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STATE OF MISSOURI
BRADLEY E. MORRIS
NUMBER PE-2002019611
PROFESSIONAL ENGINEER

MISSOURI COA #2005000817

01/12/2026

Site Information:	Page 1:
Customer: Kodiak – Premier Building Supply	*Job Number:* PM000125
Job Description: Customer - Clayton Properties Plan Name - Saffron Elevation - Farmhouse	
Address: 1344 SE Windbreak Dr, Lee's Summit, MO	

Job Engineering Criteria:		
Building Code: IRC 2018	*Design Standard:* TPI 2014	*IntelliVIEW Version:* 25.02.00B
Loading Standard: ASCE 7-16	*Design Methodology:* ASD	*JRef #:* 1YGQ96460002
Wind: Wind Speed (mph): 115	*Exposure:* B	*Design Loading (psf):* 45
Building Type: Part Enc.	*Risk Category:* II	
Snow: Pg (psf): NA	*Pf(ASD) (psf):* NA	
W2::	*Risk Category:* II	

This package contains general notes pages, 17 truss drawing(s) and 4 detail(s).

Item	Drawing Number	Truss
1	012.26.1415.19523	J4
3	012.26.1414.28320	A3
5	012.26.1413.58547	A1
7	012.26.1414.05637	A2
9	012.26.1414.47290	C1
11	012.26.1415.11083	J3
13	012.26.1414.51213	J2
15	012.26.1414.17970	A2
17	012.26.1415.08107	J2
19	GBLLETIN0118	
21	A14015ENC160118	

Item	Drawing Number	Truss
2	012.26.1415.24523	J5
4	012.26.1414.44823	B3
6	012.26.1414.33040	B2
8	012.26.1414.09117	A2
10	012.26.1415.32653	J7
12	012.26.1415.26857	J6
14	012.26.1414.49240	J1
16	012.26.1414.53140	J2
18	A14030ENC160118	
20	BRCLBSUB0119	

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:
The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high-quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:
Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:
The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed, and detailed by the Building Designer.

Connector Plate Information:
Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Bearing Information:
The bearing area factor, Cb, is considered for the allowable capacity of solid sawn wood bearings supporting trusses that are located a minimum of 3" from the end of the lumber piece.

General Notes (continued)

Coated Lumber:
Coated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Coated lumber has no adjustments to lumber properties. Coated lumber may be more brittle than uncoated lumber. Special handling care must be taken to prevent breakage during all handling activities. Refer to manufacturer literature, specifications, and code evaluation reports for restrictions, details, and requirements.

Fire Retardant Treated Lumber:
Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

Key to Terms:
Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.
BCLL = Bottom Chord standard design Live Load in pounds per square foot.
C = Coated lumber.
C-AT = AtTEK coated lumber.
C-FX = FX Lumber Guard coated lumber.
C -TE = TechWood 4400 coated lumber.
CL = Certified lumber.
Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.
FRT = Fire Retardant Treated lumber.
FRT-BF = Boraflame Fire Retardant Treated lumber
FRT-DB = D-Blaze Fire Retardant Treated lumber.
FRT-DC = Dricon Fire Retardant Treated lumber.
FRT-FP = FirePRO Fire Retardant Treated lumber.
FRT-FL = FlamePRO Fire Retardant Treated lumber.
FRT-FT = FlameTech Fire Retardant Treated lumber.
FRT-ON = OnWood Fire Retardant Treated lumber.
FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.
FRT-PR = ProWood Fire Retardant Treated lumber.
g = green lumber.
HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.
HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.
HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.
Ic = Incised lumber.
FJ = Finger Jointed lumber.
L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.
L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.
Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.
Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for all load cases.
Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for all load cases.
Max Web CSI= Maximum bending and axial Combined Stress Index for Webs for all load cases.
NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.
PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.
PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds
PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.
PP = Panel Point.
R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).
-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).
Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).
RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

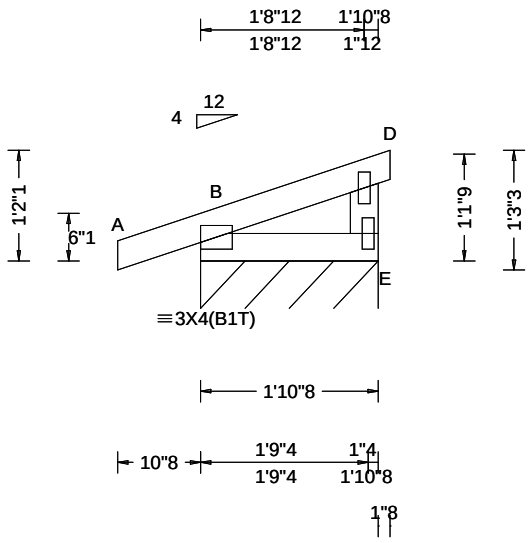
General Notes (continued)

Key to Terms (continued):
Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).
TCDL = Top Chord standard design Dead Load in pounds per square foot.
TCLL = Top Chord standard design Live Load in pounds per square foot.
U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).
VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.
VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.
VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.
VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.
W = Width of non-hanger bearing, in inches.

Refer to ASCE-7 for Wind and Seismic abbreviations.
Uppercase Acronyms not explained above are as defined in TPI 1.

References:
1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 155 Harlem Ave, North Building, 4th Floor, Glenview, IL 60025; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www. sbcacomponents.com

SEQN: 5744 GABL FROM:	Ply: 1 Qty: 1	**Job Number:** PM000125 Customer - Clayton Properties Plan Name - Saffron Elevation - Farmhouse **Truss Label:** J4	Cust: R 9646 JRef: 1YGQ96460002 T1 DrwNo: 012.26.1415.19523 TBM 01/12/2026



Loading Criteria (psf)
TCLL: 25.00
TCDL: 10.00
BCLL: 0.00
BCDL: 10.00
Des Ld: 45.00
NCBCLL: 0.00
Soffit: 0.00
Load Duration: 1.15
Spacing: 24.0 "

Wind Criteria
Speed: 115 mph
Risk Category: II
Enclosure: Part. Enc.
EXP: B Kzt: NA
TCDL: 6.0 psf
BCDL: 6.0 psf
Mean Height: 15.00 ft
MWFRS Parallel Dist: h to 2h
C&C Dist a: 3.00 ft
Loc. from endwall: not in 12.11 ft
GCpi: 0.55
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf(ASD): NA
Ce: NA Lu: NA
Cs: NA Snow Duration: NA

Building Code:
IRC 2018
Load Std: ASCE 7-16
TPI Std: 2014
Rep Fac: Varies by Ld Case
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): NA			
VERT(CL): NA			
HORZ(LL): -0.001	B	-	-
HORZ(TL): 0.001	B	-	-

Creep Factor: 2.0
Max TC CSI: 0.110
Max BC CSI: 0.022
Max Web CSI: 0.017

VIEW Ver: 25.02.00B.1125.14

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
E*	164	/-	/-	/74	/24	/21

Wind reactions based on MWFRS
E Brg Wid = 22.5 Min Req = -
Bearing B Fcperp = 425psi.
Members not listed have forces less than 375#

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;

Plating Notes
All plates are 1.5X4 except as noted.

Loading
Truss designed to support 1-4-0 top chord outlookers and cladding load not to exceed 3.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Wind
Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Snow
Overhang designed for 2.00X TC LL.

Additional Notes
See DWGS A14015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.
Top Chord overhang(s) may be field trimmed.

MISSOURI COA #2005000817
01/12/2026

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS DRAWING!
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Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.
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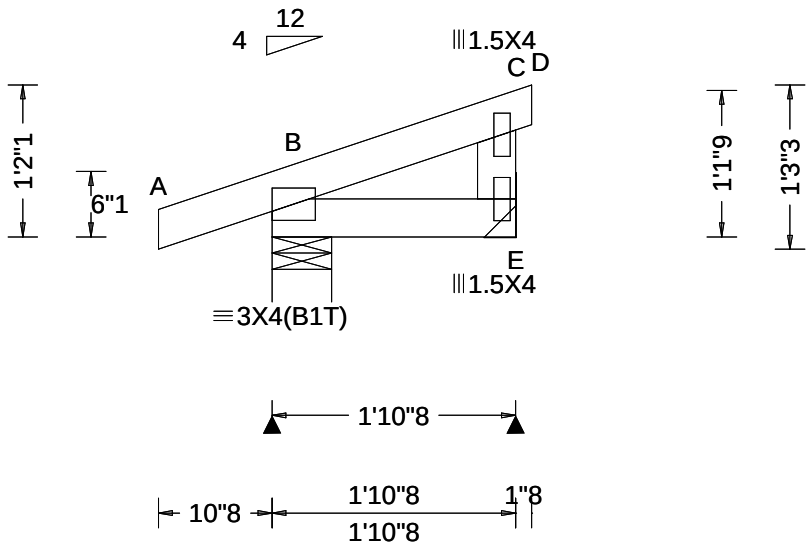
North Building, 4th Floor
Glenview, IL 60025

SEQN: 5745 MONO FROM:	Ply: 1 Qty: 5	**Job Number:** PM000125 Customer - Clayton Properties Plan Name - Saffron Elevation - Farmhouse **Truss Label:** J5	Cust: R 9646 JRef: 1YGQ96460002 T10 DrwNo: 012.26.1415.24523 BM 01/12/2026



Loading Criteria (psf)
TCLL: 25.00
TCDL: 10.00
BCLL: 0.00
BCDL: 10.00
Des Ld: 45.00
NCBCLL: 0.00
Soffit: 0.00
Load Duration: 1.15
Spacing: 24.0 "

Wind Criteria
Speed: 115 mph
Risk Category: II
Enclosure: Part. Enc.
EXP: B Kzt: NA
TCDL: 6.0 psf
BCDL: 6.0 psf
Mean Height: 15.00 ft
MWFRS Parallel Dist: h/2 to h
C&C Dist a: 3.00 ft
Loc. from endwall: not in 9.00 ft
GCpi: 0.55
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf(ASD): NA
Ce: NA Lu: NA
Cs: NA Snow Duration: NA

Building Code:
IRC 2018
Load Std: ASCE 7-16
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): NA			
VERT(CL): NA			
HORZ(LL): -0.001	B	-	-
HORZ(TL): 0.001	B	-	-

Creep Factor: 2.0
Max TC CSI: 0.108
Max BC CSI: 0.024
Max Web CSI: 0.011

VIEW Ver: 25.02.00B.1125.14

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	183	/-	/-	/69	/1	/21
E	88	/-	/-	/47	/7	/-

Wind reactions based on MWFRS
B Brg Wid = 5.5 Min Req = 1.5 (Support)
E Brg Wid = - Min Req = -
Bearing B Fcperp = 425psi.
Members not listed have forces less than 375#

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;

Wind
Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Snow
Overhang designed for 2.00X TC LL.

Additional Notes
Top Chord overhang(s) may be field trimmed.

