended truss handling and erection.

MATERIALS & FABRICATION

16. Lumber moisture content shall be 19% or less at the time of fabrication unless noted otherwise.
17. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
18. Unless expressly noted, the truss designs are not applicable for use with fire retardant or preservative treated lumber.
19. Plates shall be applied on both faces of truss at each joint and embedded fully. Knots and wane at joint locations shall be regulated in accordance with ANSI / TPI 1.
20. For a specified plate gauge and grade, the specified size is a minimum.
21. Connections not shown are the responsibility of others.
22. Adequate support shall be provided to resist gravity, lateral and uplift loads.
23. For 4X2 truss orientation, locate plates 0 - 1/16" from outside the edge of the truss.
24. Fabrication of truss shall be in accordance with ANSI / TPI 1.

OTHER NOTES

25. Camber is a non-structural consideration and is the responsibility of truss fabricator.
26. Do not cut or alter any truss member or plate without prior approval from a professional engineer.
27. Lumber design values are in accordance with ANSI / TPI 1; lumber design values are by others.
28. Install specified hangers per manufacturer recommendations.

SYMBOLS

PLATE SIZE

3X4 - The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

-, /, I, Indicates required direction of slots; Reference "Joint Details" for more information.

20 Ga Gr40 connectors required
3X10-20HS - 20 Ga Gr60 connectors required
8X10-18HS - 18 Ga Gr60 connectors required

LATERAL BRACING

When this symbol shown, continuous lateral bracing is required on the member of the truss.

BEARING

Indicates location where bearings (supports) occur.

PLATE LOCATION & ORIENTATION

The plate shall be centered on joint and/or placed in accordance with the design drawing/QC full scale details.

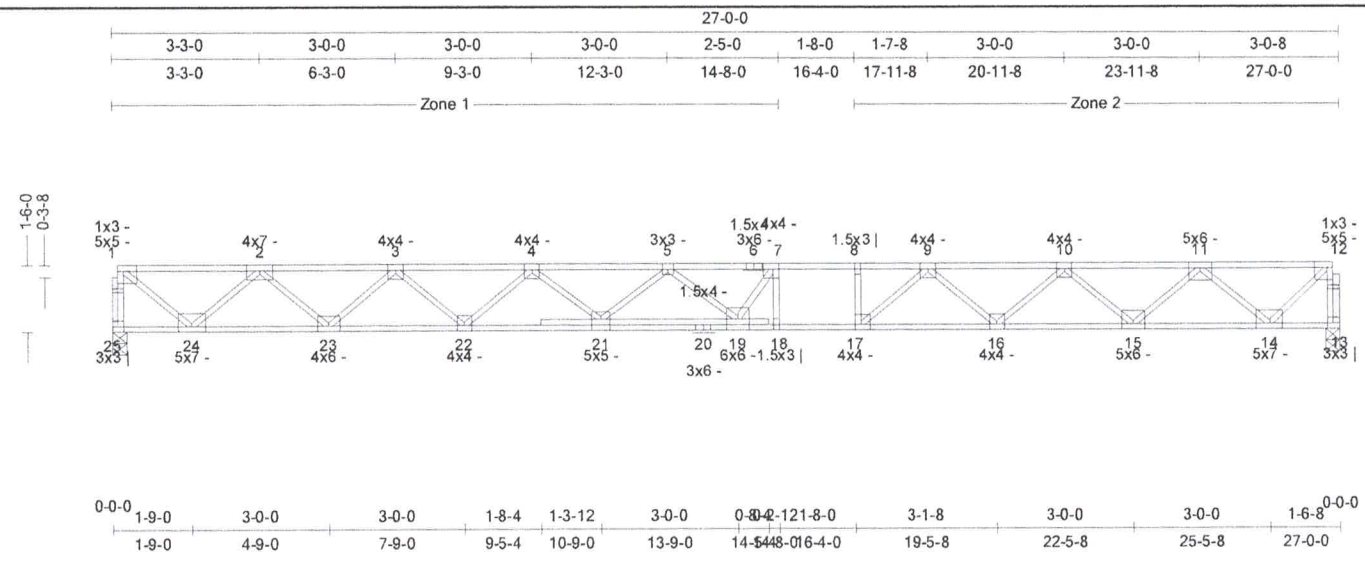
REFERENCES

- **ANSI / TPI 1:** National Design Standard for Metal Plate Connected Wood Trusses
- **BCSI:** Building Component & Safety Information - Guide to Good Practice for Handling, Installing, Restraining, & Bracing of Metal Plate Connected Wood Trusses.
- **NDS:** National Design Specification for Wood Construction
- **ESR:** 1082 published by the International Code Council. www.icc-es.org

1258287 00026033

	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss:F01 Job: QU03648_RESERVE BUILDING J_REV Date: 02/17/26 15:40:19 Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	PLY(S)	SPACING	WGT/PLY
27-0-0	0/12	8	0-0-0	0-0-0	1	19.19 in	146 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
TCLL: 40	Bldg Code:	IBC 2018/	TC:	0.70 (5-7)	Vert TL:	1.1 in	L/288	(18-19)	L/240
TCDL: 10		TPI 1-2014	BC:	0.98 (18-19)	Vert LL:	0.63 in	L/501	(18-19)	L/360
BCLL: 0	Rep Mbr:	Yes	Web:	0.35 (1-24)	Horz TL:	0.15 in		13	
BCDL: 10	Lumber D.O.L.: 100 %								

02/17/2026

STATE OF MISSOURI
ANISH KEKRE
NUMBER PE-2024041263
PROFESSIONAL ENGINEER

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
25	1	4 in	1.50 in	1,511 lbs	.	.	.	.	.
13	1	3.5 in	1.50 in	1,511 lbs	.	.	.	.	.

Material

TC: SYP 2400/1.8 4 x 2
BC: SYP 2400/1.8 4 x 2
Web: SYP #1 4 x 2

Loads

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.224		(-1,572 lbs)	4-5	0.644		(-7,252 lbs)	8-9	0.555		(-7,057 lbs)	11-12	0.187		(-1,372 lbs)
	2-3	0.365		(-4,113 lbs)	5-7	0.705		(-7,536 lbs)	9-10	0.511		(-5,760 lbs)				
	3-4	0.522		(-5,884 lbs)	7-8	0.562		(-7,057 lbs)	10-11	0.352		(-3,967 lbs)				
BC	14-15	0.280	2,812 lbs		17-18	0.979	7,057 lbs		21-22	0.601	6,685 lbs					
	15-16	0.470	5,002 lbs		18-19	0.981	7,088 lbs		22-23	0.454	5,127 lbs					
	16-17	0.738	6,459 lbs		19-21	0.334	7,555 lbs		23-24	0.289	2,993 lbs					
Web	1-25	0.159		(-1,488 lbs)	4-22	0.131		(-1,084 lbs)	8-17	0.043		(-352 lbs)	11-14	0.237		(-1,952 lbs)
	1-24	0.347	2,094 lbs		4-21	0.125	756 lbs		9-17	0.177	927 lbs		12-14	0.324	1,956 lbs	
	2-24	0.234		(-1,928 lbs)	5-21	0.050		(-404 lbs)	9-16	0.115		(-947 lbs)	12-13	0.159		(-1,492 lbs)
	2-23	0.252	1,519 lbs		5-19	0.060	313 lbs		10-16	0.170	1,029 lbs					
	3-23	0.167		(-1,375 lbs)	7-19	0.179	940 lbs		10-15	0.170		(-1,404 lbs)				
	3-22	0.170	1,027 lbs		7-18	0.088		(-741 lbs)	11-15	0.260	1,567 lbs					

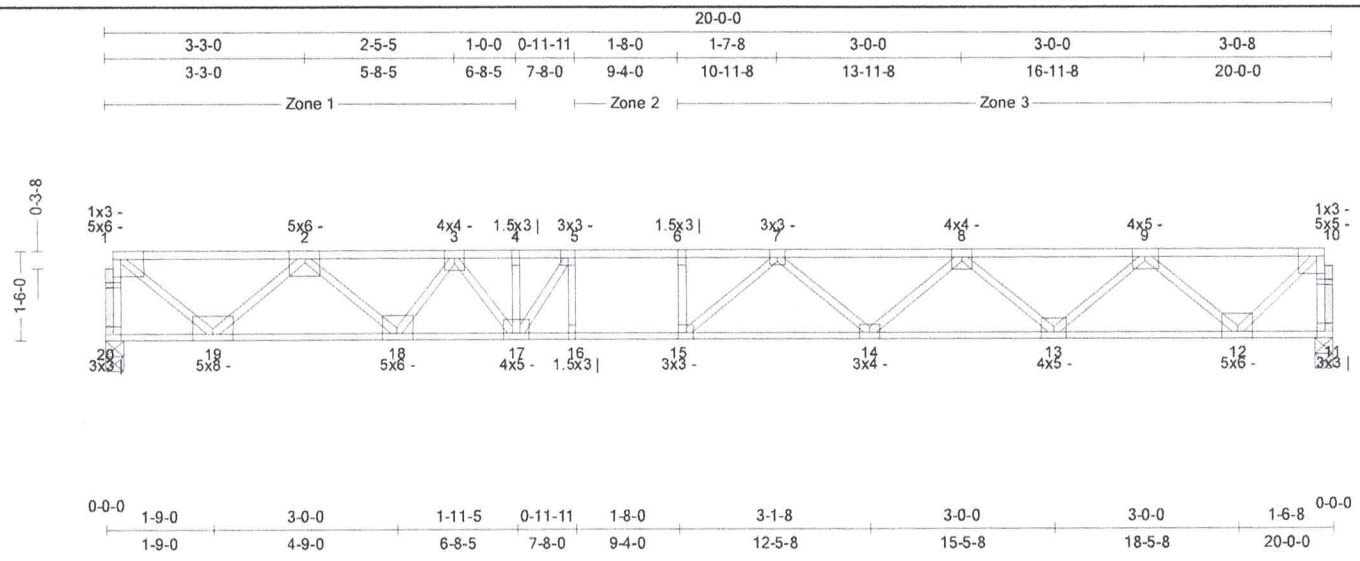
Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Unless otherwise specified by the Building Designer, two strongbacks recommended at one third points of the truss span. Strongback spacing or strongback to support should not exceed 10'-0".
3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
4) A creep factor of 2.00 has been applied for this truss analysis.
5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
6) ☐ Indicates non-structural members.

TrueBuild® Truss Software v5.8.14
Eagle Metal Products

	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss:F02 Job: QU03648_RESERVE BUILDING J_REV Date: 02/17/26 15:40:21 Page: 1 of 2

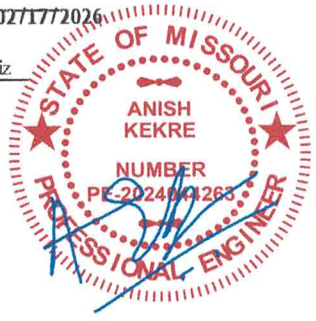
SPAN	PITCH	QTY	OHL	OHR	PLY(S)	SPACING	WGT/PLY
20-0-0	0/12	2	0-0-0	0-0-0	1	19.19 in	107 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
TCLL: 40	Bldg Code:	IBC 2018/	TC:	0.49 (5-6)	Vert TL:	0.43 in	L/540	(14-15)	L/240
TCDL: 10		TPI 1-2014	BC:	0.61 (15-16)	Vert LL:	0.24 in	L/972	15	L/360
BCLL: 0	Rep Mbr:	No	Web:	0.36 (1-19)	Horz TL:	0.09 in		11	
BCDL: 10	Lumber D.O.L.: 100 %								

02/17/2026



Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
20	1	3.5 in	1.50 in	1,520 lbs					
11	1	3.5 in	1.50 in	1,318 lbs					

Material

TC: SYP 2400/1.8 4 x 2
BC: SYP 2400/1.8 4 x 2
Web: SYP #1 4 x 2

Loads

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case L1: Std Live Load

Point Loads

Member	Location	Direction	Load	Trib Width
Top	6-8-5	Down	300 lbs	

Load Case D1: Std Dead Load

Point Loads

Member	Location	Direction	Load	Trib Width
Top	6-8-5	Down	300 lbs	

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.266		(-1,581 lbs)	4-5	0.476		(-5,386 lbs)	7-8	0.463		(-4,745 lbs)			
	2-3	0.389		(-4,144 lbs)	5-6	0.492		(-5,468 lbs)	8-9	0.327		(-3,351 lbs)			
	3-4	0.477		(-5,386 lbs)	6-7	0.488		(-5,468 lbs)	9-10	0.215		(-1,184 lbs)			
BC	12-13	0.266	2,411 lbs		15-16	0.606	5,468 lbs		18-19	0.326	3,009 lbs				
	13-14	0.419	4,182 lbs		16-17	0.606	5,468 lbs								
	14-15	0.522	5,205 lbs		17-18	0.466	4,795 lbs								
Web	1-20	0.164		(-1,499 lbs)	3-17	0.177	1,002 lbs		8-14	0.127	764 lbs		10-11	0.140	(-1,300 lbs)
	1-19	0.358	2,106 lbs		4-17	0.080		(-659 lbs)	8-13	0.139		(-1,128 lbs)			
	2-19	0.240		(-1,937 lbs)	5-17	0.066		(-527 lbs)	9-13	0.211	1,274 lbs				
	2-18	0.263	1,540 lbs		7-15	0.093	487 lbs		9-12	0.202		(-1,664 lbs)			
	3-18	0.136		(-1,154 lbs)	7-14	0.082		(-624 lbs)	10-12	0.281	1,688 lbs				

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).

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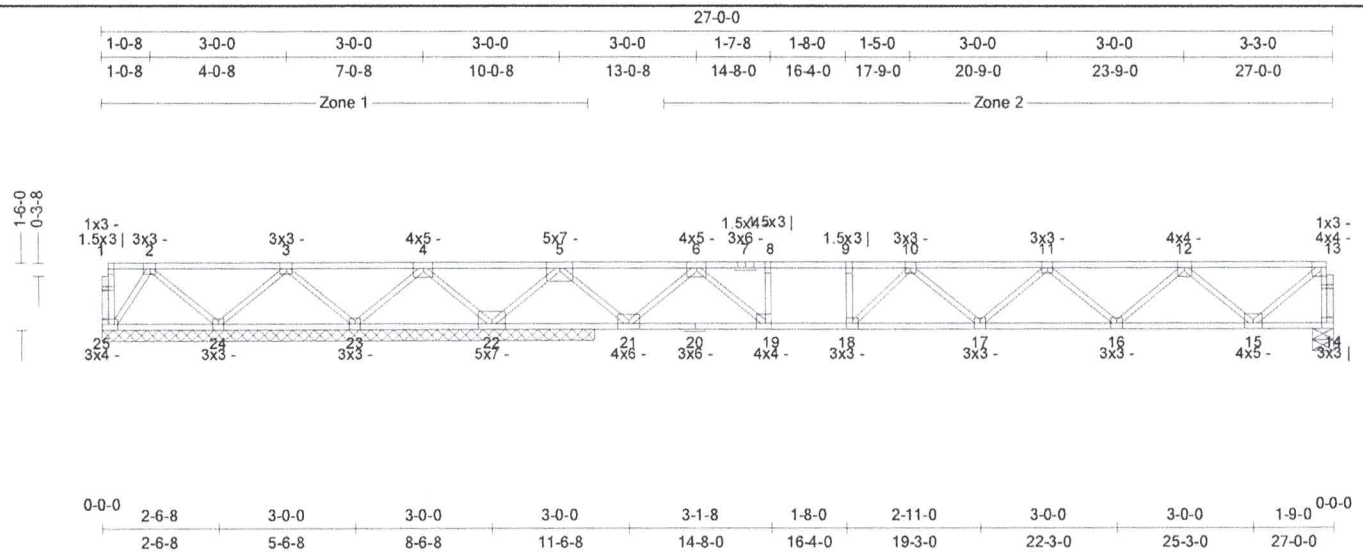
Eagle Metal Products

SPAN	PITCH	QTY	OHL	OHR	PLY(S)	SPACING	WGT/PLY
20-0-0	0/12	2	0-0-0	0-0-0	1	19.19 in	107 lbs

4) A creep factor of 2.00 has been applied for this truss analysis.
5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
6) ☐ Indicates non-structural members.

