



01/12/2026



MISSOURI COA #2005000817

Site Information:	Page 1:
Customer: Kodiak – Premier Building Supply	Job Number: PM000129
Job Description: Customer - Clayton Properties Plan Name - Sienna Elevation - Farmhouse	
Address: 1432 SE VANTAGE POINT DR, LEE'S SUMMIT, MO 64082	

Job Engineering Criteria:		
Building Code: IRC 2018	Design Standard: TPI 2014	IntelliVIEW Version: 25.02.00B
Loading Standard: ASCE 7-16	Design Methodology: ASD	JRef #: 1YGQ96460003
Wind: Wind Speed (mph): 115	Exposure: B	Design Loading (psf): 45
Building Type: Part Enc.	Risk Category: II	
Snow: Pg (psf): NA	Pf(ASD) (psf): NA	
W2::	Risk Category: II	

This package contains general notes pages, 38 truss drawing(s) and 5 detail(s).

Item	Drawing Number	Truss
1	012.26.1516.36493	E4
3	012.26.1516.29233	E3
5	012.26.1518.09287	LAY2
7	012.26.1515.52747	D3
9	012.26.1515.49910	D1
11	012.26.1518.11820	T2
13	012.26.1515.34337	A3
15	012.26.1517.08460	J3
17	012.26.1516.58427	J1
19	012.26.1515.51217	D2
21	012.26.1515.40773	B2
23	012.26.1515.42410	B3
25	012.26.1515.45833	B5
27	012.26.1517.27723	J7
29	012.26.1518.14240	V1
31	012.26.1515.39097	B1
33	012.26.1516.14750	E1
35	012.26.1516.21580	E2
37	012.26.1516.49417	G3
39	A14015ENC160118	
41	A14030ENC160118	
43	VAL180160118	

Item	Drawing Number	Truss
2	012.26.1518.25823	V6
4	012.26.1516.42683	G1
6	012.26.1515.47140	C1
8	012.26.1515.48420	C2
10	012.26.1515.31150	A1
12	012.26.1515.32890	A2
14	012.26.1517.25800	J6
16	012.26.1517.11237	J4
18	012.26.1517.48677	LAY1
20	012.26.1515.35707	A4
22	012.26.1518.16890	V4
24	012.26.1515.44140	B4
26	012.26.1517.29330	J8
28	012.26.1517.19407	J5
30	012.26.1515.37377	A5
32	012.26.1516.46767	G2
34	012.26.1517.57490	LAY1
36	012.26.1516.18317	E1
38	012.26.1518.19463	V5
40	GBLLETIN0118	
42	HIPFRAME0623	

**RELEASE FOR CONSTRUCTION  
 AS NOTED ON PLANS REVIEW  
 DEVELOPMENT SERVICES  
 LEE'S SUMMIT, MISSOURI  
 02/12/2026 4:05:15**

## General Notes

### **Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:**

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high-quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

### **Temporary Lateral Restraint and Bracing:**

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

### **Permanent Lateral Restraint and Bracing:**

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed, and detailed by the Building Designer.

### **Connector Plate Information:**

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at [www.icc-es.org](http://www.icc-es.org).

### **Bearing Information:**

The bearing area factor,  $C_b$ , is considered for the allowable capacity of solid sawn wood bearings supporting trusses that are located a minimum of 3" from the end of the lumber piece.

## **General Notes** (continued)

### **Coated Lumber:**

Coated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Coated lumber has no adjustments to lumber properties. Coated lumber may be more brittle than uncoated lumber. Special handling care must be taken to prevent breakage during all handling activities. Refer to manufacturer literature, specifications, and code evaluation reports for restrictions, details, and requirements.

### **Fire Retardant Treated Lumber:**

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

### **Key to Terms:**

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

C = Coated lumber.

C-AT = AtTEK coated lumber.

C-FX = FX Lumber Guard coated lumber.

C -TE = TechWood 4400 coated lumber.

CL = Certified lumber.

Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-BF = Borafume Fire Retardant Treated lumber

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-ON = OnWood Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

FRT-PR = ProWood Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for all load cases.

Max Web CSI = Maximum bending and axial Combined Stress Index for Webs for all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

## **General Notes** (continued)

### **Key to Terms** (continued):

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.

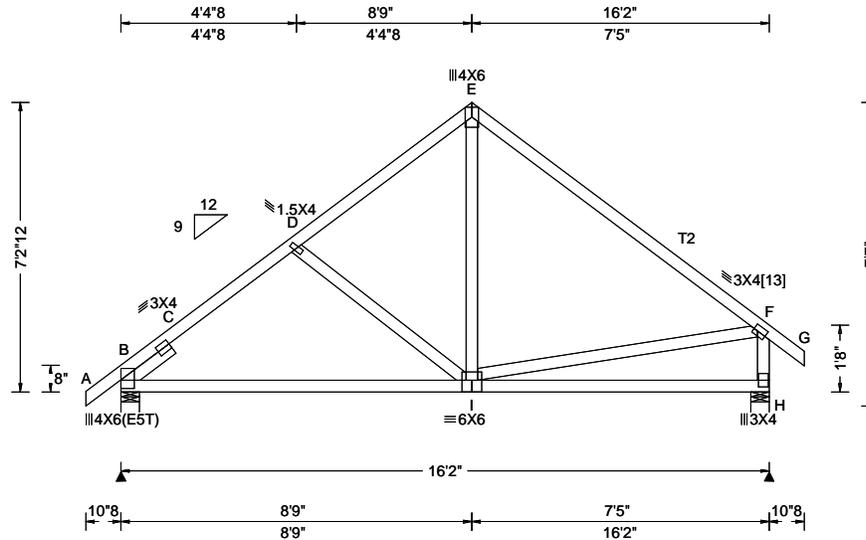
W = Width of non-hanger bearing, in inches.

Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

### **References:**

1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; [www.awc.org](http://www.awc.org).
2. ICC: International Code Council; [www.iccsafe.org](http://www.iccsafe.org).
3. Alpine, a division of ITW Building Components Group Inc.: 155 Harlem Ave, North Building, 4th Floor, Glenview, IL 60025; [www.alpineitw.com](http://www.alpineitw.com).
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; [www.tpinst.org](http://www.tpinst.org).
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; [www.sbcacomponents.com](http://www.sbcacomponents.com)



<b>Loading Criteria (psf)</b> TCLL: 25.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	<b>Wind Criteria</b> Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCDL: 6.0 psf BCDL: 6.0 psf Mean Height: 21.70 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.55 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.032 C 999 240 VERT(CL): 0.065 C 999 180 HORZ(LL): 0.025 C - - HORZ(TL): 0.050 C - - Creep Factor: 2.0 Max TC CSI: 0.576 Max BC CSI: 0.602 Max Web CSI: 0.188  VIEW Ver: 25.02.00B.1125.14	<b>▲ Maximum Reactions (lbs)</b> Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 835 /- /- /464 /46 /137 H 835 /- /- /453 /44 /- Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.5 (Support) H Brg Wid = 5.5 Min Req = 1.5 (Support) Bearings B & H Fcperp = 425psi. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp. B - C 416 -1036 D - E 194 -712 C - D 190 -914 E - F 158 -786
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**Lumber**  
Top chord: 2x4 SP #2; T2 2x4 SP 2400f-2.0E;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #2;  
Lt Slider: 2x4 SP #2; block length = 1.500'

**Additional Notes**  
Top Chord overhang(s) may be field trimmed.

**Plate Shift Table**

JT No	Plate Size	Lateral Shift	Chord Bite	JT No	Plate Size	Lateral Shift	Chord Bite
[13]	3X4	2.75	R 1.75				

**Purlins**  
In lieu of structural panels or rigid ceiling use purlins to laterally brace chords as follows:  
Chord Spacing(in oc) Start(ft) End(ft)  
BC 120 0.00 16.17  
Apply purlins to any chords above or below fillers at 24" OC unless shown otherwise above.

**Loading**  
Bottom chord checked for 10.00 psf non-concurrent live load.

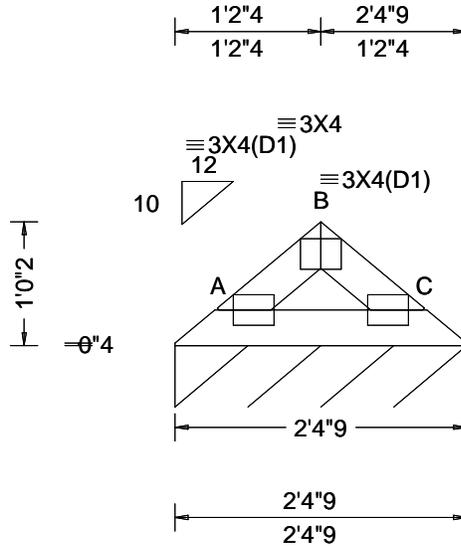
**Wind**  
Wind loads based on MWFRS with additional C&C member design.  
Right end vertical exposed to wind pressure.  
Deflection meets L/180.  
Wind loading based on both gable and hip roof types.

**Snow**  
Overhang designed for 2.00X TC LL.



**\*\*WARNING\*\* READ AND FOLLOW ALL NOTES ON THIS DRAWING!**  
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Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.  
For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org





Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 25.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 0.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCDL: 6.0 psf BCDL: 6.0 psf Mean Height: 23.26 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.55 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.001 C 999 240 VERT(CL): 0.001 C 999 180 HORZ(LL): -0.000 A - - HORZ(TL): 0.001 C - - Creep Factor: 2.0 Max TC CSI: 0.027 Max BC CSI: 0.043 Max Web CSI: 0.000  VIEW Ver: 25.02.00B.1125.14	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL C* 94 /- /- /44 /- /4 Wind reactions based on MWFRS C Brg Wid = 28.5 Min Req = - Bearing A Fcperp = 425psi. Members not listed have forces less than 375#

**Lumber**

Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;

**Purlins**

In lieu of structural panels or rigid ceiling use purlins to laterally brace chords as follows:

Chord	Spacing(in oc)	Start(ft)	End(ft)
BC	29	0.00	2.38

Apply purlins to any chords above or below fillers at 24" OC unless shown otherwise above.

**Wind**

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

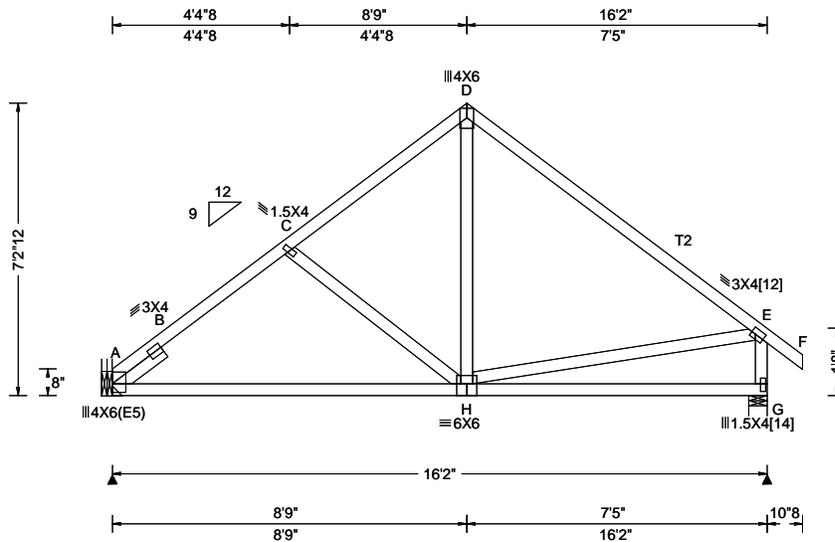
**Additional Notes**

See DWG VAL180160118 for valley details.



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<b>Loading Criteria (psf)</b> TCLL: 25.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	<b>Wind Criteria</b> Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCDL: 6.0 psf BCDL: 6.0 psf Mean Height: 22.03 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.55 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.032 B 999 240 VERT(CL): 0.065 B 999 180 HORZ(LL): 0.025 B - - HORZ(TL): 0.050 B - - Creep Factor: 2.0 Max TC CSI: 0.574 Max BC CSI: 0.603 Max Web CSI: 0.188  VIEW Ver: 25.02.00B.1125.14	<b>▲ Maximum Reactions (lbs)</b> Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 768 /- /- /450 /43 /126 G 835 /- /- /454 /47 /- Wind reactions based on MWFRS A Brg Wid = - Min Req = - G Brg Wid = 5.5 Min Req = 1.5 (Support) Bearing G Fcperp = 425psi. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp. A - B 442 -1028 C - D 199 -712 B - C 197 -914 D - E 163 -786
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**Lumber**  
Top chord: 2x4 SP #2; T2 2x4 SP 2400f-2.0E;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #2;  
Lt Slider: 2x4 SP #2; block length = 1.500'

**Additional Notes**  
Top Chord overhang(s) may be field trimmed.

**Maximum Bot Chord Forces Per Ply (lbs)**  
Chords Tens.Comp.  
A - H 696 -100

**Plate Shift Table**

JT No	Plate Size	Lateral Shift	Chord Bite	JT No	Plate Size	Lateral Shift	Chord Bite
[12]	3X4	2.75 R	1.75	[14]	1.5X4	S	2.25

**Maximum Web Forces Per Ply (lbs)**  
Webs Tens.Comp. Webs Tens. Comp.  
H - E 447 0 E - G 185 -779

**Purlins**  
In lieu of structural panels or rigid ceiling use purlins to laterally brace chords as follows:  
Chord Spacing(in oc) Start(ft) End(ft)  
BC 120 0.00 16.17  
Apply purlins to any chords above or below fillers at 24" OC unless shown otherwise above.

**Loading**  
Bottom chord checked for 10.00 psf non-concurrent live load.

**Wind**  
Wind loads based on MWFRS with additional C&C member design.  
Right end vertical exposed to wind pressure.  
Deflection meets L/180.  
Wind loading based on both gable and hip roof types.

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Overhang designed for 2.00X TC LL.



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For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org



SEQN: 5774 FROM: Page 2 of 2	COMN Ply: 1 Qty: 5	Job Number: PM000129 Customer - Clayton Properties Plan Name - Sienna Elevation - Farmhouse Truss Label: E3	Cust: R 9646 JRef: 1YGQ96460003 T12 DrwNo: 012.26.1516.29233 / BM 01/12/2026
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**Hangers / Ties**

Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.

Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

Bearing at location x=0' uses the following support conditions: 0'

- Bearing A (0', 18'1") LUS24
- Supporting Member: (2)2x8 SP #1
- (4) 0.148"x3" nails into supporting member,
- (2) 0.148"x3" nails into supported member.



MISSOURI COA #2005000817  
01/12/2026

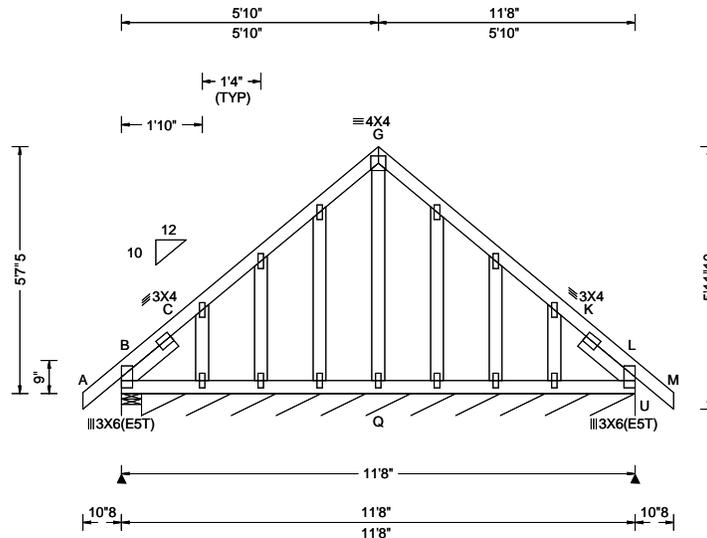
**\*\*WARNING\*\*** READ AND FOLLOW ALL NOTES ON THIS DRAWING!  
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**ALPINE**

North Building, 4th Floor  
Glenview, IL 60025

**RELEASE FOR CONSTRUCTION  
AS NOTED BY ANS REVIEW  
DEVELOPMENT SERVICES  
LEE'S SUMMIT, MISSOURI  
02/12/2026 4:05:15**

SEQN: 5776 FROM:	GABL Ply: 1 Qty: 1	Job Number: PM000129 Customer - Clayton Properties Plan Name - Sienna Elevation - Farmhouse Truss Label: G1	Cust: R 9646 JRef: 1YGQ96460003 T13 DrwNo: 012.26.1516.42683 / BM 01/12/2026
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<b>Loading Criteria (psf)</b> TCLL: 25.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	<b>Wind Criteria</b> Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCDL: 6.0 psf BCDL: 6.0 psf Mean Height: 20.90 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.55 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  <b>Building Code:</b> IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): -0.001 K 999 240 VERT(CL): 0.002 C 999 180 HORZ(LL): -0.002 C - - HORZ(TL): 0.005 K - - Creep Factor: 2.0 Max TC CSI: 0.113 Max BC CSI: 0.033 Max Web CSI: 0.095  VIEW Ver: 25.02.00B.1125.14	<b>▲ Maximum Reactions (lbs), or *=PLF</b> Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 242 /- /- /113 /18 /114 U* 125 /- /- /73 /16 /- Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.5 (Support) U Brg Wid = 134 Min Req = - Bearings B & B Fcperp = 425psi. Members not listed have forces less than 375#
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**Lumber**  
Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #2;  
Lt Slider: 2x4 SP #2; block length = 1.500'  
Rt Slider: 2x4 SP #2; block length = 1.500'

**Plating Notes**  
All plates are 1.5X4 except as noted.

**Purlins**  
In lieu of structural panels or rigid ceiling use purlins to laterally brace chords as follows:  
Chord Spacing(in oc) Start(ft) End(ft)  
BC 120 0.00 11.67  
Apply purlins to any chords above or below fillers at 24" OC unless shown otherwise above.

**Loading**  
Truss designed to support 1-4-0 top chord outlookers and cladding load not to exceed 3.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.  
Bottom chord checked for 10.00 psf non-concurrent live load.

**Wind**  
Wind loads based on MWFRS with additional C&C member design.  
Wind loading based on both gable and hip roof types.

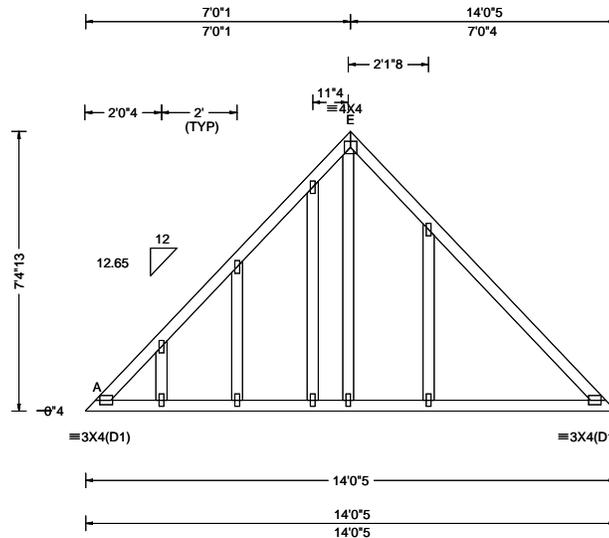
**Snow**  
Overhang designed for 2.00X TC LL.

**Additional Notes**  
See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.  
Top Chord overhang(s) may be field trimmed.



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<b>Loading Criteria (psf)</b> TCLL: 20.00 TCDL: 5.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 35.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	<b>Wind Criteria</b> Speed: 115 mph Risk Category: I Enclosure: Part. Enc. EXP: C Kzt: NA TCDL: 3.0 psf BCDL: 6.0 psf Mean Height: 0.00 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: NA GCpi: 0.55 Wind Duration: 0.00	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: 25.0 Ct: - CAT: - Pf(ASD): 16.8 Ce: - Lu: - Cs: - Snow Duration: -  <b>Building Code:</b> IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.000 - - 240 VERT(CL): 0.000 E 999 180 HORZ(LL): 0.000 - - - HORZ(TL): 0.000 F - - - Creep Factor: 2.0 Max TC CSI: 0.005 Max BC CSI: 0.003 Max Web CSI: 0.003  VIEW Ver: 25.02.00B.1125.14
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**Lumber**

Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #2;

**Plating Notes**

All plates are 1.5X4 except as noted.

**Additional Notes**

This "Hip Frame" may be used in place of purlins on the hip plane to brace the flat top chord of hip trusses. See detail drawing HIPFRAME0623, HIPFR18000623, or HIPFRSCAB0623 for additional information.

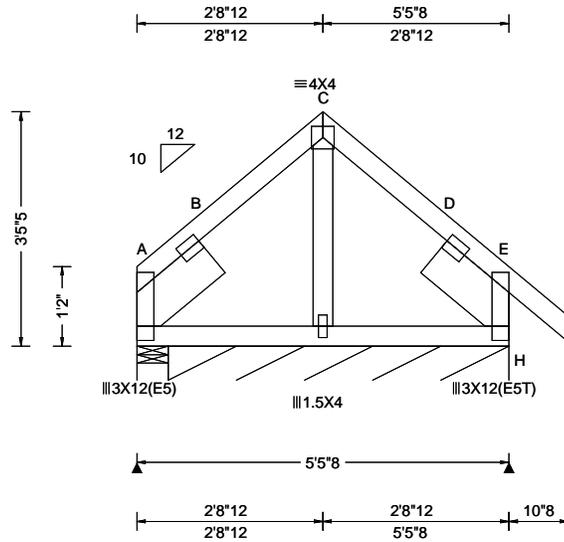


MISSOURI COA #2005000817  
01/12/2026

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SEQN: 5766 FROM:	GABL Ply: 1 Qty: 1	Job Number: PM000129 Customer - Clayton Properties Plan Name - Sienna Elevation - Farmhouse Truss Label: C1	Cust: R 9646 JRef: 1YQG96460003 T15 DrwNo: 012.26.1515.47140 / BM 01/12/2026
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<b>Loading Criteria (psf)</b> TCLL: 25.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	<b>Wind Criteria</b> Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCDL: 6.0 psf BCDL: 6.0 psf Mean Height: 15.00 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.55 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.007 B 999 240 VERT(CL): 0.011 B 999 180 HORZ(LL): 0.008 B - - HORZ(TL): 0.016 B - - Creep Factor: 2.0 Max TC CSI: 0.187 Max BC CSI: 0.079 Max Web CSI: 0.056  VIEW Ver: 25.02.00B.1125.14	<b>▲ Maximum Reactions (lbs), or *=PLF</b> Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 332 /- /- /185 /200 /59 H* 86 /- /- /43 /- /- Wind reactions based on MWFRS A Brg Wid = 5.5 Min Req = 1.5 (Support) H Brg Wid = 60.0 Min Req = - Bearings A & A Fcperp = 425psi. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. D - E 392 -346
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**Lumber**  
Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #2;  
Lt Slider: 2x8 SP #1; block length = 1.500'  
Rt Slider: 2x8 SP #1; block length = 1.500'

**Plating Notes**  
All plates are 3X4 except as noted.

**Purlins**  
In lieu of structural panels or rigid ceiling use purlins to laterally brace chords as follows:  
Chord Spacing(in oc) Start(ft) End(ft)  
BC 65 0.00 5.46  
Apply purlins to any chords above or below fillers at 24" OC unless shown otherwise above.

