



01/12/2026



MISSOURI COA #2005000817

**RELEASE FOR CONSTRUCTION  
 AS NOTED ON PLANS REVIEW  
 DEVELOPMENT SERVICES  
 LEE'S SUMMIT, MISSOURI  
 03/10/2026 3:01:08**

Site Information:	Page 1:
Customer: Kodiak – Premier Building Supply	Job Number: PM000117
Job Description: Customer - Clayton Properties Plan Name - Honeydew Elevation - Farmhouse 2 Car	
Address: MO <b>1424 SE Vantage Dr, Lee's Summit, MO 64082</b>	

Job Engineering Criteria:		
Building Code: IRC 2018	Design Standard: TPI 2014	IntelliVIEW Version: 25.02.00B
Loading Standard: ASCE 7-16	Design Methodology: ASD	JRef #: 1YGQ96460001
Wind: Wind Speed (mph): 115	Exposure: B	Design Loading (psf): 45
Building Type: Part Enc.	Risk Category: II	
Snow: Pg (psf): NA	Pf(ASD) (psf): NA	
W2::	Risk Category: II	

This package contains general notes pages, 29 truss drawing(s) and 7 detail(s).

Item	Drawing Number	Truss
1	012.26.1215.03923	LAY1
3	012.26.1214.41710	D2
5	012.26.1214.57137	G2
7	012.26.1214.46267	E2
9	012.26.1214.38430	C4
11	012.26.1214.32750	C2
13	012.26.1215.16560	V8
15	012.26.1215.06590	V10
17	012.26.1215.08330	V2
19	012.26.1215.11410	V4
21	012.26.1214.34783	C3
23	012.26.1214.44083	E1
25	012.26.1214.23680	B2
27	012.26.1214.25727	C1
29	012.26.1215.02583	J3
31	GBLLETIN0118	
33	A14030ENC160118	
35	HIPFRAME0623	

Item	Drawing Number	Truss
2	012.26.1214.59703	J2
4	012.26.1214.58390	J1
6	012.26.1215.13983	V6
8	012.26.1214.50680	E5
10	012.26.1214.39900	D1
12	012.26.1215.15287	V7
14	012.26.1215.22470	V9
16	012.26.1215.05330	V1
18	012.26.1215.09757	V3
20	012.26.1215.12680	V5
22	012.26.1214.48887	E4
24	012.26.1214.55483	G1
26	012.26.1215.01260	J3
28	012.26.1214.17143	B1
30	A14015ENC160118	
32	CNNAILSP1014	
34	BRCLBSUB0119	
36	VAL180160118	

## General Notes

### **Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:**

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high-quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

### **Temporary Lateral Restraint and Bracing:**

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

### **Permanent Lateral Restraint and Bracing:**

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed, and detailed by the Building Designer.

### **Connector Plate Information:**

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at [www.icc-es.org](http://www.icc-es.org).

### **Bearing Information:**

The bearing area factor,  $C_b$ , is considered for the allowable capacity of solid sawn wood bearings supporting trusses that are located a minimum of 3" from the end of the lumber piece.

## **General Notes** (continued)

### **Coated Lumber:**

Coated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Coated lumber has no adjustments to lumber properties. Coated lumber may be more brittle than uncoated lumber. Special handling care must be taken to prevent breakage during all handling activities. Refer to manufacturer literature, specifications, and code evaluation reports for restrictions, details, and requirements.

### **Fire Retardant Treated Lumber:**

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

### **Key to Terms:**

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

C = Coated lumber.

C-AT = AtTEK coated lumber.

C-FX = FX Lumber Guard coated lumber.

C -TE = TechWood 4400 coated lumber.

CL = Certified lumber.

Des Ld = total of TCDL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-BF = Borafume Fire Retardant Treated lumber

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-ON = OnWood Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

FRT-PR = ProWood Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for all load cases.

Max Web CSI = Maximum bending and axial Combined Stress Index for Webs for all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

## **General Notes** (continued)

### **Key to Terms** (continued):

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.

W = Width of non-hanger bearing, in inches.

Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

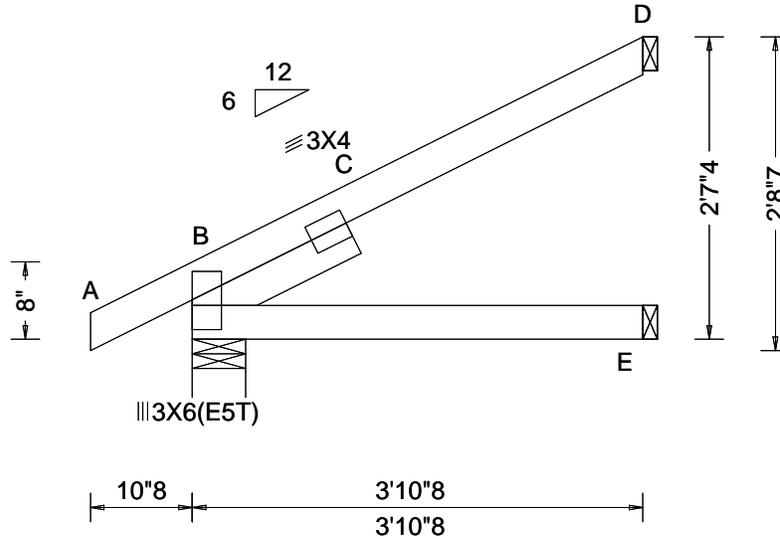
### **References:**

1. AWC: American Wood Council; 222 Catoclin Circle SE, Suite 201; Leesburg, VA 20175; [www.awc.org](http://www.awc.org).
2. ICC: International Code Council; [www.iccsafe.org](http://www.iccsafe.org).
3. Alpine, a division of ITW Building Components Group Inc.: 155 Harlem Ave, North Building, 4th Floor, Glenview, IL 60025; [www.alpineitw.com](http://www.alpineitw.com).
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; [www.tpinst.org](http://www.tpinst.org).
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; [www.sbcacomponents.com](http://www.sbcacomponents.com)



SEQN: 5719 EJAC Ply: 1 Job Number: PM000117  
 FROM: Qty: 11 Customer - Clayton Properties Plan Name - Honeydew Elevation - Farmhouse 2Car  
 Truss Label: J2

Cust: R 9646 JRef: 1 YG096460001 T10  
 DrwNo: 012-26-1214-55703  
 BM 01/12/2026



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)						
				Gravity			Non-Gravity			
TCLL: 25.00	Speed: 115 mph	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Loc	R+	/R-	/Rh	/Rw	/U	/RL
TCDL: 10.00	Risk Category: II	Pf(ASD): NA	VERT(LL): NA	B	249	-	-	/112	-	/67
BCLL: 0.00	Enclosure: Part. Enc.	Ce: NA Lu: NA	VERT(CL): NA	E	47	-	-	/42	-	-
BCDL: 10.00	EXP: B Kzt: NA	Cs: NA Snow Duration: NA	HORZ(LL): 0.017 C - -	D	132	-	-	/76	/45	-
Des Ld: 45.00	TCDL: 6.0 psf	Building Code:	HORZ(TL): 0.026 C - -	Wind reactions based on MWFRS						
NCBCLL: 0.00	BCDL: 6.0 psf	IRC 2018	Creep Factor: 2.0	B Brg Wid = 5.5 Min Req = 1.5 (Support)						
Soffit: 0.00	Mean Height: 20.00 ft	Load Std: ASCE 7-16	Max TC CSI: 0.316	E Brg Wid = 1.5 Min Req = -						
Load Duration: 1.15	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max BC CSI: 0.154	D Brg Wid = 1.5 Min Req = -						
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Fac: Yes	Max Web CSI: 0.083	Bearing B Fcperp = 425psi.						
	Loc. from endwall: not in 4.50 ft	FT/RT:20(0)/10(0)	VIEW Ver: 25.02.00B.1125.14	Members not listed have forces less than 375#						
	GCpi: 0.55	Plate Type(s):		<b>Maximum Top Chord Forces Per Ply (lbs)</b>						
	Wind Duration: 1.60	WAVE		Chords	Tens.	Comp.				

**Lumber**

Top chord: 2x4 SP #2;  
 Bot chord: 2x4 SP #2;  
 Lt Slider: 2x4 SP #2; block length = 1.500'

**Wind**

Wind loads based on MWFRS with additional C&C member design.  
 Wind loading based on both gable and hip roof types.

**Snow**

Overhang designed for 2.00X TC LL.

**Additional Notes**

Top Chord overhang(s) may be field trimmed.



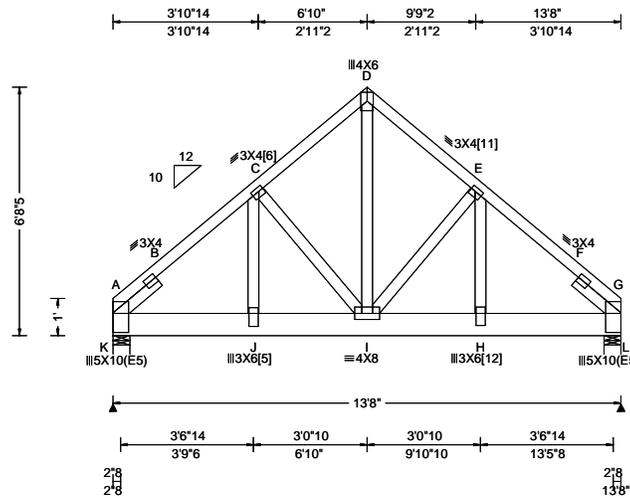
**\*\*WARNING\*\* READ AND FOLLOW ALL NOTES ON THIS DRAWING!**  
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 Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.  
 For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org



SEQN: 5713      COMN      Ply: 3      Job Number: PM000117  
 FROM:                      Qty: 1      Customer - Clayton Properties Plan Name - Honeydew Elevation - Farmhouse 2Car  
 Truss Label: D2

Cust: R 9646      JRef: 1YQ96460001 111  
 Draw: 012-26-1214-417-10  
 BM      01/12/2026

3 Complete Trusses Required



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)						
				Gravity			Non-Gravity			
TCLL: 25.00	Speed: 115 mph	Pg: NA    Ct: NA    CAT: NA	PP Deflection in loc L/defl L/#	Loc	R+	/R-	/Rh	/Rw	/U	/RL
TCDL: 10.00	Risk Category: II	Pf(ASD): NA	VERT(LL): 0.035 I    999    240	K	5981	-	-	-	/449	-
BCLL: 0.00	Enclosure: Part. Enc.	Ce: NA    Lu: NA	VERT(CL): 0.063 I    999    180	L	5724	-	-	-	/431	-
BCDL: 10.00	EXP: B    Kzt: NA	Cs: NA    Snow Duration: NA	HORZ(LL): 0.014 C    -    -	Wind reactions based on MWFRS						
Des Ld: 45.00	TCDL: 6.0 psf	Building Code: IRC 2018	HORZ(TL): 0.024 C    -    -	K    Brg Wid = 5.5    Min Req = 2.9 (Support)						
NCBCLL: 0.00	BCDL: 6.0 psf	Load Std: ASCE 7-16	Creep Factor: 2.0	L    Brg Wid = 5.5    Min Req = 2.8 (Support)						
Soffit: 0.00	Mean Height: 22.43 ft	TPI Std: 2014	Max TC CSI: 0.158	Bearings K & L    Fcperp = 425psi.						
Load Duration: 1.15	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Yes	Max BC CSI: 0.406	Members not listed have forces less than 375#						
Spacing: 24.0 "	C&C Dist a: 3.00 ft	FT/RT: 20(0)/10(0)	Max Web CSI: 0.480	<b>Maximum Top Chord Forces Per Ply (lbs)</b>						
	Loc. from endwall: Any	Plate Type(s): WAVE	VIEW Ver: 25.02.00B.1125.14	Chords	Tens.Comp.	Chords	Tens. Comp.			
	GCpi: 0.55			A - B	173	-2266	D - E	146	-1622	
	Wind Duration: 1.60			B - C	172	-2247	E - F	170	-2228	
				C - D	146	-1622	F - G	172	-2247	

**Lumber**  
 Top chord: 2x4 SP #2;  
 Bot chord: 2x8 SP #1;  
 Webs: 2x4 SP #2;  
 Lt Slider: 2x4 SP #2; block length = 1.500'  
 Rt Slider: 2x4 SP #2; block length = 1.500'

**Blocking**  
 Apply additional nailing over the following bearings with fasteners at 9" oc perpendicular to grain and 4" oc parallel to grain. In lieu of additional nailing, apply blocking reinforcement to prevent buckling of members over the bearings:  
 Bearing 1 located at 0.0' (blocking >= 3.50" if used)

**Nailnote**  
 Nail Schedule: 0.128"x3", min. nails  
 Top Chord: 1 Row @ 12.00" o.c.  
 Bot Chord: 2 Rows @ 3.50" o.c. (Each Row)  
 Webs : 1 Row @ 4" o.c.  
 Repeat nailing as each layer is applied. Use equal spacing between rows and stagger nails in each row to avoid splitting.

**Special Loads**  
 -----(Lumber Dur.Fac.=1.15 / Plate Dur.Fac.=1.15)  
 TC: From 76 plf at 0.00 to 76 plf at 13.67  
 BC: From 10 plf at 0.00 to 10 plf at 13.67  
 BC: 1755 lb Conc. Load at 1.67, 3.67, 5.67, 7.67, 9.67, 11.67

**Plate Shift Table**

JT No	Plate Size	Lateral Shift	Chord	JT No	Plate Size	Lateral Shift	Chord
[5]	3X6	S	4.25 [6]	[3X4]	2.50	L	1.25
[11]	3X4	2.50	R 1.25 [12]	3X6	S	4.00	

**Wind**  
 Wind loads and reactions based on MWFRS.  
 Wind loading based on both gable and hip roof types.



**\*\*WARNING\*\* READ AND FOLLOW ALL NOTES ON THIS DRAWING!**  
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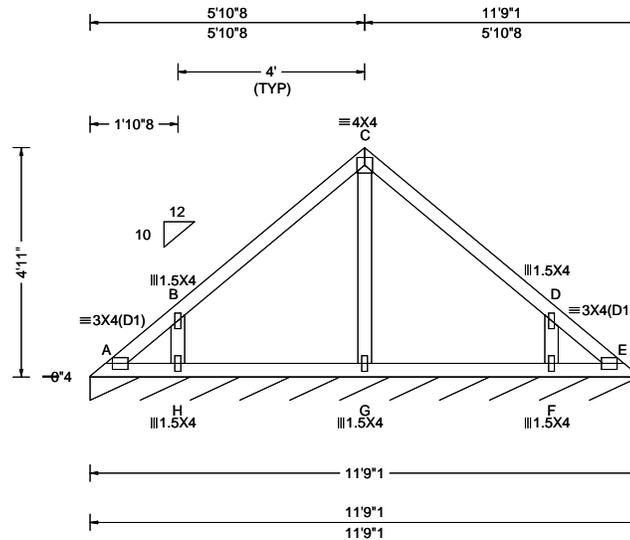






SEQN: 5728 VAL Ply: 1 Job Number: PM000117  
 FROM: Qty: 1 Customer - Clayton Properties Plan Name - Honeydew Elevation - Farmhouse 2Car  
 Truss Label: V6

Cust: R 9646 JRef: 1VQ96460001 T14  
 Draw: 012-26.1213-13983  
 BM 01/12/2026



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF						
				Gravity			Non-Gravity			
TCLL: 25.00	Speed: 115 mph	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Loc	R+	/R-	/Rh	/Rw	/U	/RL
TCDL: 10.00	Risk Category: II	Pf(ASD): NA	VERT(LL): 0.001 C 999 240	E*	96	/-	/-	/53	/4	/7
BCLL: 0.00	Enclosure: Part. Enc.	Ce: NA Lu: NA	VERT(CL): 0.001 C 999 180	Wind reactions based on MWFRS						
BCDL: 10.00	EXP: B Kzt: NA	Cs: NA Snow Duration: NA	HORZ(LL): -0.001 A - -	E Brg Wid = 141 Min Req = -						
Des Ld: 45.00	TCDL: 6.0 psf	Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	HORZ(TL): 0.002 B - -	Bearing A Fcperp = 425psi.						
NCBCLL: 10.00	BCDL: 6.0 psf		Creep Factor: 2.0	Members not listed have forces less than 375#						
Soffit: 0.00	Mean Height: 22.97 ft		Max TC CSI: 0.275							
Load Duration: 1.15	MWFRS Parallel Dist: 0 to h/2		Max BC CSI: 0.119							
Spacing: 24.0 "	C&C Dist a: 3.00 ft		Max Web CSI: 0.068							
	Loc. from endwall: Any		VIEW Ver: 25.02.00B.1125.14							
	GCpi: 0.55									
	Wind Duration: 1.60									

**Lumber**

Top chord: 2x4 SP #2;  
 Bot chord: 2x4 SP #2;  
 Webs: 2x4 SP #2;

**Loading**

Bottom chord checked for 10.00 psf non-concurrent live load.

**Wind**

Wind loads based on MWFRS with additional C&C member design.  
 Wind loading based on both gable and hip roof types.

**Additional Notes**

See DWG VAL180160118 for valley details.



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SEQN: 5714	SPEC	Ply: 1	Job Number: PM000117
FROM:		Qty: 4	Customer - Clayton Properties Plan Name - Honeydew Elevation - Farmhouse 2Car
Page 2 of 2			Truss Label: E2

Cust: R9646 JRef: 1VQ96460001 T15  
 DrwNo: 012-26.1214-46267  
 BM 01/12/2026

**Hangers / Ties**

Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.

Recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information. Additional connection required to evenly distribute hanger reaction throughout all plies of supporting girder.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

Bearing at location x=0' uses the following support conditions: 0'

Bearing A (0', 18'7") HUS28

Supporting Member: (3)2x8 SP 2400f-2.0E

(22) 0.148"x3" nails into supporting member,

(4) 0.148"x3" nails into supported member.



