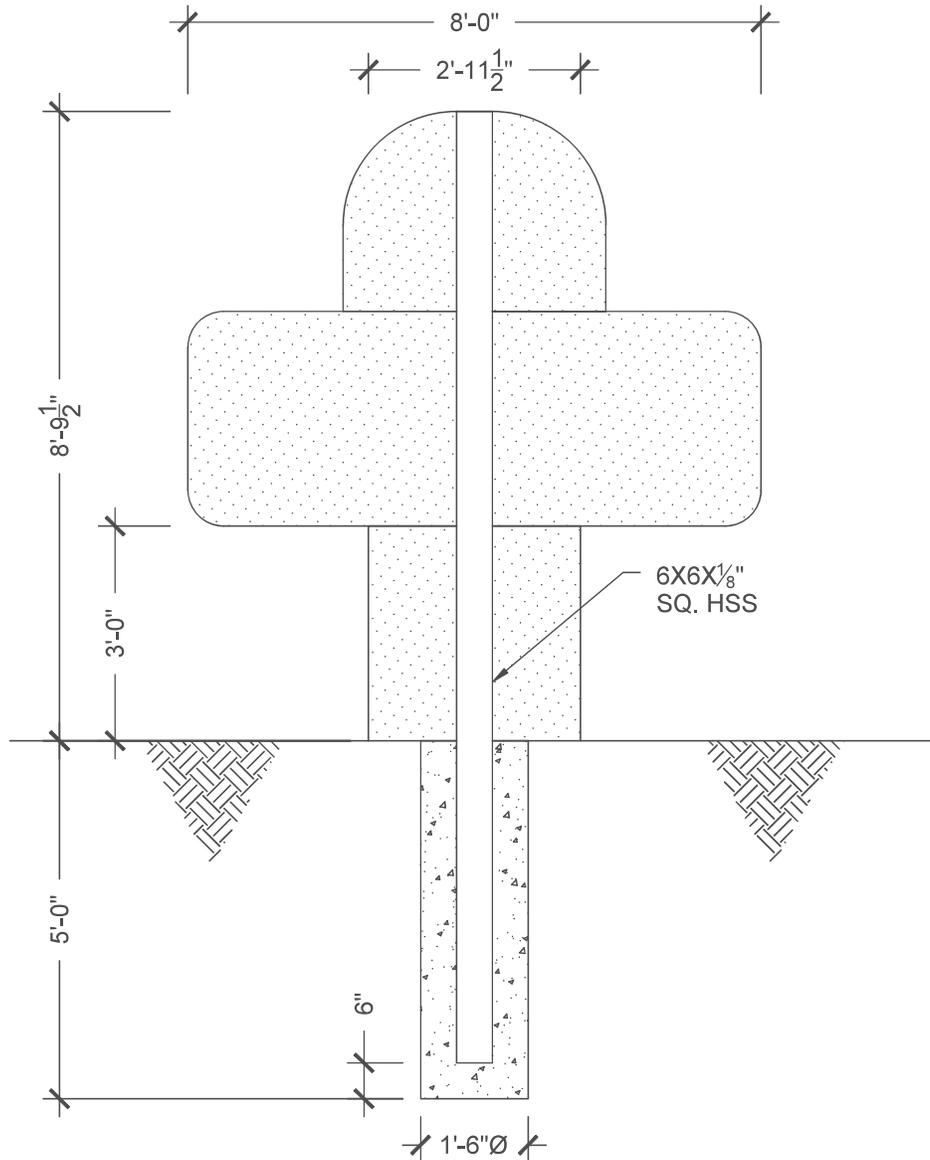




12396 WORLD TRADE DRIVE, SUITE 312
 SAN DIEGO, CA 92128
 PROJECTMANAGER@SULLAWAYENG.COM
 PHONE: 1-858-312-5150 FAX: 1-858-777-3534

PROJECT: SMALLS SLIDERS - LEE'S SUMMIT, 101 SOUTHWEST OLDHAM PKWY, SLD-10051, LEE'S SUMMIT, MO DATE: 02/12/2026
 PROJECT #: 55427 ENGINEER: PD
 CLIENT: ACE SIGN COMPANY LAST REVISED:



① ELEVATION

GENERAL NOTES

1. DESIGN CODE: IBC 2018
2. DESIGN LOADS: ASCE 7-16
3. WIND VELOCITY: 110 MPH EXPOSURE C
4. CONCRETE 2500 PSI MIN.
5. SQ. HSS STEEL ASTM A500 GR.B, Fy= 46 KSI MIN.
6. PROVIDE PROTECTION AGAINST DISSIMILAR METALS USING ANTI-CORROSIVE PAINT OR NEOPRENE GASKETS.
7. LATERAL SOIL BEARING PER IBC CLASS 4 (150 PSF/FT)
8. ALL DIMENSIONS TO BE VERIFIED PRIOR TO FABRICATION.



2/12/2026



PROJECT: SMALLS SLIDERS - LEE'S SUMMIT
 PROJ. NO.: 55427
 CLIENT: ACE SIGN COMPANY

DATE: 2/12/2026
 ENGINEER: PD

v5.5

units; pounds, feet unless noted otherwise

Applied Wind Loads; from ASCE 7-16

$F = q_z * G * C_f * A_f$ with $q_z = 0.00256 K_z K_{zt} K_d V^1$ (29.3.2 & 29.4)
 $C_f = 1.652$ (Fig. 29.3-1) 1.00 0 max. height= 8.79
 $K_{zt} = 1.0$ (26.8.2) (=1.0 unless unusual landscape) s= 5.79
 $K_z =$ from table 28.3-1 Exposure= c
 $K_d = 0.85$ for signs (table 26.6-1)
 $V = 110$ mph
 $G = 0.85$ (26.9) weight= 0.431 kips
 $s/h = 0.659$ $M_{DL} = 0.00$ k-ft
 $B/s = 1.38$

Pole Loads	structure component	height at section c.g.	K_z	q_z	pressure $q_z * G * C_f$	A_f	shear	Wind Moment M_w			
	1	1.500	0.850	22.4	31.42	8.88	279	418			
	2	4.500	0.850	22.4	31.42	24.00	754	3393			
	3	7.396	0.850	22.4	31.42	10.24	322	2378			
					sums:	43.11	1354	6.19	(M_w)	k-ft	arm= 4.6
			$P_u = 0.52$	kips			$M = 6.19$	k-ft			$M = \sqrt{M_{DL}^2 + M_w^2}$
			$M_u = \sqrt{1.2M_{DL}^2 + 1.0M_w^2} = 6.19$	k-ft							

Pole Design

section: tube

$M_u \leq \phi M_n$ with $M_n = f_y Z$ $f_y = 46$ ksi $\phi = 0.9$

H	M_u (k-ft)	Z req'd. (in)	Size (in)	t (in)	Z	USE
at grade	6.19	1.79	3.000	0.174	1.97	6X6X1/8" SQ HSS, $\phi M_n = 16.2$ k-ft

Footing Design

footprint: round

$\omega = 1.3$ IBC 1605.3.2 IBC Table 1806.2, sections 1806.3.4, 1807.3.2 S= (1.3x2x)
 $P = 1.06$ kips $S_1 = Sxd/3$ $A = 2.34xP/(S_1xb)$ S= 400
 $S_1 = 663$ $d = 0.5xA(1+(1+4.36xh/A)^{0.5})$ IBC 1807.3.2.1
 $A = 2.49$

footing: 1' - 6" dia.

5' - 0" deep