**Loading**  
Truss designed to support 1-4-0 top chord outlookers and cladding load not to exceed 3.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.  
Bottom chord checked for 10.00 psf non-concurrent live load.

**Wind**  
Wind loads based on MWFRS with additional C&C member design.  
Wind loading based on both gable and hip roof types.

**Snow**  
Overhang designed for 2.00X TC LL.

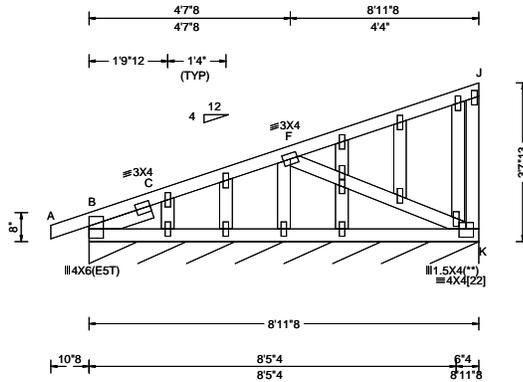
**Additional Notes**  
See DWGS A14015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.  
Top Chord overhang(s) may be field trimmed.



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SEQN: 5798 FROM:	GABL Ply: 1 Qty: 1	Job Number: PM000129 Customer - Clayton Properties Plan Name - Sienna Elevation - Farmhouse Truss Label: D3	Cust: R 9646 JRef: 1YQG96460003 T16 DrwNo: 012.26.1515.52747 / BM 01/12/2026
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<b>Loading Criteria (psf)</b> TCLL: 25.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	<b>Wind Criteria</b> Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCDL: 6.0 psf BCDL: 6.0 psf Mean Height: 15.00 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.55 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  <b>Building Code:</b> IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.008 H 999 240 VERT(CL): 0.012 H 999 180 HORZ(LL): 0.002 H - - HORZ(TL): 0.004 H - - Creep Factor: 2.0 Max TC CSI: 0.135 Max BC CSI: 0.061 Max Web CSI: 0.192  VIEW Ver: 25.02.00B.1125.14	<b>▲ Maximum Reactions (lbs), or *=PLF</b> <table border="1"> <tr> <th colspan="2">Gravity</th> <th colspan="4">Non-Gravity</th> </tr> <tr> <th>Loc</th> <th>R+ / R-</th> <th>/ Rh</th> <th>/ Rw</th> <th>/ U</th> <th>/ RL</th> </tr> <tr> <td>K*</td> <td>125</td> <td>-</td> <td>-</td> <td>/64</td> <td>/16 /18</td> </tr> </table> Wind reactions based on MWFRS K Brg Wid = 107 Min Req = - Bearing B Fcperp = 425psi. Members not listed have forces less than 375#	Gravity		Non-Gravity				Loc	R+ / R-	/ Rh	/ Rw	/ U	/ RL	K*	125	-	-	/64	/16 /18
Gravity		Non-Gravity																				
Loc	R+ / R-	/ Rh	/ Rw	/ U	/ RL																	
K*	125	-	-	/64	/16 /18																	

**Lumber**  
 Top chord: 2x4 SP #2;  
 Bot chord: 2x4 SP #2;  
 Webs: 2x4 SP #2;  
 Lt Slider: 2x4 SP #2; block length = 1.500'

**Plating Notes**  
 All plates are 1.5X4 except as noted.  
 (\*\*) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

**Plate Shift Table**

JT No	Plate Size	Lateral Shift	Chord Bite	JT No	Plate Size	Lateral Shift	Chord Bite
[22]	4X4	S	2.25				

**Wind**  
 Wind loads based on MWFRS with additional C&C member design.  
 Right end vertical exposed to wind pressure. Deflection meets L/180.  
 Wind loading based on both gable and hip roof types.

**Snow**  
 Overhang designed for 2.00X TC LL.

**Additional Notes**  
 See DWGS A14015ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.  
 Top Chord overhang(s) may be field trimmed.

**Purlins**  
 In lieu of structural panels or rigid ceiling use purlins to laterally brace chords as follows:  

Chord	Spacing(in oc)	Start(ft)	End(ft)
BC	108	0.00	8.96

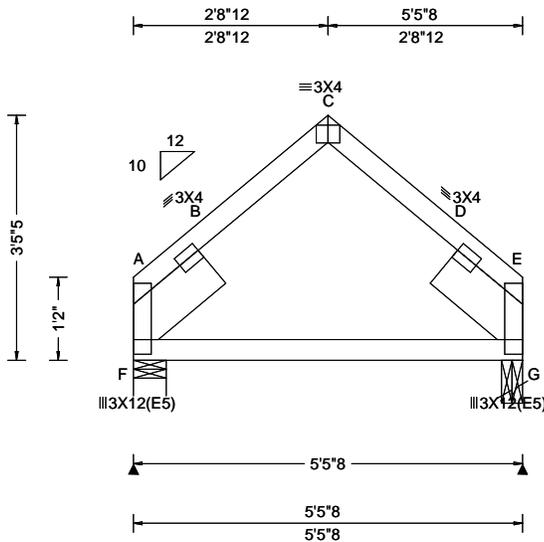
 Apply purlins to any chords above or below fillers at 24" OC unless shown otherwise above.

**Loading**  
 Truss designed to support 1-4-0 top chord outlookers and cladding load not to exceed 3.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.  
 Bottom chord checked for 10.00 psf non-concurrent live load.



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<b>Loading Criteria (psf)</b> TCLL: 25.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	<b>Wind Criteria</b> Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCDL: 6.0 psf BCDL: 6.0 psf Mean Height: 15.00 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.55 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.003 B 999 240 VERT(CL): 0.008 B 999 180 HORZ(LL): 0.003 B - - HORZ(TL): 0.011 B - - Creep Factor: 2.0 Max TC CSI: 0.105 Max BC CSI: 0.194 Max Web CSI: 0.010  VIEW Ver: 25.02.00B.1125.14	<b>▲ Maximum Reactions (lbs)</b> Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL F 262 /- /- /150 /- /35 G 262 /- /- /150 /- /- Wind reactions based on MWFRS F Brg Wid = 5.5 Min Req = 1.5 (Support) G Brg Wid = 3.5 Min Req = 1.5 (Support) Bearings F & G Fcperp = 425psi. Members not listed have forces less than 375#
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**Lumber**

Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Lt Slider: 2x8 SP #1; block length = 1.500'  
Rt Slider: 2x8 SP #1; block length = 1.500'

**Purlins**

In lieu of structural panels or rigid ceiling use purlins to laterally brace chords as follows:  
Chord Spacing(in oc) Start(ft) End(ft)  
BC 66 0.00 5.46  
Apply purlins to any chords above or below fillers at 24" OC unless shown otherwise above.

**Loading**

Bottom chord checked for 10.00 psf non-concurrent live load.

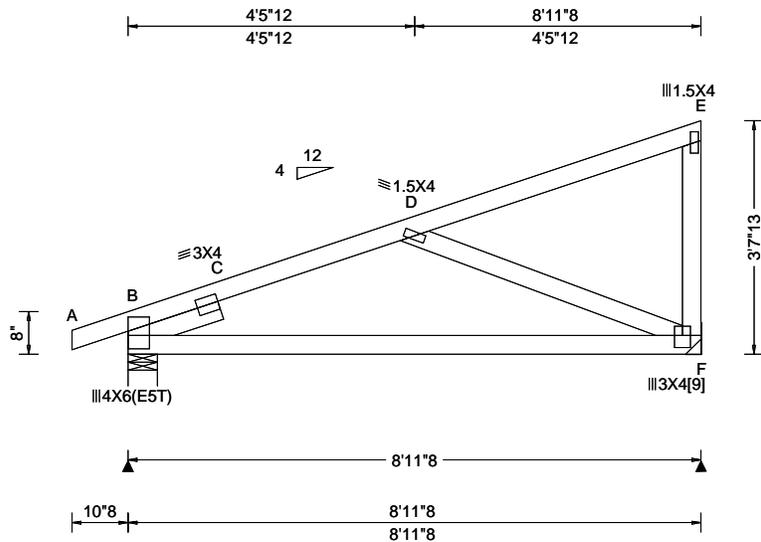
**Wind**

Wind loads based on MWFRS with additional C&C member design.  
Wind loading based on both gable and hip roof types.



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<b>Loading Criteria (psf)</b> TCLL: 25.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	<b>Wind Criteria</b> Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCDL: 6.0 psf BCDL: 6.0 psf Mean Height: 15.00 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.55 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.024 C 999 240 VERT(CL): 0.055 C 999 180 HORZ(LL): 0.009 C - - HORZ(TL): 0.023 C - - Creep Factor: 2.0 Max TC CSI: 0.458 Max BC CSI: 0.525 Max Web CSI: 0.272  VIEW Ver: 25.02.00B.1125.14	<b>▲ Maximum Reactions (lbs)</b> Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 473 /- /- /231 /- /82 F 408 /- /- /234 /21 /- Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.5 (Support) F Brg Wid = - Min Req = - Bearing B Fcperp = 425psi. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp. B - C 511 -649 C - D 225 -571
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**Lumber**  
Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #2;  
Lt Slider: 2x4 SP #2; block length = 1.500'

**Plate Shift Table**

JT No	Plate Size	Lateral Shift	Chord Bite	JT No	Plate Size	Lateral Shift	Chord Bite
[ 9 ]	3X4	S	2.25				

**Snow**  
Overhang designed for 2.00X TC LL.

**Additional Notes**  
Top Chord overhang(s) may be field trimmed.

**Maximum Bot Chord Forces Per Ply (lbs)**  
Chords Tens.Comp.  
B - F 534 -346

**Maximum Web Forces Per Ply (lbs)**  
Webs Tens.Comp.  
D - F 375 -558

**Purlins**  
In lieu of structural panels or rigid ceiling use purlins to laterally brace chords as follows:  
Chord Spacing(in oc) Start(ft) End(ft)  
BC 108 0.00 8.96  
Apply purlins to any chords above or below fillers at 24" OC unless shown otherwise above.

**Hangers / Ties**  
(J) Hanger Support Required, by others

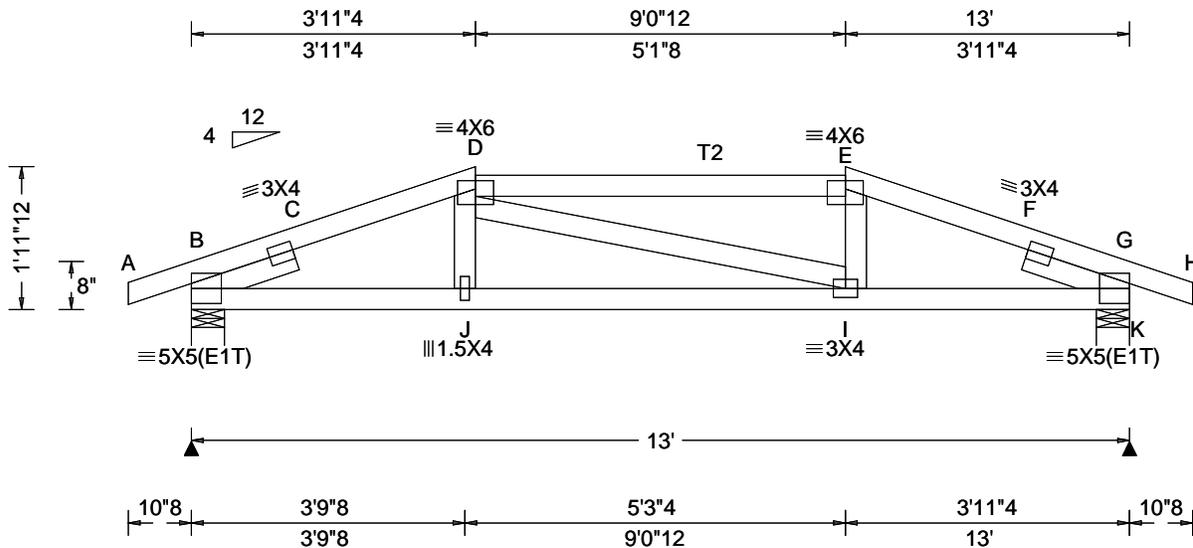
**Loading**  
Bottom chord checked for 10.00 psf non-concurrent live load.

**Wind**  
Wind loads based on MWFRS with additional C&C member design.  
Right end vertical not exposed to wind pressure.  
Wind loading based on both gable and hip roof types.



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<b>Loading Criteria (psf)</b> TCLL: 25.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 0.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	<b>Wind Criteria</b> Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCDL: 6.0 psf BCDL: 6.0 psf Mean Height: 15.00 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.55 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.050 D 999 240 VERT(CL): 0.091 D 999 180 HORZ(LL): 0.017 G - - HORZ(TL): 0.031 G - - Creep Factor: 2.0 Max TC CSI: 0.469 Max BC CSI: 0.886 Max Web CSI: 0.048  VIEW Ver: 25.02.00B.1125.14	<b>▲ Maximum Reactions (lbs)</b> Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL B 949 /- /- /- /40 /- K 949 /- /- /- /40 /- Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.5 (Support) K Brg Wid = 5.5 Min Req = 1.5 (Support) Bearings B & K Fcperp = 425psi. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp. B - C 61 -1918 E - F 60 -1886 C - D 58 -1885 F - G 63 -1918 D - E 57 -1775  <b>Maximum Bot Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp. B - J 1779 -55 I - G 1780 -58 J - I 1756 -61
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**Lumber**  
Top chord: 2x4 SP #2; T2 2x4 SP 2400f-2.0E;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #2;  
Lt Slider: 2x4 SP #2; block length = 1.500'  
Rt Slider: 2x4 SP #2; block length = 1.500'

**Additional Notes**  
Top Chord overhang(s) may be field trimmed.

**Special Loads**  
-----(Lumber Dur.Fac.=1.15 / Plate Dur.Fac.=1.15)  
TC: From 71 plf at -0.88 to 71 plf at 0.00  
TC: From 36 plf at 0.00 to 36 plf at 13.00  
TC: From 71 plf at 13.00 to 71 plf at 13.88  
BC: From 10 plf at 0.00 to 10 plf at 13.00  
TC: 132 lb Conc. Load at 4.00, 6.00, 7.00, 9.00  
BC: 230 lb Conc. Load at 2.00, 11.00  
BC: 47 lb Conc. Load at 4.00, 6.00, 7.00, 9.00

**Purlins**  
In lieu of structural panels or rigid ceiling use purlins to laterally brace chords as follows:  
Chord Spacing(in oc) Start(ft) End(ft)  
TC 24 3.94 9.06  
BC 120 0.00 13.00  
Apply purlins to any chords above or below fillers at 24" OC unless shown otherwise above.

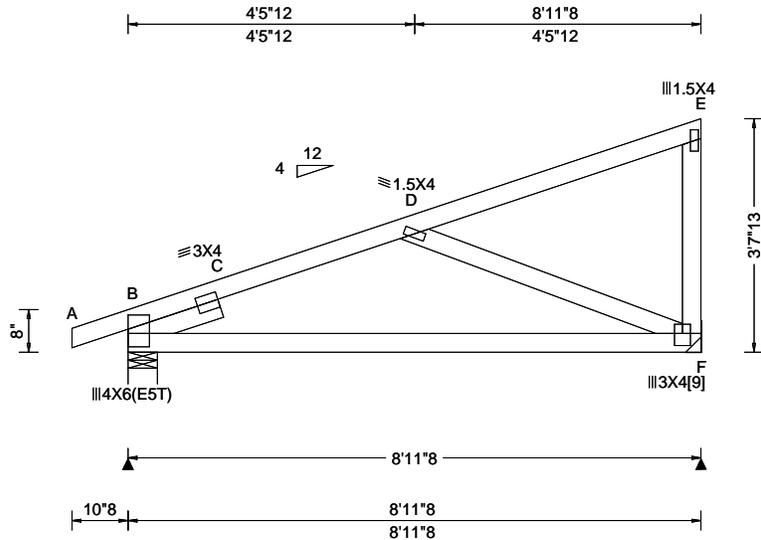
**Wind**  
Wind loads and reactions based on MWFRS.  
Wind loading based on both gable and hip roof types.

**Snow**  
Overhang designed for 2.00X TC LL.



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<b>Loading Criteria (psf)</b> TCLL: 25.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	<b>Wind Criteria</b> Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCDL: 6.0 psf BCDL: 6.0 psf Mean Height: 15.00 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.55 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.024 C 999 240 VERT(CL): 0.055 C 999 180 HORZ(LL): 0.009 C - - HORZ(TL): 0.023 C - - Creep Factor: 2.0 Max TC CSI: 0.458 Max BC CSI: 0.525 Max Web CSI: 0.272  VIEW Ver: 25.02.00B.1125.14	<b>▲ Maximum Reactions (lbs)</b> Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 473 /- /- /231 /- /82 F 408 /- /- /234 /21 /- Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.5 (Support) F Brg Wid = - Min Req = - Bearing B Fcperp = 425psi. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp. B - C 445 -649 C - D 156 -571
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**Lumber**  
Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #2;  
Lt Slider: 2x4 SP #2; block length = 1.500'

**Plate Shift Table**

JT No	Plate Size	Lateral Shift	Chord Bite	JT No	Plate Size	Lateral Shift	Chord Bite
[ 9 ]	3X4	S	2.25				

**Snow**  
Overhang designed for 2.00X TC LL.

**Additional Notes**  
Top Chord overhang(s) may be field trimmed.

**Maximum Bot Chord Forces Per Ply (lbs)**  
Chords Tens.Comp.  
B - F 534 -255

**Maximum Web Forces Per Ply (lbs)**  
Webs Tens.Comp.  
D - F 277 -558

**Purlins**  
In lieu of structural panels or rigid ceiling use purlins to laterally brace chords as follows:  
Chord Spacing(in oc) Start(ft) End(ft)  
BC 108 0.00 8.96  
Apply purlins to any chords above or below fillers at 24" OC unless shown otherwise above.

**Hangers / Ties**  
(J) Hanger Support Required, by others

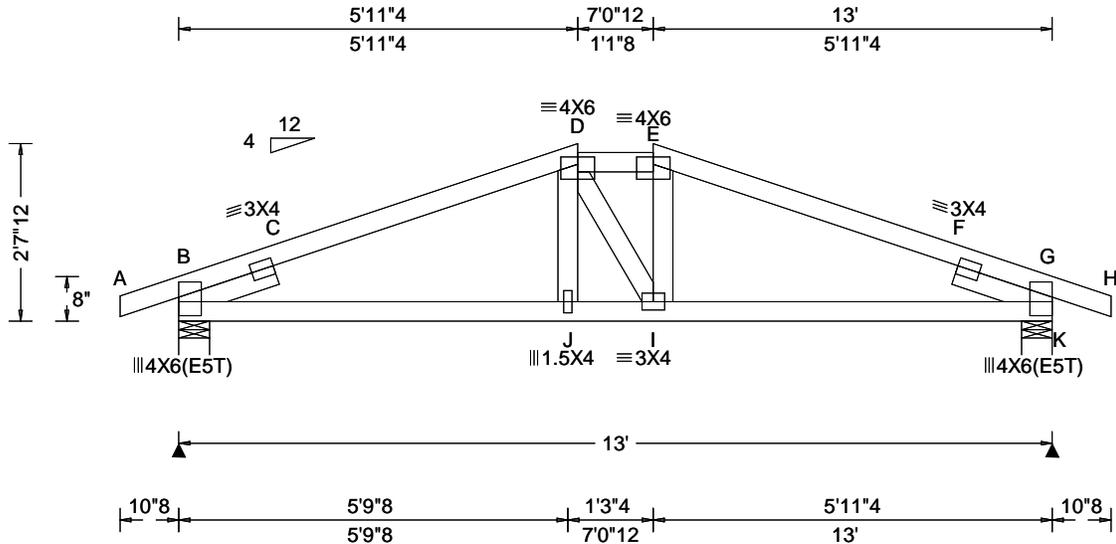
**Loading**  
Bottom chord checked for 10.00 psf non-concurrent live load.

**Wind**  
Wind loads based on MWFRS with additional C&C member design.  
Right end vertical not exposed to wind pressure.  
Wind loading based on both gable and hip roof types.



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<b>Loading Criteria (psf)</b> TCLL: 25.00 TC DL: 10.00 BC LL: 0.00 BC DL: 10.00 Des Ld: 45.00 NCBC LL: 0.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	<b>Wind Criteria</b> Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TC DL: 6.0 psf BC DL: 6.0 psf Mean Height: 15.00 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.55 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.071 F 999 240 VERT(CL): 0.109 F 999 180 HORZ(LL): 0.027 C - - HORZ(TL): 0.041 C - - Creep Factor: 2.0 Max TC CSI: 0.528 Max BC CSI: 0.448 Max Web CSI: 0.139 VIEW Ver: 25.02.00B.1125.14	<b>▲ Maximum Reactions (lbs)</b> <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/ R-</th> <th>/ Rh</th> <th>/ Rw</th> <th>/ U</th> <th>/ RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>656</td> <td>-</td> <td>-</td> <td>/333</td> <td>/10</td> <td>/16</td> </tr> <tr> <td>K</td> <td>656</td> <td>-</td> <td>-</td> <td>/333</td> <td>/10</td> <td>-</td> </tr> </tbody> </table>						Loc	Gravity			Non-Gravity			R+	/ R-	/ Rh	/ Rw	/ U	/ RL	B	656	-	-	/333	/10	/16	K	656	-	-	/333	/10	-
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**Lumber**

Top chord: 2x4 SP #2;  
 Bot chord: 2x4 SP #2;  
 Webs: 2x4 SP #2;  
 Lt Slider: 2x4 SP #2; block length = 1.500'  
 Rt Slider: 2x4 SP #2; block length = 1.500'

**Purlins**

In lieu of structural panels or rigid ceiling use purlins to laterally brace chords as follows:

Chord	Spacing(in oc)	Start(ft)	End(ft)
TC	24	5.94	7.06
BC	120	0.00	13.00

Apply purlins to any chords above or below fillers at 24" OC unless shown otherwise above.

**Wind**

Wind loads based on MWFRS with additional C&C member design.  
 Wind loading based on both gable and hip roof types.

**Snow**

Overhang designed for 2.00X TC LL.