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01/12/2026

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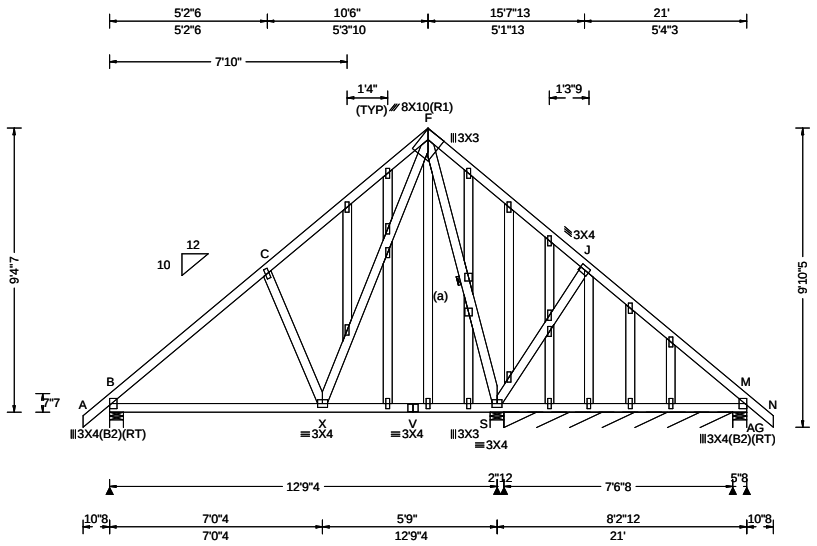
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 5737 GABL FROM:	Ply: 1 Qty: 1	**Job Number:** PM000125 Customer - Clayton Properties Plan Name - Saffron Elevation - Farmhouse **Truss Label:** A3	Cust: R 9646 JRef: 1YGQ96460002 T11 DrwNo: 012.26.1414.28320 I BM 01/12/2026



Loading Criteria (psf)

TCLL:	25.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	45.00
NCBCLL:	10.00
Soffit:	0.00

Load Duration: 1.15
Spacing: 24.0 "

Wind Criteria
Speed: 115 mph
Risk Category: II
Enclosure: Part. Enc.
EXP: B Kzt: NA
TCDL: 6.0 psf
BCDL: 6.0 psf
Mean Height: 23.21 ft
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.55
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf(ASD): NA
Ce: NA Lu: NA
Cs: NA Snow Duration: NA

Building Code:
IRC 2018
Load Std: ASCE 7-16
TPI Std: 2014
Rep Fac: Varies by Ld Case
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.037	D	999	240
VERT(CL): 0.069	D	999	180
HORZ(LL): 0.024	D	-	-
HORZ(TL): 0.045	D	-	-

Creep Factor: 2.0
Max TC CSI: 0.739
Max BC CSI: 0.342
Max Web CSI: 0.443

VIEW Ver: 25.02.00B.1125.14

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	898	/-	/-	/482	/89	/208
S	1074	/-	/-	/513	/82	/-
S*	93	/-	/-	/64	/19	/-
AG	256	/-	/-	/119	/-	/-

Wind reactions based on MWFRS
B Brg Wid = 5.5 Min Req = 1.5 (Support)
S Brg Wid = 5.5 Min Req = 1.5 (Truss)
S Brg Wid = 90.5 Min Req = -
AG Brg Wid = 5.5 Min Req = 1.5 (Support)
Bearings B, S, S, & AG Fcperp = 425psi.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	188	- 947	C - F	426	- 710

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.
B - X	592	- 79

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
C - X	272	- 455	F - S	0	- 808
X - F	720	- 285			

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;

Bracing
(a) Continuous lateral restraint equally spaced on member.

Plating Notes
All plates are 1.5X4 except as noted.

Loading
Truss designed to support 1-4-0 top chord outlookers and cladding load not to exceed 3.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.
Bottom chord checked for 10.00 psf non-concurrent live load.

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Snow
Overhang designed for 2.00X TC LL.

Additional Notes
See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.
Top Chord overhang(s) may be field trimmed.

MISSOURI COA #2005000817
01/12/2026

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For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

ALPINE
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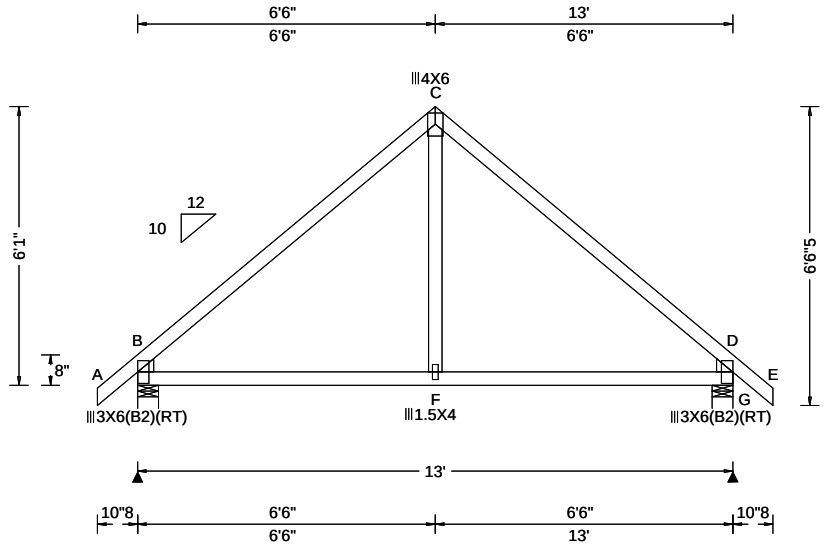
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 5739 COMN FROM:	Ply: 1 Qty: 3	**Job Number:** PM000125 Customer - Clayton Properties Plan Name - Saffron Elevation - Farmhouse **Truss Label:** B3	Cust: R 9646 JRef: 1YGQ96460002 T12 DrwNo: 012.26.1414.44823 BM 01/12/2026



Loading Criteria (psf)

TCLL:	25.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	45.00
NCBCLL:	10.00
Soffit:	0.00

Load Duration: 1.15
Spacing: 24.0 "

Wind Criteria
Speed: 115 mph
Risk Category: II
Enclosure: Part. Enc.
EXP: B Kzt: NA
TCDL: 6.0 psf
BCDL: 6.0 psf
Mean Height: 21.59 ft
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.55
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf(ASD): NA
Ce: NA Lu: NA
Cs: NA Snow Duration: NA

Building Code:
IRC 2018
Load Std: ASCE 7-16
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.007	F	999	240
VERT(CL): 0.012	F	999	180
HORZ(LL): 0.008	B	-	-
HORZ(TL): 0.016	D	-	-

Creep Factor: 2.0
Max TC CSI: 0.640
Max BC CSI: 0.445
Max Web CSI: 0.087

VIEW Ver: 25.02.00B.1125.14

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	693	/-	/-	/378	/34	/92
G	693	/-	/-	/378	/34	/-

Wind reactions based on MWFRS
B Brg Wid = 5.5 Min Req = 1.5 (Support)
G Brg Wid = 5.5 Min Req = 1.5 (Support)
Bearings B & G Fcperp = 425psi.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	155	-693	C - D	154	-693

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - F	435	0	F - D	435	0

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;
Lt Wedge: 2x4 SP #2;Rt Wedge: 2x4 SP #2;

Loading
Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Bottom chord checked for 10.00 psf non-concurrent live load.

Live loads applied in combination per ASCE 7 sec. 2.4.1 use 0.75 factor for multiple live loads.

Wind
Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Snow
Overhang designed for 2.00X TC LL.

Additional Notes
Top Chord overhang(s) may be field trimmed.

MISSOURI COA #2005000817
01/12/2026

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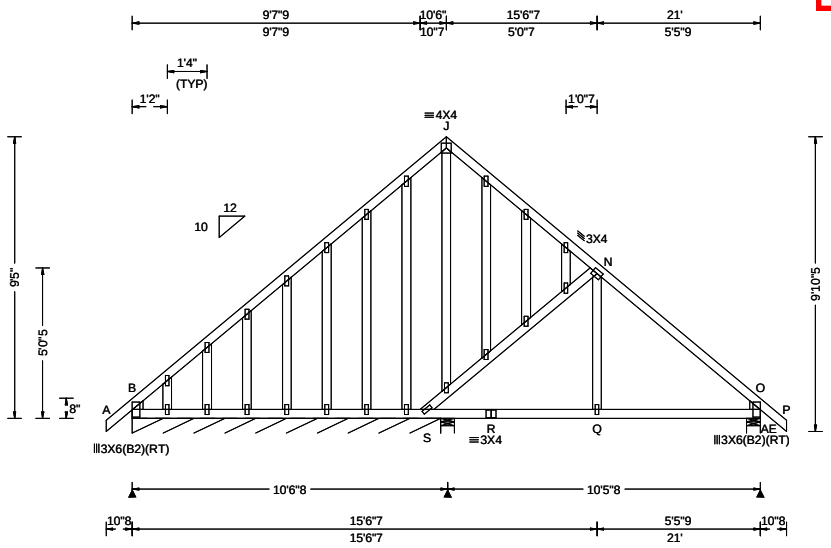
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 5735 GABL FROM:	Ply: 1 Qty: 1	**Job Number:** PM000125 Customer - Clayton Properties Plan Name - Saffron Elevation - Farmhouse **Truss Label:** A1	Cust: R 9646 JRef: 1YGQ96460002 T14 DrwNo: 012.26.1413.58547 I BM 01/12/2026



Loading Criteria (psf)

TCLL:	25.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	45.00
NCBCLL:	10.00
Soffit:	0.00

Load Duration: 1.15
Spacing: 24.0 "

Wind Criteria
Speed: 115 mph
Risk Category: II
Enclosure: Part. Enc.
EXP: B Kzt: NA
TCDL: 6.0 psf
BCDL: 6.0 psf
Mean Height: 23.26 ft
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.55
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf(ASD): NA
Ce: NA Lu: NA
Cs: NA Snow Duration: NA

Building Code:
IRC 2018
Load Std: ASCE 7-16
TPI Std: 2014
Rep Fac: Varies by Ld Case
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.043	L	999	240
VERT(CL): 0.084	L	999	180
HORZ(LL): 0.037	AC	-	-
HORZ(TL): 0.073	AC	-	-

Creep Factor: 2.0
Max TC CSI: 0.698
Max BC CSI: 0.260
Max Web CSI: 0.144

VIEW Ver: 25.02.00B.1125.14

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B*	186	/-	/-	/98	/31	/20
S	118	/-	/-	/98	/-	/-
AE	942	/-	/-	/461	/45	/-

Wind reactions based on MWFRS
B Brg Wid = 123 Min Req = -
S Brg Wid = 5.5 Min Req = 1.5 (Truss)
AE Brg Wid = 5.5 Min Req = 1.5 (Support)
Bearings B, S, & AE Fcperp = 425psi.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
J - N	201	- 459	N - O	100	- 1011

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
S - R	1274	0	Q - O	636	0
R - Q	637	0			

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.
S - N	308	- 640

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;
Lt Wedge: 2x4 SP #2;Rt Wedge: 2x4 SP #2;

Plating Notes
All plates are 1.5X4 except as noted.

Loading
Truss designed to support 1-4-0 top chord outlookers and cladding load not to exceed 3.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.
Bottom chord checked for 10.00 psf non-concurrent live load.

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Snow
Overhang designed for 2.00X TC LL.

Additional Notes
See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.
Top Chord overhang(s) may be field trimmed.

