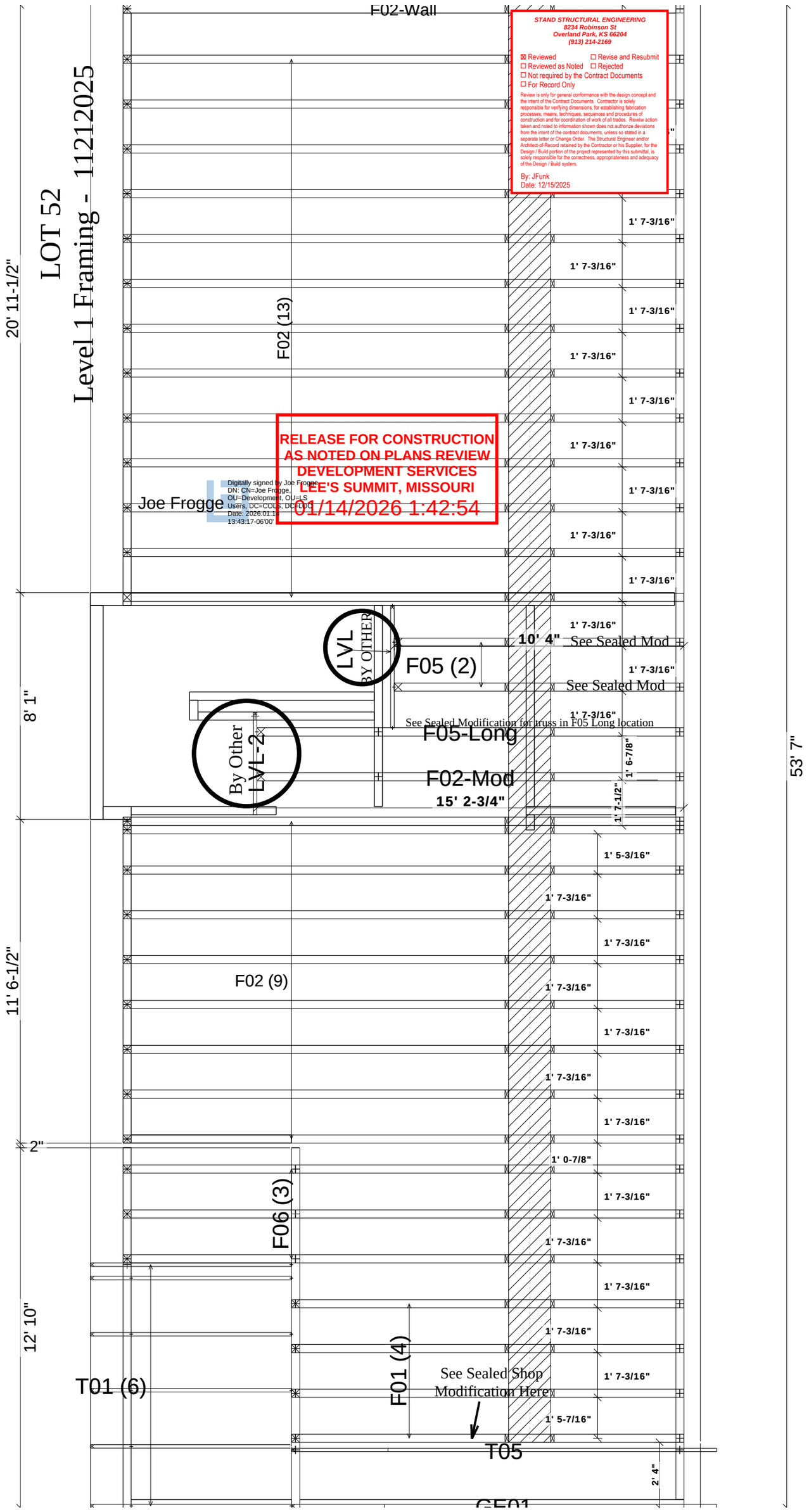
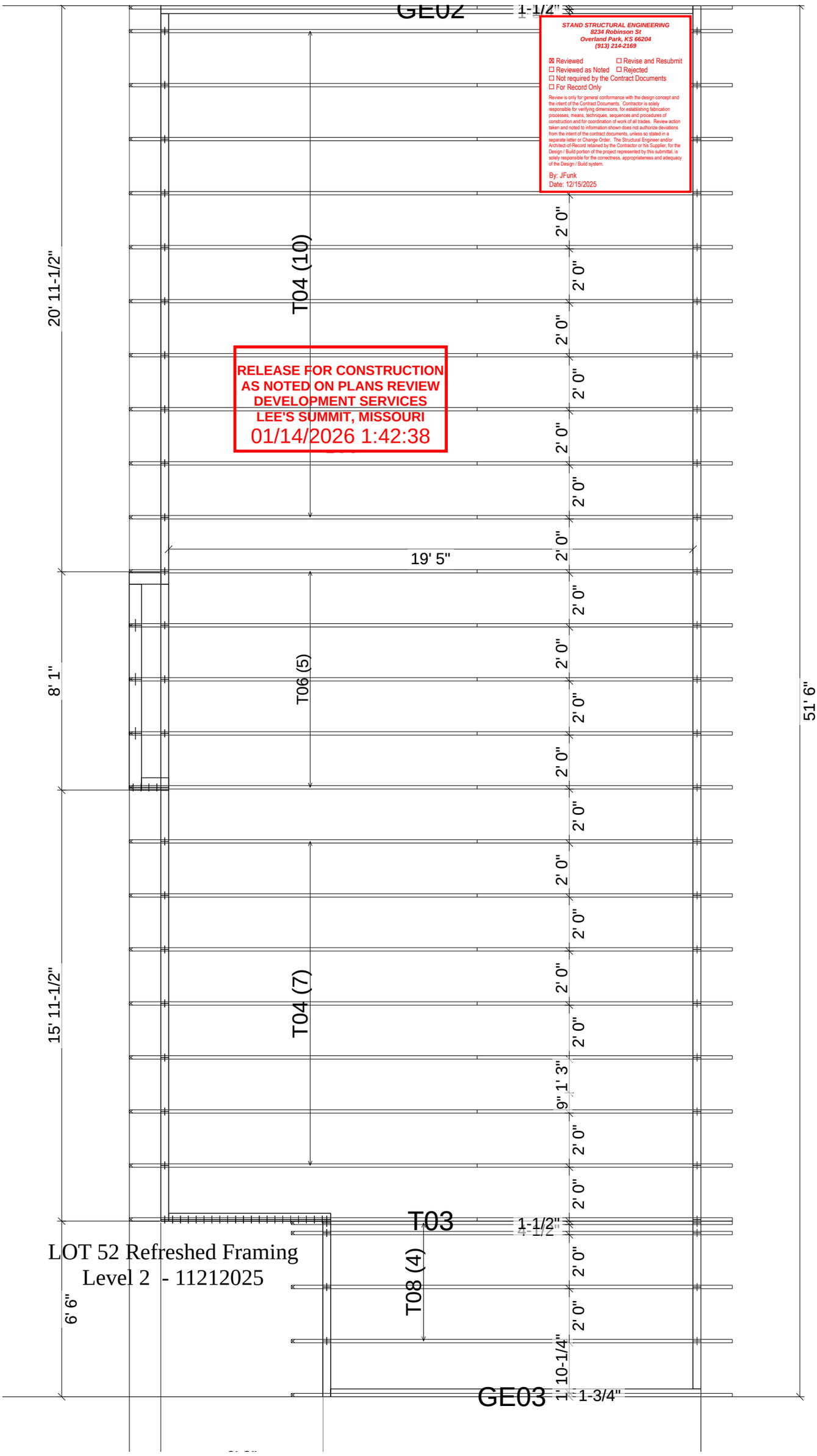