	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss:F03 Job: QU03648_RESERVE BUILDING J_REV Date: 02/17/26 15:40:22 Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	PLY(S)	SPACING	WGT/PLY
27-0-0	0/12	1	0-0-0	0-0-0	1	19.19 in	136 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
TCLL: 40	Bldg Code:	IBC 2018/	TC:	0.68 (4-5)	Vert TL:	0.38 in	L/497	(17-18)	L/240
TCDL: 10		TPI 1-2014	BC:	0.61 (17-18)	Vert LL:	0.21 in	L/889	(17-18)	L/360
BCLL: 0	Rep Mbr:	No	Web:	0.27 (5-22)	Horz TL:	0.04 in		14	
BCDL: 10	Lumber D.O.L.: 100 %								

02/17/2026

STATE OF MISSOURI
ANISH KEKRE
NUMBER PE-2024044253
PROFESSIONAL ENGINEER

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
14	1	5.5 in	1.50 in	939 lbs	·	·	·	·	·
22	1	129.5 in	N/A	2,535 lbs	·	·	·	·	1,170 lbs
23	1	129.5 in	N/A	178 lbs	·	·	·	·	-1,009 lbs
24	1	129.5 in	N/A	370 lbs	·	·	·	·	-256 lbs
25	1	129.5 in	N/A	164 lbs	·	·	·	·	110 lbs

Material

TC: SYP 2400/1.8 4 x 2
BC: SYP 2400/1.8 4 x 2
Web: SYP #1 4 x 2

Loads

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	13-9-12	Down	Proj	80 plf	80 plf	

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	3-4	0.507	686 lbs		6-8	0.534		(-2,368 lbs)	10-11	0.271	(-2,773 lbs)
	4-5	0.680	2,169 lbs		8-9	0.510		(-2,368 lbs)	11-12	0.253	(-2,214 lbs)
	5-6	0.387		(-533 lbs)	9-10	0.487		(-2,368 lbs)	12-13	0.242	(-926 lbs)
BC	15-16	0.211	1,722 lbs		18-19	0.609	2,368 lbs				
	16-17	0.377	2,617 lbs		19-21	0.540	1,562 lbs				
	17-18	0.614	2,750 lbs		21-22	0.099		(-533 lbs)			
Web	2-24	0.048		(-375 lbs)	6-21	0.170		(-1,396 lbs)	12-15	0.131	(-1,079 lbs)
	3-23	0.091		(-712 lbs)	6-19	0.178	1,074 lbs		13-15	0.204	1,234 lbs
	4-23	0.109	656 lbs		8-19	0.048		(-444 lbs)	13-14	0.098	(-919 lbs)
	4-22	0.169		(-1,355 lbs)	10-18	0.070		(-545 lbs)			
	5-22	0.272		(-2,219 lbs)	11-16	0.067		(-547 lbs)			
	5-21	0.239	1,446 lbs		12-16	0.110	667 lbs				

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).

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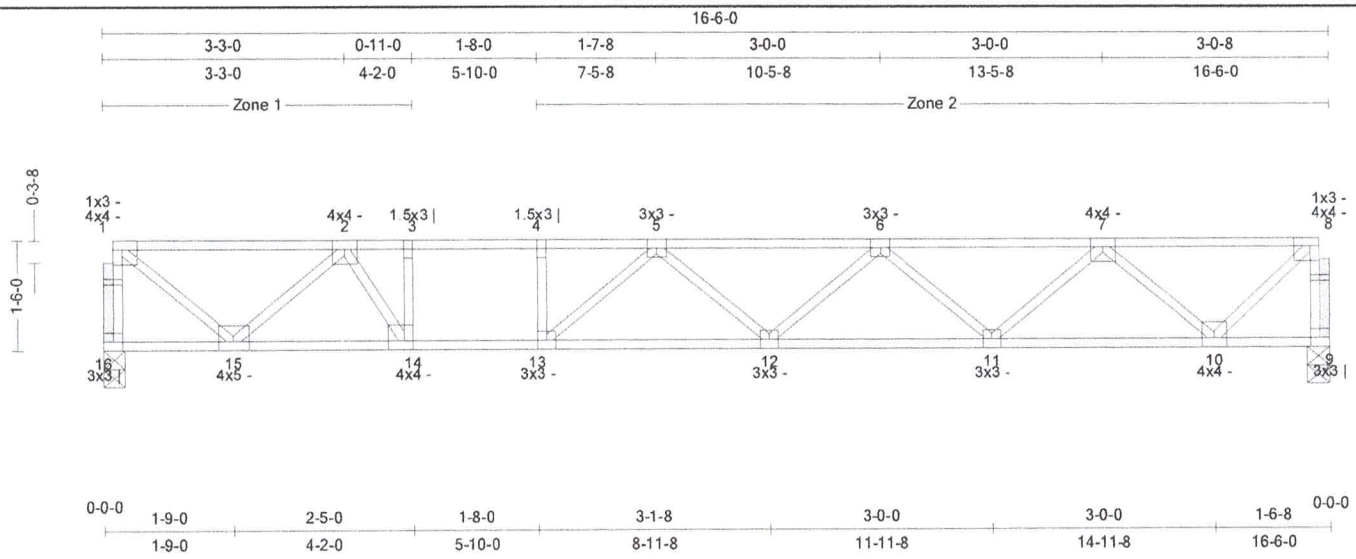
Eagle Metal Products

SPAN	PITCH	QTY	OHL	OHR	PLY(S)	SPACING	WGT/PLY
27-0-0	0/12	1	0-0-0	0-0-0	1	19.19 in	136 lbs

4) A creep factor of 2.00 has been applied for this truss analysis.
5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
6) ☐ Indicates non-structural members.

	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss:F04 Job: QU03648_RESERVE BUILDING J_REV Date: 02/17/26 15:40:24 Page: 1 of 1

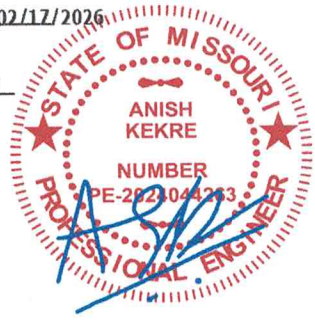
SPAN	PITCH	QTY	OHL	OHR	PLY(S)	SPACING	WGT/PLY
16-6-0	0/12	4	0-0-0	0-0-0	1	19.19 in	85 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
TCLL: 40	Bldg Code:	IBC 2018/ TPI 1-2014	TC:	0.86 (3-4)	Vert TL:	0.4 in	L/473	(12-13)	L/240
TCDL: 10			BC:	0.95 (13-14)	Vert LL:	0.23 in	L/848	(12-13)	L/360
BCLL: 0	Rep Mbr:	Yes	Web:	0.19 (1-15)	Horz TL:	0.04 in		9	
BCDL: 10	Lumber D.O.L.:	100 %							

02/17/2026



Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
16	1	3.5 in	1.50 in	923 lbs	.	.	.	.	.
9	1	3.5 in	1.50 in	923 lbs	.	.	.	.	.

Material

TC: SYP#1 4 x 2
BC: SYP#1 4 x 2
Web: SYP#1 4 x 2

Loads

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.383		(-864 lbs)	3-4	0.855		(-2,236 lbs)	5-6	0.353		(-2,670 lbs)	7-8	0.294	(-800 lbs)
	2-3	0.831		(-2,236 lbs)	4-5	0.687		(-2,236 lbs)	6-7	0.326		(-2,093 lbs)			
BC	10-11	0.328	1,593 lbs		12-13	0.887	2,654 lbs		14-15	0.874	1,728 lbs				
	11-12	0.599	2,506 lbs		13-14	0.951	2,236 lbs								
Web	1-16	0.093		(-873 lbs)	3-14	0.067		(-548 lbs)	7-10	0.131		(-1,076 lbs)			
	1-15	0.191	1,151 lbs		5-13	0.076		(-559 lbs)	8-10	0.189	1,140 lbs				
	2-15	0.142		(-1,172 lbs)	6-11	0.068		(-560 lbs)	8-9	0.097		(-909 lbs)			
	2-14	0.176	964 lbs		7-11	0.112	678 lbs								

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
4) A creep factor of 2.00 has been applied for this truss analysis.
5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
6) ☐ Indicates non-structural members.

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Eagle Metal Products

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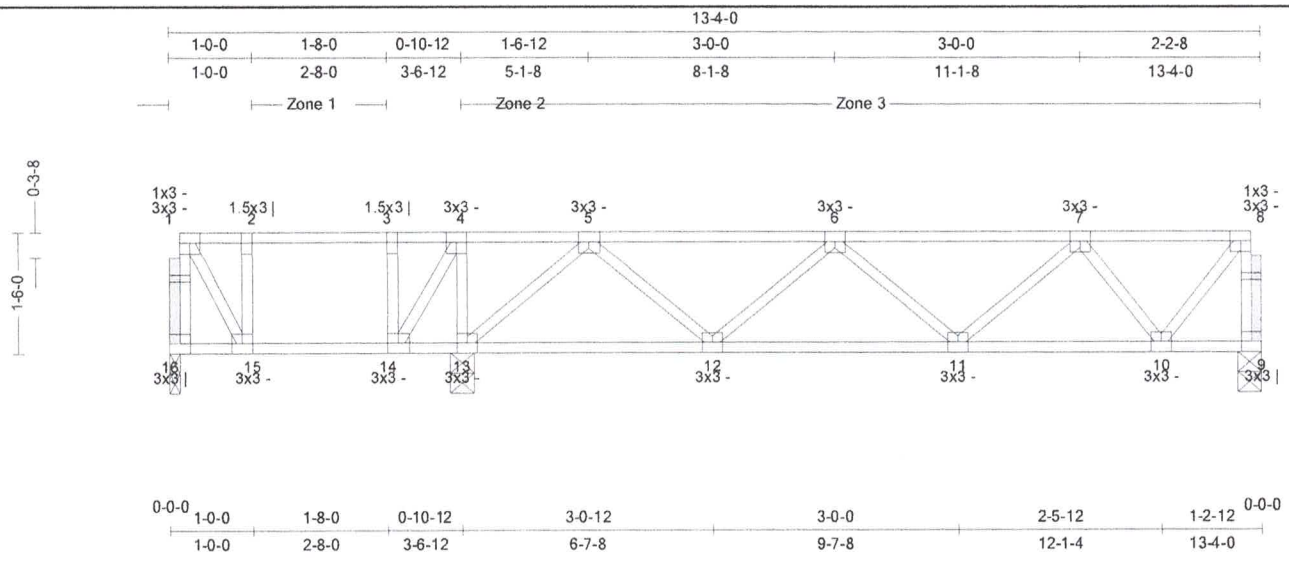
SPAN	PITCH	QTY	OHL	OHR	PLY(S)	SPACING	WGT/PLY
13-4-0	0/12	5	0-0-0	0-0-0	1	19.19 in	72 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
TCLL: 40	Bldg Code:	IBC 2018/	TC:	0.26 (6-7)	Vert TL:	0.03 in	L/999	(11-12)	L/240
TCDL: 10		TPI 1-2014	BC:	0.24 (11-12)	Vert LL:	0.01 in	L/999	(11-12)	L/360
BCLL: 0	Rep Mbr:	Yes	Web:	0.10 (8-10)	Horz TL:	0.01 in		9	
BCDL: 10	Lumber D.O.L.: 100 %								

02/17/2026

STATE OF MISSOURI
ANISH KEKRE
NUMBER PE-2024044263
PROFESSIONAL ENGINEER

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
13	1	3.5 in	1.50 in	784 lbs	·	·	·	·	·
16	1	1.5 in	1.50 in	166 lbs	·	·	·	·	·
9	1	3.5 in	1.50 in	542 lbs	·	·	·	·	·

Material

TC: SYP#1 4 x 2
BC: SYP#1 4 x 2
Web: SYP#1 4 x 2

Loads

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	5-6	0.256		(-753 lbs)	7-8	0.188		(-352 lbs)
	6-7	0.259		(-836 lbs)				
BC	10-11	0.177	640 lbs		12-13	0.171	463 lbs	
	11-12	0.242	935 lbs					
Web	5-13	0.087		(-714 lbs)	8-9	0.056		(-527 lbs)
	5-12	0.065	394 lbs					
	7-10	0.057		(-496 lbs)				
	8-10	0.096	583 lbs					

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
4) A creep factor of 2.00 has been applied for this truss analysis.
5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
6) ▭ Indicates non-structural members.

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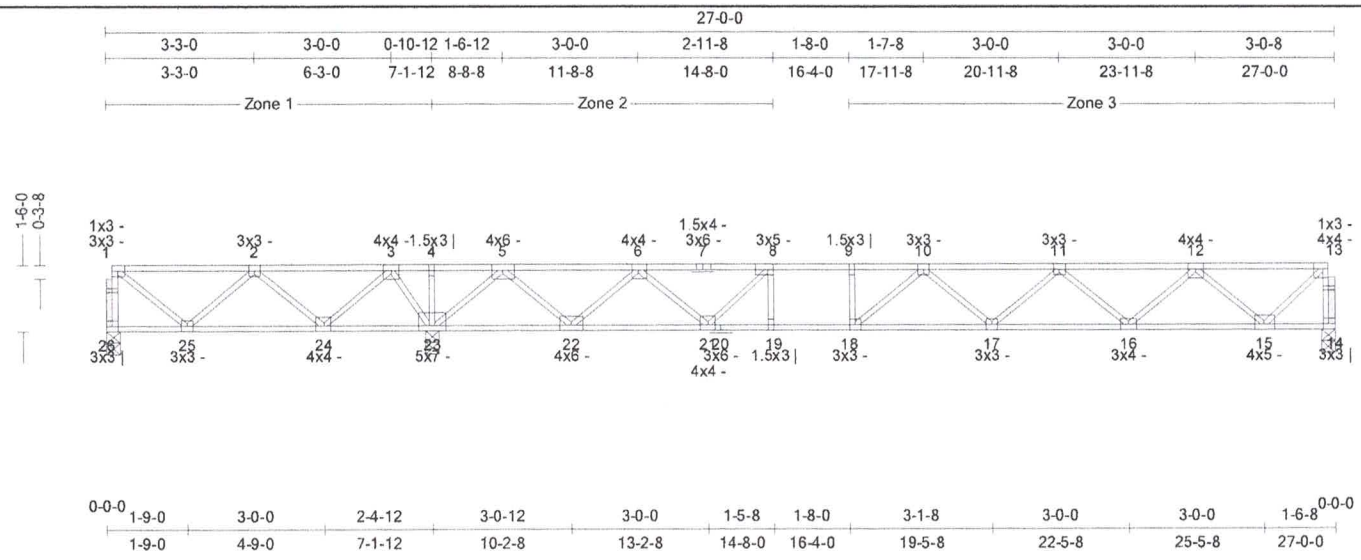
	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss:F06 Job: QU03648_RESERVE BUILDING J_REV Date: 02/17/26 15:40:26 Page: 1 of 2

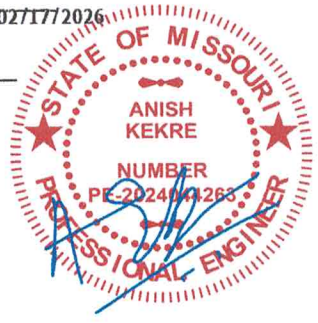
SPAN	PITCH	QTY	OHL	OHR	PLY(S)	SPACING	WGT/PLY
27-0-0	0/12	1	0-0-0	0-0-0	2	19.19 in	137 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
TCLL: 40	Bldg Code:	IBC 2018/	TC:	0.48 (8-9)	Vert TL:	0.39 in	L/604	(17-18)	L/240
TCDL: 10		TPI 1-2014	BC:	0.75 (18-19)	Vert LL:	0.11 in	L/999	(17-18)	L/360
BCLL: 0	Rep Mbr:	No	Web:	0.24 (5-22)	Horz TL:	0.03 in		14	
BCDL: 10	Lumber D.O.L.:	100 %							