MISSOURI COA #2005000817
01/12/2026

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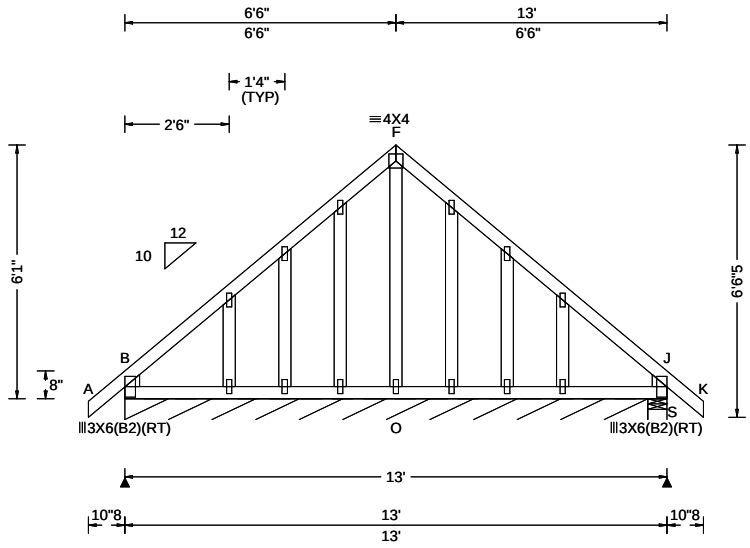
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 5738 GABL FROM:	Ply: 1 Qty: 1	**Job Number:** PM000125 Customer - Clayton Properties Plan Name - Saffron Elevation - Farmhouse **Truss Label:** B2	Cust: R 9646 JRef: 1YGQ96460002 T16 DrwNo: 012.26.1414.33040 BM 01/12/2026



Loading Criteria (psf)	**Wind Criteria**	**Snow Criteria** (Pg,Pf in PSF)	**Defl/CSI Criteria**
TCLL: 25.00	Speed: 115 mph	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#
TCDL: 10.00	Risk Category: II	Pf(ASD): NA	VERT(LL): 0.001 G 999 240
BCLL: 0.00	Enclosure: Part. Enc.	Ce: NA Lu: NA	VERT(CL): 0.002 F 999 180
BCDL: 10.00	EXP: B Kzt: NA	Cs: NA Snow Duration: NA	HORZ(LL): -0.001 B - -
Des Ld: 45.00	TCDL: 6.0 psf		HORZ(TL): 0.005 I - -
NCBCLL: 10.00	BCDL: 6.0 psf	Building Code:	Creep Factor: 2.0
Soffit: 0.00	Mean Height: 21.59 ft	IRC 2018	Max TC CSI: 0.116
Load Duration: 1.15	MWFRS Parallel Dist: 0 to h/2	Load Std: ASCE 7-16	Max BC CSI: 0.051
Spacing: 24.0 "	C&C Dist a: 3.00 ft	TPI Std: 2014	Max Web CSI: 0.175
	Loc. from endwall: Any	Rep Fac: Varies by Ld Case	
	GCpi: 0.55	FT/RT:20(0)/10(0)	
	Wind Duration: 1.60	Plate Type(s): WAVE	VIEW Ver: 25.02.00B.1125.14

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B*	124	/-	/-	/68	/13	/10
S	266	/-	/-	/123	/6	/-

Wind reactions based on MWFRS
B Brg Wid = 150 Min Req = -
S Brg Wid = 5.5 Min Req = 1.5 (Support)
Bearings B & S Fcperp = 425psi.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;
Lt Wedge: 2x4 SP #2;Rt Wedge: 2x4 SP #2;

Plating Notes

All plates are 1.5X4 except as noted.

Loading

Truss designed to support 1-4-0 top chord outlookers and cladding load not to exceed 3.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Bottom chord checked for 10.00 psf non-concurrent live load.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Snow

Overhang designed for 2.00X TC LL.

Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Top Chord overhang(s) may be field trimmed.

MISSOURI COA #2005000817
01/12/2026

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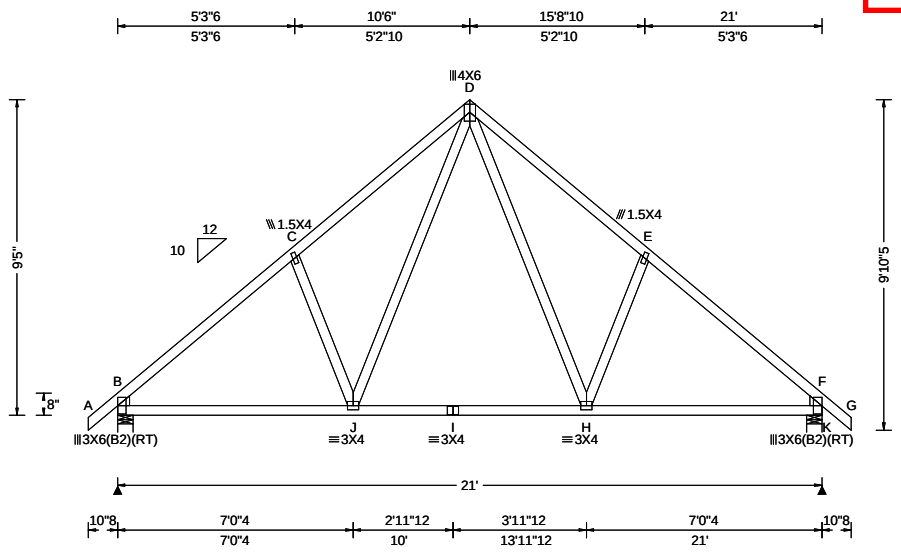
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 5748 COMN FROM:	Ply: 1 Qty: 20	**Job Number:** PM000125 Customer - Clayton Properties Plan Name - Saffron Elevation - Farmhouse **Truss Label:** A2	Cust: R 9646 JRef: 1YGQ96460002 T17 DrwNo: 012.26.1414.05637 BM 01/12/2026



Loading Criteria (psf)

TCLL:	25.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	45.00
NCBCLL:	10.00
Soffit:	0.00

Load Duration: 1.15
Spacing: 24.0 "

Wind Criteria
Speed: 115 mph
Risk Category: II
Enclosure: Part. Enc.
EXP: B Kzt: NA
TCDL: 6.0 psf
BCDL: 6.0 psf
Mean Height: 23.26 ft
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: not in 4.50 ft
GCpi: 0.55
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf(ASD): NA
Ce: NA Lu: NA
Cs: NA Snow Duration: NA

Building Code:
IRC 2018
Load Std: ASCE 7-16
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.040	J	999	240
VERT(CL): 0.073	J	999	201
HORZ(LL): 0.027	F	-	-
HORZ(TL): 0.043	F	-	-

Creep Factor: 2.0
Max TC CSI: 0.427
Max BC CSI: 0.606
Max Web CSI: 0.166

VIEW Ver: 25.02.00B.1125.14

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	1076	/-	/-	/606	/70	/150
K	1076	/-	/-	/606	/70	/-

Wind reactions based on MWFRS
B Brg Wid = 5.5 Min Req = 1.5 (Support)
K Brg Wid = 5.5 Min Req = 1.5 (Support)
Bearings B & K Fcperp = 425psi.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	129	-1271	D - E	242	-1120
C - D	242	-1120	E - F	129	-1271

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - J	878	0	I - H	595	0
J - I	595	0	H - F	878	0

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
J - D	476	-100	D - H	475	-100

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;
Lt Wedge: 2x4 SP #2;Rt Wedge: 2x4 SP #2;

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Bottom chord checked for 10.00 psf non-concurrent live load.

Live loads applied in combination per ASCE 7 sec. 2.4.1 use 0.75 factor for multiple live loads.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Snow

Overhang designed for 2.00X TC LL.

Additional Notes

Top Chord overhang(s) may be field trimmed.

MISSOURI COA #2005000817
01/12/2026

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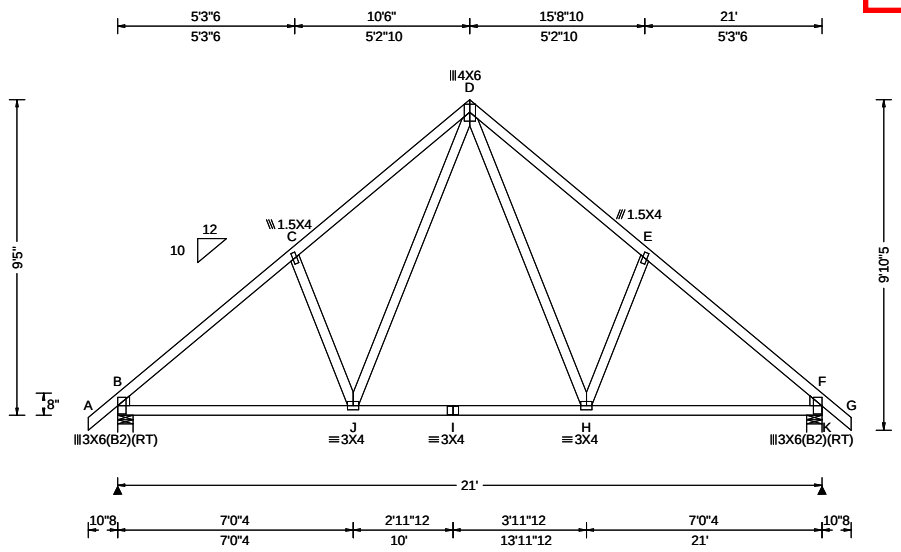
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 5749 COMN FROM:	Ply: 1 Qty: 20	**Job Number:** PM000125 Customer - Clayton Properties Plan Name - Saffron Elevation - Farmhouse **Truss Label:** A2	Cust: R 9646 JRef: 1YGQ96460002 T18 DrwNo: 012.26.1414.69.117 BM 01/12/2026



Loading Criteria (psf)

TCLL:	25.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	45.00
NCBCLL:	10.00
Soffit:	0.00

Load Duration: 1.15
Spacing: 24.0 "

Wind Criteria
Speed: 115 mph
Risk Category: II
Enclosure: Part. Enc.
EXP: B Kzt: NA
TCDL: 6.0 psf
BCDL: 6.0 psf
Mean Height: 23.26 ft
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: not in 4.50 ft
GCpi: 0.55
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf(ASD): NA
Ce: NA Lu: NA
Cs: NA Snow Duration: NA

Building Code:
IRC 2018
Load Std: ASCE 7-16
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in		loc	L/defl	L/#
VERT(LL):	0.040	J	999	240
VERT(CL):	0.073	J	999	201
HORZ(LL):	0.027	F	-	-
HORZ(TL):	0.043	F	-	-

Creep Factor: 2.0
Max TC CSI: 0.427
Max BC CSI: 0.606
Max Web CSI: 0.166

VIEW Ver: 25.02.00B.1125.14

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	1076	/-	/-	/606	/70	/150
K	1076	/-	/-	/606	/70	/-

Wind reactions based on MWFRS
B Brg Wid = 5.5 Min Req = 1.5 (Support)
K Brg Wid = 5.5 Min Req = 1.5 (Support)
Bearings B & K Fcperp = 425psi.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	129	-1271	D - E	242	-1120
C - D	242	-1120	E - F	129	-1271

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - J	878	0	I - H	595	0
J - I	595	0	H - F	878	0

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
J - D	476	-100	D - H	475	-100

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;
Lt Wedge: 2x4 SP #2;Rt Wedge: 2x4 SP #2;

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Bottom chord checked for 10.00 psf non-concurrent live load.

Live loads applied in combination per ASCE 7 sec. 2.4.1 use 0.75 factor for multiple live loads.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Snow

Overhang designed for 2.00X TC LL.

Additional Notes

Top Chord overhang(s) may be field trimmed.

