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01/12/2026



Site Information:	Page 1:
Customer: Kodiak – Premier Building Supply	Job Number: PM000117
Job Description: Customer - Clayton Properties Plan Name - Honeydew Elevation - Farmhouse 2Car	
Address: MO 1433 SE Vantage Dr, Lee's Summit, MO	

Job Engineering Criteria:		
Building Code: IRC 2018	Design Standard: TPI 2014	IntelliVIEW Version: 25.02.00B
Loading Standard: ASCE 7-16	Design Methodology: ASD	JRef #: 1YGQ96460001
Wind: Wind Speed (mph): 115	Exposure: B	Design Loading (psf): 45
Building Type: Part Enc.	Risk Category: II	
Snow: Pg (psf): NA	Pf(ASD) (psf): NA	
W2::	Risk Category: II	

This package contains general notes pages, 29 truss drawing(s) and 7 detail(s).

Item	Drawing Number	Truss	Item	Drawing Number	Truss
1	012.26.1215.03923	LAY1	2	012.26.1214.59703	J2
3	012.26.1214.41710	D2	4	012.26.1214.58390	J1
5	012.26.1214.57137	G2	6	012.26.1215.13983	V6
7	012.26.1214.46267	E2	8	012.26.1214.50680	E5
9	012.26.1214.38430	C4	10	012.26.1214.39900	D1
11	012.26.1214.32750	C2	12	012.26.1215.15287	V7
13	012.26.1215.16560	V8	14	012.26.1215.22470	V9
15	012.26.1215.06590	V10	16	012.26.1215.05330	V1
17	012.26.1215.08330	V2	18	012.26.1215.09757	V3
19	012.26.1215.11410	V4	20	012.26.1215.12680	V5
21	012.26.1214.34783	C3	22	012.26.1214.48887	E4
23	012.26.1214.44083	E1	24	012.26.1214.55483	G1
25	012.26.1214.23680	B2	26	012.26.1215.01260	J3
27	012.26.1214.25727	C1	28	012.26.1214.17143	B1
29	012.26.1215.02583	J3	30	A14015ENC160118	
31	GBLLETIN0118		32	CNNAILSP1014	
33	A14030ENC160118		34	BRCLBSUB0119	
35	HIPFRAME0623		36	VAL180160118	

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General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high-quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed, and detailed by the Building Designer.

Connector Plate Information:

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Bearing Information:

The bearing area factor, C_b , is considered for the allowable capacity of solid sawn wood bearings supporting trusses that are located a minimum of 3" from the end of the lumber piece.

General Notes (continued)

Coated Lumber:

Coated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Coated lumber has no adjustments to lumber properties. Coated lumber may be more brittle than uncoated lumber. Special handling care must be taken to prevent breakage during all handling activities. Refer to manufacturer literature, specifications, and code evaluation reports for restrictions, details, and requirements.

Fire Retardant Treated Lumber:

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

Key to Terms:

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

C = Coated lumber.

C-AT = AtTEK coated lumber.

C-FX = FX Lumber Guard coated lumber.

C-TE = TechWood 4400 coated lumber.

CL = Certified lumber.

Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-BF = Boraflame Fire Retardant Treated lumber

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-ON = OnWood Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

FRT-PR = ProWood Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for all load cases.

Max Web CSI = Maximum bending and axial Combined Stress Index for Webs for all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

General Notes (continued)

Key to Terms (continued):

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.

W = Width of non-hanger bearing, in inches.

Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

References:

1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 155 Harlem Ave, North Building, 4th Floor, Glenview, IL 60025; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www.sbcacomponents.com

SEQN: 5734	HIP_	Ply: 1	Job Number: PM000117	Cust: R 9646 JRef:1YGQ96460001 T1
FROM:		Qty: 1	Customer - Clayton Properties Plan Name - Honeydew Elevation - Farmhouse 2Car	DrwNo: 012.26.1215.03923
			Truss Label: LAY1	/ BM 01/12/2026

Lumber

Plating Notes

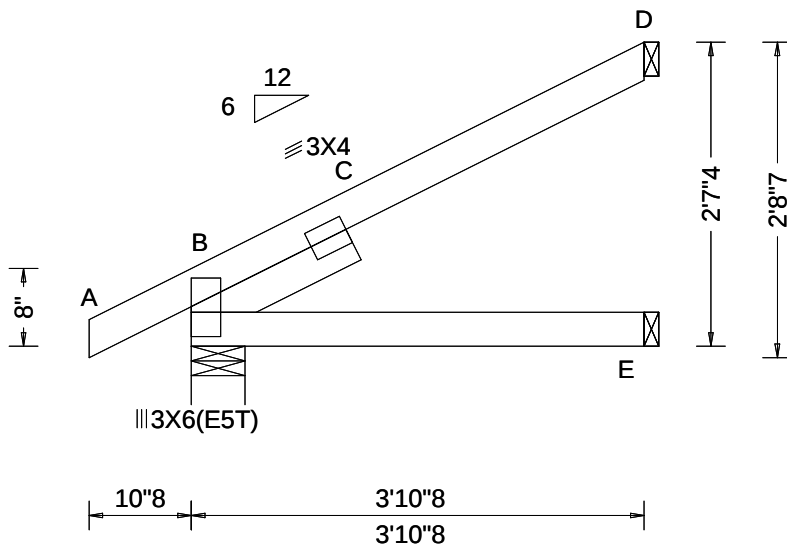
Additional Notes



****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!
****IMPORTANT**** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS
 Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Components Safety Information), by BCSI and SBCA, for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI, unless otherwise noted, and shall install temporary bracing, sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have continuous lateral restraint (CLR) installed with diagonal bracing installed on the CLR per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to Job's General Notes page for additional information.
 Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.
 For more information see these web sites: Alpine: alpineitw.com; TPI: tointst.org; SBCA: sbaccomponents.com; ICC: iccsafe.org; AWC: awc.org

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SEQN: 5719 FROM:	EJAC Ply: 1 Qty: 11	Job Number: PM000117 Customer - Clayton Properties Plan Name - Honeydew Elevation - Farmhouse 2Car Truss Label: J2	Cust: R 9646 JRef: 1YGQ96460001 T10 DrwNo: 012.26.1214.59703 / BM 01/12/2026
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 25.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 0.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCDL: 6.0 psf BCDL: 6.0 psf Mean Height: 20.00 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.55 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.017 C - - HORZ(TL): 0.026 C - - Creep Factor: 2.0 Max TC CSI: 0.316 Max BC CSI: 0.154 Max Web CSI: 0.083 VIEW Ver: 25.02.00B.1125.14	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 249 /- /- /112 /- /67 E 47 /- /- /42 /- /- D 132 /- /- /76 /45 /- Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.5 (Support) E Brg Wid = 1.5 Min Req = - D Brg Wid = 1.5 Min Req = - Bearing B Fcperp = 425psi. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp.

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Lt Slider: 2x4 SP #2; block length = 1.500'

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Snow

Overhang designed for 2.00X TC LL.

Additional Notes

Top Chord overhang(s) may be field trimmed.

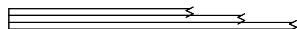


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01/12/2026

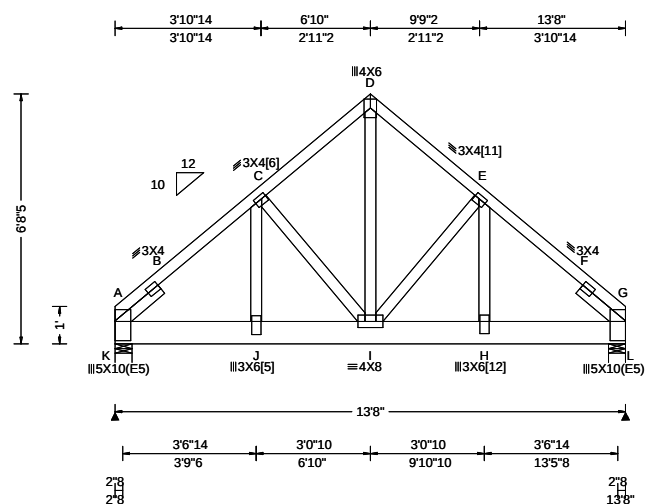
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For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbacompnents.com; ICC: iccsafe.org; AWC: awc.org



SEQN: 5713 FROM:	COMN Ply: 3 Qty: 1	Job Number: PM000117 Customer - Clayton Properties Plan Name - Honeydew Elevation - Farmhouse 2Car Truss Label: D2	Cust: R 9646 JRef: 1YQG96460001 T11 DrwNo: 012.26.1214.41710 / BM 01/12/2026
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3 Complete Trusses Required



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 25.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 0.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCDL: 6.0 psf BCDL: 6.0 psf Mean Height: 22.43 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.55 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.035 I 999 240 VERT(CL): 0.063 I 999 180 HORZ(LL): 0.014 C - - HORZ(TL): 0.024 C - - Creep Factor: 2.0 Max TC CSI: 0.158 Max BC CSI: 0.406 Max Web CSI: 0.480 VIEW Ver: 25.02.00B.1125.14	Gravity Loc R+ / R- / Rh / Rw / U / RL K 5981 /- /- /- /449 /- L 5724 /- /- /- /431 /- Wind reactions based on MWFRS K Brg Wid = 5.5 Min Req = 2.9 (Support) L Brg Wid = 5.5 Min Req = 2.8 (Support) Bearings K & L Fcperp = 425psi. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 173 -2266 D - E 146 -1622 B - C 172 -2247 E - F 170 -2228 C - D 146 -1622 F - G 172 -2247

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x8 SP #1;
Webs: 2x4 SP #2;
Lt Slider: 2x4 SP #2; block length = 1.500'
Rt Slider: 2x4 SP #2; block length = 1.500'

Nailnote

Nail Schedule: 0.128"x3", min. nails
Top Chord: 1 Row @ 12.00" o.c.
Bot Chord: 2 Rows @ 3.50" o.c. (Each Row)
Webs: 1 Row @ 4" o.c.
Repeat nailing as each layer is applied. Use equal spacing between rows and stagger nails in each row to avoid splitting.

Special Loads

----- (Lumber Dur.Fac.=1.15 / Plate Dur.Fac.=1.15)
TC: From 76 plf at 0.00 to 76 plf at 13.67
BC: From 10 plf at 0.00 to 10 plf at 13.67
BC: 1755 lb Conc. Load at 1.67, 3.67, 5.67, 7.67
9.67, 11.67

Plate Shift Table

JT No	Plate Size	Lateral Shift	Chord Bite	JT No	Plate Size	Lateral Shift	Chord Bite
[5]	3X6	S	4.25	[6]	3X4	2.50	L 1.25
[11]	3X4	2.50	R 1.25	[12]	3X6	S	4.00

Wind

Wind loads and reactions based on MWFRS.
Wind loading based on both gable and hip roof types.

Blocking

Apply additional nailing over the following bearings with fasteners at 9" oc perpendicular to grain and 4" oc parallel to grain. In lieu of additional nailing, apply blocking reinforcement to prevent buckling of members over the bearings:
Bearing 1 located at 0.0' (blocking >= 3.50" if used)

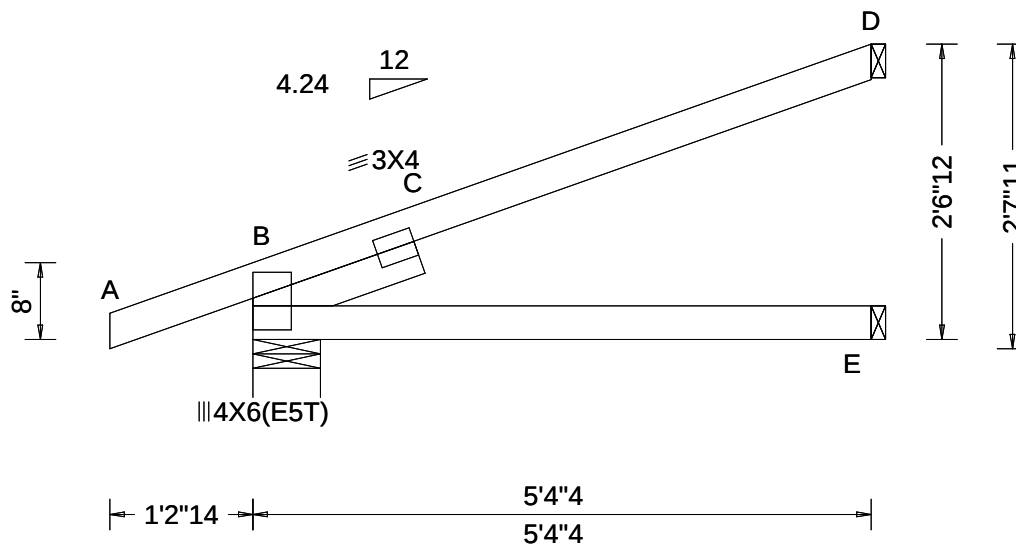


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SEQN: 5718 FROM:	HIP_	Ply: 1 Qty: 2	Job Number: PM000117 Customer - Clayton Properties Plan Name - Honeydew Elevation - Farmhouse 2Car Truss Label: J1	Cust: R 9646 JRef:1YGQ96460001 T12 DrwNo: 012.26.1214.58390 / BM 01/12/2026
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 25.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 0.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCDL: 6.0 psf BCDL: 6.0 psf Mean Height: 19.98 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.55 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.015 C - - HORZ(TL): 0.028 C - - Creep Factor: 2.0 Max TC CSI: 0.506 Max BC CSI: 0.246 Max Web CSI: 0.048 VIEW Ver: 25.02.00B.1125.14	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 144 /- /- /- /20 /- E 30 /- /- /- /7 /- /- D 61 /- /- /- /13 /- Wind reactions based on MWFRS B Brg Wid = 7.0 Min Req = 1.5 (Support) E Brg Wid = 1.5 Min Req = - D Brg Wid = 1.5 Min Req = - Bearing B Fcperp = 425psi. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Lt Slider: 2x4 SP #2; block length = 1.500'

Special Loads

----- (Lumber Dur.Fac.=1.15 / Plate Dur.Fac.=1.15)
TC: From 0 plf at -1.24 to 71 plf at 0.00
TC: From 2 plf at 0.00 to 2 plf at 5.36
BC: From 2 plf at 0.00 to 2 plf at 5.36
TC: 128 lb Conc. Load at 2.60
BC: 38 lb Conc. Load at 2.60

Wind

Wind loads and reactions based on MWFRS.
Wind loading based on both gable and hip roof types.

Additional Notes

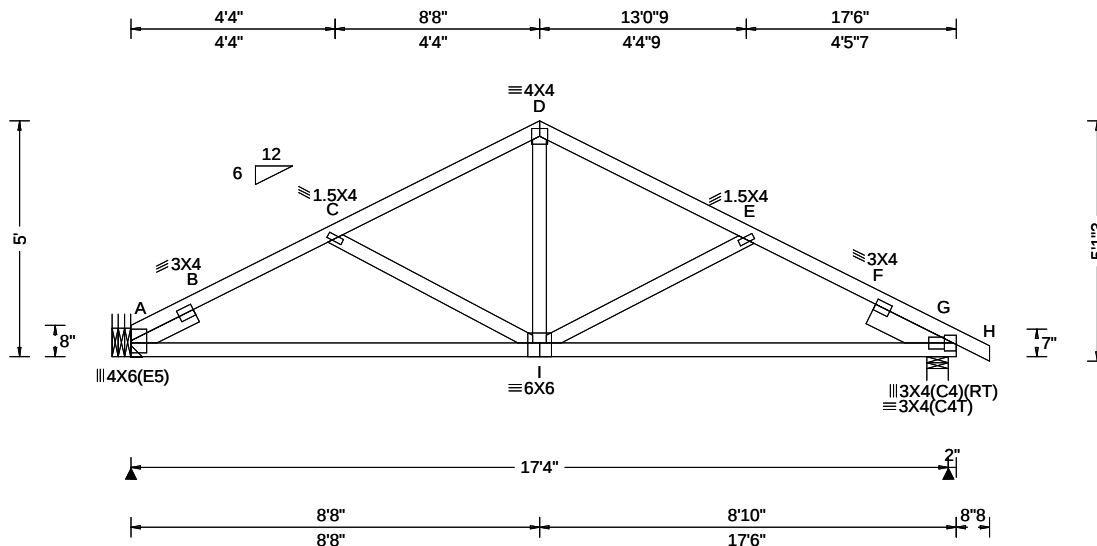
Top Chord overhang(s) may be field trimmed.