LOT 52
Level 1 Framing - 11212025
F02-Wall
STAND STRUCTURAL ENGINEERING
8234 Robinson St
Overland Park, KS 66204
(913) 214-2169
☒ Reviewed ☐ Revise and Resubmit
☐ Reviewed as Noted ☐ Rejected
☐ Not required by the Contract Documents
☐ For Record Only
Review is only for general conformance with the design concept and the intent of the Contract Documents. Contractor is solely responsible for verifying dimensions, for establishing fabrication processes, means, techniques, sequences and procedures of construction and for coordination of work of all trades. Review action taken and noted to information shown does not authorize deviations from the intent of the contract documents, unless so stated in a separate letter or Change Order. The Structural Engineer and/or Architect-of-Record retained by the Contractor or his Supplier, for the Design / Build portion of the project represented by this submittal, is solely responsible for the correctness, appropriateness and adequacy of the Design / Build system.
By: JFunk
Date: 12/15/2025
20' 11-1/2"
F02 (13)
1' 7-3/16"
RELEASE FOR CONSTRUCTION
AS NOTED ON PLANS REVIEW
DEVELOPMENT SERVICES
LEE'S SUMMIT, MISSOURI
01/14/2026 1:42:54
Joe Frogge
Digitally signed by Joe Frogge
DN: CN=Joe Frogge, OU=Development, OU=LS Users, DC=COLS, DC=LOC
Date: 2026.01.14 13:43:17-06'00'
LVL BY OTHER
F05 (2)
10' 4"
See Sealed Mod
8' 1"
By Other LVL-2
F05-Long
See Sealed Modification for truss in F05 Long location
F02-Mod
15' 2-3/4"
1' 6-7/8"
1' 7-1/2"
53' 7"
11' 6-1/2"
F02 (9)
1' 5-3/16"
2"
1' 0-7/8"
F06 (3)
12' 10"
T01 (6)
F01 (4)
See Sealed Shop Modification Here
1' 5-7/16"
T05
2' 4"
GE01