MISSOURI COA #2005000817
01/12/2026

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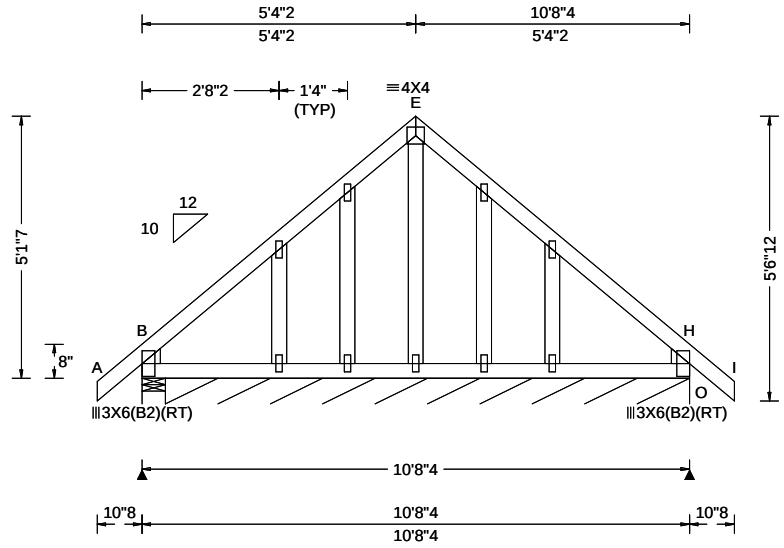
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 5740 GABL FROM:	Ply: 1 Qty: 1	**Job Number:** PM000125 Customer - Clayton Properties Plan Name - Saffron Elevation - Farmhouse **Truss Label:** C1	Cust: R 9646 JRef: 1YGQ96460002 T2 DrwNo: 012.26.1414.47290 BM 01/12/2026



Loading Criteria (psf)
TCLL: 25.00
TCDL: 10.00
BCLL: 0.00
BCDL: 10.00
Des Ld: 45.00
NCBCLL: 10.00
Soffit: 0.00
Load Duration: 1.15
Spacing: 24.0 "

Wind Criteria
Speed: 115 mph
Risk Category: II
Enclosure: Part. Enc.
EXP: B Kzt: NA
TCDL: 6.0 psf
BCDL: 6.0 psf
Mean Height: 21.11 ft
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.55
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf(ASD): NA
Ce: NA Lu: NA
Cs: NA Snow Duration: NA

Building Code:
IRC 2018
Load Std: ASCE 7-16
TPI Std: 2014
Rep Fac: Varies by Ld Case
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.001	G	999	240
VERT(CL): 0.001	E	999	180
HORZ(LL): -0.001	B	-	-
HORZ(TL): 0.004	G	-	-

Creep Factor: 2.0
Max TC CSI: 0.122
Max BC CSI: 0.041
Max Web CSI: 0.063

VIEW Ver: 25.02.00B.1125.14

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	279	/-	/-	/152	/40	/105
O*	121	/-	/-	/67	/11	/-

Wind reactions based on MWFRS
B Brg Wid = 5.5 Min Req = 1.5 (Support)
O Brg Wid = 122 Min Req = -
Bearings B & B Fcperp = 425psi.
Members not listed have forces less than 375#

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;
Lt Wedge: 2x4 SP #2;Rt Wedge: 2x4 SP #2;

Plating Notes
All plates are 1.5X4 except as noted.

Loading
Truss designed to support 1-4-0 top chord outlookers and cladding load not to exceed 3.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.
Bottom chord checked for 10.00 psf non-concurrent live load.

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Snow
Overhang designed for 2.00X TC LL.

Additional Notes
See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.
Top Chord overhang(s) may be field trimmed.

MISSOURI COA #2005000817
01/12/2026

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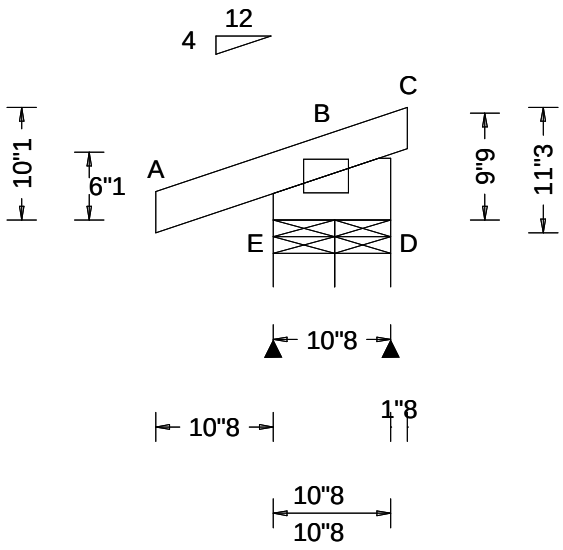
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 5747 GABL FROM:	Ply: 1 Qty: 1	**Job Number:** PM000125 Customer - Clayton Properties Plan Name - Saffron Elevation - Farmhouse **Truss Label:** J7	Cust: R 9646 JRef: 1YGQ96460002 T3 DrwNo: 012.26.1418.32653 BM 01/12/2026



Loading Criteria (psf)
TCLL: 25.00
TCDL: 10.00
BCLL: 0.00
BCDL: 10.00
Des Ld: 45.00
NCBCLL: 0.00
Soffit: 0.00
Load Duration: 1.15
Spacing: 24.0 "

Wind Criteria
Speed: 115 mph
Risk Category: II
Enclosure: Part. Enc.
EXP: B Kzt: NA
TCDL: 6.0 psf
BCDL: 6.0 psf
Mean Height: 15.00 ft
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.55
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf(ASD): NA
Ce: NA Lu: NA
Cs: NA Snow Duration: NA

Building Code:
IRC 2018
Load Std: ASCE 7-16
TPI Std: 2014
Rep Fac: Varies by Ld Case
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): -0.001	D	999	240
VERT(CL): -0.001	D	999	180
HORZ(LL): -0.000	D	-	-
HORZ(TL): 0.000	D	-	-

Creep Factor: 2.0
Max TC CSI: 0.109
Max BC CSI: 0.009
Max Web CSI: 0.000

VIEW Ver: 25.02.00B.1125.14

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
E	212	/-	/-	/69	/92	/35
D	42	/-68	/-	/23	/8	/-

Wind reactions based on MWFRS
E Brg Wid = 5.5 Min Req = 1.5 (Support)
D Brg Wid = 5.0 Min Req = 1.5 (Support)
Bearings E & B Fcperp = 425psi.
Members not listed have forces less than 375#

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x6 SP #2;

Plating Notes
All plates are 3X4(A1T) except as noted.

Loading
Truss designed to support 1-4-0 top chord outlookers and cladding load not to exceed 3.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Snow
Overhang designed for 2.00X TC LL.

Additional Notes
Top Chord overhang(s) may be field trimmed.

STATE OF MISSOURI
BRADLEY E. MORRIS
NUMBER PE-2002019611
PROFESSIONAL ENGINEER

MISSOURI COA #2005000817
01/12/2026

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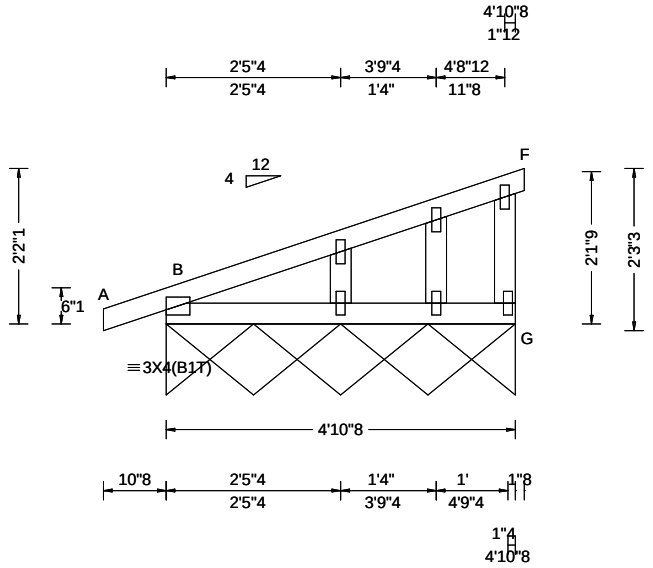
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 5743 GABL FROM:	Ply: 1 Qty: 1	**Job Number:** PM000125 Customer - Clayton Properties Plan Name - Saffron Elevation - Farmhouse **Truss Label:** J3	Cust: R 9646 JRef: 1YGQ96460002 T4 DrwNo: 012.26.1415.11083 BM 01/12/2026



Loading Criteria (psf)
TCLL: 25.00
TCDL: 10.00
BCLL: 0.00
BCDL: 10.00
Des Ld: 45.00
NCBCLL: 0.00
Soffit: 0.00
Load Duration: 1.15
Spacing: 24.0 "

Wind Criteria
Speed: 115 mph
Risk Category: II
Enclosure: Part. Enc.
EXP: B Kzt: NA
TCDL: 6.0 psf
BCDL: 6.0 psf
Mean Height: 15.00 ft
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.55
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf(ASD): NA
Ce: NA Lu: NA
Cs: NA Snow Duration: NA

Building Code:
IRC 2018
Load Std: ASCE 7-16
TPI Std: 2014
Rep Fac: Varies by Ld Case
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): -0.000	B	999	240
VERT(CL): 0.000	B	999	180
HORZ(LL): -0.001	B	-	-
HORZ(TL): 0.002	D	-	-

Creep Factor: 2.0
Max TC CSI: 0.116
Max BC CSI: 0.042
Max Web CSI: 0.035

VIEW Ver: 25.02.00B.1125.14

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
G*	133	/-	/-	/66	/39	/23

Wind reactions based on MWFRS
G Brg Wid = 58.5 Min Req = -
Bearing B Fcperp = 425psi.
Members not listed have forces less than 375#

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;

Plating Notes
All plates are 1.5X4 except as noted.

Loading
Truss designed to support 1-4-0 top chord outlookers and cladding load not to exceed 3.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Wind
Wind loads based on MWFRS with additional C&C member design.
Right end vertical exposed to wind pressure. Deflection meets L/180.
Wind loading based on both gable and hip roof types.

Snow
Overhang designed for 2.00X TC LL.

Additional Notes
See DWGS A14015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.
Top Chord overhang(s) may be field trimmed.