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SEQN: 5717 FROM:	COMN Ply: 1 Qty: 2	Job Number: PM000117 Customer - Clayton Properties Plan Name - Honeydew Elevation - Farmhouse 2Car Truss Label: G2	Cust: R 9646 JRef: 1YGQ96460001 T13 DrwNo: 012.26.1214.57137 / BM 01/12/2026
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 25.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCDL: 6.0 psf BCDL: 6.0 psf Mean Height: 21.20 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.55 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.032 I 999 240 VERT(CL): 0.057 I 999 180 HORZ(LL): 0.015 G - - HORZ(TL): 0.030 B - - Creep Factor: 2.0 Max TC CSI: 0.347 Max BC CSI: 0.559 Max Web CSI: 0.153 VIEW Ver: 25.02.00B.1125.14	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 789 /- /- /449 /53 /65 G 880 /- /- /474 /58 /- Wind reactions based on MWFRS A Brg Wid = - Min Req = - G Brg Wid = 5.5 Min Req = 1.5 (Support) Bearing G Fcperp = 425psi. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 447 -1260 D - E 240 -931 B - C 310 -1215 E - F 299 -1176 C - D 242 -934 F - G 451 -1297

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;
Lt Slider: 2x4 SP #2; block length = 1.500'
Rt Slider: 2x6 SP #2; block length = 1.833'

Hangers / Ties
Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.
Recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information. Additional connection required to evenly distribute hanger reaction throughout all plies of supporting girder.
Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.
Bearing at location x=0' uses the following support conditions: 0'
Bearing A (0', 18'7") HUS28
Supporting Member: (3)2x8 SP 2400f-2.0E
(22) 0.148"x3" nails into supporting member,
(4) 0.148"x3" nails into supported member.

Loading
Bottom chord checked for 10.00 psf non-concurrent live load.

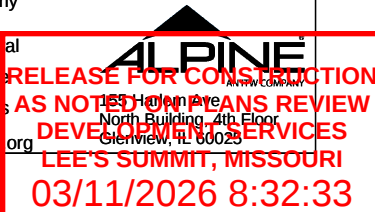
Wind
Wind loads based on MWFRS with additional C&C member design.
Right cantilever is exposed to wind
Wind loading based on both gable and hip roof types.

Snow
Overhang designed for 2.00X TC LL.

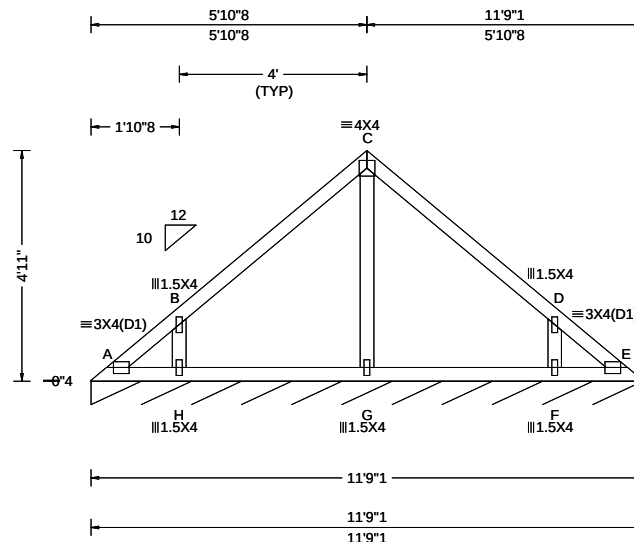
Additional Notes
Top Chord overhang(s) may be field trimmed.



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SEQN: 5728 FROM:	VAL Qty: 1	Ply: 1 Qty: 1	Job Number: PM000117 Customer - Clayton Properties Plan Name - Honeydew Elevation - Farmhouse 2Car Truss Label: V6	Cust: R 9646 JRef:1YGQ96460001 T14 DrwNo: 012.26.1215.13983 / BM 01/12/2026
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 25.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCDL: 6.0 psf BCDL: 6.0 psf Mean Height: 22.97 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.55 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.001 C 999 240 VERT(CL): 0.001 C 999 180 HORZ(LL): -0.001 A - - HORZ(TL): 0.002 B - - Creep Factor: 2.0 Max TC CSI: 0.275 Max BC CSI: 0.119 Max Web CSI: 0.068 VIEW Ver: 25.02.00B.1125.14	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL E* 96 /- /- /53 /4 /7 Wind reactions based on MWFRS E Brg Wid = 141 Min Req = - Bearing A Fcperp = 425psi. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;

Loading

Bottom chord checked for 10.00 psf non-concurrent live load.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

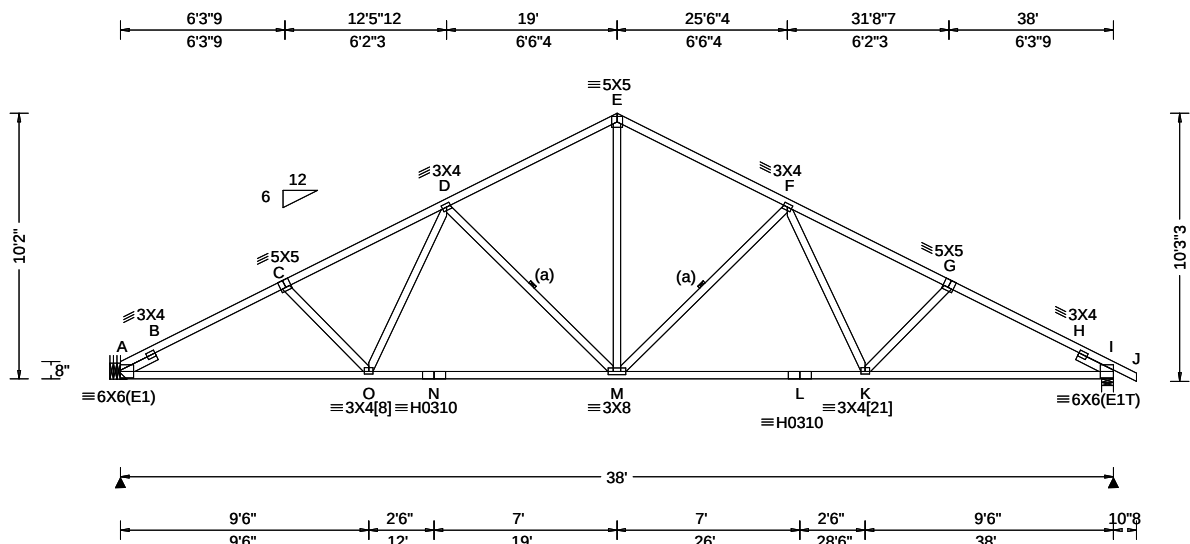
See DWG VAL180160118 for valley details.



MISSOURI COA #2005000817
01/12/2026

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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 25.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCDL: 6.0 psf BCDL: 6.0 psf Mean Height: 23.78 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.80 ft Loc. from endwall: not in 4.50 ft GCpi: 0.55 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS	PP Deflection in loc L/defl L/# VERT(LL): 0.158 M 999 364 VERT(CL): 0.294 M 999 364 HORZ(LL): 0.055 I - - HORZ(TL): 0.100 I - - Creep Factor: 2.0 Max TC CSI: 0.737 Max BC CSI: 0.509 Max Web CSI: 0.311 VIEW Ver: 25.02.00B.1125.14	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 1755 /- /- /1013 /117 /145 I 1819 /- /- /1022 /123 /- Wind reactions based on MWFRS A Brg Wid = - Min Req = - I Brg Wid = 5.5 Min Req = 2.3 (Support) Bearing I Fcperp = 425psi. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 510 -3160 E - F 278 -2040 B - C 266 -3113 F - G 264 -2848 C - D 266 -2848 G - H 265 -3113 D - E 279 -2040 H - I 482 -3195

Lumber Top chord: 2x4 SP #2; Bot chord: 2x4 SP 2400f-2.0E; Webs: 2x4 SP #2; Lt Slider: 2x4 SP #2; block length = 1.500' Rt Slider: 2x4 SP #2; block length = 1.500'	Additional Notes Top Chord overhang(s) may be field trimmed.																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																																			
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Loading
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.
Bottom chord checked for 10.00 psf non-concurrent live load.
Live loads applied in combination per ASCE 7 sec. 2.4.1 use 0.75 factor for multiple live loads.

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Snow
Overhang designed for 2.00X TC LL.



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SEQN: 5714	SPEC	Ply: 1	Job Number: PM000117	Cust: R 9646 JRef: 1YGQ96460001 T15
FROM:		Qty: 4	Customer - Clayton Properties Plan Name - Honeydew Elevation - Farmhouse 2Car	DrwNo: 012.26.1214.46267
Page 2 of 2			Truss Label: E2	/ BM 01/12/2026

Hangers / Ties

Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.

Recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information. Additional connection required to evenly distribute hanger reaction throughout all plies of supporting girder.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

Bearing at location x=0' uses the following support conditions: 0'

Bearing A (0', 18'7") HUS28

Supporting Member: (3)2x8 SP 2400f-2.0E

(22) 0.148"x3" nails into supporting member,

(4) 0.148"x3" nails into supported member.



MISSOURI COA #2005000817
01/12/2026

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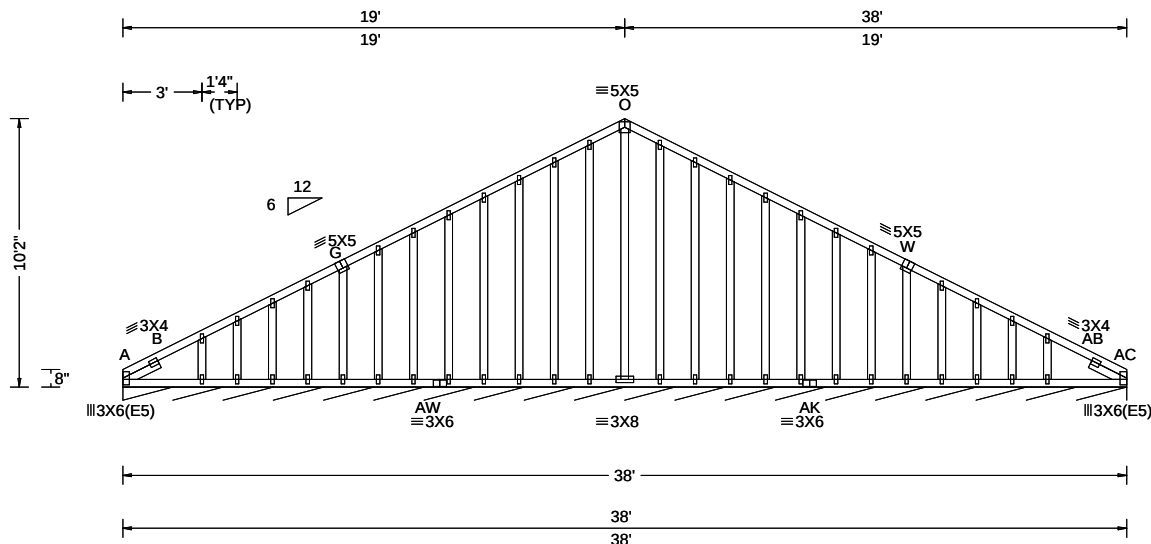
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ALPINE
1851 Horton Ave
North Building, 4th Floor
Glenview, IL 60025
RELEASE FOR CONSTRUCTION
AS NOTED BY ENGINEERS REVIEW
DEVELOPMENT SERVICES
LEE'S SUMMIT, MISSOURI
03/11/2026 8:32:33

SEQN: 5733 FROM:	GABL Ply: 1 Qty: 1	Job Number: PM000117 Customer - Clayton Properties Plan Name - Honeydew Elevation - Farmhouse 2Car Truss Label: E5	Cust: R 9646 JRef: 1YQG96460001 T16 DrwNo: 012.26.1214.50680 / BM 01/12/2026
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 25.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCDL: 6.0 psf BCDL: 6.0 psf Mean Height: 24.00 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.80 ft Loc. from endwall: Any GCpi: 0.55 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.011 B 999 364 VERT(CL): 0.017 B 999 364 HORZ(LL): 0.006 B - - HORZ(TL): 0.014 AB - - Creep Factor: 2.0 Max TC CSI: 0.221 Max BC CSI: 0.075 Max Web CSI: 0.443 VIEW Ver: 25.02.00B.1125.14	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL AC*127 /- /- /64 /13 /5 Wind reactions based on MWFRS AC Brg Wid = 456 Min Req = - Bearing A Fcperp = 425psi. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. G - O 386 -109 O - W 380 -102

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;
Lt Slider: 2x4 SP #2; block length = 1.500'
Rt Slider: 2x4 SP #2; block length = 1.500'

Plating Notes

All plates are 1.5X4 except as noted.

Loading

Truss designed to support 1-4-0 top chord outlookers and cladding load not to exceed 3.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Bottom chord checked for 10.00 psf non-concurrent live load.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

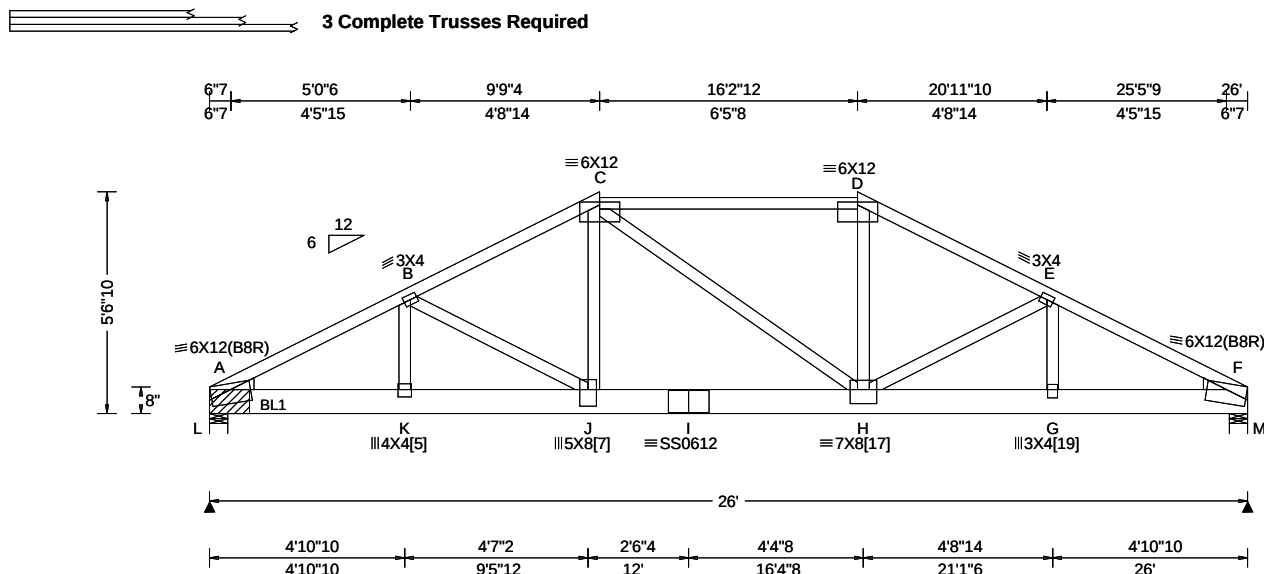


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01/12/2026

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SEQN: 5711 FROM:	HIPS Qty: 1	Ply: 3	Job Number: PM000117 Customer - Clayton Properties Plan Name - Honeydew Elevation - Farmhouse 2Car Truss Label: C4	Cust: R 9646 JRef: 1YQG96460001 T18 DrwNo: 012.26.1214.38430 / BM 01/12/2026
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 25.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 0.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCDL: 6.0 psf BCDL: 6.0 psf Mean Height: 21.69 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.55 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE, 18SS	PP Deflection in loc L/defl L/# VERT(LL): 0.214 H 999 246 VERT(CL): 0.383 H 805 246 HORZ(LL): 0.046 B - - HORZ(TL): 0.082 B - - Creep Factor: 2.0 Max TC CSI: 0.878 Max BC CSI: 0.505 Max Web CSI: 0.575 VIEW Ver: 25.02.00B.1125.14	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL L 11553 -/- /- /- /888 -/- M 10591 -/- /- /- /829 -/- Wind reactions based on MWFRS L Brg Wid = 5.5 Min Req = - M Brg Wid = 5.5 Min Req = 5.1 (Support) Bearings L & M Fcperp = 425psi. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 500 - 6494 D - E 438 - 5691 B - C 442 - 5751 E - F 492 - 6306 C - D 384 - 5036

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x8 SP 2400f-2.0E;
Webs: 2x4 SP #2;
Lt Wedge: 2x4 SP #2; Rt Wedge: 2x4 SP #2;

Nailnote
Nail Schedule: 0.128"x3", min. nails
Top Chord: 1 Row @ 12.00" o.c.
Bot Chord: 2 Rows @ 3.50" o.c. (Each Row)
Webs : 1 Row @ 4" o.c.
Repeat nailing as each layer is applied. Use equal spacing between rows and stagger nails in each row to avoid splitting.