02/17/2026



Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
23	1	3.5 in	1.50 in	4,135 lbs	·	·	·	·	·
26	1	3.5 in	1.50 in	179 lbs	-274 lbs	·	·	-274 lbs	·
14	1	3.5 in	1.50 in	1,915 lbs	·	·	·	·	·

Material

TC: SYP 2400/1.8 4 x 2
BC: SYP 2400/1.8 4 x 2
Web: SYP #1 4 x 2

Loads

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	27-0-0	Down	Proj	110 plf	110 plf	

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	2-3	0.431	1,027 lbs		6-8	0.469		(-1,848 lbs)	10-11	0.292	(-2,871 lbs)
	3-4	0.369	2,144 lbs		8-9	0.480		(-2,587 lbs)	11-12	0.278	(-2,206 lbs)
	4-5	0.418	2,144 lbs		9-10	0.460		(-2,587 lbs)	12-13	0.247	(-823 lbs)
BC	15-16	0.191	1,687 lbs		18-19	0.745	2,587 lbs		22-23	0.082	(-796 lbs)
	16-17	0.348	2,689 lbs		19-21	0.745	2,587 lbs		23-24	0.162	(-1,625 lbs)
	17-18	0.662	2,934 lbs		21-22	0.181	1,160 lbs		24-25	0.057	(-517 lbs)
Web	1-25	0.045		(-335 lbs)	5-22	0.240	1,371 lbs		12-16	0.122	704 lbs
	2-25	0.069	360 lbs		6-22	0.163		(-1,282 lbs)	12-15	0.149	(-1,172 lbs)
	2-24	0.103		(-800 lbs)	6-21	0.162	934 lbs		13-15	0.206	1,173 lbs
	3-24	0.151	860 lbs		8-21	0.128		(-1,037 lbs)	13-14	0.108	(-950 lbs)
	3-23	0.112		(-949 lbs)	10-18	0.066		(-489 lbs)			
	5-23	0.227		(-1,796 lbs)	11-16	0.084		(-656 lbs)			

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
4) A creep factor of 2.00 has been applied for this truss analysis.
5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
6) ☐ Indicates non-structural members.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes.
This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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Eagle Metal Products

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	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss:F06 Job: QU03648_RESERVE BUILDING J_REV Date: 02/17/26 15:40:27 Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	PLY(S)	SPACING	WGT/PLY
27-0-0	0/12	1	0-0-0	0-0-0	2	19.19 in	137 lbs

7) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 26 may need to be considered.
8) The forces shown for this multi-ply truss are per ply and the reactions are for all plies. Two identical trusses shall be built and attached as follows: SDS Simpson 0.250"x6" Screws TC - 1 row @ 2-0-0 oc, BC - 1 row @ 2-0-0 oc , Webs - 1 @ 2-0-0 oc, minimum one fastener per web.
9) Screws shall be installed in the same truss ply that the hangers are attached to. If both plies are loaded, the screws shall be divided between the two plies, with the spacing on each side twice the minimum indicated.
10) Strongbacks shall be attached to each ply.
11) Center screw vertically on the 1-1/2" dimension of chords and webs. If splitting occurs, it may be necessary to pre-drill the holes in accordance with the NDS.
12) Install screws per manufacturer recommendations.

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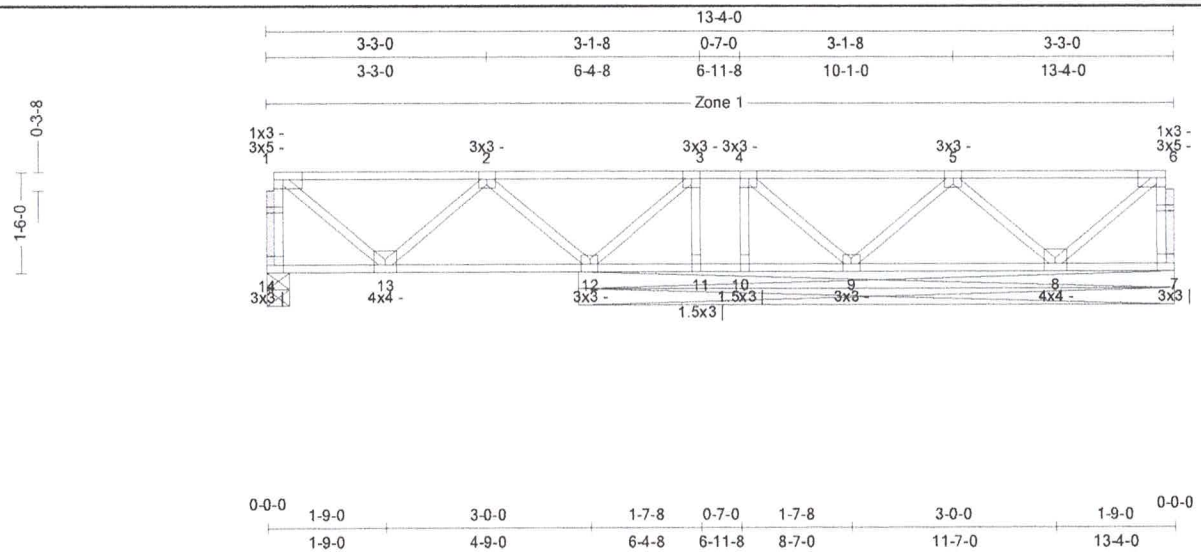
	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss:F07 Job: QU03648_RESERVE BUILDING J_REV Date: 02/17/26 15:40:28 Page: 1 of 1

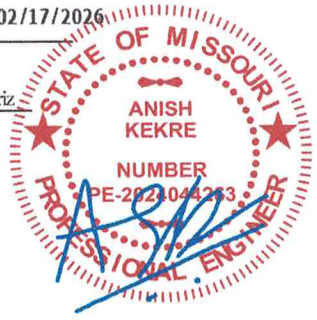
SPAN	PITCH	QTY	OHL	OHR	PLY(S)	SPACING	WGT/PLY
13-4-0	0/12	1	0-0-0	0-0-0	1	19.19 in	70 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	**General**		**CSI**		**Deflection**		**L/**	**(loc)**	**Allowed**
TCLL: 40	Bldg Code:	IBC 2018/	TC:	0.41 (4-5)	Vert TL:	0.07 in	L/734	(12-13)	L/240
TCDL: 10		TPI 1-2014	BC:	0.39 (10-11)	Vert LL:	0.04 in	L/999	12	L/360
BCLL: 0	Rep Mbr:	No	Web:	0.16 (1-13)	Horz TL:	0.02 in		7	
BCDL: 10	Lumber D.O.L.:	100 %							

02/17/2026



Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
14	1	4 in	1.50 in	746 lbs	.	.	.	.	.
7	1	105 in	1.50 in	746 lbs	.	.	.	.	.

Material

TC: SYP#1 4 x 2
BC: SYP#1 4 x 2
Web: SYP#1 4 x 2

Loads

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.405		(-708 lbs)	3-4	0.211		(-1,750 lbs)	5-6	0.405	(-708 lbs)
	2-3	0.413		(-1,566 lbs)	4-5	0.413		(-1,566 lbs)			
BC	8-9	0.305	1,296 lbs		10-11	0.389	1,750 lbs		12-13	0.305	1,296 lbs
	9-10	0.389	1,750 lbs		11-12	0.389	1,750 lbs				
Web	1-14	0.077		(-726 lbs)	2-12	0.061	367 lbs		6-8	0.156	943 lbs
	1-13	0.156	943 lbs		5-9	0.061	367 lbs		6-7	0.077	(-726 lbs)
	2-13	0.097		(-797 lbs)	5-8	0.097		(-797 lbs)			

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
4) A creep factor of 2.00 has been applied for this truss analysis.
5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
6) ☐ Indicates non-structural members.

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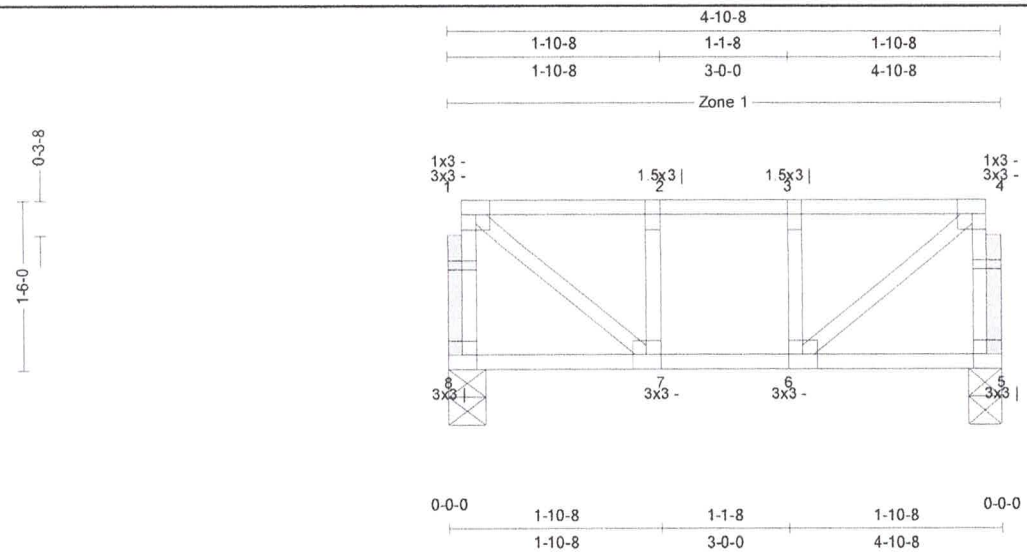
Eagle Metal Products

	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss:F08 Job: QU03648_RESERVE BUILDING J_REV Date: 02/17/26 15:40:29 Page: 1 of 1

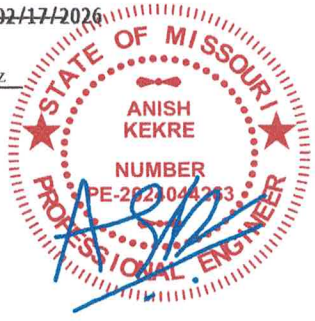
SPAN	PITCH	QTY	OHL	OHR	PLY(S)	SPACING	WGT/PLY
4-10-8	0/12	4	0-0-0	0-0-0	1	19.19 in	30 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
TCLL: 40	Bldg Code:	IBC 2018/	TC:	0.10 (1-2)	Vert TL:	0.01 in	L/999	(5-6)	L/240
TCDL: 10		TPI 1-2014	BC:	0.09 (6-7)	Vert LL:	0.01 in	L/999	(5-6)	L/360
BCLL: 0	Rep Mbr:	Yes	Web:	0.05 (1-7)	Horz TL:	0 in		5	
BCDL: 10	Lumber D.O.L.:	100 %							

02/17/2026



Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
8	1	4 in	1.50 in	273 lbs	.	.	.	.	.
5	1	3.5 in	1.50 in	273 lbs	.	.	.	.	.

Material

TC: SYP #1 4 x 2
BC: SYP #1 4 x 2
Web: SYP #1 4 x 2

Loads

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	
BC	
Web	

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
3) A creep factor of 2.00 has been applied for this truss analysis.
4) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
5) ☐ Indicates non-structural members.

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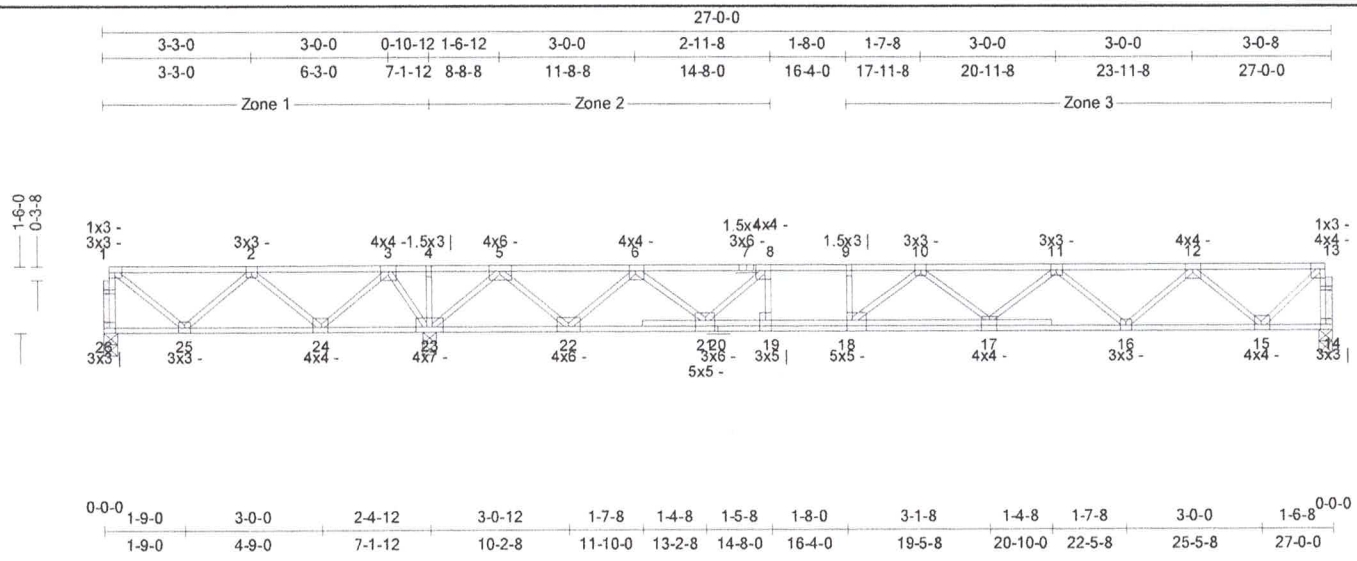
Eagle Metal Products

	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss:F09 Job: QU03648_RESERVE BUILDING J_REV Date: 02/17/26 15:40:30 Page: 1 of 1

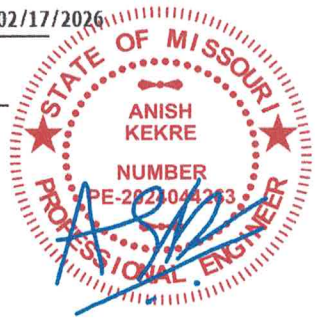
SPAN	PITCH	QTY	OHL	OHR	PLY(S)	SPACING	WGT/PLY
27-0-0	0/12	2	0-0-0	0-0-0	1	19.19 in	150 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
TCLL: 40	Bldg Code:	IBC 2018/	TC:	0.68 (4-5)	Vert TL:	0.29 in	L/803	(17-18)	L/240
TCDL: 10		TPI 1-2014	BC:	0.71 (19-21)	Vert LL:	0.16 in	L/999	(17-18)	L/360
BCLL: 0	Rep Mbr:	No	Web:	0.24 (5-22)	Horz TL:	0.02 in		14	
BCDL: 10	Lumber D.O.L.: 100 %								

02/17/2026



Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
23	1	3.5 in	1.50 in	2,158 lbs	·	·	·	·	·
26	1	3.5 in	1.50 in	194 lbs	-288 lbs	·	·	-288 lbs	·
14	1	3.5 in	1.50 in	947 lbs	·	·	·	·	·

Material

TC: SYP#1 4 x 2
BC: SYP#1 4 x 2
Web: SYP#1 4 x 2

Loads

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.499	359 lbs		4-5	0.676	2,433 lbs		9-10	0.313		(-2,567 lbs)	12-13	0.338		(-822 lbs)
	2-3	0.653	1,206 lbs		6-8	0.407		(-1,749 lbs)	10-11	0.373		(-2,918 lbs)				
	3-4	0.633	2,433 lbs		8-9	0.425		(-2,567 lbs)	11-12	0.379		(-2,176 lbs)				
BC	15-16	0.365	1,644 lbs		18-19	0.707	2,567 lbs		22-23	0.137		(-1,078 lbs)				
	16-17	0.601	2,672 lbs		19-21	0.707	2,567 lbs		23-24	0.218		(-1,889 lbs)				
	17-18	0.638	2,896 lbs		21-22	0.295	978 lbs		24-25	0.104		(-751 lbs)				
Web	1-25	0.065		(-479 lbs)	5-22	0.241	1,456 lbs		11-17	0.054	329 lbs					
	2-25	0.101	532 lbs		6-22	0.162		(-1,332 lbs)	11-16	0.082		(-670 lbs)				
	2-24	0.103		(-849 lbs)	6-21	0.170	1,025 lbs		12-16	0.120	722 lbs					
	3-24	0.161	973 lbs		8-21	0.130		(-1,124 lbs)	12-15	0.135		(-1,115 lbs)				
	3-23	0.110		(-997 lbs)	8-19	0.094	495 lbs		13-15	0.194	1,171 lbs					
	5-23	0.219		(-1,805 lbs)	10-18	0.064		(-484 lbs)	13-14	0.099		(-931 lbs)				

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
4) A creep factor of 2.00 has been applied for this truss analysis.
5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
6) ☐ Indicates non-structural members.
7) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 26 may need to be considered.