MISSOURI COA #2005000817
01/12/2026

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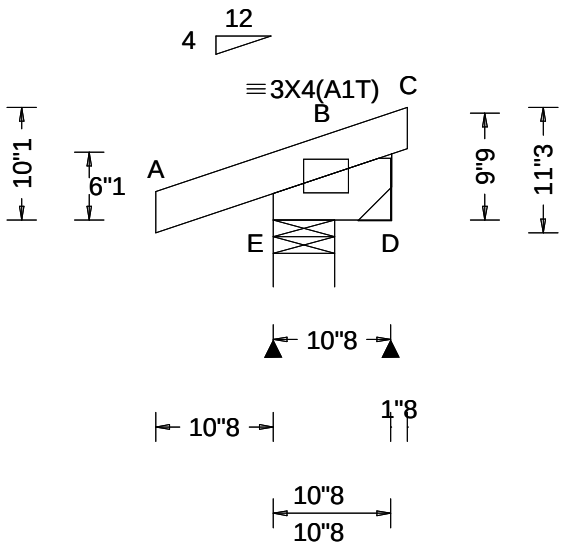
ALPINE
AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 5746 MONO FROM:	Ply: 1 Qty: 5	**Job Number:** PM000125 Customer - Clayton Properties Plan Name - Saffron Elevation - Farmhouse **Truss Label:** J6	Cust: R 9646 JRef: 1YGQ96460002 T5 DrwNo: 012.26.1415.26857 BM 01/12/2026



Loading Criteria (psf)
TCLL: 25.00
TCDL: 10.00
BCLL: 0.00
BCDL: 10.00
Des Ld: 45.00
NCBCLL: 0.00
Soffit: 0.00
Load Duration: 1.15
Spacing: 24.0 "

Wind Criteria
Speed: 115 mph
Risk Category: II
Enclosure: Part. Enc.
EXP: B Kzt: NA
TCDL: 6.0 psf
BCDL: 6.0 psf
Mean Height: 15.00 ft
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.55
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf(ASD): NA
Ce: NA Lu: NA
Cs: NA Snow Duration: NA

Building Code:
IRC 2018
Load Std: ASCE 7-16
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): -0.001	D	999	240
VERT(CL): -0.001	D	999	180
HORZ(LL): -0.000	D	-	-
HORZ(TL): 0.000	D	-	-

Creep Factor: 2.0
Max TC CSI: 0.109
Max BC CSI: 0.009
Max Web CSI: 0.000

VIEW Ver: 25.02.00B.1125.14

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
E	212	/-	/-	/53	/8	/13
D	42	/-68	/-	/19	/5	/-

Wind reactions based on MWFRS
E Brg Wid = 5.5 Min Req = 1.5 (Support)
D Brg Wid = - Min Req = -
Bearing E Fcperp = 425psi.
Members not listed have forces less than 375#

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x6 SP #2;

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Snow
Overhang designed for 2.00X TC LL.

Additional Notes
Top Chord overhang(s) may be field trimmed.

MISSOURI COA #2005000817
01/12/2026

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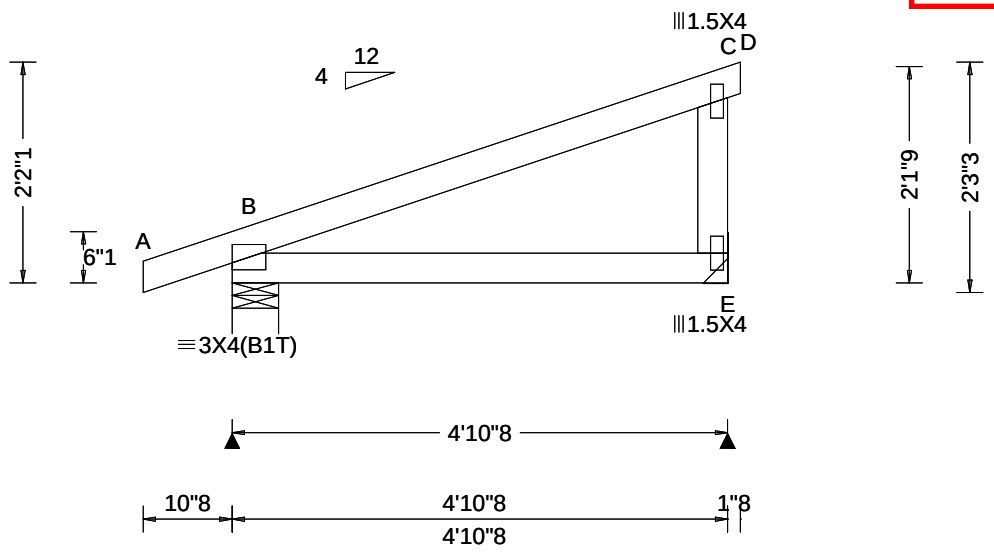
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 5742 MONO FROM:	Ply: 1 Qty: 14	**Job Number:** PM000125 Customer - Clayton Properties Plan Name - Saffron Elevation - Farmhouse **Truss Label:** J2	Cust: R 9646 JRef: 1YGQ96460002 T6 DrwNo: 012.26.1414.51213 BM 01/12/2026



Loading Criteria (psf)
TCLL: 25.00
TCDL: 10.00
BCLL: 0.00
BCDL: 10.00
Des Ld: 45.00
NCBCLL: 0.00
Soffit: 0.00
Load Duration: 1.15
Spacing: 24.0 "

Wind Criteria
Speed: 115 mph
Risk Category: II
Enclosure: Part. Enc.
EXP: B Kzt: NA
TCDL: 6.0 psf
BCDL: 6.0 psf
Mean Height: 15.00 ft
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.55
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf(ASD): NA
Ce: NA Lu: NA
Cs: NA Snow Duration: NA

Building Code:
IRC 2018
Load Std: ASCE 7-16
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): NA			
VERT(CL): NA			
HORZ(LL): 0.005	B	-	-
HORZ(TL): 0.009	B	-	-

Creep Factor: 2.0
Max TC CSI: 0.407
Max BC CSI: 0.211
Max Web CSI: 0.078

VIEW Ver: 25.02.00B.1125.14

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	299	/-	/-	/138	/-	/48
E	224	/-	/-	/127	/14	/-

Wind reactions based on MWFRS
B Brg Wid = 5.5 Min Req = 1.5 (Support)
E Brg Wid = - Min Req = -
Bearing B Fcperp = 425psi.
Members not listed have forces less than 375#

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;

Wind
Wind loads based on MWFRS with additional C&C member design.
Right end vertical not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

Snow
Overhang designed for 2.00X TC LL.

Additional Notes
Top Chord overhang(s) may be field trimmed.

STATE OF MISSOURI
BRADLEY E. MORRIS
NUMBER PE-2002019611
PROFESSIONAL ENGINEER

MISSOURI COA #2005000817
01/12/2026

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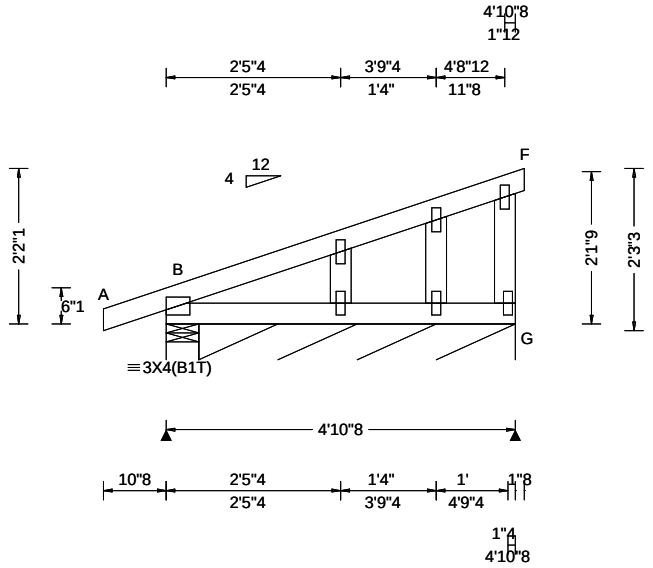
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 5741 GABL FROM:	Ply: 1 Qty: 1	**Job Number:** PM000125 Customer - Clayton Properties Plan Name - Saffron Elevation - Farmhouse **Truss Label:** J1	Cust: R 9646 JRef:1YGQ96460002 T7 DrwNo: 012.26.1414.49240 BM 01/12/2026



Loading Criteria (psf)

TCLL:	25.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	45.00
NCBCLL:	0.00
Soffit:	0.00

Load Duration: 1.15
Spacing: 24.0 "

Wind Criteria
Speed: 115 mph
Risk Category: II
Enclosure: Part. Enc.
EXP: B Kzt: NA
TCDL: 6.0 psf
BCDL: 6.0 psf
Mean Height: 15.00 ft
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.55
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf(ASD): NA
Ce: NA Lu: NA
Cs: NA Snow Duration: NA

Building Code:
IRC 2018
Load Std: ASCE 7-16
TPI Std: 2014
Rep Fac: Varies by Ld Case
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): -0.000	B	999	240
VERT(CL): 0.000	B	999	180
HORZ(LL): -0.001	B	-	-
HORZ(TL): 0.001	B	-	-

Creep Factor: 2.0
Max TC CSI: 0.116
Max BC CSI: 0.033
Max Web CSI: 0.035

VIEW Ver: 25.02.00B.1125.14

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	244	/-	/-	/92	/62	/112
G*	91	/-	/-	/55	/29	/-

Wind reactions based on MWFRS
B Brg Wid = 5.5 Min Req = 1.5 (Support)
G Brg Wid = 53.0 Min Req = -
Bearings B & B Fcperp = 425psi.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;

Plating Notes

All plates are 1.5X4 except as noted.

Loading

Truss designed to support 1-4-0 top chord outlookers and cladding load not to exceed 3.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical exposed to wind pressure. Deflection meets L/180.

Wind loading based on both gable and hip roof types.

Snow

Overhang designed for 2.00X TC LL.

Additional Notes

See DWGS A14015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.

Top Chord overhang(s) may be field trimmed.

MISSOURI COA #2005000817
01/12/2026

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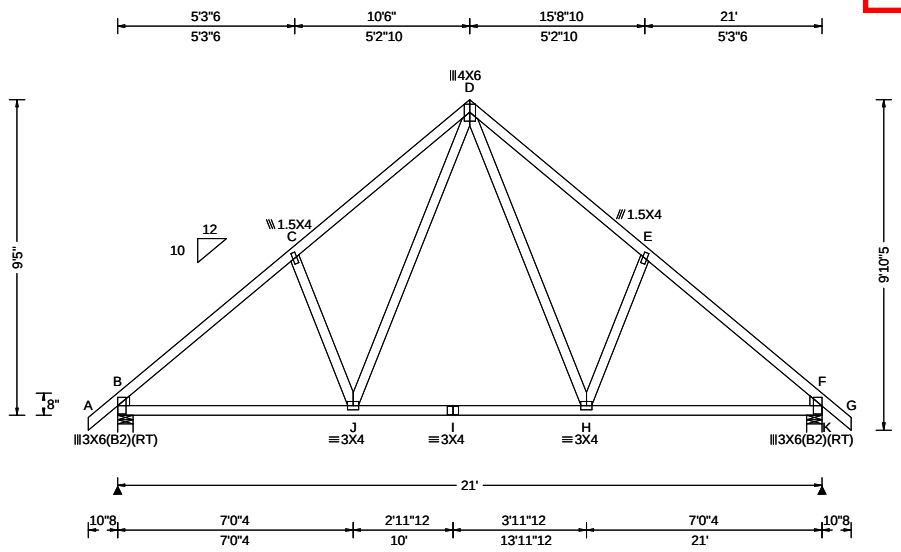
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 5736 COMN FROM:	Ply: 1 Qty: 20	**Job Number:** PM000125 Customer - Clayton Properties Plan Name - Saffron Elevation - Farmhouse **Truss Label:** A2	Cust: R 9646 JRef: 1YGQ96460002 T9 DrwNo: 012.26.1414.17970 TBM 01/12/2026



Loading Criteria (psf)

TCLL:	25.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	45.00
NCBCLL:	10.00
Soffit:	0.00

Load Duration: 1.15
Spacing: 24.0 "

Wind Criteria
Speed: 115 mph
Risk Category: II
Enclosure: Part. Enc.
EXP: B Kzt: NA
TCDL: 6.0 psf
BCDL: 6.0 psf
Mean Height: 23.26 ft
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: not in 4.50 ft
GCpi: 0.55
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf(ASD): NA
Ce: NA Lu: NA
Cs: NA Snow Duration: NA

Building Code:
IRC 2018
Load Std: ASCE 7-16
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): 0.040	J	999	240
VERT(CL): 0.073	J	999	201
HORZ(LL): 0.027	F	-	-
HORZ(TL): 0.043	F	-	-

Creep Factor: 2.0
Max TC CSI: 0.427
Max BC CSI: 0.606
Max Web CSI: 0.166

VIEW Ver: 25.02.00B.1125.14

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	1076	/-	/-	/606	/70	/150
K	1076	/-	/-	/606	/70	/-

Wind reactions based on MWFRS
B Brg Wid = 5.5 Min Req = 1.5 (Support)
K Brg Wid = 5.5 Min Req = 1.5 (Support)
Bearings B & K Fcperp = 425psi.
Members not listed have forces less than 375#

Maximum Top Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - C	129	- 1271	D - E	242	- 1120
C - D	242	- 1120	E - F	129	- 1271

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.	Comp.	Chords	Tens.	Comp.
B - J	878	0	I - H	595	0
J - I	595	0	H - F	878	0

Maximum Web Forces Per Ply (lbs)

Webs	Tens.	Comp.	Webs	Tens.	Comp.
J - D	476	- 100	D - H	475	- 100

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;
Lt Wedge: 2x4 SP #2;Rt Wedge: 2x4 SP #2;

Loading

Truss passed check for 20 psf additional bottom chord live load in areas with 42"-high x 24"-wide clearance.