Special Loads
----- (Lumber Dur.Fac.=1.15 / Plate Dur.Fac.=1.15)
TC: From 72 plf at 0.00 to 72 plf at 26.00
BC: From 10 plf at 0.00 to 10 plf at 26.00
BC: 1755 lb Conc. Load at 2.00, 4.00, 6.00, 8.00, 10.00, 12.00, 14.00, 16.00, 18.00, 20.00
BC: 875 lb Conc. Load at 20.94
BC: 789 lb Conc. Load at 22.00, 24.00

Plate Shift Table

JT No	Plate Size	Lateral Shift	Chord Bite	JT No	Plate Size	Lateral Shift	Chord Bite
[5]	4X4	S	2.25	[7]	5X8	S	5.00
[17]	7X8	S	4.25	[19]	3X4	S	2.50

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads and reactions based on MWFRS.
Wind loading based on both gable and hip roof types.

Bearing Block(s)
Brg blocks: 0.128"x3", min. nails
brg x-loc #blocks length/blk #nails/blk wall plate
1 0.000' 1 12" 4 SPF Standard
Brg block to be same size and species as chord.
Refer to drawing CNNAILSP1014 for more information.

Maximum Bot Chord Forces Per Ply (lbs)
Chords Tens.Comp. Chords Tens. Comp.

A - K	5765	-441	I - H	5022	-383
K - J	5744	-440	H - G	5578	-432
J - I	5022	-383	G - F	5595	-433

Maximum Web Forces Per Ply (lbs)
Webs Tens.Comp. Webs Tens. Comp.

K - B	686	-34	D - H	2326	-151
B - J	55	-683	H - E	50	-548
J - C	2344	-153	E - G	557	-29

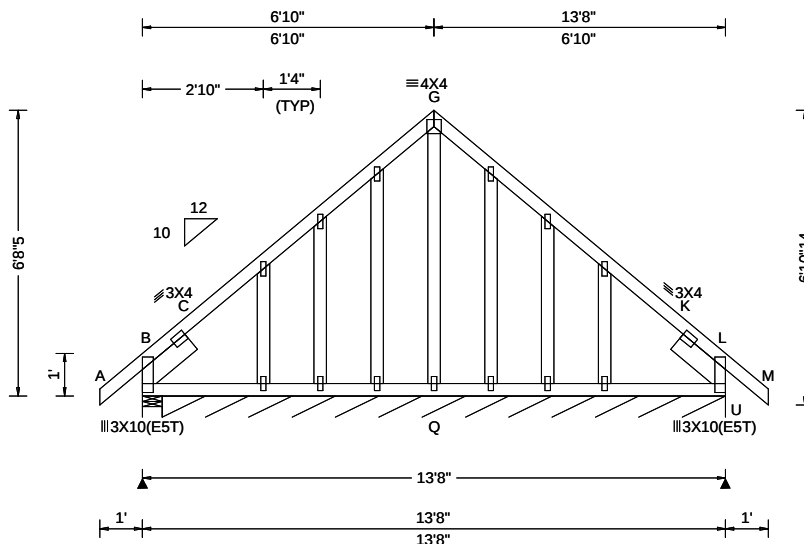


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01/12/2026

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SEQN: 5712 FROM:	GABL Ply: 1 Qty: 1	Job Number: PM000117 Customer - Clayton Properties Plan Name - Honeydew Elevation - Farmhouse 2Car Truss Label: D1	Cust: R 9646 JRef:1YGQ96460001 T19 DrwNo: 012.26.1214.39900 / BM 01/12/2026
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 25.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCDL: 6.0 psf BCDL: 6.0 psf Mean Height: 22.01 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.55 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.005 K 999 240 VERT(CL): 0.012 C 999 180 HORZ(LL): 0.006 C - - HORZ(TL): 0.017 K - - Creep Factor: 2.0 Max TC CSI: 0.146 Max BC CSI: 0.063 Max Web CSI: 0.145 VIEW Ver: 25.02.00B.1125.14	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 355 /- /- /191 /21 /135 U* 121 /- /- /68 /11 /- Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.5 (Support) U Brg Wid = 158 Min Req = - Bearings B & B Fcperp = 425psi. Members not listed have forces less than 375# Maximum Gable Forces Per Ply (lbs) Gables Tens.Comp. G - Q 88 -411

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;
Lt Slider: 2x6 SP #2; block length = 1.500'
Rt Slider: 2x6 SP #2; block length = 1.500'

Plating Notes

All plates are 1.5X4 except as noted.

Loading

Truss designed to support 1-4-0 top chord outlookers and cladding load not to exceed 3.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Bottom chord checked for 10.00 psf non-concurrent live load.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Snow

Overhang designed for 2.00X TC LL.

Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Top Chord overhang(s) may be field trimmed.

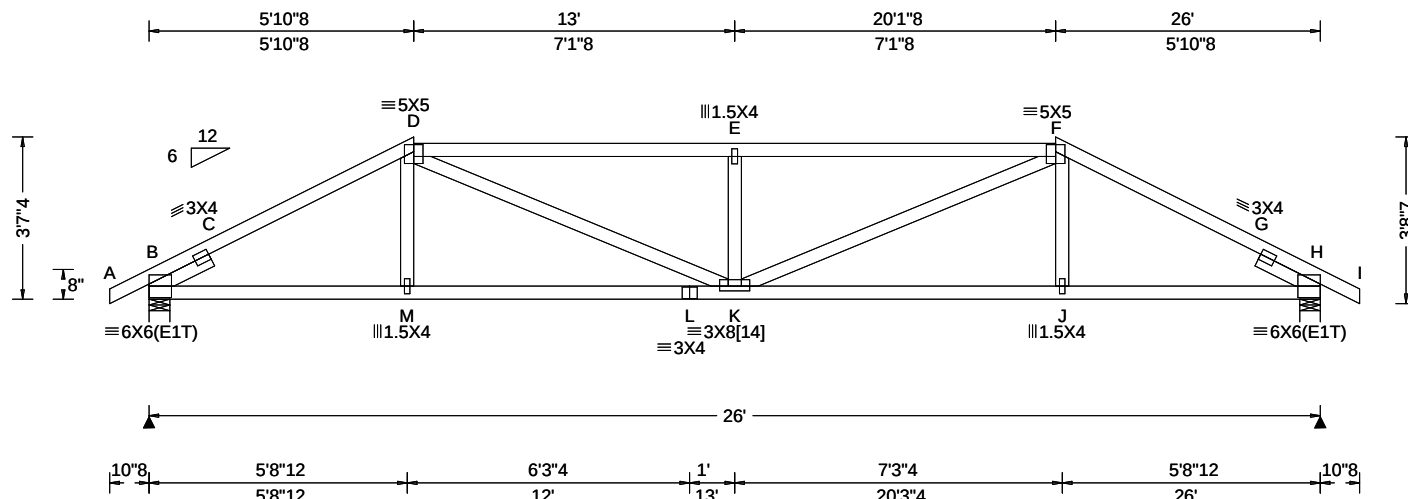


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SEQN: 5709 FROM:	HIPS Ply: 1 Qty: 1	Job Number: PM000117 Customer - Clayton Properties Plan Name - Honeydew Elevation - Farmhouse 2Car Truss Label: C2	Cust: R 9646 JRef: 1YGQ96460001 T2 DrwNo: 012.26.1214.32750 / BM 01/12/2026
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 25.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCDL: 6.0 psf BCDL: 6.0 psf Mean Height: 20.50 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.55 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.140 E 999 249 VERT(CL): 0.260 E 999 249 HORZ(LL): 0.037 H - - HORZ(TL): 0.068 H - - Creep Factor: 2.0 Max TC CSI: 0.858 Max BC CSI: 0.635 Max Web CSI: 0.285 VIEW Ver: 25.02.00B.1125.14	Gravity Loc R+ / R- / Rh / Rw / U / RL B 1265 - / - / - / 678 / 88 / 40 H 1265 - / - / - / 678 / 88 / - Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.6 (Support) H Brg Wid = 5.5 Min Req = 1.6 (Support) Bearings B & H Fcperp = 425psi. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 470 -2138 E - F 684 -2583 C - D 462 -2058 F - G 463 -2058 D - E 684 -2583 G - H 470 -2138

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;
Lt Slider: 2x4 SP #2; block length = 1.500'
Rt Slider: 2x4 SP #2; block length = 1.500'

Plate Shift Table

JT No	Plate Size	Lateral Shift	Chord Bite	JT No	Plate Size	Lateral Shift	Chord Bite
[14]	3X8	S	1.25				

Loading

Bottom chord checked for 10.00 psf non-concurrent live load.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Snow

Overhang designed for 2.00X TC LL.

Additional Notes

Top Chord overhang(s) may be field trimmed.

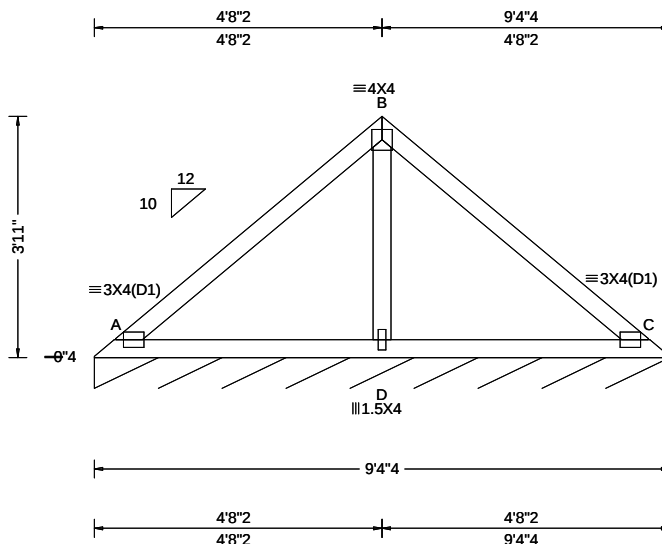


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SEQN: 5729 FROM:	VAL	Ply: 1 Qty: 1	Job Number: PM000117 Customer - Clayton Properties Plan Name - Honeydew Elevation - Farmhouse 2Car Truss Label: V7	Cust: R 9646 JRef: 1YGQ96460001 T21 DrwNo: 012.26.1215.15287 / BM 01/12/2026
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 25.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCDL: 6.0 psf BCDL: 6.0 psf Mean Height: 23.47 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.55 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.012 C 999 240 VERT(CL): 0.021 C 999 180 HORZ(LL): -0.007 C - - HORZ(TL): 0.013 C - - Creep Factor: 2.0 Max TC CSI: 0.387 Max BC CSI: 0.262 Max Web CSI: 0.132 VIEW Ver: 25.02.00B.1125.14	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL C* 96 /- /- /53 /4 /6 Wind reactions based on MWFRS C Brg Wid = 112 Min Req = - Bearing A Fcperp = 425psi. Members not listed have forces less than 375# Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. B - D 191 -565

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;

Loading

Bottom chord checked for 10.00 psf non-concurrent live load.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

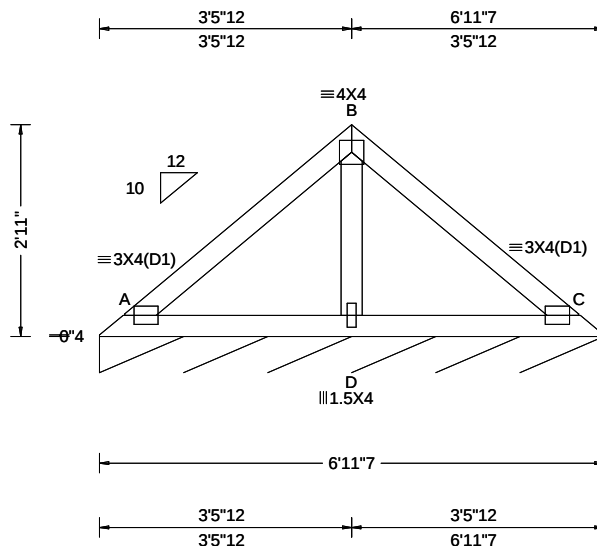
See DWG VAL180160118 for valley details.



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SEQN: 5730 FROM:	VAL Qty: 1	Ply: 1 Qty: 1	Job Number: PM000117 Customer - Clayton Properties Plan Name - Honeydew Elevation - Farmhouse 2Car Truss Label: V8	Cust: R 9646 JRef: 1YGQ96460001 T22 DrwNo: 012.26.1215.16560 / BM 01/12/2026
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 25.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCDL: 6.0 psf BCDL: 6.0 psf Mean Height: 23.97 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.55 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.005 C 999 240 VERT(CL): 0.009 C 999 180 HORZ(LL): -0.003 C - - HORZ(TL): 0.005 C - - Creep Factor: 2.0 Max TC CSI: 0.195 Max BC CSI: 0.133 Max Web CSI: 0.054 VIEW Ver: 25.02.00B.1125.14	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL C* 95 /- /- /52 /4 /6 Wind reactions based on MWFRS C Brg Wid = 83.5 Min Req = - Bearing A Fcperp = 425psi. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;

Loading

Bottom chord checked for 10.00 psf non-concurrent live load.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWG VAL180160118 for valley details.

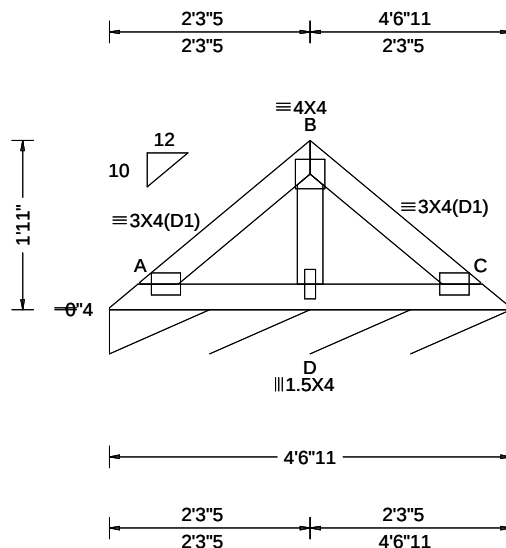


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01/12/2026

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SEQN: 5731 FROM:	VAL	Ply: 1 Qty: 1	Job Number: PM000117 Customer - Clayton Properties Plan Name - Honeydew Elevation - Farmhouse 2Car Truss Label: V9	Cust: R 9646 JRef: 1YQG96460001 T23 DrwNo: 012.26.1215.22470 / BM 01/12/2026
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 25.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 0.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCDL: 6.0 psf BCDL: 6.0 psf Mean Height: 24.47 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.55 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.001 C 999 240 VERT(CL): 0.002 C 999 180 HORZ(LL): -0.001 C - - HORZ(TL): 0.002 C - - Creep Factor: 2.0 Max TC CSI: 0.069 Max BC CSI: 0.048 Max Web CSI: 0.023 VIEW Ver: 25.02.00B.1125.14	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL C* 95 /- /- /50 /3 /6 Wind reactions based on MWFRS C Brg Wid = 54.7 Min Req = - Bearing A Fcperp = 425psi. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWG VAL180160118 for valley details.

