MiTek®

RE: P241184-01 - Roof

MiTek, Inc.
16023 Swingley Ridge Rd.
Chesterfield, MO 63017
314.434.1200

Site Information:

Project Customer: Clayton Properties Project Name: Baily Farms Clubhouse
Lot/Block: Subdivision: Bailey Farms
Model:
Address:
City: Lee's Summit State: MO

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2018/TPI2014
Wind Code: ASCE 7-16
Wind Speed: 117 mph
Roof Load: 40.0 psf
Mean Roof Height (feet): 35

Design Program: MiTek 20/20 8.6
Design Method: MWFRS (Envelope)/C-C hybrid Wind ASCE 7-16
Floor Load: N/A psf

Exposure Category: B

No.	Seal#	Truss Name	Date
1	I77196678	A1	10/22/25
2	I77196679	A2	10/22/25
3	I77196680	A3	10/22/25
4	I77196681	B1	10/22/25
5	I77196682	B2	10/22/25
6	I77196683	B3	10/22/25
7	I77196684	B4	10/22/25
8	I77196685	B5	10/22/25
9	I77196686	B7	10/22/25
10	I77196687	C1	10/22/25
11	I77196688	C2	10/22/25
12	I77196689	C3	10/22/25
13	I77196690	PB1	10/22/25
14	I77196691	PB2	10/22/25
15	I77196692	PB3	10/22/25
16	I77196693	PB4	10/22/25

The truss drawing(s) referenced above have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Premier Building Supply (Springhill, KS)20300 W 207th Street.

Truss Design Engineer's Name: Lu, Jie
My license renewal date for the state of Missouri is December 31, 2026.

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

October 22,2025

Lu, Jie

Job	Truss	Truss Type	Qty	Ply	Roof
P241184-01	A1	Piggyback Base Girder	1	3	I77196678 Job Reference (optional)

Premier Building Supply (Springhill, KS), Spring Hills, KS - 66083, Run: 8.63 E Aug 30 2023 Print: 8.630 E Aug 30 2023 MiTek Industries, Inc. Wed Oct 22 16:07:45 Page: 1
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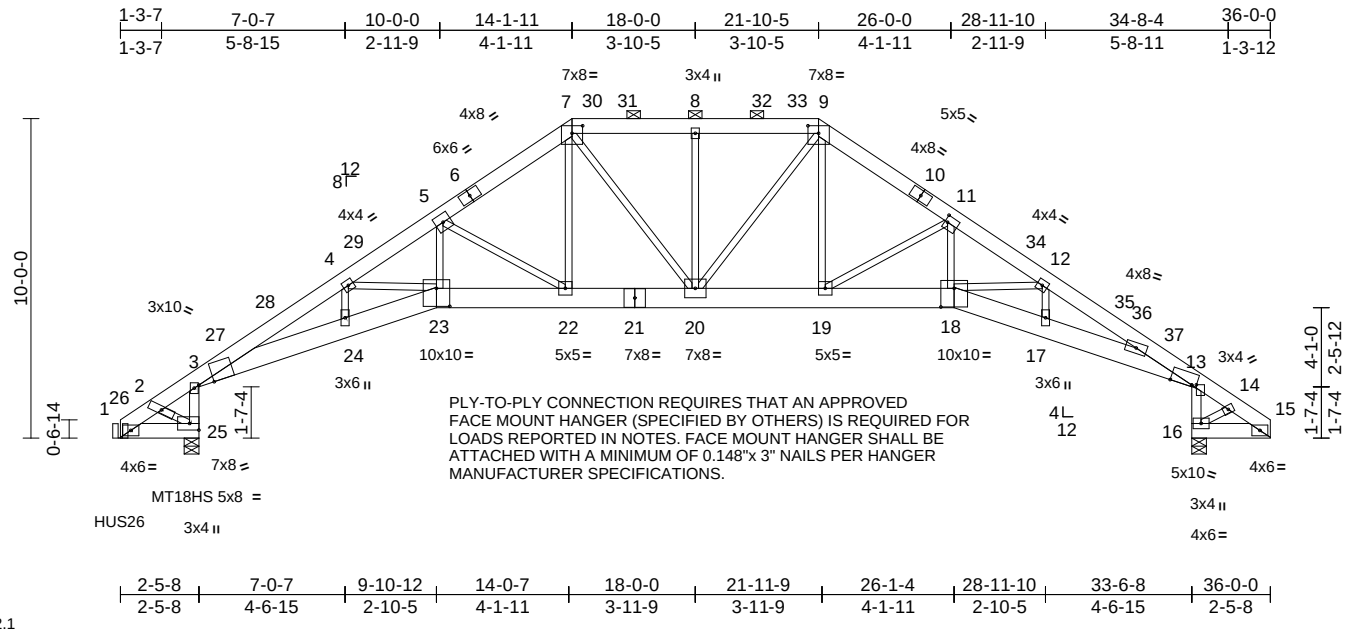

Scale = 1:72.1

Plate Offsets (X, Y): [3:0-5-14,Edge], [7:0-4-0,0-2-13], [9:0-4-0,0-2-13], [11:0-1-0,0-2-8], [13:0-8-6,Edge], [13:0-0-1,0-1-12], [18:0-5-0,0-7-0], [23:0-5-0,0-6-12]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.67	Vert(LL)	-0.27	20	>999	240	MT20	244/190
Snow (Pf/Pg)	21.9/20.0	Lumber DOL	1.15	BC	0.47	Vert(CT)	-0.50	19-20	>753	180	MT18HS	197/144
TCDL	10.0	Rep Stress Incr	NO	WB	0.68	Horz(CT)	0.55	16	n/a	n/a		
BCLL	0.0	Code	IRC2018/TPI2014	Matrix-S								
BCDL	10.0										Weight: 760 lb	FT = 20%

LUMBER
TOP CHORD 2x6 SP No.2 *Except* 1-6,10-15:2x6 SP 2400F 2.0E
BOT CHORD 2x8 SP 2400F 2.0E *Except* 1-25,16-15:2x6 SP No.2, 25-3,13-16:2x4 SP No.2
WEBS 2x3 SPF No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except 2-0-0 oc purlins (6-0-0 max.): 7-9.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 6-0-0 oc bracing: 1-25,3-25,13-16.

REACTIONS (lb/size) 16=6526/0-5-8, 25=8960/0-5-8
Max Horiz 25=329 (LC 62)
Max Uplift 16=-648 (LC 64), 25=-700 (LC 16)
Max Grav 16=6892 (LC 40), 25=9326 (LC 40)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-26=-89/2841, 2-26=-75/2915, 2-3=-166/1915, 3-27=-19283/3168, 27-28=-19187/3186, 4-28=-18867/3279, 4-29=-20549/2586, 5-29=-20468/2656, 5-6=-14998/1673, 6-7=-14882/1758, 7-30=-14687/1661, 30-31=-14685/1661, 8-31=-14684/1662, 8-32=-14684/1662, 32-33=-14685/1662, 9-33=-14686/1661, 9-10=-15106/1591, 10-11=-15211/1506, 11-34=-21432/1966, 12-34=-21489/1908, 12-35=-21116/1874, 35-36=-21213/1833, 36-37=-21279/1819, 13-37=-21369/1808
BOT CHORD 1-25=-2244/77, 3-25=-10706/665, 22-23=-1985/16937, 21-22=-1154/12269, 20-21=-1154/12269, 19-20=-960/12449, 18-19=-1361/17648, 13-16=-6693/651, 15-16=-31/280, 17-18=-1504/18637, 13-17=-1487/18418, 3-24=-3051/16342, 23-24=-2960/16713
WEBS 5-23=-996/5201, 5-22=-5587/1056, 7-22=-594/3759, 9-19=-365/4045, 9-20=-575/3998, 11-18=-451/5906, 11-19=-6057/650, 8-20=-5256/583, 7-20=-504/4305, 4-23=-151/1919, 12-18=-106/485, 4-24=-1339/122, 12-17=-666/122, 14-16=-316/36, 2-25=-195/2677

NOTES
1) N/A
2) 3-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2x6 - 3 rows staggered at 0-4-0 oc.
Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-4-0 oc, 2x4 - 1 row at 0-9-0 oc, 2x8 - 3 rows staggered at 0-5-0 oc.
Web connected as follows: 2x3 - 1 row at 0-9-0 oc.
3) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
4) Unbalanced roof live loads have been considered for this design.
5) Wind: ASCE 7-16; Vult=117mph (3-second gust) Vasd=92mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. III; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-0-0 to 5-0-0, Interior (1) 5-0-0 to 14-1-11, Exterior(2R) 14-1-11 to 21-2-9, Interior (1) 21-2-9 to 21-10-5, Exterior(2R) 21-10-5 to 28-11-14, Interior (1) 28-11-14 to 36-0-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
6) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=21.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.1; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0
7) Unbalanced snow loads have been considered for this design.
8) Provide adequate drainage to prevent water ponding.
9) All plates are MT20 plates unless otherwise indicated.
10) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

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11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 648 lb uplift at joint 16 and 700 lb uplift at joint 25.
12) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
13) Girder carries tie-in span(s): 8-4-8 from 0-5-8 to 31-6-8
14) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
15) Use Simpson Strong-Tie HUS26 (14-10d Girder, 4-10d Truss) or equivalent at 0-0-0 from the left end to connect truss(es) to front face of bottom chord.
16) Fill all nail holes where hanger is in contact with lumber.
17) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 5030 lb down and 512 lb up at 18-0-0 on top chord, and 2213 lb down and 225 lb up at 18-0-9 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
   Uniform Loads (lb/ft)
   Vert: 1-26=-54, 3-26=-178 (F=-124), 3-7=-178 (F=-124), 7-9=-188 (F=-124), 9-36=-178 (F=-124), 13-36=-54, 13-15=-54, 1-25=-20, 18-23=-20, 15-16=-20, 13-18=-20, 3-23=-20
   Concentrated Loads (lb)
   Vert: 1=-1693 (F), 8=-5000 (F), 20=-2200 (F)

Job	Truss	Truss Type	Qty	Ply	Roof
P241184-01	A2	Piggyback Base	8	1	I77196679
					Job Reference (optional)

Premier Building Supply (Springhill, KS), Spring Hills, KS - 66083,

Run: 8.63 S Sep 26 2024 Print: 8.630 S Sep 26 2024 MiTek Industries, Inc. Mon Oct 20 16:02:22

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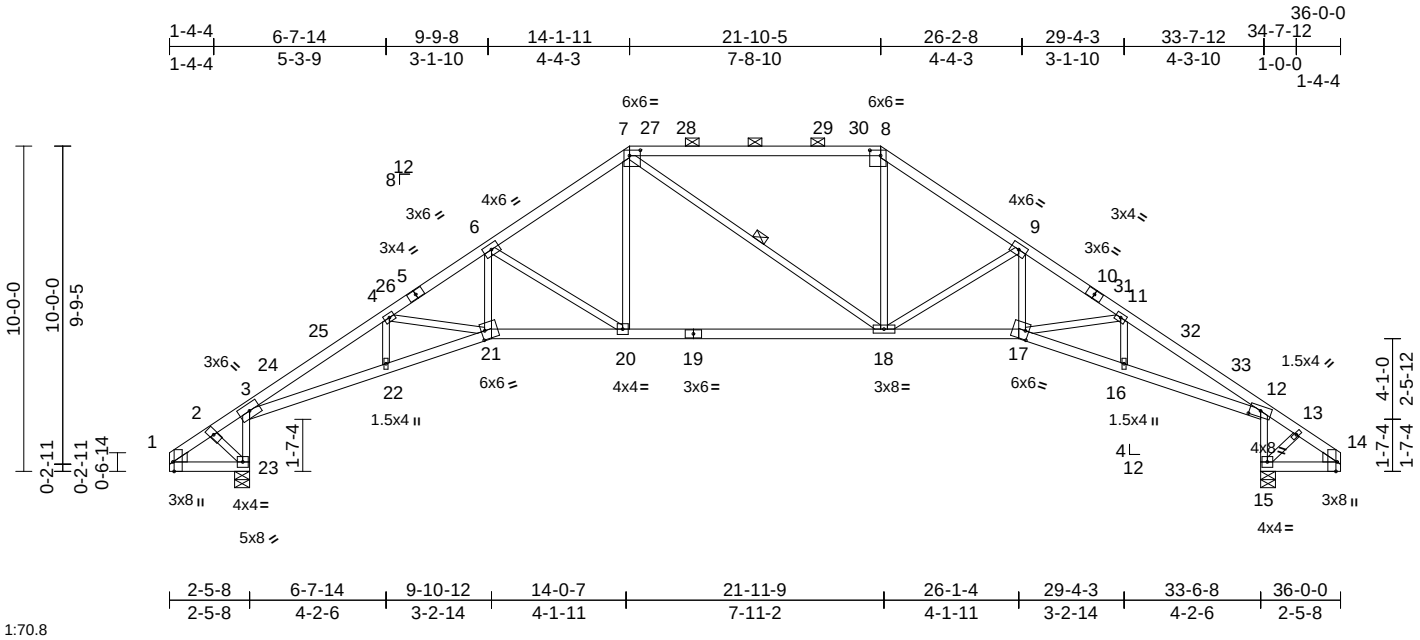