Bottom chord checked for 10.00 psf non-concurrent live load.

Live loads applied in combination per ASCE 7 sec. 2.4.1 use 0.75 factor for multiple live loads.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Snow

Overhang designed for 2.00X TC LL.

Additional Notes

Top Chord overhang(s) may be field trimmed.

MISSOURI COA #2005000817
01/12/2026

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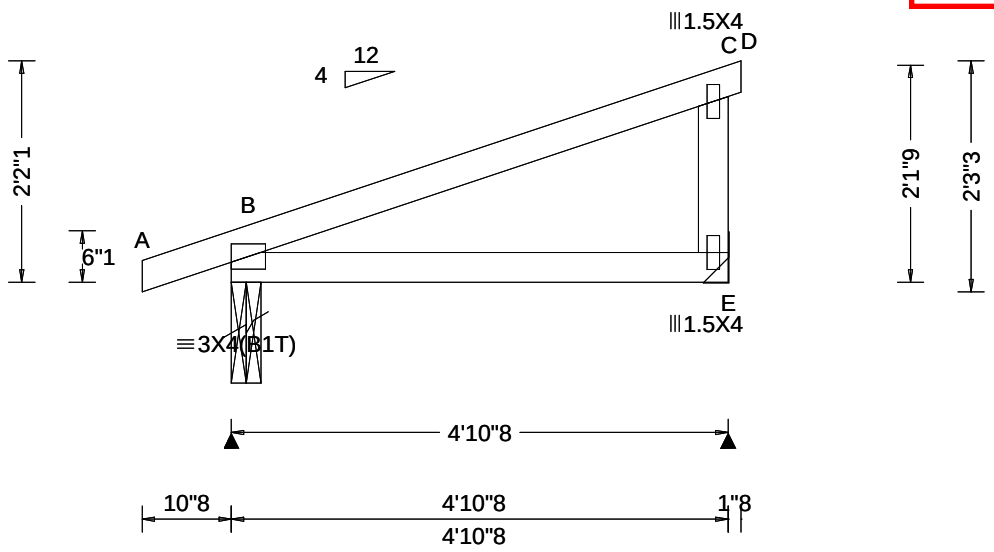
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 5752 MONO FROM:	Ply: 1 Qty: 2	**Job Number:** PM000125 Customer - Clayton Properties Plan Name - Saffron Elevation - Farmhouse **Truss Label:** J2	Cust: R 9646 JRef: 1YGQ96460002 T13 DrwNo: 012.26.1414.53140 BM 01/12/2026



Loading Criteria (psf)

TCLL:	25.00
TCDL:	10.00
BCLL:	0.00
BCDL:	10.00
Des Ld:	45.00
NCBCLL:	0.00
Soffit:	0.00

Load Duration: 1.15
Spacing: 24.0 "

Wind Criteria
Speed: 115 mph
Risk Category: II
Enclosure: Part. Enc.
EXP: B Kzt: NA
TCDL: 6.0 psf
BCDL: 6.0 psf
Mean Height: 15.00 ft
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.55
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf(ASD): NA
Ce: NA Lu: NA
Cs: NA Snow Duration: NA

Building Code:
IRC 2018
Load Std: ASCE 7-16
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): NA			
VERT(CL): NA			
HORZ(LL): 0.005	B	-	-
HORZ(TL): 0.009	B	-	-

Creep Factor: 2.0
Max TC CSI: 0.407
Max BC CSI: 0.211
Max Web CSI: 0.078

VIEW Ver: 25.02.00B.1125.14

▲Maximum Reactions (lbs)

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
B	299	/-	/-	/138	/-	/48
E	224	/-	/-	/127	/14	/-

Wind reactions based on MWFRS
B Brg Wid = 3.5 Min Req = 1.5 (Support)
E Brg Wid = - Min Req = -
Bearing B Fcperp = 425psi.
Members not listed have forces less than 375#

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.

Snow

Overhang designed for 2.00X TC LL.

Additional Notes

Top Chord overhang(s) may be field trimmed.

STATE OF MISSOURI
BRADLEY E. MORRIS
NUMBER PE-2002019611
PROFESSIONAL ENGINEER

MISSOURI COA #2005000817
01/12/2026

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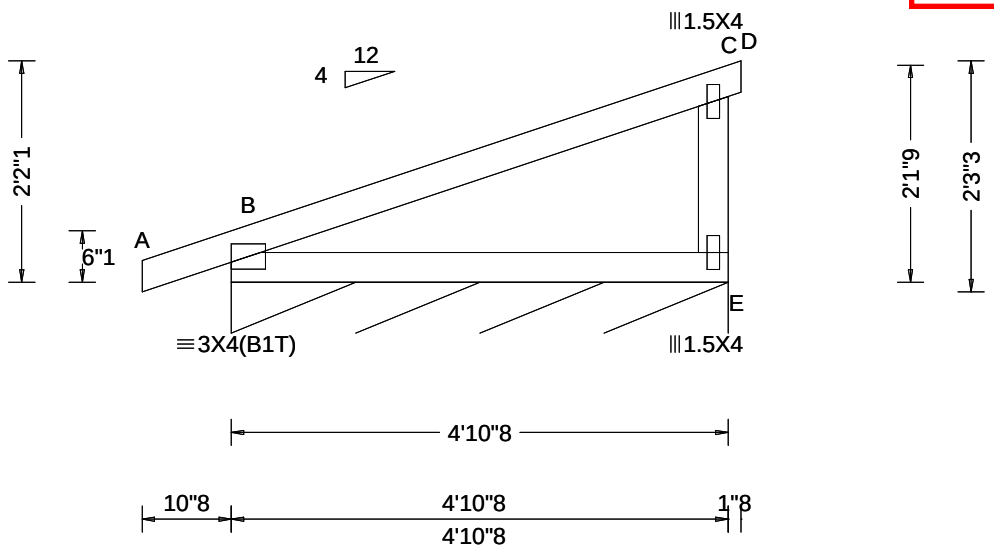
ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

SEQN: 5755 MONO FROM:	Ply: 1 Qty: 1	**Job Number:** PM000125 Customer - Clayton Properties Plan Name - Saffron Elevation - Farmhouse **Truss Label:** J2	Cust: R 9646 JRef: 1YGQ96460002 T8 DrwNo: 012.26.1415.08107 BM 01/12/2026



Loading Criteria (psf)
TCLL: 25.00
TCDL: 10.00
BCLL: 0.00
BCDL: 10.00
Des Ld: 45.00
NCBCLL: 0.00
Soffit: 0.00
Load Duration: 1.15
Spacing: 24.0 "

Wind Criteria
Speed: 115 mph
Risk Category: II
Enclosure: Part. Enc.
EXP: B Kzt: NA
TCDL: 6.0 psf
BCDL: 6.0 psf
Mean Height: 15.00 ft
MWFRS Parallel Dist: 0 to h/2
C&C Dist a: 3.00 ft
Loc. from endwall: Any
GCpi: 0.55
Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
Pg: NA Ct: NA CAT: NA
Pf(ASD): NA
Ce: NA Lu: NA
Cs: NA Snow Duration: NA

Building Code:
IRC 2018
Load Std: ASCE 7-16
TPI Std: 2014
Rep Fac: Yes
FT/RT:20(0)/10(0)
Plate Type(s):
WAVE

Defl/CSI Criteria

PP Deflection in	loc	L/defl	L/#
VERT(LL): NA			
VERT(CL): NA			
HORZ(LL): 0.007	B	-	-
HORZ(TL): 0.012	B	-	-

Creep Factor: 2.0
Max TC CSI: 0.407
Max BC CSI: 0.211
Max Web CSI: 0.081

VIEW Ver: 25.02.00B.1125.14

▲Maximum Reactions (lbs), or *=PLF

	Gravity			Non-Gravity		
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL
E*	106	/-	/-	/53	/2	/9

Wind reactions based on MWFRS
E Brg Wid = 58.5 Min Req = -
Bearing B Fcperp = 425psi.
Members not listed have forces less than 375#

Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #2;

Wind
Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Snow
Overhang designed for 2.00X TC LL.

Additional Notes
Top Chord overhang(s) may be field trimmed.