**Additional Notes**

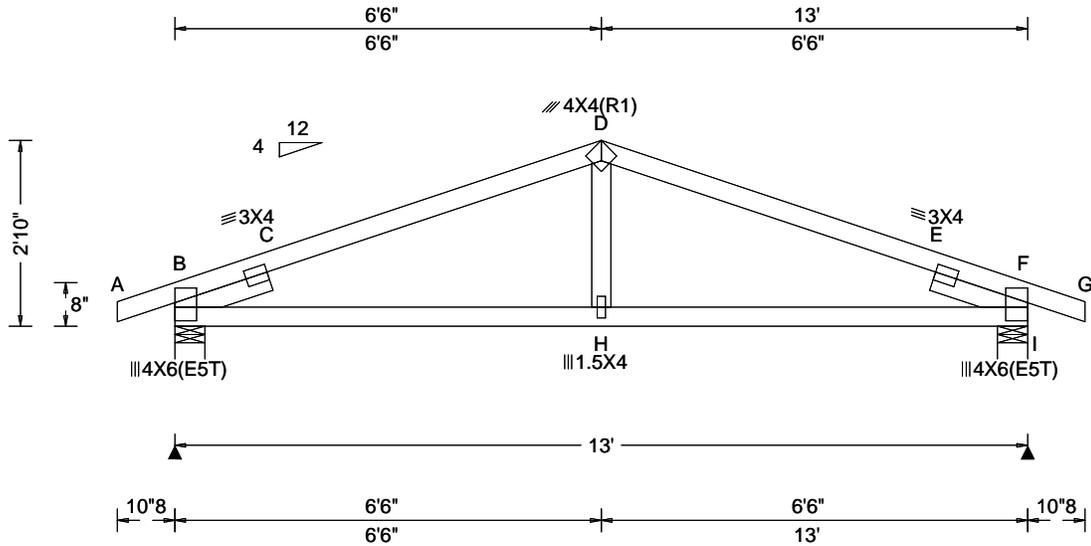
Top Chord overhang(s) may be field trimmed.



MISSOURI COA #2005000817  
 01/12/2026

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 LEE'S SUMMIT, MISSOURI  
 02/12/2026 4:05:16



<b>Loading Criteria (psf)</b> TCLL: 25.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	<b>Wind Criteria</b> Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCDL: 6.0 psf BCDL: 6.0 psf Mean Height: 15.00 ft MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.55 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.077 C 999 240 VERT(CL): 0.119 C 999 180 HORZ(LL): 0.030 C - - HORZ(TL): 0.046 C - - Creep Factor: 2.0 Max TC CSI: 0.750 Max BC CSI: 0.539 Max Web CSI: 0.143  VIEW Ver: 25.02.00B.1125.14	<b>▲ Maximum Reactions (lbs)</b> Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 656 /- /- /334 /10 /17 I 656 /- /- /334 /10 /- Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.5 (Support) I Brg Wid = 5.5 Min Req = 1.5 (Support) Bearings B & I Fcperp = 425psi. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp. B - C 387 -1366 D - E 183 -1004 C - D 183 -1004 E - F 387 -1366  <b>Maximum Bot Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp. B - H 912 -112 H - F 912 -112
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**Lumber**

Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #2;  
Lt Slider: 2x4 SP #2; block length = 1.500'  
Rt Slider: 2x4 SP #2; block length = 1.500'

**Purlins**

In lieu of structural panels or rigid ceiling use purlins to laterally brace chords as follows:  
Chord Spacing(in oc) Start(ft) End(ft)  
BC 120 0.00 13.00  
Apply purlins to any chords above or below fillers at 24" OC unless shown otherwise above.

**Loading**

Bottom chord checked for 10.00 psf non-concurrent live load.

**Wind**

Wind loads based on MWFRS with additional C&C member design.  
Wind loading based on both gable and hip roof types.

**Snow**

Overhang designed for 2.00X TC LL.

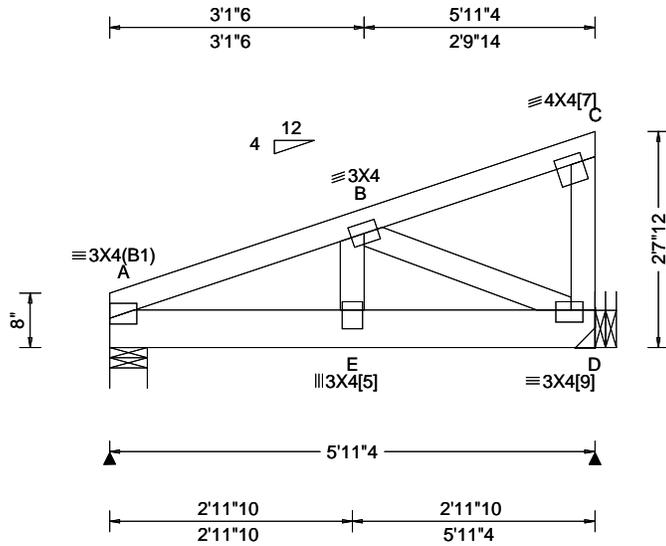
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<b>Loading Criteria (psf)</b>	<b>Wind Criteria</b>	<b>Snow Criteria (Pg,Pf in PSF)</b>	<b>Defl/CSI Criteria</b>	<b>▲ Maximum Reactions (lbs)</b>
TCLL: 25.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 0.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCDL: 6.0 psf BCDL: 6.0 psf Mean Height: 15.00 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.55 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.018 E 999 240 VERT(CL): 0.032 E 999 180 HORZ(LL): -0.005 C - - HORZ(TL): 0.009 C - - Creep Factor: 2.0 Max TC CSI: 0.320 Max BC CSI: 0.593 Max Web CSI: 0.225  VIEW Ver: 25.02.00B.1125.14	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 860 /- /- /- /31 /- D 805 /- /- /- /29 /- Wind reactions based on MWFRS A Brg Wid = 5.5 Min Req = 1.5 (Support) D Brg Wid = - Min Req = - Bearing A Fcperp = 425psi. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. A - B 45 - 1303

**Lumber**

Top chord: 2x4 SP #2;  
Bot chord: 2x6 SP #2;  
Webs: 2x4 SP #2;

**Special Loads**

----(Lumber Dur.Fac.=1.15 / Plate Dur.Fac.=1.15)  
TC: From 71 plf at 0.00 to 71 plf at 5.94  
BC: From 10 plf at 0.00 to 10 plf at 5.94  
BC: 592 lb Conc. Load at 1.94, 3.94

**Plate Shift Table**

JT No	Plate Size	Lateral Shift	Chord Bite	JT No	Plate Size	Lateral Shift	Chord Bite
[5]	3X4	S	2.75	[7]	4X4	1.92	L 1.25
[9]	3X4	R	1.75				

**Purlins**

In lieu of structural panels or rigid ceiling use purlins to laterally brace chords as follows:

Chord	Spacing(in oc)	Start(ft)	End(ft)
BC	68	0.30	5.94

Apply purlins to any chords above or below fillers at 24" OC unless shown otherwise above.

**Wind**

Wind loads and reactions based on MWFRS.  
Right end vertical not exposed to wind pressure.  
Wind loading based on both gable and hip roof types.



MISSOURI COA #2005000817  
01/12/2026

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SEQN: 5783 FROM: Page 2 of 2	MONO Ply: 1 Qty: 1	Job Number: PM000129 Customer - Clayton Properties Plan Name - Sienna Elevation - Farmhouse Truss Label: J6	Cust: R 9646 JRef: 1YGQ96460003 T22 DrwNo: 012.26.1517.25800 / BM 01/12/2026
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**Hangers / Ties**

Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.

Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

Bearing at location x=5'8"4 uses the following support conditions: 5'8"4

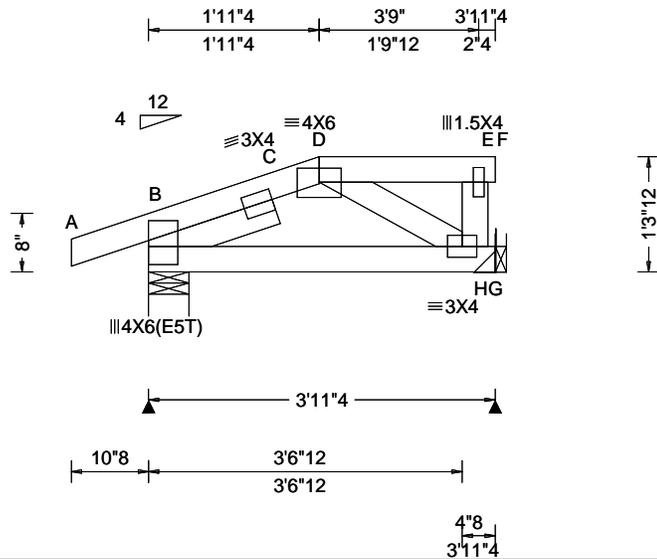
- Bearing D (5'8"4, 9'1"2) LUS24
- Supporting Member: (2)2x6 SP #2
- (4) 0.148"x3" nails into supporting member,
- (2) 0.148"x3" nails into supported member.



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**ALPINE**

RELEASE FOR CONSTRUCTION  
 AS NOTED BY ANS REVIEW  
 DEVELOPMENT SERVICES  
 LEE'S SUMMIT, MISSOURI  
 02/12/2026 4:05:16



<b>Loading Criteria (psf)</b>	<b>Wind Criteria</b>	<b>Snow Criteria (Pg,Pf in PSF)</b>	<b>Defl/CSI Criteria</b>	<b>▲ Maximum Reactions (lbs)</b>
TCLL: 25.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 0.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCDL: 6.0 psf BCDL: 6.0 psf Mean Height: 15.00 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.55 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.003 C 999 240 VERT(CL): 0.006 C 999 180 HORZ(LL): 0.001 C - - HORZ(TL): 0.002 C - - Creep Factor: 2.0 Max TC CSI: 0.100 Max BC CSI: 0.188 Max Web CSI: 0.033  VIEW Ver: 25.02.00B.1125.14	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity B 291 /- /- /- /17 /- G 230 /- /- /- /2 /- Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.5 (Support) G Brg Wid = - Min Req = - Bearing B Fcperp = 425psi. Members not listed have forces less than 375#

**Lumber**  
Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #2;  
Lt Slider: 2x4 SP #2; block length = 1.500'

**Special Loads**  
-----(Lumber Dur.Fac.=1.15 / Plate Dur.Fac.=1.15)  
TC: From 71 plf at -0.88 to 71 plf at 3.94  
BC: From 20 plf at 0.00 to 20 plf at 3.94  
TC: 71 lb Conc. Load at 1.95  
BC: 23 lb Conc. Load at 1.95

**Purlins**  
In lieu of structural panels or rigid ceiling use purlins to laterally brace chords as follows:  
Chord Spacing(in oc) Start(ft) End(ft)  
TC 24 1.94 3.94  
BC 47 0.00 3.94  
Apply purlins to any chords above or below fillers at 24" OC unless shown otherwise above.

**Wind**  
Wind loads and reactions based on MWFRS.  
Right end vertical not exposed to wind pressure.  
Wind loading based on both gable and hip roof types.

**Snow**  
Overhang designed for 2.00X TC LL.

**Additional Notes**  
Top Chord overhang(s) may be field trimmed.



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SEQN: 5780 FROM: Page 2 of 2	NEJA Ply: 1 Qty: 2	Job Number: PM000129 Customer - Clayton Properties Plan Name - Sienna Elevation - Farmhouse Truss Label: J3	Cust: R 9646 JRef: 1YQG96460003 T23 DrwNo: 012.26.1517.08460 / BM 01/12/2026
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**Hangers / Ties**

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Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

Bearing at location  $x=3'8"4$  , $y=9'1"2$  uses the following support conditions: 3'8"4

- Bearing H (3'8"4, 9'1"2) LUS24
- Supporting Member: (1)2x4 SP #2
- (4) 0.148"x3" nails into supporting member,
- (2) 0.148"x3" nails into supported member.

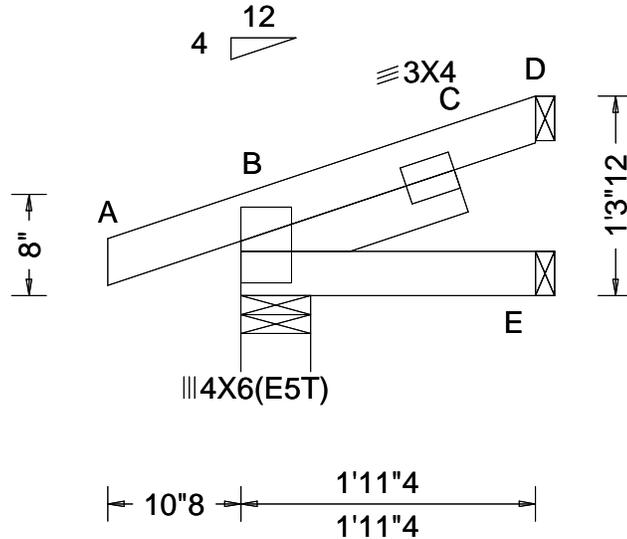


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**ALPINE**

North Building, 4th Floor  
Glenview, IL 60025

**RELEASE FOR CONSTRUCTION  
 AS NOTED BY ANS REVIEW  
 DEVELOPMENT SERVICES  
 LEE'S SUMMIT, MISSOURI  
 02/12/2026 4:05:16**



<b>Loading Criteria (psf)</b> TCLL: 25.00 TC DL: 10.00 BC LL: 0.00 BC DL: 10.00 Des Ld: 45.00 NCBC LL: 0.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	<b>Wind Criteria</b> Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TC DL: 6.0 psf BC DL: 6.0 psf Mean Height: 15.00 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.55 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  <b>Building Code:</b> IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.001 C - - HORZ(TL): 0.002 C - - Creep Factor: 2.0 Max TC CSI: 0.100 Max BC CSI: 0.024 Max Web CSI: 0.008  VIEW Ver: 25.02.00B.1125.14	<b>▲ Maximum Reactions (lbs)</b> <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>170</td> <td>/-</td> <td>/-</td> <td>/65</td> <td>/-</td> <td>/21</td> </tr> <tr> <td>E</td> <td>20</td> <td>/-</td> <td>/-</td> <td>/19</td> <td>/-</td> <td>/-</td> </tr> <tr> <td>D</td> <td>68</td> <td>/-2</td> <td>/-</td> <td>/31</td> <td>/16</td> <td>/-</td> </tr> </tbody> </table> Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.5 (Support) E Brg Wid = 1.5 Min Req = - D Brg Wid = 1.5 Min Req = - Bearing B Fcperp = 425psi. Members not listed have forces less than 375#	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	B	170	/-	/-	/65	/-	/21	E	20	/-	/-	/19	/-	/-	D	68	/-2	/-	/31	/16	/-
				Loc		Gravity			Non-Gravity																													
R+	/R-	/Rh	/Rw		/U	/RL																																
B	170	/-	/-	/65	/-	/21																																
E	20	/-	/-	/19	/-	/-																																
D	68	/-2	/-	/31	/16	/-																																
<b>Lumber</b> Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Lt Slider: 2x4 SP #2; block length = 1.500'																																						

**Purlins**

In lieu of structural panels or rigid ceiling use purlins to laterally brace chords as follows:

Chord	Spacing(in oc)	Start(ft)	End(ft)
BC	23	0.00	1.94

Apply purlins to any chords above or below fillers at 24" OC unless shown otherwise above.

**Wind**

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

**Snow**

Overhang designed for 2.00X TC LL.

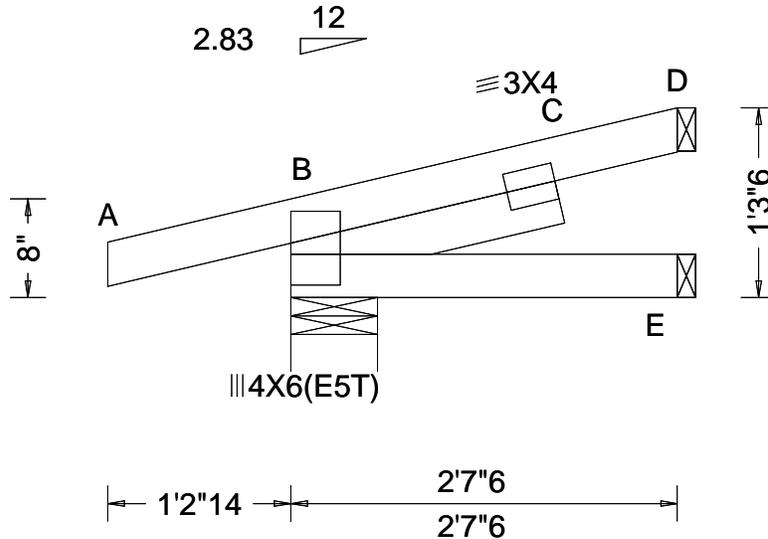
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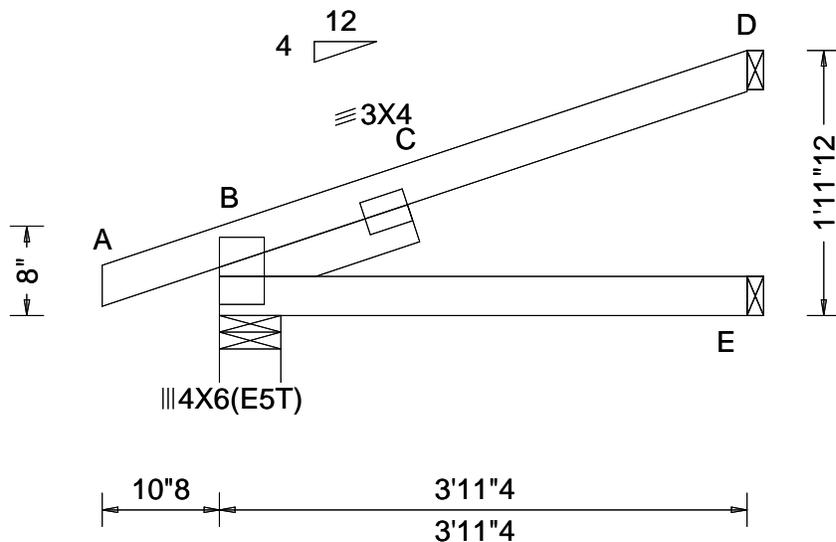


<b>Loading Criteria (psf)</b> TCCL: 25.00 TCCL: 10.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCCL: 0.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	<b>Wind Criteria</b> Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCCL: 6.0 psf BCDL: 6.0 psf Mean Height: 15.00 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.55 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  <b>Building Code:</b> IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): -0.000 C - - HORZ(TL): 0.000 C - - Creep Factor: 2.0 Max TC CSI: 0.039 Max BC CSI: 0.004 Max Web CSI: 0.003  VIEW Ver: 25.02.00B.1125.14	<b>▲ Maximum Reactions (lbs)</b> <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>56</td> <td>/-</td> <td>/-</td> <td>/-</td> <td>/7</td> <td>/-</td> </tr> <tr> <td>E</td> <td>2</td> <td>/-</td> <td>/-</td> <td>/2</td> <td>/-</td> <td>/-</td> </tr> <tr> <td>D</td> <td>3</td> <td>/-4</td> <td>/-</td> <td>/3</td> <td>/-</td> <td>/-</td> </tr> </tbody> </table> Wind reactions based on MWFRS B Brg Wid = 7.0 Min Req = 1.5 (Support) E Brg Wid = 1.5 Min Req = - D Brg Wid = 1.5 Min Req = - Bearing B Fcperp = 425psi. Members not listed have forces less than 375#	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	B	56	/-	/-	/-	/7	/-	E	2	/-	/-	/2	/-	/-	D	3	/-4	/-	/3	/-	/-
				Loc		Gravity			Non-Gravity																													
R+	/R-	/Rh	/Rw		/U	/RL																																
B	56	/-	/-	/-	/7	/-																																
E	2	/-	/-	/2	/-	/-																																
D	3	/-4	/-	/3	/-	/-																																
<b>Lumber</b> Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Lt Slider: 2x4 SP #2; block length = 1.853'  <b>Purlins</b> In lieu of structural panels or rigid ceiling use purlins to laterally brace chords as follows: <table border="1"> <thead> <tr> <th>Chord</th> <th>Spacing(in oc)</th> <th>Start(ft)</th> <th>End(ft)</th> </tr> </thead> <tbody> <tr> <td>BC</td> <td>31</td> <td>0.00</td> <td>2.62</td> </tr> </tbody> </table> Apply purlins to any chords above or below fillers at 24" OC unless shown otherwise above.  <b>Wind</b> Wind loads and reactions based on MWFRS. Wind loading based on both gable and hip roof types.  <b>Additional Notes</b> Top Chord overhang(s) may be field trimmed.					Chord	Spacing(in oc)	Start(ft)	End(ft)	BC	31	0.00	2.62																										
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<b>Loading Criteria (psf)</b> TCLL: 25.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 0.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	<b>Wind Criteria</b> Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCDL: 6.0 psf BCDL: 6.0 psf Mean Height: 15.00 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.55 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.013 C - - HORZ(TL): 0.020 C - - Creep Factor: 2.0 Max TC CSI: 0.322 Max BC CSI: 0.157 Max Web CSI: 0.077  VIEW Ver: 25.02.00B.1125.14	<b>▲ Maximum Reactions (lbs)</b> Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity B 248 /- /- /111 /- /38 E 47 /- /- /41 /- /- D 132 /- /- /62 /29 /- Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.5 (Support) E Brg Wid = 1.5 Min Req = - D Brg Wid = 1.5 Min Req = - Bearing B Fcperp = 425psi. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. B - C 333 -398
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**Lumber**  
Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Lt Slider: 2x4 SP #2; block length = 1.500'

**Purlins**  
In lieu of structural panels or rigid ceiling use purlins to laterally brace chords as follows:  
Chord Spacing(in oc) Start(ft) End(ft)  
BC 47 0.00 3.94  
Apply purlins to any chords above or below fillers at 24" OC unless shown otherwise above.

**Wind**  
Wind loads based on MWFRS with additional C&C member design.  
Wind loading based on both gable and hip roof types.