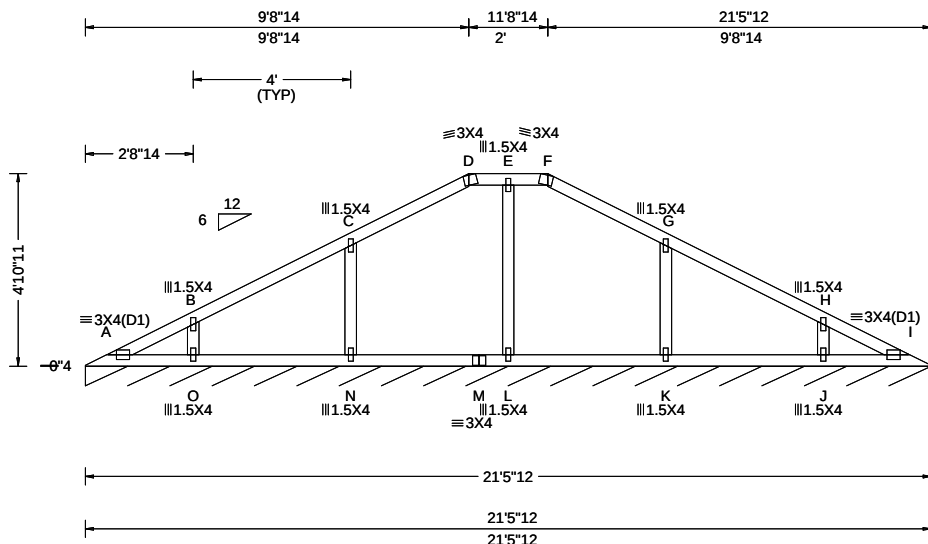


MISSOURI COA #2005000817
01/12/2026

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SEQN: 5722 FROM:	VAL Qty: 1	Job Number: PM000117 Customer - Clayton Properties Plan Name - Honeydew Elevation - Farmhouse 2Car Truss Label: V1	Cust: R 9646 JRef:1YGQ96460001 T25 DrwNo: 012.26.1215.05330 / BM 01/12/2026
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 25.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCDL: 6.0 psf BCDL: 6.0 psf Mean Height: 22.96 ft MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.55 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.006 F 999 240 VERT(CL): 0.009 F 999 203 HORZ(LL): 0.002 G - - HORZ(TL): 0.003 G - - Creep Factor: 2.0 Max TC CSI: 0.204 Max BC CSI: 0.118 Max Web CSI: 0.082 VIEW Ver: 25.02.00B.1125.14	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL P* 92 /- /- /49 /1 /3 Wind reactions based on MWFRS P Brg Wid = 257 Min Req = - Bearing A Fcperp = 425psi. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;

Loading

Bottom chord checked for 10.00 psf non-concurrent live load.

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWG VAL180160118 for valley details.

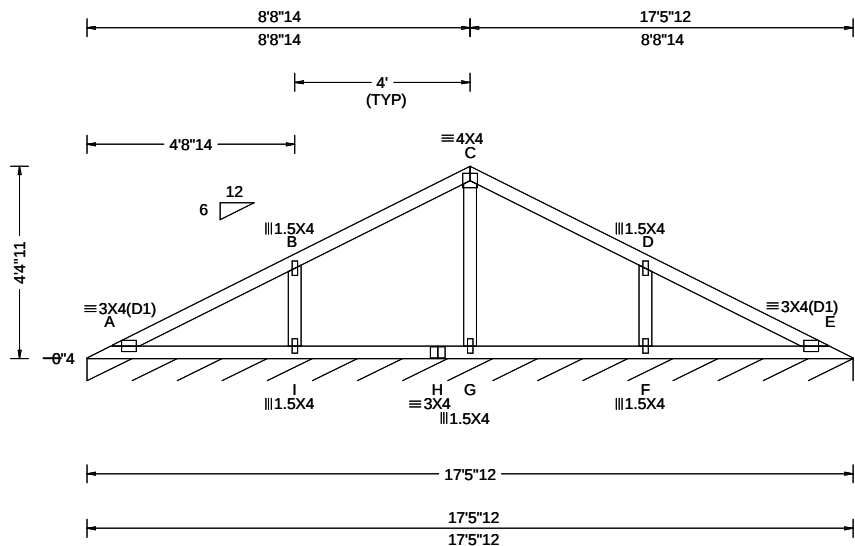


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01/12/2026

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SEQN: 5724 FROM:	VAL Ply: 1 Qty: 1	Job Number: PM000117 Customer - Clayton Properties Plan Name - Honeydew Elevation - Farmhouse 2Car Truss Label: V2	Cust: R 9646 JRef: 1YGQ96460001 T26 DrwNo: 012.26.1215.08330 / BM 01/12/2026
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 25.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCDL: 6.0 psf BCDL: 6.0 psf Mean Height: 23.71 ft MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.55 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.012 E 999 240 VERT(CL): 0.023 A 999 180 HORZ(LL): -0.004 E - - HORZ(TL): 0.008 E - - Creep Factor: 2.0 Max TC CSI: 0.426 Max BC CSI: 0.189 Max Web CSI: 0.100 VIEW Ver: 25.02.00B.1125.14	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL J* 92 /- /- /49 /1 /3 Wind reactions based on MWFRS J Brg Wid = 209 Min Req = - Bearing A Fcperp = 425psi. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;

Loading

Bottom chord checked for 10.00 psf non-concurrent live load.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWG VAL180160118 for valley details.

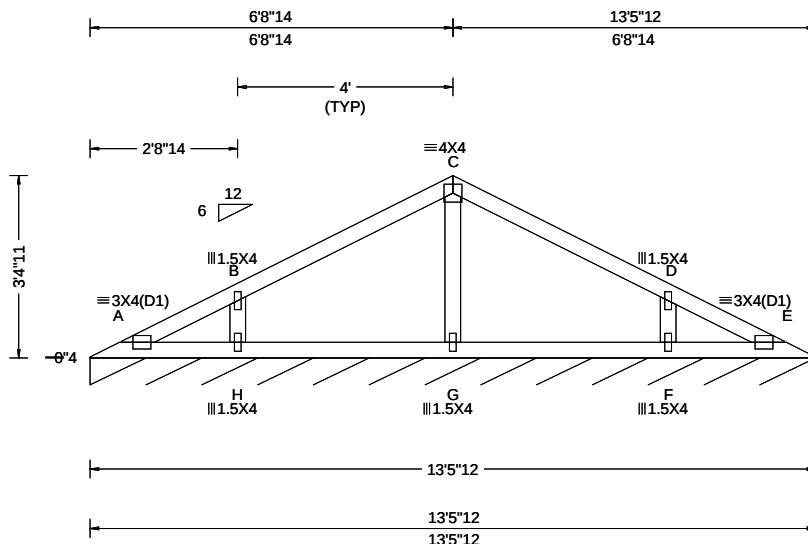


MISSOURI COA #2005000817
01/12/2026

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SEQN: 5725 FROM:	VAL Qty: 1	Job Number: PM000117 Customer - Clayton Properties Plan Name - Honeydew Elevation - Farmhouse 2Car Truss Label: V3	Cust: R 9646 JRef: 1YGQ96460001 T27 DrwNo: 012.26.1215.09757 / BM 01/12/2026
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 25.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCDL: 6.0 psf BCDL: 6.0 psf Mean Height: 24.21 ft MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.55 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.001 C 999 240 VERT(CL): 0.001 E 999 180 HORZ(LL): -0.000 A - - HORZ(TL): 0.001 B - - Creep Factor: 2.0 Max TC CSI: 0.252 Max BC CSI: 0.112 Max Web CSI: 0.044 VIEW Ver: 25.02.00B.1125.14	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL I* 92 /- /- /49 /1 /3 Wind reactions based on MWFRS I Brg Wid = 161 Min Req = - Bearing A Fcperp = 425psi. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;

Loading

Bottom chord checked for 10.00 psf non-concurrent live load.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWG VAL180160118 for valley details.

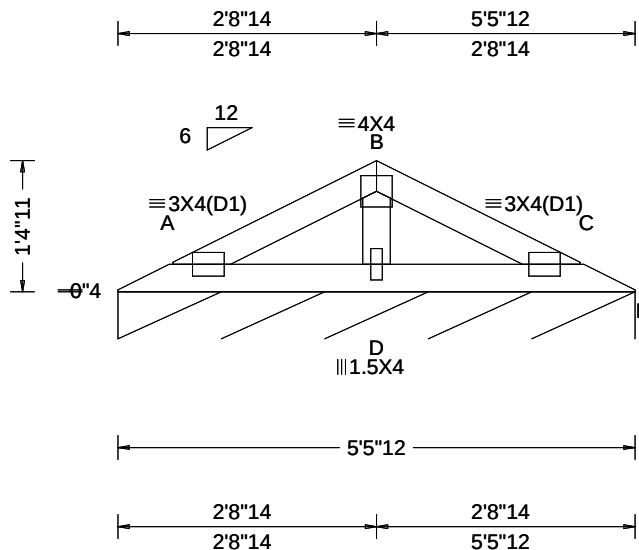


MISSOURI COA #2005000817
01/12/2026

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SEQN: 5727 FROM:	VAL	Ply: 1 Qty: 1	Job Number: PM000117 Customer - Clayton Properties Plan Name - Honeydew Elevation - Farmhouse 2Car Truss Label: V5	Cust: R 9646 JRef: 1YGQ96460001 T29 DrwNo: 012.26.1215.12680 / BM 01/12/2026
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 25.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 0.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCDL: 6.0 psf BCDL: 6.0 psf Mean Height: 25.21 ft MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.55 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.003 C 999 240 VERT(CL): 0.005 C 999 180 HORZ(LL): -0.001 C - - HORZ(TL): 0.002 C - - Creep Factor: 2.0 Max TC CSI: 0.091 Max BC CSI: 0.076 Max Web CSI: 0.025 VIEW Ver: 25.02.00B.1125.14	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL E* 92 /- /- /45 /- /3 Wind reactions based on MWFRS E Brg Wid = 65.8 Min Req = - Bearing A Fcperp = 425psi. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWG VAL180160118 for valley details.

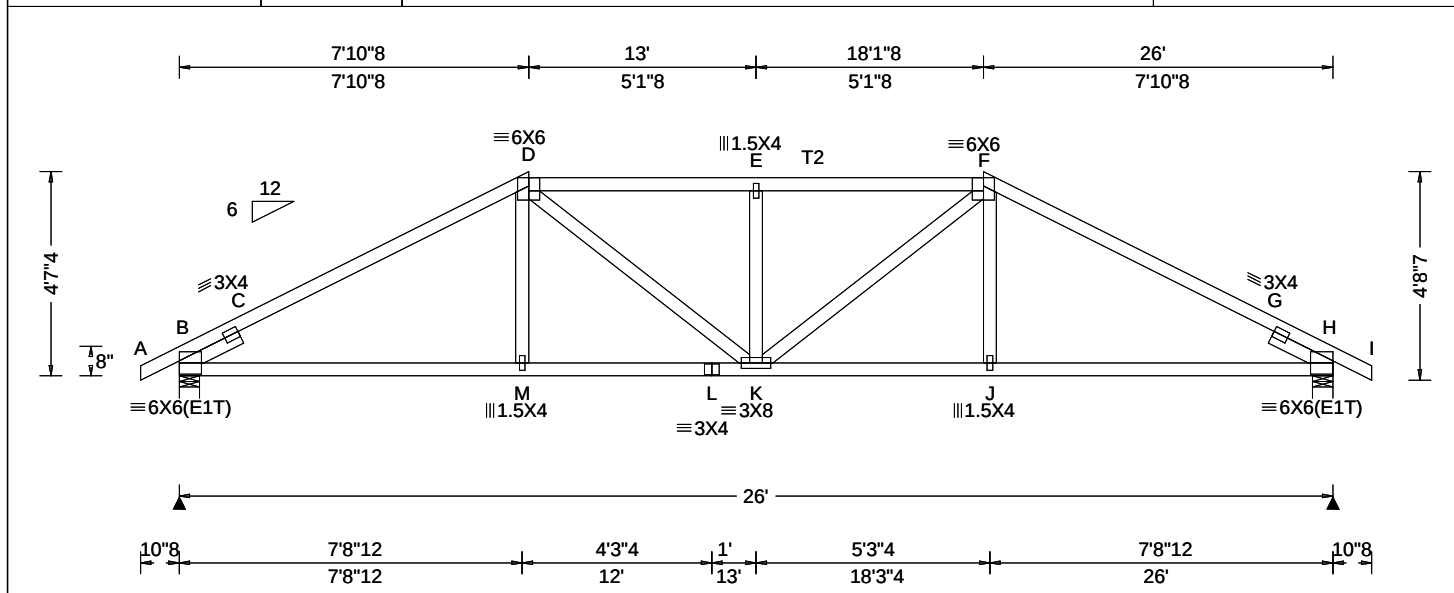


MISSOURI COA #2005000817
01/12/2026

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RELEASE FOR CONSTRUCTION
AS NOTED BY INSPECTION REVIEW
DEVELOPMENT SERVICES
LEE'S SUMMIT, MISSOURI
03/11/2026 8:32:34

SEQN: 5710 FROM:	HIPS Qty: 1	Ply: 1 Qty: 1	Job Number: PM000117 Customer - Clayton Properties Plan Name - Honeydew Elevation - Farmhouse 2Car Truss Label: C3	Cust: R 9646 JRef: 1YGQ96460001 T3 DrwNo: 012.26.1214.34783 / BM 01/12/2026
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 25.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCDL: 6.0 psf BCDL: 6.0 psf Mean Height: 21.00 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.55 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.123 C 999 249 VERT(CL): 0.197 C 999 249 HORZ(LL): 0.066 C - - HORZ(TL): 0.105 C - - Creep Factor: 2.0 Max TC CSI: 0.719 Max BC CSI: 0.764 Max Web CSI: 0.191 VIEW Ver: 25.02.00B.1125.14	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity B 1265 /- /- /686 /92 /53 H 1265 /- /- /686 /92 /- Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.6 (Support) H Brg Wid = 5.5 Min Req = 1.6 (Support) Bearings B & H Fcperp = 425psi. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 615 -2160 E - F 481 -1891 C - D 397 -1943 F - G 397 -1943 D - E 481 -1891 G - H 615 -2160

Lumber
Top chord: 2x4 SP 2400f-2.0E; T2 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;
Lt Slider: 2x4 SP #2; block length = 1.500'
Rt Slider: 2x4 SP #2; block length = 1.500'

Loading
Bottom chord checked for 10.00 psf non-concurrent live load.

Purlins
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

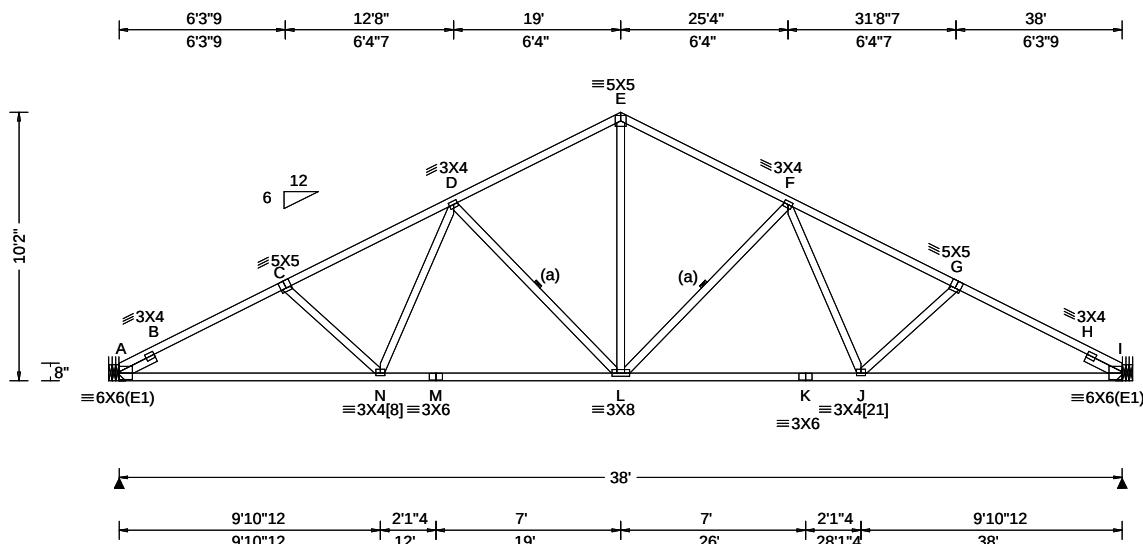
Snow
Overhang designed for 2.00X TC LL.

Additional Notes
Top Chord overhang(s) may be field trimmed.