Scale = 1:70.8

Plate Offsets (X, Y): [1:0-3-8,Edge], [7:0-4-0,0-2-0], [8:0-4-0,0-2-0], [12:0-4-0,0-2-3], [14:0-3-8,Edge], [17:0-1-4,0-3-4], [21:0-1-4,0-3-4]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.92	Vert(LL)	-0.29	20-21	>999	240	MT20	244/190
Snow (Pf/Pg)	21.9/20.0	Lumber DOL	1.15	BC	0.93	Vert(CT)	-0.56	18-20	>666	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.92	Horz(CT)	0.67	15	n/a	n/a		
BCLL	0.0	Code	IRC2018/TPI2014	Matrix-S								
BCDL	10.0										Weight: 159 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2 *Except* 7-8:2x4 SP 2400F 2.0E
BOT CHORD	2x4 SP No.2 *Except* 23-3,21-6,9-17:2x3 SPF No.2
WEBS	2x3 SPF No.2
WEDGE	Left: 2x4 SP No.2 Right: 2x4 SP No.2

BRACING

TOP CHORD	Structural wood sheathing directly applied or 2-6-4 oc purlins, except 2-0-0 oc purlins (2-2-0 max.): 7-8.
BOT CHORD	Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS	1 Row at midpt 7-18

REACTIONS (size) 15=0-5-8, 23=0-5-8
Max Horiz 23=-202 (LC 12)
Max Uplift 15=-49 (LC 17), 23=-49 (LC 16)
Max Grav 15=1713 (LC 40), 23=1713 (LC 40)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=-91/135, 2-3=-45/132, 3-4=-4033/228, 4-6=-3767/145, 6-7=-2382/80, 7-8=-1863/77, 8-9=-2384/51, 9-11=-3767/8, 11-12=-4033/0, 12-13=-52/135, 13-14=-86/120
BOT CHORD	1-23=-108/93, 3-23=-1754/206, 6-21=-77/1183, 20-21=-110/3122, 18-20=-37/1861, 17-18=0/3121, 9-17=0/1181, 3-22=-290/3388, 21-22=-280/3449, 16-17=0/3449, 12-16=0/3388, 14-15=-79/83
WEBS	6-20=-1473/182, 7-20=-13/932, 7-18=-171/175, 8-18=0/933, 9-18=-1470/63, 12-15=-1756/166, 2-23=-367/421, 13-15=-117/111, 11-17=-220/85, 11-16=-139/59, 4-22=-139/61, 4-21=-314/171

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=117mph (3-second gust) Vasd=92mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. III; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-0-0 to 5-0-0, Interior (1) 5-0-0 to 14-1-11, Exterior(2R) 14-1-11 to 21-2-9, Interior (1) 21-2-9 to 21-10-5, Exterior(2R) 21-10-5 to 28-11-3, Interior (1) 28-11-3 to 36-0-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=21.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.1; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0
4) Unbalanced snow loads have been considered for this design.
5) Provide adequate drainage to prevent water ponding.
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 49 lb uplift at joint 23 and 49 lb uplift at joint 15.
9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

October 22,2025

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Job	Truss	Truss Type	Qty	Ply	Roof
P241184-01	A3	Piggyback Base Supported Gable	1	1	I77196680
					Job Reference (optional)

Premier Building Supply (Springhill, KS), Spring Hills, KS - 66083, Run: 8.63 S Sep 26 2024 Print: 8.630 S Sep 26 2024 MiTek Industries, Inc. Mon Oct 20 16:02:22 Page: 1
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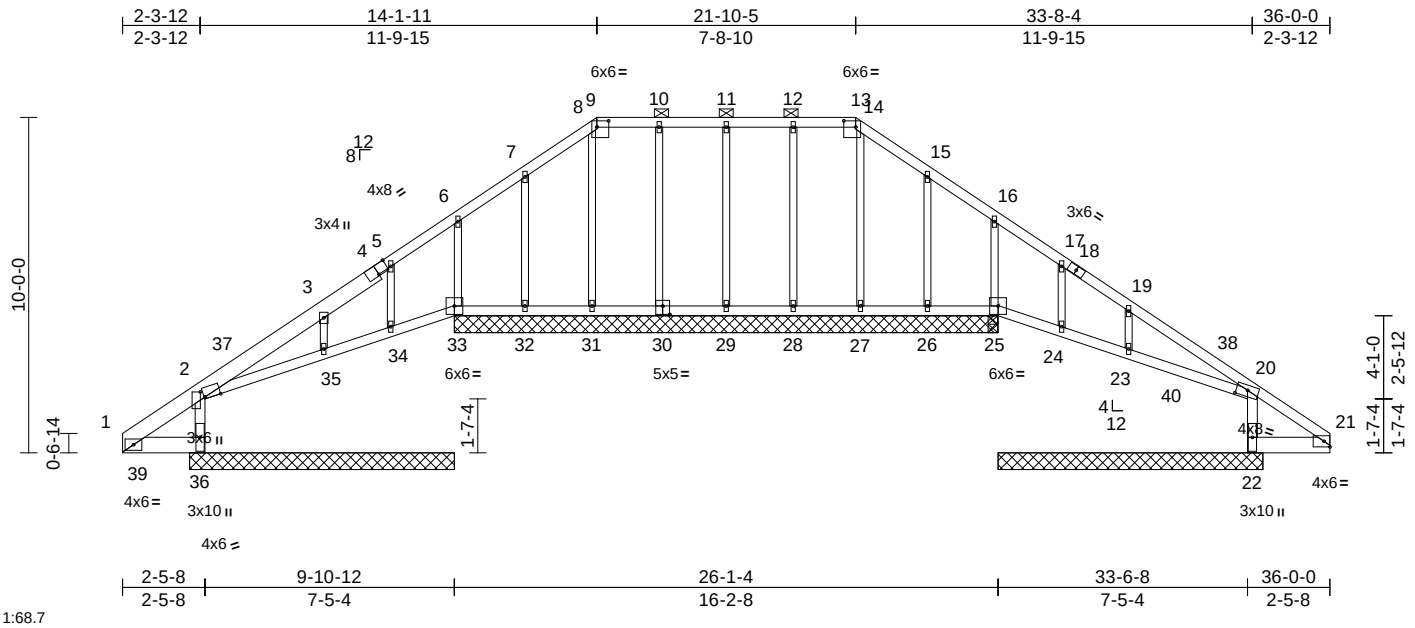

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Plate Offsets (X, Y): [2:0-5-12,Edge], [2:0-1-13,0-1-10], [4:0-3-14,Edge], [9:0-4-4,0-2-4], [13:0-4-4,0-2-4], [20:0-4-0,0-2-3], [30:0-2-8,0-3-0]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.81	Vert(LL)	-0.03	20-23	>999	240	MT20	244/190
Snow (Pf/Pg)	21.9/20.0	Lumber DOL	1.15	BC	0.91	Vert(CT)	-0.04	2-35	>999	180		
TCDL	10.0	Rep Stress Incr	NO	WB	0.33	Horz(CT)	-0.30	22	n/a	n/a		
BCLL	0.0	Code	IRC2018/TPI2014	Matrix-S								
BCDL	10.0										Weight: 171 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2 *Except* 1-4:2x6 SP 2400F 2.0E, 18-21:2x4 SP 2400F 2.0E
BOT CHORD 2x4 SP No.2 *Except* 1-36,22-21:2x6 SP No.2
WEBS 2x4 SP No.2
OTHERS 2x3 SPF No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-3-8 oc purlins, except 2-0-0 oc purlins (6-0-0 max.): 9-13.
BOT CHORD Rigid ceiling directly applied or 3-5-4 oc bracing.

REACTIONS (size) 22=7-10-12, 23=7-10-12, 24=7-10-12, 25=7-10-12, 26=16-2-8, 27=16-2-8, 28=16-2-8, 29=16-2-8, 30=16-2-8, 31=16-2-8, 32=16-2-8, 33=16-2-8, 34=7-10-12, 35=7-10-12, 36=7-10-12
Max Horiz 36=237 (LC 13)
Max Uplift 22=-1346 (LC 56), 23=-504 (LC 55), 24=-209 (LC 56), 25=-350 (LC 56), 26=-89 (LC 56), 27=-287 (LC 47), 28=-79 (LC 47), 29=-58 (LC 45), 30=-95 (LC 48), 31=-377 (LC 48), 32=-83 (LC 53), 33=-439 (LC 53), 34=-73 (LC 55), 35=-493 (LC 48), 36=-1403 (LC 45)
Max Grav 22=1495 (LC 51), 23=879 (LC 72), 24=516 (LC 71), 25=661 (LC 71), 26=563 (LC 92), 27=655 (LC 76), 28=525 (LC 81), 29=495 (LC 81), 30=521 (LC 81), 31=729 (LC 75), 32=555 (LC 90), 33=749 (LC 70), 34=443 (LC 92), 35=940 (LC 69), 36=1977 (LC 52)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-430/482, 2-3=-3008/2952, 3-5=-2180/2195, 5-6=-1850/1869, 6-7=-1505/1571, 7-8=-1200/1258, 8-9=-604/648, 9-10=-700/775, 10-11=-414/517, 11-12=-313/416, 12-13=-599/675, 13-14=-521/566, 14-15=-1080/1141, 15-16=-1380/1419, 16-17=-1683/1705, 17-19=-1950/1954, 19-20=-2722/2675, 20-21=-261/247
BOT CHORD 1-36=-516/558, 32-33=-1294/1391, 31-32=-1002/1071, 29-31=-724/821, 28-29=-348/443, 27-28=-626/721, 26-27=-903/971, 25-26=-1196/1291, 2-35=-2415/2416, 34-35=-1947/1970, 33-34=-1655/1685, 24-25=-1553/1601, 23-24=-1840/1873, 20-23=-2259/2318, 21-22=-321/321
WEBS 2-36=-1901/1448, 20-22=-1594/1519, 11-29=-196/54, 10-30=-204/91, 8-31=-454/391, 7-32=-207/79, 6-33=-203/87, 5-34=-201/77, 3-35=-348/357, 12-28=-204/75, 14-27=-375/301, 15-26=-206/83, 16-25=-206/77, 17-24=-173/155, 19-23=-320/298

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=117mph (3-second gust) Vasd=92mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. III; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Corner(3E) 0-0-0 to 5-0-0, Exterior(2N) 5-0-0 to 14-1-11, Corner(3R) 14-1-11 to 19-1-11, Exterior(2N) 19-1-11 to 21-10-5, Corner(3R) 21-10-5 to 26-10-5, Exterior(2N) 26-10-5 to 36-0-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=21.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.1; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0
5) Unbalanced snow loads have been considered for this design.
6) Provide adequate drainage to prevent water ponding.