**Snow**  
Overhang designed for 2.00X TC LL.

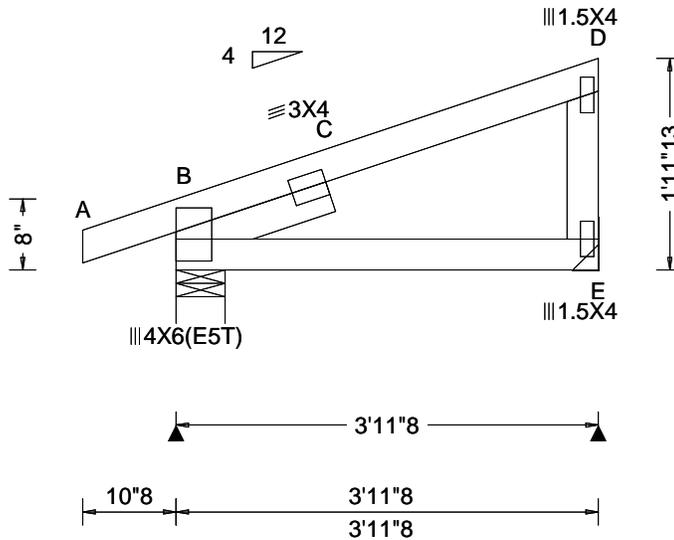
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SEQN: 5769 FROM:	MONO Ply: 1 Qty: 5	Job Number: PM000129 Customer - Clayton Properties Plan Name - Sienna Elevation - Farmhouse Truss Label: D2	Cust: R 9646 JRef: 1YQG96460003 T27 DrwNo: 012.26.1515.51217 / BM 01/12/2026
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<b>Loading Criteria (psf)</b> TCCL: 25.00 TCCL: 10.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCCL: 0.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	<b>Wind Criteria</b> Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCCL: 6.0 psf BCDL: 6.0 psf Mean Height: 15.00 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.55 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.012 C - - HORZ(TL): 0.019 C - - Creep Factor: 2.0 Max TC CSI: 0.302 Max BC CSI: 0.145 Max Web CSI: 0.075 VIEW Ver: 25.02.00B.1125.14	<b>▲ Maximum Reactions (lbs)</b> <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>249</td> <td>/-</td> <td>/-</td> <td>/112</td> <td>/-</td> <td>/38</td> </tr> <tr> <td>E</td> <td>180</td> <td>/-</td> <td>/-</td> <td>/103</td> <td>/11</td> <td>/-</td> </tr> </tbody> </table> Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.5 (Support) E Brg Wid = - Min Req = - Bearing B Fcperp = 425psi. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> <table border="1"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> </tr> </thead> <tbody> <tr> <td>B - C</td> <td>317 -390</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	B	249	/-	/-	/112	/-	/38	E	180	/-	/-	/103	/11	/-	Chords	Tens.Comp.	B - C	317 -390
Loc	Gravity			Non-Gravity																															
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Chords	Tens.Comp.																																		
B - C	317 -390																																		

**Lumber**

Top chord: 2x4 SP #2;  
 Bot chord: 2x4 SP #2;  
 Webs: 2x4 SP #2;  
 Lt Slider: 2x4 SP #2; block length = 1.500'

**Purlins**

In lieu of structural panels or rigid ceiling use purlins to laterally brace chords as follows:  

Chord	Spacing(in oc)	Start(ft)	End(ft)
BC	48	0.00	3.96

 Apply purlins to any chords above or below fillers at 24" OC unless shown otherwise above.

**Hangers / Ties**

(J) Hanger Support Required, by others

**Wind**

Wind loads based on MWFRS with additional C&C member design.  
 Right end vertical not exposed to wind pressure.  
 Wind loading based on both gable and hip roof types.

**Snow**

Overhang designed for 2.00X TC LL.

**Additional Notes**

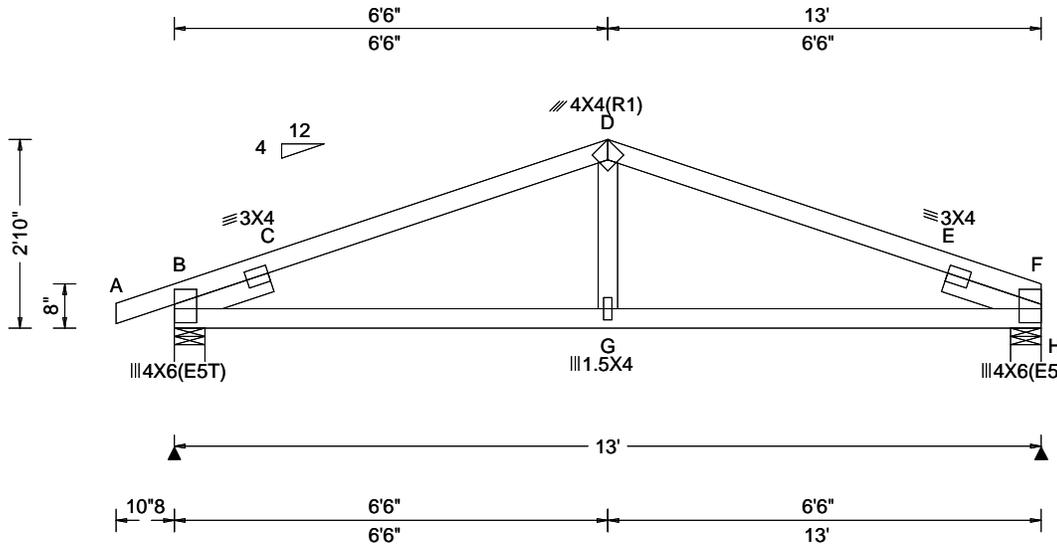
Top Chord overhang(s) may be field trimmed.



MISSOURI COA #2005000817  
01/12/2026

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<b>Loading Criteria (psf)</b> TCLL: 25.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	<b>Wind Criteria</b> Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCDL: 6.0 psf BCDL: 6.0 psf Mean Height: 15.00 ft MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.55 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.077 E 999 240 VERT(CL): 0.119 E 999 180 HORZ(LL): 0.030 C - - HORZ(TL): 0.046 C - - Creep Factor: 2.0 Max TC CSI: 0.750 Max BC CSI: 0.539 Max Web CSI: 0.143  VIEW Ver: 25.02.00B.1125.14	<b>▲ Maximum Reactions (lbs)</b> Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 656 /- /- /334 /10 /21 H 592 /- /- /317 /8 /- Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.5 (Support) H Brg Wid = 5.5 Min Req = 1.5 (Support) Bearings B & H Fcperp = 425psi. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp. B - C 387 -1368 D - E 185 -1004 C - D 184 -1004 E - F 401 -1356  <b>Maximum Bot Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp. B - G 912 -127 G - F 912 -127
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**Lumber**  
Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #2;  
Lt Slider: 2x4 SP #2; block length = 1.500'  
Rt Slider: 2x4 SP #2; block length = 1.500'

**Purlins**  
In lieu of structural panels or rigid ceiling use purlins to laterally brace chords as follows:  
Chord Spacing(in oc) Start(ft) End(ft)  
BC 120 0.00 13.00  
Apply purlins to any chords above or below fillers at 24" OC unless shown otherwise above.

**Loading**  
Bottom chord checked for 10.00 psf non-concurrent live load.

**Wind**  
Wind loads based on MWFRS with additional C&C member design.  
Wind loading based on both gable and hip roof types.

**Snow**  
Overhang designed for 2.00X TC LL.

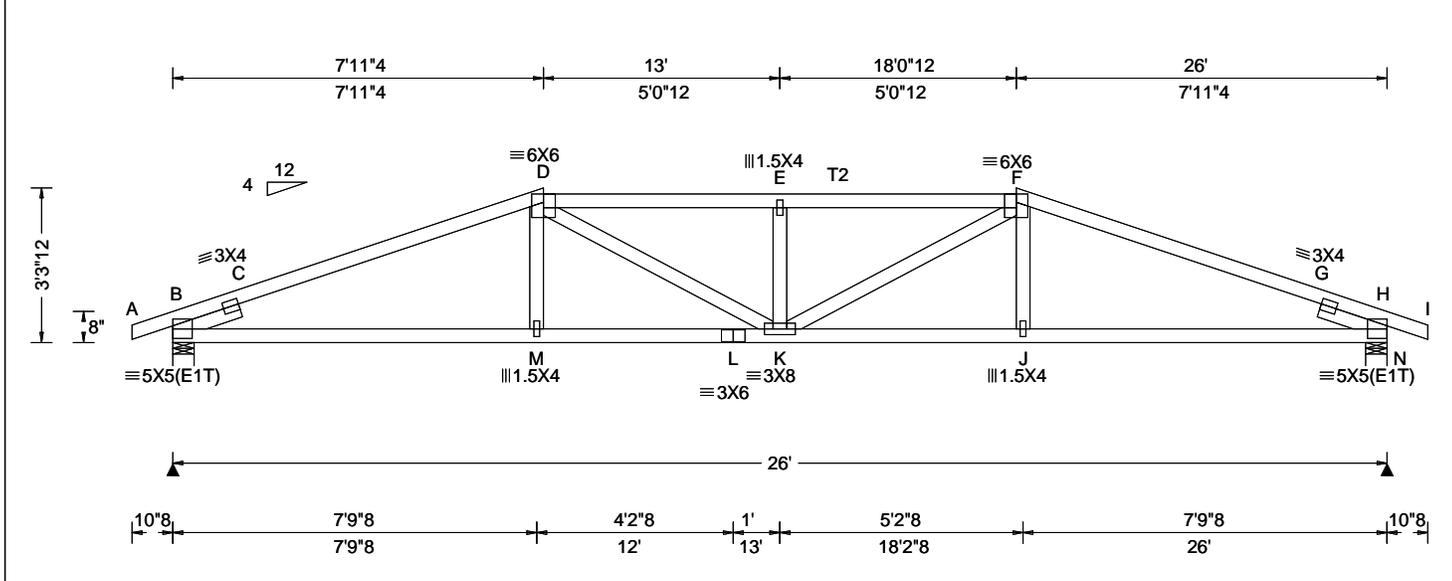
**Additional Notes**  
Top Chord overhang(s) may be field trimmed.



MISSOURI COA #2005000817  
01/12/2026

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<b>Loading Criteria (psf)</b> TCLL: 25.00 TC DL: 10.00 BC LL: 0.00 BC DL: 10.00 Des Ld: 45.00 NCBC LL: 10.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	<b>Wind Criteria</b> Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TC DL: 6.0 psf BC DL: 6.0 psf Mean Height: 15.00 ft MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.55 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.158 E 999 249 VERT(CL): 0.290 E 999 249 HORZ(LL): 0.049 H - - HORZ(TL): 0.090 H - - Creep Factor: 2.0 Max TC CSI: 0.748 Max BC CSI: 0.877 Max Web CSI: 0.253 VIEW Ver: 25.02.00B.1125.14	<b>▲ Maximum Reactions (lbs)</b> <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>1247</td> <td>-</td> <td>-</td> <td>/646</td> <td>-</td> <td>/21</td> </tr> <tr> <td>N</td> <td>1247</td> <td>-</td> <td>-</td> <td>/646</td> <td>-</td> <td>-</td> </tr> </tbody> </table>						Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	B	1247	-	-	/646	-	/21	N	1247	-	-	/646	-	-	
				Loc	Gravity			Non-Gravity																													
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Chords	Tens.Comp.	Chords	Tens. Comp.																																		
B - C	627 -2601	E - F	544 -2689																																		
C - D	436 -2535	F - G	436 -2535																																		
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<b>Maximum Web Forces Per Ply (lbs)</b> <table border="1"> <thead> <tr> <th>Webs</th> <th>Tens.Comp.</th> <th>Webs</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>D - K</td> <td>412 -159</td> <td>K - F</td> <td>412 -159</td> </tr> </tbody> </table>				Webs	Tens.Comp.	Webs	Tens. Comp.	D - K	412 -159	K - F	412 -159																										
Webs	Tens.Comp.	Webs	Tens. Comp.																																		
D - K	412 -159	K - F	412 -159																																		

**Lumber**  
 Top chord: 2x4 SP 2400f-2.0E; T2 2x4 SP #2;  
 Bot chord: 2x4 SP #2;  
 Webs: 2x4 SP #2;  
 Lt Slider: 2x4 SP #2; block length = 1.500'  
 Rt Slider: 2x4 SP #2; block length = 1.500'

**Purlins**  
 In lieu of structural panels or rigid ceiling use purlins to laterally brace chords as follows:

Chord	Spacing(in oc)	Start(ft)	End(ft)
TC	24	7.94	18.06
BC	120	0.00	26.00

Apply purlins to any chords above or below fillers at 24" OC unless shown otherwise above.

**Loading**  
 Bottom chord checked for 10.00 psf non-concurrent live load.

**Wind**  
 Wind loads based on MWFRS with additional C&C member design.  
 Wind loading based on both gable and hip roof types.

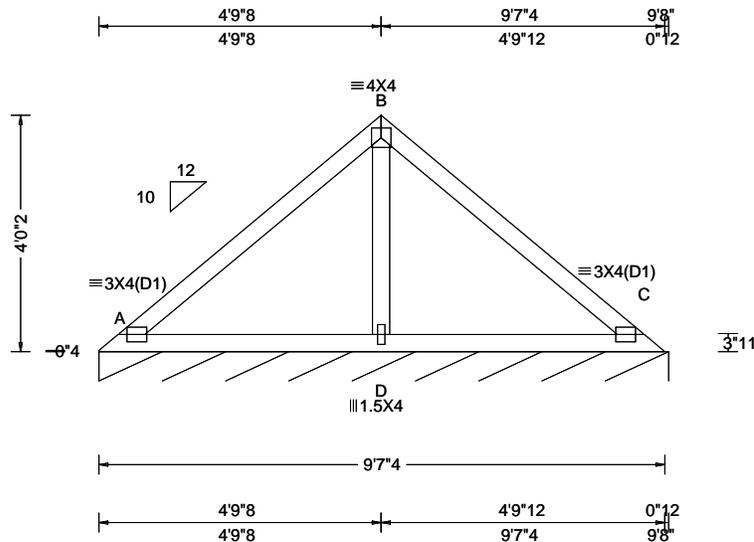
**Snow**  
 Overhang designed for 2.00X TC LL.

**Additional Notes**  
 Top Chord overhang(s) may be field trimmed.



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<b>Loading Criteria (psf)</b>	<b>Wind Criteria</b>	<b>Snow Criteria (Pg,Pf in PSF)</b>	<b>Defl/CSI Criteria</b>	<b>▲ Maximum Reactions (lbs), or *=PLF</b>
TCLL: 25.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCDL: 6.0 psf BCDL: 6.0 psf Mean Height: 21.76 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.55 Wind Duration: 1.60	Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.013 C 999 240 VERT(CL): 0.024 C 999 180 HORZ(LL): -0.008 C - - HORZ(TL): 0.014 C - - Creep Factor: 2.0 Max TC CSI: 0.412 Max BC CSI: 0.677 Max Web CSI: 0.147  VIEW Ver: 25.02.00B.1125.14	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL C* 95 /- /- /52 /3 /6 Wind reactions based on MWFRS C Brg Wid = 115 Min Req = - Bearing A Fcperp = 425psi. Members not listed have forces less than 375# <b>Maximum Web Forces Per Ply (lbs)</b> Webs Tens.Comp. B - D 194 -602

**Lumber**  
Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #2;

**Purlins**  
In lieu of structural panels or rigid ceiling use purlins to laterally brace chords as follows:  
Chord Spacing(in oc) Start(ft) End(ft)  
BC 115 0.00 9.60  
Apply purlins to any chords above or below fillers at 24" OC unless shown otherwise above.

**Loading**  
Bottom chord checked for 10.00 psf non-concurrent live load.

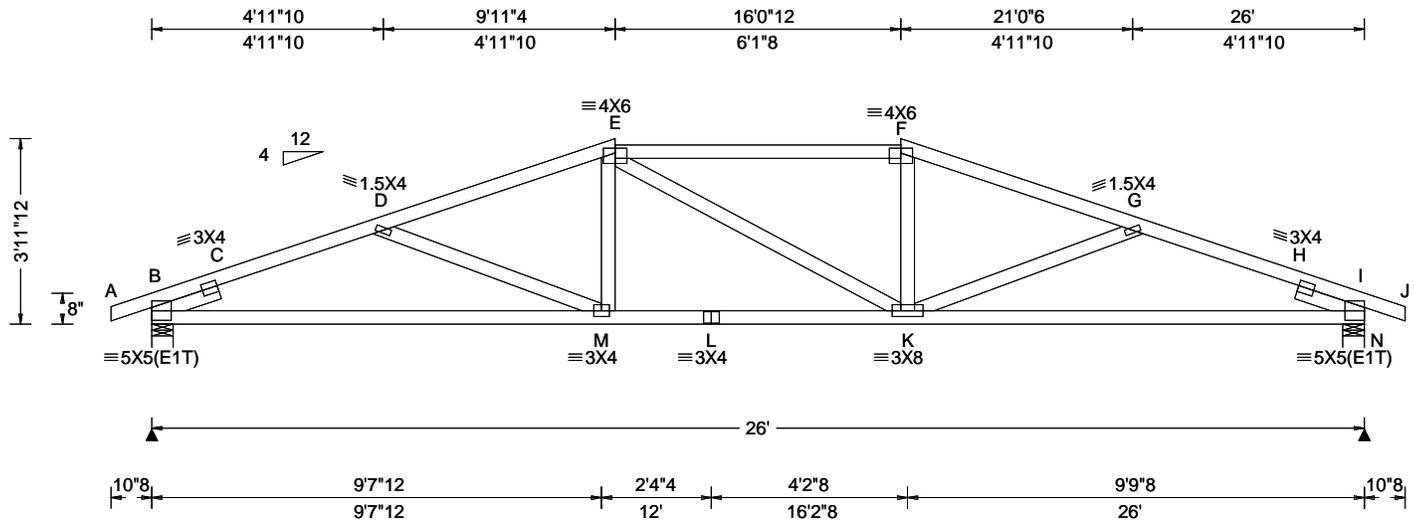
**Wind**  
Wind loads based on MWFRS with additional C&C member design.  
Wind loading based on both gable and hip roof types.

**Additional Notes**  
See DWG VAL180160118 for valley details.



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<b>Loading Criteria (psf)</b> TCLL: 25.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	<b>Wind Criteria</b> Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCDL: 6.0 psf BCDL: 6.0 psf Mean Height: 15.00 ft MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.55 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.146 E 999 249 VERT(CL): 0.261 E 999 249 HORZ(LL): 0.051 I - - HORZ(TL): 0.091 I - - Creep Factor: 2.0 Max TC CSI: 0.657 Max BC CSI: 0.928 Max Web CSI: 0.183  VIEW Ver: 25.02.00B.1125.14	<b>▲ Maximum Reactions (lbs)</b> Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 1247 /- /- /648 /- /27 N 1247 /- /- /648 /- /- Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.6 (Support) N Brg Wid = 5.5 Min Req = 1.6 (Support) Bearings B & N Fcperp = 425psi. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp. B - C 569 -2660 F - G 344 -2253 C - D 400 -2608 G - H 401 -2608 D - E 344 -2260 H - I 579 -2659 E - F 353 -2086  <b>Maximum Bot Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp. B - M 2430 -332 L - K 2082 -241 M - L 2082 -241 K - I 2430 -331
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**Lumber**  
Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #2;  
Lt Slider: 2x4 SP #2; block length = 1.500'  
Rt Slider: 2x4 SP #2; block length = 1.500'

**Purlins**  
In lieu of structural panels or rigid ceiling use purlins to laterally brace chords as follows:  
Chord Spacing(in oc) Start(ft) End(ft)  
TC 24 9.94 16.06  
BC 120 0.00 26.00  
Apply purlins to any chords above or below fillers at 24" OC unless shown otherwise above.

**Loading**  
Bottom chord checked for 10.00 psf non-concurrent live load.

**Wind**  
Wind loads based on MWFRS with additional C&C member design.  
Wind loading based on both gable and hip roof types.

**Snow**  
Overhang designed for 2.00X TC LL.

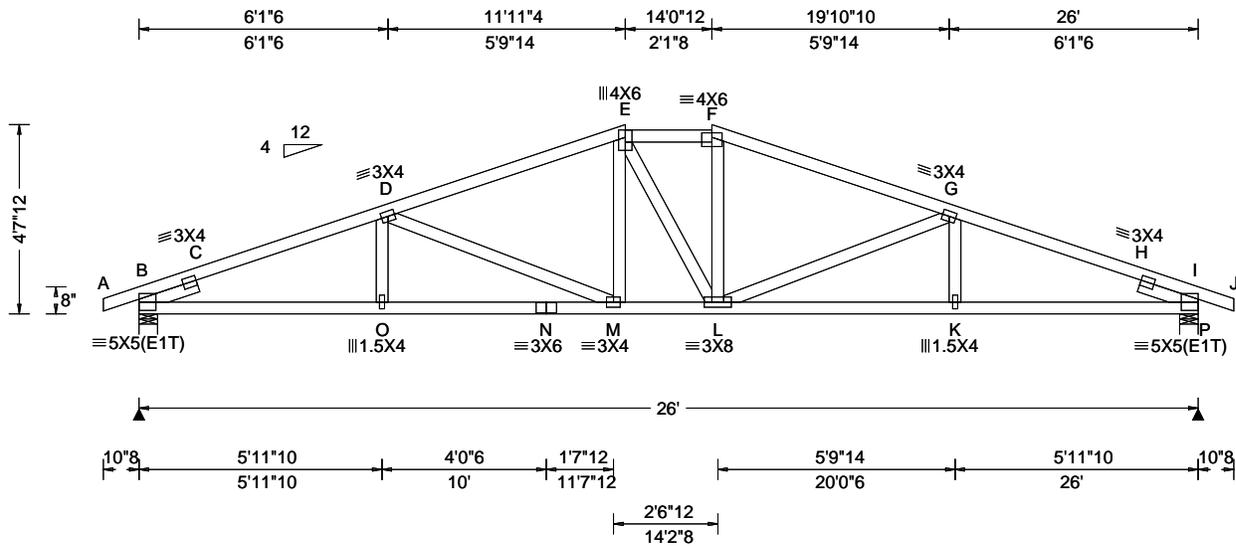
**Additional Notes**  
Top Chord overhang(s) may be field trimmed.



MISSOURI COA #2005000817  
01/12/2026

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<b>Loading Criteria (psf)</b> TCCL: 25.00 TCCL: 10.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	<b>Wind Criteria</b> Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCCL: 6.0 psf BCDL: 6.0 psf Mean Height: 15.00 ft MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.55 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.139 M 999 249 VERT(CL): 0.256 M 999 249 HORZ(LL): 0.049 I - - HORZ(TL): 0.089 I - - Creep Factor: 2.0 Max TC CSI: 0.653 Max BC CSI: 0.726 Max Web CSI: 0.425 VIEW Ver: 25.02.00B.1125.14	<b>▲ Maximum Reactions (lbs)</b> Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 1247 /- /- /651 /- /33 P 1247 /- /- /651 /- /- Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.6 (Support) P Brg Wid = 5.5 Min Req = 1.6 (Support) Bearings B & P Fcperp = 425psi. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp.					
				B - C 306 -2663 F - G 253 -1992 C - D 261 -2598 G - H 261 -2598 D - E 254 -2001 H - I 305 -2663 E - F 263 -1819					

**Lumber**  
 Top chord: 2x4 SP #2;  
 Bot chord: 2x4 SP #2;  
 Webs: 2x4 SP #2;  
 Lt Slider: 2x4 SP #2; block length = 1.500'  
 Rt Slider: 2x4 SP #2; block length = 1.500'

**Purlins**  
 In lieu of structural panels or rigid ceiling use purlins to laterally brace chords as follows:  
 Chord Spacing(in oc) Start(ft) End(ft)  
 TC 24 11.94 14.06  
 BC 120 0.00 26.00  
 Apply purlins to any chords above or below fillers at 24" OC unless shown otherwise above.