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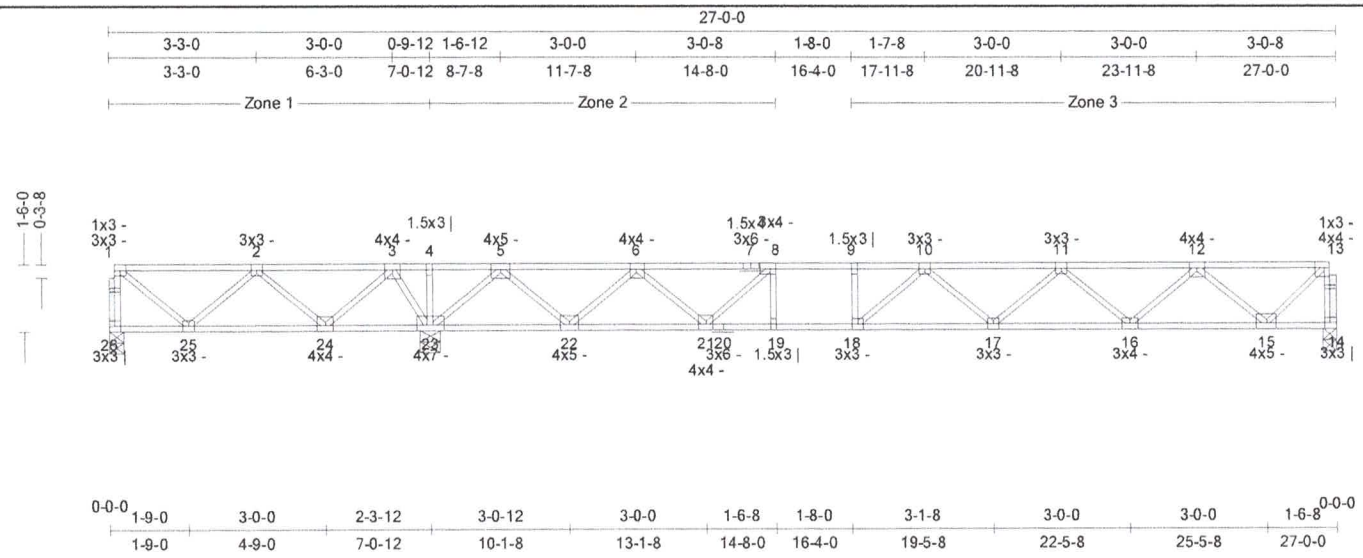
	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss:F10 Job: QU03648_RESERVE BUILDING J_REV Date: 02/17/26 15:40:32 Page: 1 of 1

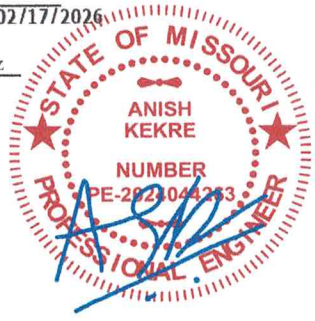
SPAN	PITCH	QTY	OHL	OHR	PLY(S)	SPACING	WGT/PLY
27-0-0	0/12	4	0-0-0	0-0-0	1	19.19 in	137 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
TCLL: 40	Bldg Code:	IBC 2018/	TC:	0.40 (9-10)	Vert TL:	0.4 in	L/583	(17-18)	L/240
TCDL: 10		TPI 1-2014	BC:	0.65 (19-21)	Vert LL:	0.23 in	L/999	(17-18)	L/360
BCLL: 0	Rep Mbr:	Yes	Web:	0.24 (5-22)	Horz TL:	0.03 in		14	
BCDL: 10	Lumber D.O.L.:	100 %							

02/17/2026



Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
23	1	5.5 in	1.50 in	2,092 lbs	·	·	·	·	·
26	1	4 in	1.50 in	206 lbs	-252 lbs	·	·	-252 lbs	·
14	1	3.5 in	1.50 in	971 lbs	·	·	·	·	·

Material

TC: SYP 2400/1.8 4 x 2
BC: SYP 2400/1.8 4 x 2
Web: SYP #1 4 x 2

Loads

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.264	319 lbs		4-5	0.331	2,169 lbs		9-10	0.397	(-2,640 lbs)	12-13	0.180	(-847 lbs)
	2-3	0.329	1,087 lbs		6-8	0.382		(-1,875 lbs)	10-11	0.260	(-2,929 lbs)			
	3-4	0.296	2,169 lbs		8-9	0.381		(-2,640 lbs)	11-12	0.200	(-2,247 lbs)			
BC	15-16	0.188	1,692 lbs		18-19	0.647	2,640 lbs		22-23	0.074	(-824 lbs)			
	16-17	0.344	2,714 lbs		19-21	0.647	2,640 lbs		23-24	0.145	(-1,701 lbs)			
	17-18	0.558	2,970 lbs		21-22	0.173	1,155 lbs		24-25	0.067	(-672 lbs)			
Web	1-25	0.057		(-425 lbs)	5-22	0.236	1,427 lbs		11-16	0.077	(-633 lbs)			
	2-25	0.091	479 lbs		6-22	0.153		(-1,259 lbs)	12-16	0.125	753 lbs			
	2-24	0.095		(-786 lbs)	6-21	0.162	976 lbs		12-15	0.139		(-1,147 lbs)		
	3-24	0.150	908 lbs		8-21	0.125		(-1,045 lbs)	13-15	0.200	1,207 lbs			
	3-23	0.101		(-921 lbs)	8-19	0.057	303 lbs		13-14	0.102		(-956 lbs)		
	5-23	0.217		(-1,791 lbs)	10-18	0.067		(-497 lbs)						

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
4) A creep factor of 2.00 has been applied for this truss analysis.
5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
6) ☐ Indicates non-structural members.
7) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 26 may need to be considered.

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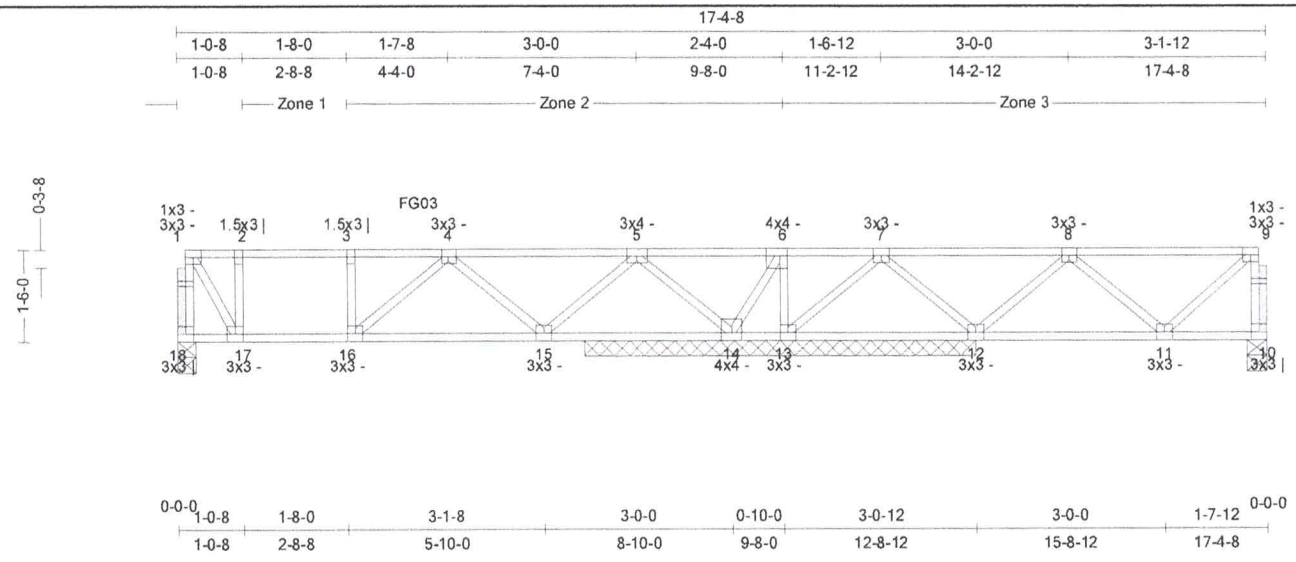
	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss:FG02 Job: QU03648_RESERVE BUILDING J_REV Date: 02/17/26 15:40:34 Page: 1 of 2

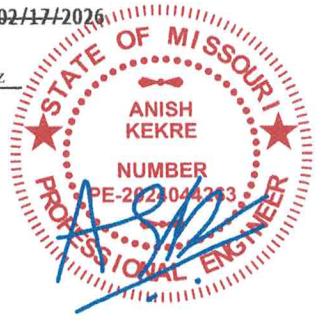
SPAN	PITCH	QTY	OHL	OHR	PLY(S)	SPACING	WGT/PLY
17-4-8	0/12	1	0-0-0	0-0-0	2	50.47 in	90 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code :	IBC 2018/ TPI 1-2014	TC :	0.60 (5-6)	Vert TL:	0.06 in	L/999	(15-16)	L/240
TCLL : 40			BC :	0.27 (16-17)	Vert LL:	0.04 in	L/999	(15-16)	L/360
TCDL : 10	Rep Mbr :	No	Web :	0.12 (5-14)	Horz TL:	0 in		10	
BCLL : 0	Lumber D.O.L. : 100 %								
BCDL : 10									

02/17/2026



Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
18	1	3.5 in	1.50 in	300 lbs	·	·	·	·	·
10	1	4 in	1.50 in	67 lbs	·	·	·	·	·
12	1	75 in	N/A	19 lbs	-69 lbs	·	·	-69 lbs	·
13	1	75 in	N/A	0 lbs	-160 lbs	·	·	-160 lbs	·
14	1	75 in	N/A	2,548 lbs	·	·	·	·	·

Material

TC: SYP#1 4 x 2
BC: SYP#1 4 x 2
Web: SYP#1 4 x 2

Loads

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case L1: Std Live Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	17-4-8	Down	Proj	23.65 plf	23.65 plf	
Top	0-0-0	3-10-4	Down	Proj	12.5 plf	12.5 plf	
Top	3-10-4	12-6-0	Down	Proj	144.58 plf	144.58 plf	
Top	12-6-0	17-4-8	Down	Proj	16.67 plf	16.67 plf	

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	17-4-8	Down	Proj	5.91 plf	5.91 plf	
Top	0-0-0	3-10-4	Down	Proj	3.13 plf	3.13 plf	
Top	3-10-4	12-6-0	Down	Proj	36.15 plf	36.15 plf	
Top	12-6-0	17-4-8	Down	Proj	4.17 plf	4.17 plf	
Bot	0-0-0	17-4-8	Down	Proj	5.91 plf	5.91 plf	
Bot	0-0-0	3-10-4	Down	Proj	3.13 plf	3.13 plf	

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	5-6	0.601	1,120 lbs			
	6-7	0.355	734 lbs			
BC	12-13	0.042	(-316 lbs)	14-15	0.058	(-440 lbs)
	13-14	0.082	(-734 lbs)			

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SPAN	PITCH	QTY	OHL	OHR	PLY(S)	SPACING	WGT/PLY
17-4-8	0/12	1	0-0-0	0-0-0	2	50.47 in	90 lbs

Web	1-17	0.054	303 lbs	6-14	0.089		(-744 lbs)
	4-15	0.057	(-435 lbs)	6-13	0.082	454 lbs	
	5-15	0.087	483 lbs	7-13	0.073		(-557 lbs)
	5-14	0.121	(-922 lbs)				

Truss to Truss Connection Summary

Carried Truss	Carrying Chord	Carrying Offset
FG03	TC	3-10-4

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
4) A creep factor of 2.00 has been applied for this truss analysis.
5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
6) ☐ Indicates non-structural members.
7) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 12, 13 may need to be considered.
8) The forces shown for this multi-ply truss are per ply and the reactions are for all plies. Two identical trusses shall be built and attached as follows: SDS Simpson 0.250"x6" Screws TC - 1 row @ 1-10-12 oc, BC - 1 row @ 2-0-0 oc , Webs - 1 @ 2-0-0 oc, minimum one fastener per web.

Provided the hanger connections do not adequately transfer the applied load to all plies: in addition to connectors shown above, attach girder plies with supplemental SDS Simpson 0.250"x6" Screws as follows within 24" of the location shown:
TC: 3-10-4,(1)Connectors

Connectors shall not encroach on other girder ply connectors or truss-to-truss connectors in accordance with the NDS or the connector manufacturer recommendations.
9) Screws shall be installed in the same truss ply that the hangers are attached to. If both plies are loaded, the screws shall be divided between the two plies, with the spacing on each side twice the minimum indicated.
10) Strongbacks shall be attached to each ply.
11) Center screw vertically on the 1-1/2" dimension of chords and webs. If splitting occurs, it may be necessary to pre-drill the holes in accordance with the NDS.
12) Install screws per manufacturer recommendations.

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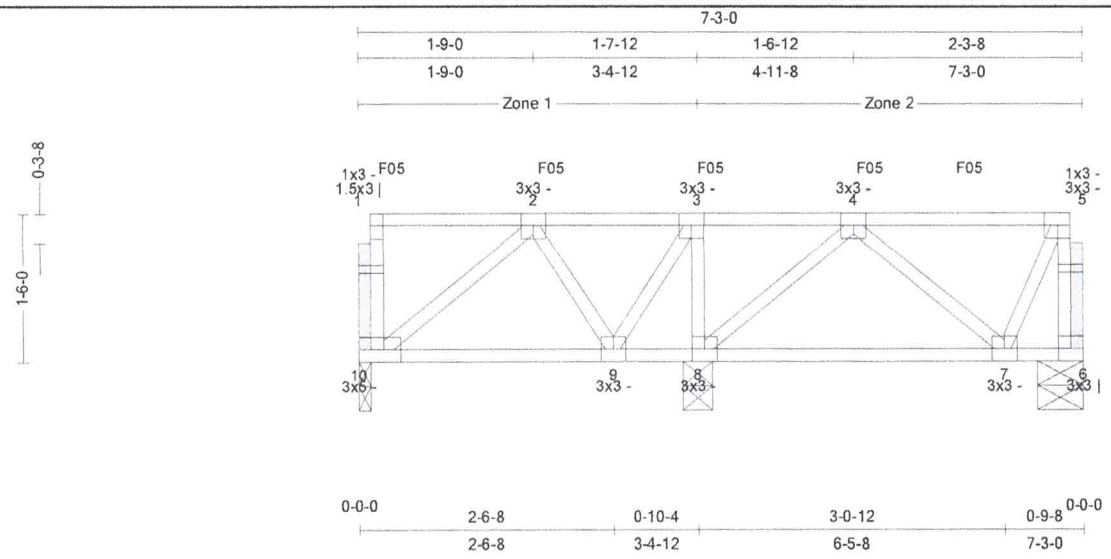
SPAN	PITCH	QTY	OHL	OHR	PLY(S)	SPACING	WGT/PLY
7-3-0	0/12	1	0-0-0	0-0-0	1	52.75 in	44 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code :	IBC 2018/ TPI 1-2014	TC :	0.34 (4-5)	Vert TL:	0.02 in	L/999	(9-10)	L/240
TCLL : 40			BC :	0.22 (9-10)	Vert LL:	0.01 in	L/999	(9-10)	L/360
TCDL : 10	Rep Mbr :	No	Web :	0.05 (3-9)	Horz TL:	0 in		6	
BCLL : 0	Lumber D.O.L. :	100 %							
BCDL : 10									

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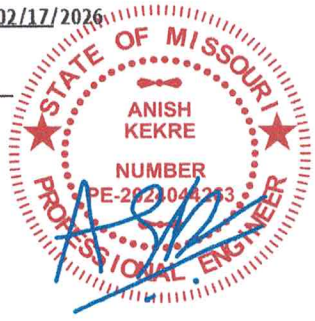

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
8	1	3.5 in	1.50 in	600 lbs	.	.	.	.	.
10	1	1.5 in	1.50 in	319 lbs	.	.	.	.	.
6	1	5.5 in	1.50 in	200 lbs	.	.	.	.	.