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7) All plates are 1.5x4 MT20 unless otherwise indicated.
8) Gable studs spaced at 2-0-0 oc.
9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
10) Bearings are assumed to be: Joint 35 SP No.2 crushing capacity of 565 psi, Joint 23 SP No.2 crushing capacity of 565 psi, Joint 26 SP No.2 crushing capacity of 565 psi, Joint 25 SP No.2 crushing capacity of 565 psi.
11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1403 lb uplift at joint 36, 439 lb uplift at joint 33, 95 lb uplift at joint 30, 350 lb uplift at joint 25, 1346 lb uplift at joint 22, 58 lb uplift at joint 29, 377 lb uplift at joint 31, 83 lb uplift at joint 32, 73 lb uplift at joint 34, 493 lb uplift at joint 35, 79 lb uplift at joint 28, 287 lb uplift at joint 27, 89 lb uplift at joint 26, 209 lb uplift at joint 24 and 504 lb uplift at joint 23.
12) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
13) This truss has been designed for a total drag load of 5000 lb. Lumber DOL=(1.33) Plate grip DOL=(1.33) Connect truss to resist drag loads along bottom chord from 0-0-0 to 36-0-0 for 138.9 plf.
14) Girder carries tie-in span(s): 9-0-0 from 0-5-8 to 31-6-8
15) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
16) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
   Uniform Loads (lb/ft)
   Vert: 1-9=-54, 9-13=-64, 13-21=-54, 1-39=-20, 36-39=-157 (F=-137), 25-33=-157 (F=-137), 2-33=-157 (F=-137), 25-40=-157 (F=-137), 20-40=-20, 21-22=-20

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Job	Truss	Truss Type	Qty	Ply	Roof
P241184-01	B1	Piggyback Base Girder	1	2	I77196681
					Job Reference (optional)

Premier Building Supply (Springhill, KS), Spring Hills, KS - 66083, Run: 8.63 E May 25 2023 Print: 8.630 E May 25 2023 MiTek Industries, Inc. Wed Oct 22 08:54:11 Page: 1
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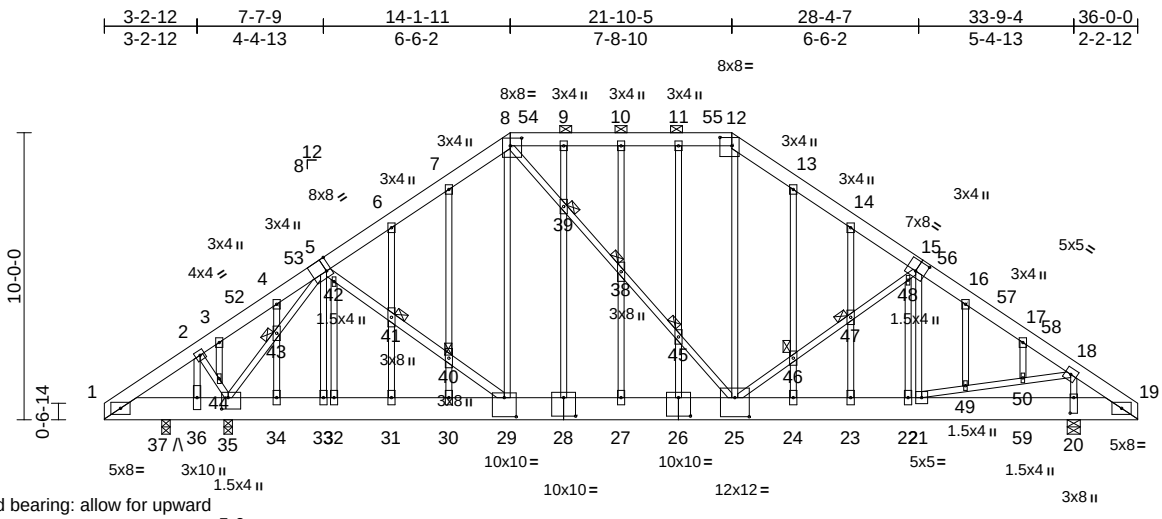

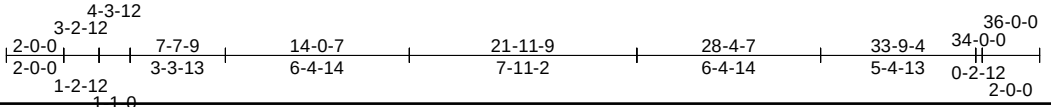

Scale = 1:80.3

Plate Offsets (X, Y): [5:0-2-0,Edge], [8:0-4-12,0-3-4], [12:0-5-0,0-3-8], [15:0-4-0,0-4-8], [20:0-6-8,0-1-8], [25:0-6-0,0-8-0], [26:0-5-0,0-7-12], [28:0-5-0,0-7-12], [29:0-5-0,0-8-0], [35:0-2-12,0-4-12]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.71	Vert(LL)	-0.17	27	>999	240	MT20	197/144
Snow (Pf/Pg)	21.9/20.0	Lumber DOL	1.15	BC	0.76	Vert(CT)	-0.32	27	>999	180		
TCDL	10.0	Rep Stress Incr	NO	WB	0.99	Horz(CT)	0.03	20	n/a	n/a		
BCLL	0.0	Code	IRC2018/TPI2014	Matrix-S								
BCDL	10.0										Weight: 714 lb	FT = 20%

LUMBER

TOP CHORD 2x6 SP No.2 *Except* 8-12:2x6 SP 2400F 2.0E
BOT CHORD 2x10 SP 2400F 2.0E
WEBS 2x3 SPF No.2 *Except* 5-29:2x4 SP No.2
OTHERS 2x3 SPF No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-3-14 oc purlins, except 2-0-0 oc purlins (6-0-0 max.): 8-12.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 6-0-0 oc bracing: 20-21,19-20.
JOINTS 1 Brace at Jt(s): 38, 39, 40, 41, 43, 45, 46, 47

REACTIONS (lb/size) 20=6817/0-5-8, 35=7855/0-3-8, 37=0/0-3-8
Max Horiz 37=-195 (LC 50)
Max Uplift 20=-2489 (LC 56), 35=-2704 (LC 53), 37=REL
Max Grav 20=8214 (LC 71), 35=8399 (LC 70), 37=1820 (LC 48)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-1179/227, 2-3=-1371/54, 3-52=-1078/69, 4-52=-1048/117, 4-53=-1414/182, 5-53=-1487/214, 5-6=-8922/1998, 6-7=-8835/1748, 7-8=-8334/1515, 8-54=-7597/1592, 9-54=-7526/1444, 9-10=-7363/1324, 10-11=-7269/1060, 11-55=-7394/1264, 12-55=-7466/1358, 12-13=-9057/1861, 13-14=-9600/2112, 14-15=-9901/2442, 15-56=-7398/1761, 16-56=-7718/1980, 16-57=-8337/2202, 17-57=-8680/2438, 17-58=-8680/2492, 18-58=-8946/2675, 18-19=-3488/1064
BOT CHORD 1-37=-171/1061, 36-37=-110/830, 35-36=-121/736, 34-35=-888/4398, 33-34=-613/4193, 32-33=-348/3994, 31-32=-288/3949, 30-31=-263/4167, 29-30=-594/4415, 28-29=-607/6818, 27-28=-932/7043, 26-27=-1225/7263, 25-26=-1576/7526, 24-25=-985/6280, 23-24=-1343/6549, 22-23=-1668/6792, 21-22=-1728/6837, 21-59=-912/2991, 20-59=-912/2991, 19-20=-941/3080

October 22,2025

Continued on page 2

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WEBS 8-29=-580/2667, 8-39=-941/1280, 38-39=-897/1233, 38-45=-889/1166, 25-45=-953/1289, 12-25=-575/3130, 2-44=-311/1077, 35-44=-373/1247, 2-36=-2060/559, 35-43=-6738/2129, 5-43=-6473/2047, 5-42=-1383/4311, 41-42=-1346/4203, 40-41=-1297/4018, 29-40=-1414/4425, 5-33=-1755/545, 25-46=-710/2013, 46-47=-650/1825, 47-48=-679/1925, 15-48=-675/1870, 15-21=-3548/1075, 21-49=-1521/4641, 49-50=-1424/4350, 18-50=-1444/4410, 18-20=-5381/1748, 10-38=-3286/389, 27-38=-3194/380, 9-39=-696/203, 28-39=-674/144, 7-40=-82/244, 30-40=-541/280, 6-41=-837/207, 31-41=-557/151, 32-42=-75/218, 4-43=-950/288, 34-43=-630/185, 3-44=-70/282, 11-45=-510/105, 26-45=-623/180, 13-46=-72/273, 24-46=-119/90, 14-47=-409/113, 23-47=-278/142, 22-48=-106/56, 16-49=-351/1055, 17-50=-290/98

NOTES

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
   Top chords connected as follows: 2x6 - 2 rows staggered at 0-2-0 oc.
   Bottom chords connected as follows: 2x10 - 4 rows staggered at 0-9-0 oc.
   Web connected as follows: 2x3 - 1 row at 0-6-0 oc, 2x4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-16; Vult=117mph (3-second gust) Vasd=92mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. III; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-0-0 to 5-0-0, Interior (1) 5-0-0 to 14-1-11, Exterior(2R) 14-1-11 to 21-2-9, Interior (1) 21-2-9 to 21-10-5, Exterior(2R) 21-10-5 to 28-11-3, Interior (1) 28-11-3 to 36-0-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
5) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
6) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=21.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.1; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
7) Unbalanced snow loads have been considered for this design.
8) Provide adequate drainage to prevent water ponding.
9) All plates are 3x6 MT20 unless otherwise indicated.
10) Gable studs spaced at 2-0-0 oc.
11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
12) All bearings are assumed to be SP 2400F 2.0E crushing capacity of 805 psi.
13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2489 lb uplift at joint 20 and 2704 lb uplift at joint 35.
14) "/\" indicates Released bearing: allow for upward movement at joint(s) 37.
15) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
16) This truss has been designed for a total drag load of 5200 lb. Lumber DOL=(1.33) Plate grip DOL=(1.33) Connect truss to resist drag loads along bottom chord from 0-0-0 to 32-0-0 for 162.5 plf.
17) Girder carries tie-in span(s): 8-4-0 from 0-0-0 to 36-0-0
18) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
19) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 5030 lb down and 512 lb up at 18-0-0 on top chord, and 2515 lb down and 256 lb up at 18-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
   Uniform Loads (lb/ft)
   Vert: 1-8=-177 (F=-123), 8-12=-187 (F=-123), 12-19=-177 (F=-123), 1-19=-20
   Concentrated Loads (lb)
   Vert: 10=-5000 (F), 27=-2500 (F)

Job	Truss	Truss Type	Qty	Ply	Roof
P241184-01	B2	Piggyback Base	3	1	I77196682
					Job Reference (optional)

Premier Building Supply (Springhill, KS), Spring Hills, KS - 66083, Run: 8.63 S Sep 26 2024 Print: 8.630 S Sep 26 2024 MiTek Industries, Inc. Mon Oct 20 16:02:24 Page: 1
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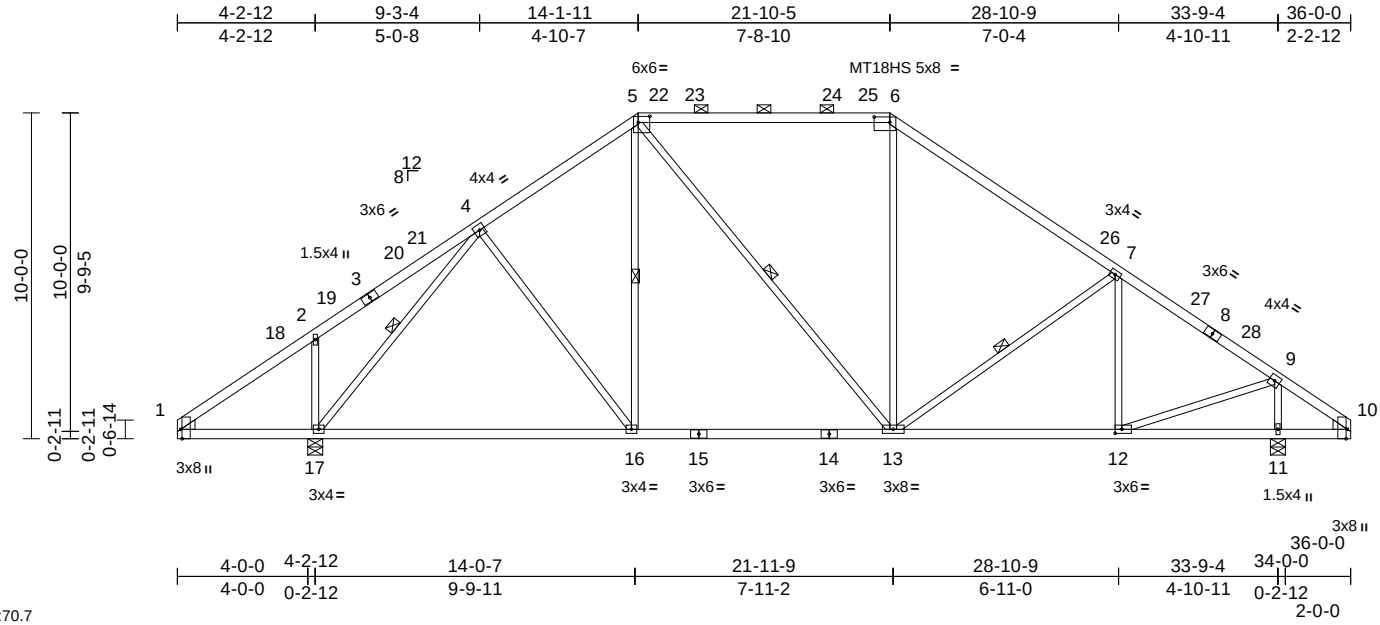

Scale = 1:70.7

Plate Offsets (X, Y): [1:0-3-8,Edge], [5:0-4-4,0-2-4], [6:0-5-12,0-2-0], [10:0-3-8,Edge], [12:0-2-8,0-1-8]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.80	Vert(LL)	-0.18	16-17	>999	240	MT20	244/190
Snow (Pf/Pg)	21.9/20.0	Lumber DOL	1.15	BC	0.73	Vert(CT)	-0.36	16-17	>988	180	MT18HS	197/144
TCDL	10.0	Rep Stress Incr	YES	WB	0.97	Horz(CT)	0.04	11	n/a	n/a		
BCLL	0.0	Code	IRC2018/TPI2014	Matrix-S								
BCDL	10.0										Weight: 173 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2 *Except* 5-6,6-8:2x4 SP 2400F 2.0E
BOT CHORD 2x4 SP No.2
WEBS 2x3 SPF No.2
WEDGE Left: 2x4 SP No.2
Right: 2x4 SP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-8-0 oc purlins, except
2-0-0 oc purlins (5-2-11 max.): 5-6.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 5-16, 5-13, 7-13, 4-17