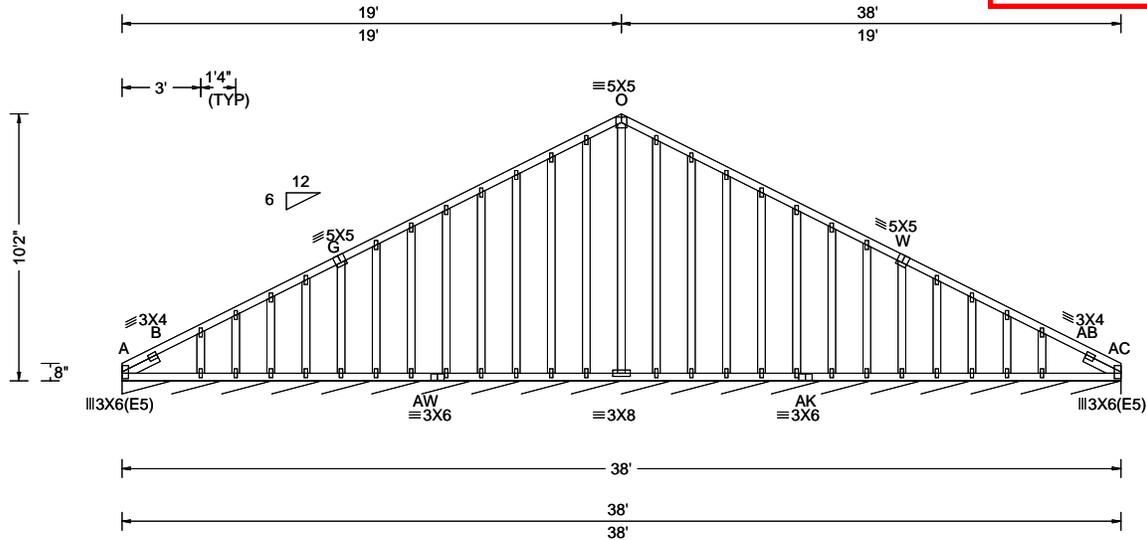
MISSOURI COA #2005000817  
 01/12/2026

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SEQN: 5733 GABL Ply: 1 Job Number: PM000117  
 FROM: Qty: 1 Customer - Clayton Properties Plan Name - Honeydew Elevation - Farmhouse 2Car  
 Truss Label: E5

Cust: R 9646 JRef: 1VGO96460001 116  
 DrwNo: 012-26.1214.50680  
 BM 01/12/2026



<b>Loading Criteria (psf)</b> TCCL: 25.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	<b>Wind Criteria</b> Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCCL: 6.0 psf BCDL: 6.0 psf Mean Height: 24.00 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.80 ft Loc. from endwall: Any GCpi: 0.55 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.011 B 999 364 VERT(CL): 0.017 B 999 364 HORZ(LL): 0.006 B - - HORZ(TL): 0.014 AB - - Creep Factor: 2.0 Max TC CSI: 0.221 Max BC CSI: 0.075 Max Web CSI: 0.443 VIEW Ver: 25.02.00B.1125.14	<b>▲ Maximum Reactions (lbs), or *=PLF</b> Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL AC*127 /- /- /64 /13 /5 Wind reactions based on MWFRS AC Brg Wid = 456 Min Req = - Bearing A Fcperp = 425psi. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp. G - O 386 -109 O - W 380 -102

**Lumber**  
 Top chord: 2x4 SP #2;  
 Bot chord: 2x4 SP #2;  
 Webs: 2x4 SP #2;  
 Lt Slider: 2x4 SP #2; block length = 1.500'  
 Rt Slider: 2x4 SP #2; block length = 1.500'

**Plating Notes**  
 All plates are 1.5X4 except as noted.

**Loading**  
 Truss designed to support 1-4-0 top chord outlookers and cladding load not to exceed 3.00 PSF one face and 24.0' span opposite face. Top chord must not be cut or notched, unless specified otherwise.  
 Bottom chord checked for 10.00 psf non-concurrent live load.

**Wind**  
 Wind loads based on MWFRS with additional C&C member design.  
 Wind loading based on both gable and hip roof types.

**Additional Notes**  
 See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

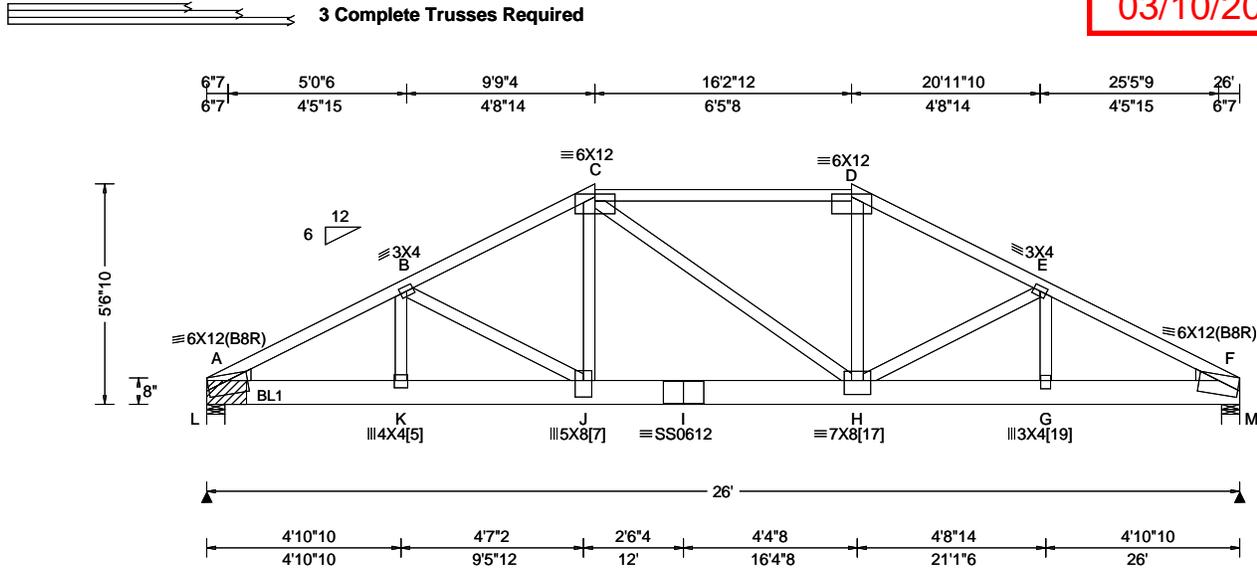


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SEQN: 5711 HIPS Ply: 3 Job Number: PM000117  
 FROM: Qty: 1 Customer - Clayton Properties Plan Name - Honeydew Elevation - Farmhouse 2Car  
 Truss Label: C4

Cust: R 9646 JRef: 1 YG096460001 118  
 DrwNo: 012-26-1214-36430  
 Date: 01/12/2026



**Loading Criteria (psf)**

TCLL:	25.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	45.00
NCBCLL:	0.00
Soffit:	0.00
Load Duration:	1.15
Spacing:	24.0 "

**Wind Criteria**

Speed:	115 mph
Risk Category:	II
Enclosure:	Part. Enc.
EXP:	B Kzt: NA
TCDL:	6.0 psf
BCDL:	6.0 psf
Mean Height:	21.69 ft
MWFRS Parallel Dist:	0 to h/2
C&C Dist a:	3.00 ft
Loc. from endwall:	not in 9.00 ft
GCpi:	0.55
Wind Duration:	1.60

**Snow Criteria (Pg,Pf in PSF)**

Pg:	NA	Ct:	NA	CAT:	NA
Pf(ASD):	NA	Ce:	NA	Lu:	NA
Cs:	NA	Snow Duration:	NA		
Building Code:	IRC 2018				
Load Std:	ASCE 7-16				
TPI Std:	2014				
Rep Fac:	Yes				
FT/RT:	20(0)/10(0)				
Plate Type(s):	WAVE, 18SS				

**Defl/CSI Criteria**

PP Deflection in loc L/defl L/#	
VERT(LL):	0.214 H 999 246
VERT(CL):	0.383 H 805 246
HORZ(LL):	0.046 B - -
HORZ(TL):	0.082 B - -
Creep Factor:	2.0
Max TC CSI:	0.878
Max BC CSI:	0.505
Max Web CSI:	0.575
VIEW Ver:	25.02.00B.1125.14

**Maximum Reactions (lbs)**

Loc	Gravity			Non-Gravity		
	R+	/R-	/Rh	/Rw	/U	/RL
L	11553	-	-	-	/888	-
M	10591	-	-	-	/829	-

Wind reactions based on MWFRS  
 L Brg Wid = 5.5 Min Req = -  
 M Brg Wid = 5.5 Min Req = 5.1 (Support)  
 Bearings L & M Fcperp = 425psi.  
 Members not listed have forces less than 375#

**Maximum Top Chord Forces Per Ply (lbs)**

Chords	Tens.Comp.	Chords	Tens. Comp.
A - B	500 -6494	D - E	438 -5691
B - C	442 -5751	E - F	492 -6306
C - D	384 -5036		

**Lumber**  
 Top chord: 2x4 SP #2;  
 Bot chord: 2x8 SP 2400f-2.0E;  
 Webs: 2x4 SP #2;  
 Lt Wedge: 2x4 SP #2; Rt Wedge: 2x4 SP #2;

**Nailnote**  
 Nail Schedule: 0.128"x3", min. nails  
 Top Chord: 1 Row @ 12.00" o.c.  
 Bot Chord: 2 Rows @ 3.50" o.c. (Each Row)  
 Webs : 1 Row @ 4" o.c.  
 Repeat nailing as each layer is applied. Use equal spacing between rows and stagger nails in each row to avoid splitting.

**Wind**  
 Wind loads and reactions based on MWFRS.  
 Wind loading based on both gable and hip roof types.

**Bearing Block(s)**  
 Brg blocks: 0.128"x3", min. nails  
 brg x-loc #blocks length/blk #nails/blk wall plate  
 1 0.000' 1 12" 4 SPF Standard  
 Brg block to be same size and species as chord.  
 Refer to drawing CNNAILSP1014 for more information.

**Maximum Bot Chord Forces Per Ply (lbs)**

Chords	Tens.Comp.	Chords	Tens. Comp.
A - K	5765 -441	I - H	5022 -383
K - J	5744 -440	H - G	5578 -432
J - I	5022 -383	G - F	5595 -433

**Maximum Web Forces Per Ply (lbs)**

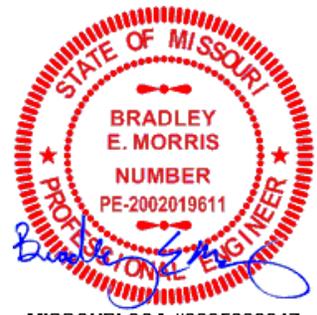
Webs	Tens.Comp.	Webs	Tens. Comp.
K - B	686 -34	D - H	2326 -151
B - J	55 -683	H - E	50 -548
J - C	2344 -153	E - G	557 -29

**Special Loads**  
 -----(Lumber Dur.Fac.=1.15 / Plate Dur.Fac.=1.15)  
 TC: From 72 plf at 0.00 to 72 plf at 26.00  
 BC: From 10 plf at 0.00 to 10 plf at 26.00  
 BC: 1755 lb Conc. Load at 2.00, 4.00, 6.00, 8.00  
 10.00, 12.00, 14.00, 16.00, 18.00, 20.00  
 BC: 875 lb Conc. Load at 20.94  
 BC: 789 lb Conc. Load at 22.00, 24.00

**Plate Shift Table**

JT No	Plate Size	Lateral Shift	Chord Bite	JT No	Plate Size	Lateral Shift	Chord Bite
[5]	4X4	S	2.25	[7]	5X8	S	5.00
[17]	7X8	S	4.25	[19]	3X4	S	2.50

**Purlins**  
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.



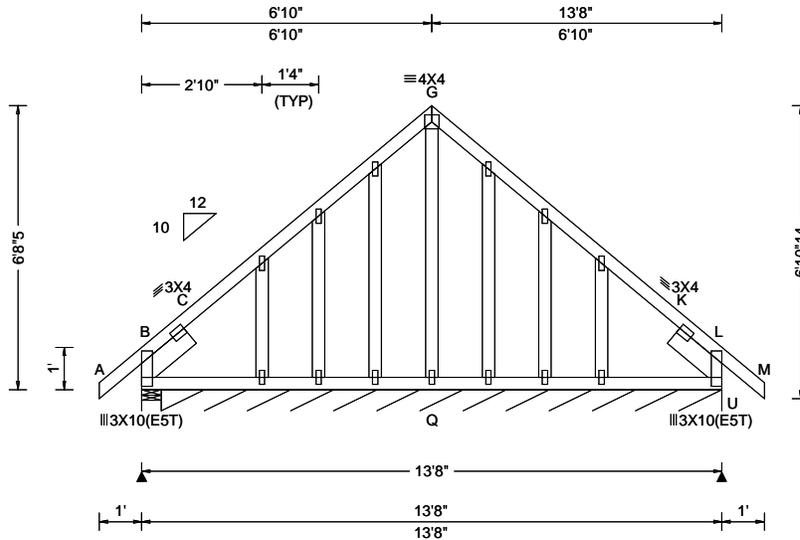
MISSOURI COA #2005000817  
 01/12/2026

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SEQN: 5712 FROM:	GABL Qty: 1	Ply: 1 Qty: 1	Job Number: PM000117 Customer - Clayton Properties Plan Name - Honeydew Elevation - Farmhouse 2Car Truss Label: D1
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Cust: R 9646 JRef: TVG096460001 T19  
 DrwNo: 012-26.1214.59900  
 BM 01/12/2026



<b>Loading Criteria (psf)</b> TCLL: 25.00 TCDL: 10.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	<b>Wind Criteria</b> Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCDL: 6.0 psf BCDL: 6.0 psf Mean Height: 22.01 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.55 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.005 K 999 240 VERT(CL): 0.012 C 999 180 HORZ(LL): 0.006 C - - HORZ(TL): 0.017 K - - Creep Factor: 2.0 Max TC CSI: 0.146 Max BC CSI: 0.063 Max Web CSI: 0.145 VIEW Ver: 25.02.00B.1125.14	<b>▲ Maximum Reactions (lbs), or *=PLF</b> <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>355</td> <td>-</td> <td>-</td> <td>/191</td> <td>/21</td> <td>/135</td> </tr> <tr> <td>U*</td> <td>121</td> <td>-</td> <td>-</td> <td>/68</td> <td>/11</td> <td>-</td> </tr> </tbody> </table> Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.5 (Support) U Brg Wid = 158 Min Req = - Bearings B & B Fcperp = 425psi. Members not listed have forces less than 375# <b>Maximum Gable Forces Per Ply (lbs)</b> Gables Tens.Comp. G - Q 88 -411	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	B	355	-	-	/191	/21	/135	U*	121	-	-	/68	/11	-
Loc	Gravity			Non-Gravity																											
	R+	/R-	/Rh	/Rw	/U	/RL																									
B	355	-	-	/191	/21	/135																									
U*	121	-	-	/68	/11	-																									

**Lumber**  
 Top chord: 2x4 SP #2;  
 Bot chord: 2x4 SP #2;  
 Webs: 2x4 SP #2;  
 Lt Slider: 2x6 SP #2; block length = 1.500'  
 Rt Slider: 2x6 SP #2; block length = 1.500'

**Plating Notes**  
 All plates are 1.5X4 except as noted.

**Loading**  
 Truss designed to support 1-4-0 top chord outlookers and cladding load not to exceed 3.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.  
 Bottom chord checked for 10.00 psf non-concurrent live load.

**Wind**  
 Wind loads based on MWFRS with additional C&C member design.  
 Wind loading based on both gable and hip roof types.

**Snow**  
 Overhang designed for 2.00X TC LL.

**Additional Notes**  
 See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.  
 Top Chord overhang(s) may be field trimmed.

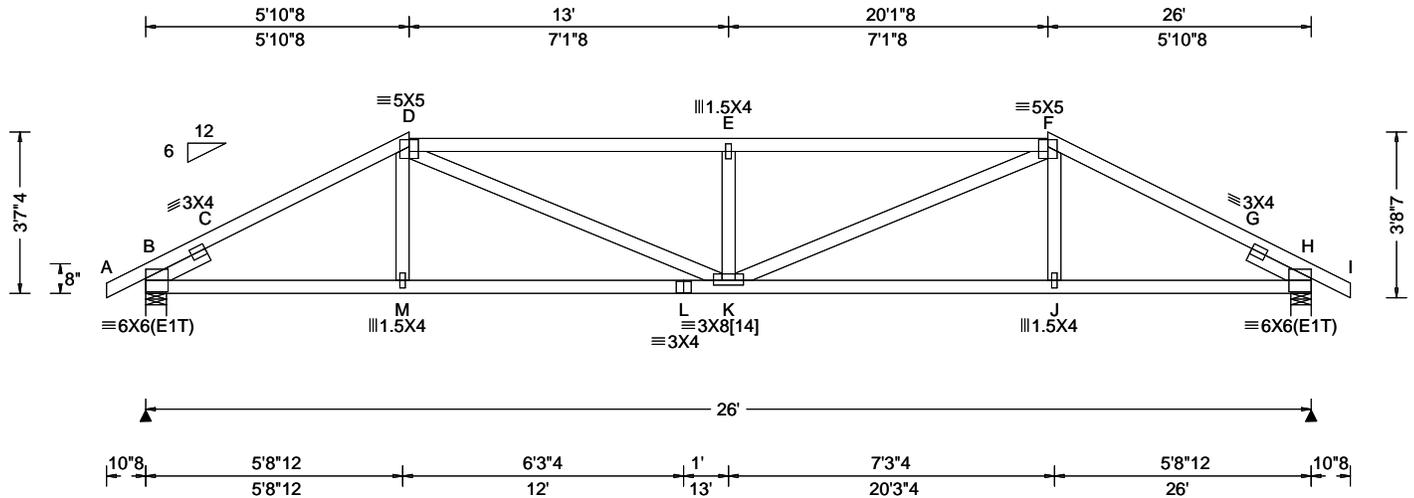


MISSOURI COA #2005000817  
 01/12/2026

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SEQN: 5709 FROM:	HIPS Qty: 1	Ply: 1 Qty: 1	Job Number: PM000117 Customer - Clayton Properties Plan Name - Honeydew Elevation - Farmhouse 2Car Truss Label: C2	Cust: R 9646 JRef: 1VQ96460001 12 Drawn: 01/22/2024 12:14:52 J80 BM 01/12/2024
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<b>Loading Criteria (psf)</b> TCCL: 25.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	<b>Wind Criteria</b> Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCCL: 6.0 psf BCDL: 6.0 psf Mean Height: 20.50 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.55 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.140 E 999 249 VERT(CL): 0.260 E 999 249 HORZ(LL): 0.037 H - - HORZ(TL): 0.068 H - - Creep Factor: 2.0 Max TC CSI: 0.858 Max BC CSI: 0.635 Max Web CSI: 0.285 VIEW Ver: 25.02.00B.1125.14	<b>▲ Maximum Reactions (lbs)</b> Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 1265 /- /- /678 /88 /40 H 1265 /- /- /678 /88 /- Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.6 (Support) H Brg Wid = 5.5 Min Req = 1.6 (Support) Bearings B & H Fcperp = 425psi. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp. B - C 470 -2138 E - F 684 -2583 C - D 462 -2058 F - G 463 -2058 D - E 684 -2583 G - H 470 -2138
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**Lumber**

Top chord: 2x4 SP #2;  
 Bot chord: 2x4 SP #2;  
 Webs: 2x4 SP #2;  
 Lt Slider: 2x4 SP #2; block length = 1.500'  
 Rt Slider: 2x4 SP #2; block length = 1.500'

**Plate Shift Table**

JT No	Plate Size	Lateral Shift	Chord Bite	JT No	Plate Size	Lateral Shift	Chord Bite
[14]	3X8	S	1.25				

**Loading**

Bottom chord checked for 10.00 psf non-concurrent live load.

**Purlins**

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

**Wind**

Wind loads based on MWFRS with additional C&C member design.  
 Wind loading based on both gable and hip roof types.

**Snow**

Overhang designed for 2.00X TC LL.

**Additional Notes**

Top Chord overhang(s) may be field trimmed.