Material

TC: SYP#1 4 x 2
BC: SYP#1 4 x 2
Web: SYP#1 4 x 2

Loads

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Bot	0-0-0	3-6-8	Down	Proj	43.96 plf	43.96 plf	
Bot	3-6-8	7-3-0	Down	Proj	13.96 plf	13.96 plf	

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC			
BC			
Web	3-8	0.038	(-352 lbs)

Truss to Truss Connection Summary

Carried Truss	Carrying Chord	Carrying Offset
F05	TC	0-4-0
F05	TC	1-11-3
F05	TC	3-6-6
F05	TC	5-1-9
F05	TC	6-1-7

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
3) A creep factor of 2.00 has been applied for this truss analysis.
4) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.

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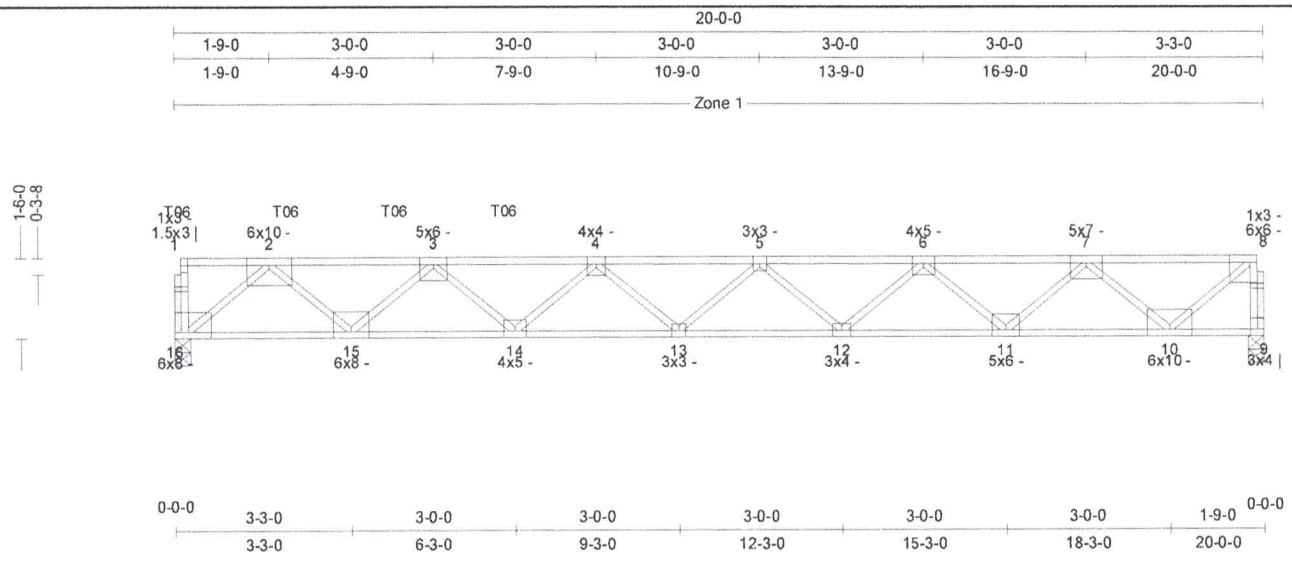
	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss:FG03 Job: QU03648_RESERVE BUILDING J_REV Date: 02/17/26 15:40:35 Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	PLY(S)	SPACING	WGT/PLY
7-3-0	0/12	1	0-0-0	0-0-0	1	52.75 in	44 lbs

5) ☐ Indicates non-structural members.

	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss:FG04 Job: QU03648_RESERVE BUILDING J_REV Date: 02/17/26 15:40:37 Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	PLY(S)	SPACING	WGT/PLY
20-0-0	0/12	1	0-0-0	0-0-0	2	50.84 in	106 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code:	IBC 2018/	TC:	0.83 (4-5)	Vert TL:	0.58 in	L/400	(12-13)	L/240
TCLL: 40		TPI 1-2014	BC:	0.73 (12-13)	Vert LL:	0.23 in	L/999	(12-13)	L/360
TCDL: 10	Rep Mbr:	No	Web:	0.47 (8-10)	Horz TL:	0.12 in		9	
BCLL: 0	Lumber D.O.L.:	100 %							
BCDL: 10									

02/17/2026

STATE OF MISSOURI
ANISH KEKRE
NUMBER PE-2021044233
PROFESSIONAL ENGINEER

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
16	1	3.5 in	1.50 in	4,046 lbs	.	.	.	.	.
9	1	3.5 in	1.50 in	4,051 lbs	.	.	.	.	.

Material

TC: SYP 2400/1.8 4 x 2
BC: SYP 2400/1.8 4 x 2
Web: SYP #1 4 x 2

Loads

1) This truss has been designed for the effects of (psf) sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has not been designed for the effects of unbalanced snow loads.
3) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case L1: Std Live Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	20-0-0	Down	Proj	140.52 plf	140.52 plf	
Top	7-0-12	20-0-0	Down	Proj	28.96 plf	28.96 plf	

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	20-0-0	Down	Proj	110 plf	110 plf	
Top	0-0-0	20-0-0	Down	Proj	35.13 plf	35.13 plf	
Top	7-0-12	20-0-0	Down	Proj	7.24 plf	7.24 plf	
Bot	0-0-0	20-0-0	Down	Proj	35.13 plf	35.13 plf	
Bot	7-0-12	20-0-0	Down	Proj	7.24 plf	7.24 plf	

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	2-3	0.570	(-3,695 lbs)	4-5	0.827	(-7,052 lbs)	6-7	0.703	(-5,081 lbs)		
	3-4	0.818	(-6,031 lbs)	5-6	0.775	(-6,727 lbs)	7-8	0.544	(-2,028 lbs)		
BC	10-11	0.414	3,860 lbs	12-13	0.727	7,159 lbs	14-15	0.537	5,142 lbs		
	11-12	0.626	6,160 lbs	13-14	0.703	6,805 lbs	15-16	0.270	2,118 lbs		

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SPAN	PITCH	QTY	OHL	OHR	PLY(S)	SPACING	WGT/PLY
20-0-0	0/12	1	0-0-0	0-0-0	2	50.84 in	106 lbs

Web	2-16	0.368		(-2,822 lbs)	4-14	0.131		(-1,050 lbs)	6-11	0.188		(-1,463 lbs)	8-9	0.222	(-1,995 lbs)
	2-15	0.376	2,138 lbs		4-13	0.055	335 lbs		7-11	0.283	1,657 lbs				
	3-15	0.255		(-1,963 lbs)	5-12	0.081		(-602 lbs)	7-10	0.312		(-2,485 lbs)			
	3-14	0.200	1,206 lbs		6-12	0.132	770 lbs		8-10	0.466	2,701 lbs				

Truss to Truss Connection Summary

Carried Truss	Carrying Chord	Carrying Offset
T06	TC	0-0-12
T06	TC	2-0-12
T06	TC	4-0-12
T06	TC	6-0-12

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
4) A creep factor of 2.00 has been applied for this truss analysis.
5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
6) ☐ Indicates non-structural members.
7) The forces shown for this multi-ply truss are per ply and the reactions are for all plies. Two identical trusses shall be built and attached as follows: SDS Simpson 0.250"x6" Screws TC - 1 row @ 1-2-12 oc, BC - 1 row @ 2-0-0 oc , Webs - 1 @ 2-0-0 oc, minimum one fastener per web.

Provided the hanger connections do not adequately transfer the applied load to all plies: in addition to connectors shown above, attach girder plies with supplemental SDS Simpson 0.250"x6" Screws as follows within 24" of the location shown:
TC: 0-0-12,(1)Connectors
TC: 2-0-12,(1)Connectors
TC: 4-0-12,(1)Connectors
TC: 6-0-12,(1)Connectors

Connectors shall not encroach on other girder ply connectors or truss-to-truss connectors in accordance with the NDS or the connector manufacturer recommendations.
8) Screws shall be installed in the same truss ply that the hangers are attached to. If both plies are loaded, the screws shall be divided between the two plies, with the spacing on each side twice the minimum indicated.
9) Strongbacks shall be attached to each ply.
10) Center screw vertically on the 1-1/2" dimension of chords and webs. If splitting occurs, it may be necessary to pre-drill the holes in accordance with the NDS.
11) Install screws per manufacturer recommendations.

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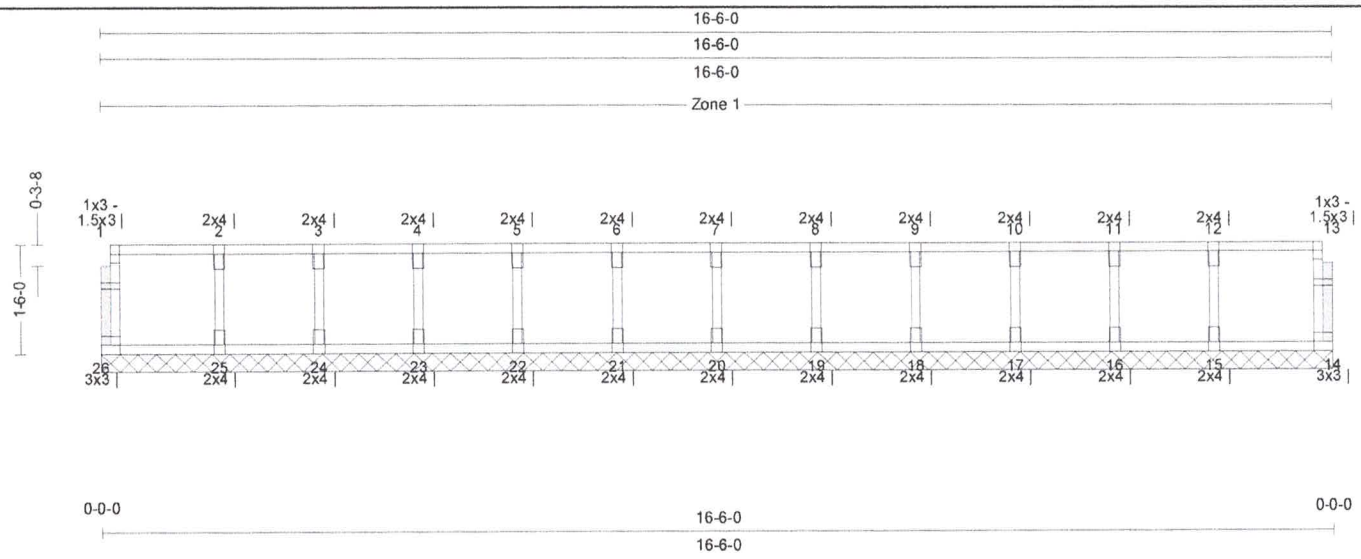
SPAN	PITCH	QTY	OHL	OHR	PLY(S)	SPACING	WGT/PLY
16-6-0	0/12	1	0-0-0	0-0-0	1	19.19 in	75 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
TCLL: 40	Bldg Code:	IBC 2018/	TC:	0.07 (12-13)	Vert TL:	0 in UP	L/999	14	L/240
TCDL: 10		TPI 1-2014	BC:	0.02 (15-16)	Vert LL:	0 in	L/999	14	L/360
BCLL: 0	Rep Mbr:	No	Web:	0.04 (1-26)	Horz TL:	0 in			
BCDL: 10	Lumber D.O.L.:	100 %							

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STATE OF MISSOURI
ANISH KEKRE
NUMBER PE-2021044263
PROFESSIONAL ENGINEER

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	-	154 lbs	101 plf	-	-	-	-	-5 lbs

Material

TC: SYP#1 4 x 2
BC: SYP#1 4 x 2
Web: SYP#1 4 x 2

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC		
BC		
Web		

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
3) Gable requires continuous bottom chord bearing.
4) Continuous bearing knee-wall/ladder floor trusses are not designed for any loads from levels above. Additional blocking, by others, may be required in order to transfer loads.
5) Gable webs placed at 16 " OC, U.N.O.
6) Attach gable webs with 2x4 20ga plates, U.N.O.
7) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
8) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
9) A creep factor of 2.00 has been applied for this truss analysis.
10) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
11) ☐ Indicates non-structural members.

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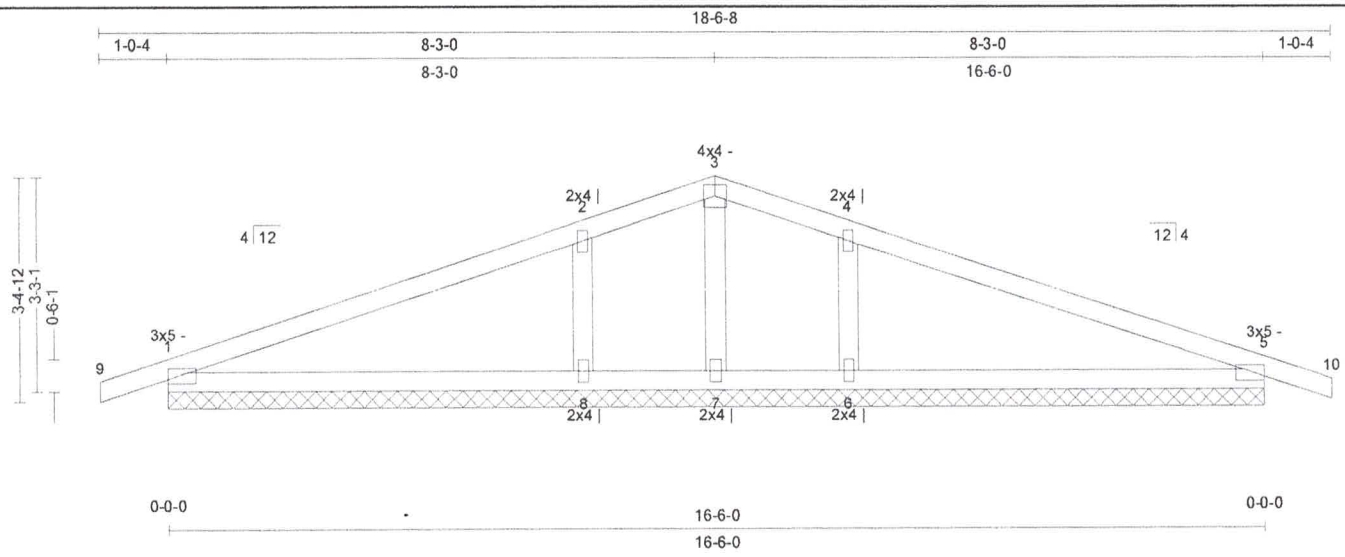
SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
16-6-0	4/12	1	1-0-4	1-0-4	0-0-0	0-0-0	1	24 in	61 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2018/	TC:	0.37 (4-5)	Vert TL:	0.02 in	L/999	(5-6)	L/240
TCDL: 10		TPI 1-2014	BC:	0.14 (5-6)	Vert LL:	0 in	L/999	5	L/360
BCLL: 0	Rep Mbr:	No	Web:	0.05 (4-6)	Horz TL:	0 in			
BCDL: 10	Lumber D.O.L.:	115 %							

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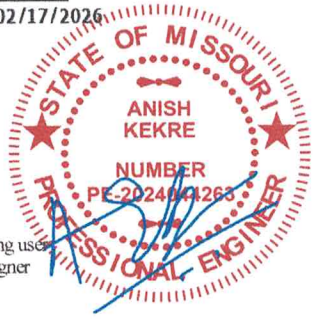

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1		1,295 lbs	266 plf	-977 lbs	-168 lbs	-575 lbs	-977 lbs	711 lbs

Material

TC: SYP#1 2 x 4
BC: SYP#1 2 x 4
Web: SYP#2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.374	1,114 lbs	(-526 lbs)							
	4-5	0.374	1,114 lbs	(-526 lbs)							
BC											
Web	2-8	0.050		(-355 lbs)	4-6	0.050		(-355 lbs)			

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Gable requires continuous bottom chord bearing.
3) Gable webs placed at 24 " OC, U.N.O.
4) Attach gable webs with 2x4 20ga plates, U.N.O.
5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
6) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
7) A creep factor of 2.00 has been applied for this truss analysis.
8) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
9) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 7, 5, 1 may need to be considered.
10) Listed wind uplift reactions based on MWFRS & C&C loading.