**Maximum Bot Chord Forces Per Ply (lbs)**

Chords	Tens.Comp.	Chords	Tens. Comp.
B - O	2412 -200	M - L	1816 -128
O - N	2408 -202	L - K	2407 -201
N - M	2408 -202	K - I	2412 -198

**Maximum Web Forces Per Ply (lbs)**

Webs	Tens.Comp.	Webs	Tens. Comp.
D - M	81 -620	L - G	81 -626

**Loading**  
 Bottom chord checked for 10.00 psf non-concurrent live load.

**Wind**  
 Wind loads based on MWFRS with additional C&C member design.  
 Wind loading based on both gable and hip roof types.

**Snow**  
 Overhang designed for 2.00X TC LL.

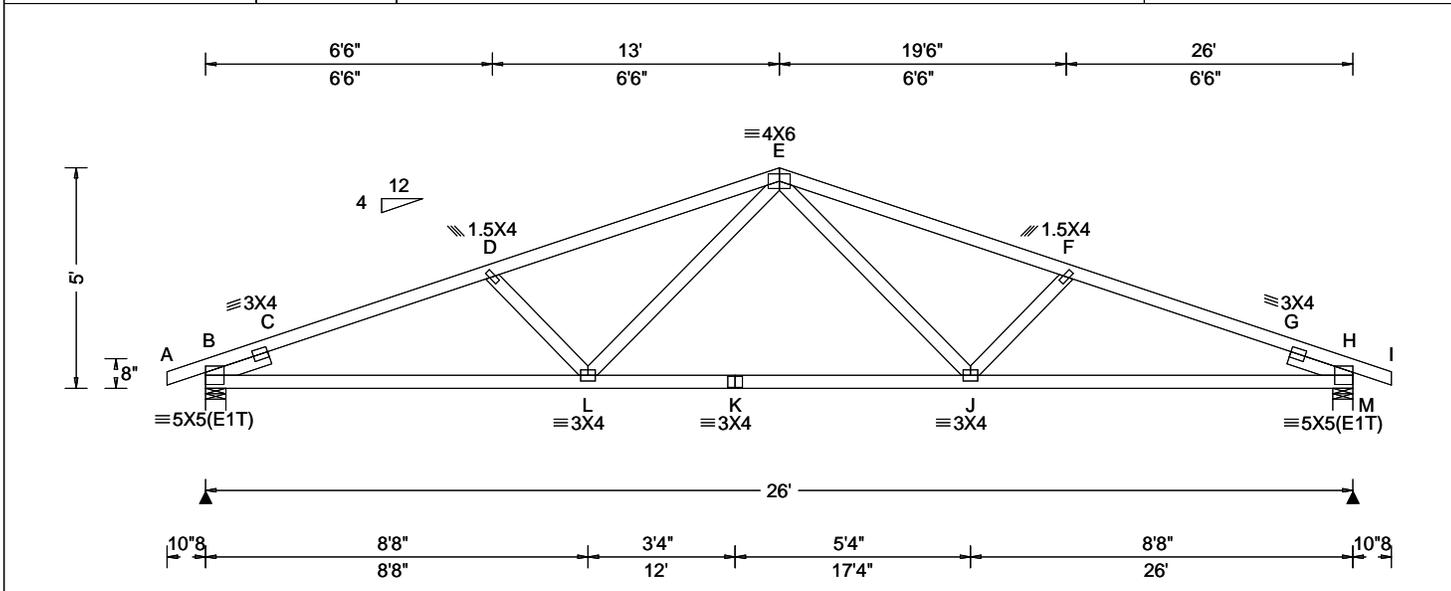
**Additional Notes**  
 Top Chord overhang(s) may be field trimmed.



MISSOURI COA #2005000817  
 01/12/2026

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<b>Loading Criteria (psf)</b> TCLL: 25.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	<b>Wind Criteria</b> Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCDL: 6.0 psf BCDL: 6.0 psf Mean Height: 15.00 ft MWFRS Parallel Dist: h to 2h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.55 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.139 E 999 249 VERT(CL): 0.252 E 999 249 HORZ(LL): 0.046 H - - HORZ(TL): 0.083 H - - Creep Factor: 2.0 Max TC CSI: 0.775 Max BC CSI: 0.875 Max Web CSI: 0.159  VIEW Ver: 25.02.00B.1125.14	<b>▲ Maximum Reactions (lbs)</b> Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 1247 -/- /- /652 -/- /36 M 1247 -/- /- /652 -/- /- Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.6 (Support) M Brg Wid = 5.5 Min Req = 1.6 (Support) Bearings B & M Fcperp = 425psi. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp. B - C 418 -2650 E - F 214 -2299 C - D 244 -2587 F - G 244 -2587 D - E 214 -2299 G - H 418 -2650
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**Lumber**  
Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #2;  
Lt Slider: 2x4 SP #2; block length = 1.500'  
Rt Slider: 2x4 SP #2; block length = 1.500'

**Purlins**  
In lieu of structural panels or rigid ceiling use purlins to laterally brace chords as follows:  
Chord Spacing(in oc) Start(ft) End(ft)  
BC 120 0.00 26.00  
Apply purlins to any chords above or below fillers at 24" OC unless shown otherwise above.

**Loading**  
Bottom chord checked for 10.00 psf non-concurrent live load.

**Wind**  
Wind loads based on MWFRS with additional C&C member design.  
Wind loading based on both gable and hip roof types.

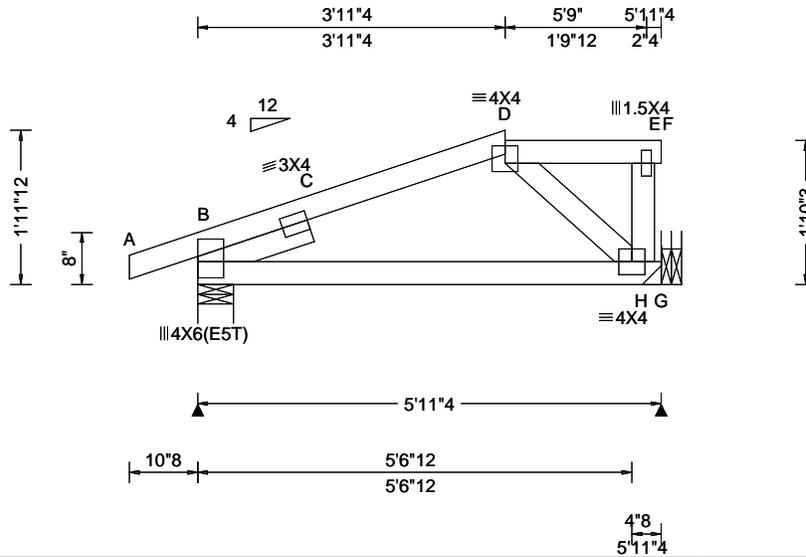
**Snow**  
Overhang designed for 2.00X TC LL.

**Additional Notes**  
Top Chord overhang(s) may be field trimmed.



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**ALPINE**  
RELEASE FOR CONSTRUCTION  
AS NOTED BY PLANS REVIEW  
DEVELOPMENT SERVICES  
LEE'S SUMMIT, MISSOURI  
02/12/2026 4:05:17



<b>Loading Criteria (psf)</b> TCLL: 25.00 TC DL: 10.00 BC LL: 0.00 BC DL: 10.00 Des Ld: 45.00 NCBC LL: 0.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	<b>Wind Criteria</b> Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TC DL: 6.0 psf BC DL: 6.0 psf Mean Height: 15.00 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.55 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.030 C 999 240 VERT(CL): 0.050 C 999 180 HORZ(LL): 0.012 C - - HORZ(TL): 0.020 C - - Creep Factor: 2.0 Max TC CSI: 0.345 Max BC CSI: 0.249 Max Web CSI: 0.109 VIEW Ver: 25.02.00B.1125.14	<b>▲ Maximum Reactions (lbs)</b> <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>333</td> <td>-</td> <td>-</td> <td>/158</td> <td>/0</td> <td>/38</td> </tr> <tr> <td>G</td> <td>274</td> <td>-</td> <td>-</td> <td>/150</td> <td>/10</td> <td>-</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	B	333	-	-	/158	/0	/38	G	274	-	-	/150	/10	-
				Loc		Gravity			Non-Gravity																						
R+	/R-	/Rh	/Rw		/U	/RL																									
B	333	-	-	/158	/0	/38																									
G	274	-	-	/150	/10	-																									
Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.5 (Support) G Brg Wid = - Min Req = - Bearing B Fcperp = 425psi. Members not listed have forces less than 375#																															

**Lumber**  
 Top chord: 2x4 SP #2;  
 Bot chord: 2x4 SP #2;  
 Webs: 2x4 SP #2;  
 Lt Slider: 2x4 SP #2; block length = 1.500'

**Purlins**  
 In lieu of structural panels or rigid ceiling use purlins to laterally brace chords as follows:

Chord	Spacing(in oc)	Start(ft)	End(ft)
TC	24	3.94	5.94
BC	71	0.00	5.94

Apply purlins to any chords above or below fillers at 24" OC unless shown otherwise above.

**Wind**  
 Wind loads based on MWFRS with additional C&C member design.  
 Right end vertical not exposed to wind pressure.  
 Wind loading based on both gable and hip roof types.

**Snow**  
 Overhang designed for 2.00X TC LL.

**Additional Notes**  
 Top Chord overhang(s) may be field trimmed.



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SEQN: 5785	MHJA	Ply: 1	Job Number: PM000129	Cust: R 9646 JRef: 1YGQ96460003 T33
FROM:		Qty: 1	Customer - Clayton Properties Plan Name - Sienna Elevation - Farmhouse	DrwNo: 012.26.1517.29330
Page 2 of 2			Truss Label: J8	/ BM 01/12/2026

**Hangers / Ties**

Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.

Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

Bearing at location  $x=5'8"4$  uses the following support conditions:  $5'8"4$

Bearing H ( $5'8"4, 9'1"2$ ) LUS24

Supporting Member: (2)2x6 SP 2400f-2.0E

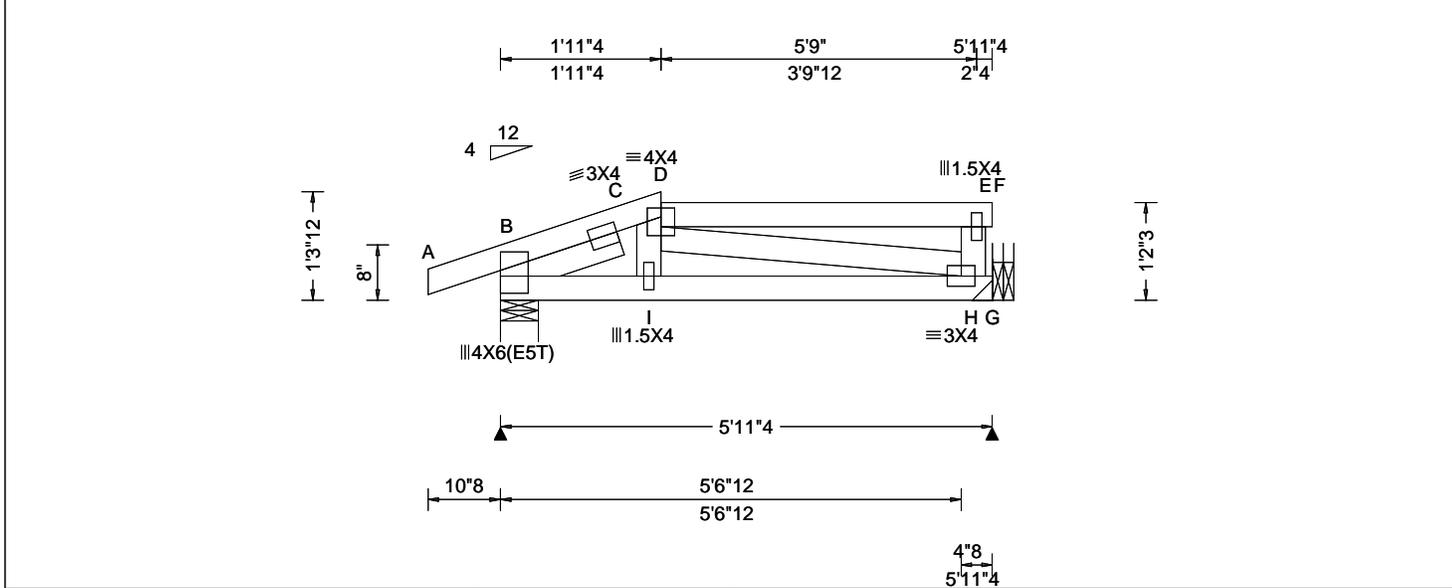
(4) 0.148"x3" nails into supporting member,

(2) 0.148"x3" nails into supported member.



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**ALPINE**  
 RELEASE FOR CONSTRUCTION  
 AS NOTED BY ANS REVIEW  
 DEVELOPMENT SERVICES  
 LEE'S SUMMIT, MISSOURI  
 02/12/2026 4:05:17



<b>Loading Criteria</b> (psf) TCLL: 25.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 0.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	<b>Wind Criteria</b> Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCDL: 6.0 psf BCDL: 6.0 psf Mean Height: 15.00 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.55 Wind Duration: 1.60	<b>Snow Criteria</b> (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.006 C 999 240 VERT(CL): 0.011 D 999 180 HORZ(LL): 0.002 C - - HORZ(TL): 0.003 C - - Creep Factor: 2.0 Max TC CSI: 0.364 Max BC CSI: 0.206 Max Web CSI: 0.112  VIEW Ver: 25.02.00B.1125.14	<b>▲ Maximum Reactions (lbs)</b> Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL B 348 /- /- /- /29 /- G 240 /- /- /- /8 /- Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.5 (Support) G Brg Wid = - Min Req = - Bearing B Fcperp = 425psi. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp. B - C 3 -477 C - D 0 -442
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**Lumber**  
Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #2;  
Lt Slider: 2x4 SP #2; block length = 1.500'

**Additional Notes**  
Top Chord overhang(s) may be field trimmed.

**Maximum Bot Chord Forces Per Ply (lbs)**

Chords	Tens.Comp.	Chords	Tens. Comp.
B - I	419	I - H	412 -5

**Special Loads**  
-----(Lumber Dur.Fac.=1.15 / Plate Dur.Fac.=1.15)  
TC: From 71 plf at -0.88 to 71 plf at 1.94  
TC: From 36 plf at 1.94 to 36 plf at 5.94  
BC: From 10 plf at 0.00 to 10 plf at 5.94  
TC: 71 lb Conc. Load at 1.95  
TC: 68 lb Conc. Load at 4.00  
BC: 23 lb Conc. Load at 1.95  
BC: 20 lb Conc. Load at 4.00

**Maximum Web Forces Per Ply (lbs)**

Webs	Tens.Comp.
D - H	3 -398

**Purlins**  
In lieu of structural panels or rigid ceiling use purlins to laterally brace chords as follows:  
Chord Spacing(in oc) Start(ft) End(ft)  
TC 24 1.94 5.94  
BC 71 0.00 5.94  
Apply purlins to any chords above or below fillers at 24" OC unless shown otherwise above.

**Wind**  
Wind loads and reactions based on MWFRS.  
Right end vertical not exposed to wind pressure.  
Wind loading based on both gable and hip roof types.

**Snow**  
Overhang designed for 2.00X TC LL.



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SEQN: 5784 FROM: Page 2 of 2	NEJA Ply: 1 Qty: 1	Job Number: PM000129 Customer - Clayton Properties Plan Name - Sienna Elevation - Farmhouse Truss Label: J7	Cust: R 9646 JRef: 1YGQ96460003 T34 DrwNo: 012.26.1517.27723 / BM 01/12/2026
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**Hangers / Ties**

Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.

Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

Bearing at location x=5'8"4 uses the following support conditions: 5'8"4

Bearing H (5'8"4, 9'1"2) LUS24

Supporting Member: (2)2x6 SP 2400f-2.0E

(4) 0.148"x3" nails into supporting member,

(2) 0.148"x3" nails into supported member.



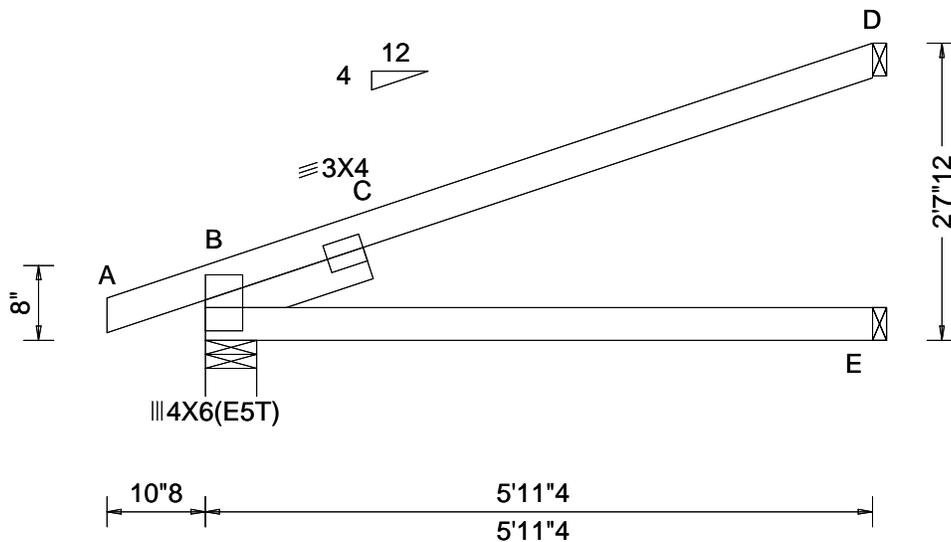
MISSOURI COA #2005000817  
01/12/2026

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**ALPINE**  
**RELEASE FOR CONSTRUCTION**  
**AS NOTED BY ENGINEERS REVIEW**  
**DEVELOPMENT SERVICES**  
 LEE'S SUMMIT, MISSOURI  
 02/12/2026 4:05:17



<b>Loading Criteria (psf)</b> TCLL: 25.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 0.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	<b>Wind Criteria</b> Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCDL: 6.0 psf BCDL: 6.0 psf Mean Height: 15.00 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.55 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.042 C - - HORZ(TL): 0.066 C - - Creep Factor: 2.0 Max TC CSI: 0.752 Max BC CSI: 0.425 Max Web CSI: 0.195  VIEW Ver: 25.02.00B.1125.14	<b>▲ Maximum Reactions (lbs)</b> Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 337 /- /- /159 /- /56 E 74 /- /- /63 /- /- D 196 /- /- /92 /42 /- Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.5 (Support) E Brg Wid = 1.5 Min Req = - D Brg Wid = 1.5 Min Req = - Bearing B Fcperp = 425psi. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. B - C 920 - 1032
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**Lumber**  
Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Lt Slider: 2x4 SP #2; block length = 1.500'

**Purlins**  
In lieu of structural panels or rigid ceiling use purlins to laterally brace chords as follows:  
Chord Spacing(in oc) Start(ft) End(ft)  
BC 71 0.00 5.94  
Apply purlins to any chords above or below fillers at 24" OC unless shown otherwise above.

**Wind**  
Wind loads based on MWFRS with additional C&C member design.  
Wind loading based on both gable and hip roof types.

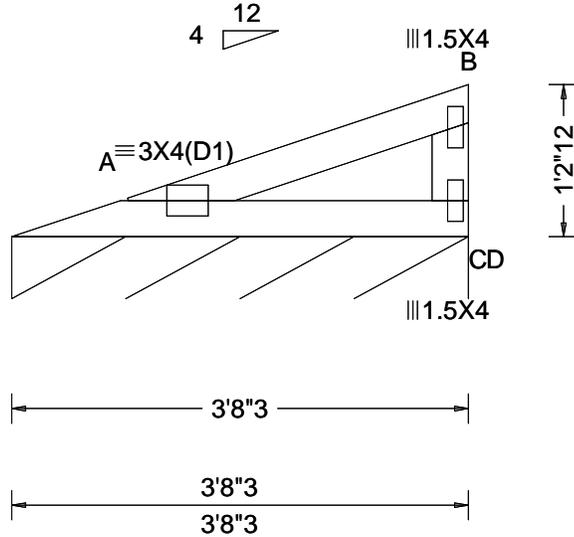
**Snow**  
Overhang designed for 2.00X TC LL.

**Additional Notes**  
Top Chord overhang(s) may be field trimmed.



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<b>Loading Criteria (psf)</b> TCCL: 25.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 0.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	<b>Wind Criteria</b> Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCDL: 6.0 psf BCDL: 6.0 psf Mean Height: 15.00 ft MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.55 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.002 A - - HORZ(TL): 0.004 A - - Creep Factor: 2.0 Max TC CSI: 0.175 Max BC CSI: 0.160 Max Web CSI: 0.021  VIEW Ver: 25.02.00B.1125.14	<b>▲ Maximum Reactions (lbs), or *=PLF</b> Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL D* 91 /- /- /43 /- /7 Wind reactions based on MWFRS D Brg Wid = 44.2 Min Req = - Bearing A Fcperp = 425psi. Members not listed have forces less than 375#
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**Lumber**

Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #2;

**Purlins**

In lieu of structural panels or rigid ceiling use purlins to laterally brace chords as follows:

Chord	Spacing(in oc)	Start(ft)	End(ft)
BC	44	0.00	3.68

Apply purlins to any chords above or below fillers at 24" OC unless shown otherwise above.

**Wind**

Wind loads based on MWFRS with additional C&C member design.

Right end vertical exposed to wind pressure.  
Deflection does not meet L/180.

Wind loading based on both gable and hip roof types.

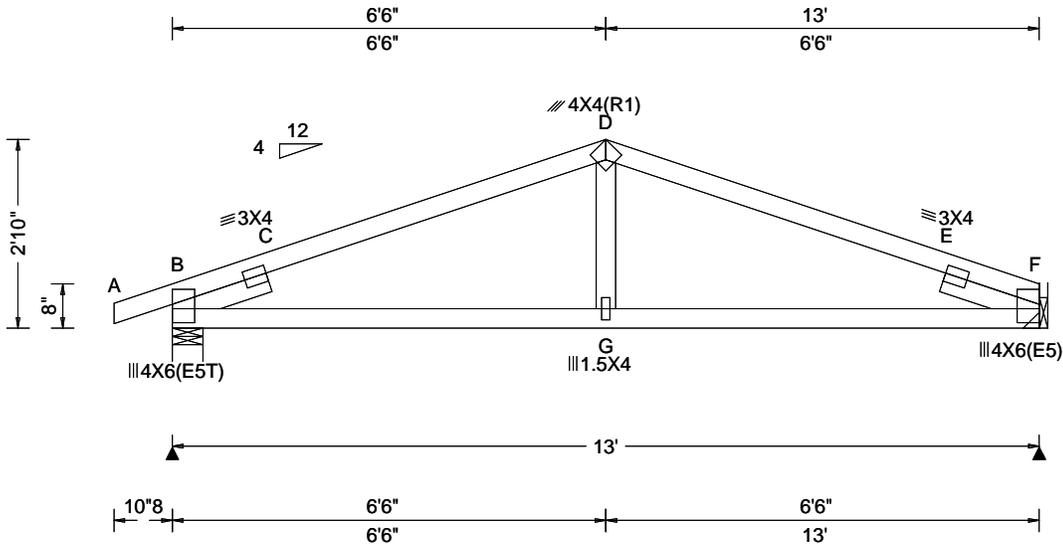
**Additional Notes**

See DWG VAL180160118 for valley details.