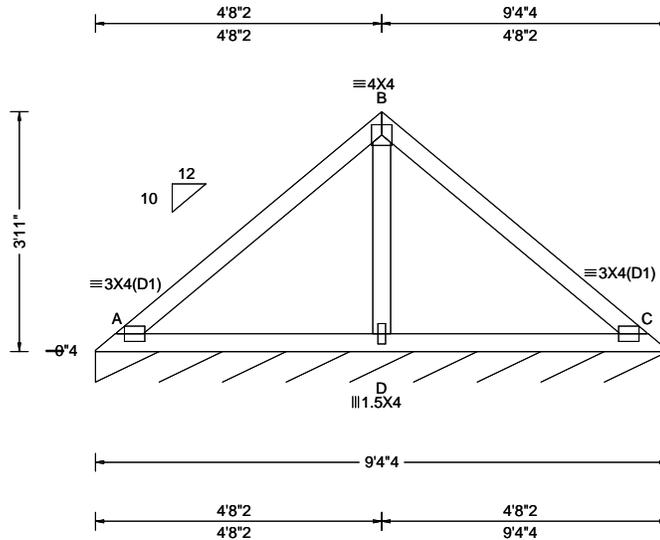
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SEQN: 5729	VAL	Ply: 1	Job Number: PM000117	Cust: R 9646 JRef: TVG096460001 T21 FROM: Qty: 1 Customer - Clayton Properties Plan Name - Honeydew Elevation - Farmhouse 2Car Truss Label: V7
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Cust: R 9646 JRef: TVG096460001 T21  
 DrwNo: 012-26.1213-15287  
 BM 01/12/2026



<b>Loading Criteria (psf)</b> TCLL: 25.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	<b>Wind Criteria</b> Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCCL: 6.0 psf BCDL: 6.0 psf Mean Height: 23.47 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.55 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.012 C 999 240 VERT(CL): 0.021 C 999 180 HORZ(LL): -0.007 C - - HORZ(TL): 0.013 C - - Creep Factor: 2.0 Max TC CSI: 0.387 Max BC CSI: 0.262 Max Web CSI: 0.132 VIEW Ver: 25.02.00B.1125.14	<b>▲ Maximum Reactions (lbs), or *=PLF</b> <table style="width: 100%; border-collapse: collapse;"> <thead> <tr> <th colspan="2"></th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>Loc</th> <th>R+</th> <th>/ R-</th> <th>/ Rh</th> <th>/ Rw</th> <th>/ U</th> <th>/ RL</th> </tr> </thead> <tbody> <tr> <td>C*</td> <td>96</td> <td>/-</td> <td>/-</td> <td>/53</td> <td>/4</td> <td>/6</td> </tr> </tbody> </table> Wind reactions based on MWFRS C Brg Wid = 112 Min Req = - Bearing A Fcperp = 425psi. Members not listed have forces less than 375# <b>Maximum Web Forces Per Ply (lbs)</b> <table style="width: 100%; border-collapse: collapse;"> <thead> <tr> <th>Webs</th> <th>Tens.Comp.</th> </tr> </thead> <tbody> <tr> <td>B - D</td> <td>191 -565</td> </tr> </tbody> </table>			Gravity			Non-Gravity			Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL	C*	96	/-	/-	/53	/4	/6	Webs	Tens.Comp.	B - D	191 -565
		Gravity			Non-Gravity																									
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL																								
C*	96	/-	/-	/53	/4	/6																								
Webs	Tens.Comp.																													
B - D	191 -565																													

**Lumber**

Top chord: 2x4 SP #2;  
 Bot chord: 2x4 SP #2;  
 Webs: 2x4 SP #2;

**Loading**

Bottom chord checked for 10.00 psf non-concurrent live load.

**Wind**

Wind loads based on MWFRS with additional C&C member design.  
 Wind loading based on both gable and hip roof types.

**Additional Notes**

See DWG VAL180160118 for valley details.

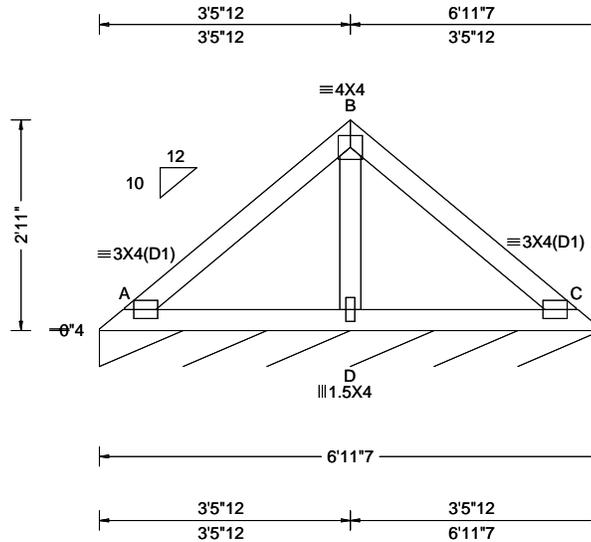


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SEQN: 5730 VAL Ply: 1 Job Number: PM000117  
 FROM: Qty: 1 Customer - Clayton Properties Plan Name - Honeydew Elevation - Farmhouse 2Car  
 Truss Label: V8

Cust: R 9646 JRef: TVG096460001 T22  
 Draw: 012-26.1213-16360  
 BM 01/12/2026



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 25.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCDL: 6.0 psf BCDL: 6.0 psf Mean Height: 23.97 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.55 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.005 C 999 240 VERT(CL): 0.009 C 999 180 HORZ(LL): -0.003 C - - HORZ(TL): 0.005 C - - Creep Factor: 2.0 Max TC CSI: 0.195 Max BC CSI: 0.133 Max Web CSI: 0.054  VIEW Ver: 25.02.00B.1125.14	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL C* 95 /- /- /52 /4 /6 Wind reactions based on MWFRS C Brg Wid = 83.5 Min Req = - Bearing A Fcperp = 425psi. Members not listed have forces less than 375#

**Lumber**

Top chord: 2x4 SP #2;  
 Bot chord: 2x4 SP #2;  
 Webs: 2x4 SP #2;

**Loading**

Bottom chord checked for 10.00 psf non-concurrent live load.

**Wind**

Wind loads based on MWFRS with additional C&C member design.  
 Wind loading based on both gable and hip roof types.

**Additional Notes**

See DWG VAL180160118 for valley details.



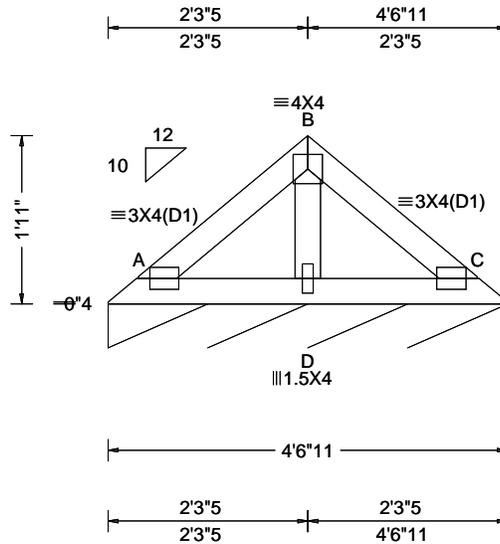
MISSOURI COA #2005000817  
 01/12/2026

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SEQN: 5731 VAL Ply: 1 Job Number: PM000117  
 FROM: Qty: 1 Customer - Clayton Properties Plan Name - Honeydew Elevation - Farmhouse 2Car  
 Truss Label: V9

Cust: R 9646 JRef: 1 VGO96460001 123  
 DrwNo: 012-26.1219.22470  
 BM 01/12/2026



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF						
				Gravity			Non-Gravity			
TCLL: 25.00	Speed: 115 mph	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Loc	R+	/R-	/Rh	/Rw	/U	/RL
TCDL: 10.00	Risk Category: II	Pf(ASD): NA	VERT(LL): 0.001 C 999 240	C*	95	/-	/-	/50	/3	/6
BCLL: 0.00	Enclosure: Part. Enc.	Ce: NA Lu: NA	VERT(CL): 0.002 C 999 180	Wind reactions based on MWFRS						
BCDL: 10.00	EXP: B Kzt: NA	Cs: NA Snow Duration: NA	HORZ(LL): -0.001 C - -	C Brg Wid = 54.7 Min Req = -						
Des Ld: 45.00	TCDL: 6.0 psf	Building Code:	HORZ(TL): 0.002 C - -	Bearing A Fcperp = 425psi.						
NCBCLL: 0.00	BCDL: 6.0 psf	IRC 2018	Creep Factor: 2.0	Members not listed have forces less than 375#						
Soffit: 0.00	Mean Height: 24.47 ft	Load Std: ASCE 7-16	Max TC CSI: 0.069							
Load Duration: 1.15	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max BC CSI: 0.048							
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Fac: Yes	Max Web CSI: 0.023							
	Loc. from endwall: not in 9.00 ft	FT/RT:20(0)/10(0)	VIEW Ver: 25.02.00B.1125.14							
	GCpi: 0.55	Plate Type(s):								
	Wind Duration: 1.60	WAVE								

**Lumber**

Top chord: 2x4 SP #2;  
 Bot chord: 2x4 SP #2;  
 Webs: 2x4 SP #2;

**Wind**

Wind loads based on MWFRS with additional C&C member design.  
 Wind loading based on both gable and hip roof types.

**Additional Notes**

See DWG VAL180160118 for valley details.



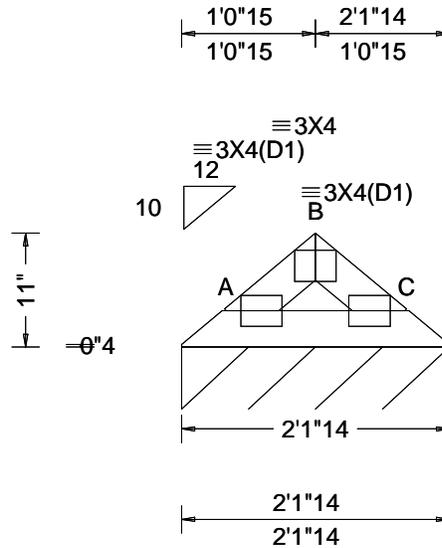
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 01/12/2026

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SEQN: 5723 VAL Ply: 1 Job Number: PM000117  
 FROM: Qty: 1 Customer - Clayton Properties Plan Name - Honeydew Elevation - Farmhouse 2Car  
 Truss Label: V10

Cust: R 9646 JRef: TVG096460001 T24  
 Draw: 012-26-1215-06390  
 BM 01/12/2026



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF						
				Gravity			Non-Gravity			
TCLL: 25.00	Speed: 115 mph	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Loc	R+	/R-	/Rh	/Rw	/U	/RL
TCDL: 10.00	Risk Category: II	Pf(ASD): NA	VERT(LL): 0.001 C 999 240	C*	94	/-	/-	/43	/-	/4
BCLL: 0.00	Enclosure: Part. Enc.	Ce: NA Lu: NA	VERT(CL): 0.001 C 999 180	Wind reactions based on MWFRS						
BCDL: 10.00	EXP: B Kzt: NA	Cs: NA Snow Duration: NA	HORZ(LL): -0.000 A - -	C Brg Wid = 25.9 Min Req = -						
Des Ld: 45.00	TCDL: 6.0 psf	Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	HORZ(TL): 0.001 A - -	Bearing A Fcperp = 425psi.						
NCBCLL: 0.00	BCDL: 6.0 psf		Creep Factor: 2.0	Members not listed have forces less than 375#						
Soffit: 0.00	Mean Height: 24.97 ft		Max TC CSI: 0.022							
Load Duration: 1.15	MWFRS Parallel Dist: 0 to h/2		Max BC CSI: 0.035							
Spacing: 24.0 "	C&C Dist a: 3.00 ft		Max Web CSI: 0.000							
	Loc. from endwall: not in 9.00 ft		VIEW Ver: 25.02.00B.1125.14							
	GCpi: 0.55									
	Wind Duration: 1.60									

**Lumber**

Top chord: 2x4 SP #2;  
 Bot chord: 2x4 SP #2;

**Wind**

Wind loads based on MWFRS with additional C&C member design.  
 Wind loading based on both gable and hip roof types.

**Additional Notes**

See DWG VAL180160118 for valley details.



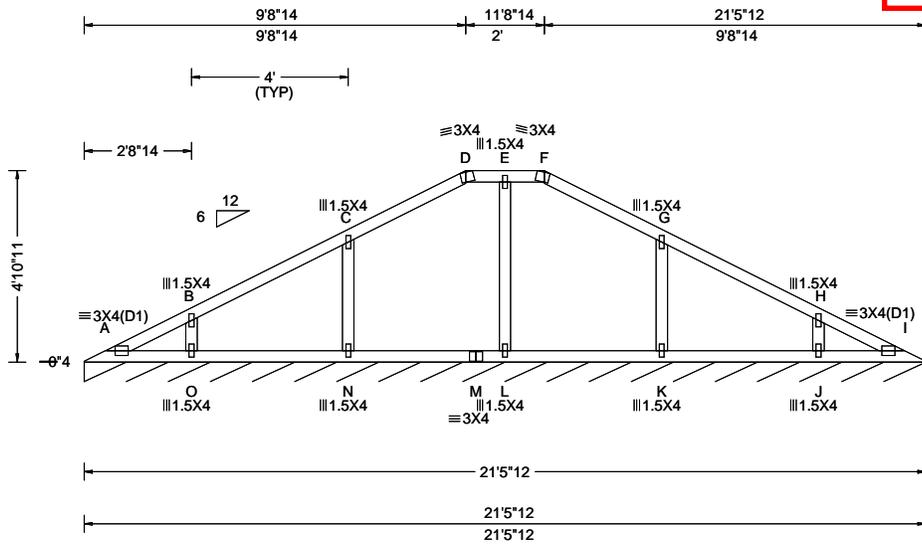
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SEQN: 5722 VAL Ply: 1 Job Number: PM000117  
 FROM: Qty: 1 Customer - Clayton Properties Plan Name - Honeydew Elevation - Farmhouse 2Car  
 Truss Label: V1

Cust: R 9646 JRef: TVG096460001 125  
 Draw: 012-26.1219.05330  
 BM 01/12/2026



<b>Loading Criteria (psf)</b> TCLL: 25.00 TCCL: 10.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	<b>Wind Criteria</b> Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCCL: 6.0 psf BCDL: 6.0 psf Mean Height: 22.96 ft MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.55 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.006 F 999 240 VERT(CL): 0.009 F 999 203 HORZ(LL): 0.002 G - - HORZ(TL): 0.003 G - - Creep Factor: 2.0 Max TC CSI: 0.204 Max BC CSI: 0.118 Max Web CSI: 0.082 VIEW Ver: 25.02.00B.1125.14	<b>▲ Maximum Reactions (lbs), or *=PLF</b> <table border="1"> <thead> <tr> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>Loc</th> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>P*</td> <td>92</td> <td>-</td> <td>-</td> <td>/49</td> <td>/1</td> <td>/3</td> </tr> </tbody> </table> Wind reactions based on MWFRS P Brg Wid = 257 Min Req = - Bearing A Fcperp = 425psi. Members not listed have forces less than 375#	Gravity			Non-Gravity			Loc	R+	/R-	/Rh	/Rw	/U	/RL	P*	92	-	-	/49	/1	/3
Gravity			Non-Gravity																					
Loc	R+	/R-	/Rh	/Rw	/U	/RL																		
P*	92	-	-	/49	/1	/3																		

**Lumber**

Top chord: 2x4 SP #2;  
 Bot chord: 2x4 SP #2;  
 Webs: 2x4 SP #2;

**Loading**

Bottom chord checked for 10.00 psf non-concurrent live load.

**Purlins**

In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

**Wind**

Wind loads based on MWFRS with additional C&C member design.  
 Wind loading based on both gable and hip roof types.

**Additional Notes**

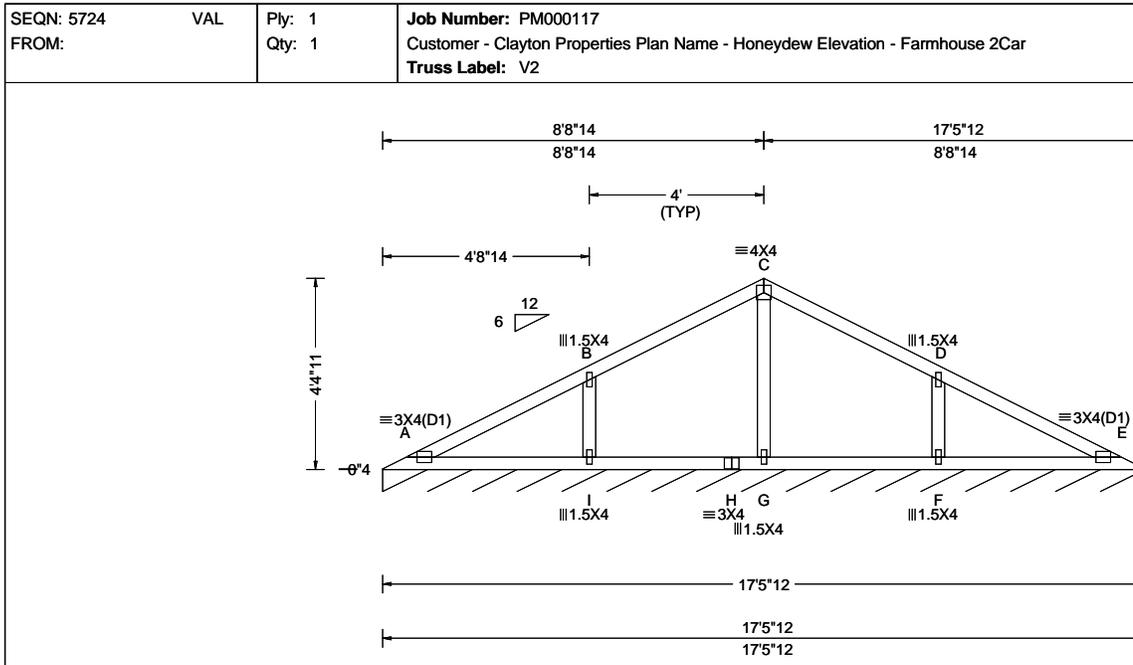
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<b>Loading Criteria (psf)</b> TCCL: 25.00 TCDL: 10.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	<b>Wind Criteria</b> Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCDL: 6.0 psf BCDL: 6.0 psf Mean Height: 23.71 ft MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.55 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.012 E 999 240 VERT(CL): 0.023 A 999 180 HORZ(LL): -0.004 E - - HORZ(TL): 0.008 E - - Creep Factor: 2.0 Max TC CSI: 0.426 Max BC CSI: 0.189 Max Web CSI: 0.100 VIEW Ver: 25.02.00B.1125.14	<b>▲ Maximum Reactions (lbs), or *=PLF</b> <table border="1"> <thead> <tr> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>Loc</th> <th>R+</th> <th>/ R-</th> <th>/ Rh</th> <th>/ Rw</th> <th>/ U</th> <th>/ RL</th> </tr> </thead> <tbody> <tr> <td>J*</td> <td>92</td> <td>/-</td> <td>/-</td> <td>/49</td> <td>/1</td> <td>/3</td> </tr> </tbody> </table> Wind reactions based on MWFRS J Brg Wid = 209 Min Req = - Bearing A Fcperp = 425psi. Members not listed have forces less than 375#	Gravity			Non-Gravity			Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL	J*	92	/-	/-	/49	/1	/3
Gravity			Non-Gravity																					
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL																		
J*	92	/-	/-	/49	/1	/3																		

**Lumber**

Top chord: 2x4 SP #2;  
 Bot chord: 2x4 SP #2;  
 Webs: 2x4 SP #2;

**Loading**

Bottom chord checked for 10.00 psf non-concurrent live load.

**Wind**

Wind loads based on MWFRS with additional C&C member design.  
 Wind loading based on both gable and hip roof types.