STAND STRUCTURAL ENGINEERING
8234 Robinson St
Overland Park, KS 66204
(913) 214-2169

☒ Reviewed ☐ Revise and Resubmit
☐ Reviewed as Noted ☐ Rejected
☐ Not required by the Contract Documents
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By: JFunk
Date: 12/15/2025

RELEASE FOR CONSTRUCTION
AS NOTED ON PLANS REVIEW
DEVELOPMENT SERVICES
LEE'S SUMMIT, MISSOURI
01/14/2026 1:42:38

LOT 52 Refreshed Framing
Level 2 - 11212025

EAGLE METAL

The truss designs referenced below have been prepared by me or under my direct supervision based on the truss design criteria and requirements ("design criteria") provided by **Quality Line Truss**.

These truss designs are intended for the fabrication of individual building components that will perform to the design criteria provided. Any variance from the design criteria will render the affected truss designs inapplicable.

Listed below are the truss designs included in this package and covered by this seal.

Job: **LOT 52_E2_REFRESHED FRAMING_11202025** - 1253819
GE01, GE02, GE03, T01, T03, T04, T05, T06, T08, F01, F02-Mod, F02-Wall, F02, F05-Long, F05, F06

Any location identification is for file reference only. No determination of the appropriateness of design criteria for any specific project has been made in preparing the truss designs.

Please refer to individual truss designs for specific design criteria.

RELEASE FOR CONSTRUCTION
AS NOTED ON PLANS REVIEW
DEVELOPMENT SERVICES
LEE'S SUMMIT, MISSOURI
01/14/2026 1:42:31

STAND STRUCTURAL ENGINEERING
8234 Robinson St
Overland Park, KS 66204
(913) 214-2169

☒ Reviewed ☐ Revise and Resubmit
☐ Reviewed as Noted ☐ Rejected
☐ Not required by the Contract Documents
☐ For Record Only

Review is only for general conformance with the design concept and the intent of the Contract Documents. Contractor is solely responsible for verifying dimensions, for establishing fabrication processes, means, techniques, sequences and procedures of construction and for coordination of work of all trades. Review action taken and noted to information shown does not authorize deviations from the intent of the contract documents, unless so stated in a separate letter or Change Order. The Structural Engineer and/or Architect-of-Record retained by the Contractor or his Supplier, for the Design / Build portion of the project represented by this submittal, is solely responsible for the correctness, appropriateness and adequacy of the Design / Build system.