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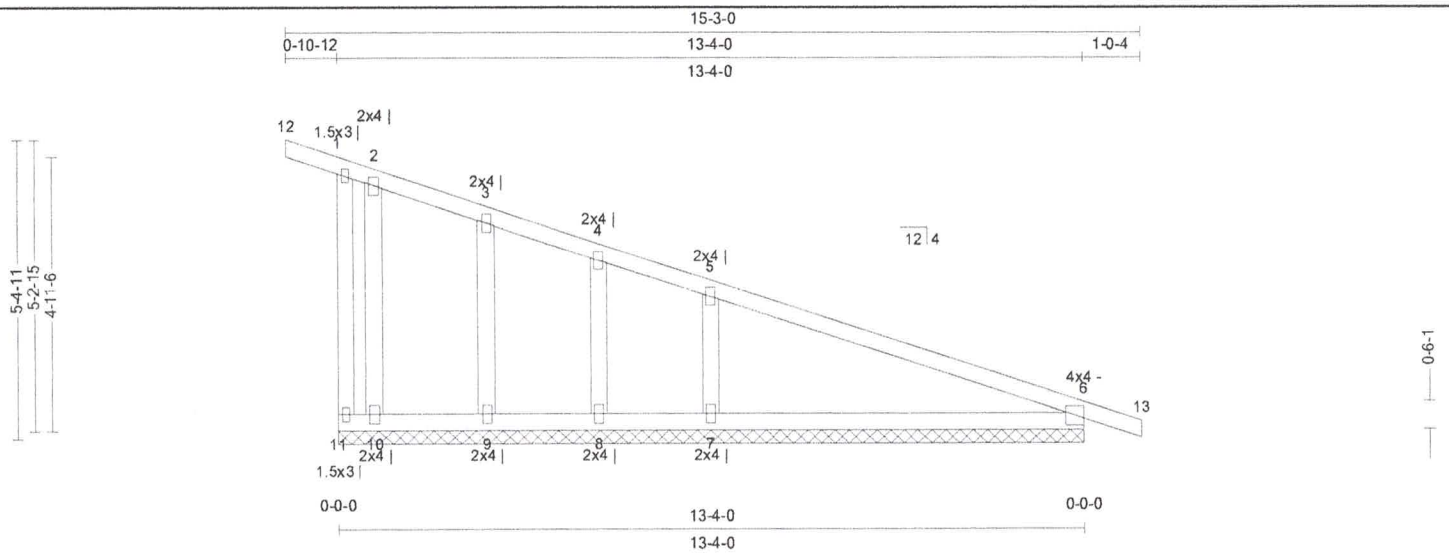
SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
13-4-0	-4 /12	1	0-10-12	1-0-4	0-0-0	0-0-0	1	24 in	65 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2018/	TC:	0.52 (5-6)	Vert TL:	0.03 in	L/999	(6-7)	L/240
TCDL: 10		TPI 1-2014	BC:	0.16 (6-7)	Vert LL:	0 in UP	L/999	(6-7)	L/360
BCLL: 0	Rep Mbr:	No	Web:	0.21 (1-11)	Horz TL:	0 in			
BCDL: 10	Lumber D.O.L.: 115 %								

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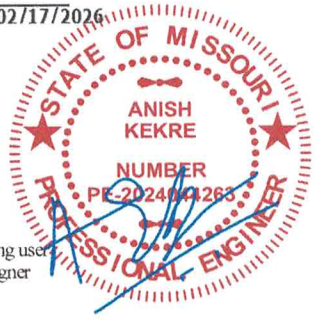

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1		1,735 lbs	257 plf	-1,378 lbs	-281 lbs	-931 lbs	-1,378 lbs	915 lbs

Material

TC: SYP #1 2 x 4
BC: SYP #1 2 x 4
Web: SYP #2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	5-6	0.517	1,536 lbs	(-911 lbs)				
BC								
Web	5-7	0.053	300 lbs	(-345 lbs)				

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Gable requires continuous bottom chord bearing.
3) Gable webs placed at 24 " OC, U.N.O.
4) Attach gable webs with 2x4 20g a plates, U.N.O.
5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
6) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
7) A creep factor of 2.00 has been applied for this truss analysis.
8) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
9) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 8, 6 may need to be considered.
10) Listed wind uplift reactions based on MWFRS & C&C loading.

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Eagle Metal Products

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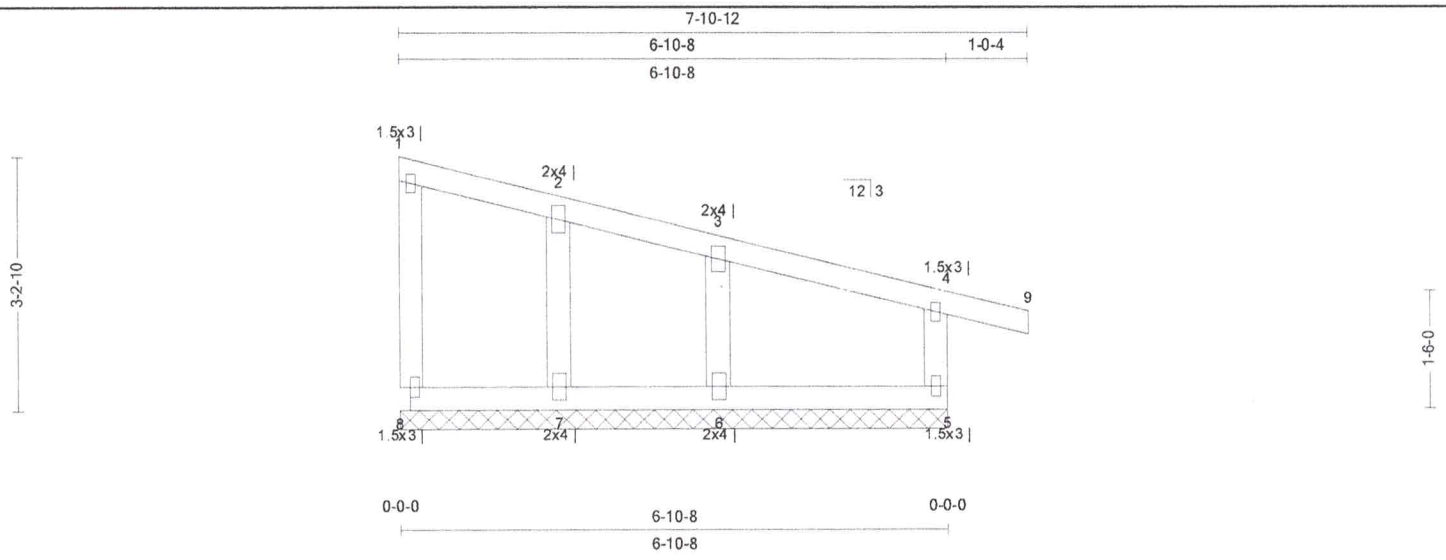
SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
6-10-8	-3 /12	2	0-0-0	1-0-4	0-0-0	0-0-0	1	24 in	32 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2018/	TC:	0.16 (1-2)	Vert TL:	0 in	L/999	(5-6)	L/240
TCDL: 10		TPI 1-2014	BC:	0.04 (5-6)	Vert LL:	0 in	L/999	5	L/360
BCLL: 0	Rep Mbr:	No	Web:	0.38 (4-5)	Horz TL:	0 in			
BCDL: 10	Lumber D.O.L.: 115 %								

02/17/2026

STATE OF MISSOURI
ANISH KEKRE
NUMBER PE-2024044263
PROFESSIONAL ENGINEER

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	-	198 lbs	91 plf	-	-56 lbs	-255 lbs	-255 lbs	-160 lbs

Material

TC: SYP#1 2 x 4
BC: SYP#1 2 x 4
Web: SYP#2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (20 psf) sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has not been designed for the effects of unbalanced snow loads.
3) This truss has been designed to account for the effects of ice dams forming at the eaves.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC		
BC		
Web		

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Gable requires continuous bottom chord bearing.
3) Gable webs placed at 24 " OC, U.N.O.
4) Attach gable webs with 1.5x3 20 ga plates, U.N.O.
5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
6) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
7) A creep factor of 2.00 has been applied for this truss analysis.
8) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
9) Please refer to Eagle Metal's Engineering Details sheet titled, Girder Ledger Detail.
10) Listed wind uplift reactions based on MWFRS & C&C loading.

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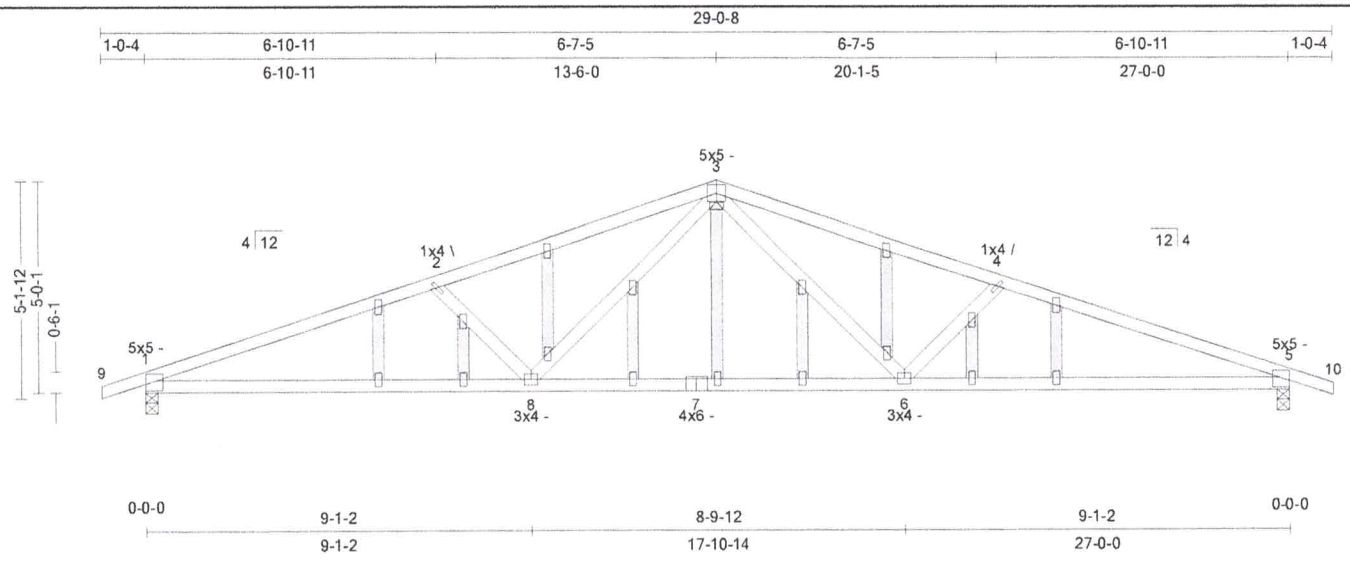
	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss:SGE01 Job: QU03648_RESERVE BUILDING J_REV Date: 02/17/26 15:40:44 Page: 1 of 1

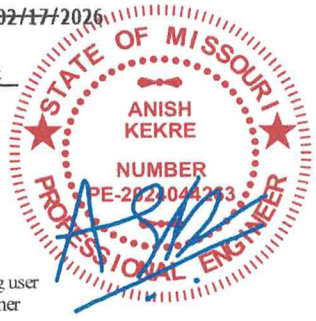
SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
27-0-0	4 /12	1	1-0-4	1-0-4	0-0-0	0-0-0	1	24 in	139 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2018/	TC:	0.62 (2-3)	Vert TL:	0.48 in	L/661	(6-7)	L/240
TCDL: 10		TPI 1-2014	BC:	1.00 (8-1)	Vert LL:	0.16 in	L/999	7	L/360
BCLL: 0	Rep Mbr:	No	Web:	0.14 (3-8)	Horz TL:	0.1 in		5	
BCDL: 10	Lumber D.O.L.: 115 %								

02/17/2026



Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	3.5 in	1.67 in	1,411 lbs	·	-106 lbs	-319 lbs	-319 lbs	-4 lbs
5	1	3.5 in	1.67 in	1,411 lbs	·	-106 lbs	-319 lbs	-319 lbs	·

Material

TC: SYP#1 2 x 4
BC: SYP#1 2 x 4
Web: SYP#1 2 x 4

Bracing

TC: Sheathed or Purlins at 3-1-0, Purlin design by Others.
BC: Sheathed

Loads

1) This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
4) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.544	583 lbs	(-3,032 lbs)	3-4	0.618	531 lbs	(-2,743 lbs)							
	2-3	0.618	531 lbs	(-2,743 lbs)	4-5	0.544	583 lbs	(-3,032 lbs)							
BC	5-6	1.002	2,814 lbs	(-448 lbs)	6-8	0.859	1,951 lbs		8-1	1.002	2,814 lbs	(-448 lbs)			
Web	2-8	0.081		(-421 lbs)	3-8	0.144	870 lbs		3-6	0.144	870 lbs		4-6	0.081	(-421 lbs)

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Gable webs placed at 24 " OC, U.N.O.
3) Attach structural gable blocks with 2x4 20g a plates, U.N.O.
4) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
5) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
6) A creep factor of 2.00 has been applied for this truss analysis.
7) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
8) ☐ Indicates non-structural members.
9) Listed wind uplift reactions based on MWFRS & C&C loading.