**Additional Notes**

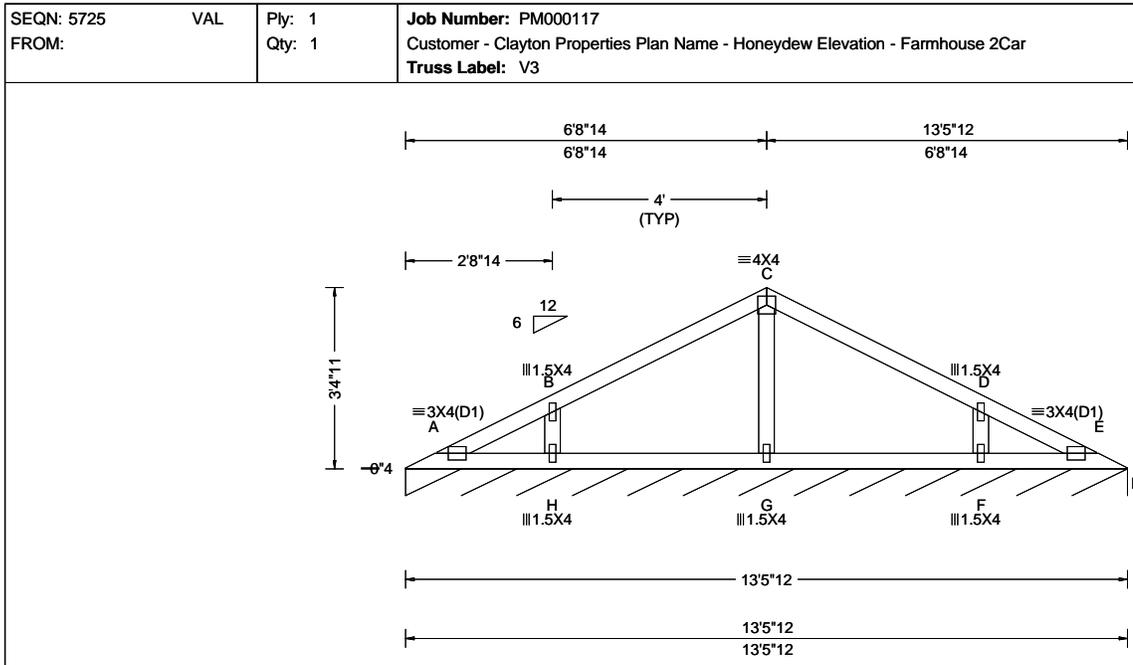
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MISSOURI COA #2005000817  
 01/12/2026

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<b>Loading Criteria (psf)</b> TCCL: 25.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	<b>Wind Criteria</b> Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCCL: 6.0 psf BCDL: 6.0 psf Mean Height: 24.21 ft MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.55 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.001 C 999 240 VERT(CL): 0.001 E 999 180 HORZ(LL): -0.000 A - - HORZ(TL): 0.001 B - - Creep Factor: 2.0 Max TC CSI: 0.252 Max BC CSI: 0.112 Max Web CSI: 0.044 VIEW Ver: 25.02.00B.1125.14	<b>▲ Maximum Reactions (lbs), or *=PLF</b> Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL I* 92 /- /- /49 /1 /3 Wind reactions based on MWFRS I Brg Wid = 161 Min Req = - Bearing A Fcperp = 425psi. Members not listed have forces less than 375#
--	---	---	--	---

**Lumber**

Top chord: 2x4 SP #2;  
 Bot chord: 2x4 SP #2;  
 Webs: 2x4 SP #2;

**Loading**

Bottom chord checked for 10.00 psf non-concurrent live load.

**Wind**

Wind loads based on MWFRS with additional C&C member design.  
 Wind loading based on both gable and hip roof types.

**Additional Notes**

See DWG VAL180160118 for valley details.

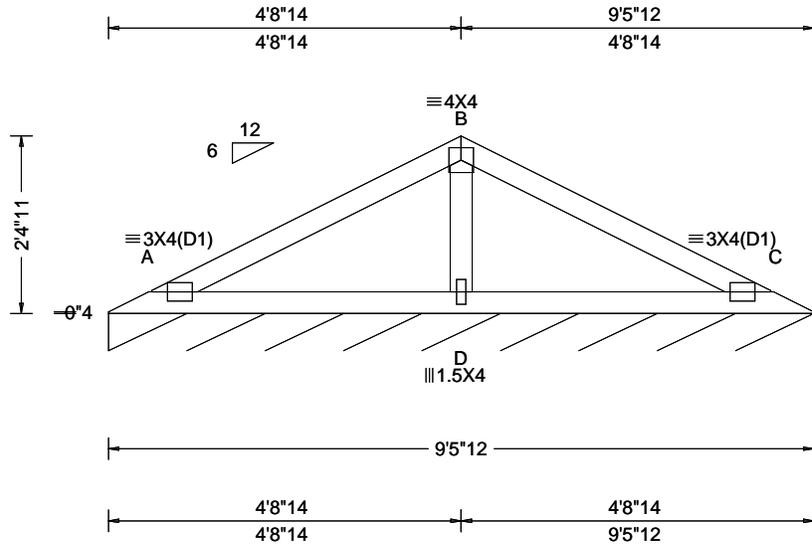


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SEQN: 5726 VAL Ply: 1 Job Number: PM000117  
 FROM: Qty: 1 Customer - Clayton Properties Plan Name - Honeydew Elevation - Farmhouse 2Car  
 Truss Label: V4

Cust: R 9646 JRef: 1VQ96460001 T28  
 DrwNo: 012-26.1213.114.10  
 BM 01/12/2026



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF						
				Gravity			Non-Gravity			
TCLL: 25.00	Speed: 115 mph	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Loc	R+	/R-	/Rh	/Rw	/U	/RL
TCDL: 10.00	Risk Category: II	Pf(ASD): NA	VERT(LL): 0.016 C 999 240	E*	92	/-	/-	/47	/-	/3
BCLL: 0.00	Enclosure: Part. Enc.	Ce: NA Lu: NA	VERT(CL): 0.028 C 999 180	Wind reactions based on MWFRS						
BCDL: 10.00	EXP: B Kzt: NA	Cs: NA Snow Duration: NA	HORZ(LL): -0.007 C - -	E Brg Wid = 113 Min Req = -						
Des Ld: 45.00	TCDL: 6.0 psf	Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	HORZ(TL): 0.011 C - -	Bearing A Fcperp = 425psi.						
NCBCLL: 0.00	BCDL: 6.0 psf		Creep Factor: 2.0	Members not listed have forces less than 375#						
Soffit: 0.00	Mean Height: 24.71 ft		Max TC CSI: 0.354	<b>Maximum Web Forces Per Ply (lbs)</b>						
Load Duration: 1.15	MWFRS Parallel Dist: h/2 to h		Max BC CSI: 0.241	Webs	Tens.Comp.					
Spacing: 24.0 "	C&C Dist a: 3.00 ft		Max Web CSI: 0.071	B - D	208	-534				
	Loc. from endwall: not in 9.00 ft		VIEW Ver: 25.02.00B.1125.14							
	GCpi: 0.55									
	Wind Duration: 1.60									

**Lumber**

Top chord: 2x4 SP #2;  
 Bot chord: 2x4 SP #2;  
 Webs: 2x4 SP #2;

**Wind**

Wind loads based on MWFRS with additional C&C member design.  
 Wind loading based on both gable and hip roof types.

**Additional Notes**

See DWG VAL180160118 for valley details.



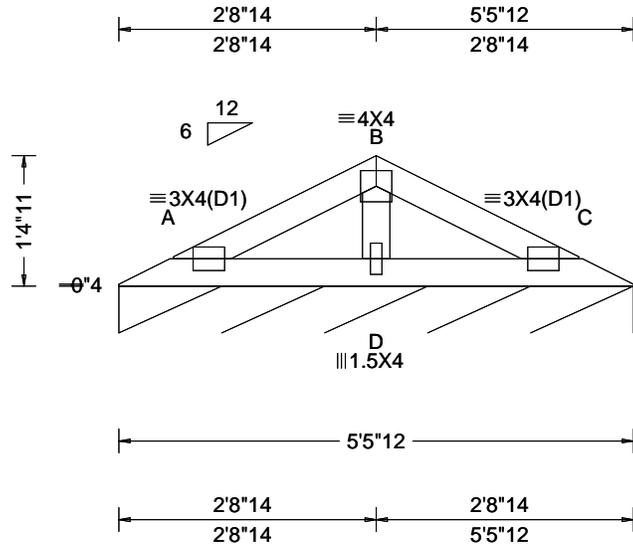
MISSOURI COA #2005000817  
 01/12/2026

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SEQN: 5727 VAL Ply: 1 Job Number: PM000117  
 FROM: Qty: 1 Customer - Clayton Properties Plan Name - Honeydew Elevation - Farmhouse 2Car  
 Truss Label: V5

Cust: R 9646 JRef: TVG096460001 T29  
 DrwNo: 012-26.1215.12680  
 Date: 01/12/2026



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF						
				Gravity			Non-Gravity			
TCLL: 25.00	Speed: 115 mph	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Loc	R+	/R-	/Rh	/Rw	/U	/RL
TCDL: 10.00	Risk Category: II	Pf(ASD): NA	VERT(LL): 0.003 C 999 240	E*	92	/-	/-	/45	/-	/3
BCLL: 0.00	Enclosure: Part. Enc.	Ce: NA Lu: NA	VERT(CL): 0.005 C 999 180	Wind reactions based on MWFRS						
BCDL: 10.00	EXP: B Kzt: NA	Cs: NA Snow Duration: NA	HORZ(LL): -0.001 C - -	E Brg Wid = 65.8 Min Req = -						
Des Ld: 45.00	TCDL: 6.0 psf	Building Code:	HORZ(TL): 0.002 C - -	Bearing A Fcperp = 425psi.						
NCBCLL: 0.00	BCDL: 6.0 psf	IRC 2018	Creep Factor: 2.0	Members not listed have forces less than 375#						
Soffit: 0.00	Mean Height: 25.21 ft	Load Std: ASCE 7-16	Max TC CSI: 0.091							
Load Duration: 1.15	MWFRS Parallel Dist: h/2 to h	TPI Std: 2014	Max BC CSI: 0.076							
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Fac: Yes	Max Web CSI: 0.025							
	Loc. from endwall: not in 9.00 ft	FT/RT:20(0)/10(0)	VIEW Ver: 25.02.00B.1125.14							
	GCpi: 0.55	Plate Type(s):								
	Wind Duration: 1.60	WAVE								

**Lumber**

Top chord: 2x4 SP #2;  
 Bot chord: 2x4 SP #2;  
 Webs: 2x4 SP #2;

**Wind**

Wind loads based on MWFRS with additional C&C member design.  
 Wind loading based on both gable and hip roof types.

**Additional Notes**

See DWG VAL180160118 for valley details.



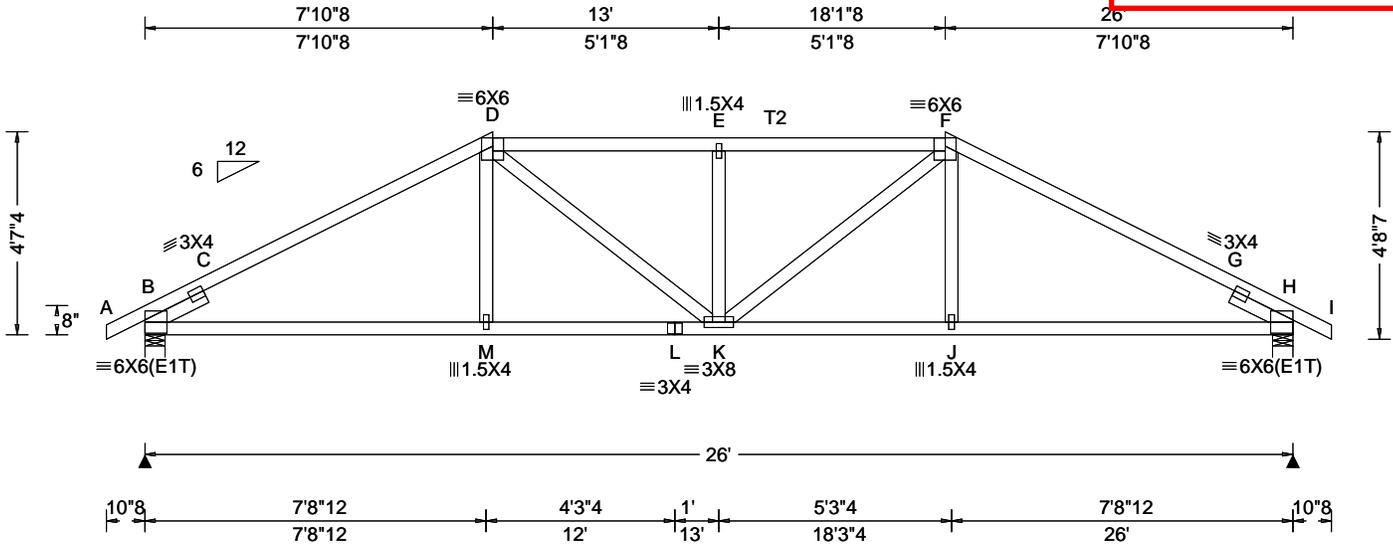
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 01/12/2026

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SEQN: 5710 HIPS Ply: 1 Job Number: PM000117  
FROM: Qty: 1 Customer - Clayton Properties Plan Name - Honeydew Elevation - Farmhouse 2Car  
Truss Label: C3

Cust: R 9646 JRef: 1VQ96460001 T3  
DrwNo: 012-26-1214-34783  
Date: 01/12/2026



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)						
				Gravity			Non-Gravity			
TCLL: 25.00	Speed: 115 mph	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Loc	R+	/R-	/Rh	/Rw	/U	/RL
TCDL: 10.00	Risk Category: II	Pf(ASD): NA	VERT(LL): 0.123 C 999 249	B	1265	-	-	/686	/92	/53
BCLL: 0.00	Enclosure: Part. Enc.	Ce: NA Lu: NA	VERT(CL): 0.197 C 999 249	H	1265	-	-	/686	/92	-
BCDL: 10.00	EXP: B Kzt: NA	Cs: NA Snow Duration: NA	HORZ(LL): 0.066 C - -	Wind reactions based on MWFRS						
Des Ld: 45.00	TCDL: 6.0 psf	Building Code: IRC 2018	HORZ(TL): 0.105 C - -	B Brg Wid = 5.5 Min Req = 1.6 (Support)						
NCBCLL: 10.00	BCDL: 6.0 psf	Load Std: ASCE 7-16	Creep Factor: 2.0	H Brg Wid = 5.5 Min Req = 1.6 (Support)						
Soffit: 0.00	Mean Height: 21.00 ft	TPI Std: 2014	Max TC CSI: 0.719	Bearings B & H Fcperp = 425psi.						
Load Duration: 1.15	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Yes	Max BC CSI: 0.764	Members not listed have forces less than 375#						
Spacing: 24.0 "	C&C Dist a: 3.00 ft	FT/RT:20(0)/10(0)	Max Web CSI: 0.191	<b>Maximum Top Chord Forces Per Ply (lbs)</b>						
	Loc. from endwall: not in 4.50 ft	Plate Type(s): WAVE	VIEW Ver: 25.02.00B.1125.14	Chords	Tens.Comp.	Chords	Tens.	Comp.		
	GCpi: 0.55			B - C	615	-2160	E - F	481	-1891	
	Wind Duration: 1.60			C - D	397	-1943	F - G	397	-1943	
				D - E	481	-1891	G - H	615	-2160	

**Lumber**  
Top chord: 2x4 SP 2400f-2.0E; T2 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #2;  
Lt Slider: 2x4 SP #2; block length = 1.500'  
Rt Slider: 2x4 SP #2; block length = 1.500'

**Loading**  
Bottom chord checked for 10.00 psf non-concurrent live load.

**Purlins**  
In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

**Wind**  
Wind loads based on MWFRS with additional C&C member design.  
Wind loading based on both gable and hip roof types.

**Snow**  
Overhang designed for 2.00X TC LL.

**Additional Notes**  
Top Chord overhang(s) may be field trimmed.



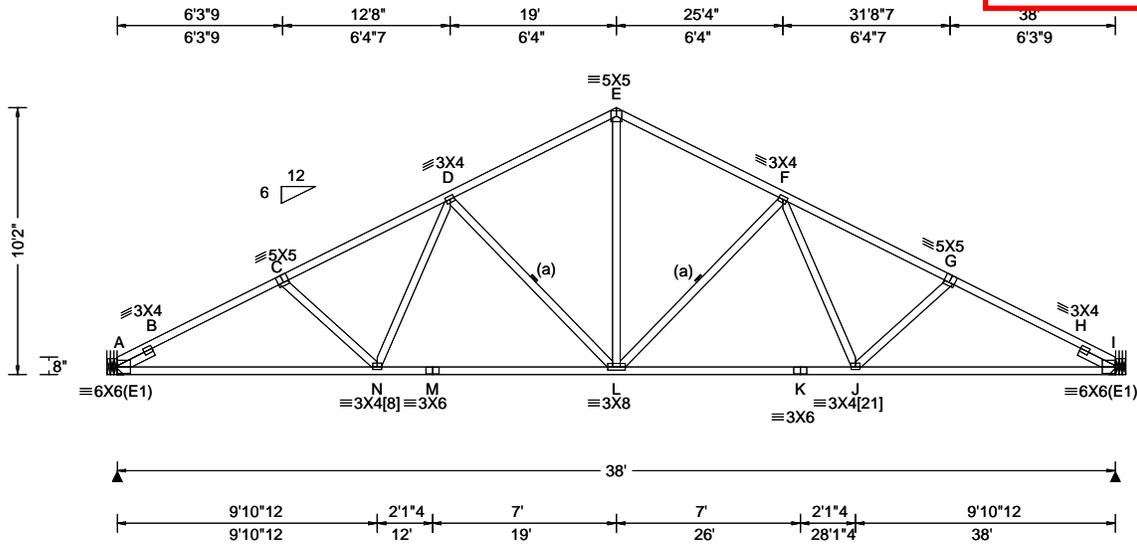
MISSOURI COA #2005000817  
01/12/2026

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SEQN: 5715    SPEC    Ply: 1    Job Number: PM000117  
 FROM:                      Qty: 6    Customer - Clayton Properties Plan Name - Honeydew Elevation - Farmhouse 2Car  
 Page 1 of 2                      Truss Label: E4

Cust: R 9646    JRef: 1YQ96460001 T30  
 DrwNo: 012-26.1214.48387  
 Date: 01/12/2026



<b>Loading Criteria (psf)</b> TCLL: 25.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	<b>Wind Criteria</b> Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B    Kzt: NA TCDL: 6.0 psf BCDL: 6.0 psf Mean Height: 24.00 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.80 ft Loc. from endwall: Any GCpi: 0.55 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA    Ct: NA    CAT: NA Pf(ASD): NA Ce: NA    Lu: NA Cs: NA    Snow Duration: NA Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.159 L 999 364 VERT(CL): 0.294 L 999 364 HORZ(LL): 0.055 I - - HORZ(TL): 0.100 I - - Creep Factor: 2.0 Max TC CSI: 0.764 Max BC CSI: 0.498 Max Web CSI: 0.415 VIEW Ver: 25.02.00B.1125.14	<b>▲ Maximum Reactions (lbs)</b> <table border="1"> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> <tr> <td>A</td> <td>1755</td> <td>-</td> <td>-</td> <td>/1013</td> <td>/121</td> <td>/140</td> </tr> <tr> <td>I</td> <td>1755</td> <td>-</td> <td>-</td> <td>/1013</td> <td>/121</td> <td>-</td> </tr> </table> Wind reactions based on MWFRS A Brg Wid = -    Min Req = - I Brg Wid = -    Min Req = - Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> <table border="1"> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> <tr> <td>A - B</td> <td>724 -3158</td> <td>E - F</td> <td>527 -2034</td> </tr> <tr> <td>B - C</td> <td>602 -3112</td> <td>F - G</td> <td>572 -2821</td> </tr> <tr> <td>C - D</td> <td>573 -2821</td> <td>G - H</td> <td>601 -3112</td> </tr> <tr> <td>D - E</td> <td>528 -2034</td> <td>H - I</td> <td>724 -3158</td> </tr> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	A	1755	-	-	/1013	/121	/140	I	1755	-	-	/1013	/121	-	Chords	Tens.Comp.	Chords	Tens. Comp.	A - B	724 -3158	E - F	527 -2034	B - C	602 -3112	F - G	572 -2821	C - D	573 -2821	G - H	601 -3112	D - E	528 -2034	H - I	724 -3158
Loc	Gravity			Non-Gravity																																															
	R+	/R-	/Rh	/Rw	/U	/RL																																													
A	1755	-	-	/1013	/121	/140																																													
I	1755	-	-	/1013	/121	-																																													
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C - D	573 -2821	G - H	601 -3112																																																
D - E	528 -2034	H - I	724 -3158																																																

**Lumber**

Top chord: 2x4 SP #2;  
 Bot chord: 2x4 SP 2400f-2.0E;  
 Webs: 2x4 SP #2;  
 Lt Slider: 2x4 SP #2; block length = 1.500'  
 Rt Slider: 2x4 SP #2; block length = 1.500'

**Bracing**

(a) Continuous lateral restraint equally spaced on member.