REACTIONS (size) 11=0-5-8, 17=0-5-8
Max Horiz 17=-202 (LC 12)
Max Uplift 11=-52 (LC 17), 17=-52 (LC 16)
Max Grav 11=1597 (LC 40), 17=1829 (LC 40)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-174/262, 2-4=-73/276, 4-5=-1196/109, 5-6=-966/137, 6-7=-1374/105, 7-9=-1532/72, 9-10=-75/116
BOT CHORD 1-17=-138/163, 16-17=-91/842, 13-16=-32/869, 12-13=0/1216, 11-12=-56/69, 10-11=-56/69
WEBS 5-16=-52/213, 5-13=-92/220, 6-13=0/282, 2-17=-430/179, 7-13=-346/160, 7-12=-333/87, 9-12=-28/1322, 9-11=-1496/96, 4-16=-51/295, 4-17=-1547/104

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=117mph (3-second gust) Vasd=92mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. III; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-0-0 to 5-0-0, Interior (1) 5-0-0 to 14-1-11, Exterior(2R) 14-1-11 to 21-2-9, Interior (1) 21-2-9 to 21-10-5, Exterior(2R) 21-10-5 to 28-10-9, Interior (1) 28-10-9 to 36-0-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=21.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.1; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0
4) Unbalanced snow loads have been considered for this design.
5) Provide adequate drainage to prevent water ponding.
6) All plates are MT20 plates unless otherwise indicated.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 52 lb uplift at joint 17 and 52 lb uplift at joint 11.
10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

October 22,2025

MiTek®
16023 Swingley Ridge Rd.
Chesterfield, MO 63017
314.434.1200 / MiTek-US.com

Job	Truss	Truss Type	Qty	Ply	Roof
P241184-01	B3	Piggyback Base	1	1	I77196683
					Job Reference (optional)

Premier Building Supply (Springhill, KS), Spring Hills, KS - 66083, Run: 8.63 S Sep 26 2024 Print: 8.630 S Sep 26 2024 MiTek Industries, Inc. Mon Oct 20 16:02:24 Page: 1

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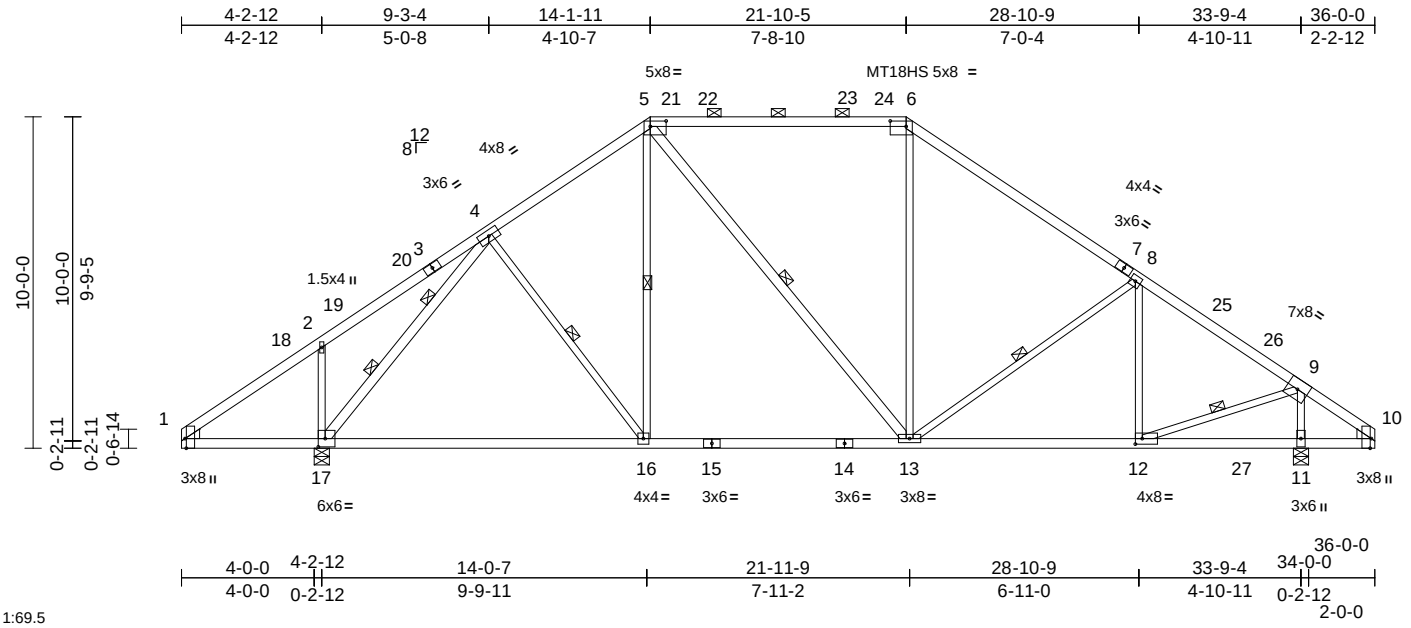

Scale = 1:69.5

Plate Offsets (X, Y): [1:0-3-8,Edge], [5:0-5-12,0-2-0], [6:0-5-12,0-2-0], [10:0-3-8,Edge], [12:0-2-8,0-2-0], [17:0-2-8,0-3-0]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.87	Vert(LL)	-0.18	16-17	>999	240	MT20	244/190
Snow (Pf/Pg)	21.9/20.0	Lumber DOL	1.15	BC	0.81	Vert(CT)	-0.36	16-17	>993	180	MT18HS	197/144
TCDL	10.0	Rep Stress Incr	NO	WB	0.82	Horz(CT)	0.04	11	n/a	n/a		
BCLL	0.0	Code	IRC2018/TPI2014	Matrix-S								
BCDL	10.0										Weight: 187 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP 2400F 2.0E
BOT CHORD	2x4 SP No.2
WEBS	2x3 SPF No.2 *Except* 13-5,17-4:2x4 SP No.2
WEDGE	Left: 2x4 SP No.2 Right: 2x4 SP No.2

BRACING

TOP CHORD	Structural wood sheathing directly applied or 4-10-13 oc purlins, except 2-0-0 oc purlins (5-5-7 max.): 5-6.	
BOT CHORD	Rigid ceiling directly applied or 4-9-15 oc bracing.	
WEBS	1 Row at midpt	5-16, 5-13, 8-13, 9-12, 4-16
WEBS	2 Rows at 1/3 pts	4-17

REACTIONS (size) 11=0-5-8, 17=0-5-8
Max Horiz 17=-202 (LC 50)
Max Uplift 11=-2150 (LC 56), 17=-2150 (LC 53)
Max Grav 11=2769 (LC 65), 17=2866 (LC 66)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=-842/841, 2-4=-1594/1708, 4-5=-1616/993, 5-6=-1255/764, 6-8=-2567/1864, 8-9=-2835/2074, 9-10=-414/400
BOT CHORD	1-17=-743/743, 16-17=-1017/1363, 13-16=-1492/1917, 12-13=-1450/2112, 11-12=-483/526, 10-11=-75/118
WEBS	5-16=-684/865, 5-13=-986/1044, 6-13=-305/518, 2-17=-431/238, 8-13=-841/803, 8-12=-820/788, 9-12=-2075/2754, 9-11=-2847/2158, 4-16=-873/1020, 4-17=-3363/2686

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=117mph (3-second gust) Vasd=92mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. III; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-0-0 to 5-0-0, Interior (1) 5-0-0 to 14-1-11, Exterior(2R) 14-1-11 to 21-2-9, Interior (1) 21-2-9 to 21-10-5, Exterior(2R) 21-10-5 to 28-10-9, Interior (1) 28-10-9 to 36-0-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=21.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.1; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0
4) Unbalanced snow loads have been considered for this design.
5) Provide adequate drainage to prevent water ponding.
6) All plates are MT20 plates unless otherwise indicated.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2150 lb uplift at joint 17 and 2150 lb uplift at joint 11.
10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
11) This truss has been designed for a total drag load of 5200 lb. Lumber DOL=(1.33) Plate grip DOL=(1.33) Connect truss to resist drag loads along bottom chord from 0-0-0 to 32-0-0 for 162.5 plf.
12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

October 22,2025

MiTek®
16023 Swingley Ridge Rd.
Chesterfield, MO 63017
314.434.1200 / MiTek-US.com

Job	Truss	Truss Type	Qty	Ply	Roof
P241184-01	B4	Piggyback Base	4	1	I77196684
					Job Reference (optional)

Premier Building Supply (Springhill, KS), Spring Hills, KS - 66083, Run: 8.63 S Sep 26 2024 Print: 8.630 S Sep 26 2024 MiTek Industries, Inc. Mon Oct 20 16:02:24 Page: 1
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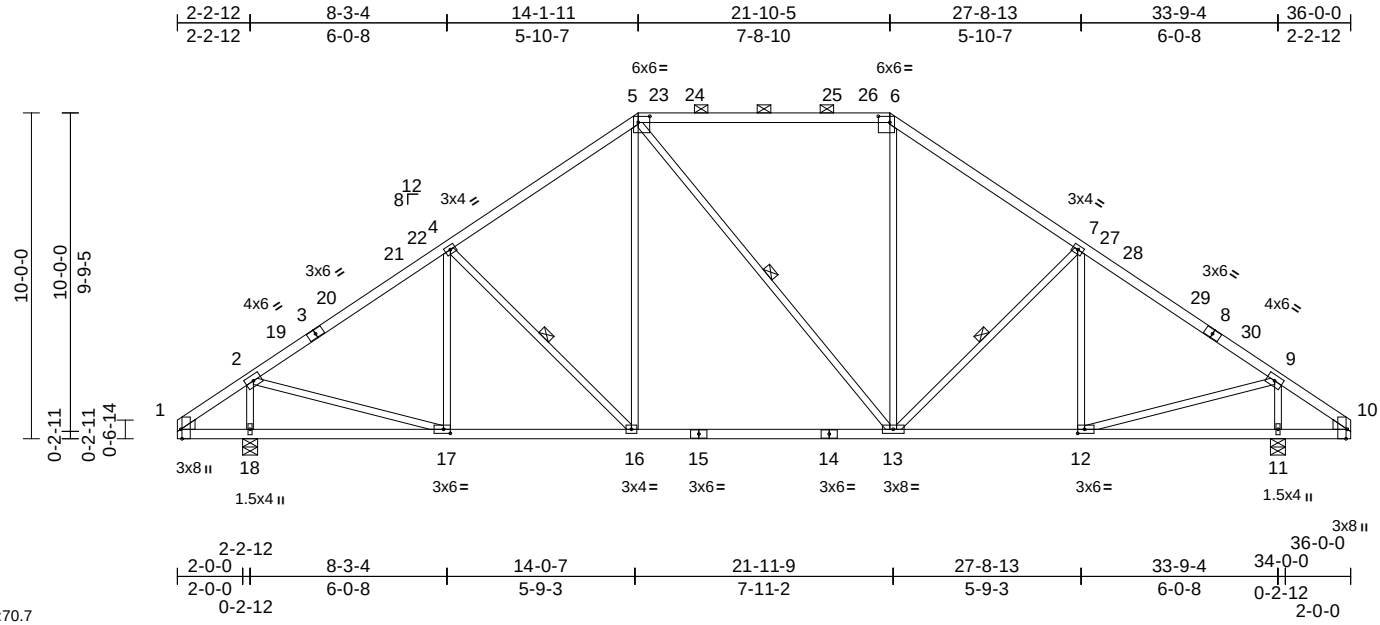


Scale = 1:70.7

Plate Offsets (X, Y): [1:0-3-8,Edge], [5:0-4-4,0-2-4], [6:0-4-4,0-2-4], [10:0-3-8,Edge], [12:0-2-8,0-1-8], [17:0-2-8,0-1-8]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.84	Vert(LL)	-0.10	13-16	>999	240	MT20	244/190
Snow (Pf/Pg)	21.9/20.0	Lumber DOL	1.15	BC	0.54	Vert(CT)	-0.22	13-16	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.45	Horz(CT)	0.04	11	n/a	n/a		
BCLL	0.0	Code	IRC2018/TPI2014	Matrix-S								
BCDL	10.0										Weight: 177 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2 *Except* 5-6:2x4 SP 2400F 2.0E
BOT CHORD 2x4 SP No.2
WEBS 2x3 SPF No.2
WEDGE Left: 2x4 SP No.2
Right: 2x4 SP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-6-10 oc purlins, except 2-0-0 oc purlins (4-5-7 max.): 5-6.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 5-13, 4-16, 7-13