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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 25.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCDL: 6.0 psf BCDL: 6.0 psf Mean Height: 24.00 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.80 ft Loc. from endwall: Any GCpi: 0.55 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.159 L 999 364 VERT(CL): 0.294 L 999 364 HORZ(LL): 0.055 I - - HORZ(TL): 0.100 I - - Creep Factor: 2.0 Max TC CSI: 0.764 Max BC CSI: 0.498 Max Web CSI: 0.415 VIEW Ver: 25.02.00B.1125.14	Gravity Loc R+ / R- / Rh / Rw / U / RL A 1755 - / - / - /1013 /121 /140 I 1755 - / - / - /1013 /121 - Wind reactions based on MWFRS A Brg Wid = - Min Req = - I Brg Wid = - Min Req = - Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 724 - 3158 E - F 527 - 2034 B - C 602 - 3112 F - G 572 - 2821 C - D 573 - 2821 G - H 601 - 3112 D - E 528 - 2034 H - I 724 - 3158

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP 2400f-2.0E;
Webs: 2x4 SP #2;
Lt Slider: 2x4 SP #2; block length = 1.500'
Rt Slider: 2x4 SP #2; block length = 1.500'

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plate Shift Table

JT No	Plate Size	Lateral Shift	Chord Bite	JT No	Plate Size	Lateral Shift	Chord Bite
[8]	3X4	S	1.25	[21]	3X4	S	1.25

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Bottom chord checked for 10.00 psf non-concurrent live load.

Live loads applied in combination per ASCE 7 sec. 2.4.1 use 0.75 factor for multiple live loads.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.



MISSOURI COA #2005000817
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SEQN: 5715	SPEC	Ply: 1	Job Number: PM000117	Cust: R 9646 JRef: 1YQG96460001 T30
FROM:		Qty: 6	Customer - Clayton Properties Plan Name - Honeydew Elevation - Farmhouse 2Car	DrwNo: 012.26.1214.48887
Page 2 of 2			Truss Label: E4	/ BM 01/12/2026

Hangers / Ties

Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.

Recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information. Additional connection required to evenly distribute hanger reaction throughout all plies of supporting girder.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

Bearing at location x=0' uses the following support conditions: 0'

Bearing A (0', 18'7") HUS28

Supporting Member: (3)2x8 SP 2400f-2.0E

(22) 0.148"x3" nails into supporting

member,

(4) 0.148"x3" nails into supported

member.

Bearing I (37'9", 18'7") HUS26

Supporting Member: (3)2x8 SP #1

(14) 0.148"x3" nails into supporting

member,

(4) 0.148"x3" nails into supported

member.



MISSOURI COA #2005000817
01/12/2026

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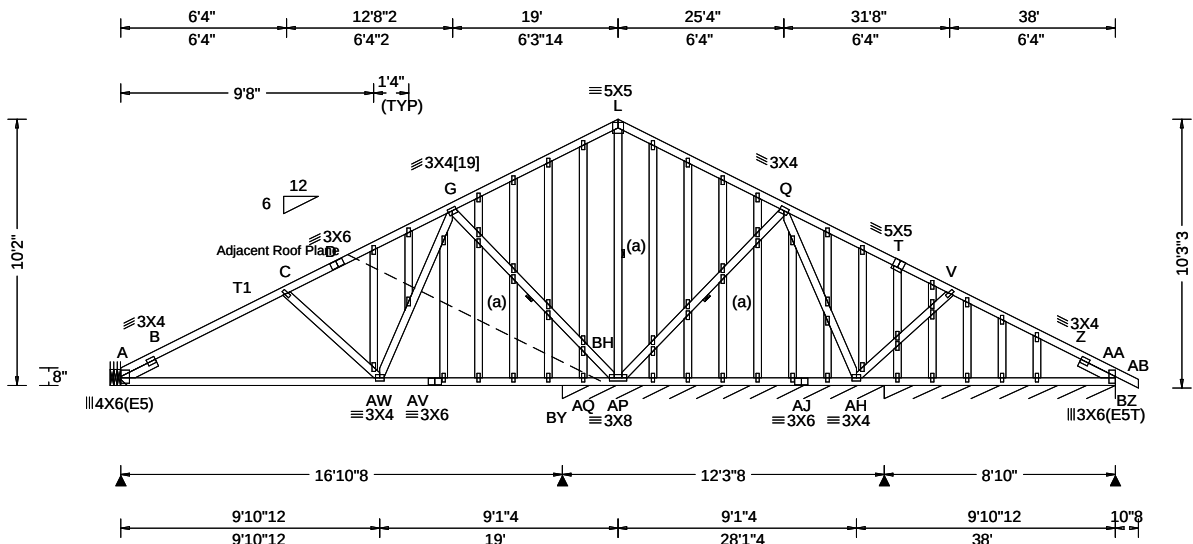
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 25.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCDL: 6.0 psf BCDL: 6.0 psf Mean Height: 23.78 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.80 ft Loc. from endwall: not in 6.06 ft GCpi: 0.55 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.085 B 999 240 VERT(CL): 0.142 B 999 180 HORZ(LL): 0.046 B - - HORZ(TL): 0.078 B - - Creep Factor: 2.0 Max TC CSI: 0.542 Max BC CSI: 0.642 Max Web CSI: 0.884 VIEW Ver: 25.02.00B.1125.14	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 871 -/- /- /478 /79 /201 BY*244 -/- /- /122 /26 -/- BZ*118 -/- /- /70 /13 -/- AH -/-100 Wind reactions based on MWFRS A Brg Wid = - Min Req = - BY Brg Wid = 147 Min Req = - BZ Brg Wid = 106 Min Req = - Bearings BY & AG Fcperp = 425psi. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.

Lumber
Top chord: 2x4 SP #2; T1 2x4 SP 2400f-2.0E;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;
Lt Slider: 2x4 SP #2; block length = 1.500'
Rt Slider: 2x4 SP #2; block length = 1.500'

Bracing
(a) Continuous lateral restraint equally spaced on member.

Plating Notes
All plates are 1.5X4 except as noted.

Plate Shift Table

JT No	Plate Size	Lateral Shift	Chord Bite	JT No	Plate Size	Lateral Shift	Chord Bite
[19]	3X4	S	1.25				

Loading
Truss designed to support 1-4-0 top chord outlookers and cladding load not to exceed 3.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.
Bottom chord checked for 10.00 psf non-concurrent live load.

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Snow
Overhang designed for 2.00X TC LL.

Additional Notes
See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.
Top Chord overhang(s) may be field trimmed.

Chords	Tens.Comp.	Chords	Tens. Comp.
A - B	597 -1720	D - G	187 -665
B - C	169 -1217	G - L	592 -46
C - D	117 -750	L - Q	558 0

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.
A-AW	1015 -91

Maximum Web Forces Per Ply (lbs)

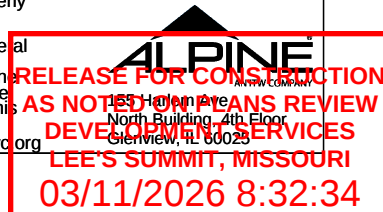
Webs	Tens.Comp.	Webs	Tens. Comp.
C-AW	244 -753	G-AP	220 -1088
AW-G	810 -124	L-AP	0 -536

Maximum Gable Forces Per Ply (lbs)

Gables	Tens.Comp.
AQ-BH	124 -482



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SEQN: 5732	GABL	Ply: 1	Job Number: PM000117	Cust: R 9646 JRef:1YGQ96460001 T31
FROM:		Qty: 1	Customer - Clayton Properties Plan Name - Honeydew Elevation - Farmhouse 2Car	DrwNo: 012.26.1214.44083
Page 2 of 2			Truss Label: E1	/ BM 01/12/2026

Hangers / Ties

Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.

Recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information. Additional connection required to evenly distribute hanger reaction throughout all plies of supporting girder.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

Bearing at location x=0' uses the following support conditions: 0'

Bearing A (0', 18'7") HUS28

Supporting Member: (3)2x8 SP 2400f-2.0E

(22) 0.148"x3" nails into supporting member,

(4) 0.148"x3" nails into supported member.



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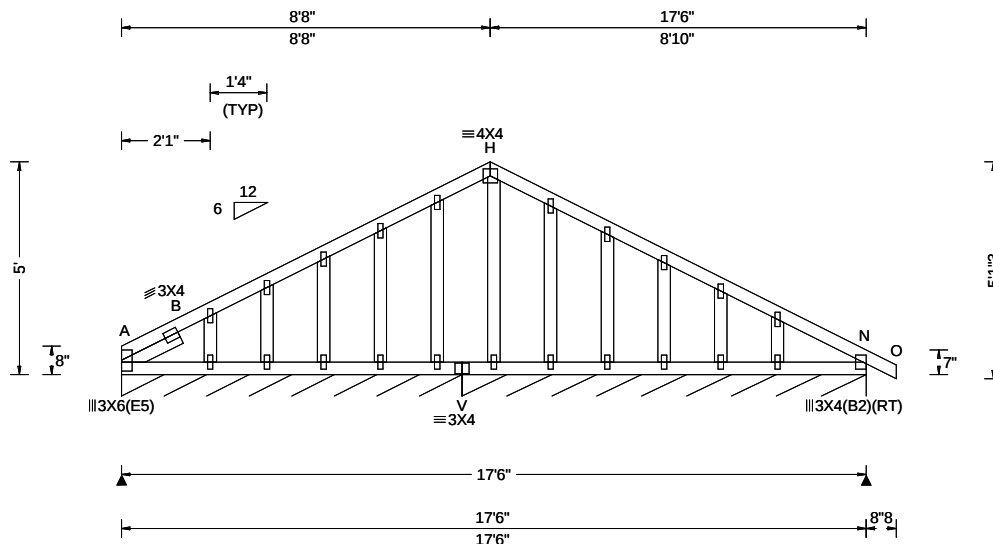
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SEQN: 5716 FROM:	GABL Ply: 1 Qty: 1	Job Number: PM000117 Customer - Clayton Properties Plan Name - Honeydew Elevation - Farmhouse 2Car Truss Label: G1	Cust: R 9646 JRef:1YQG96460001 T4 DrwNo: 012.26.1214.55483 / BM 01/12/2026
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 25.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCDL: 6.0 psf BCDL: 6.0 psf Mean Height: 21.20 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.55 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.003 B 999 240 VERT(CL): 0.004 B 999 180 HORZ(LL): 0.002 B - - HORZ(TL): 0.003 B - - Creep Factor: 2.0 Max TC CSI: 0.100 Max BC CSI: 0.028 Max Web CSI: 0.033 VIEW Ver: 25.02.00B.1125.14	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A* 121 /- /- /74 /21 /11 V* 125 /- /- /68 /8 /- Wind reactions based on MWFRS A Brg Wid = 96.0 Min Req = - V Brg Wid = 114 Min Req = - Bearings A & V are a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;
Lt Slider: 2x4 SP #2; block length = 1.500'

Plating Notes

All plates are 1.5X4 except as noted.

Loading

Truss designed to support 1-4-0 top chord outlookers and cladding load not to exceed 3.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Bottom chord checked for 10.00 psf non-concurrent live load.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Snow

Overhang designed for 2.00X TC LL.

Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Top Chord overhang(s) may be field trimmed.

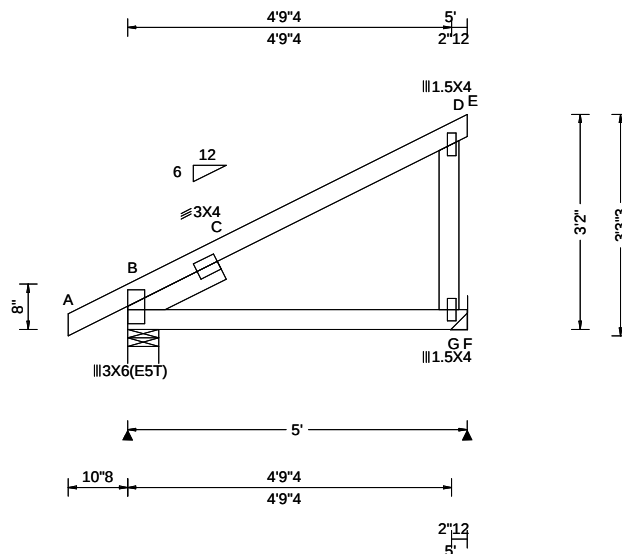


MISSOURI COA #2005000817
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SEQN: 5707 FROM:	MONO Ply: 1 Qty: 4	Job Number: PM000117 Customer - Clayton Properties Plan Name - Honeydew Elevation - Farmhouse 2Car Truss Label: B2	Cust: R 9646 JRef:1YGQ96460001 T5 DrwNo: 012.26.1214.23680 / BM 01/12/2026
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 25.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCDL: 6.0 psf BCDL: 6.0 psf Mean Height: 15.00 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.55 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.030 C - - HORZ(TL): 0.047 C - - Creep Factor: 2.0 Max TC CSI: 0.465 Max BC CSI: 0.233 Max Web CSI: 0.153 VIEW Ver: 25.02.00B.1125.14	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 294 /- /- /138 /- /71 F 237 /- /- /155 /21 /- Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.5 (Support) F Brg Wid = - Min Req = - Bearing B Fcperp = 425psi. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. B - C 684 -838

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;
Lt Slider: 2x4 SP #2; block length = 1.500'

Loading

Bottom chord checked for 10.00 psf non-concurrent live load.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Snow

Overhang designed for 2.00X TC LL.

Additional Notes

Top Chord overhang(s) may be field trimmed.

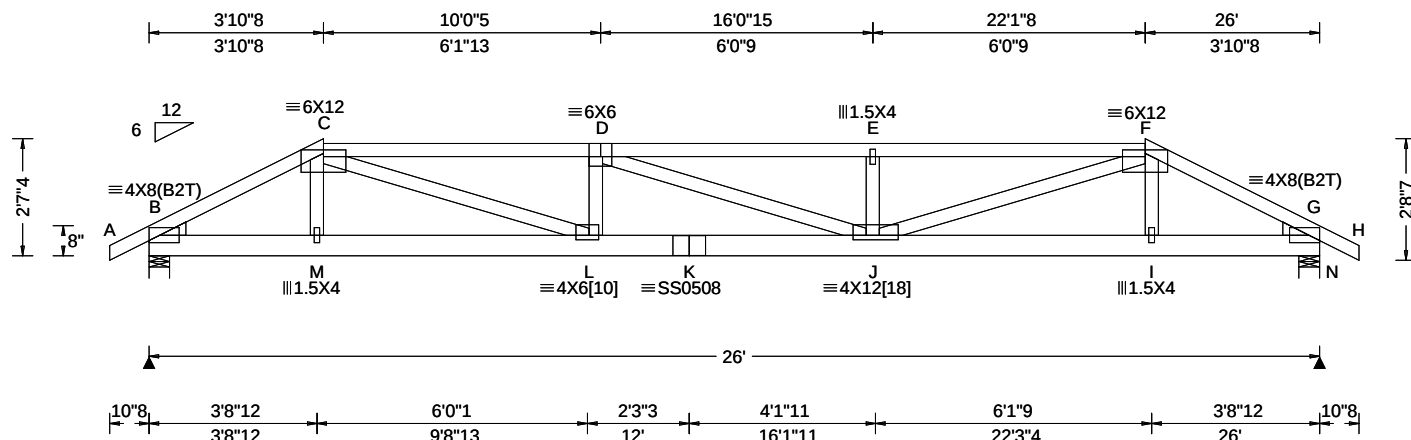


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SEQN: 5708 FROM:	HIPS Ply: 1 Qty: 1	Job Number: PM000117 Customer - Clayton Properties Plan Name - Honeydew Elevation - Farmhouse 2Car Truss Label: C1	Cust: R 9646 JRef: 1YQG96460001 T7 DrwNo: 012.26.1214.25727 / BM 01/12/2026
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 25.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 0.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCDL: 6.0 psf BCDL: 6.0 psf Mean Height: 20.00 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.55 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT: 20(0)/10(0) Plate Type(s): WAVE, 18SS	PP Deflection in loc L/defl L/# VERT(LL): 0.301 E 999 247 VERT(CL): 0.548 E 564 247 HORZ(LL): 0.051 C - - HORZ(TL): 0.093 C - - Creep Factor: 2.0 Max TC CSI: 0.874 Max BC CSI: 0.467 Max Web CSI: 0.722 VIEW Ver: 25.02.00B.1125.14	Gravity Loc R+ / R- / Rh / Rw / U / RL B 1919 /- /- /- /222 /- N 1919 /- /- /- /222 /- Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 2.4 (Support) N Brg Wid = 5.5 Min Req = 2.4 (Support) Bearings B & N Fcperp = 425psi. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 382 -3298 E - F 687 -5667 C - D 673 -5615 F - G 382 -3310 D - E 686 -5665

Lumber

Top chord: 2x4 SP 2400f-2.0E;
Bot chord: 2x6 SP 2400f-2.0E;
Webs: 2x4 SP #2;
Lt Wedge: 2x4 SP #2; Rt Wedge: 2x4 SP #2;

Special Loads

----- (Lumber Dur.Fac.=1.15 / Plate Dur.Fac.=1.15)
TC: From 72 plf at -0.88 to 72 plf at 3.88
TC: From 36 plf at 3.88 to 36 plf at 22.12
TC: From 72 plf at 22.12 to 72 plf at 26.88
BC: From 20 plf at 0.00 to 20 plf at 3.88
BC: From 10 plf at 3.88 to 10 plf at 22.12
BC: From 20 plf at 22.12 to 20 plf at 26.00
TC: 193 lb Conc. Load at 3.88,22.12
TC: 132 lb Conc. Load at 5.94, 7.94, 9.94,11.94
13.00,14.06,16.06,18.06,20.06
BC: 78 lb Conc. Load at 3.88,22.12
BC: 47 lb Conc. Load at 5.94, 7.94, 9.94,11.94
13.00,14.06,16.06,18.06,20.06

Plate Shift Table

JT No	Plate Size	Lateral Shift	Chord Bite	JT No	Plate Size	Lateral Shift	Chord Bite
[10]	4X6	2.50	R 1.25	[18]	4X12	5.00	R 1.25

Purlins

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

Wind

Wind loads and reactions based on MWFRS.
Wind loading based on both gable and hip roof types.