**Plate Shift Table**

JT No	Plate Size	Lateral Shift	Chord Bite	JT No	Plate Size	Lateral Shift	Chord Bite
[ 8]	3X4	S	1.25	[21]	3X4	S	1.25

**Loading**

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Bottom chord checked for 10.00 psf non-concurrent live load.

Live loads applied in combination per ASCE 7 sec. 2.4.1 use 0.75 factor for multiple live loads.

**Wind**

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.



MISSOURI COA #2005000817  
 01/12/2026

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SEQN: 5715	SPEC	Ply: 1	Job Number: PM000117
FROM:		Qty: 6	Customer - Clayton Properties Plan Name - Honeydew Elevation - Farmhouse 2Car
Page 2 of 2			Truss Label: E4

Cust: R 9646 JRef: 1 YG096460001 T30  
 DrwNo: 012-26.1214-48887  
 BM 01/12/2026

**Hangers / Ties**

Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.

Recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information. Additional connection required to evenly distribute hanger reaction throughout all plies of supporting girder.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

Bearing at location x=0' uses the following support conditions: 0'

Bearing A (0', 18'7") HUS28

- Supporting Member: (3)2x8 SP 2400f-2.0E
- (22) 0.148"x3" nails into supporting member,
- (4) 0.148"x3" nails into supported member.

Bearing I (37'9", 18'7") HUS26

- Supporting Member: (3)2x8 SP #1
- (14) 0.148"x3" nails into supporting member,
- (4) 0.148"x3" nails into supported member.



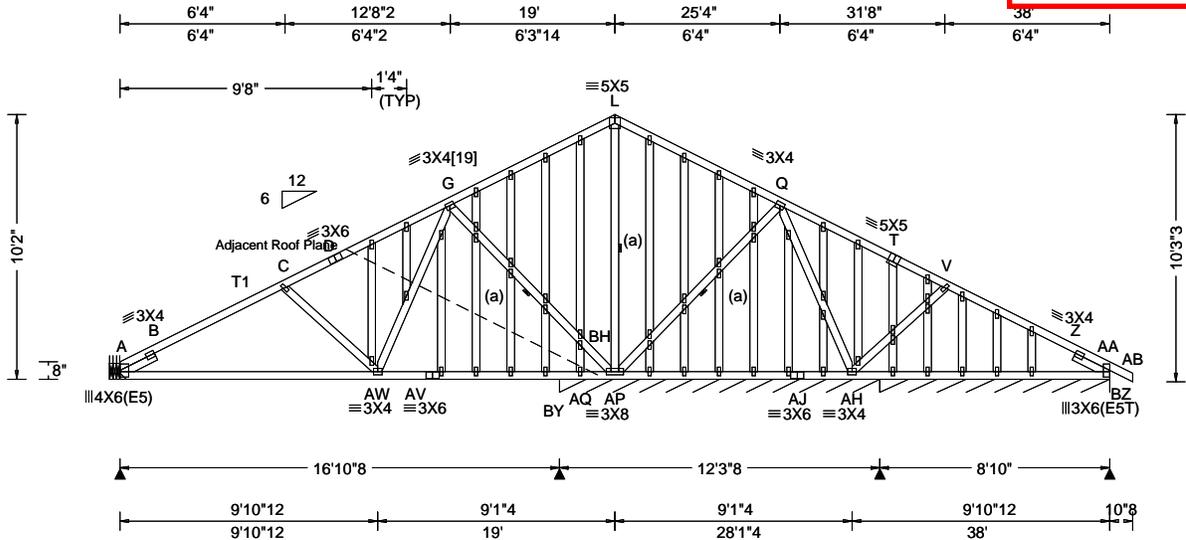
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SEQN: 5732 GABL Ply: 1 Job Number: PM000117  
FROM: Qty: 1 Customer - Clayton Properties Plan Name - Honeydew Elevation - Farmhouse 2Car  
Page 1 of 2 Truss Label: E1

Cust: R9646 JRef: 1VQ96460001 T31  
DrwNo: 012-26-1214-44083  
Date: 01/12/2026



<b>Loading Criteria (psf)</b> TCCL: 25.00 TCCL: 10.00 BCLL: 0.00 BCLL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	<b>Wind Criteria</b> Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCCL: 6.0 psf BCDL: 6.0 psf Mean Height: 23.78 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.80 ft Loc. from endwall: not in 6.06 ft GCpi: 0.55 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT: 20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.085 B 999 240 VERT(CL): 0.142 B 999 180 HORZ(LL): 0.046 B - - HORZ(TL): 0.078 B - - Creep Factor: 2.0 Max TC CSI: 0.542 Max BC CSI: 0.642 Max Web CSI: 0.884 VIEW Ver: 25.02.00B.1125.14	<b>▲ Maximum Reactions (lbs), or *=PLF</b> <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>A</td> <td>871</td> <td>-</td> <td>-</td> <td>/478</td> <td>/79</td> <td>/201</td> </tr> <tr> <td>BY*</td> <td>244</td> <td>-</td> <td>-</td> <td>/122</td> <td>/26</td> <td>-</td> </tr> <tr> <td>BZ*</td> <td>118</td> <td>-</td> <td>-</td> <td>/70</td> <td>/13</td> <td>-</td> </tr> <tr> <td>AH</td> <td colspan="6">-/100</td> </tr> </tbody> </table> Wind reactions based on MWFRS A Brg Wid = - Min Req = - BY Brg Wid = 147 Min Req = - BZ Brg Wid = 106 Min Req = - Bearings BY & AG Fcperp = 425psi. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> <table border="1"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>A - B</td> <td>597 -1720</td> <td>D - G</td> <td>187 -665</td> </tr> <tr> <td>B - C</td> <td>169 -1217</td> <td>G - L</td> <td>592 -46</td> </tr> <tr> <td>C - D</td> <td>117 -750</td> <td>L - Q</td> <td>558 0</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	A	871	-	-	/478	/79	/201	BY*	244	-	-	/122	/26	-	BZ*	118	-	-	/70	/13	-	AH	-/100						Chords	Tens.Comp.	Chords	Tens. Comp.	A - B	597 -1720	D - G	187 -665	B - C	169 -1217	G - L	592 -46	C - D	117 -750	L - Q	558 0
Loc	Gravity			Non-Gravity																																																									
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C - D	117 -750	L - Q	558 0																																																										

**Lumber**  
 Top chord: 2x4 SP #2; T1 2x4 SP 2400f-2.0E;  
 Bot chord: 2x4 SP #2;  
 Webs: 2x4 SP #2;  
 Lt Slider: 2x4 SP #2; block length = 1.500'  
 Rt Slider: 2x4 SP #2; block length = 1.500'

**Bracing**  
 (a) Continuous lateral restraint equally spaced on member.

**Plating Notes**  
 All plates are 1.5X4 except as noted.

**Plate Shift Table**

JT No	Plate Size	Lateral Shift	Chord Bite	JT No	Plate Size	Lateral Shift	Chord Bite
[19]	3X4	S	1.25				

**Snow**  
 Overhang designed for 2.00X TC LL.

**Additional Notes**  
 See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.  
 Top Chord overhang(s) may be field trimmed.

**Maximum Bot Chord Forces Per Ply (lbs)**

Chords	Tens.Comp.
A - AW	1015 -91

**Maximum Web Forces Per Ply (lbs)**

Webs	Tens.Comp.	Webs	Tens. Comp.
C - AW	244 -753	G - AP	220 -1088
AW - G	810 -124	L - AP	0 -536

**Maximum Gable Forces Per Ply (lbs)**

Gables	Tens.Comp.
AQ - BH	124 -482



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SEQN: 5732	GABL	Ply: 1	Job Number: PM000117
FROM:		Qty: 1	Customer - Clayton Properties Plan Name - Honeydew Elevation - Farmhouse 2Car
Page 2 of 2			Truss Label: E1

Cust: R 9646 JRef: 1VGO96460001 T31  
 DrwNo: 012-26.1214-44083  
 BM 01/12/2026

**Hangers / Ties**

Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.

Recommended connection based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information. Additional connection required to evenly distribute hanger reaction throughout all plies of supporting girder.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

Bearing at location x=0' uses the following support conditions: 0'

Bearing A (0', 18'7") HUS28

Supporting Member: (3)2x8 SP 2400f-2.0E

(22) 0.148"x3" nails into supporting member,

(4) 0.148"x3" nails into supported member.



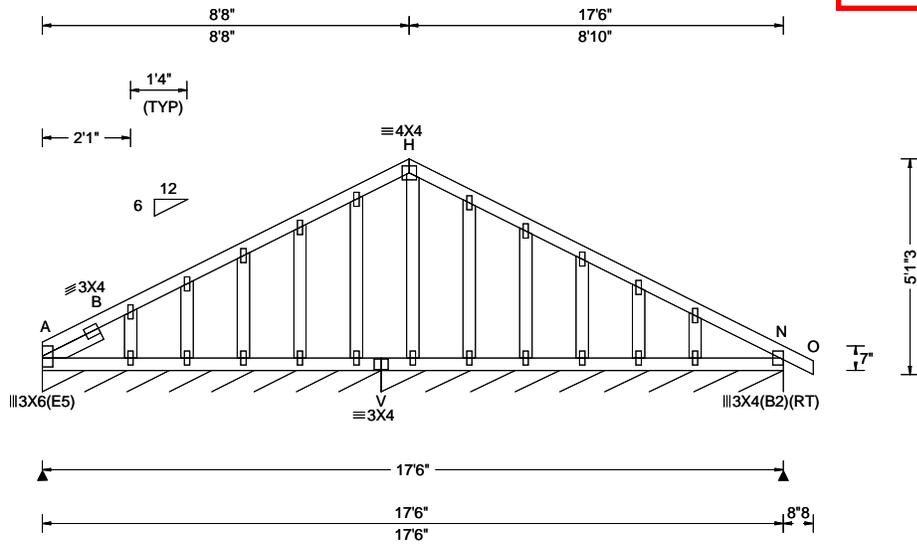
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 01/12/2026

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SEQN: 5716 FROM:	GABL Qty: 1	<b>Job Number:</b> PM000117 Customer - Clayton Properties Plan Name - Honeydew Elevation - Farmhouse 2Car <b>Truss Label:</b> G1
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Cust: R 9646 JRef: TVG096460001 14  
 DrwNo: 012-26.1214.55483  
 Date: 01/12/2026



<b>Loading Criteria (psf)</b> TCLL: 25.00 TC DL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	<b>Wind Criteria</b> Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TC DL: 6.0 psf BCDL: 6.0 psf Mean Height: 21.20 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.55 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.003 B 999 240 VERT(CL): 0.004 B 999 180 HORZ(LL): 0.002 B - - HORZ(TL): 0.003 B - - Creep Factor: 2.0 Max TC CSI: 0.100 Max BC CSI: 0.028 Max Web CSI: 0.033 VIEW Ver: 25.02.00B.1125.14	<b>▲ Maximum Reactions (lbs), or *=PLF</b> <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>A*</td> <td>121</td> <td>-</td> <td>-</td> <td>/74</td> <td>/21</td> <td>/11</td> </tr> <tr> <td>V*</td> <td>125</td> <td>-</td> <td>-</td> <td>/68</td> <td>/8</td> <td>-</td> </tr> </tbody> </table> Wind reactions based on MWFRS A Brg Wid = 96.0 Min Req = - V Brg Wid = 114 Min Req = - Bearings A & V are a rigid surface. Members not listed have forces less than 375#	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	A*	121	-	-	/74	/21	/11	V*	125	-	-	/68	/8	-
Loc	Gravity			Non-Gravity																											
	R+	/R-	/Rh	/Rw	/U	/RL																									
A*	121	-	-	/74	/21	/11																									
V*	125	-	-	/68	/8	-																									

**Lumber**

Top chord: 2x4 SP #2;  
 Bot chord: 2x4 SP #2;  
 Webs: 2x4 SP #2;  
 Lt Slider: 2x4 SP #2; block length = 1.500'

**Plating Notes**

All plates are 1.5X4 except as noted.

**Loading**

Truss designed to support 1-4-0 top chord outlookers and cladding load not to exceed 3.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Bottom chord checked for 10.00 psf non-concurrent live load.

**Wind**

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

**Snow**

Overhang designed for 2.00X TC LL.

**Additional Notes**

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.  
 Top Chord overhang(s) may be field trimmed.



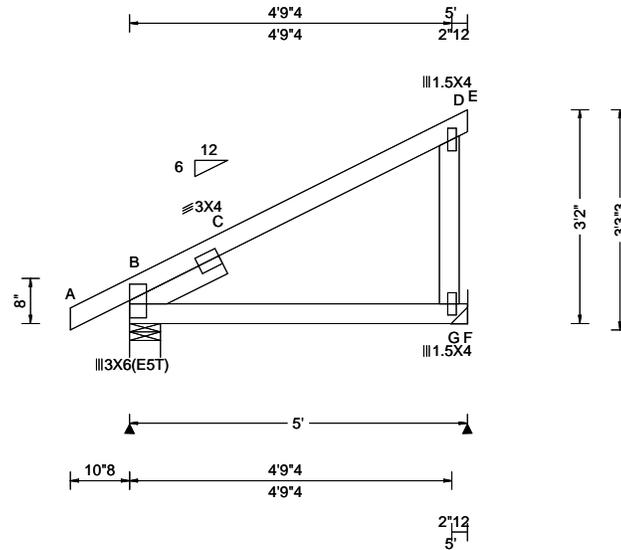
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 01/12/2026

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SEQN: 5707      MONO      Ply: 1      Job Number: PM000117  
 FROM:                      Qty: 4      Customer - Clayton Properties Plan Name - Honeydew Elevation - Farmhouse 2Car  
 Truss Label: B2

Cust: R 9646    JRef: 1VQ96460001 15  
 DrwNo: 012-26.1214.23680  
 Date: 01/12/2026



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)						
				Gravity			Non-Gravity			
TCLL: 25.00	Speed: 115 mph	Pg: NA    Ct: NA    CAT: NA	PP Deflection in    loc    L/defl    L/#	Loc	R+	/R-	/Rh	/Rw	/U	/RL
TCDL: 10.00	Risk Category: II	Pf(ASD): NA	VERT(LL): NA	B	294	/-	/-	/138	/-	/71
BCLL: 0.00	Enclosure: Part. Enc.	Ce: NA    Lu: NA	VERT(CL): NA	F	237	/-	/-	/155	/21	/-
BCDL: 10.00	EXP: B    Kzt: NA	Cs: NA    Snow Duration: NA	HORZ(LL): 0.030 C    -    -	Wind reactions based on MWFRS						
Des Ld: 45.00	TCDL: 6.0 psf	Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	HORZ(TL): 0.047 C    -    -	B    Brg Wid = 5.5    Min Req = 1.5 (Support)						
NCBCLL: 10.00	BCDL: 6.0 psf		Creep Factor: 2.0	F    Brg Wid = -    Min Req = -						
Soffit: 0.00	Mean Height: 15.00 ft		Max TC CSI: 0.465	Bearing B Fcperp = 425psi.						
Load Duration: 1.15	MWFRS Parallel Dist: 0 to h/2		Max BC CSI: 0.233	Members not listed have forces less than 375#						
Spacing: 24.0 "	C&C Dist a: 3.00 ft		Max Web CSI: 0.153	<b>Maximum Top Chord Forces Per Ply (lbs)</b>						
	Loc. from endwall: Any		VIEW Ver: 25.02.00B.1125.14	Chords    Tens.Comp.						
	GCpi: 0.55			B - C	684	-838				
	Wind Duration: 1.60									

**Lumber**

Top chord: 2x4 SP #2;  
 Bot chord: 2x4 SP #2;  
 Webs: 2x4 SP #2;  
 Lt Slider: 2x4 SP #2; block length = 1.500'

**Loading**

Bottom chord checked for 10.00 psf non-concurrent live load.

**Wind**

Wind loads based on MWFRS with additional C&C member design.  
 Right end vertical not exposed to wind pressure.  
 Wind loading based on both gable and hip roof types.

**Snow**

Overhang designed for 2.00X TC LL.

**Additional Notes**

Top Chord overhang(s) may be field trimmed.



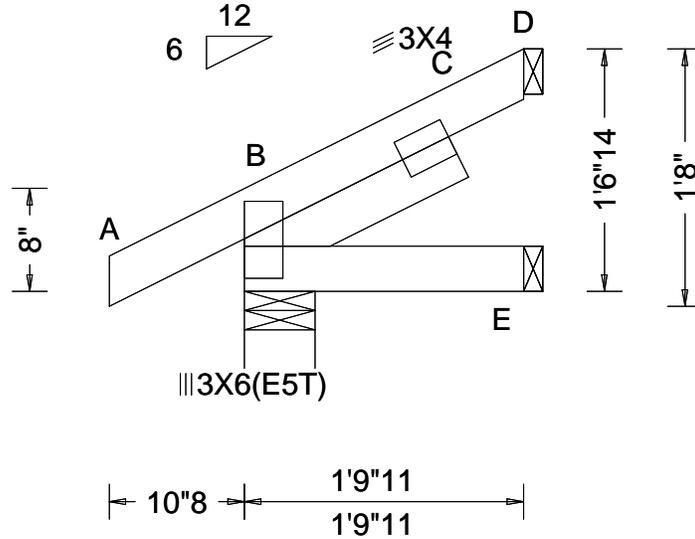
MISSOURI COA #2005000817  
 01/12/2026

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SEQN: 5720 JACK Ply: 1 Job Number: PM000117  
 FROM: Qty: 2 Customer - Clayton Properties Plan Name - Honeydew Elevation - Farmhouse 2Car  
 Truss Label: J3

Cust: R 9646 JRef: 1VGO96460001 T6  
 DrwNo: 012-26-1215-01-260  
 BM 01/12/2026



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)						
				Gravity			Non-Gravity			
TCLL: 25.00	Speed: 115 mph	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Loc	R+	/R-	/Rh	/Rw	/U	/RL
TCDL: 10.00	Risk Category: II	Pf(ASD): NA	VERT(LL): NA	B	171	-	-	/63	-	/35
BCLL: 0.00	Enclosure: Part. Enc.	Ce: NA Lu: NA	VERT(CL): NA	E	19	-	-	/18	-	-
BCDL: 10.00	EXP: B Kzt: NA	Cs: NA Snow Duration: NA	HORZ(LL): 0.001 C - -	D	64	-4	-	/36	/23	-
Des Ld: 45.00	TCDL: 6.0 psf	Building Code:	HORZ(TL): 0.002 C - -	Wind reactions based on MWFRS						
NCBCLL: 0.00	BCDL: 6.0 psf	IRC 2018	Creep Factor: 2.0	B Brg Wid = 5.5 Min Req = 1.5 (Support)						
Soffit: 0.00	Mean Height: 19.48 ft	Load Std: ASCE 7-16	Max TC CSI: 0.104	E Brg Wid = 1.5 Min Req = -						
Load Duration: 1.15	MWFRS Parallel Dist: 0 to h/2	TPI Std: 2014	Max BC CSI: 0.021	D Brg Wid = 1.5 Min Req = -						
Spacing: 24.0 "	C&C Dist a: 3.00 ft	Rep Fac: Yes	Max Web CSI: 0.007	Bearing B Fcperp = 425psi.						
	Loc. from endwall: Any	FT/RT:20(0)/10(0)	VIEW Ver: 25.02.00B.1125.14	Members not listed have forces less than 375#						
	GCpi: 0.55	Plate Type(s):								
	Wind Duration: 1.60	WAVE								

**Lumber**

Top chord: 2x4 SP #2;  
 Bot chord: 2x4 SP #2;  
 Lt Slider: 2x4 SP #2; block length = 1.500'

**Wind**

Wind loads based on MWFRS with additional C&C member design.  
 Wind loading based on both gable and hip roof types.