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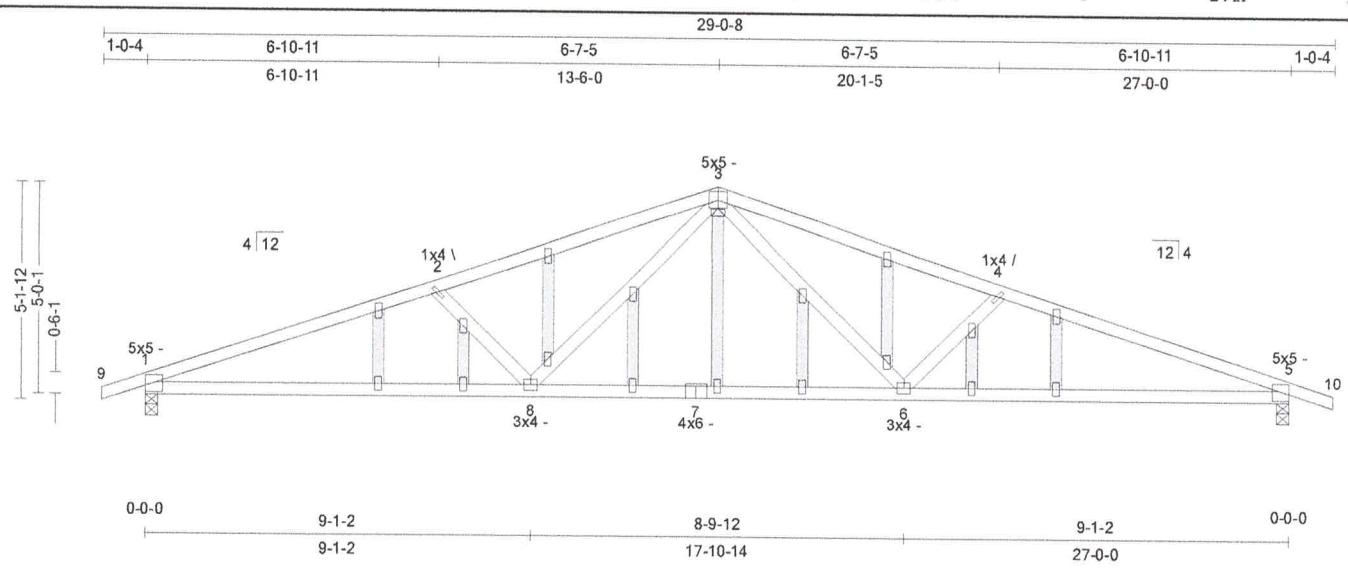
Eagle Metal Products

	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss:SGE02 Job: QU03648_RESERVE BUILDING J_REV Date: 02/17/26 15:40:45 Page: 1 of 1

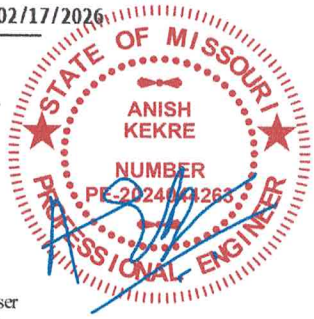
SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
27-0-0	4/12	1	1-0-4	1-0-4	0-0-0	0-0-0	1	24 in	139 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2018/ TPI 1-2014	TC:	0.36 (2-3)	Vert TL:	0.43 in	L/740	(6-7)	L/240
TCDL: 10	Rep Mbr:	No	BC:	0.58 (8-1)	Vert LL:	0.14 in	L/999	(6-7)	L/360
BCLL: 0	Lumber D.O.L.:	115 %	Web:	0.14 (3-8)	Horz TL:	0.09 in		5	
BCDL: 10									

02/17/2026



Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	3.5 in	1.50 in	1,411 lbs	·	-106 lbs	-319 lbs	-319 lbs	-4 lbs
5	1	3.5 in	1.50 in	1,411 lbs	·	-106 lbs	-319 lbs	-319 lbs	·

Material

TC: SYP 2400/1.8 2 x 4
BC: SYP 2400/1.8 2 x 4
Web: SYP #1 2 x 4

Bracing

TC: Sheathed or Purlins at 4-2-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE 7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE 7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
4) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.347	583 lbs	(-3,032 lbs)	3-4	0.356	531 lbs	(-2,743 lbs)						
	2-3	0.356	531 lbs	(-2,743 lbs)	4-5	0.347	583 lbs	(-3,032 lbs)						
BC	5-6	0.577	2,814 lbs	(-448 lbs)	6-8	0.503	1,951 lbs		8-1	0.577	2,814 lbs	(-448 lbs)		
Web	2-8	0.081		(-421 lbs)	3-8	0.144	870 lbs		3-6	0.144	870 lbs		4-6	0.081	(-421 lbs)

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Gable webs placed at 24 " OC, U.N.O.
3) Attach structural gable blocks with 2x4 20ga plates, U.N.O.
4) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
5) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
6) A creep factor of 2.00 has been applied for this truss analysis.
7) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
8) ▭ Indicates non-structural members.
9) Listed wind uplift reactions based on MWFRS & C&C loading.

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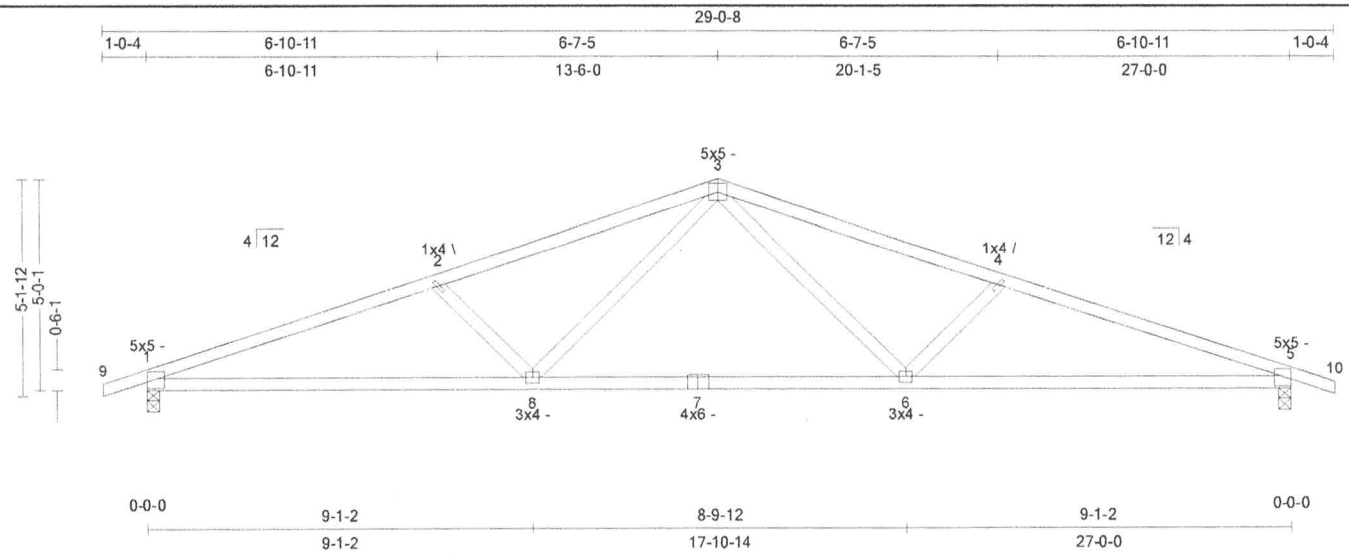
Eagle Metal Products

	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss: T01 Job: QU03648_RESERVE BUILDING J_REV Date: 02/17/26 15:40:47 Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
27-0-0	4/12	15	1-0-4	1-0-4	0-0-0	0-0-0	1	24 in	107 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2018/	TC:	0.52 (2-3)	Vert TL:	0.48 in	L/661	(6-7)	L/240
TCDL: 10		TPI 1-2014	BC:	0.89 (8-1)	Vert LL:	0.16 in	L/999	7	L/360
BCLL: 0	Rep Mbr:	Yes	Web:	0.14 (3-8)	Horz TL:	0.1 in		5	
BCDL: 10	Lumber D.O.L.: 115%								

02/17/2026

STATE OF MISSOURI
ANISH KEKRE
NUMBER PE-2024044263
PROFESSIONAL ENGINEER

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	3.5 in	1.67 in	1,411 lbs	·	-106 lbs	-319 lbs	-319 lbs	-4 lbs
5	1	3.5 in	1.67 in	1,411 lbs	·	-106 lbs	-319 lbs	-319 lbs	·

Material

TC: SYP#1 2 x 4
BC: SYP#1 2 x 4
Web: SYP#1 2 x 4

Bracing

TC: Sheathed or Purlins at 3-4-0, Purlin design by Others.
BC: Sheathed or Purlins at 9-6-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
4) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.469	583 lbs	(-3,032 lbs)	3-4	0.522	531 lbs	(-2,743 lbs)							
	2-3	0.522	531 lbs	(-2,743 lbs)	4-5	0.469	583 lbs	(-3,032 lbs)							
BC	5-6	0.890	2,814 lbs	(-448 lbs)	6-8	0.760	1,951 lbs		8-1	0.890	2,814 lbs	(-448 lbs)			
Web	2-8	0.081		(-421 lbs)	3-8	0.144	870 lbs		3-6	0.144	870 lbs		4-6	0.081	(-421 lbs)

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20% (Cq = 0.80).
3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
4) A creep factor of 2.00 has been applied for this truss analysis.
5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
6) Listed wind uplift reactions based on MWFRS & C&C loading.

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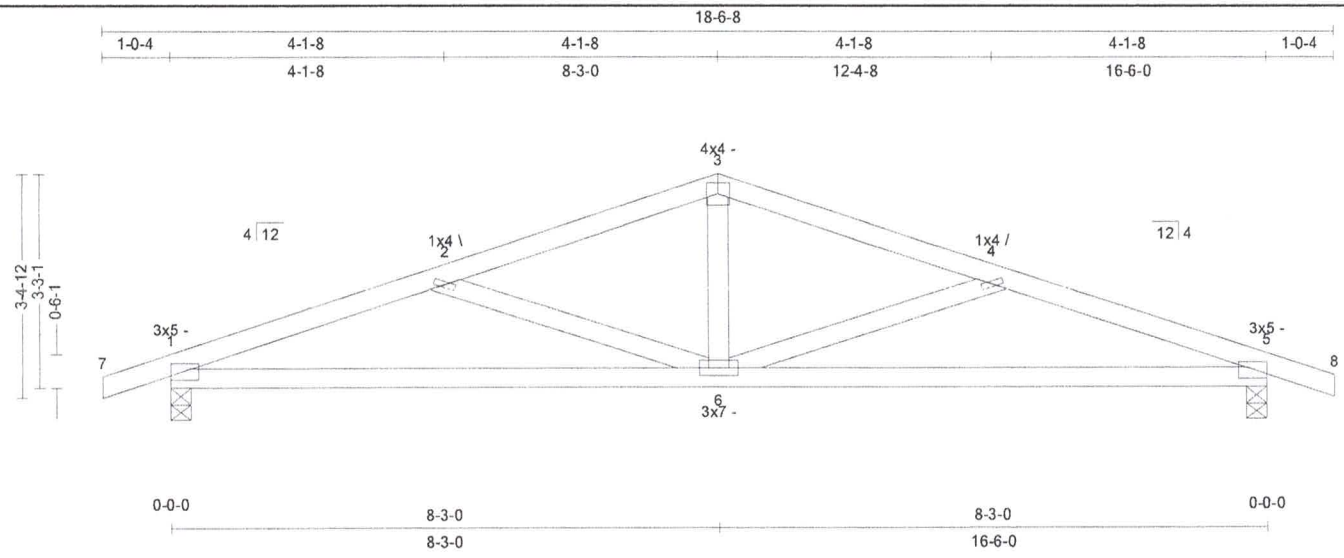
Eagle Metal Products

	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss: T02 Job: QU03648_RESERVE BUILDING J_REV Date: 02/17/26 15:40:48 Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
16-6-0	4/12	4	1-0-4	1-0-4	0-0-0	0-0-0	1	24 in	67 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2018/	TC:	0.22 (4-5)	Vert TL:	0.17 in	L/999	(5-6)	L/240
TCDL: 10		TPI 1-2014	BC:	0.63 (6-1)	Vert LL:	0.07 in	L/999	(5-6)	L/360
BCLL: 0	Rep Mbr:	Yes	Web:	0.13 (2-6)	Horz TL:	0.04 in		5	
BCDL: 10	Lumber D.O.L.: 115 %								

02/17/2026

STATE OF MISSOURI
ANISH KEKRE
NUMBER PE-2024044263
PROFESSIONAL ENGINEER

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	3.5 in	1.50 in	886 lbs	·	-72 lbs	-338 lbs	-338 lbs	-5 lbs
5	1	3.5 in	1.50 in	886 lbs	·	-72 lbs	-338 lbs	-338 lbs	·

Material

TC: SYP #1 2 x 4
BC: SYP #1 2 x 4
Web: SYP #1 2 x 4

Bracing

TC: Sheathed or Purlins at 4-11-0, Purlin design by Others.
BC: Sheathed or Purlins at 9-2-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
4) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.225	610 lbs	(-1,683 lbs)	3-4	0.207	446 lbs	(-1,352 lbs)			
	2-3	0.207	446 lbs	(-1,352 lbs)	4-5	0.225	610 lbs	(-1,683 lbs)			
BC	5-6	0.627	1,560 lbs	(-485 lbs)	6-1	0.627	1,560 lbs	(-485 lbs)			
Web	2-6	0.125		(-403 lbs)	3-6	0.088	534 lbs		4-6	0.125	(-403 lbs)

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
4) A creep factor of 2.00 has been applied for this truss analysis.
5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
6) Listed wind uplift reactions based on MWFRS & C&C loading.

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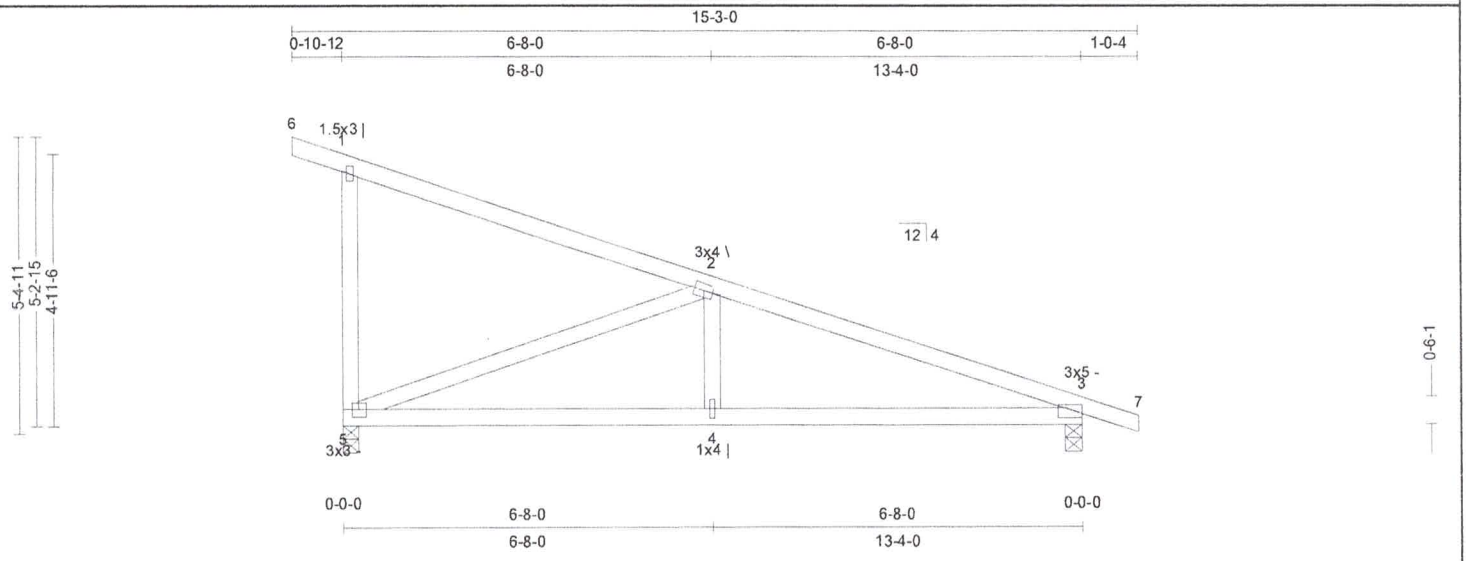
Eagle Metal Products

	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss: T03 Job: QU03648_RESERVE BUILDING J_REV Date: 02/17/26 15:40:49 Page: 1 of 1

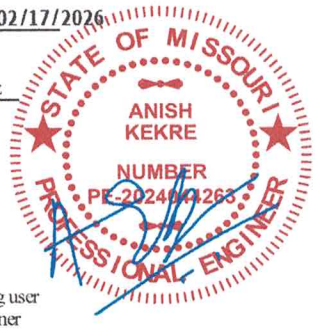
SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
13-4-0	-4/12	2	0-10-12	1-0-4	0-0-0	0-0-0	1	24 in	59 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2018/	TC:	0.50 (1-2)	Vert TL:	0.13 in	L/999	(4-5)	L/240
TCDL: 10		TPI 1-2014	BC:	0.61 (4-5)	Vert LL:	0.05 in	L/999	(4-5)	L/360
BCLL: 0	Rep Mbr:	No	Web:	0.83 (2-5)	Horz TL:	0.02 in		3	
BCDL: 10	Lumber D.O.L.: 115 %								

02/17/2026



Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
5	1	3.5 in	1.50 in	728 lbs	·	-88 lbs	-363 lbs	-363 lbs	-178 lbs
3	1	3.5 in	1.50 in	720 lbs	·	-33 lbs	-310 lbs	-310 lbs	·