By: JFunk
Date: 12/15/2025

Arturo A. Hernandez (MO, 2006000095)
My license expiration date for the state of MO is 12/31/2026.

IMPORTANT NOTE: The responsibility of the engineer sealing this package, as a Truss Engineer, is solely for design of individual trusses as individual building components based upon design criteria provided by others and set forth in the referenced truss drawings. The truss design criteria for the components have not been verified as appropriate for any particular building, project or use. Adequacy and suitability of design criteria and requirements for the truss designs for any specific project are the responsibility of the building designer, not the Truss Engineer, per ANSI/TPI-1, Chapter 2.

DESIGN NOTES

1. The Truss Design Drawing(s) provided with these Design Notes have been prepared under and are subject to ANSI / TPI 1 published by the Truss Plate Institute, www.tpinst.org. Capitalized terms have the meanings provided in ANSI / TPI 1.
2. Copies of each Truss Design Drawing shall be furnished to the installation contractor, Building Designer, Owner and all persons fabricating, handling, installing, bracing, or erecting the trusses.

DESIGN LIMITATIONS

3. The Truss Design Drawing is based upon specifications provided by the Building Designer in accordance with ANS1 / TPI 1. Neither the Truss Designer, Eagle, nor an engineer who seals this design (if any) assumes any responsibility for the adequacy or accuracy of specifications provided by the Building Designer.
4. The Building Designer is solely responsible for the suitability based upon the Truss Design Drawing and shall be responsible for reviewing and verifying that the information shown is in general conformance with the design of the Building.
5. Each Truss Design Drawing is for the individual building component (a truss). A seal on the Truss Design Drawing indicates acceptance of professional engineering responsibility solely for the individual truss.
6. Each Truss Design Drawing assumes trusses will be suitably protected from the environment.

HANDLING, INSTALLING, & BRACING

7. Refer to Building Component Safety Information (BCSI) for handling, installing, restraining and bracing trusses. Copies can be obtained from the Structural Building Components Association, www.sbcindustry.com.
8. Bracing shown on each Truss Design Drawing is for lateral support of individual truss components only to reduce buckling lengths. All temporary and permanent bracing, including lateral load and diagonal or cross bracing, are the responsibility, respectively, of the erector and Building Designer.
9. Eagle is not responsible for improper truss fabrication, handling, erection or bracing.
10. Compression chords shall be laterally braced by the roof or floor sheathing, directly attached, or have purlins provided at spacing shown, unless noted otherwise.
11. Bottom chord required bracing shall be at 1Oft spacing or less, if no structural rated ceiling is installed, unless noted otherwise.
12. Strongbacking shall be installed on all parallel chord trusses, including flooring systems, to limit deflection and reduce vibration. Refer to BCSI-B7.
13. Never exceed the design loading shown. Never stack building or other materials on inadequately braced truss; refer to BCSI.
14. Concentration of construction loads greater than the design loads shall not be applied to the trusses at any time; refer to BCSI.
15. Trusses shall be handled with care prior to erection to avoid damage. Refer to BCSI for recommended truss handling and erection.

MATERIALS & FABRICATION

16. Lumber moisture content shall be 19% or less at the time of fabrication unless noted otherwise.
17. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
18. Unless expressly noted, the truss designs are not applicable for use with fire retardant or preservative treated lumber.
19. Plates shall be applied on both faces of truss at each joint and embedded fully. Knots and wane at joint locations shall be regulated in accordance with ANSI / TPI 1.
20. For a specified plate gauge and grade, the specified size is a minimum.
21. Connections not shown are the responsibility of others.
22. Adequate support shall be provided to resist gravity, lateral and uplift loads.
23. For 4X2 truss orientation, locate plates 0 - 1/16" from outside the edge of the truss.
24. Fabrication of truss shall be in accordance with ANSI / TPI 1.