REACTIONS (size) 11=0-5-8, 18=0-5-8
Max Horiz 18=-202 (LC 12)
Max Uplift 11=-49 (LC 17), 18=-49 (LC 16)
Max Grav 11=1713 (LC 40), 18=1713 (LC 40)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-80/64, 2-4=-1725/69, 4-5=-1511/127, 5-6=-1103/148, 6-7=-1512/127, 7-9=-1725/70, 9-10=-80/64
BOT CHORD 1-18=0/65, 17-18=-192/221, 16-17=-59/1331, 13-16=-19/1102, 12-13=0/1331, 11-12=0/66, 10-11=0/66
WEBS 5-16=-24/395, 5-13=-134/135, 6-13=0/371, 2-18=-1592/128, 9-11=-1592/128, 4-16=-353/156, 7-13=-352/156, 4-17=-265/76, 2-17=-24/1323, 7-12=-266/76, 9-12=-24/1323

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=117mph (3-second gust) Vasd=92mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. III; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-0-0 to 5-0-0, Interior (1) 5-0-0 to 14-1-11, Exterior(2R) 14-1-11 to 21-2-9, Interior (1) 21-2-9 to 21-10-5, Exterior(2R) 21-10-5 to 28-11-3, Interior (1) 28-11-3 to 36-0-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=21.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.1; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0
4) Unbalanced snow loads have been considered for this design.
5) Provide adequate drainage to prevent water ponding.
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 49 lb uplift at joint 18 and 49 lb uplift at joint 11.
9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

October 22,2025

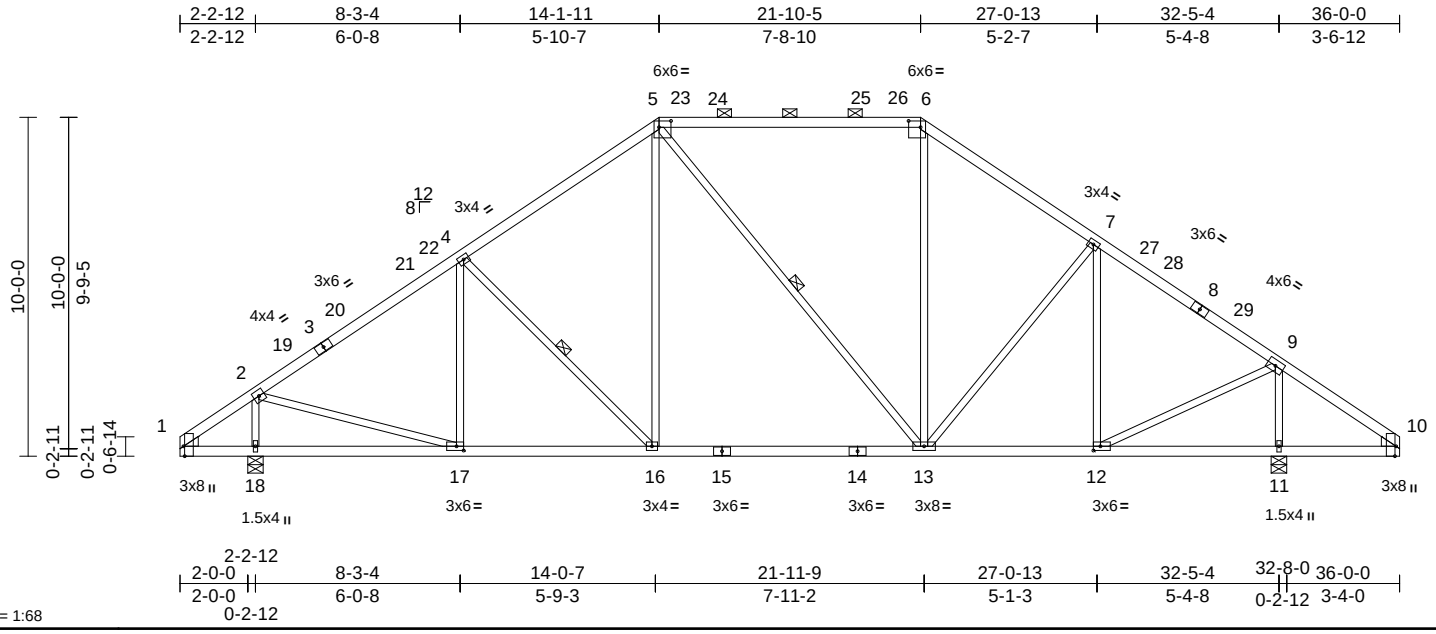
MiTek®
16023 Swingley Ridge Rd.
Chesterfield, MO 63017
314.434.1200 / MiTek-US.com

Job	Truss	Truss Type	Qty	Ply	Roof
P241184-01	B5	Piggyback Base	3	1	I77196685
					Job Reference (optional)

Premier Building Supply (Springhill, KS), Spring Hills, KS - 66083,

Run: 8.63 S Sep 26 2024 Print: 8.630 S Sep 26 2024 MiTek Industries, Inc. Mon Oct 20 16:02:25 ID:g8J_NwxPdKEPQ16_pNI4tHyFLre-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1



Scale = 1:68

Plate Offsets (X, Y): [1:0-3-8,Edge], [5:0-4-4,0-2-4], [6:0-4-4,0-2-4], [10:0-3-8,Edge], [12:0-2-8,0-1-8], [17:0-2-8,0-1-8]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.82	Vert(LL)	-0.10	13-16	>999	240	MT20	244/190
Snow (Pf/Pg)	21.9/20.0	Lumber DOL	1.15	BC	0.53	Vert(CT)	-0.21	13-16	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.43	Horz(CT)	0.03	11	n/a	n/a		
BCLL	0.0	Code	IRC2018/TPI2014	Matrix-S								
BCDL	10.0										Weight: 177 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2 *Except* 5-6:2x4 SP 2400F 2.0E
BOT CHORD	2x4 SP No.2
WEBS	2x3 SPF No.2
WEDGE	Left: 2x4 SP No.2 Right: 2x4 SP No.2

BRACING

TOP CHORD	Structural wood sheathing directly applied or 3-9-5 oc purlins, except 2-0-0 oc purlins (4-11-7 max.): 5-6.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 6-0-0 oc bracing: 11-12,10-11.
WEBS	1 Row at midpt 5-13, 4-16

REACTIONS (size) 11=0-5-8, 18=0-5-8
Max Horiz 18=-202 (LC 12)
Max Uplift 11=-51 (LC 17), 18=-51 (LC 16)
Max Grav 11=1783 (LC 40), 18=1637 (LC 40)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=-80/65, 2-4=-1630/72, 4-5=-1400/118, 5-6=-946/139, 6-7=-1302/121, 7-9=-1352/67, 9-10=-125/230
BOT CHORD	1-18=0/65, 17-18=-193/220, 16-17=-66/1252, 13-16=-31/1009, 12-13=0/1011, 11-12=-118/119, 10-11=-118/119
WEBS	5-16=-23/406, 5-13=-192/91, 6-13=-3/302, 2-18=-1518/124, 9-11=-1665/148, 4-16=-368/155, 4-17=-242/74, 2-17=-18/1242, 7-13=-166/202, 7-12=-453/77, 9-12=-32/1220

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=117mph (3-second gust) Vasd=92mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. III; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-0-0 to 5-0-0, Interior (1) 5-0-0 to 14-1-11, Exterior(2R) 14-1-11 to 21-2-9, Interior (1) 21-2-9 to 21-10-5, Exterior(2R) 21-10-5 to 28-11-3, Interior (1) 28-11-3 to 36-0-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=21.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.1; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0
4) Unbalanced snow loads have been considered for this design.
5) Provide adequate drainage to prevent water ponding.
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 51 lb uplift at joint 18 and 51 lb uplift at joint 11.
9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

October 22,2025

MiTek®
16023 Swingley Ridge Rd.
Chesterfield, MO 63017
314.434.1200 / MiTek-US.com

Job	Truss	Truss Type	Qty	Ply	Roof
P241184-01	B7	Piggyback Base Structural Gable	1	1	I77196686
					Job Reference (optional)

Premier Building Supply (Springhill, KS), Spring Hills, KS - 66083, Run: 8.63 E May 25 2023 Print: 8.630 E May 25 2023 MiTek Industries, Inc. Wed Oct 22 09:05:34 Page: 1
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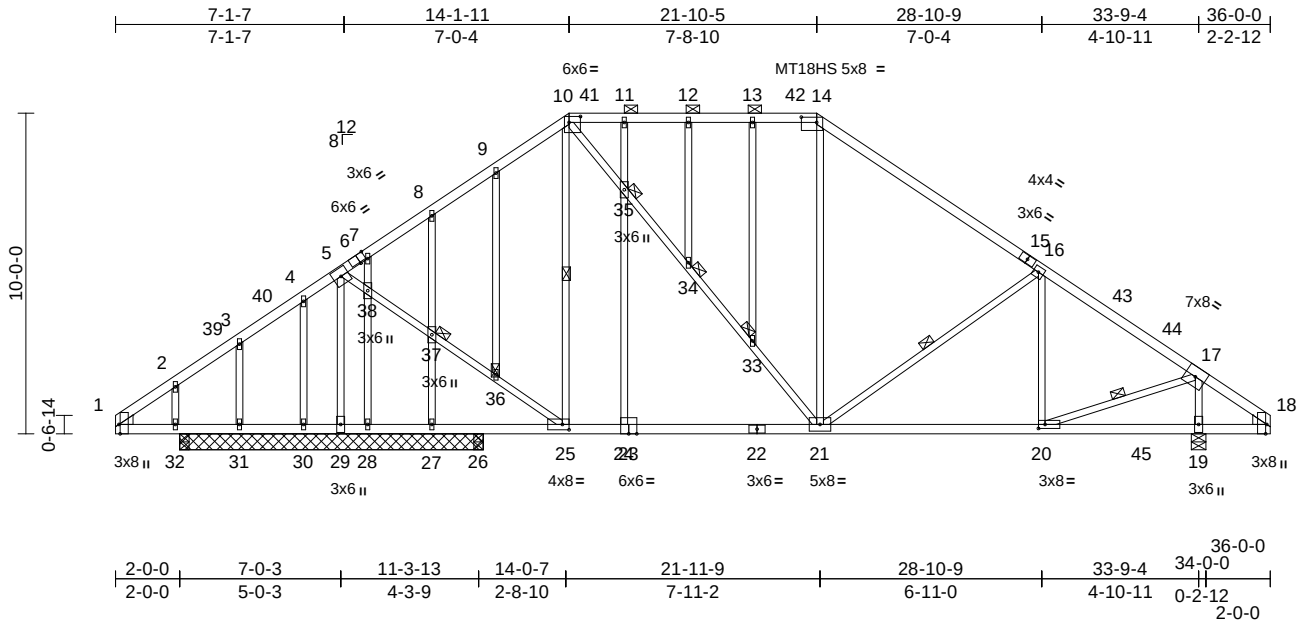


Scale = 1:71.8

Plate Offsets (X, Y): [1:0-3-8,Edge], [6:0-2-9,Edge], [10:0-4-4,0-2-4], [14:0-5-12,0-2-0], [18:0-3-8,Edge], [20:0-2-8,0-1-8], [25:0-2-8,0-2-0]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.85	Vert(LL)	-0.16	21-24	>999	240	MT20	244/190
Snow (Pf/Pg)	21.9/20.0	Lumber DOL	1.15	BC	0.82	Vert(CT)	-0.25	21-24	>999	180	MT18HS	197/144
TCDL	10.0	Rep Stress Incr	NO	WB	0.87	Horz(CT)	0.05	19	n/a	n/a		
BCLL	0.0	Code	IRC2018/TPI2014	Matrix-S								
BCDL	10.0										Weight: 207 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP 2400F 2.0E *Except* 10-14:2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x3 SPF No.2
OTHERS	2x3 SPF No.2
WEDGE	Left: 2x4 SP No.2 Right: 2x4 SP No.2

BRACING

TOP CHORD	Structural wood sheathing directly applied or 5-1-6 oc purlins, except 2-0-0 oc purlins (4-4-5 max.): 10-14.
BOT CHORD	Rigid ceiling directly applied or 4-2-14 oc bracing.
WEBS	1 Row at midpt 10-25, 16-21, 17-20
JOINTS	1 Brace at Jt(s): 33, 34, 35, 36, 37