Material

TC: SYP#1 2 x 4
BC: SYP#1 2 x 4
Web: SYP#1 2 x 4

Bracing

TC: Sheathed or Purlins at 5-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
4) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	2-3	0.466	356 lbs	(-1,175 lbs)							
BC	3-4	0.607	1,063 lbs		4-5	0.607	1,063 lbs				
Web	2-5	0.828	502 lbs	(-1,134 lbs)	2-4	0.053	321 lbs				

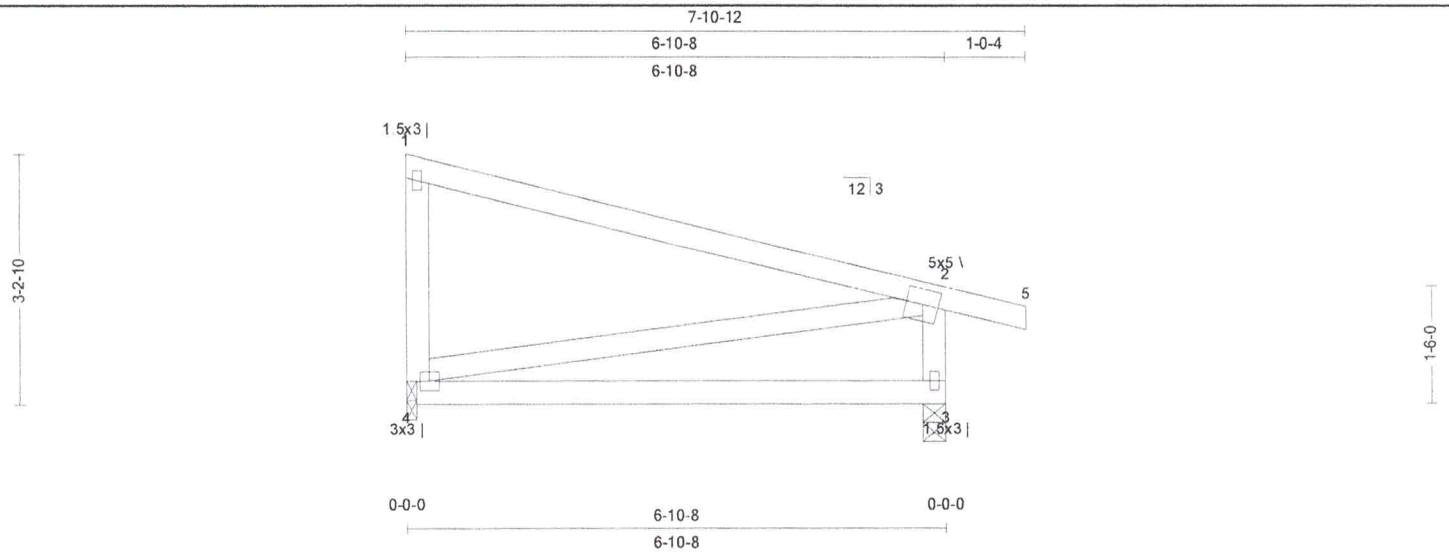
Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
4) A creep factor of 2.00 has been applied for this truss analysis.
5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
6) Listed wind uplift reactions based on MWFRS & C&C loading.

TrueBuild® Truss Software v5.8.14
Eagle Metal Products

	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss: T04 Job: QU03648_RESERVE BUILDING J_REV Date: 02/17/26 15:40:51 Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
6-10-8	-3 /12	9	0-0-0	1-0-4	0-0-0	0-0-0	1	24 in	35 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2018/	TC:	0.62 (1-2)	Vert TL:	0.21 in	L/377	(3-4)	L/240
TCDL: 10		TPI 1-2014	BC:	0.44 (3-4)	Vert LL:	0.1 in	L/755	(3-4)	L/360
BCLL: 0	Rep Mbr:	Yes	Web:	0.12 (1-4)	Horz TL:	0 in		4	
BCDL: 10	Lumber D.O.L.: 115 %								

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
4	1	1.5 in	1.50 in	335 lbs	·	-39 lbs	-270 lbs	-270 lbs	·
3	1	3.5 in	1.50 in	414 lbs	·	-38 lbs	-343 lbs	-343 lbs	-121 lbs

02/17/2026

STATE OF MISSOURI
ANISH KEKRE
NUMBER PE-2024044263
PROFESSIONAL ENGINEER

Material

TC: SYP#1 2 x 4
BC: SYP#1 2 x 4
Web: SYP#1 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (20 psf) sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has not been designed for the effects of unbalanced snow loads.
3) This truss has been designed to account for the effects of ice dams forming at the eaves.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
5) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC						
BC						
Web	2-3	0.063	385 lbs			

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
4) A creep factor of 2.00 has been applied for this truss analysis.
5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
6) Please refer to Eagle Metal's Engineering Details sheet titled, Girder Ledger Detail.
7) Listed wind uplift reactions based on MWFRS & C&C loading.

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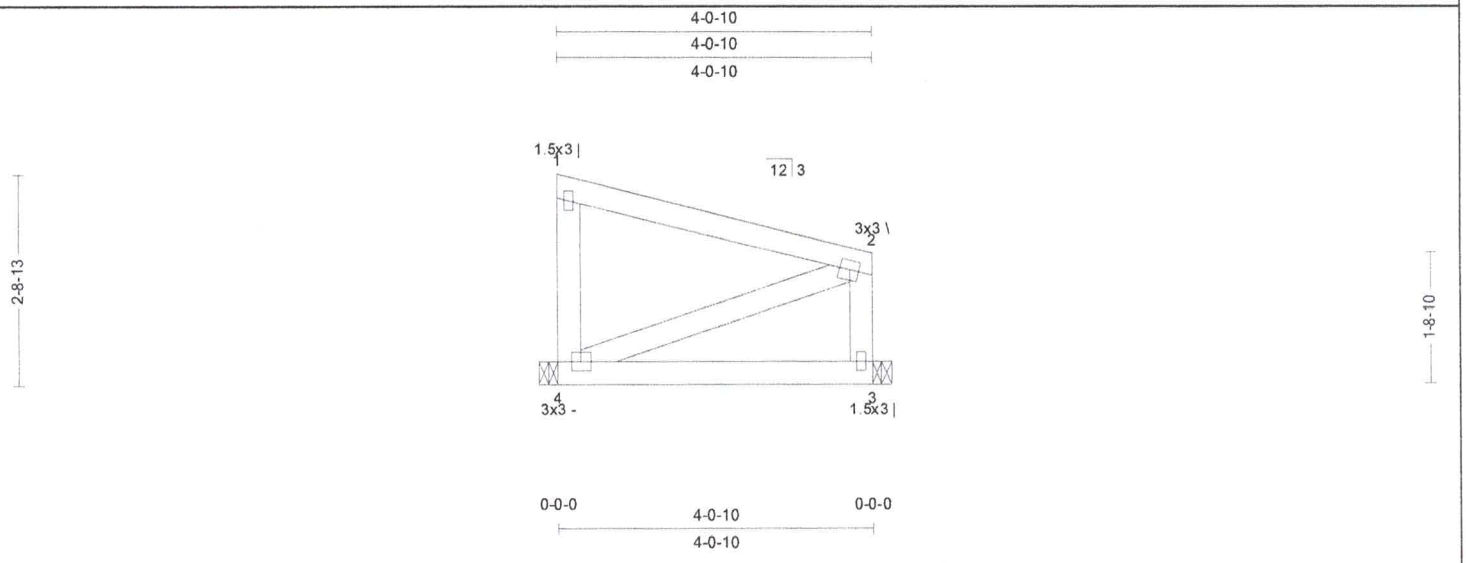
Eagle Metal Products

	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss: T06 Job: QU03648_RESERVE BUILDING J_REV Date: 02/17/26 15:40:52 Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
4-0-10	-3 /12	5	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	21 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2018/	TC:	0.26 (1-2)	Vert TL:	0.02 in	L/999	(3-4)	L/240
TCDL: 10		TPI 1-2014	BC:	0.14 (3-4)	Vert LL:	0.01 in	L/999	(3-4)	L/360
BCLL: 0	Rep Mbr:	Yes	Web:	0.09 (1-4)	Horz TL:	0 in		4	
BCDL: 10	Lumber D.O.L.:	115 %							

02/17/2026

STATE OF MISSOURI
ANISH KEKRE
NUMBER PE-2024044263
PROFESSIONAL ENGINEER

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
4	1	3.5 in	1.50 in	203 lbs	·	-29 lbs	-208 lbs	-208 lbs	·
3	1	3.5 in	1.50 in	203 lbs	·	-11 lbs	-143 lbs	-143 lbs	-92 lbs

Material

TC: SYP#1 2 x 4
BC: SYP#1 2 x 4
Web: SYP#1 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (20 psf) sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has not been designed for the effects of unbalanced snow loads.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
4) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

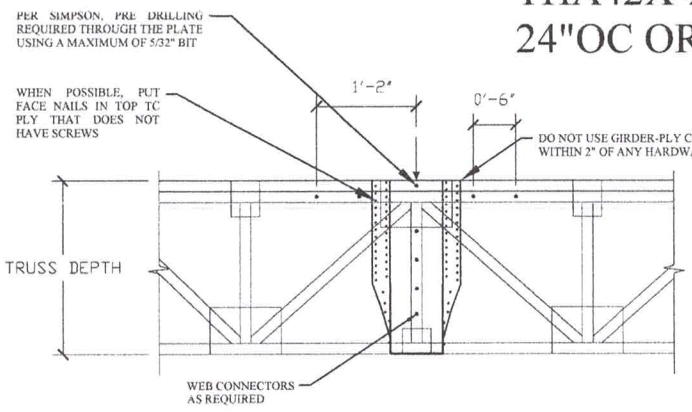
TC				
BC				
Web				

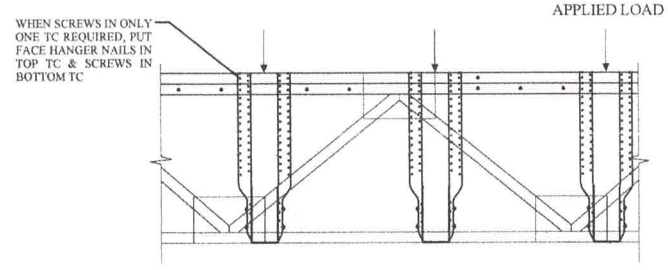
Notes

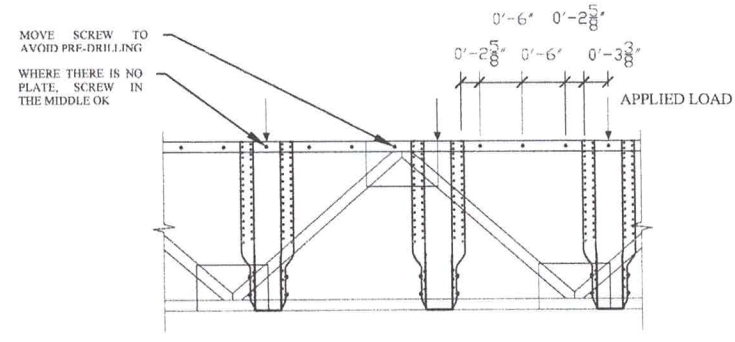
1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Nailing schedule shall be specified by truss manufacturer per NDS.
4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) A creep factor of 2.00 has been applied for this truss analysis.
6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
7) Listed wind uplift reactions based on MWFRS & C&C loading.

TrueBuild®Truss Software v5.8.14
Eagle Metal Products

THA42X-2/THA4XX WITH TRUSSES @ 24"OC OR AN ISOLATED CONNECTION



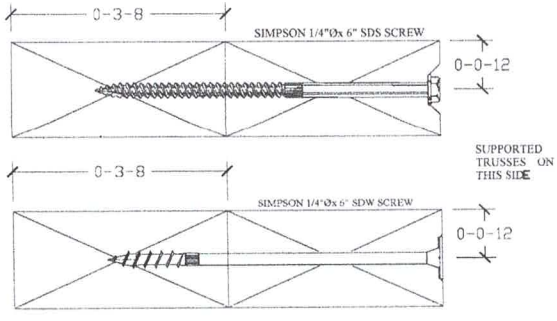




1. TO TRANSFER UNIFORM LOADS APPLIED TO SIMPLY SUPPORTED SPANS ON ASSEMBLY TOP CHORD:
   - 1.1. SPACE SCREWS AS REQUIRED TO TRANSFER HALF THE LOAD INTO THE SUPPORTING TRUSS.
   - 1.2. MINIMUM SCREW SPACING SHALL BE 4" O.C.
2. TO TRANSFER CONCENTRATED LOADS APPLIED TO SIMPLY SUPPORTED SPANS ON AN ASSEMBLY TOP CHORD OR VERTICAL WEB:
   - 2.1. CONCENTRATED LOADS MUST BE APPLIED AT THE PANEL JOINTS.
   - 2.2. SCREWS TO BE INSTALLED WITHIN 12" OF THE CONCENTRATED LOAD ON TOP-CHORD ASSEMBLY
3. GAP BETWEEN THE TRUSSES SHALL NOT EXCEED 1/8" O.C.
4. FLOOR SHEATHING SHALL BE SCREWED OR NAILED TO EACH TOP-CHORD PLY. (FASTENER SPACING PER THE APPLICABLE CODE REQUIREMENTS, OR 12" O.C.)
5. SDW SCREWS SHALL NOT BE INSTALLED IN AREAS WHERE LUMBER WANE EXCEEDS 1/4".
6. HANGERS ON SKEWED GIRDERS:
   - 6.1. HANGER LOADS NOT EXCEEDING 34" O.C. ON A SKEWED GIRDER (RESULTING FROM UNIFORMLY SPACED JOISTS UP TO 24" O.C.) MAY BE CONVERTED TO A UNIFORM LOAD.
   - 6.2. OR GIRDERS WITH HANGER LOAD SPACING IN EXCESS OF 34" O.C. THE LOADS SHALL BE CONSIDERED AS CONCENTRATED LOADS AT THE APPLICABLE LOCATIONS.
7. OTHER CONFIGURATIONS ACCEPTABLE AS LONG AS APPROVED BY TRUSS DESIGN ENGINEER.
8. CONNECTION HAS NO UP-LIFT CAPACITY

TRUSS DEPTH	TOTAL # SCREWS INTO TC (s)	AVAILABLE WEB SCREWS @ 4"OC	# WEB(S)	TOTAL SCREWS	SPF/HF CAPACITY	SP/DFL CAPACITY
1-0-0	3	1	0	3	1,200	1,680
1-0-0	3	1	1	4	1,600	2,240
1-0-0	3	1	2	5	2,000	2,800
1-0-0	3	1	3	6	2,400	3,360
1-0-0	6*	1	0	6*	2,400	3,360
1-2-0	3	1	0	3	1,200	1,680
1-2-0	3	1	1	4	1,600	2,240
1-2-0	3	1	2	5	2,000	2,800
1-2-0	3	1	3	6	2,400	3,360
1-2-0	6*	1	0	6*	2,400	3,360
1-4-0	3	2	0	3	1,200	1,680
1-4-0	3	2	1	5	2,000	2,800
1-4-0	3	2	2	7	2,800	3,920
1-4-0	6*	2	0	6*	2,400	3,360
1-6-0	3	3	0	3	1,200	1,680
1-6-0	3	3	1	6	2,400	3,360
1-6-0	6*	3	0	6*	2,800	3,360
1-8-0	3	3	0	3	1,200	1,680
1-8-0	3	3	1	6	2,400	3,360
1-8-0	6*	3	0	6*	2,400	3,360
1-10-0	3	3	0	3	1,200	1,680
1-10-0	3	3	1	6	2,400	3,360
1-10-0	6*	3	0	6*	2,400	3,360
2-0-0	3	4	0	3	1,200	1,680
2-0-0	3	4	1	7	2,800	3,920
2-0-0	6*	4	0	6*	2,400	3,360

* = DOUBLE TOP CHORD REQUIRED



EAGLE METAL

MULTI-PLY GIRDER CONNECTOR DETAIL

REV: 2.0	DRAWING NUMBER
ENG: MDV	DF-500
CAD: EEM	
DATE: 07/12/16	

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