STATE OF MISSOURI
BRADLEY E. MORRIS
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ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

Gable Stud Reinforcement Detail

ASCE 7-16: 140 mph Wind Speed, 30' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Or: 120 mph Wind Speed, 30' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00
Or: 120 mph Wind Speed, 30' Mean Height, Enclosed, Exposure D, Kzt = 1.00
Or: 100 mph wind speed, 30' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

Max Gable Vertical Length

2x4 Gable Vertical Spacing	Species	Brace Grade	No Braces	(1) 1x4 "L" Brace * Group A	(1) 1x4 "L" Brace * Group B	(1) 2x4 "L" Brace * Group A	(1) 2x4 "L" Brace * Group B	(2) 2x4 "L" Brace ** Group A	(2) 2x4 "L" Brace ** Group B	(1) 2x6 "L" Brace * Group A	(1) 2x6 "L" Brace * Group B	(2) 2x6 "L" Brace ** Group A	(2) 2x6 "L" Brace ** Group B
24" o.c.	SPF HF	#1 / #2	4' 1"	6' 11"	7' 2"	8' 2"	8' 6"	9' 9"	10' 2"	12' 10"	13' 4"	14' 0"	14' 0"
		#3	3' 10"	6' 2"	6' 7"	8' 1"	8' 5"	9' 8"	10' 0"	12' 8"	13' 2"	14' 0"	14' 0"
		Stud	3' 10"	6' 2"	6' 6"	8' 1"	8' 5"	9' 8"	10' 0"	12' 8"	13' 2"	14' 0"	14' 0"
		Standard	3' 10"	5' 3"	5' 7"	7' 0"	7' 6"	9' 6"	10' 0"	11' 0"	11' 10"	14' 0"	14' 0"
	SP DFL	#1	4' 2"	7' 0"	7' 3"	8' 3"	8' 7"	9' 10"	10' 3"	13' 0"	13' 6"	14' 0"	14' 0"
		#2	4' 1"	6' 11"	7' 2"	8' 2"	8' 6"	9' 9"	10' 2"	12' 10"	13' 4"	14' 0"	14' 0"
		#3	4' 0"	5' 7"	5' 11"	7' 5"	7' 11"	9' 8"	10' 1"	11' 7"	12' 5"	14' 0"	14' 0"
		Stud	4' 0"	5' 7"	5' 11"	7' 5"	7' 11"	9' 8"	10' 1"	11' 7"	12' 5"	14' 0"	14' 0"
		Standard	3' 9"	4' 11"	5' 13"	6' 6"	7' 0"	8' 10"	9' 6"	10' 3"	11' 0"	13' 11"	14' 0"
16" o.c.	SPF HF	#1 / #2	4' 8"	7' 11"	8' 3"	9' 4"	9' 9"	11' 2"	11' 7"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 5"	7' 6"	8' 3"	9' 3"	9' 7"	11' 0"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 5"	7' 6"	8' 0"	9' 3"	9' 7"	11' 0"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 5"	6' 5"	6' 10"	8' 7"	9' 2"	11' 0"	11' 6"	13' 6"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	4' 10"	8' 0"	8' 4"	9' 6"	9' 10"	11' 3"	11' 9"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	4' 8"	7' 11"	8' 3"	9' 4"	9' 9"	11' 2"	11' 7"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 7"	6' 10"	7' 3"	9' 1"	9' 8"	11' 1"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 7"	6' 10"	7' 3"	9' 1"	9' 8"	11' 1"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 5"	6' 0"	6' 5"	8' 0"	8' 7"	10' 10"	11' 6"	12' 7"	13' 15"	14' 0"	14' 0"
12" o.c.	SPF HF	#1 / #2	5' 2"	8' 9"	9' 1"	10' 4"	10' 9"	11' 2"	12' 9"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 10"	8' 7"	8' 11"	10' 2"	10' 7"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 10"	8' 7"	8' 11"	10' 2"	10' 7"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 10"	7' 5"	7' 11"	9' 11"	10' 7"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	5' 4"	8' 10"	9' 2"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	5' 2"	8' 9"	9' 1"	10' 4"	10' 9"	12' 3"	12' 9"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 0"	7' 10"	8' 4"	10' 3"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 0"	7' 10"	8' 4"	10' 3"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 10"	6' 11"	7' 4"	9' 3"	9' 10"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"

Bracing Group Species and Grades:

Group A:

Spruce-Pine-Fir	
#1 / #2	Standard
#3	Stud

Hem-Fir	
#2	Stud
#3	Standard

Douglas Fir-Larch
#3
Stud
Standard

Southern Pine***
#3
Stud
Standard

Group B:

Hem-Fir
#1 & Btr
#1

Douglas Fir-Larch
#1
#2

Southern Pine***
#1
#2

1x4 Braces shall be SRB (Stress-Rated Board).

***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes:

Wind Load deflection criterion is L/240.

Provide uplift connections for 100 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang.

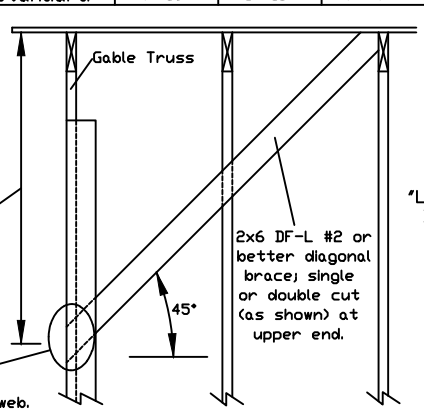
Attach "L" braces with 10d (0.128"x3.0" min) nails.

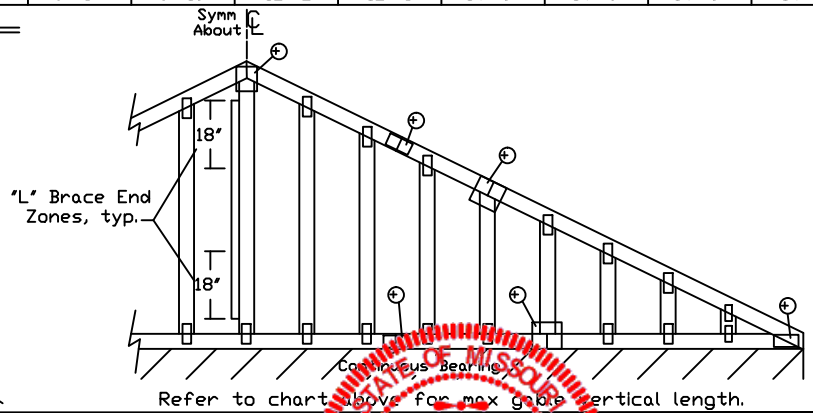
* For (1) "L" brace: space nails at 2" o.c. in 18" end zones and 4" o.c. between zones.

** For (2) "L" braces: space nails at 3" o.c. in 18" end zones and 6" o.c. between zones.

"L" bracing must be a minimum of 80% of web member length.

Diagonal brace option: vertical length may be doubled when diagonal brace is used. Connect diagonal brace for 525# at each end. Max web total length is 14'.





Refer to chart above for max gable vertical length.

Gable Vertical Plate Sizes	
Vertical Length	No Splice
Less than 4' 0"	2X4
Greater than 4' 0", but less than 11' 6"	3X4
Greater than 11' 6"	4X4

+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.

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For more information see this job's general notes page and these web sites:
ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org

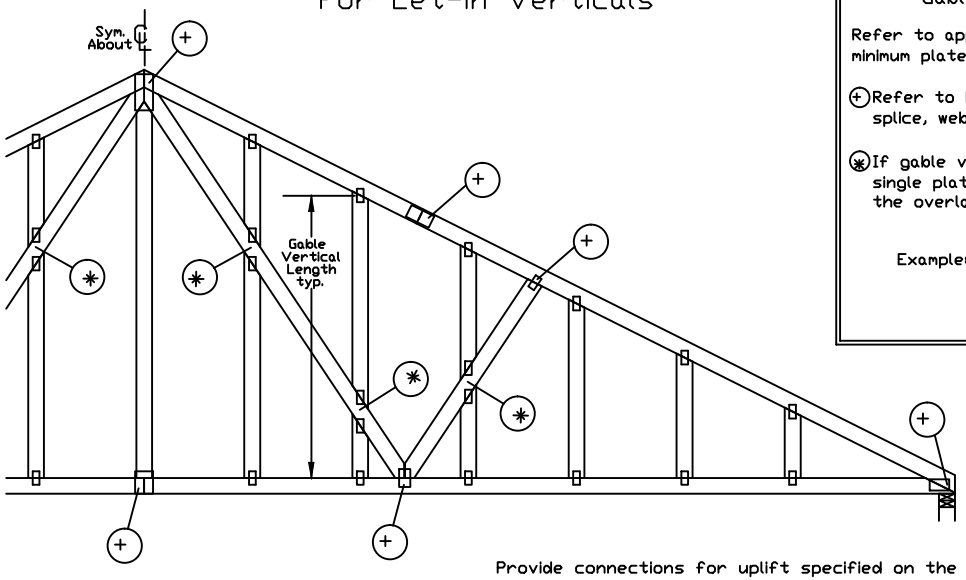
STATE OF MISSOURI
BRADLEY E. MORRIS
NUMBER PE-2002019611
PROFESSIONAL ENGINEER
01/26/2018
MISSOURI COA #2005000817

ALPINE AN ITW COMPANY
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

MAX. TOT. LD. 60 PSF
MAX. SPACING 24.0"

REF	ASCE7-16-GAB14030
DATE	01/26/2018
DRWG	A14030ENC160118

Gable Detail
For Let-in Verticals



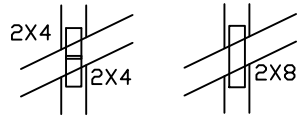
Gable Truss Plate Sizes

Refer to appropriate Alpine gable detail for minimum plate sizes for vertical studs.

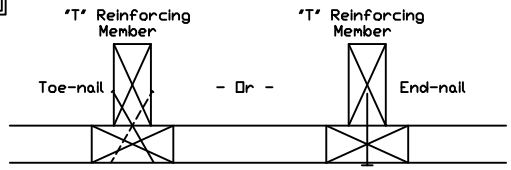
⊕ Refer to Engineered truss design for peak, splice, web, and heel plates.

⊛ If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web.

Example:



"T" Reinforcement Attachment Detail



Provide connections for uplift specified on the engineered truss design.

Attach each "T" reinforcing member with

End Driven Nails:
10d Common (0.148"x 3.",min) Nails at 4" o.c. plus
(4) nails in the top and bottom chords.

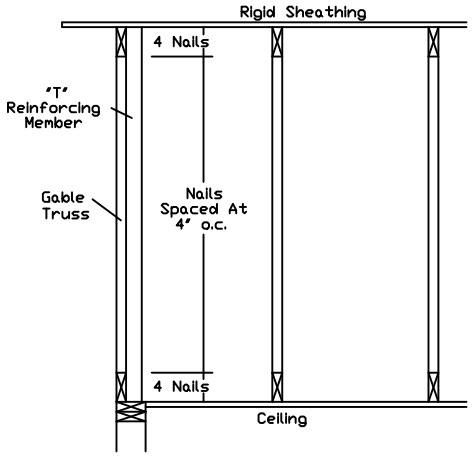
Toenailed Nails:
10d Common (0.148"x3",min) Toenails at 4" o.c. plus
(4) toenails in the top and bottom chords.

This detail to be used with the appropriate Alpine gable detail for ASCE wind load.

ASCE 7-05 Gable Detail Drawings
A13015051014, A12015051014, A11015051014, A10015051014, A14015051014, A13030051014, A12030051014, A11030051014, A10030051014, A14030051014

ASCE 7-10 & ASCE 7-16 Gable Detail Drawings
A11515ENC100118, A12015ENC100118, A14015ENC100118, A16015ENC100118, A18015ENC100118, A20015ENC100118, A20015END100118, A20015PED100118, A11530ENC100118, A12030ENC100118, A14030ENC100118, A16030ENC100118, A18030ENC100118, A20030ENC100118, A20030END100118, A20030PED100118, S11515ENC100118, S12015ENC100118, S14015ENC100118, S16015ENC100118, S18015ENC100118, S20015ENC100118, S20015END100118, S20015PED100118, S11530ENC100118, S12030ENC100118, S14030ENC100118, S16030ENC100118, S18030ENC100118, S20030ENC100118, S20030END100118, S20030PED100118

See appropriate Alpine gable detail for maximum [illegible] gable vertical length.



To convert from "L" to "T" reinforcing members, multiply "T" increase by length (based on appropriate Alpine gable detail).

Maximum allowable "T" reinforced gable vertical length is 14' from top to bottom chord.

"T" reinforcing member material must match size, specie, and grade of the "L" reinforcing member.