REACTIONS (lb/size) 19=1158/0-5-8, 26=-28/0-3-8, 27=232/9-5-9, 28=230/9-5-9, 29=582/9-5-9, 30=257/9-5-9, 31=106/9-5-9, 32=200/0-3-8
Max Horiz 31=-202 (LC 49)
Max Uplift 19=-2005 (LC 56), 26=-102 (LC 81), 27=-157 (LC 53), 28=-318 (LC 45), 29=-1119 (LC 45), 30=-383 (LC 53), 31=-477 (LC 54), 32=-535 (LC 53)
Max Grav 19=2553 (LC 65), 26=102 (LC 51), 27=405 (LC 82), 28=470 (LC 66), 29=1399 (LC 66), 30=522 (LC 66), 31=528 (LC 49), 32=621 (LC 66)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-324/319, 2-39=-179/202, 3-39=-141/200, 3-40=-300/373, 4-40=-551/655, 4-5=-601/663, 5-6=-1027/690, 6-7=-901/578, 7-8=-1135/746, 8-9=-894/489, 9-10=-717/390, 10-41=-914/525, 11-41=-853/331, 11-12=-752/310, 12-13=-856/474, 13-42=-1028/643, 14-42=-1109/724, 14-15=-2326/1756, 15-16=-2398/1820, 16-43=-1903/1330, 43-44=-2246/1734, 17-44=-2613/1962, 17-18=-362/348

BOT CHORD 1-32=-323/321, 31-32=-296/293, 30-31=-725/704, 29-30=-888/867, 28-29=-1013/991, 27-28=-1295/1244, 26-27=-1483/1477, 25-26=-1882/1859, 24-25=-743/1013, 23-24=-743/1027, 22-23=-1006/1271, 21-22=-1602/1857, 20-21=-1381/1952, 20-45=-418/460, 19-45=-72/115, 18-19=-72/115

WEBS 5-38=-1556/1916, 37-38=-1566/1922, 36-37=-1567/1936, 25-36=-1586/1948, 10-25=-1013/866, 10-35=-1017/1210, 34-35=-988/1164, 33-34=-987/1166, 21-33=-990/1173, 14-21=-362/496, 5-29=-1471/1149, 16-21=-814/760, 16-20=-748/740, 17-20=-1926/2515, 17-19=-2611/2011, 13-33=-6/20, 12-34=-54/13, 11-35=-204/125, 24-35=-236/102, 9-36=-34/43, 8-37=-352/135, 27-37=-399/149, 7-38=-448/346, 28-38=-458/352, 4-30=-463/349, 3-31=-307/318, 2-32=-360/327

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=117mph (3-second gust) Vasd=92mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. III; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-0-0 to 5-0-0, Interior (1) 5-0-0 to 14-1-11, Exterior(2R) 14-1-11 to 21-2-9, Interior (1) 21-2-9 to 21-10-5, Exterior(2R) 21-10-5 to 28-10-9, Interior (1) 28-10-9 to 36-0-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=21.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.1; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.

October 22,2025

Continued on page 2

MiTek®
16023 Swingley Ridge Rd.
Chesterfield, MO 63017
314.434.1200 / MiTek-US.com

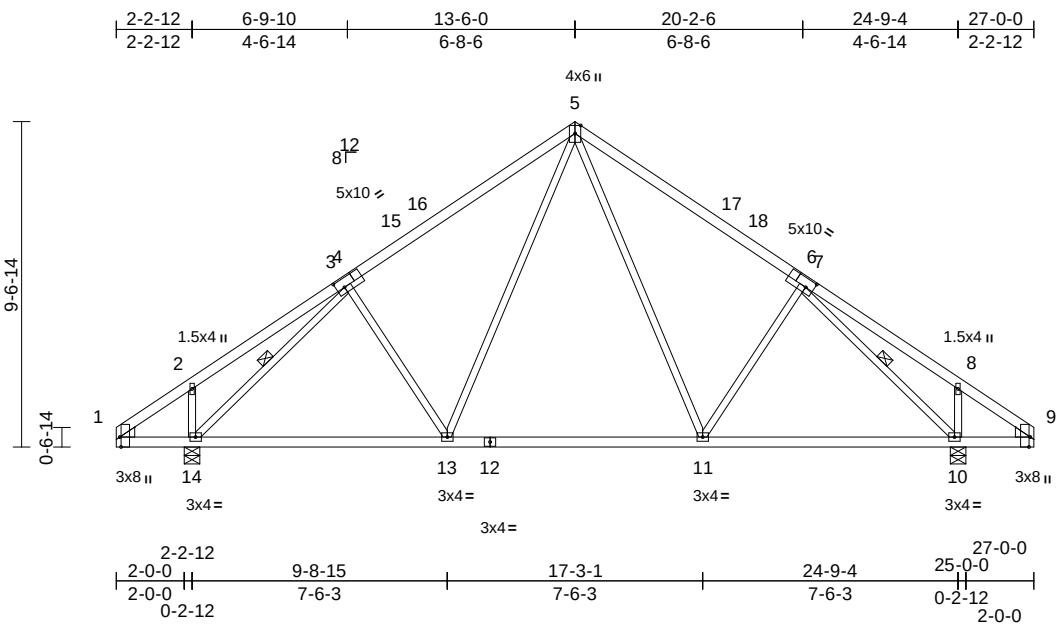
5) Unbalanced snow loads have been considered for this design.
6) Provide adequate drainage to prevent water ponding.
7) All plates are MT20 plates unless otherwise indicated.
8) All plates are 1.5x4 MT20 unless otherwise indicated.
9) Gable studs spaced at 2-0-0 oc.
10) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
11) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1119 lb uplift at joint 29, 2005 lb uplift at joint 19, 157 lb uplift at joint 27, 318 lb uplift at joint 28, 383 lb uplift at joint 30, 477 lb uplift at joint 31, 535 lb uplift at joint 32 and 102 lb uplift at joint 26.
13) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
14) This truss has been designed for a total drag load of 4500 lb. Lumber DOL=(1.33) Plate grip DOL=(1.33) Connect truss to resist drag loads along bottom chord from 0-0-0 to 32-0-0 for 140.6 plf.
15) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	Roof
P241184-01	C1	Common	8	1	I77196687
					Job Reference (optional)

Premier Building Supply (Springhill, KS), Spring Hills, KS - 66083,

Run: 8.63 S Sep 26 2024 Print: 8.630 S Sep 26 2024 MiTek Industries, Inc. Mon Oct 20 16:02:26 ID:VIgFez?ADA_Z8yZ79dPU6YyFLrY-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



Scale = 1:67.8

Plate Offsets (X, Y): [1:0-3-8,Edge], [4:0-2-12,0-3-0], [6:0-2-12,0-3-0], [9:0-3-8,Edge]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.89	Vert(LL)	-0.06	13-14	>999	240	MT20	197/144
Snow (Pf/Pg)	16.9/20.0	Lumber DOL	1.15	BC	0.48	Vert(CT)	-0.12	13-14	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.42	Horz(CT)	0.02	10	n/a	n/a		
BCLL	0.0	Code	IRC2018/TPI2014	Matrix-S								
BCDL	10.0										Weight: 127 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x3 SPF No.2
WEDGE	Left: 2x4 SP No.2 Right: 2x4 SP No.2

BRACING

TOP CHORD	Structural wood sheathing directly applied or 2-2-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS	1 Row at midpt 3-14, 7-10

REACTIONS (size) 10=0-5-8, 14=0-5-8
Max Horiz 14=-193 (LC 12)
Max Uplift 10=-51 (LC 17), 14=-51 (LC 16)
Max Grav 10=1080 (LC 2), 14=1080 (LC 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=-106/91, 2-3=-74/135, 3-5=-874/138, 5-7=-874/138, 7-8=-74/135, 8-9=-106/91
BOT CHORD	1-14=-33/96, 13-14=-95/731, 11-13=0/545, 10-11=0/677, 9-10=-33/96
WEBS	5-11=-72/292, 5-13=-72/292, 2-14=-197/108, 3-14=-1019/94, 3-13=-142/189, 7-11=-142/189, 7-10=-1019/94, 8-10=-197/108

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=117mph (3-second gust) Vasd=92mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. III; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-0-0 to 5-0-0, Interior (1) 5-0-0 to 13-6-0, Exterior(2R) 13-6-0 to 18-6-0, Interior (1) 18-6-0 to 27-0-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=16.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.1; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
4) Unbalanced snow loads have been considered for this design.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 51 lb uplift at joint 14 and 51 lb uplift at joint 10.
8) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard

October 22,2025

MiTek®
16023 Swingley Ridge Rd.
Chesterfield, MO 63017
314.434.1200 / MiTek-US.com

Job	Truss	Truss Type	Qty	Ply	Roof
P241184-01	C2	Common Structural Gable	1	1	I77196688
					Job Reference (optional)

Premier Building Supply (Springhill, KS), Spring Hills, KS - 66083, Run: 8.63 S Sep 26 2024 Print: 8.630 S Sep 26 2024 MiTek Industries, Inc. Mon Oct 20 16:02:26 Page: 1
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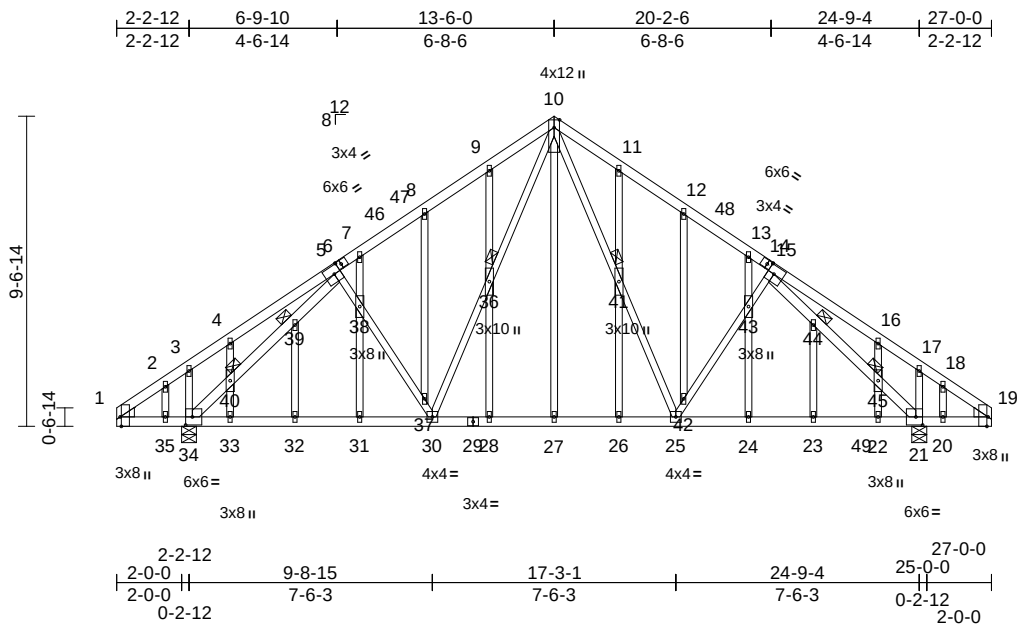

Scale = 1:71.1

Plate Offsets (X, Y): [1:0-3-8,Edge], [5:0-2-8,0-3-0], [6:0-1-11,0-1-8], [14:0-1-11,0-1-8], [15:0-2-8,0-3-0], [19:0-3-8,Edge], [21:0-2-8,0-3-0], [34:0-2-8,0-3-0]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.51	Vert(LL)	0.09	23-24	>999	240	MT20	244/190
Snow (Pf/Pg)	16.9/20.0	Lumber DOL	1.15	BC	0.94	Vert(CT)	-0.11	23-24	>999	180		
TCDL	10.0	Rep Stress Incr	NO	WB	0.93	Horz(CT)	0.03	21	n/a	n/a		
BCLL	0.0	Code	IRC2018/TPI2014	Matrix-S								
BCDL	10.0										Weight: 183 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x3 SPF No.2 *Except* 5-34,15-21:2x4 SP No.2
OTHERS 2x3 SPF No.2
WEDGE Left: 2x4 SP No.2
Right: 2x4 SP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-1-4 oc purlins.
BOT CHORD Rigid ceiling directly applied or 4-1-15 oc bracing.
WEBS 2 Rows at 1/3 pts 5-34, 15-21
JOINTS 1 Brace at Jt(s): 36, 41

REACTIONS (size) 21=0-5-8, 34=0-5-8
Max Horiz 34=-191 (LC 46)
Max Uplift 21=-1942 (LC 56), 34=-1940 (LC 53)
Max Grav 21=2457 (LC 51), 34=2455 (LC 50)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-234/188, 2-3=-363/343, 3-4=-547/497, 4-5=-1251/1286, 5-7=-1873/1262, 7-8=-1569/1036, 8-9=-1233/709, 9-10=-864/412, 10-11=-880/444, 11-12=-1223/704, 12-13=-1567/1039, 13-15=-1870/1263, 15-16=-1251/1285, 16-17=-548/498, 17-18=-363/344, 18-19=-234/188
BOT CHORD 1-35=-146/218, 34-35=-288/361, 33-34=-1381/1820, 32-33=-1132/1572, 31-32=-741/1180, 30-31=-350/885, 28-30=-298/772, 27-28=-356/816, 26-27=-736/1160, 25-26=-1091/1504, 24-25=-1040/1509, 23-24=-1431/1900, 22-23=-1725/2193, 21-22=-1725/2193, 20-21=-137/209, 19-20=-137/209
WEBS 10-41=-651/748, 25-41=-689/789, 30-36=-653/758, 10-36=-681/793, 3-34=-401/273, 34-40=-2821/2241, 39-40=-2949/2337, 5-39=-3118/2499, 5-38=-622/646, 37-38=-637/660, 30-37=-710/697, 25-42=-714/703, 42-43=-632/655, 15-43=-621/646, 15-44=-3117/2506, 44-45=-2948/2342, 21-45=-2821/2248, 17-21=-400/273, 10-27=-26/154, 9-36=-117/61, 28-36=-92/73, 8-37=-215/102, 7-38=-130/253, 31-38=-94/211, 32-39=-228/219, 4-40=-173/264, 33-40=-69/99, 2-35=-44/45, 11-41=-90/63, 26-41=-102/75, 12-42=-200/89, 13-43=-128/250, 24-43=-97/214, 23-44=-227/220, 16-45=-172/263, 22-45=-69/98, 18-20=-44/45