Snow

Overhang designed for 2.00X TC LL.

Additional Notes

Top Chord overhang(s) may be field trimmed.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
B - M	2890 -330	K - J	5714 -702
M - L	2892 -338	J - I	2901 -337
L - K	5714 -702	I - G	2900 -330

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
C - L	2904 -356	E - J	224 -750
L - D	223 -755	J - F	2944 -371

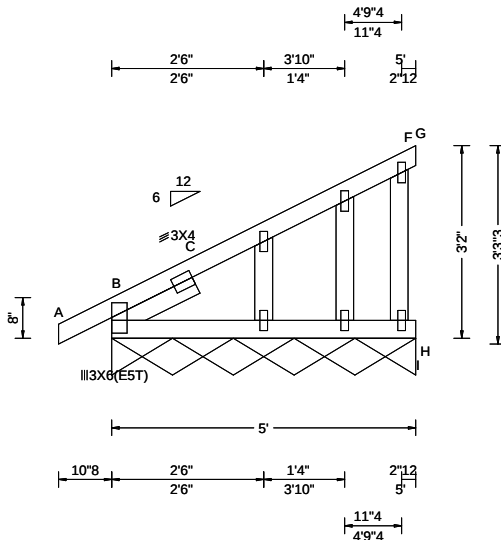


MISSOURI COA #2005000817
01/12/2026

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For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

ALPINE
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AS NOTED BY INSPECTION REVIEW
DEVELOPMENT SERVICES
LEE'S SUMMIT, MISSOURI
03/11/2026 8:32:35

SEQN: 5706 FROM:	GABL Qty: 1	Ply: 1 Qty: 1	Job Number: PM000117 Customer - Clayton Properties Plan Name - Honeydew Elevation - Farmhouse 2Car Truss Label: B1	Cust: R 9646 JRef: 1YGQ96460001 T8 DrwNo: 012.26.1214.17143 / BM 01/12/2026
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 25.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCDL: 6.0 psf BCDL: 6.0 psf Mean Height: 15.00 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.55 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT: 20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.003 C 999 240 VERT(CL): 0.005 C 999 180 HORZ(LL): 0.002 C - - HORZ(TL): 0.004 C - - Creep Factor: 2.0 Max TC CSI: 0.054 Max BC CSI: 0.016 Max Web CSI: 0.130 VIEW Ver: 25.02.00B.1125.14	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL H* 134 /- /- /74 /36 /33 Wind reactions based on MWFRS H Brg Wid = 60.0 Min Req = - Bearing B Fcperp = 425psi. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP 2400f-2.0E;
Bot chord: 2x4 SP 2400f-2.0E;
Webs: 2x4 SP #2;
Lt Slider: 2x4 SP #2; block length = 1.500'

Plating Notes

All plates are 1.5X4 except as noted.

Loading

Truss designed to support 1-4-0 top chord outlookers and cladding load not to exceed 3.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Bottom chord checked for 10.00 psf non-concurrent live load.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical exposed to wind pressure.
Deflection meets L/180.

Wind loading based on both gable and hip roof types.

Snow

Overhang designed for 2.00X TC LL.

Additional Notes

See DWGS A14015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Top Chord overhang(s) may be field trimmed.

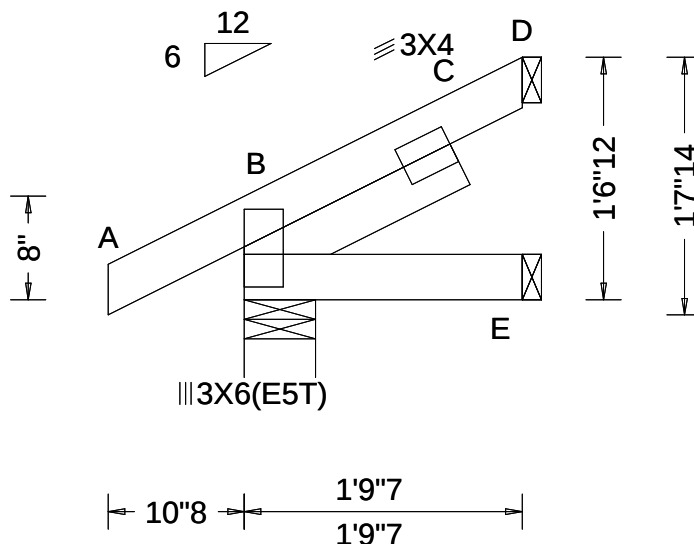


MISSOURI COA #2005000817
01/12/2026

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SEQN: 5721 FROM:	JACK Ply: 1 Qty: 2	Job Number: PM000117 Customer - Clayton Properties Plan Name - Honeydew Elevation - Farmhouse 2Car Truss Label: J3	Cust: R 9646 JRef: 1YGQ96460001 T9 DrwNo: 012.26.1215.02583 / BM 01/12/2026
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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 25.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 0.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCDL: 6.0 psf BCDL: 6.0 psf Mean Height: 19.48 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.55 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.001 C - - HORZ(TL): 0.002 C - - Creep Factor: 2.0 Max TC CSI: 0.104 Max BC CSI: 0.020 Max Web CSI: 0.006 VIEW Ver: 25.02.00B.1125.14	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 171 /- /- /63 /- /34 E 19 /- /- /18 /- /- D 64 /-5 /- /35 /22 /- Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.5 (Support) E Brg Wid = 1.5 Min Req = - D Brg Wid = 1.5 Min Req = - Bearing B Fcperp = 425psi. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Lt Slider: 2x4 SP #2; block length = 1.500'

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Snow

Overhang designed for 2.00X TC LL.

Additional Notes

Top Chord overhang(s) may be field trimmed.



MISSOURI COA #2005000817
01/12/2026

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Gable Stud Reinforcement Detail

ASCE 7-16: 140 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Or: 120 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00

Or: 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure D, Kzt = 1.00

Or: 100 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

Max Gable Vertical Length	2x4 Gable Vertical		Brace Grade	No Braces	(1) 1x4 'L' Brace *		(1) 2x4 'L' Brace *		(2) 2x4 'L' Brace **		(1) 2x6 'L' Brace *		(2) 2x6 'L' Brace **	
	Spacing	Species			Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B
24" O.C.	SPF	#1 / #2	#1	4' 3"	7' 3"	7' 7"	8' 7"	8' 11"	10' 3"	10' 8"	13' 6"	14' 0"	14' 0"	14' 0"
			#3	4' 1"	6' 7"	7' 1"	8' 6"	8' 10"	10' 1"	10' 6"	13' 4"	13' 10"	14' 0"	14' 0"
			Stud	4' 1"	6' 7"	7' 0"	8' 6"	8' 10"	10' 1"	10' 6"	13' 4"	13' 10"	14' 0"	14' 0"
		Standard	#1	4' 6"	5' 8"	6' 0"	7' 7"	8' 1"	10' 1"	10' 6"	11' 10"	12' 8"	14' 0"	14' 0"
			#2	4' 3"	7' 3"	7' 7"	8' 7"	8' 11"	10' 3"	10' 8"	13' 6"	14' 0"	14' 0"	14' 0"
			#3	4' 2"	6' 0"	6' 4"	7' 11"	8' 6"	10' 2"	10' 7"	12' 5"	13' 4"	14' 0"	14' 0"
	SP	DFL	Stud	4' 2"	6' 0"	6' 4"	7' 11"	8' 6"	10' 2"	10' 7"	12' 5"	13' 4"	14' 0"	14' 0"
			Standard	4' 0"	5' 3"	5' 7"	7' 0"	7' 6"	10' 2"	11' 0"	11' 10"	14' 0"	14' 0"	14' 0"
		Standard	#1 / #2	4' 11"	8' 4"	8' 8"	9' 10"	10' 3"	11' 8"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	4' 8"	8' 1"	8' 8"	9' 8"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	4' 8"	8' 1"	8' 6"	9' 8"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	#1	5' 1"	8' 5"	8' 9"	9' 11"	10' 4"	11' 10"	12' 4"	14' 0"	14' 0"	14' 0"	14' 0"
16" O.C.	SPF	#2	#2	4' 11"	8' 4"	8' 8"	9' 10"	10' 3"	11' 8"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	4' 9"	7' 4"	7' 9"	9' 9"	10' 3"	11' 8"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	4' 9"	7' 4"	7' 9"	9' 9"	10' 2"	11' 8"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	#1 / #2	4' 8"	6' 5"	6' 10"	8' 7"	9' 2"	11' 7"	12' 1"	13' 6"	14' 0"	14' 0"	14' 0"
			#3	5' 1"	9' 0"	9' 4"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	5' 1"	9' 0"	9' 4"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"
	SP	DFL	#1	5' 8"	9' 3"	9' 8"	10' 11"	11' 4"	13' 0"	13' 6"	14' 0"	14' 0"	14' 0"	14' 0"
			#2	5' 5"	9' 2"	9' 6"	10' 10"	11' 3"	12' 11"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	5' 3"	8' 5"	9' 0"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	Stud	5' 3"	8' 5"	9' 0"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"
			Standard	5' 1"	7' 5"	7' 11"	9' 11"	10' 7"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"
			Standard	5' 1"	7' 5"	7' 11"	9' 11"	10' 7"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"

Bracing Group Species and Grades:

Group A:			
Spruce-Pine-Fir		Hem-Fir	
#1 / #2	Standard	#2	Stud
#3	Stud	#3	Standard
Douglas Fir-Larch		Southern Pine***	
#3		#3	
Stud		Stud	
Standard		Standard	

Group B:			
Hem-Fir			
#1 & Btr			
#1			
Douglas Fir-Larch		Southern Pine***	
#1		#1	
#2		#2	

1x4 Braces shall be SRB (Stress-Rated Board).

***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes:

Wind Load deflection criterion is L/240.

Provide uplift connections for 55 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12' plywood overhang.

Attach 'L' braces with 10d (0.128"x3.0" min) nails.

* For (1) 'L' brace: space nails at 2' o.c. in 18' end zones and 4' o.c. between zones.
 ** For (2) 'L' braces: space nails at 3' o.c. in 18' end zones and 6' o.c. between zones.

'L' bracing must be a minimum of 80% of web member length.

Gable Vertical Plate Sizes

Vertical Length	No Splice
Less than 4' 0"	1X4 or 2X3
Greater than 4' 0"	3X4

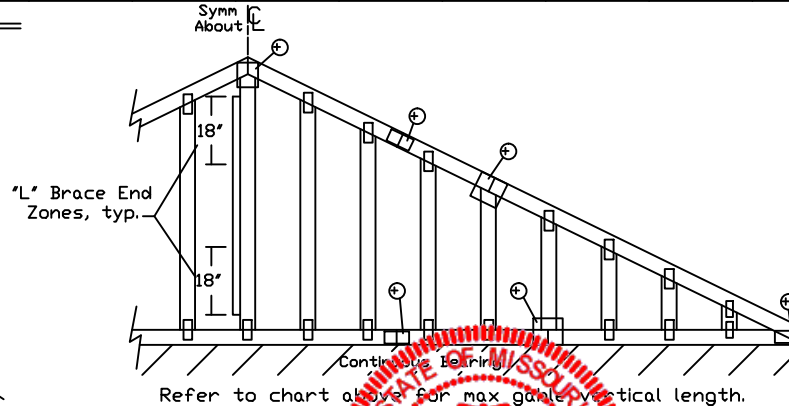
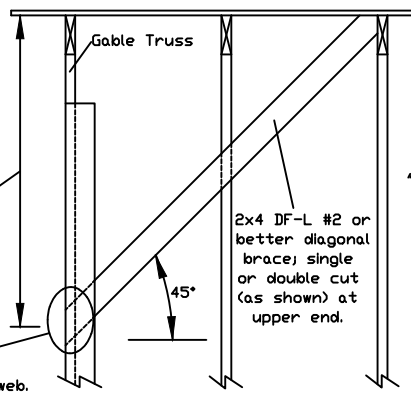
+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.

Diagonal brace option: vertical length may be doubled when diagonal brace is used. Connect diagonal brace for 450# at each end. Max web total length is 14'.

Vertical length shown in table above.

Connect diagonal at midpoint of vertical web.



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BRADLEY
 E. MORRIS
 NUMBER
 PE-2002019611

MISSOURI COA #2005000817

REF ASCE7-16-GAB14015

DATE 01/26/2018

DRWG A14015ENC160118

MAX. TOT. LD. 60 PSF

01/12/2026

MAX. SPACING 24.0'

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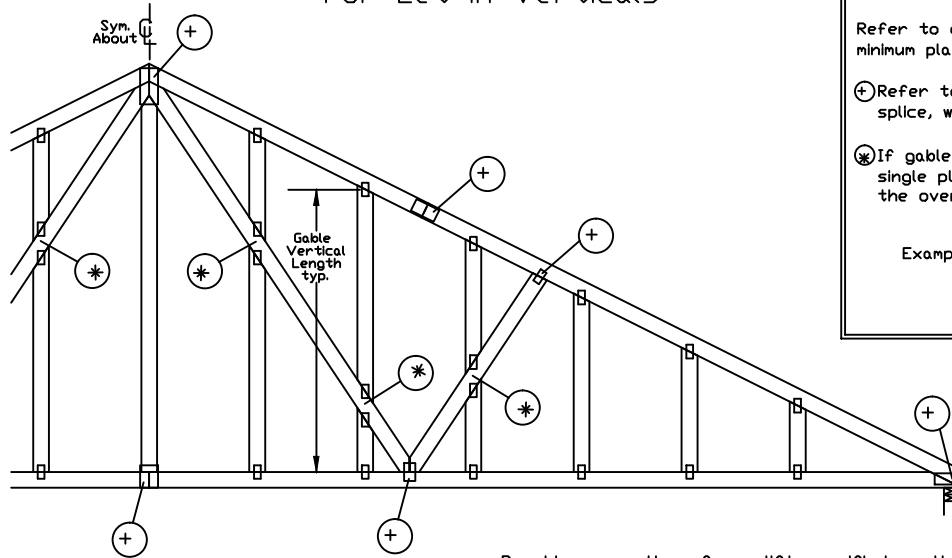
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155 Harlem Ave
 North Building, 4th Floor
 Glenview, IL 60025

Gable Detail For Let-in Verticals

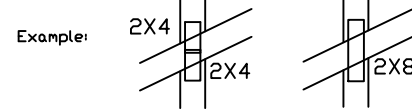


Gable Truss Plate Sizes

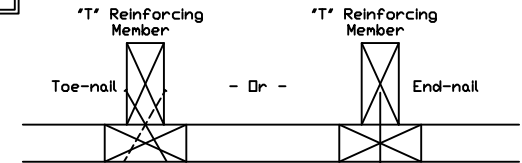
Refer to appropriate Alpine gable detail for minimum plate sizes for vertical studs.