Web Length Increase w/ "T" Brace

"T" Reinf. Mbr. Size	"T" Increase
2x4	30 %
2x6	20 %

Example:
ASCE 7-10 Wind Speed = 120 mph
Mean Roof Height = 30 ft, Kzt = 1.00
Gable Vertical = 24'o.c. SP #3
"T" Reinforcing Member Size = 2x4
"T" Brace Increase (From Above) = 30% = 1.30
(1) 2x4 "L" Brace Length = 8' 7"
Maximum "T" Reinforced Gable Vertical Length
1.30 x 8' 7" = 11' 2"

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North Building, 4th Floor
Glenview, IL 60025

STATE OF MISSOURI
BRADLEY E. MORRIS
NUMBER PE-2002019611
PROFESSIONAL ENGINEER
01/12/2026
MISSOURI COA #2005000817

MAX. TOT. LD. 60 PSF
DUR. FAC. ANY
MAX. SPACING 24.0"

REF	LET-IN VERT
DATE	01/02/2018
DRWG	GBLLETIN0118

CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

Notes:

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforecement or scab reinforcement.

Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

Use scabs instead of L- or T- reinforcement on webs with intersecting truss joints, such as K-web joints, that may interfere with proper application along the narrow face of the web.

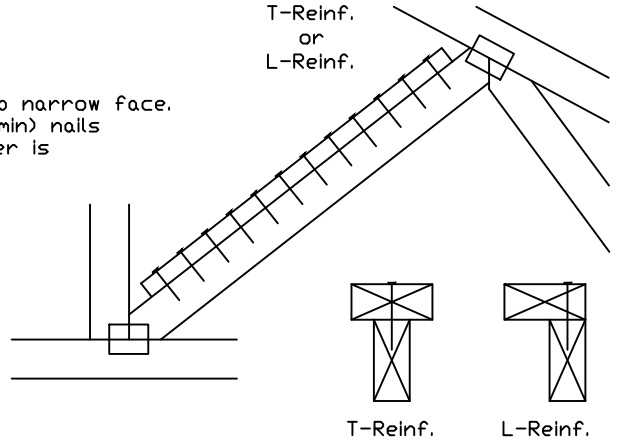
Web Member Size	Specified CLR Restraint	Alternative Reinforecement T- or L- Reinf.	Alternative Reinforecement Scab Reinf.
2x3 or 2x4	1 row	2x4	1-2x4
2x3 or 2x4	2 rows	2x6	2-2x4
2x6	1 row	2x4	1-2x6
2x6	2 rows	2x6	2-2x4(*)
2x8	1 row	2x6	1-2x8
2x8	2 rows	2x6	2-2x6(*)

T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

(*) Center scab on wide face of web. Apply (1) scab to each face of web.

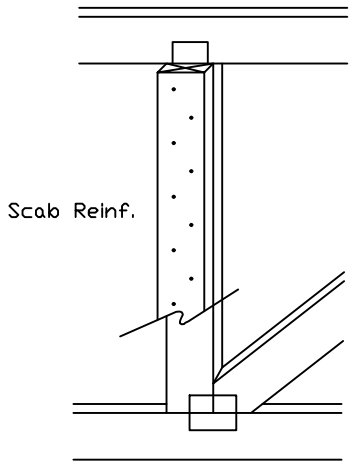
T-Reinforcement or L-Reinforcement:

Apply to either side of web narrow face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



Scab Reinforcement:

Apply scab(s) to wide face of web. No more than (1) scab per face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



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****WARNING** READ AND FOLLOW ALL NOTES ON THIS DRAWING!**
****IMPORTANT** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.**

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Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses.

A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see this job's general notes page and these web sites: ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org

STATE OF MISSOURI
BRADLEY E. MORRIS
NUMBER PE-2002019611
PROFESSIONAL ENGINEER
MISSOURI COA #2005000817
01/12/2026

TC LL	PSF	REF CLR Subst.
TC DL	PSF	DATE 01/02/19
BC DL	PSF	DRWG BRCLBSUB0119
BC LL	PSF	
TOT. LD.	PSF	
DUR. FAC.		
SPACING		

Gable Stud Reinforcement Detail

ASCE 7-16: 140 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Or: 120 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00
Or: 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure D, Kzt = 1.00
Or: 100 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

Max Gable Vertical Length

2x4 Gable Vertical Spacing	Species	Brace Grade	No Braces	(1) 1x4 "L" Brace * Group A	(1) 1x4 "L" Brace * Group B	(1) 2x4 "L" Brace * Group A	(1) 2x4 "L" Brace * Group B	(2) 2x4 "L" Brace ** Group A	(2) 2x4 "L" Brace ** Group B	(1) 2x6 "L" Brace * Group A	(1) 2x6 "L" Brace * Group B	(2) 2x6 "L" Brace ** Group A	(2) 2x6 "L" Brace ** Group B
24" o.c.	SPF HF	#1 / #2	4' 3"	7' 3"	7' 7"	8' 7"	8' 11"	10' 3"	10' 8"	13' 6"	14' 0"	14' 0"	14' 0"
		#3	4' 1"	6' 7"	7' 1"	8' 6"	8' 10"	10' 1"	10' 6"	13' 4"	13' 10"	14' 0"	14' 0"
		Stud	4' 1"	6' 7"	7' 0"	8' 6"	8' 10"	10' 1"	10' 6"	13' 4"	13' 10"	14' 0"	14' 0"
		Standard	4' 1"	5' 8"	6' 0"	7' 7"	8' 1"	10' 1"	10' 6"	11' 10"	12' 8"	14' 0"	14' 0"
	SP DFL	#1	4' 6"	7' 4"	7' 8"	8' 8"	9' 0"	10' 4"	10' 9"	13' 8"	14' 0"	14' 0"	14' 0"
		#2	4' 3"	7' 3"	7' 7"	8' 7"	8' 11"	10' 3"	10' 8"	13' 6"	14' 0"	14' 0"	14' 0"
		#3	4' 2"	6' 0"	6' 4"	7' 11"	8' 6"	10' 2"	10' 7"	12' 5"	13' 4"	14' 0"	14' 0"
		Stud	4' 2"	6' 0"	6' 4"	7' 11"	8' 6"	10' 2"	10' 7"	12' 5"	13' 4"	14' 0"	14' 0"
		Standard	4' 0"	5' 3"	5' 7"	7' 0"	7' 6"	9' 6"	10' 2"	11' 0"	11' 10"	14' 0"	14' 0"
16" o.c.	SPF HF	#1 / #2	4' 11"	8' 4"	8' 8"	9' 10"	10' 3"	11' 8"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 8"	8' 1"	8' 8"	9' 8"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 8"	8' 1"	8' 6"	9' 8"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 8"	6' 11"	7' 5"	9' 3"	9' 11"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	5' 1"	8' 5"	8' 9"	9' 11"	10' 4"	11' 10"	12' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	4' 11"	8' 4"	8' 8"	9' 10"	10' 3"	11' 8"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	4' 9"	7' 4"	7' 9"	9' 9"	10' 2"	11' 8"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	4' 9"	7' 4"	7' 9"	9' 9"	10' 2"	11' 8"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	4' 8"	6' 5"	6' 10"	8' 7"	9' 2"	11' 7"	12' 1"	13' 6"	14' 0"	14' 0"	14' 0"
12" o.c.	SPF HF	#1 / #2	5' 5"	9' 2"	9' 6"	10' 10"	11' 3"	11' 8"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 1"	9' 0"	9' 4"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 1"	9' 0"	9' 4"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 1"	8' 0"	8' 6"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"
	SP DFL	#1	5' 8"	9' 3"	9' 8"	10' 11"	11' 4"	13' 0"	13' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		#2	5' 5"	9' 2"	9' 6"	10' 10"	11' 3"	12' 11"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"
		#3	5' 3"	8' 5"	9' 0"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud	5' 3"	8' 5"	9' 0"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"
		Standard	5' 1"	7' 5"	7' 11"	9' 11"	10' 7"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"

Bracing Group Species and Grades:

Group A:

Spruce-Pine-Fir	
#1 / #2	Standard
#3	Stud

Hem-Fir	
#2	Stud
#3	Standard

Douglas Fir-Larch
#3
Stud
Standard

Southern Pine***
#3
Stud
Standard

Group B:

Hem-Fir
#1 & Btr
#1

Douglas Fir-Larch
#1
#2

Southern Pine***
#1
#2

1x4 Braces shall be SRB (Stress-Rated Board).

***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes:

Wind Load deflection criterion is L/240.

Provide uplift connections for 55 plf over continuous bearing (5 psf TC Dead Load).

Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12" plywood overhang.

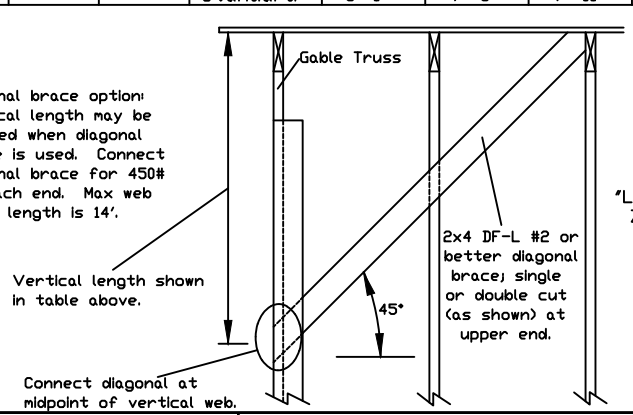
Attach "L" braces with 10d (0.128"x3.0" min) nails.

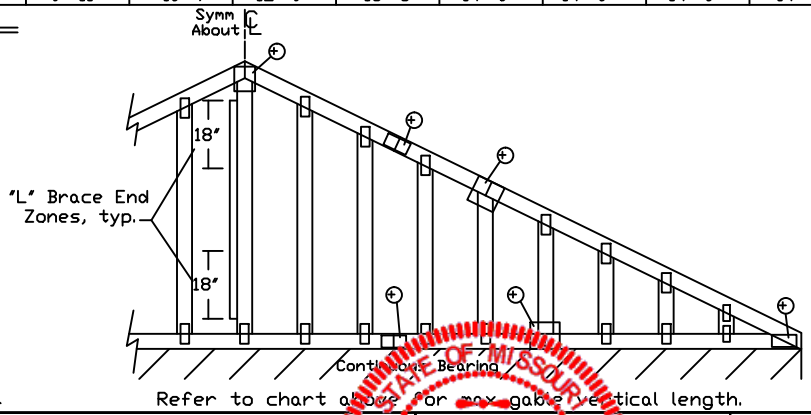
* For (1) "L" brace: space nails at 2" o.c. in 18" end zones and 4" o.c. between zones.

** For (2) "L" braces: space nails at 3" o.c. in 18" end zones and 6" o.c. between zones.

"L" bracing must be a minimum of 80% of web member length.

Diagonal brace option: vertical length may be doubled when diagonal brace is used. Connect diagonal brace for 450# at each end. Max web total length is 14'.





Refer to chart above for max gable vertical length.

Gable Vertical Plate Sizes

Vertical Length	No Splice
Less than 4' 0"	1X4 or 2X3
Greater than 4' 0"	3X4

+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.

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STATE OF MISSOURI
BRADLEY E. MORRIS
NUMBER PE-2002019611
PROFESSIONAL ENGINEER
01/12/2026
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MAX. TOT. LD. 60 PSF
MAX. SPACING 24.0"

REF ASCE7-16-GAB14015
DATE 01/26/2018
DRWG A14015ENC160118