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<b>Loading Criteria (psf)</b> TCLL: 25.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	<b>Wind Criteria</b> Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCDL: 6.0 psf BCDL: 6.0 psf Mean Height: 15.00 ft MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.55 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.077 E 999 240 VERT(CL): 0.119 E 999 180 HORZ(LL): 0.030 C - - HORZ(TL): 0.046 C - - Creep Factor: 2.0 Max TC CSI: 0.750 Max BC CSI: 0.539 Max Web CSI: 0.143  VIEW Ver: 25.02.00B.1125.14	<b>▲ Maximum Reactions (lbs)</b> Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 656 /- /- /334 /10 /21 F 592 /- /- /317 /8 /- Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.5 (Support) F Brg Wid = - Min Req = - Bearing B Fcperp = 425psi. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp. B - C 387 -1368 D - E 185 -1004 C - D 184 -1004 E - F 401 -1356  <b>Maximum Bot Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp. B - G 912 -127 G - F 912 -127
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**Lumber**

Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #2;  
Lt Slider: 2x4 SP #2; block length = 1.500'  
Rt Slider: 2x4 SP #2; block length = 1.500'

**Purlins**

In lieu of structural panels or rigid ceiling use purlins to laterally brace chords as follows:  
Chord Spacing(in oc) Start(ft) End(ft)  
BC 120 0.00 13.00  
Apply purlins to any chords above or below fillers at 24" OC unless shown otherwise above.

**Loading**

Bottom chord checked for 10.00 psf non-concurrent live load.

**Wind**

Wind loads based on MWFRS with additional C&C member design.  
Wind loading based on both gable and hip roof types.

**Snow**

Overhang designed for 2.00X TC LL.

**Additional Notes**

Top Chord overhang(s) may be field trimmed.



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SEQN: 5760 FROM: Page 2 of 2	COMN Ply: 1 Qty: 2	Job Number: PM000129 Customer - Clayton Properties Plan Name - Sienna Elevation - Farmhouse Truss Label: A5	Cust: R 9646 JRef: 1YQG96460003 T37 DrwNo: 012.26.1515.37377 / BM 01/12/2026
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**Hangers / Ties**

Simpson Construction Hardware is specified based on the most current information provided by Simpson Strong-Tie. Please refer to the most recent Simpson Strong-Tie catalog for additional information.

Recommended hanger connections are based on manufacturer tested capacities and calculations. Conditions may exist that require different connections than indicated. Refer to manufacturer publication for additional information.

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating coverage.

Bearing at location x=12'9" uses the following support conditions: 12'9"

- Bearing F (12'9", 9'1"2) LUS24
- Supporting Member: (1)2x6 SP #2
- (4) 0.148"x3" nails into supporting member,
- (2) 0.148"x3" nails into supported member.



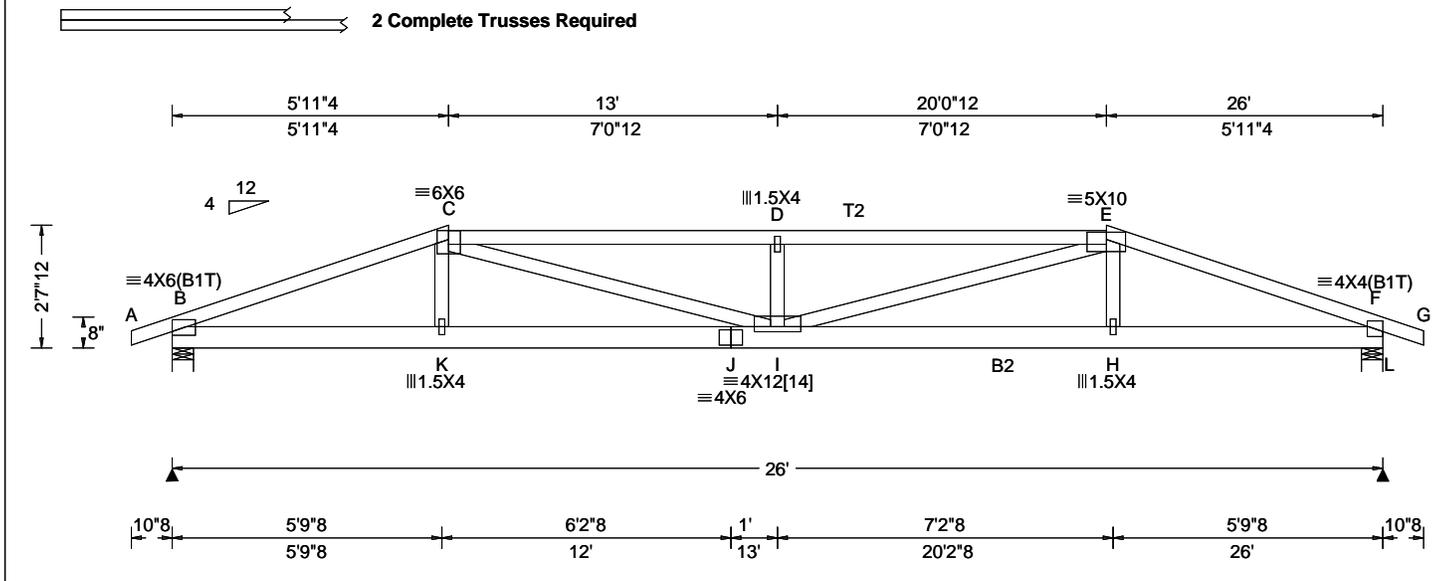
MISSOURI COA #2005000817  
01/12/2026

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**ALPINE**

North Building, 4th Floor  
Glenview, IL 60025

**RELEASE FOR CONSTRUCTION  
AS NOTED FOR REVIEW  
DEVELOPMENT SERVICES  
LEE'S SUMMIT, MISSOURI  
02/12/2026 4:05:17**



**Loading Criteria (psf)**

TCLL: 25.00  
 TC DL: 10.00  
 BC LL: 0.00  
 BC DL: 10.00  
 Des Ld: 45.00  
 NCBC LL: 0.00  
 Soffit: 0.00  
 Load Duration: 1.15  
 Spacing: 24.0 "

**Wind Criteria**

Speed: 115 mph  
 Risk Category: II  
 Enclosure: Part. Enc.  
 EXP: B Kzt: NA  
 TC DL: 6.0 psf  
 BC DL: 6.0 psf  
 Mean Height: 15.00 ft  
 MWFRS Parallel Dist: 0 to h/2  
 C&C Dist a: 3.00 ft  
 Loc. from endwall: not in 9.00 ft  
 GCpi: 0.55  
 Wind Duration: 1.60

**Snow Criteria (Pg,Pf in PSF)**

Pg: NA Ct: NA CAT: NA  
 Pf(ASD): NA  
 Ce: NA Lu: NA  
 Cs: NA Snow Duration: NA

Building Code:  
 IRC 2018  
 Load Std: ASCE 7-16  
 TPI Std: 2014  
 Rep Fac: Varies by Ld Case  
 FT/RT:20(0)/10(0)  
 Plate Type(s):  
 WAVE

**Defl/CSI Criteria**

PP Deflection in loc L/defl L/#  
 VERT(LL): 0.253 D 999 246  
 VERT(CL): 0.459 D 672 246  
 HORZ(LL): 0.034 F - -  
 HORZ(TL): 0.062 F - -  
 Creep Factor: 2.0  
 Max TC CSI: 0.698  
 Max BC CSI: 0.816  
 Max Web CSI: 0.473

VIEW Ver: 25.02.00B.1125.14

**▲ Maximum Reactions (lbs)**

Loc	Gravity			Non-Gravity		
	R+	/R-	/Rh	/Rw	/U	/RL
B	2371	-	-	-	/93	-
L	1932	-	-	-	/65	-

Wind reactions based on MWFRS  
 B Brg Wid = 5.5 Min Req = 1.7 (Support)  
 L Brg Wid = 5.5 Min Req = 1.5 (Support)  
 Bearings B & L Fcperp = 425psi.  
 Members not listed have forces less than 375#

**Maximum Top Chord Forces Per Ply (lbs)**

Chords	Tens.Comp.	Chords	Tens. Comp.
B - C	92 -2664	D - E	135 -3883
C - D	135 -3884	E - F	66 -2190

**Lumber**  
 Top chord: 2x4 SP #2; T2 2x4 SP 2400f-2.0E;  
 Bot chord: 2x6 SP 2400f-2.0E; B2 2x6 SP #2;  
 Webs: 2x4 SP #2;

**Nailnote**  
 Nail Schedule: 0.128"x3", min. nails  
 Top Chord: 1 Row @12.00" o.c.  
 Bot Chord: 1 Row @12.00" o.c.  
 Webs : 1 Row @ 4" o.c.  
 Use equal spacing between rows and stagger nails in each row to avoid splitting.

**Purlins**  
 In lieu of structural panels or rigid ceiling use purlins to laterally brace chords as follows:

Chord	Spacing(in oc)	Start(ft)	End(ft)
TC	24	5.94	20.06
BC	120	0.30	25.70

Apply purlins to any chords above or below fillers at 24" OC unless shown otherwise above.

**Snow**  
 Overhang designed for 2.00X TC LL.

**Additional Notes**  
 Top Chord overhang(s) may be field trimmed.

**Maximum Bot Chord Forces Per Ply (lbs)**

Chords	Tens.Comp.	Chords	Tens. Comp.
B - K	2514 -86	I - H	2048 -60
K - J	2507 -89	H - F	2045 -57
J - I	2507 -89		

**Maximum Web Forces Per Ply (lbs)**

Webs	Tens.Comp.	Webs	Tens. Comp.
C - I	1446 -47	I - E	1927 -78
D - I	82 -466		

**Special Loads**  
 -----(Lumber Dur.Fac.=1.15 / Plate Dur.Fac.=1.15)  
 TC: From 71 plf at -0.88 to 71 plf at 0.00  
 TC: From 36 plf at 0.00 to 36 plf at 12.94  
 TC: From 71 plf at 12.94 to 71 plf at 26.88  
 BC: From 10 plf at 0.00 to 10 plf at 12.94  
 BC: From 20 plf at 12.94 to 20 plf at 26.00  
 TC: 196 lb Conc. Load at 6.00, 8.00,10.00,12.00  
 BC: 240 lb Conc. Load at 2.00  
 BC: 274 lb Conc. Load at 4.00  
 BC: 74 lb Conc. Load at 6.00, 8.00,10.00,12.00  
 BC: 805 lb Conc. Load at 12.94

**Plate Shift Table**

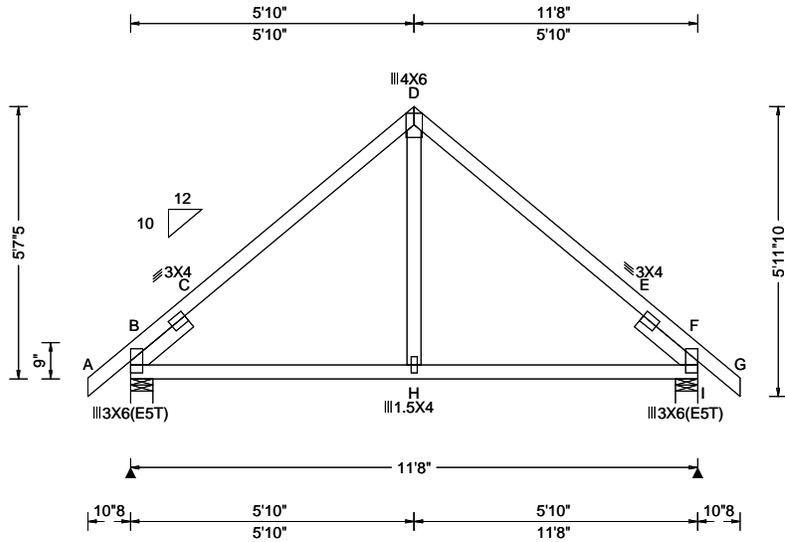
JT No	Plate Size	Lateral Shift	Chord Bite	JT No	Plate Size	Lateral Shift	Chord Bite
[14]	4X12	S	1.25				

**Wind**  
 Wind loads and reactions based on MWFRS.  
 Wind loading based on both gable and hip roof types.

**Professional Engineer Seal:**  
 STATE OF MISSOURI  
 BRADLEY E. MORRIS  
 NUMBER PE-2002019611  
 PROFESSIONAL ENGINEER  
 MISSOURI COA #2005000817  
 01/12/2026

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**ALPINE**  
 RELEASE FOR CONSTRUCTION  
 AS NOTED BY PLANS REVIEW  
 DEVELOPMENT SERVICES  
 LEE'S SUMMIT, MISSOURI  
 02/12/2026 4:05:17



<b>Loading Criteria (psf)</b> TCLL: 25.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	<b>Wind Criteria</b> Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCDL: 6.0 psf BCDL: 6.0 psf Mean Height: 20.90 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.55 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.050 E 999 240 VERT(CL): 0.082 E 999 180 HORZ(LL): 0.046 C - - HORZ(TL): 0.075 C - - Creep Factor: 2.0 Max TC CSI: 0.685 Max BC CSI: 0.405 Max Web CSI: 0.165  VIEW Ver: 25.02.00B.1125.14	<b>▲ Maximum Reactions (lbs)</b> Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 629 -/ - /342 /24 /82 I 629 -/ - /342 /24 /- Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.5 (Support) I Brg Wid = 5.5 Min Req = 1.5 (Support) Bearings B & I Fcperp = 425psi. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp. B - C 678 -1063 D - E 149 -574 C - D 150 -574 E - F 676 -1062
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**Lumber**

Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #2;  
Lt Slider: 2x4 SP #2; block length = 1.500'  
Rt Slider: 2x4 SP #2; block length = 1.500'

**Purlins**

In lieu of structural panels or rigid ceiling use purlins to laterally brace chords as follows:  
Chord Spacing(in oc) Start(ft) End(ft)  
BC 120 0.00 11.67  
Apply purlins to any chords above or below fillers at 24" OC unless shown otherwise above.

**Loading**

Bottom chord checked for 10.00 psf non-concurrent live load.

**Wind**

Wind loads based on MWFRS with additional C&C member design.  
Wind loading based on both gable and hip roof types.

**Snow**

Overhang designed for 2.00X TC LL.

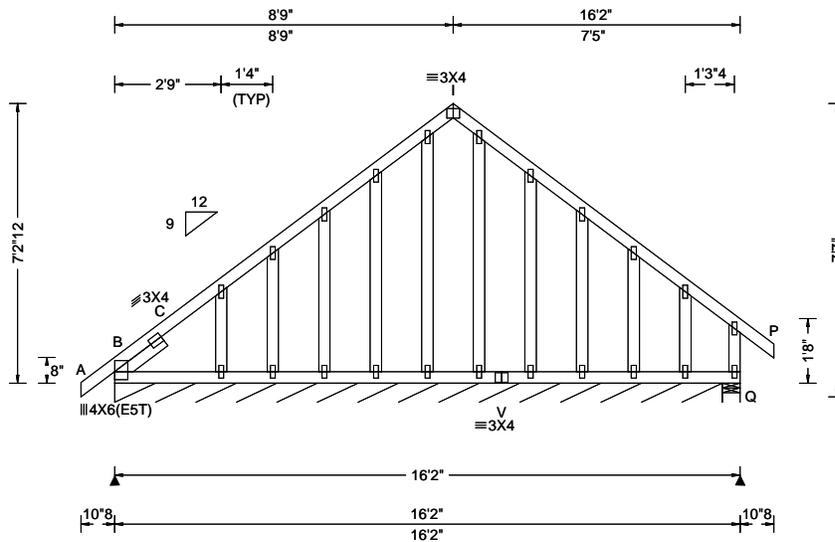
**Additional Notes**

Top Chord overhang(s) may be field trimmed.



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<b>Loading Criteria (psf)</b> TCLL: 25.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	<b>Wind Criteria</b> Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCDL: 6.0 psf BCDL: 6.0 psf Mean Height: 21.70 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.55 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.006 C 999 240 VERT(CL): 0.015 I 999 180 HORZ(LL): 0.005 C - - HORZ(TL): 0.032 O - - Creep Factor: 2.0 Max TC CSI: 0.199 Max BC CSI: 0.052 Max Web CSI: 0.196  VIEW Ver: 25.02.00B.1125.14	<b>▲ Maximum Reactions (lbs), or *=PLF</b> Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B* 129 /- /- /69 /9 /14 Q 210 /- /- /99 /53 /- Wind reactions based on MWFRS B Brg Wid = 188 Min Req = - Q Brg Wid = 5.5 Min Req = 1.5 (Support) Bearings B & Q Fcperp = 425psi. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp. C - I 474 -225 I - P 472 -196
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**Lumber**  
Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #2;  
Lt Slider: 2x4 SP #2; block length = 1.500'

**Plating Notes**  
All plates are 1.5X4 except as noted.

**Purlins**  
In lieu of structural panels or rigid ceiling use purlins to laterally brace chords as follows:  
Chord Spacing(in oc) Start(ft) End(ft)  
BC 120 0.00 16.17  
Apply purlins to any chords above or below fillers at 24" OC unless shown otherwise above.

**Loading**  
Truss designed to support 1-4-0 top chord outlookers and cladding load not to exceed 3.00 PSF one face and 24.0' span opposite face. Top chord must not be cut or notched, unless specified otherwise.  
Bottom chord checked for 10.00 psf non-concurrent live load.

**Wind**  
Wind loads based on MWFRS with additional C&C member design.  
Right end vertical exposed to wind pressure.  
Deflection meets L/180.  
Wind loading based on both gable and hip roof types.

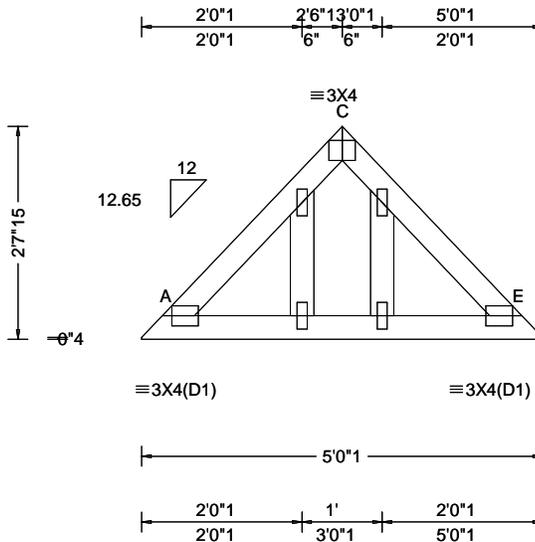
**Snow**  
Overhang designed for 2.00X TC LL.

**Additional Notes**  
See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.  
Top Chord overhang(s) may be field trimmed.



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Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria
TCLL: 20.00 TCDL: 5.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 35.00 NCBCLL: 0.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	Speed: 115 mph Risk Category: I Enclosure: Part. Enc. EXP: C Kzt: NA TCDL: 3.0 psf BCDL: 6.0 psf Mean Height: 0.00 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: NA GCpi: 0.55 Wind Duration: 0.00	Pg: 25.0 Ct: - CAT: - Pf(ASD): 16.8 Ce: - Lu: - Cs: - Snow Duration: -  Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.000 - - 240 VERT(CL): 0.000 E 999 180 HORZ(LL): 0.000 - - - HORZ(TL): 0.000 E - - - Creep Factor: 2.0 Max TC CSI: 0.001 Max BC CSI: 0.001 Max Web CSI: 0.000  VIEW Ver: 25.02.00B.1125.14

**Lumber**

Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #2;

**Plating Notes**

All plates are 1.5X4 except as noted.

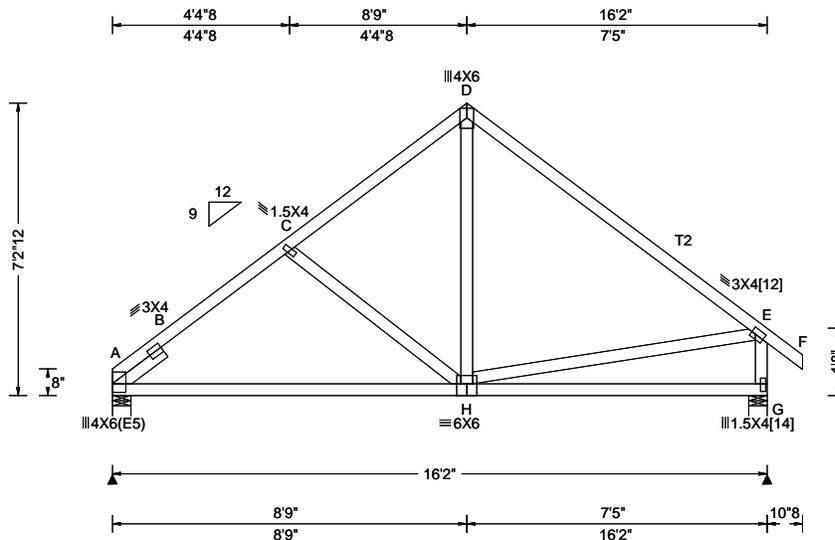
**Additional Notes**

This "Hip Frame" may be used in place of purlins on the hip plane to brace the flat top chord of hip trusses. See detail drawing HIPFRAME0623, HIPFR18000623, or HIPFRSCAB0623 for additional information.



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<b>Loading Criteria (psf)</b> TCLL: 25.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	<b>Wind Criteria</b> Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCDL: 6.0 psf BCDL: 6.0 psf Mean Height: 22.03 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.55 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.032 B 999 240 VERT(CL): 0.065 B 999 180 HORZ(LL): 0.025 B - - HORZ(TL): 0.050 B - - Creep Factor: 2.0 Max TC CSI: 0.574 Max BC CSI: 0.603 Max Web CSI: 0.188  VIEW Ver: 25.02.00B.1125.14	<b>▲ Maximum Reactions (lbs)</b> Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 768 /- /- /450 /43 /126 G 835 /- /- /454 /47 /- Wind reactions based on MWFRS A Brg Wid = 5.5 Min Req = 1.5 (Support) G Brg Wid = 5.5 Min Req = 1.5 (Support) Bearings A & G Fcperp = 425psi. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp. A - B 442 -1028 C - D 199 -712 B - C 197 -914 D - E 163 -786
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**Lumber**  
Top chord: 2x4 SP #2; T2 2x4 SP 2400f-2.0E;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #2;  
Lt Slider: 2x4 SP #2; block length = 1.500'

**Additional Notes**  
Top Chord overhang(s) may be field trimmed.

**Maximum Bot Chord Forces Per Ply (lbs)**  
Chords Tens.Comp.  
A - H 696 -100

**Plate Shift Table**

JT No	Plate Size	Lateral Shift	Chord Bite	JT No	Plate Size	Lateral Shift	Chord Bite
[12]	3X4	2.75 R	1.75	[14]	1.5X4	S	2.25

**Maximum Web Forces Per Ply (lbs)**  
Webs Tens.Comp. Webs Tens. Comp.  
H - E 447 0 E - G 185 -779

**Purlins**  
In lieu of structural panels or rigid ceiling use purlins to laterally brace chords as follows:  
Chord Spacing(in oc) Start(ft) End(ft)  
BC 120 0.00 16.17  
Apply purlins to any chords above or below fillers at 24" OC unless shown otherwise above.