**Snow**

Overhang designed for 2.00X TC LL.

**Additional Notes**

Top Chord overhang(s) may be field trimmed.



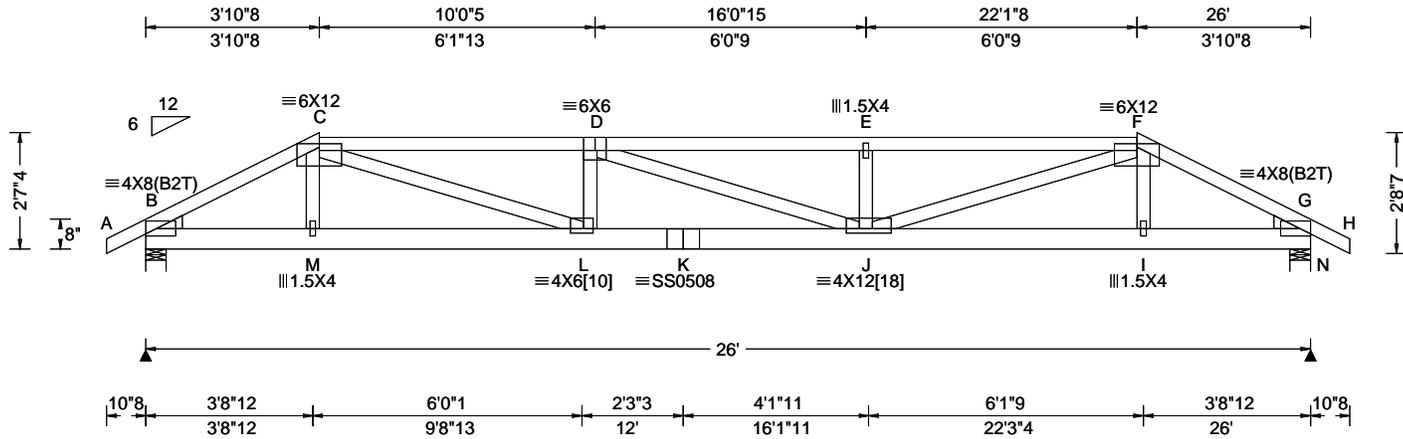
MISSOURI COA #2005000817  
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SEQN: 5708 HIPS Ply: 1 Job Number: PM000117  
 FROM: Qty: 1 Customer - Clayton Properties Plan Name - Honeydew Elevation - Farmhouse 2Car  
 Truss Label: C1

Cust: R 9646 JRef: 1 YG096460001 17  
 DrwNo: 012-26-1214-23127  
 Date: 01/12/2026



Loading Criteria (psf)
TCLL: 25.00
TCDL: 10.00
BCLL: 0.00
BCDL: 10.00
Des Ld: 45.00
NCBCLL: 0.00
Soffit: 0.00
Load Duration: 1.15
Spacing: 24.0 "

Wind Criteria
Speed: 115 mph
Risk Category: II
Enclosure: Part. Enc.
EXP: B Kzt: NA
TCDL: 6.0 psf
BCDL: 6.0 psf
Mean Height: 20.00 ft
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: not in 4.50 ft
GCpi: 0.55
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf(ASD): NA
Ce: NA Lu: NA
Cs: NA Snow Duration: NA
Building Code: IRC 2018
Load Std: ASCE 7-16
TPI Std: 2014
Rep Fac: Varies by Ld Case
FT/RT:20(0)/10(0)
Plate Type(s): WAVE, 18SS

Defl/CSI Criteria
PP Deflection in loc L/defl L/#
VERT(LL): 0.301 E 999 247
VERT(CL): 0.548 E 564 247
HORZ(LL): 0.051 C - -
HORZ(TL): 0.093 C - -
Creep Factor: 2.0
Max TC CSI: 0.874
Max BC CSI: 0.467
Max Web CSI: 0.722
VIEW Ver: 25.02.00B.1125.14

▲ Maximum Reactions (lbs)						
Gravity				Non-Gravity		
Loc	R+	/R-	/Rh	/Rw	/U	/RL
B	1919	-	-	-	/222	-
N	1919	-	-	-	/222	-
Wind reactions based on MWFRS						
B Brg Wid = 5.5 Min Req = 2.4 (Support)						
N Brg Wid = 5.5 Min Req = 2.4 (Support)						
Bearings B & N Fcperp = 425psi.						
Members not listed have forces less than 375#						
Maximum Top Chord Forces Per Ply (lbs)						
Chords	Tens.Comp.	Chords	Tens. Comp.			
B - C	382 - 3298	E - F	687 - 5667			
C - D	673 - 5615	F - G	382 - 3310			
D - E	686 - 5665					

**Lumber**  
 Top chord: 2x4 SP 2400f-2.0E;  
 Bot chord: 2x6 SP 2400f-2.0E;  
 Webs: 2x4 SP #2;  
 Lt Wedge: 2x4 SP #2; Rt Wedge: 2x4 SP #2;

**Snow**  
 Overhang designed for 2.00X TC LL.

**Additional Notes**  
 Top Chord overhang(s) may be field trimmed.

Maximum Bot Chord Forces Per Ply (lbs)				
Chords	Tens.Comp.	Chords	Tens. Comp.	
B - M	2890 - 330	K - J	5714 - 702	
M - L	2892 - 338	J - I	2901 - 337	
L - K	5714 - 702	I - G	2900 - 330	

**Special Loads**  
 -----(Lumber Dur.Fac.=1.15 / Plate Dur.Fac.=1.15)  
 TC: From 72 plf at -0.88 to 72 plf at 3.88  
 TC: From 36 plf at 3.88 to 36 plf at 22.12  
 TC: From 72 plf at 22.12 to 72 plf at 26.88  
 BC: From 20 plf at 0.00 to 20 plf at 3.88  
 BC: From 10 plf at 3.88 to 10 plf at 22.12  
 BC: From 20 plf at 22.12 to 20 plf at 26.00  
 TC: 193 lb Conc. Load at 3.88,22.12  
 TC: 132 lb Conc. Load at 5.94, 7.94, 9.94,11.94  
 13.00,14.06,16.06,18.06,20.06  
 BC: 78 lb Conc. Load at 3.88,22.12  
 BC: 47 lb Conc. Load at 5.94, 7.94, 9.94,11.94  
 13.00,14.06,16.06,18.06,20.06

Maximum Web Forces Per Ply (lbs)			
Webs	Tens.Comp.	Webs	Tens. Comp.
C - L	2904 - 356	E - J	224 - 750
L - D	223 - 755	J - F	2944 - 371

Plate Shift Table					
JT No	Plate Size	Lateral Shift	Chord Bite	JT No	Plate Size
[10]	4X6	2.50	R 1.25	[18]	4X12
					5.00 R 1.25

**Purlins**  
 In lieu of structural panels use purlins to brace all flat TC @ 24" oc.

**Wind**  
 Wind loads and reactions based on MWFRS.  
 Wind loading based on both gable and hip roof types.



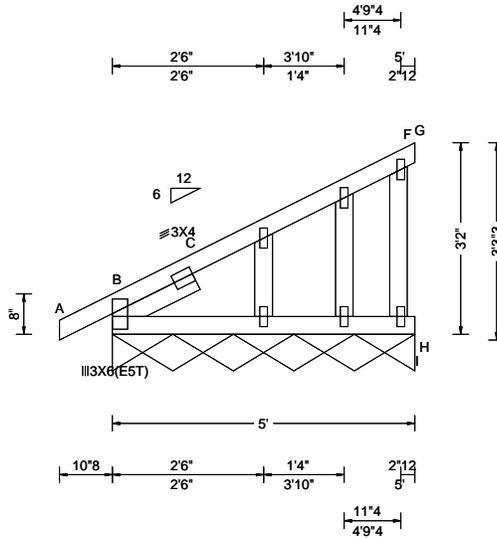
MISSOURI COA #2005000817  
 01/12/2026

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SEQN: 5706 GABL Ply: 1 Job Number: PM000117  
 FROM: Qty: 1 Customer - Clayton Properties Plan Name - Honeydew Elevation - Farmhouse 2Car  
 Truss Label: B1

Cust: R 9646 JRef: 1VQ96460001 T8  
 DrwNo: 012-26.1214-17143  
 BM 01/12/2026



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF						
				Gravity			Non-Gravity			
				Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
TCLL: 25.00	Speed: 115 mph	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	H*	134	/-	/-	/74	/36	/33
TCDL: 10.00	Risk Category: II	Pf(ASD): NA	VERT(LL): 0.003 C 999 240	Wind reactions based on MWFRS						
BCLL: 0.00	Enclosure: Part. Enc.	Ce: NA Lu: NA	VERT(CL): 0.005 C 999 180	H Brg Wid = 60.0 Min Req = -						
BCDL: 10.00	EXP: B Kzt: NA	Cs: NA Snow Duration: NA	HORZ(LL): 0.002 C - - -	Bearing B Fcperp = 425psi.						
Des Ld: 45.00	TCDL: 6.0 psf		HORZ(TL): 0.004 C - - -	Members not listed have forces less than 375#						
NCBCLL: 10.00	BCDL: 6.0 psf	Building Code:	Creep Factor: 2.0							
Soffit: 0.00	Mean Height: 15.00 ft	IRC 2018	Max TC CSI: 0.054							
Load Duration: 1.15	MWFRS Parallel Dist: 0 to h/2	Load Std: ASCE 7-16	Max BC CSI: 0.016							
Spacing: 24.0 "	C&C Dist a: 3.00 ft	TPI Std: 2014	Max Web CSI: 0.130							
	Loc. from endwall: Any	Rep Fac: Varies by Ld Case								
	GCpi: 0.55	FT/RT:20(0)/10(0)								
	Wind Duration: 1.60	Plate Type(s):	VIEW Ver: 25.02.00B.1125.14							
		WAVE								

**Lumber**

Top chord: 2x4 SP 2400F-2.0E;  
 Bot chord: 2x4 SP 2400F-2.0E;  
 Webs: 2x4 SP #2;  
 Lt Slider: 2x4 SP #2; block length = 1.500'

**Plating Notes**

All plates are 1.5X4 except as noted.

**Loading**

Truss designed to support 1-4-0 top chord outlookers and cladding load not to exceed 3.00 PSF one face and 24.0' span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Bottom chord checked for 10.00 psf non-concurrent live load.

**Wind**

Wind loads based on MWFRS with additional C&C member design.

Right end vertical exposed to wind pressure. Deflection meets L/180.

Wind loading based on both gable and hip roof types.

**Snow**

Overhang designed for 2.00X TC LL.

**Additional Notes**

See DWGS A14015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Top Chord overhang(s) may be field trimmed.



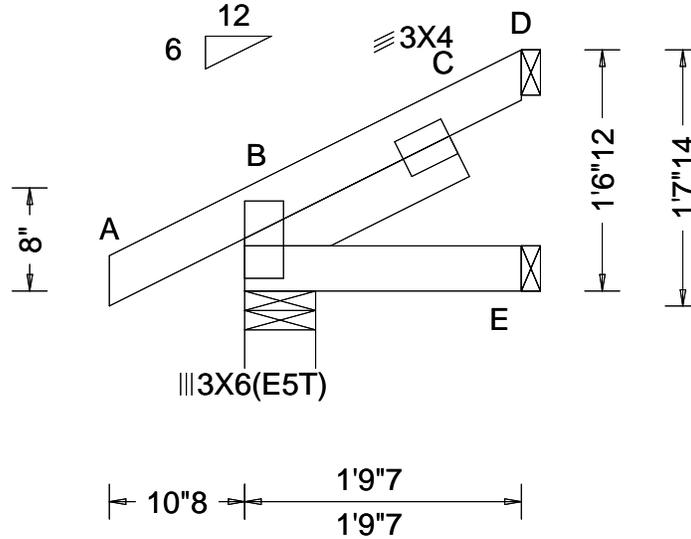
MISSOURI COA #2005000817  
 01/12/2026

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SEQN: 5721 JACK Ply: 1 Job Number: PM000117  
 FROM: Qty: 2 Customer - Clayton Properties Plan Name - Honeydew Elevation - Farmhouse 2Car  
 Truss Label: J3

Cust: R 9646 JRef: 1VGO96460001 T9  
 DrwNo: 012-26.1215.02583  
 BM 01/12/2026



<b>Loading Criteria (psf)</b> TCCL: 25.00 TCCL: 10.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCCL: 0.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	<b>Wind Criteria</b> Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCCL: 6.0 psf BCDL: 6.0 psf Mean Height: 19.48 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.55 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.001 C - - HORZ(TL): 0.002 C - - Creep Factor: 2.0 Max TC CSI: 0.104 Max BC CSI: 0.020 Max Web CSI: 0.006 VIEW Ver: 25.02.00B.1125.14	<b>▲ Maximum Reactions (lbs)</b> Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL
				B 171 /- /- /63 /- /34 E 19 /- /- /18 /- /- D 64 /-5 /- /35 /22 /- Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.5 (Support) E Brg Wid = 1.5 Min Req = - D Brg Wid = 1.5 Min Req = - Bearing B Fcperp = 425psi. Members not listed have forces less than 375#

**Lumber**

Top chord: 2x4 SP #2;  
 Bot chord: 2x4 SP #2;  
 Lt Slider: 2x4 SP #2; block length = 1.500'

**Wind**

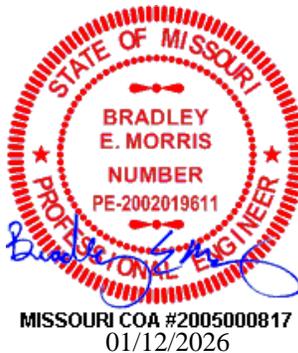
Wind loads based on MWFRS with additional C&C member design.  
 Wind loading based on both gable and hip roof types.

**Snow**

Overhang designed for 2.00X TC LL.

**Additional Notes**

Top Chord overhang(s) may be field trimmed.



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# Gable Stud Reinforcement Detail

ASCE 7-16: 140 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.0

- Dr: 120 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00
- Dr: 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure D, Kzt = 1.00
- Dr: 100 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

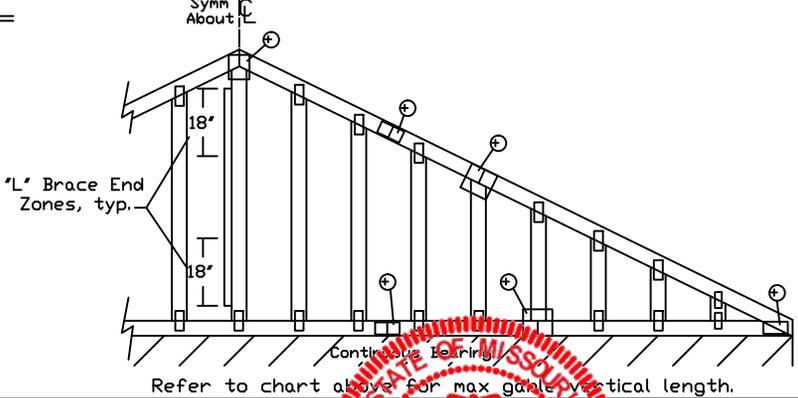
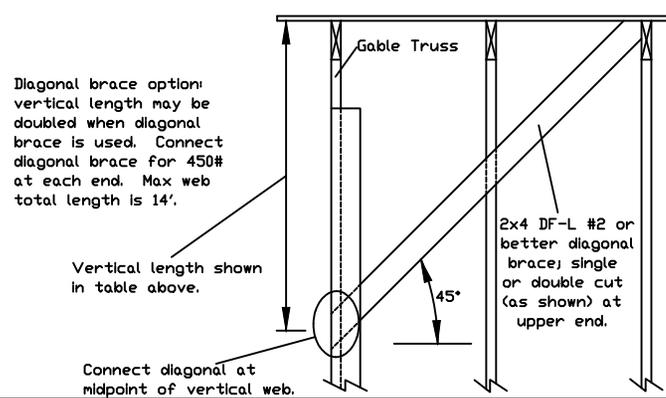
Max Gable Vertical Length	2x4 Gable Vertical Spacing		Brace Grade	No Braces	(1) 1x4 'L' Brace *		(1) 2x4 'L' Brace *		(2) 2x4 'L' Brace **		(1) 2x6 'L' Brace *		(2) 2x6 'L' Brace **		
	Species	Grade			Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	
	24" o.c.	SPF	#1 / #2	HF	#1 / #2	4' 3"	7' 3"	7' 7"	8' 7"	8' 11"	10' 3"	10' 8"	13' 6"	14' 0"	14' 0"
#3					4' 1"	6' 7"	7' 1"	8' 6"	8' 10"	10' 1"	10' 6"	13' 4"	13' 10"	14' 0"	14' 0"
Stud					4' 1"	6' 7"	7' 0"	8' 6"	8' 10"	10' 1"	10' 6"	13' 4"	13' 10"	14' 0"	14' 0"
Standard			#1	4' 1"	5' 8"	6' 0"	7' 7"	8' 1"	10' 1"	10' 6"	11' 10"	12' 8"	14' 0"	14' 0"	
			#2	4' 6"	7' 4"	7' 8"	8' 8"	9' 0"	10' 4"	10' 9"	13' 8"	14' 0"	14' 0"	14' 0"	
			#3	4' 3"	7' 3"	7' 7"	8' 7"	8' 11"	10' 3"	10' 8"	13' 6"	14' 0"	14' 0"	14' 0"	
SP		DFL	#1	4' 2"	6' 0"	6' 4"	7' 11"	8' 6"	10' 2"	10' 7"	12' 5"	13' 4"	14' 0"	14' 0"	
			Stud	4' 2"	6' 0"	6' 4"	7' 11"	8' 6"	10' 2"	10' 7"	12' 5"	13' 4"	14' 0"	14' 0"	
			Standard	4' 0"	5' 3"	5' 7"	7' 0"	7' 6"	9' 6"	10' 2"	11' 0"	11' 10"	14' 0"	14' 0"	
		SPF	#1 / #2	4' 11"	8' 4"	8' 8"	9' 10"	10' 3"	11' 8"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"	
			#3	4' 8"	8' 1"	8' 8"	9' 8"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	
			Stud	4' 8"	8' 1"	8' 6"	9' 8"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	
SP	DFL	#1	4' 8"	6' 11"	7' 5"	9' 3"	9' 11"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"		
		Stud	4' 8"	6' 11"	7' 5"	9' 3"	9' 11"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"		
		Standard	4' 8"	6' 11"	7' 5"	9' 3"	9' 11"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"		
	SPF	#1 / #2	5' 1"	8' 5"	8' 9"	9' 11"	10' 4"	11' 10"	12' 4"	14' 0"	14' 0"	14' 0"	14' 0"		
		#2	4' 11"	8' 4"	8' 8"	9' 10"	10' 3"	11' 8"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"		
		#3	4' 9"	7' 4"	7' 9"	9' 9"	10' 2"	11' 8"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"		
SP	DFL	#1	4' 9"	7' 4"	7' 9"	9' 9"	10' 2"	11' 8"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"		
		Stud	4' 9"	7' 4"	7' 9"	9' 9"	10' 2"	11' 8"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"		
		Standard	4' 8"	6' 5"	6' 10"	8' 7"	9' 2"	11' 7"	12' 1"	13' 6"	14' 0"	14' 0"	14' 0"		
	SPF	#1 / #2	5' 5"	9' 2"	9' 6"	10' 10"	11' 3"	11' 8"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"		
		#3	5' 1"	9' 0"	9' 4"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"		
		Stud	5' 1"	9' 0"	9' 4"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"		
SP	DFL	#1	5' 1"	8' 0"	8' 6"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"		
		Stud	5' 1"	8' 0"	8' 6"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"		
		Standard	5' 1"	8' 0"	8' 6"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"		
	SPF	#1	5' 8"	9' 3"	9' 8"	10' 11"	11' 4"	13' 0"	13' 6"	14' 0"	14' 0"	14' 0"	14' 0"		
		#2	5' 5"	9' 2"	9' 6"	10' 10"	11' 3"	12' 11"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"		
		#3	5' 3"	8' 5"	9' 0"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"		
SP	DFL	#1	5' 3"	8' 5"	9' 0"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"		
		Stud	5' 3"	8' 5"	9' 0"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"		
		Standard	5' 1"	7' 5"	7' 11"	9' 11"	10' 7"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"		

**Bracing Group Species and Grades:**

Group A:			
Spruce-Pine-Fir		Hem-Fir	
#1 / #2	Standard	#2	Stud
#3	Stud	#3	Standard
Douglas Fir-Larch		Southern Pine***	
#3	Stud	#3	Stud
Standard	Standard	Standard	Standard
Group B:			
Hem-Fir			
#1 & Btr			
#1			
Douglas Fir-Larch		Southern Pine***	
#1	Stud	#1	Stud
#2	Standard	#2	Standard

1x4 Braces shall be SRB (Stress-Rated Board).  
 \*\*\*For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

**Gable Truss Detail Notes:**  
 Wind Load deflection criterion is L/240.  
 Provide uplift connections for 55 plf over continuous bearing (5 psf TC Dead Load).  
 Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang.