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=117mph (3-second gust) Vasd=92mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. III; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-0-0 to 5-0-0, Interior (1) 5-0-0 to 13-4-9, Exterior(2R) 13-4-9 to 18-4-9, Interior (1) 18-4-9 to 27-0-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=16.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.1; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
5) Unbalanced snow loads have been considered for this design.
6) All plates are 1.5x4 MT20 unless otherwise indicated.
7) Gable studs spaced at 2-0-0 oc.
8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
9) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1940 lb uplift at joint 34 and 1942 lb uplift at joint 21.
11) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

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12) This truss has been designed for a total drag load of 4500 lb. Lumber DOL=(1.33) Plate grip DOL=(1.33) Connect truss to resist drag loads along bottom chord from 0-0-0 to 23-0-0 for 195.7 plf.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	Roof
P241184-01	C3	Common Supported Gable	1	1	I77196689
					Job Reference (optional)

Premier Building Supply (Springhill, KS), Spring Hills, KS - 66083, Run: 8.63 S Sep 26 2024 Print: 8.630 S Sep 26 2024 MiTek Industries, Inc. Mon Oct 20 16:02:26 Page: 1
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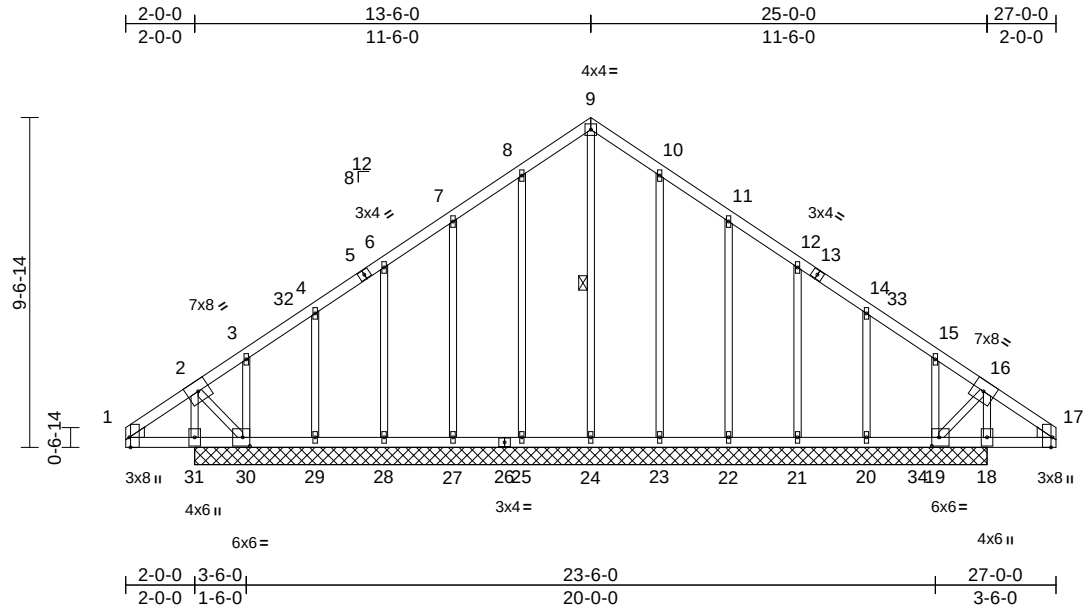

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Plate Offsets (X, Y): [1:0-3-8,Edge], [17:0-3-8,Edge], [19:0-2-8,0-3-0], [30:0-2-8,0-3-0]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.40	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf/Pg)	16.9/20.0	Lumber DOL	1.15	BC	0.38	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	NO	WB	0.79	Horiz(TL)	0.01	18	n/a	n/a		
BCLL	0.0	Code	IRC2018/TPI2014	Matrix-S								
BCDL	10.0										Weight: 143 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x3 SPF No.2
OTHERS 2x3 SPF No.2
WEDGE Left: 2x4 SP No.2
Right: 2x4 SP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-6-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing, Except:
5-5-5 oc bracing: 29-30
5-8-12 oc bracing: 21-22
5-1-11 oc bracing: 20-21
4-9-3 oc bracing: 19-20.
WEBS 1 Row at midpt 9-24

REACTIONS (size) 18=23-0-0, 19=23-0-0, 20=23-0-0, 21=23-0-0, 22=23-0-0, 23=23-0-0, 24=23-0-0, 25=23-0-0, 27=23-0-0, 28=23-0-0, 29=23-0-0, 30=23-0-0, 31=23-0-0
Max Horiz 31=-193 (LC 46)
Max Uplift 18=-2557 (LC 48), 19=-1703 (LC 55), 20=-51 (LC 56), 21=-46 (LC 55), 22=-52 (LC 55), 23=-43 (LC 56), 24=-10 (LC 68), 25=-45 (LC 54), 27=-52 (LC 53), 28=-47 (LC 54), 29=-52 (LC 53), 30=-1835 (LC 54), 31=-2791 (LC 45)
Max Grav 18=2751 (LC 49), 19=1679 (LC 52), 20=172 (LC 30), 21=164 (LC 30), 22=182 (LC 23), 23=233 (LC 23), 24=236 (LC 75), 25=233 (LC 22), 27=182 (LC 22), 28=164 (LC 29), 29=172 (LC 29), 30=1824 (LC 49), 31=2985 (LC 52)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-261/266, 2-3=-1709/1688, 3-4=-1502/1470, 4-6=-1206/1208, 6-7=-933/959, 7-8=-670/724, 8-9=-428/475, 9-10=-313/360, 10-11=-550/580, 11-12=-789/811, 12-14=-1043/1047, 14-15=-1339/1308, 15-16=-1560/1524, 16-17=-276/275
BOT CHORD 1-31=-248/269, 30-31=-600/566, 29-30=-1181/1245, 28-29=-920/984, 27-28=-659/724, 25-27=-398/463, 24-25=-300/364, 23-24=-561/625, 22-23=-822/886, 21-22=-1082/1147, 20-21=-1343/1408, 19-20=-1539/1603, 18-19=-116/144, 17-18=-116/144
WEBS 9-24=-199/87, 8-25=-193/68, 7-27=-142/75, 6-28=-125/71, 4-29=-130/86, 3-30=-100/64, 2-31=-2807/2664, 10-23=-193/67, 11-22=-142/77, 12-21=-125/71, 14-20=-130/86, 15-19=-100/65, 16-18=-2588/2445, 2-30=-2270/2362, 16-19=-2088/2196

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=117mph (3-second gust) Vasd=92mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. III; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Corner(3E) 0-0-0 to 5-0-0, Exterior(2N) 5-0-0 to 13-6-0, Corner(3R) 13-6-0 to 18-6-0, Exterior(2N) 18-6-0 to 27-0-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=16.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.1; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
5) Unbalanced snow loads have been considered for this design.
6) All plates are 1.5x4 MT20 unless otherwise indicated.
7) Gable studs spaced at 2-0-0 oc.
8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
9) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.

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10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 10 lb uplift at joint 24, 45 lb uplift at joint 25, 52 lb uplift at joint 27, 47 lb uplift at joint 28, 52 lb uplift at joint 29, 1835 lb uplift at joint 30, 2791 lb uplift at joint 31, 43 lb uplift at joint 23, 52 lb uplift at joint 22, 46 lb uplift at joint 21, 51 lb uplift at joint 20, 1703 lb uplift at joint 19 and 2557 lb uplift at joint 18.
11) Non Standard bearing condition. Review required.
12) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
13) This truss has been designed for a total drag load of 3000 lb. Lumber DOL=(1.33) Plate grip DOL=(1.33) Connect truss to resist drag loads along bottom chord from 0-0-0 to 23-0-0 for 130.4 plf.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	Roof
P241184-01	PB1	Piggyback	2	1	I77196690
					Job Reference (optional)

Premier Building Supply (Springhill, KS), Spring Hills, KS - 66083,

Run: 8.63 S Sep 26 2024 Print: 8.630 S Sep 26 2024 MiTek Industries, Inc. Mon Oct 20 16:02:27 ID:60hkJ_XjfLh_E_Ose0pllXynd2F-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

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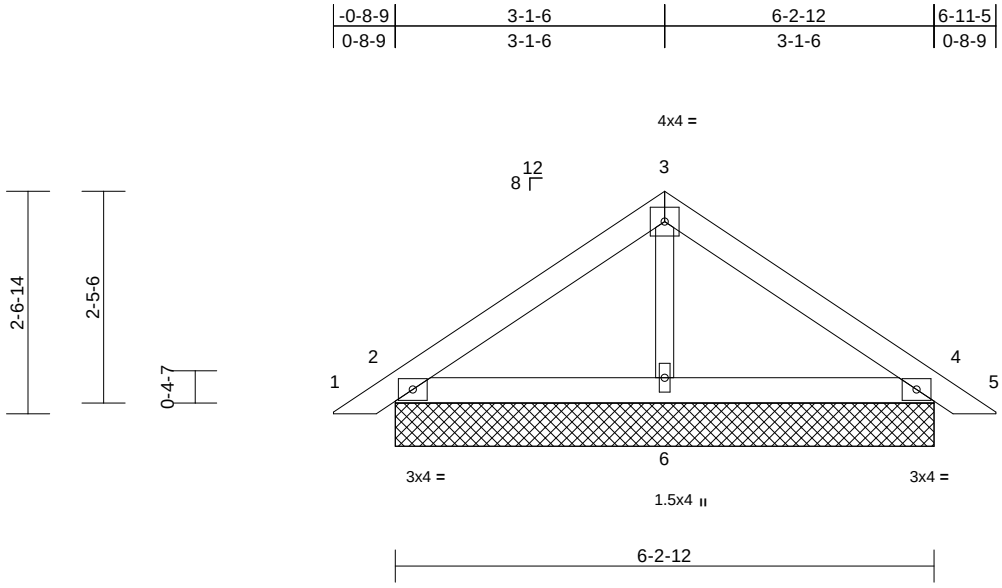

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Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.24	Vert(LL)	n/a	-	n/a	999	MT20	197/144
Snow (Pf/Pg)	16.9/20.0	Lumber DOL	1.15	BC	0.10	Vert(CT)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.03	Horz(CT)	0.00	4	n/a	n/a		
BCLL	0.0	Code	IRC2018/TPI2014	Matrix-P								
BCDL	10.0										Weight: 24 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
OTHERS	2x3 SPF No.2

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (size) 2=6-2-12, 4=6-2-12, 6=6-2-12
Max Horiz 2=-50 (LC 14)
Max Uplift 2=-29 (LC 16), 4=-35 (LC 17)
Max Grav 2=216 (LC 23), 4=216 (LC 24), 6=217 (LC 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/20, 2-3=-97/55, 3-4=-97/56, 4-5=0/20
BOT CHORD	2-6=-7/42, 4-6=-7/42
WEBS	3-6=-139/61

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=117mph (3-second gust) Vasd=92mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. III; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=16.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.1; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
5) Unbalanced snow loads have been considered for this design.
6) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 16.9 psf on overhangs non-concurrent with other live loads.
7) Gable requires continuous bottom chord bearing.
8) Gable studs spaced at 2-0-0 oc.
9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
10) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 29 lb uplift at joint 2 and 35 lb uplift at joint 4.
12) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
13) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