**Loading**  
Bottom chord checked for 10.00 psf non-concurrent live load.

**Wind**  
Wind loads based on MWFRS with additional C&C member design.  
Right end vertical exposed to wind pressure.  
Deflection meets L/180.  
Wind loading based on both gable and hip roof types.

**Snow**  
Overhang designed for 2.00X TC LL.

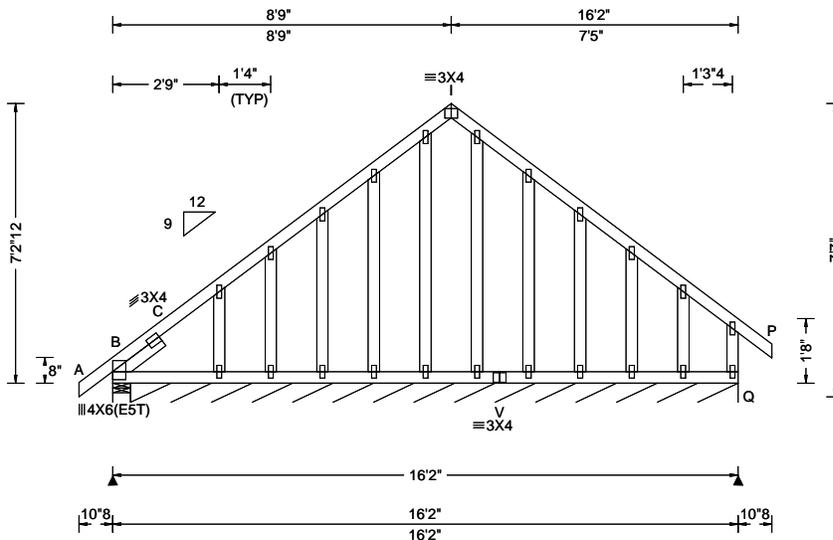


MISSOURI COA #2005000817  
01/12/2026

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**ALPINE**

**RELEASE FOR CONSTRUCTION**  
**AS NOTED BY PLANS REVIEW**  
**DEVELOPMENT SERVICES**  
LEE'S SUMMIT, MISSOURI  
02/12/2026 4:05:17



<b>Loading Criteria (psf)</b> TCLL: 25.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	<b>Wind Criteria</b> Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCDL: 6.0 psf BCDL: 6.0 psf Mean Height: 21.70 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.55 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.006 C 999 240 VERT(CL): 0.015 I 999 180 HORZ(LL): 0.005 C - - HORZ(TL): 0.032 O - - Creep Factor: 2.0 Max TC CSI: 0.199 Max BC CSI: 0.052 Max Web CSI: 0.196  VIEW Ver: 25.02.00B.1125.14	<b>▲ Maximum Reactions (lbs), or *=PLF</b> Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL B 254 /- /- /209 /90 /220 Q* 126 /- /- /72 /10 /- Wind reactions based on MWFRS B Brg Wid = 5.5 Min Req = 1.5 (Support) Q Brg Wid = 188 Min Req = - Bearings B & B Fcperp = 425psi. Members not listed have forces less than 375# <b>Maximum Top Chord Forces Per Ply (lbs)</b> Chords Tens.Comp. Chords Tens. Comp. C - I 474 -225 I - P 472 -196
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**Lumber**  
Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #2;  
Lt Slider: 2x4 SP #2; block length = 1.500'

**Plating Notes**  
All plates are 1.5X4 except as noted.

**Purlins**  
In lieu of structural panels or rigid ceiling use purlins to laterally brace chords as follows:  
Chord Spacing(in oc) Start(ft) End(ft)  
BC 120 0.00 16.17  
Apply purlins to any chords above or below fillers at 24" OC unless shown otherwise above.

**Loading**  
Truss designed to support 1-4-0 top chord outlookers and cladding load not to exceed 3.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.  
Bottom chord checked for 10.00 psf non-concurrent live load.

**Wind**  
Wind loads based on MWFRS with additional C&C member design.  
Right end vertical exposed to wind pressure.  
Deflection meets L/180.  
Wind loading based on both gable and hip roof types.

**Snow**  
Overhang designed for 2.00X TC LL.  
  
**Additional Notes**  
See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements.  
Top Chord overhang(s) may be field trimmed.

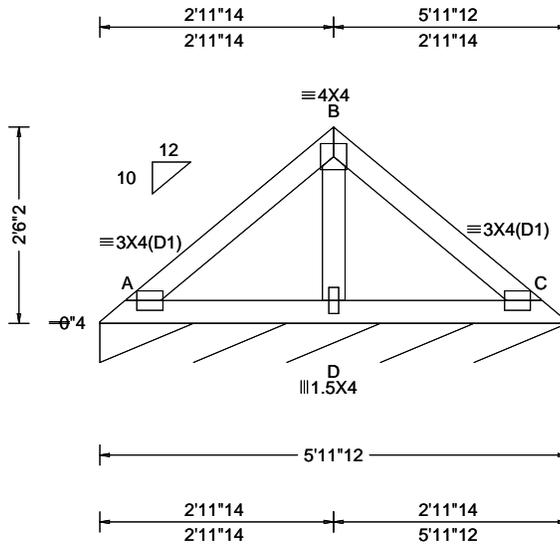


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SEQN: 5790 FROM:	VAL Ply: 1 Qty: 1	Job Number: PM000129 Customer - Clayton Properties Plan Name - Sienna Elevation - Farmhouse Truss Label: V5	Cust: R 9646 JRef: 1YGQ96460003 T9 DrwNo: 012.26.1518.19463 / BM 01/12/2026
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<b>Loading Criteria (psf)</b> TCCL: 25.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 45.00 NCBCLL: 0.00 Soffit: 0.00 Load Duration: 1.15 Spacing: 24.0 "	<b>Wind Criteria</b> Speed: 115 mph Risk Category: II Enclosure: Part. Enc. EXP: B Kzt: NA TCDL: 6.0 psf BCDL: 6.0 psf Mean Height: 22.51 ft MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: not in 4.50 ft GCpi: 0.55 Wind Duration: 1.60	<b>Snow Criteria (Pg,Pf in PSF)</b> Pg: NA Ct: NA CAT: NA Pf(ASD): NA Ce: NA Lu: NA Cs: NA Snow Duration: NA  Building Code: IRC 2018 Load Std: ASCE 7-16 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	<b>Defl/CSI Criteria</b> PP Deflection in loc L/defl L/# VERT(LL): 0.003 C 999 240 VERT(CL): 0.005 C 999 180 HORZ(LL): -0.002 C - - HORZ(TL): 0.003 C - - Creep Factor: 2.0 Max TC CSI: 0.136 Max BC CSI: 0.102 Max Web CSI: 0.038  VIEW Ver: 25.02.00B.1125.14	<b>▲ Maximum Reactions (lbs), or *=PLF</b> Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL C* 95 /- /- /51 /2 /6 Wind reactions based on MWFRS C Brg Wid = 71.7 Min Req = - Bearing A Fcperp = 425psi. Members not listed have forces less than 375#
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**Lumber**

Top chord: 2x4 SP #2;  
Bot chord: 2x4 SP #2;  
Webs: 2x4 SP #2;

**Purlins**

In lieu of structural panels or rigid ceiling use purlins to laterally brace chords as follows:  
Chord Spacing(in oc) Start(ft) End(ft)  
BC 72 0.00 5.98  
Apply purlins to any chords above or below fillers at 24" OC unless shown otherwise above.

**Wind**

Wind loads based on MWFRS with additional C&C member design.  
Wind loading based on both gable and hip roof types.

**Additional Notes**

See DWG VAL180160118 for valley details.



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# Gable Stud Reinforcement Detail

ASCE 7-16: 140 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

- Or: 120 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00
- Or: 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure D, Kzt = 1.00
- Or: 100 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

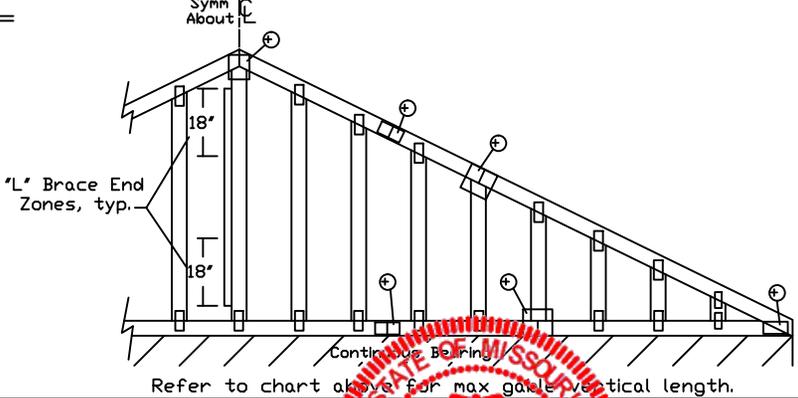
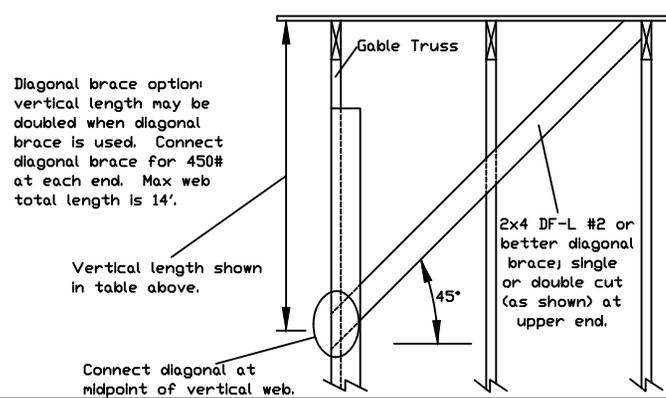
Max Gable Vertical Length	2x4 Gable Vertical Spacing		Brace Grade	No Braces	(1) 1x4 'L' Brace *		(1) 2x4 'L' Brace *		(2) 2x4 'L' Brace **		(1) 2x6 'L' Brace *		(2) 2x6 'L' Brace **		
	Species	Grade			Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	
	24" o.c.	SPF	#1 / #2	HF	#1 / #2	4' 3"	7' 3"	7' 7"	8' 7"	8' 11"	10' 3"	10' 8"	13' 6"	14' 0"	14' 0"
#3					4' 1"	6' 7"	7' 1"	8' 6"	8' 10"	10' 1"	10' 6"	13' 4"	13' 10"	14' 0"	14' 0"
Stud					4' 1"	6' 7"	7' 0"	8' 6"	8' 10"	10' 1"	10' 6"	13' 4"	13' 10"	14' 0"	14' 0"
Standard			#1	4' 1"	5' 8"	6' 0"	7' 7"	8' 1"	10' 1"	10' 6"	11' 10"	12' 8"	14' 0"	14' 0"	
			#2	4' 6"	7' 4"	7' 8"	8' 8"	9' 0"	10' 4"	10' 9"	13' 8"	14' 0"	14' 0"	14' 0"	
			#3	4' 3"	7' 3"	7' 7"	8' 7"	8' 11"	10' 3"	10' 8"	13' 6"	14' 0"	14' 0"	14' 0"	
SP		DFL	#1	4' 2"	6' 0"	6' 4"	7' 11"	8' 6"	10' 2"	10' 7"	12' 5"	13' 4"	14' 0"	14' 0"	
			Stud	4' 2"	6' 0"	6' 4"	7' 11"	8' 6"	10' 2"	10' 7"	12' 5"	13' 4"	14' 0"	14' 0"	
			Standard	4' 0"	5' 3"	5' 7"	7' 0"	7' 6"	9' 6"	10' 2"	11' 0"	11' 10"	14' 0"	14' 0"	
		SPF	#1 / #2	4' 11"	8' 4"	8' 8"	9' 10"	10' 3"	11' 8"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"	
			#3	4' 8"	8' 1"	8' 8"	9' 8"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	
			Stud	4' 8"	8' 1"	8' 6"	9' 8"	10' 1"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"	
SP	DFL	#1	4' 8"	6' 11"	7' 5"	9' 3"	9' 11"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"		
		Stud	4' 8"	6' 11"	7' 5"	9' 3"	9' 11"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"		
		Standard	4' 8"	6' 11"	7' 5"	9' 3"	9' 11"	11' 7"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"		
	SPF	#1	5' 1"	8' 5"	8' 9"	9' 11"	10' 4"	11' 10"	12' 4"	14' 0"	14' 0"	14' 0"	14' 0"		
		#2	4' 11"	8' 4"	8' 8"	9' 10"	10' 3"	11' 8"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"		
		#3	4' 9"	7' 4"	7' 9"	9' 9"	10' 2"	11' 8"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"		
SP	DFL	#1	4' 9"	7' 4"	7' 9"	9' 9"	10' 2"	11' 8"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"		
		Stud	4' 9"	7' 4"	7' 9"	9' 9"	10' 2"	11' 8"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"		
		Standard	4' 8"	6' 5"	6' 10"	8' 7"	9' 2"	11' 7"	12' 1"	13' 6"	14' 0"	14' 0"	14' 0"		
	SPF	#1 / #2	5' 5"	9' 2"	9' 6"	10' 10"	11' 3"	11' 8"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"		
		#3	5' 1"	9' 0"	9' 4"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"		
		Stud	5' 1"	9' 0"	9' 4"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"		
SP	DFL	#1	5' 1"	8' 0"	8' 6"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"		
		Stud	5' 1"	8' 0"	8' 6"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"		
		Standard	5' 1"	8' 0"	8' 6"	10' 8"	11' 1"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"		
	SPF	#1	5' 8"	9' 3"	9' 8"	10' 11"	11' 4"	13' 0"	13' 6"	14' 0"	14' 0"	14' 0"	14' 0"		
		#2	5' 5"	9' 2"	9' 6"	10' 10"	11' 3"	12' 11"	13' 5"	14' 0"	14' 0"	14' 0"	14' 0"		
		#3	5' 3"	8' 5"	9' 0"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"		
Standard	Stud	5' 3"	8' 5"	9' 0"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"			
	Stud	5' 3"	8' 5"	9' 0"	10' 9"	11' 2"	12' 10"	13' 4"	14' 0"	14' 0"	14' 0"	14' 0"			
	Standard	5' 1"	7' 5"	7' 11"	9' 11"	10' 7"	12' 9"	13' 3"	14' 0"	14' 0"	14' 0"	14' 0"			

**Bracing Group Species and Grades:**

Group A:			
Spruce-Pine-Fir		Hem-Fir	
#1 / #2	Standard	#2	Stud
#3	Stud	#3	Standard
Douglas Fir-Larch		Southern Pine***	
#3	Stud	#3	Stud
Standard		Standard	
Group B:			
Hem-Fir			
#1 & Btr			
#1			
Douglas Fir-Larch		Southern Pine***	
#1	Stud	#1	Stud
Standard		Standard	

1x4 Braces shall be SRB (Stress-Rated Board).  
 \*\*\*For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

**Gable Truss Detail Notes:**  
 Wind Load deflection criterion is L/240.  
 Provide uplift connections for 55 plf over continuous bearing (5 psf TC Dead Load).  
 Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12' plywood overhang.



Attach 'L' braces with 10d (0.128"x3.0" min) nails.  
 \* For (1) 'L' brace: space nails at 2' o.c. in 18' end zones and 4' o.c. between zones.  
 \*\* For (2) 'L' braces: space nails at 3' o.c. in 18' end zones and 6' o.c. between zones.  
 'L' bracing must be a minimum of 80% of web member length.

**Gable Vertical Plate Sizes**

Vertical Length	No Splice
Less than 4' 0"	1X4 or 2X3
Greater than 4' 0"	3X4

+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.

155 Harlem Ave  
 North Building, 4th Floor  
 Glenview, IL 60025

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For more information see this Job's general notes page and these web sites:  
 ALPINE: www.alpineitw.com; TPI: www.tpinet.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org

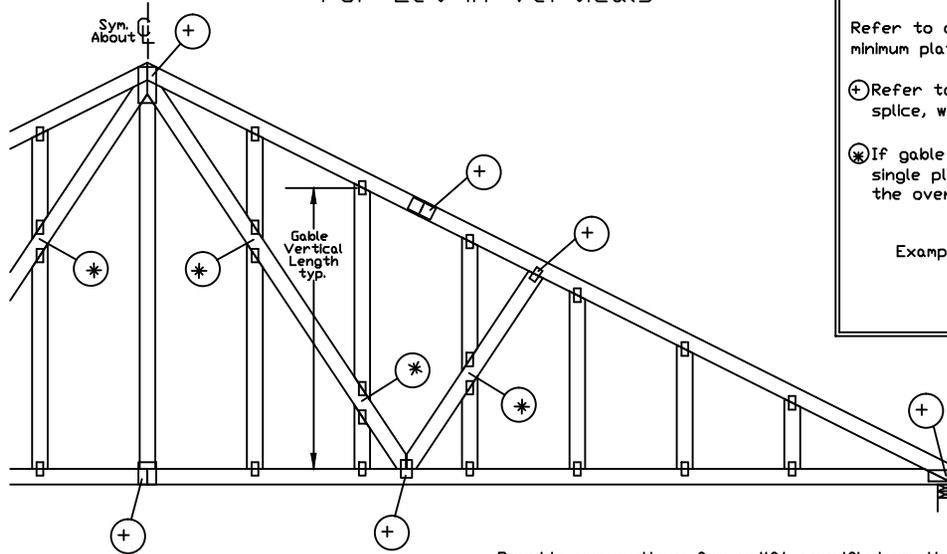
**BRADLEY E. MORRIS**  
 NUMBER  
 PE-2002019611  
 REGISTERED PROFESSIONAL ENGINEER  
 MISSOURI COA #2005000817

MAX. TOT. LD.	60 PSF
DATE	01/12/2026
MAX. SPACING	24.0"

REF	ASCE7-16-GAB14015
DATE	01/26/2018
DRWG	A14015ENC160118

RELEASE FOR CONSTRUCTION  
 AS NOTED ON PLANS REVIEW  
 DEVELOPMENT SERVICES  
 LEE'S SUMMIT, MISSOURI  
 02/12/2026 4:05:18

# Gable Detail For Let-in Verticals

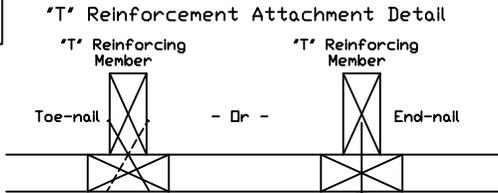


**Gable Truss Plate Sizes**

Refer to appropriate Alpine gable detail for minimum plate sizes for vertical studs.

- ⊕ Refer to Engineered truss design for peak, splice, web, and heel plates.
- ⊗ If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web.

Example:



Provide connections for uplift specified on the engineered truss design.

Attach each "T" reinforcing member with  
End Driven Nails:  
10d Common (0.148"x3",min) Nails at 4' o.c. plus  
(4) nails in the top and bottom chords.

Toenailed Nails:  
10d Common (0.148"x3",min) Toenails at 4' o.c. plus  
(4) toenails in the top and bottom chords.

This detail to be used with the appropriate Alpine gable detail for ASCE wind load.

- ASCE 7-05 Gable Detail Drawings  
A13015051014, A12015051014, A11015051014, A10015051014, A14015051014,  
A13030051014, A12030051014, A11030051014, A10030051014, A14030051014
- ASCE 7-10 & ASCE 7-16 Gable Detail Drawings  
A11515ENC100118, A12015ENC100118, A14015ENC100118, A16015ENC100118,  
A18015ENC100118, A20015ENC100118, A20015END100118, A20015PED100118,  
A11530ENC100118, A12030ENC100118, A14030ENC100118, A16030ENC100118,  
A18030ENC100118, A20030ENC100118, A20030END100118, A20030PED100118,  
S11515ENC100118, S12015ENC100118, S14015ENC100118, S16015ENC100118,  
S18015ENC100118, S20015ENC100118, S20015END100118, S20015PED100118,  
S11530ENC100118, S12030ENC100118, S14030ENC100118, S16030ENC100118,  
S18030ENC100118, S20030ENC100118, S20030END100118, S20030PED100118

See appropriate Alpine gable detail for maximum length for gable vertical length.

To convert from "L" to "T" reinforcing members, multiply "T" increase by length (based on appropriate Alpine gable detail).

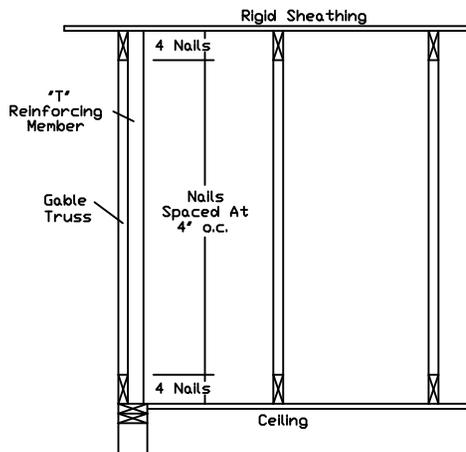
Maximum allowable "T" reinforced gable vertical length is 14' from top to bottom chord.

"T" reinforcing member material must match size, specie, and grade of the "L" reinforcing member.