Attach 'L' braces with 10d (0.128"x3.0" min) nails.  
 \* For (1) 'L' brace: space nails at 2' o.c. in 18' end zones and 4' o.c. between zones.  
 \*\* For (2) 'L' braces: space nails at 3' o.c. in 18' end zones and 6' o.c. between zones.  
 'L' bracing must be a minimum of 80% of web member length.

Gable Vertical Plate Sizes	
Vertical Length	No Splice
Less than 4' 0"	1X4 or 2X3
Greater than 4' 0"	3X4

+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.

155 Harlem Ave  
 North Building, 4th Floor  
 Glenview, IL 60025

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For more information see this Job's general notes page and these web sites:  
 ALPINE: www.alpineitw.com; TPI: www.tpinstr.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org

**BRADLEY E. MORRIS**  
 NUMBER  
 PE-2002019611

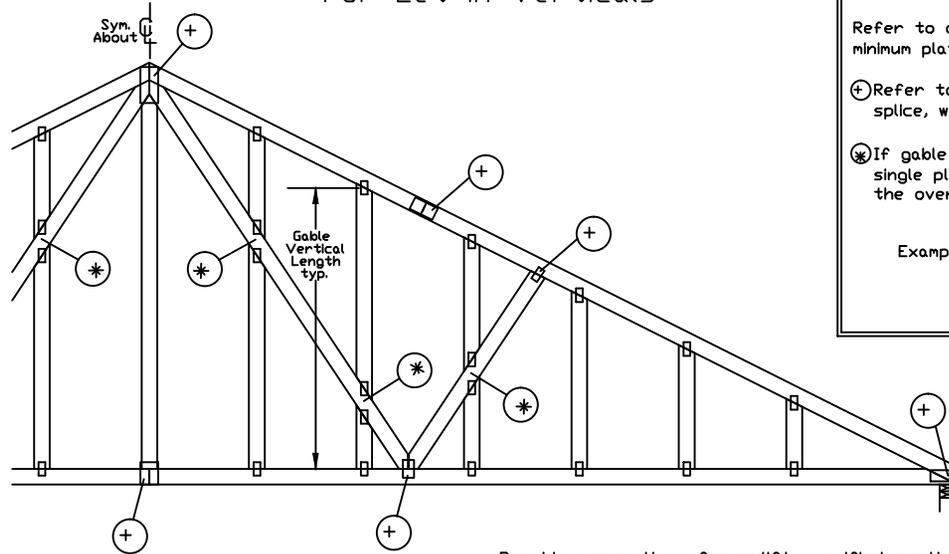
**REGISTERED PROFESSIONAL ENGINEER**

MISSOURI COA #2005000817

MAX. TOT. LD. 60 PSF  
 01/12/2026  
 MAX. SPACING 24.0"

REF	ASCE7-16-GAB14015
DATE	01/26/2018
DRWG	A14015ENC160118

# Gable Detail For Let-in Verticals

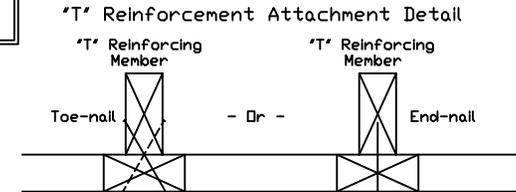


**Gable Truss Plate Sizes**

Refer to appropriate Alpine gable detail for minimum plate sizes for vertical studs.

- ⊕ Refer to Engineered truss design for peak, splice, web, and heel plates.
- ⊗ If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web.

Example:



Provide connections for uplift specified on the engineered truss design.

Attach each 'T' reinforcing member with  
 End Driven Nails:  
 10d Common (0.148"x3",min) Nails at 4' o.c. plus  
 (4) nails in the top and bottom chords.

Toenailed Nails:  
 10d Common (0.148"x3",min) Toenails at 4' o.c. plus  
 (4) toenails in the top and bottom chords.

This detail to be used with the appropriate Alpine gable detail for ASCE wind load.

- ASCE 7-05 Gable Detail Drawings  
 A13015051014, A12015051014, A11015051014, A10015051014, A14015051014,  
 A13030051014, A12030051014, A11030051014, A10030051014, A14030051014
- ASCE 7-10 & ASCE 7-16 Gable Detail Drawings  
 A11515ENC100118, A12015ENC100118, A14015ENC100118, A16015ENC100118,  
 A18015ENC100118, A20015ENC100118, A20015END100118, A20015PED100118,  
 A11530ENC100118, A12030ENC100118, A14030ENC100118, A16030ENC100118,  
 A18030ENC100118, A20030ENC100118, A20030END100118, A20030PED100118,  
 S11515ENC100118, S12015ENC100118, S14015ENC100118, S16015ENC100118,  
 S18015ENC100118, S20015ENC100118, S20015END100118, S20015PED100118,  
 S11530ENC100118, S12030ENC100118, S14030ENC100118, S16030ENC100118,  
 S18030ENC100118, S20030ENC100118, S20030END100118, S20030PED100118

See appropriate Alpine gable detail for maximum 'T' reinforced gable vertical length.

To convert from 'L' to 'T' reinforcing members, multiply 'T' increase by length (based on appropriate Alpine gable detail).

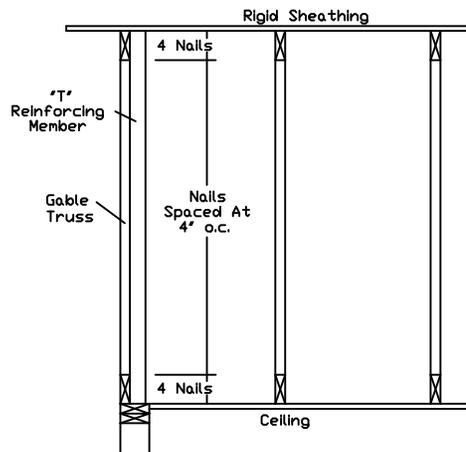
Maximum allowable 'T' reinforced gable vertical length is 14' from top to bottom chord.

'T' reinforcing member material must match size, specie, and grade of the 'L' reinforcing member.

**Web Length Increase w/ 'T' Brace**

'T' Reinf. Mbr. Size	'T' Increase
2x4	30 %
2x6	20 %

Example:  
 ASCE 7-10 Wind Speed = 120 mph  
 Mean Roof Height = 30 ft, Kzt = 1.00  
 Gable Vertical = 24' o.c. SP #3  
 'T' Reinforcing Member Size = 2x4  
 'T' Brace Increase (From Above) = 30% = 1.30  
 (1) 2x4 'L' Brace Length = 8' 7"  
 Maximum 'T' Reinforced Gable Vertical Length  
 1.30 x 8' 7" = 11' 2"



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**IMPORTANT: FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLER**

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 ALPINE: www.alpineitw.com; TPI: www.tpinstr.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org



155 Harlem Ave  
 North Building, 4th Floor  
 Glenview, IL 60025

REF	LET-IN VERT
DATE	01/02/2018
DRWG	GBLLETIN0118
MAX. TOT. LD.	60 PSF
MAX. SPACING	24.0"

# NAIL SPACING DETAIL

MINIMUM SPACING FOR SINGLE BLOCK IS SHOWN. DOUBLE NAIL SPACINGS AND STAGGER NAILING FOR TWO BLOCKS. GREATER SPACING MAY BE REQUIRED TO AVOID SPLITTING.

BLOCK LOCATION, SIZE, LENGTH, GRADE AND TOTAL NUMBER AND TYPE OF NAILS ARE TO BE SPECIFIED ON SEALED DESIGN REFERENCING THIS DETAIL.

LOAD PERPENDICULAR TO GRAIN

- A - EDGE DISTANCE AND SPACING BETWEEN STAGGERED ROWS OF NAILS (6 NAIL DIAMETERS)
- B - SPACING OF NAILS IN A ROW (12 NAIL DIAMETERS)
- C - END DISTANCE (15 NAIL DIAMETERS)

LOAD PARALLEL TO GRAIN

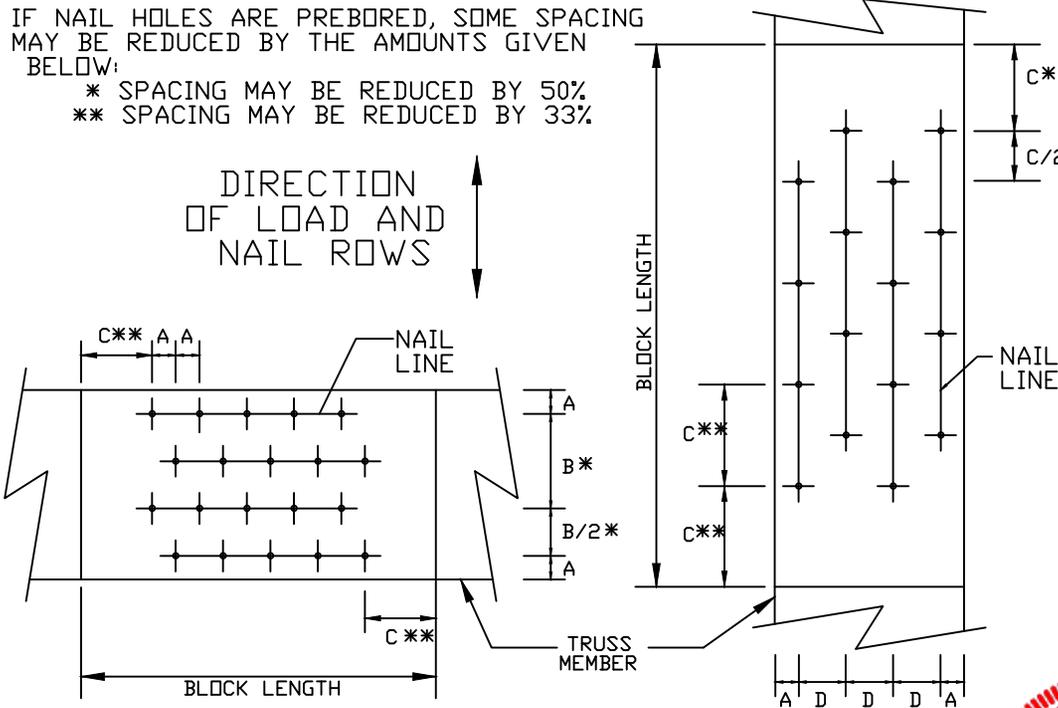
- A - EDGE DISTANCE (6 NAIL DIAMETERS)
- C - SPACING OF NAILS IN A ROW AND END DISTANCE (15 NAIL DIAMETERS)
- D - SPACING BETWEEN STAGGERED ROWS OF NAILS (7 1/2 NAIL DIAMETERS)

IF NAIL HOLES ARE PREBORED, SOME SPACING MAY BE REDUCED BY THE AMOUNTS GIVEN BELOW:

- \* SPACING MAY BE REDUCED BY 50%
- \*\* SPACING MAY BE REDUCED BY 33%

MINIMUM NAIL SPACING DISTANCES

NAIL TYPE	DISTANCES			
	A	B*	C**	D
8d BOX (0.113"X 2.5",MIN)	3/4"	1 3/8"	1 3/4"	7/8"
10d BOX (0.128"X 3.",MIN)	7/8"	1 5/8"	2"	1"
12d BOX (0.128"X 3.25",MIN)	7/8"	1 5/8"	2"	1"
16d BOX (0.135"X 3.5",MIN)	7/8"	1 5/8"	2 1/8"	1 1/8"
20d BOX (0.148"X 4.",MIN)	1"	1 7/8"	2 1/4"	1 1/8"
8d COMMON (0.131"X 2.5",MIN)	7/8"	1 5/8"	2"	1"
10d COMMON (0.148"X 3.",MIN)	1"	1 7/8"	2 1/4"	1 1/8"
12d COMMON (0.148"X 3.25",MIN)	1"	1 7/8"	2 1/4"	1 1/8"
16d COMMON (0.162"X 3.5",MIN)	1"	2"	2 1/2"	1 1/4"
GUN (0.120"X 2.5",MIN)	3/4"	1 1/2"	1 7/8"	1"
GUN (0.131"X 2.5",MIN)	7/8"	1 5/8"	2"	1"
GUN (0.120"X 3.",MIN)	3/4"	1 1/2"	1 7/8"	1"
GUN (0.131"X 3.",MIN)	7/8"	1 5/8"	2"	1"



LOAD APPLIED PERPENDICULAR TO GRAIN      LOAD APPLIED PARALLEL TO GRAIN

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 ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbca.components.com; ICC: www.iccsafe.org



01/12/2026

REF NAIL SPACE  
 DATE 10/01/14  
 DRWG CNNAILSP1014

**RELEASE FOR CONSTRUCTION**  
**AS NOTED ON PLANS REVIEW**  
**DEVELOPMENT SERVICES**  
**LEE'S SUMMIT, MISSOURI**  
**03/10/2026 3:01:15**

# Gable Stud Reinforcement Detail

ASCE 7-16: 140 mph Wind Speed, 30' Mean Height, Enclosed, Exposure C, Kzt = 1.00

- Dr: 120 mph Wind Speed, 30' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00
- Dr: 120 mph Wind Speed, 30' Mean Height, Enclosed, Exposure D, Kzt = 1.00
- Dr: 100 mph wind speed, 30' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

Max Gable Vertical Length	2x4 Gable Vertical Spacing		Brace Grade	No Braces	(1) 1x4 'L' Brace *		(1) 2x4 'L' Brace *		(2) 2x4 'L' Brace **		(1) 2x6 'L' Brace *		(2) 2x6 'L' Brace **	
	Species	#1 / #2			Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B
			24" o.c.	SPF	#1 / #2	#1	4' 1"	6' 11"	7' 2"	8' 2"	8' 6"	9' 9"	10' 2"	12' 10"
#3	3' 10"	6' 2"				6' 7"	8' 1"	8' 5"	9' 8"	10' 0"	12' 8"	13' 2"	14' 0"	14' 0"
Stud	3' 10"	6' 2"				6' 6"	8' 1"	8' 5"	9' 8"	10' 0"	12' 8"	13' 2"	14' 0"	14' 0"
HF	#1	3' 10"			5' 3"	5' 7"	7' 0"	7' 6"	9' 6"	10' 0"	11' 0"	11' 10"	14' 0"	14' 0"
	#2	4' 2"			7' 0"	7' 3"	8' 3"	8' 7"	9' 10"	10' 3"	13' 0"	13' 6"	14' 0"	14' 0"
	Standard	3' 10"			5' 3"	5' 7"	7' 0"	7' 6"	9' 6"	10' 0"	11' 0"	11' 10"	14' 0"	14' 0"
SP	#1	#1		4' 2"	7' 0"	7' 3"	8' 3"	8' 7"	9' 10"	10' 3"	13' 0"	13' 6"	14' 0"	14' 0"
		#2		4' 1"	6' 11"	7' 2"	8' 2"	8' 6"	9' 9"	10' 2"	12' 10"	13' 4"	14' 0"	14' 0"
		#3		4' 0"	5' 7"	5' 11"	7' 5"	7' 11"	9' 8"	10' 1"	11' 7"	12' 5"	14' 0"	14' 0"
	DFL	#1		4' 0"	5' 7"	5' 11"	7' 5"	7' 11"	9' 8"	10' 1"	11' 7"	12' 5"	14' 0"	14' 0"
		#2		3' 9"	4' 11"	5' 13"	6' 6"	7' 0"	8' 10"	9' 6"	10' 3"	11' 0"	13' 11"	14' 0"
		Standard		3' 9"	4' 11"	5' 13"	6' 6"	7' 0"	8' 10"	9' 6"	10' 3"	11' 0"	13' 11"	14' 0"
16" o.c.	SPF	#1 / #2	#1	4' 8"	7' 11"	8' 3"	9' 4"	9' 9"	11' 2"	11' 7"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	4' 5"	7' 6"	8' 3"	9' 3"	9' 7"	11' 0"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	4' 5"	7' 6"	8' 0"	9' 3"	9' 7"	11' 0"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		HF	#1	4' 10"	8' 0"	8' 4"	9' 6"	9' 10"	11' 3"	11' 9"	14' 0"	14' 0"	14' 0"	14' 0"
			#2	4' 8"	7' 11"	8' 3"	9' 4"	9' 9"	11' 2"	11' 7"	14' 0"	14' 0"	14' 0"	14' 0"
			Standard	4' 5"	6' 5"	6' 10"	8' 7"	9' 2"	11' 0"	11' 6"	13' 6"	14' 0"	14' 0"	14' 0"
	SP	#1	#1	4' 10"	8' 0"	8' 4"	9' 6"	9' 10"	11' 3"	11' 9"	14' 0"	14' 0"	14' 0"	14' 0"
			#2	4' 7"	6' 10"	7' 3"	8' 3"	8' 7"	9' 10"	10' 3"	13' 0"	13' 6"	14' 0"	14' 0"
			Standard	4' 5"	6' 0"	6' 5"	8' 0"	8' 7"	10' 10"	11' 6"	12' 7"	13' 15"	14' 0"	14' 0"
		DFL	#1	4' 7"	6' 10"	7' 3"	8' 3"	8' 7"	9' 10"	10' 3"	13' 0"	13' 6"	14' 0"	14' 0"
			#2	4' 5"	6' 0"	6' 5"	8' 0"	8' 7"	10' 10"	11' 6"	12' 7"	13' 15"	14' 0"	14' 0"
			Standard	4' 5"	6' 0"	6' 5"	8' 0"	8' 7"	10' 10"	11' 6"	12' 7"	13' 15"	14' 0"	14' 0"
12" o.c.	SPF	#1 / #2	#1	5' 2"	8' 9"	9' 1"	10' 4"	10' 9"	11' 2"	12' 9"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	4' 10"	8' 7"	8' 11"	10' 2"	10' 7"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	4' 10"	8' 7"	8' 11"	10' 2"	10' 7"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		HF	#1	5' 4"	8' 10"	9' 2"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
			#2	5' 2"	8' 9"	9' 1"	10' 4"	10' 9"	12' 3"	12' 9"	14' 0"	14' 0"	14' 0"	14' 0"
			Standard	4' 10"	7' 5"	7' 11"	9' 11"	10' 7"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
	SP	#1	#1	5' 4"	8' 10"	9' 2"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
			#2	5' 2"	8' 9"	9' 1"	10' 4"	10' 9"	12' 3"	12' 9"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	5' 0"	7' 10"	8' 4"	10' 3"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		DFL	#1	5' 0"	7' 10"	8' 4"	10' 3"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
			#2	5' 0"	7' 10"	8' 4"	10' 3"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
			Standard	4' 10"	6' 11"	7' 4"	9' 3"	9' 10"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"