LOAD CASE(S) Standard

October 22,2025

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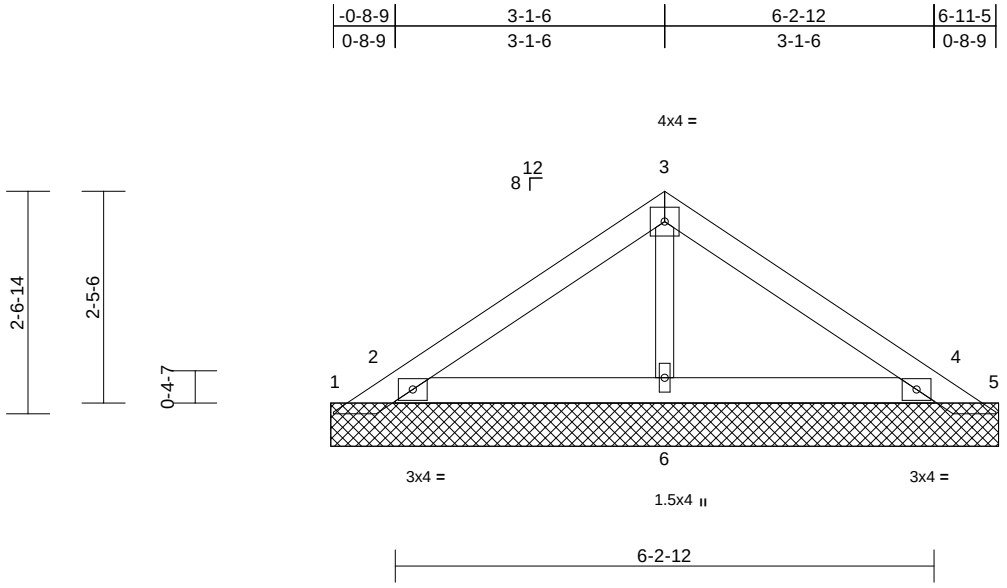
Job	Truss	Truss Type	Qty	Ply	Roof
P241184-01	PB2	Piggyback	19	1	I77196691
					Job Reference (optional)

Premier Building Supply (Springhill, KS), Spring Hills, KS - 66083,

Run: 8.63 S Sep 26 2024 Print: 8.630 S Sep 26 2024 MiTek Industries, Inc. Mon Oct 20 16:02:27 ID:92Xi9zUfjWMZQYv?YnNG9oyndBL-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f



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Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.21	Vert(LL)	n/a	-	n/a	999	MT20	197/144
Snow (Pf/Pg)	16.9/20.0	Lumber DOL	1.15	BC	0.10	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.02	Horiz(TL)	0.00	4	n/a	n/a		
BCLL	0.0	Code	IRC2018/TPI2014	Matrix-P								
BCDL	10.0										Weight: 24 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
OTHERS	2x3 SPF No.2

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (size) 1=7-8-10, 2=7-8-10, 4=7-8-10, 5=7-8-10, 6=7-8-10
Max Horiz 1=-50 (LC 12)
Max Uplift 1=-167 (LC 23), 2=-129 (LC 16), 4=-117 (LC 17), 5=-167 (LC 24)
Max Grav 1=96 (LC 16), 2=412 (LC 23), 4=412 (LC 24), 5=77 (LC 17), 6=195 (LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=-76/120, 2-3=-114/52, 3-4=-114/52, 4-5=-61/105
BOT CHORD	2-6=-12/38, 4-6=-12/38
WEBS	3-6=-117/38

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=117mph (3-second gust) Vasd=92mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. III; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=16.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.1; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
5) Unbalanced snow loads have been considered for this design.
6) Gable requires continuous bottom chord bearing.
7) Gable studs spaced at 2-0-0 oc.
8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
9) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 167 lb uplift at joint 1, 167 lb uplift at joint 5, 129 lb uplift at joint 2 and 117 lb uplift at joint 4.
11) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
12) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

LOAD CASE(S) Standard

October 22,2025

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Job	Truss	Truss Type	Qty	Ply	Roof
P241184-01	PB3	Piggyback	1	2	I77196692
					Job Reference (optional)

Premier Building Supply (Springhill, KS), Spring Hills, KS - 66083, Run: 8.63 S Sep 26 2024 Print: 8.630 S Sep 26 2024 MiTek Industries, Inc. Mon Oct 20 16:02:27 Page: 1
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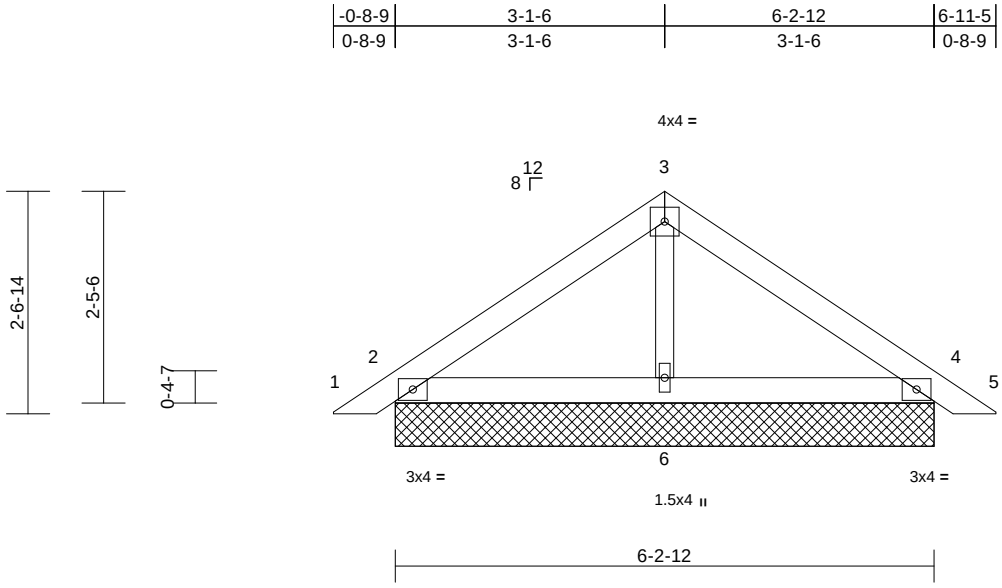

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Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.12	Vert(LL)	n/a	-	n/a	999	MT20	197/144
Snow (Pf/Pg)	16.9/20.0	Lumber DOL	1.15	BC	0.05	Vert(CT)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.01	Horz(CT)	0.00	4	n/a	n/a		
BCLL	0.0	Code	IRC2018/TPI2014	Matrix-P								
BCDL	10.0										Weight: 49 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x3 SPF No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (size) 2=6-2-12, 4=6-2-12, 6=6-2-12
Max Horiz 2=-50 (LC 14)
Max Uplift 2=-29 (LC 16), 4=-35 (LC 17)
Max Grav 2=216 (LC 23), 4=216 (LC 24), 6=217 (LC 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/20, 2-3=-97/55, 3-4=-97/56, 4-5=0/20
BOT CHORD 2-6=-7/42, 4-6=-7/42
WEBS 3-6=-139/61

NOTES
1) 2-ply truss to be connected together as follows:
Top chords connected with 10d (0.131"x3") nails as follows: 2x4 - 1 row at 0-9-0 oc.
Bottom chords connected with 10d (0.131"x3") nails as follows: 2x4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-16; Vult=117mph (3-second gust) Vasd=92mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. III; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
5) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
6) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=16.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.1; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
7) Unbalanced snow loads have been considered for this design.
8) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 16.9 psf on overhangs non-concurrent with other live loads.
9) Gable requires continuous bottom chord bearing.
10) Gable studs spaced at 2-0-0 oc.
11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
12) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 29 lb uplift at joint 2 and 35 lb uplift at joint 4.
14) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
15) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

LOAD CASE(S) Standard

October 22,2025

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Job	Truss	Truss Type	Qty	Ply	Roof
P241184-01	PB4	Piggyback	1	3	I77196693
					Job Reference (optional)

Premier Building Supply (Springhill, KS), Spring Hills, KS - 66083,

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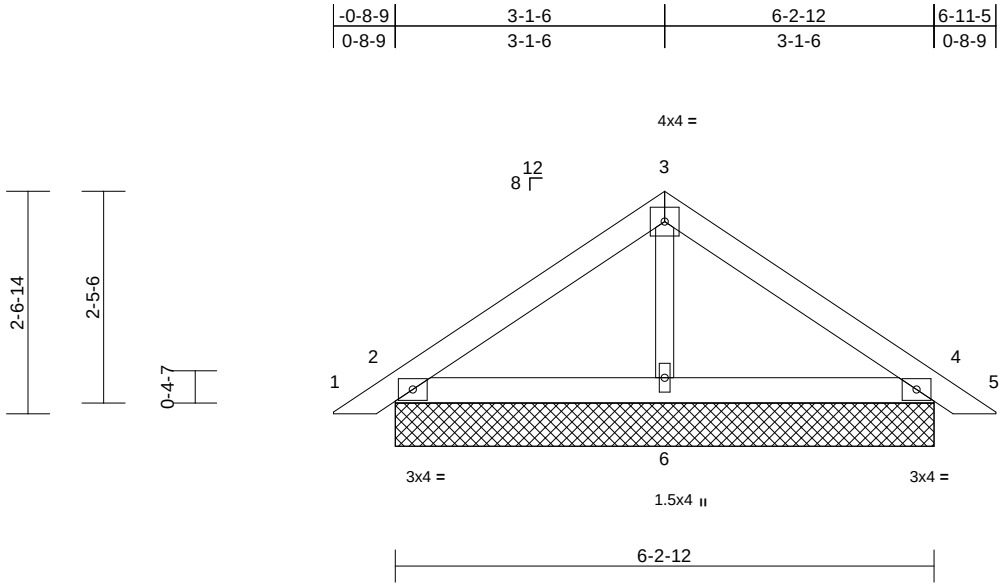

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Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.08	Vert(LL)	n/a	-	n/a	999	MT20	197/144
Snow (Pf/Pg)	16.9/20.0	Lumber DOL	1.15	BC	0.03	Vert(CT)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.01	Horz(CT)	0.00	4	n/a	n/a		
BCLL	0.0	Code	IRC2018/TPI2014	Matrix-P								
BCDL	10.0										Weight: 73 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
OTHERS	2x3 SPF No.2

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (size) 2=6-2-12, 4=6-2-12, 6=6-2-12
Max Horiz 2=-50 (LC 14)
Max Uplift 2=-29 (LC 16), 4=-35 (LC 17)
Max Grav 2=216 (LC 23), 4=216 (LC 24), 6=217 (LC 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/20, 2-3=-97/55, 3-4=-97/56, 4-5=0/20
BOT CHORD	2-6=-7/42, 4-6=-7/42
WEBS	3-6=-139/61

NOTES

1) 3-ply truss to be connected together as follows:
Top chords connected with 10d (0.131"x3") nails as follows: 2x4 - 1 row at 0-9-0 oc.
Bottom chords connected with 10d (0.131"x3") nails as follows: 2x4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-16; Vult=117mph (3-second gust) Vasd=92mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Ke=1.00; Cat. III; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
5) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
6) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=16.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.1; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
7) Unbalanced snow loads have been considered for this design.
8) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 16.9 psf on overhangs non-concurrent with other live loads.
9) Gable requires continuous bottom chord bearing.
10) Gable studs spaced at 2-0-0 oc.
11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
12) All bearings are assumed to be SP No.2 crushing capacity of 565 psi.
13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 29 lb uplift at joint 2 and 35 lb uplift at joint 4.
14) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
15) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

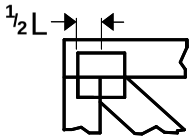
LOAD CASE(S) Standard

October 22,2025

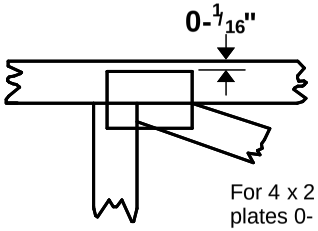
MiTek®
16023 Swingley Ridge Rd.
Chesterfield, MO 63017
314.434.1200 / MiTek-US.com

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- 1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

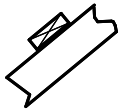
* Plate location details available in MiTek software or upon request.

PLATE SIZE

4 x 4

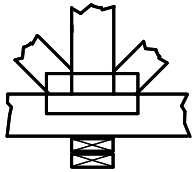
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING

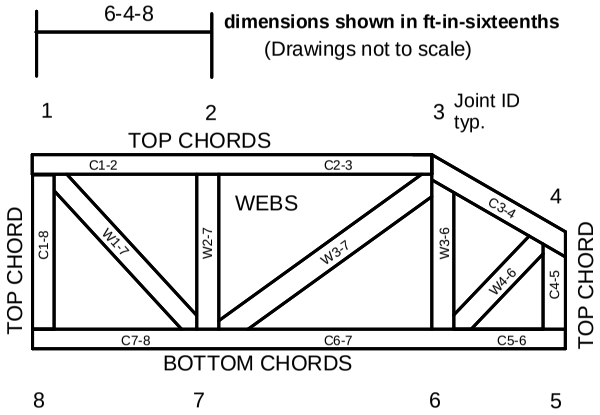


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number/letter where bearings occur. Min size shown is for crushing only.

Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-22: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

Product Code Approvals

ICC-ES Reports:

ESR-1988, ESR-2362, ESR-2685, ESR-3282
ESR-4722, ESL-1388

Design General Notes

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

MiTek®

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.
21. The design does not take into account any dynamic or other loads other than those expressly stated.