⊕ Refer to Engineered truss design for peak, splice, web, and heel plates.

⊗ If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web.



'T' Reinforcement Attachment Detail



To convert from 'L' to 'T' reinforcing members, multiply 'T' increase by length (based on appropriate Alpine gable detail).

Maximum allowable 'T' reinforced gable vertical length is 14' from top to bottom chord.

'T' reinforcing member material must match size, specie, and grade of the 'L' reinforcing member.

Web Length Increase w/ 'T' Brace

'T' Reinf. Mbr. Size	'T' Increase
2x4	30 %
2x6	20 %

Example:

ASCE 7-10 Wind Speed = 120 mph

Mean Roof Height = 30 ft, Kzt = 1.00

Gable Vertical = 24' o.c. SP #3

'T' Reinforcing Member Size = 2x4

'T' Brace Increase (From Above) = 30% = 1.30

(1) 2x4 'L' Brace Length = 8' 7"

Maximum 'T' Reinforced Gable Vertical Length
1.30 x 8' 7" = 11' 2"

Provide connections for uplift specified on the engineered truss design.

Attach each 'T' reinforcing member with

End Driven Nails:

10d Common (0.148"x3",min) Nails at 4' o.c. plus
(4) nails in the top and bottom chords.

Toenailed Nails:

10d Common (0.148"x3",min) Toenails at 4' o.c. plus
(4) toenails in the top and bottom chords.

This detail to be used with the appropriate Alpine gable detail for ASCE wind load.

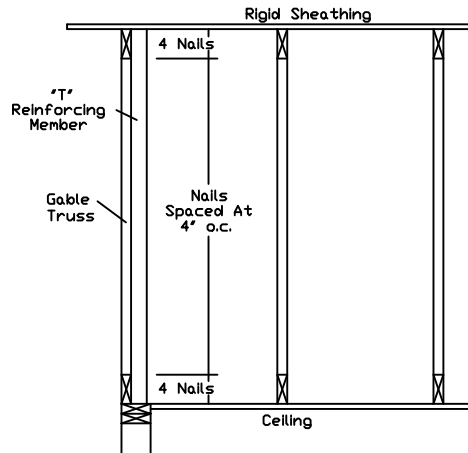
ASCE 7-05 Gable Detail Drawings

A13015051014, A12015051014, A11015051014, A10015051014, A14015051014,
A13030051014, A12030051014, A11030051014, A10030051014, A14030051014

ASCE 7-10 & ASCE 7-16 Gable Detail Drawings

A11515ENC100118, A12015ENC100118, A14015ENC100118, A16015ENC100118,
A18015ENC100118, A20015ENC100118, A20015END100118, A20015PED100118,
A11530ENC100118, A12030ENC100118, A14030ENC100118, A16030ENC100118,
A18030ENC100118, A20030ENC100118, A20030END100118, A20030PED100118,
S11515ENC100118, S12015ENC100118, S14015ENC100118, S16015ENC100118,
S18015ENC100118, S20015ENC100118, S20015END100118, S20015PED100118,
S11530ENC100118, S12030ENC100118, S14030ENC100118, S16030ENC100118,
S18030ENC100118, S20030ENC100118, S20030END100118, S20030PED100118

See appropriate Alpine gable detail for maximum 'T' reinforced gable vertical length.



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ALPINE
AN ITW COMPANY

155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

BRADLEY
E. MORRIS
NUMBER

PE-2002019611

PROFESSIONAL ENGINEER

MISSOURI

COA #2005000817

REF LET-IN VERT

DATE 01/02/2018

DRWG GBLLETIN0118

MAX. TOT. LD. 60 PSF

01/13/2026 FAC. ANY

MAX. SPACING 24.0"

RELEASE FOR CONSTRUCTION
AS NOTED ON PLANS REVIEW
DEVELOPMENT SERVICES
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NAIL SPACING DETAIL

MINIMUM SPACING FOR SINGLE BLOCK IS SHOWN. DOUBLE NAIL SPACINGS AND STAGGER NAILING FOR TWO BLOCKS. GREATER SPACING MAY BE REQUIRED TO AVOID SPLITTING.

BLOCK LOCATION, SIZE, LENGTH, GRADE AND TOTAL NUMBER AND TYPE OF NAILS ARE TO BE SPECIFIED ON SEALED DESIGN REFERENCING THIS DETAIL.

LOAD PERPENDICULAR TO GRAIN

A - EDGE DISTANCE AND SPACING BETWEEN STAGGERED ROWS OF NAILS (6 NAIL DIAMETERS)

B - SPACING OF NAILS IN A ROW (12 NAIL DIAMETERS)

C - END DISTANCE (15 NAIL DIAMETERS)

LOAD PARALLEL TO GRAIN

A - EDGE DISTANCE (6 NAIL DIAMETERS)

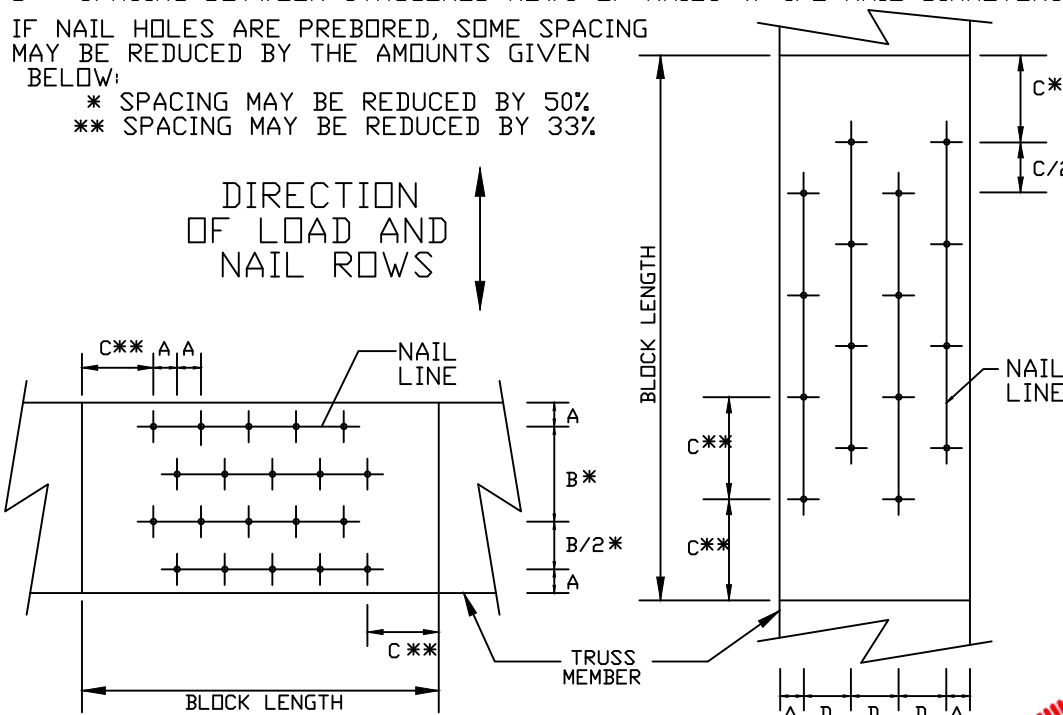
C - SPACING OF NAILS IN A ROW AND END DISTANCE (15 NAIL DIAMETERS)

D - SPACING BETWEEN STAGGERED ROWS OF NAILS (7 1/2 NAIL DIAMETERS)

IF NAIL HOLES ARE PREBORED, SOME SPACING MAY BE REDUCED BY THE AMOUNTS GIVEN BELOW:

* SPACING MAY BE REDUCED BY 50%

** SPACING MAY BE REDUCED BY 33%



MINIMUM NAIL SPACING DISTANCES

NAIL TYPE	DISTANCES			
	A	B*	C**	D
8d BOX (0.113"X 2.5",MIN)	3/4"	1 3/8"	1 3/4"	7/8"
10d BOX (0.128"X 3",MIN)	7/8"	1 5/8"	2"	1"
12d BOX (0.128"X 3.25",MIN)	7/8"	1 5/8"	2"	1"
16d BOX (0.135"X 3.5",MIN)	7/8"	1 5/8"	2 1/8"	1 1/8"
20d BOX (0.148"X 4",MIN)	1"	1 7/8"	2 1/4"	1 1/8"
8d COMMON (0.131"X 2.5",MIN)	7/8"	1 5/8"	2"	1"
10d COMMON (0.148"X 3",MIN)	1"	1 7/8"	2 1/4"	1 1/8"
12d COMMON (0.148"X 3.25",MIN)	1"	1 7/8"	2 1/4"	1 1/8"
16d COMMON (0.162"X 3.5",MIN)	1"	2"	2 1/2"	1 1/4"
GUN (0.120"X 2.5",MIN)	3/4"	1 1/2"	1 7/8"	1"
GUN (0.131"X 2.5",MIN)	7/8"	1 5/8"	2"	1"
GUN (0.120"X 3",MIN)	3/4"	1 1/2"	1 7/8"	1"
GUN (0.131"X 3",MIN)	7/8"	1 5/8"	2"	1"

LOAD APPLIED PERPENDICULAR TO GRAIN

LOAD APPLIED PARALLEL TO GRAIN

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ALPINE: www.alpineitw.com; TPI: www.tpinet.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org



155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

BRADLEY
E. MORRIS
NUMBER

PE-2002019611

PROFESSIONAL ENGINEER

MISSOURI COA #2005000817

01/12/2026

REF NAIL SPACE
DATE 10/01/14
DRWG CNNAILSP1014

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AS NOTED ON PLANS REVIEW
DEVELOPMENT SERVICES
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03/11/2026 8:32:35

Gable Stud Reinforcement Detail

ASCE 7-16: 140 mph Wind Speed, 30' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Or: 120 mph Wind Speed, 30' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00

Or: 120 mph Wind Speed, 30' Mean Height, Enclosed, Exposure D, Kzt = 1.00

Or: 100 mph wind speed, 30' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

Max Gable Vertical Length	2x4 Gable Vertical		Brace Grade	No Braces	(1) 1x4 'L' Brace *		(1) 2x4 'L' Brace *		(2) 2x4 'L' Brace **		(1) 2x6 'L' Brace *		(2) 2x6 'L' Brace **	
	Spacing	Species			Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B
24" O.C.	SPF	#1 / #2	#1	4' 1"	6' 11"	7' 2"	8' 2"	8' 6"	9' 9"	10' 2"	12' 10"	13' 4"	14' 0"	14' 0"
			#3	3' 10"	6' 2"	6' 7"	8' 1"	8' 5"	9' 8"	10' 0"	12' 8"	13' 2"	14' 0"	14' 0"
			Stud	3' 10"	6' 2"	6' 6"	8' 1"	8' 5"	9' 8"	10' 0"	12' 8"	13' 2"	14' 0"	14' 0"
		Standard	#1	4' 2"	7' 0"	7' 3"	8' 3"	8' 7"	9' 10"	10' 3"	13' 0"	13' 6"	14' 0"	14' 0"
			#2	4' 1"	6' 11"	7' 2"	8' 2"	8' 6"	9' 9"	10' 2"	12' 10"	13' 4"	14' 0"	14' 0"
			#3	4' 0"	5' 7"	5' 11"	7' 5"	7' 11"	9' 8"	10' 1"	11' 7"	12' 5"	14' 0"	14' 0"
	SP	DFL	Stud	4' 0"	5' 7"	5' 11"	7' 5"	7' 11"	9' 8"	10' 1"	11' 7"	12' 5"	14' 0"	14' 0"
			Standard	3' 9"	4' 11"	5' 13"	6' 6"	7' 0"	8' 10"	9' 6"	10' 3"	11' 0"	13' 11"	14' 0"
		Standard	#1 / #2	4' 8"	7' 11"	8' 3"	9' 4"	9' 9"	11' 2"	11' 7"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	4' 5"	7' 6"	8' 3"	9' 3"	9' 7"	11' 0"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	4' 5"	7' 6"	8' 0"	9' 3"	9' 7"	11' 0"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	#1	4' 5"	6' 5"	6' 10"	8' 7"	9' 2"	11' 0"	11' 6"	13' 6"	14' 0"	14' 0"	14' 0"
16" O.C.	SPF	#1	#2	4' 10"	8' 0"	8' 4"	9' 6"	9' 10"	11' 3"	11' 9"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	4' 8"	7' 11"	8' 3"	9' 4"	9' 9"	11' 2"	11' 7"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	4' 7"	6' 10"	7' 3"	9' 1"	9' 8"	11' 1"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	#1	4' 5"	6' 0"	6' 5"	8' 0"	8' 7"	10' 10"	11' 6"	12' 7"	13' 15"	14' 0"	14' 0"
			#2	4' 10"	8' 9"	9' 1"	10' 4"	10' 9"	11' 2"	12' 9"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	4' 10"	8' 7"	8' 11"	10' 2"	10' 7"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
12" O.C.	SPF	#1	#2	4' 10"	8' 7"	8' 11"	10' 2"	10' 7"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	4' 10"	8' 7"	8' 11"	10' 2"	10' 7"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	#1	5' 4"	8' 10"	9' 2"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
			#2	5' 2"	8' 9"	9' 1"	10' 4"	10' 9"	12' 3"	12' 9"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	5' 0"	7' 10"	8' 4"	10' 3"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	Stud	5' 0"	7' 10"	8' 4"	10' 3"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
12" O.C.	SP	#1	#2	5' 0"	7' 10"	8' 4"	10' 3"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	5' 0"	7' 10"	8' 4"	10' 3"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
			Standard	4' 10"	6' 11"	7' 4"	9' 3"	9' 10"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	#1	5' 2"	8' 9"	9' 1"	10' 4"	10' 9"	12' 3"	12' 9"	14' 0"	14' 0"	14' 0"	14' 0"
			#2	5' 0"	7' 10"	8' 4"	10' 3"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	5' 0"	7' 10"	8' 4"	10' 3"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"

Bracing Group Species and Grades:

Group A:			
Spruce-Pine-Fir		Hem-Fir	
#1 / #2	Standard	#2	Stud
#3	Stud	#3	Standard
Douglas Fir-Larch		Southern Pine***	
#3		#3	
Stud		Stud	
Standard		Standard	

Group B:			
Hem-Fir			
#1 & Btr			
#1			
Douglas Fir-Larch		Southern Pine***	
#1		#1	
#2		#2	

1x4 Braces shall be SRB (Stress-Rated Board).

***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes:

Wind Load deflection criterion is L/240.

Provide uplift connections for 100 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12' plywood overhang.

Attach 'L' braces with 10d (0.128"x3.0" min) nails.

* For (1) 'L' brace: space nails at 2' o.c. in 18' end zones and 4' o.c. between zones.

** For (2) 'L' braces: space nails at 3' o.c. in 18' end zones and 6' o.c. between zones.

'L' bracing must be a minimum of 80% of web member length.

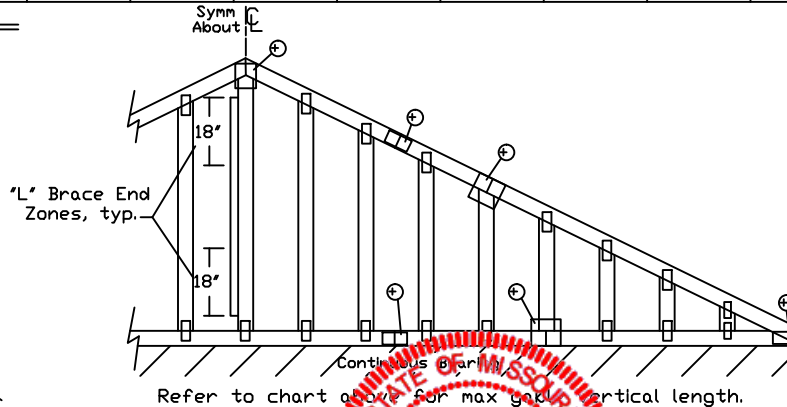
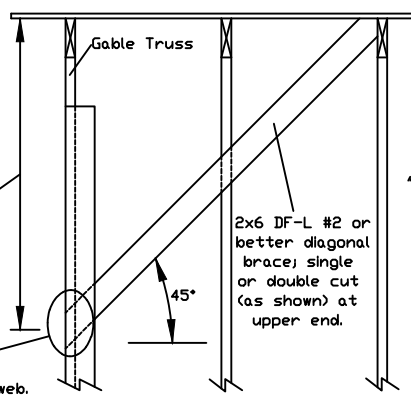
Gable Vertical Plate Sizes	
Vertical Length	No Splice
Less than 4' 0"	2X4
Greater than 4' 0", but less than 11' 6"	3X4
Greater than 11' 6"	4X4
+ Refer to common truss design for peak, splice, and heel plates.	