**Bracing Group Species and Grades:**

**Group A:**

Spruce-Pine-Fir		Hem-Fir	
#1 / #2	Standard	#2	Stud
#3	Stud	#3	Standard

**Douglas Fir-Larch**

#3
Stud
Standard

**Southern Pine\*\*\***

#3
Stud
Standard

**Group B:**

**Hem-Fir**

#1 & Btr
#1

**Douglas Fir-Larch**

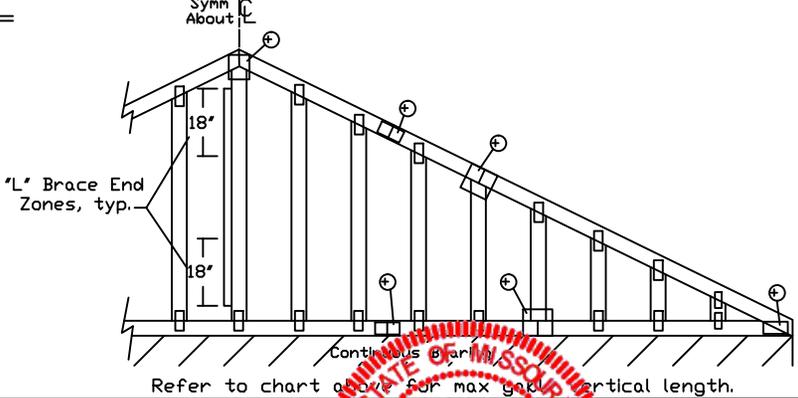
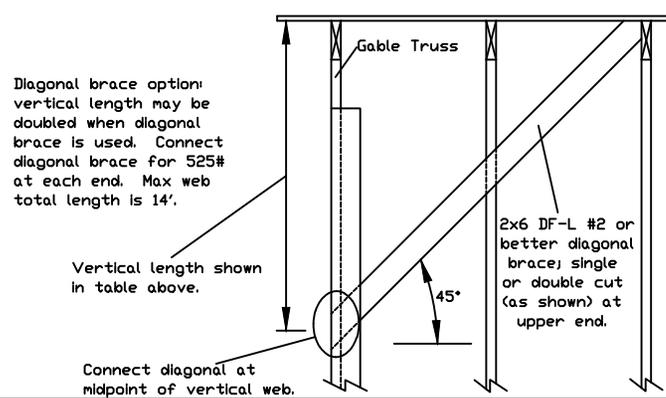
#1
#2

**Southern Pine\*\*\***

#1
#2

1x4 Braces shall be SRB (Stress-Rated Board).  
 \*\*\*For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

**Gable Truss Detail Notes:**  
 Wind Load deflection criterion is L/240.  
 Provide uplift connections for 100 plf over continuous bearing (5 psf TC Dead Load).  
 Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12' plywood overhang.



Attach 'L' braces with 10d (0.128"x3.0" min) nails.  
 \* For (1) 'L' brace: space nails at 2' o.c. in 18" end zones and 4' o.c. between zones.  
 \*\* For (2) 'L' braces: space nails at 3' o.c. in 18" end zones and 6' o.c. between zones.  
 'L' bracing must be a minimum of 80% of web member length.

Gable Vertical Plate Sizes	
Vertical Length	No Splice
Less than 4' 0"	2X4
Greater than 4' 0", but less than 11' 6"	3X4
Greater than 11' 6"	4X4

+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.

155 Harlem Ave  
North Building, 4th Floor  
Glenview, IL 60025

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REF	ASCE7-16-GAB14030
DATE	01/26/2018
DRWG	A14030ENC160118

MAX. TOT. LD. 60 PSF

01/12/2026

MAX. SPACING 24.0"

# CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

## Notes:

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforcement or scab reinforcement.

Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

Use scabs instead of L- or T- reinforcement on webs with intersecting truss joints, such as K-web joints, that may interfere with proper application along the narrow face of the web.

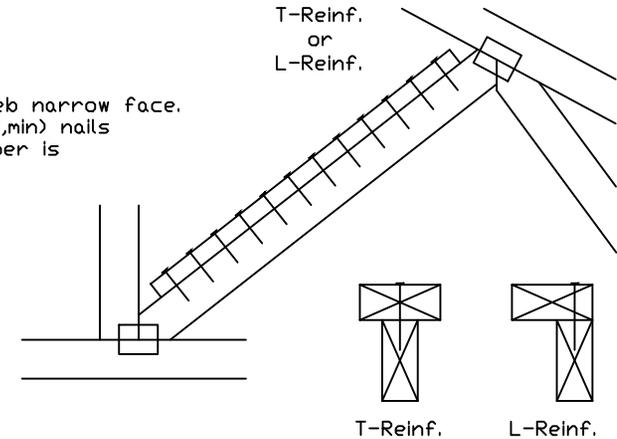
Web Member Size	Specified CLR Restraint	Alternative Reinforcement T- or L- Reinf.	Scab Reinf.
2x3 or 2x4	1 row	2x4	1-2x4
2x3 or 2x4	2 rows	2x6 or 2x4	2-2x4
2x6	1 row	2x4	1-2x6
2x6	2 rows	2x6	2-2x4(*)
2x8	1 row	2x6	1-2x8
2x8	2 rows	2x6	2-2x6(*)

T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

(\*) Center scab on wide face of web. Apply (1) scab to each face of web.

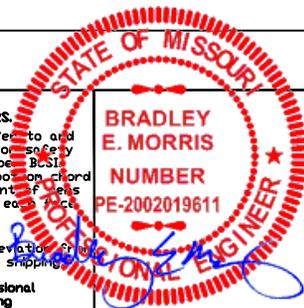
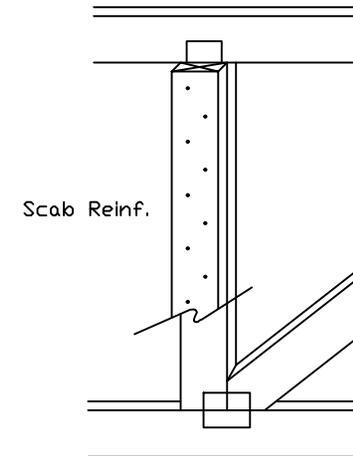
## T-Reinforcement or L-Reinforcement:

Apply to either side of web narrow face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



## Scab Reinforcement:

Apply scab(s) to wide face of web. No more than (1) scab per face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



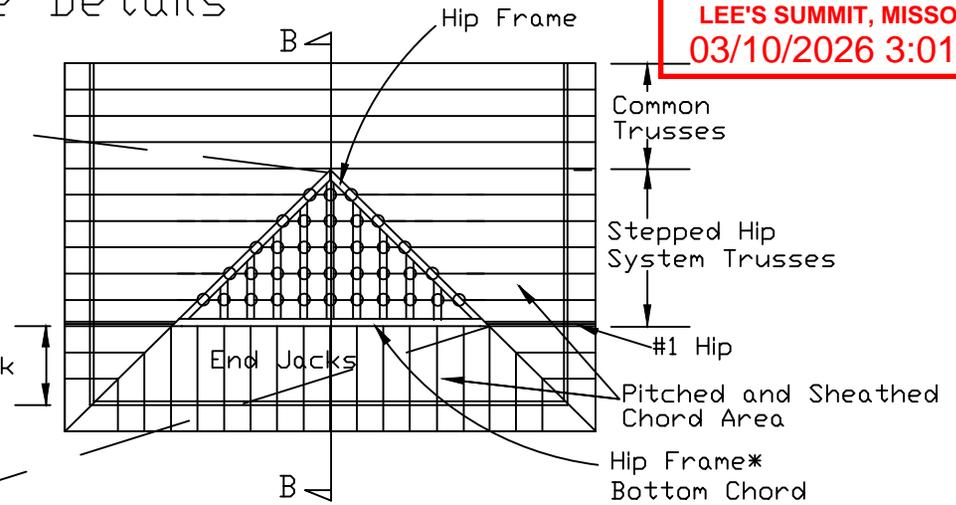
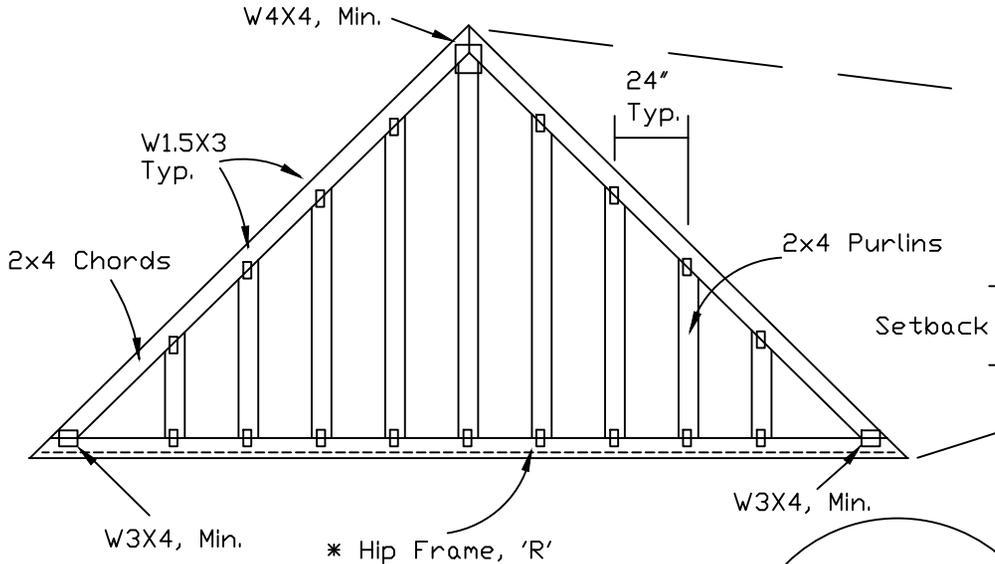
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BRADLEY E. MORRIS  
 NUMBER  
 PE-2002019611  
 MISSOURI COA #2005000817

TC LL	PSF	REF CLR Subst.
TC DL	PSF	DATE 01/02/19
BC DL	PSF	DRWG BRCLBSUB0119
BC LL	PSF	
TOT. LD.	PSF	
DR. PAC.		
SPACING		

DATE	01/02/19
DRWG	BRCLBSUB0119
DR. PAC.	
SPACING	

# \* Hip Frame Details



o- Attach hip frame to flat chords of stepped hips at all overlapping points with 2-10d (0.148"x3") common nails. Bottom chord of hip frame to be attached to #1 hip with 10d common (0.148"x3") nails @ 6" o.c. maximum spacing.

Hip frame stops at plumb cut of jacks to maintain pitch continuity.

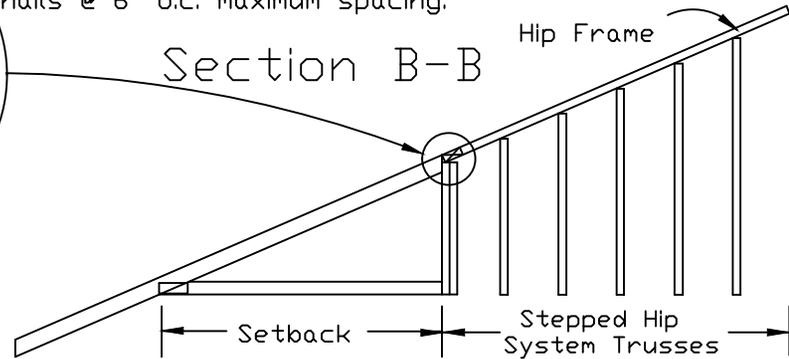
\* Hip frame lumber is SPF, So. Pine, HF, or DFL Standard, Stud grade, or better.

See Engineer's sealed design for setback, lumber, plating, loading, and duration factor required.

'R' Hip frame chords may be trimmed up to 2" to fit. purlins must be intact and properly attached.

Use this detail for:

- ASCE 7-22, 140 mph, 30' M.H., Enclosed, Exp C, or
- ASCE 7-22, 120 mph, 30' M.H., Enclosed, Exp D, or
- ASCE 7-10 & ASCE 7-16, 140 mph, 30' M.H., Enclosed, Exp C, or
- ASCE 7-10 & ASCE 7-16, 120 mph, 30' M.H., Enclosed, Exp D, or
- ASCE 7-05, 110 mph, 30' M.H., Enclosed, Cat II, Exp C, Residential, Wind TC DL=4.2 psf, Kzt=1.00



Hip Frame - provided by truss manufacturer. Hip frame is designed to provide bracing for flat top chords of hip frame system where indicated. Structural panels must be properly attached directly to hip frame purlins.



155 Harlem Ave  
 North Building, 4th Floor  
 Glenview, IL 60025

**WARNING** READ AND FOLLOW ALL NOTES ON THIS DRAWING  
**IMPORTANT** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLER  
 Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of trusses shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. Apply plates to each end of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions.  
 Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any damage to this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses.  
 A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.  
 For more information see this Job's general notes page and these web sites:  
 ALPINE: www.alpineitw.com; TPI: www.tpinet.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org



01/12/2026

REF	HIP FRAME
DATE	06/23/2023
DRWG	HIPFRAME0623

# Valley Detail - ASCE 7-16: 180 mph, 30' Mean Height, Partially Enclosed, Exp. C, Kz=1.00

Top Chord 2x4 SP #2N, SPF #1/#2, DF-L #2 or better.  
 Bot Chord 2x4 SP #2N or SPF #1/#2 or better.  
 Webs 2x4 SP #3, SPF #1/#2, DF-L #2 or better.

\*\* Attach each valley to every supporting truss with:  
 535# connection or with (1) Simpson H2.5A or equivalent connector for  
 ASCE 7-16 180 mph. 30' Mean Height, Part. Enc. Building, Exp. C, Wind TC DL=5 psf, Kzt = 1.00  
 Or  
 ASCE 7-16 160 mph. 30' Mean Height, Part. Enc. Building, Exp. D, Wind TC DL=5 psf, Kzt = 1.00

Bottom chord may be square or pitched cut as shown.

Valleys short enough to be cut as solid triangular members from a single 2x6, or larger as required, shall be permitted in lieu of fabricating from separate 2x4 members.

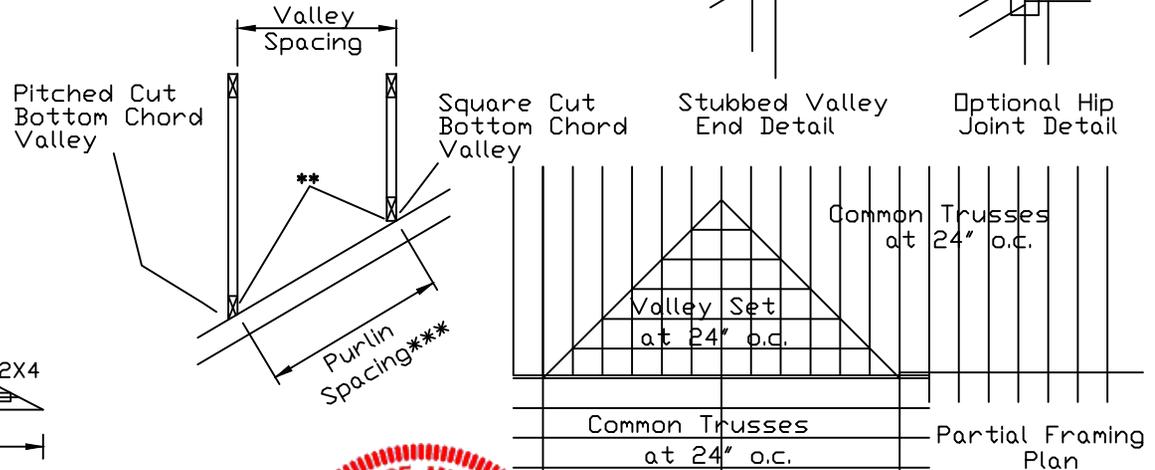
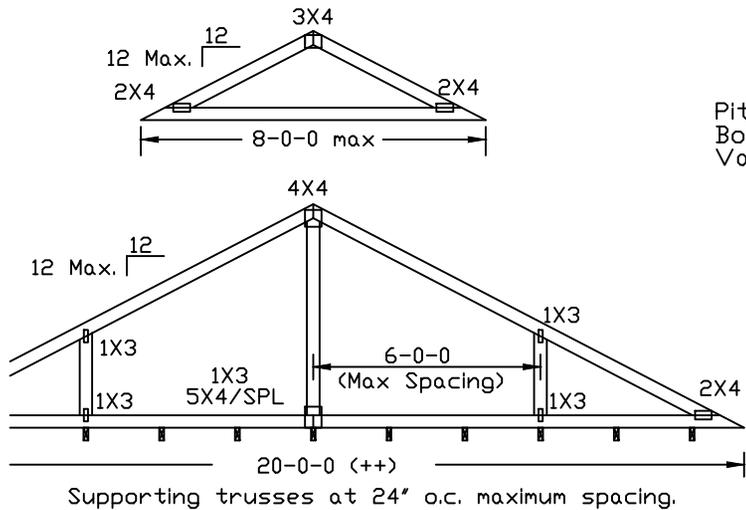
All plates shown are Alpine Wave Plates.

Unless specified otherwise on engineer's sealed design, for vertical valley webs taller than 7-9" apply 2x4 "T" reinforcement, same species and grade or better, attached with 10d box (0.128" x 3.0") nails at 6" o.c. In lieu of "T" reinforcement, 2x4 Continuous Lateral Restraint applied at mid-length of web is permitted with diagonal bracing as shown in DRWG BRCLBANC1014.

Top chord of truss beneath valley set must be braced with:  
 properly attached, rated sheathing applied prior to valley truss installation.  
 Or  
 Purlins at 24" o.c. or as otherwise specified on engineer's sealed design  
 Or  
 By valley trusses used in lieu of purlin spacing as specified on Engineer's sealed design.

\*\*\* Note that the purlin spacing for bracing the top chord of the truss beneath the valley is measured along the slope of the top chord.

++ Larger spans may be built as long as the vertical height does not exceed 14'-0".



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TC LL	30	30	40PSF	REF	VALLEY DETAIL
TC DL	20	15	7PSF	DATE	01/26/2018
BC DL	10	10	10 PSF	DRWG	VAL180160118
BC LL	0	0	0PSF		
TOT. LD.	60	55	57PSF		
OUR PACE	1.25/1.33	1.15	1.15		
SPACING	24.0"				