**Web Length Increase w/ "T" Brace**

"T" Reinf. Mbr. Size	"T" Increase
2x4	30 %
2x6	20 %

Example:  
ASCE 7-10 Wind Speed = 120 mph  
Mean Roof Height = 30 ft, Kzt = 1.00  
Gable Vertical = 24' o.c. SP #3  
"T" Reinforcing Member Size = 2x4  
"T" Brace Increase (From Above) = 30% = 1.30  
(1) 2x4 "L" Brace Length = 8' 7"  
Maximum "T" Reinforced Gable Vertical Length  
1.30 x 8' 7" = 11' 2"



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**IMPORTANT: FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS**

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For more information see this Job's general notes page and these web sites:  
ALPINE: [www.alpineitw.com](http://www.alpineitw.com); TPI: [www.tpinst.org](http://www.tpinst.org); SBCA: [www.sbcacomponents.com](http://www.sbcacomponents.com); ICC: [www.iccsafe.org](http://www.iccsafe.org)

**BRADLEY E. MORRIS**  
NUMBER  
PE-2002019611  
**PROFESSIONAL ENGINEER**  
MISSOURI COA #2005000817

REF LET-IN VERT  
DATE 01/02/2018  
DRWG GBLLETIN0118

MAX. TOT. LD. 60 PSF  
DUR. FAC. ANY  
MAX. SPACING 24.0'

**RELEASE FOR CONSTRUCTION  
AS NOTED ON PLANS REVIEW  
DEVELOPMENT SERVICES  
LEE'S SUMMIT, MISSOURI  
02/12/2026 4:05:18**

**ALPINE**  
AN ITW COMPANY  
155 Harlem Ave  
North Building, 4th Floor  
Glenview, IL 60025

# Gable Stud Reinforcement Detail

ASCE 7-16: 140 mph Wind Speed, 30' Mean Height, Enclosed, Exposure C, Kzt = 1.00

- Or: 120 mph Wind Speed, 30' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00
- Or: 120 mph Wind Speed, 30' Mean Height, Enclosed, Exposure D, Kzt = 1.00
- Or: 100 mph wind speed, 30' Mean Height, Partially Enclosed, Exposure D, Kzt = 1.00

Max Gable Vertical Length	2x4 Gable Vertical Spacing		Brace Grade	No Braces	(1) 1x4 'L' Brace *		(1) 2x4 'L' Brace *		(2) 2x4 'L' Brace **		(1) 2x6 'L' Brace *		(2) 2x6 'L' Brace **	
	Species	#1 / #2			Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B
			24" o.c.	SPF	#1 / #2	#1	4' 1"	6' 11"	7' 2"	8' 2"	8' 6"	9' 9"	10' 2"	12' 10"
#3	3' 10"	6' 2"				6' 7"	8' 1"	8' 5"	9' 8"	10' 0"	12' 8"	13' 2"	14' 0"	14' 0"
Stud	3' 10"	6' 2"				6' 6"	8' 1"	8' 5"	9' 8"	10' 0"	12' 8"	13' 2"	14' 0"	14' 0"
HF	#1	3' 10"			5' 3"	5' 7"	7' 0"	7' 6"	9' 6"	10' 0"	11' 0"	11' 10"	14' 0"	14' 0"
	#3	4' 2"			7' 0"	7' 3"	8' 3"	8' 7"	9' 10"	10' 3"	13' 0"	13' 6"	14' 0"	14' 0"
	Stud	4' 2"			7' 0"	7' 3"	8' 3"	8' 7"	9' 10"	10' 3"	13' 0"	13' 6"	14' 0"	14' 0"
SP	#2	#1		4' 1"	6' 11"	7' 2"	8' 2"	8' 6"	9' 9"	10' 2"	12' 10"	13' 4"	14' 0"	14' 0"
		#3		4' 0"	5' 7"	5' 11"	7' 5"	7' 11"	9' 8"	10' 1"	11' 7"	12' 5"	14' 0"	14' 0"
		Stud		4' 0"	5' 7"	5' 11"	7' 5"	7' 11"	9' 8"	10' 1"	11' 7"	12' 5"	14' 0"	14' 0"
	DFL	#1		3' 9"	4' 11"	5' 13"	6' 6"	7' 0"	8' 10"	9' 6"	10' 3"	11' 0"	13' 11"	14' 0"
		#3		4' 10"	8' 0"	8' 4"	9' 6"	9' 10"	11' 3"	11' 9"	14' 0"	14' 0"	14' 0"	14' 0"
		Stud		4' 10"	8' 0"	8' 4"	9' 6"	9' 10"	11' 3"	11' 9"	14' 0"	14' 0"	14' 0"	14' 0"
16" o.c.	SPF	#1 / #2	#1	4' 8"	7' 11"	8' 3"	9' 4"	9' 9"	11' 2"	11' 7"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	4' 5"	7' 6"	8' 3"	9' 3"	9' 7"	11' 0"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	4' 5"	7' 6"	8' 0"	9' 3"	9' 7"	11' 0"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"
		HF	#1	4' 10"	8' 0"	8' 4"	9' 6"	9' 10"	11' 3"	11' 9"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	4' 8"	7' 11"	8' 3"	9' 4"	9' 9"	11' 2"	11' 7"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	4' 8"	7' 11"	8' 3"	9' 4"	9' 9"	11' 2"	11' 7"	14' 0"	14' 0"	14' 0"	14' 0"
	SP	#2	#1	4' 7"	6' 10"	7' 3"	8' 3"	8' 7"	9' 8"	11' 1"	11' 6"	14' 0"	14' 0"	14' 0"
			#3	4' 7"	6' 10"	7' 3"	8' 3"	8' 7"	9' 8"	11' 1"	11' 6"	14' 0"	14' 0"	14' 0"
			Stud	4' 7"	6' 10"	7' 3"	8' 3"	8' 7"	9' 8"	11' 1"	11' 6"	14' 0"	14' 0"	14' 0"
		DFL	#1	4' 5"	6' 0"	6' 5"	8' 0"	8' 7"	10' 10"	11' 6"	12' 7"	13' 15"	14' 0"	14' 0"
			#3	4' 5"	6' 0"	6' 5"	8' 0"	8' 7"	10' 10"	11' 6"	12' 7"	13' 15"	14' 0"	14' 0"
			Stud	4' 5"	6' 0"	6' 5"	8' 0"	8' 7"	10' 10"	11' 6"	12' 7"	13' 15"	14' 0"	14' 0"
12" o.c.	SPF	#1 / #2	#1	5' 2"	8' 9"	9' 1"	10' 4"	10' 9"	11' 2"	12' 9"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	4' 10"	8' 7"	8' 11"	10' 2"	10' 7"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	4' 10"	8' 7"	8' 11"	10' 2"	10' 7"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
		HF	#1	5' 4"	8' 10"	9' 2"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	5' 2"	8' 9"	9' 1"	10' 4"	10' 9"	11' 2"	12' 9"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	5' 2"	8' 9"	9' 1"	10' 4"	10' 9"	11' 2"	12' 9"	14' 0"	14' 0"	14' 0"	14' 0"
	SP	#2	#1	5' 4"	8' 10"	9' 2"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	5' 2"	8' 9"	9' 1"	10' 4"	10' 9"	11' 2"	12' 9"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	5' 2"	8' 9"	9' 1"	10' 4"	10' 9"	11' 2"	12' 9"	14' 0"	14' 0"	14' 0"	14' 0"
		DFL	#1	5' 0"	7' 10"	8' 4"	10' 3"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	5' 0"	7' 10"	8' 4"	10' 3"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
			Stud	5' 0"	7' 10"	8' 4"	10' 3"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"
Standard	4' 10"	6' 11"	7' 4"	9' 3"	9' 10"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"			

**Bracing Group Species and Grades:**

**Group A:**

Spruce-Pine-Fir		Hem-Fir	
#1 / #2	Standard	#2	Stud
#3	Stud	#3	Standard

Douglas Fir-Larch      Southern Pine\*\*\*

#3	#3
Stud	Stud
Standard	Standard

**Group B:**

Hem-Fir

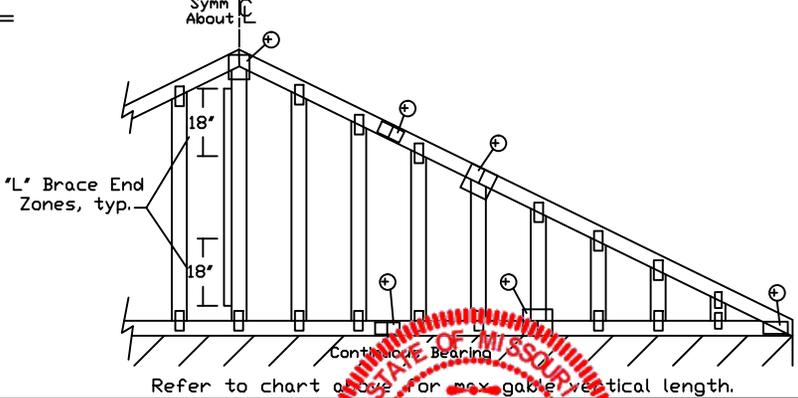
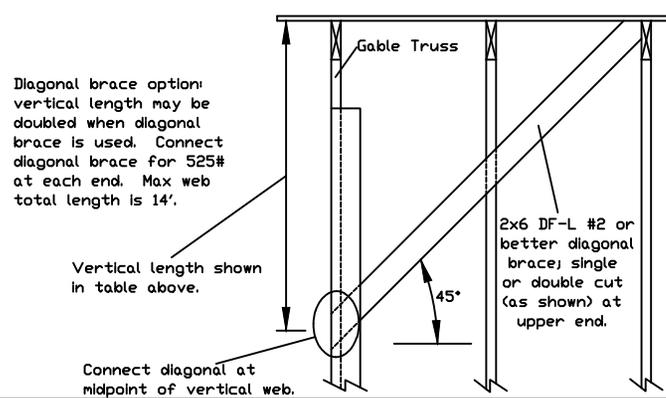
#1 & Btr
#1

Douglas Fir-Larch      Southern Pine\*\*\*

#1	#1
#2	#2

1x4 Braces shall be SRB (Stress-Rated Board).  
 \*\*\*For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

**Gable Truss Detail Notes:**  
 Wind Load deflection criterion is L/240.  
 Provide uplift connections for 100 plf over continuous bearing (5 psf TC Dead Load).  
 Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12' plywood overhang.



Attach 'L' braces with 10d (0.128"x3.0" min) nails.  
 \* For (1) 'L' brace: space nails at 2' o.c. in 18" end zones and 4' o.c. between zones.  
 \*\* For (2) 'L' braces: space nails at 3' o.c. in 18" end zones and 6' o.c. between zones.  
 'L' bracing must be a minimum of 80% of web member length.

Gable Vertical Plate Sizes	
Vertical Length	No Splice
Less than 4' 0"	2X4
Greater than 4' 0", but less than 11' 6"	3X4
Greater than 11' 6"	4X4

+ Refer to common truss design for peak, splice, and heel plates.

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155 Harlem Ave  
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Glenview, IL 60025

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 ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org

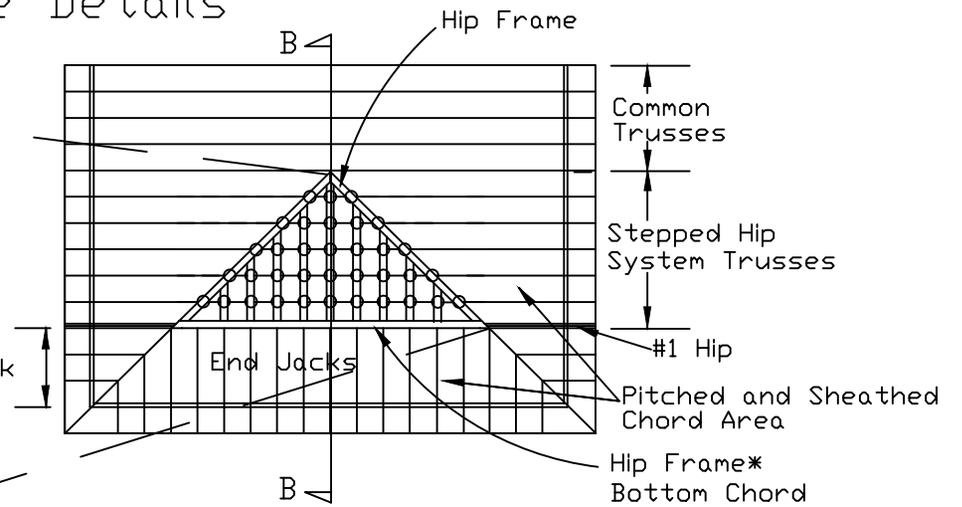
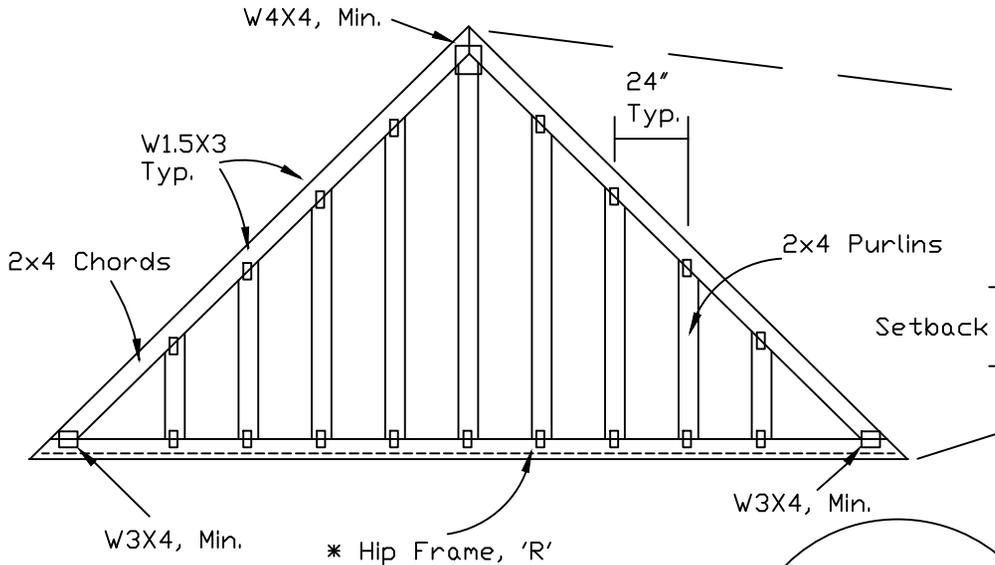
**BRADLEY E. MORRIS**  
 NUMBER  
 PE-2002019611  
 MISSOURI COA #2005000817

MAX. TOT. LD. 60 PSF  
 01/12/2026  
 MAX. SPACING 24.0"

REF ASCE7-16-GAB14030  
 DATE 01/26/2018  
 DRWG A14030ENC160118

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 AS NOTED ON PLANS REVIEW  
 DEVELOPMENT SERVICES  
 LEE'S SUMMIT, MISSOURI  
 02/12/2026 4:05:18

# \* Hip Frame Details



o- Attach hip frame to flat chords of stepped hips at all overlapping points with 2-10d (0.148"x3") common nails. Bottom chord of hip frame to be attached to #1 hip with 10d common (0.148"x3") nails @ 6" o.c. maximum spacing.

Hip frame stops at plumb cut of jacks to maintain pitch continuity.

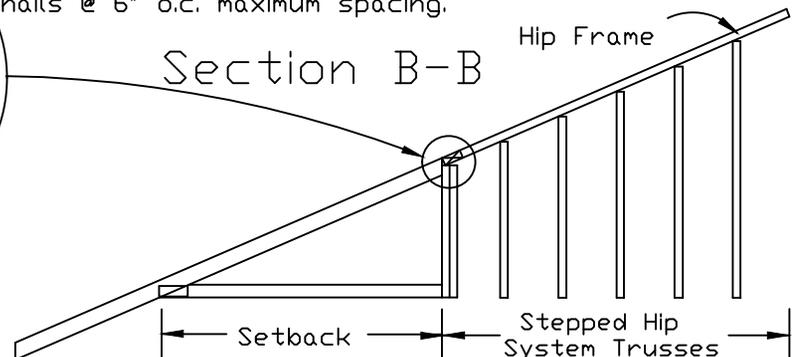
\* Hip frame lumber is SPF, So. Pine, HF, or DFL Standard, Stud grade, or better.

See Engineer's sealed design for setback, lumber, plating, loading, and duration factor required.

'R' Hip frame chords may be trimmed up to 2" to fit. purlins must be intact and properly attached.

Use this detail for:

- ASCE 7-22, 140 mph, 30' M.H., Enclosed, Exp C, or
- ASCE 7-22, 120 mph, 30' M.H., Enclosed, Exp D, or
- ASCE 7-10 & ASCE 7-16, 140 mph, 30' M.H., Enclosed, Exp C, or
- ASCE 7-10 & ASCE 7-16, 120 mph, 30' M.H., Enclosed, Exp D, or
- ASCE 7-05, 110 mph, 30' M.H., Enclosed, Cat II, Exp C, Residential, Wind TC DL=4.2 psf, Kzt=1.00



Hip Frame - provided by truss manufacturer. Hip frame is designed to provide bracing for flat top chords of hip frame system where indicated. On general panels must be properly attached directly to hip frame purlins.

**WARNING** READ AND FOLLOW ALL NOTES ON THIS DRAWING  
**IMPORTANT** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of trusses shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviations from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses.

A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see this Job's general notes page and these web sites:  
 ALPINE: [www.alpineitw.com](http://www.alpineitw.com); TPI: [www.tpinst.org](http://www.tpinst.org); SBCA: [www.sbcacomponents.com](http://www.sbcacomponents.com); ICC: [www.iccsafe.org](http://www.iccsafe.org)

BRADLEY  
 E. MORRIS  
 NUMBER  
 PE-2002019611



MISSOURI COA #2005000817

01/12/2026

REF	HIP FRAME
DATE	06/23/2023
DRWG	HIPFRAME0623

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155 Harlem Ave  
 North Building, 4th Floor  
 Glenview, IL 60025

# Valley Detail - ASCE 7-16: 180 mph, 30' Mean Height, Partially Enclosed, Exp. C, Kzt=1.00

Top Chord 2x4 SP #2N, SPF #1/#2, DF-L #2 or better.  
 Bot Chord 2x4 SP #2N or SPF #1/#2 or better.  
 Webs 2x4 SP #3, SPF #1/#2, DF-L #2 or better.

\*\* Attach each valley to every supporting truss with:  
 535# connection or with (1) Simpson H2.5A or equivalent connector for  
 ASCE 7-16 180 mph. 30' Mean Height, Part. Enc. Building, Exp. C, Wind TC DL=5 psf, Kzt = 1.00  
 Or  
 ASCE 7-16 160 mph. 30' Mean Height, Part. Enc. Building, Exp. D, Wind TC DL=5 psf, Kzt = 1.00

Bottom chord may be square or pitched cut as shown.

Valleys short enough to be cut as solid triangular members from a single 2x6, or larger as required, shall be permitted in lieu of fabricating from separate 2x4 members.

All plates shown are Alpine Wave Plates.

Unless specified otherwise on engineer's sealed design, for vertical valley webs taller than 7-9" apply 2x4 "T" reinforcement, 80% length of web, same species and grade or better, attached with 10d box (0.128" x 3.0") nails at 6" o.c. In lieu of "T" reinforcement, 2x4 Continuous Lateral Restraint applied at mid-length of web is permitted with diagonal bracing as shown in DRWG BRCLBANC1014.

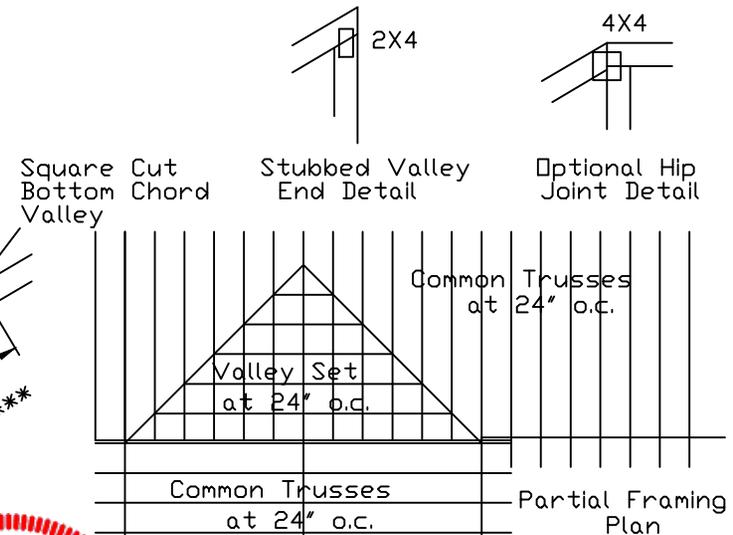
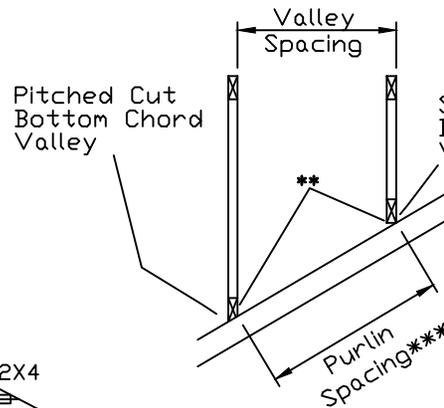
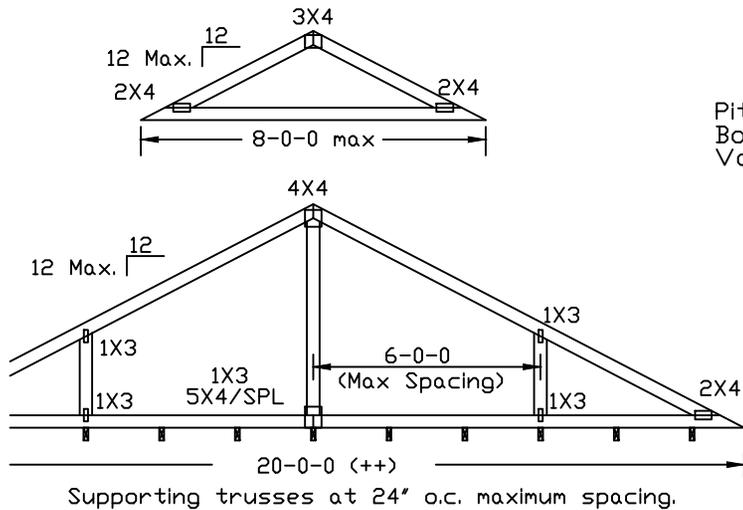
Top chord of truss beneath valley set must be braced with:  
 properly attached, rated sheathing applied prior to valley truss installation.

Or  
 Purlins at 24" o.c. or as otherwise specified on engineer's sealed design

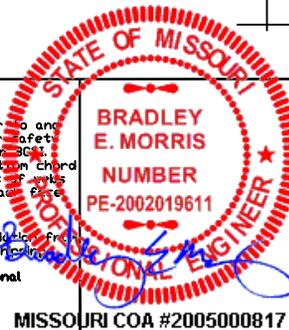
Or  
 By valley trusses used in lieu of purlin spacing as specified on Engineer's sealed design.

\*\*\* Note that the purlin spacing for bracing the top chord of the truss beneath the valley is measured along the slope of the top chord.

++ Larger spans may be built as long as the vertical height does not exceed 14'-0".



**WARNING: READ AND FOLLOW ALL NOTES ON THIS DRAWING**  
**IMPORTANT: FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.**  
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TC LL	30	30	40PSF	REF	VALLEY DETAIL
TC DL	20	15	7PSF	DATE	01/26/2018
BC DL	10	10	10 PSF	DRWG	VAL180160118
BC LL	0	0	0PSF		
TOT. LD.	60	55	57PSF		
DURFAC	25	1.33	1.15		
SPACING			24.0"		

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