Refer to the Building Designer for conditions not addressed by this detail.

Diagonal brace option: vertical length may be doubled when diagonal brace is used. Connect diagonal brace for 525# at each end. Max web total length is 14'.

Vertical length shown in table above.

Connect diagonal at midpoint of vertical web.



Refer to chart above for max gable vertical length.

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ALPINE: www.alpineitw.com TPI: www.tpinet.org SBCA: www.sbcacomponents.com ICC: www.iccsafe.org

BRADLEY E. MORRIS
REGISTERED PROFESSIONAL ENGINEER
RE-2002019611
MISSOURI COA #2005000817

REF ASCE7-16-GAB14030

DATE 01/26/2018

DRWG A14030ENC160118

MAX. TOT. LD. 60 PSF

01/12/2026

MAX. SPACING 24.0"

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CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

Notes:

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforcement or scab reinforcement.

Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

Use scabs instead of L- or T- reinforcement on webs with intersecting truss joints, such as K-web joints, that may interfere with proper application along the narrow face of the web.

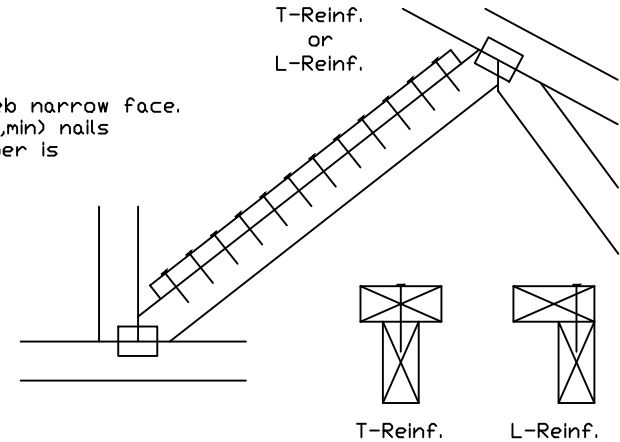
Web Member Size	Specified CLR Restraint	Alternative Reinforcement T- or L- Reinf.	Scab Reinf.
2x3 or 2x4	1 row	2x4	1-2x4
2x3 or 2x4	2 rows	2x6	2-2x4
2x6	1 row	2x4	1-2x6
2x6	2 rows	2x6	2-2x4(*)
2x8	1 row	2x6	1-2x8
2x8	2 rows	2x6	2-2x6(*)

T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

(*) Center scab on wide face of web. Apply (1) scab to each face of web.

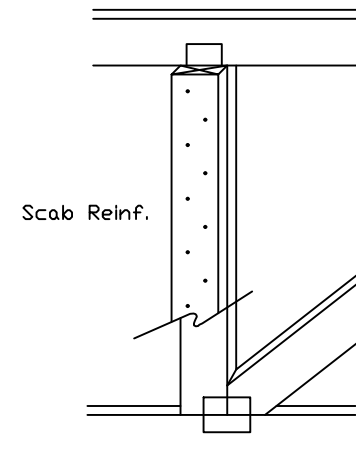
T-Reinforcement or L-Reinforcement:

Apply to either side of web narrow face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



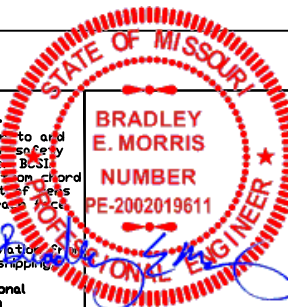
Scab Reinforcement:

Apply scab(s) to wide face of web. No more than (1) scab per face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

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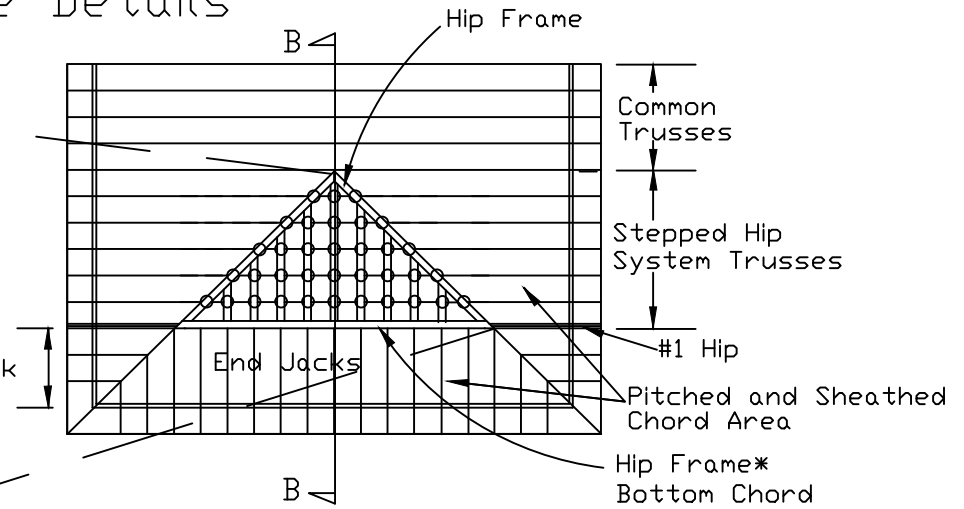
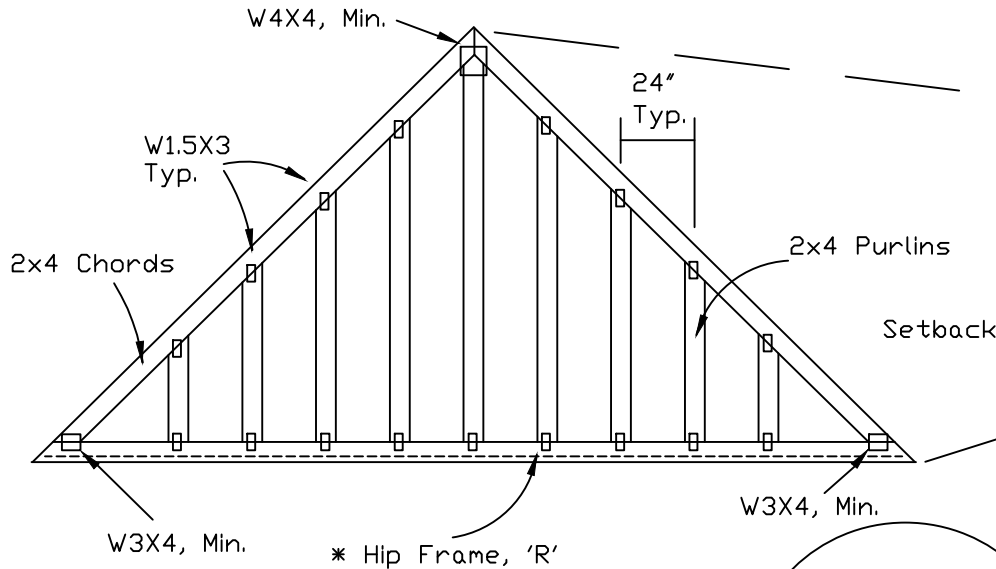


MISSOURI COA #2005000817

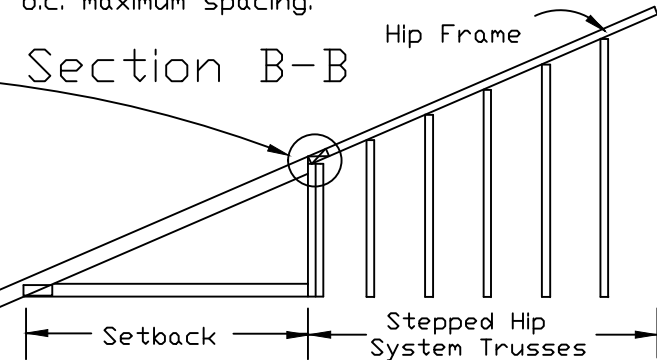
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TC DL	PSF	DATE	01/02/19
BC DL	PSF	DRWG	BRCLBSUB0119
BC LL	PSF		
TOT. LD.	PSF		
01/13/2026			
DUR. PAC.			
SPACING			

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03/11/2026 8:32:35

* Hip Frame Details



- Attach hip frame to flat chords of stepped hips at all overlapping points with 2-10d (0.148"x3") common nails. Bottom chord of hip frame to be attached to #1 hip with 10d common (0.148"x3") nails @ 6" o.c. maximum spacing.



Hip frame stops at plumb cut of jacks to maintain pitch continuity.

* Hip frame lumber is SPF, So. Pine, HF, or DFL Standard, Stud grade, or better.

See Engineer's sealed design for setback, lumber, plating, loading, and duration factor required.

'R' Hip frame chords may be trimmed up to 2" to fit. purlins must be intact and properly attached.

Use this detail for:

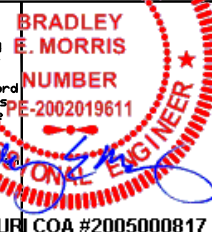
ASCE 7-22, 140 mph, 30' M.H., Enclosed, Exp C, or
ASCE 7-22, 120 mph, 30' M.H., Enclosed, Exp D, or
ASCE 7-10 & ASCE 7-16, 140 mph, 30' M.H., Enclosed, Exp C, or
ASCE 7-10 & ASCE 7-16, 120 mph, 30' M.H., Enclosed, Exp D, or
ASCE 7-05, 110 mph, 30' M.H., Enclosed, Cat II, Exp C,
Residential, Wind TC DL=4.2 psf, Kzt=1.00

Hip Frame - provided by truss manufacturer.
Hip frame is designed to provide bracing for flat top chords of hip frame system where indicated.
Structural panels must be properly attached directly to hip frame purlins.



155 Harlem Ave
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01/12/2026

REF	HIP FRAME
DATE	06/23/2023
DRWG	HIPFRAME0623

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Valley Detail - ASCE 7-16: 180 mph, 30' Mean Height, Partially Enclosed, Exp. C, Kzt=1.00

Top Chord 2x4 SP #2N, SPF #1/#2, DF-L #2 or better.
 Bot Chord 2x4 SP #2N or SPF #1/#2 or better.
 Webs 2x4 SP #3, SPF #1/#2, DF-L #2 or better.

**** Attach each valley to every supporting truss with:**
 535# connection or with (1) Simpson H2.5A or
 equivalent connector for
 ASCE 7-16 180 mph. 30' Mean Height, Part. Enc.
 Building, Exp. C, Wind TC DL=5 psf, Kzt = 1.00
 Or
 ASCE 7-16 160 mph. 30' Mean Height, Part. Enc.
 Building, Exp. D, Wind TC DL=5 psf, Kzt = 1.00

Bottom chord may be square or pitched cut
 as shown.

Valleys short enough to be cut as solid triangular
 members from a single 2x6, or larger as required,
 shall be permitted in lieu of fabricating from
 separate 2x4 members.

All plates shown are Alpine Wave Plates.

Unless specified otherwise on engineer's sealed design, for vertical
 valley webs taller than 7'-9" apply 2x4 "T" reinforcement, 80% length of
 web, same species and grade or better, attached with 10d box
 (0.128" x 3.0") nails at 6" o.c. In lieu of "T" reinforcement, 2x4 Continuous
 Lateral Restraint applied at mid-length of web is permitted with diagonal
 bracing as shown in DRWG BRCLBANC1014.

Top chord of truss beneath valley set must be braced with:
 properly attached, rated sheathing applied prior to valley truss
 installation.

Or

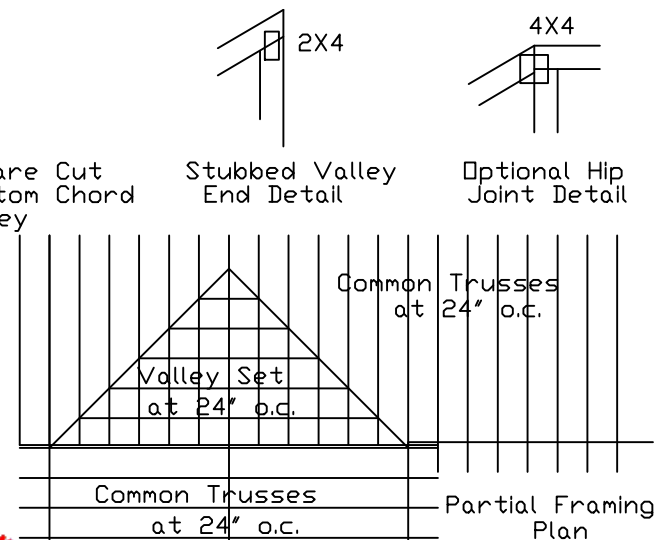
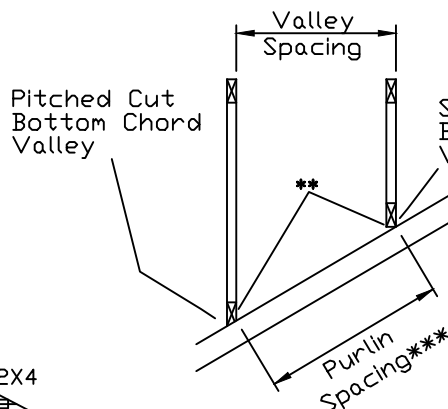
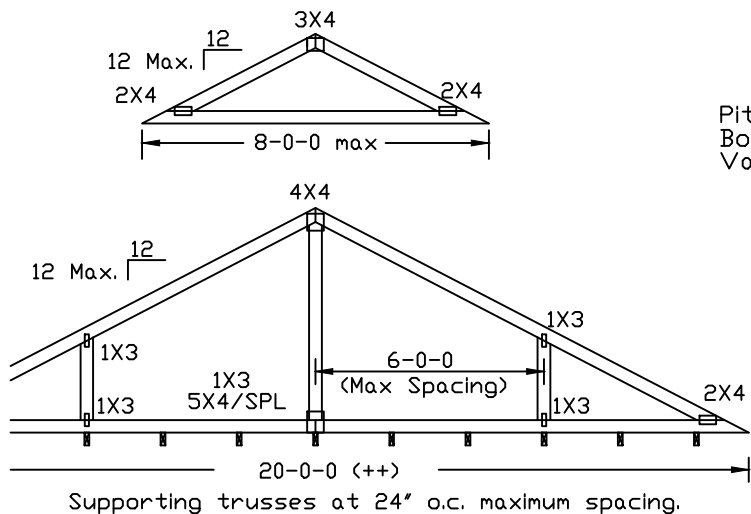
Purlins at 24" o.c. or as otherwise specified on engineer's sealed design

Or

By valley trusses used in lieu of purlin spacing as specified on
 Engineer's sealed design.

***** Note that the purlin spacing for bracing the top chord of the truss
 beneath the valley is measured along the slope of the top chord.**

**++ Larger spans may be built as long as the vertical height does
 not exceed 14'-0".**



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 this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping,
 installation & bracing of trusses.
 A seal on this drawing or cover page listing this drawing, indicates acceptance of professional
 engineering responsibility solely for the design shown. The suitability and use of this drawing
 for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.
 For more information see this Job's general notes page and these web sites:
 ALPINE: www.alpineitw.com; TPI: www.tpinet.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org



TC LL	30	30	40PSF	REF	VALLEY DETAIL
TC DL	20	15	7PSF	DATE	01/26/2018
BC DL	10	10	10 PSF	DRWG	VAL180160118
BC LL	0	0	0 PSF		
TOT. LD.	60	55	57PSF		
OUR PER	25/133	1.15	1.15		
SPACING	24.0"				

**RELEASE FOR CONSTRUCTION
 AS NOTED ON PLANS REVIEW
 DEVELOPMENT SERVICES
 LEE'S SUMMIT, MISSOURI
 03/11/2026 8:32:35**