OTHER NOTES

25. Camber is a non-structural consideration and is the responsibility of truss fabricator.
26. Do not cut or alter any truss member or plate without prior approval from a professional engineer.
27. Lumber design values are in accordance with ANSI / TPI 1; lumber design values are by others.
28. Install specified hangers per manufacturer recommendations.

SYMBOLS

PLATE SIZE

3X4 - The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

-, /, I, Indicates required direction of slots; Reference "Joint Details" for more information.

20 Ga Gr40 connectors required
3X10-20HS - 20 Ga Gr60 connectors required
8X10-18HS - 18 Ga Gr60 connectors required

LATERAL BRACING

When this symbol shown, continuous lateral bracing is required on the member of the truss.

BEARING

Indicates location where bearings (supports) occur.

PLATE LOCATION & ORIENTATION

The plate shall be centered on joint and/or placed in accordance with the design drawing/QC full scale details.

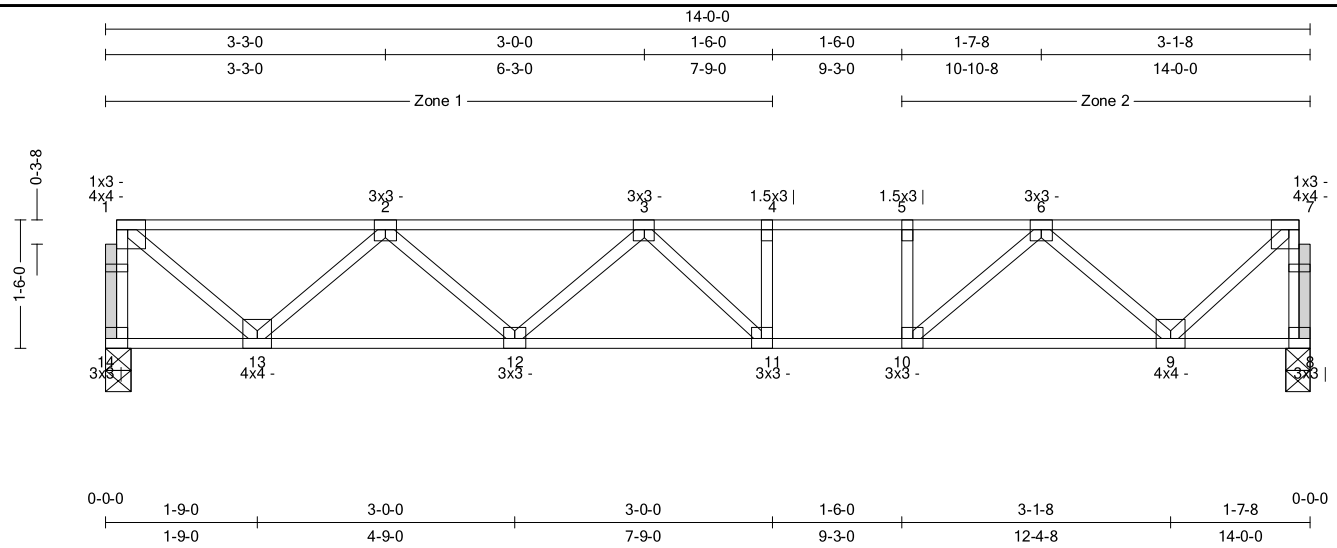
REFERENCES

- **ANSI / TPI 1:** National Design Standard for Metal Plate Connected Wood Trusses
- **BCSI:** Building Component & Safety Information - Guide to Good Practice for Handling, Installing, Restraining, & Bracing of Metal Plate Connected Wood Trusses.
- **NDS:** National Design Specification for Wood Construction
- **ESR:** 1082 published by the International Code Council. www.icc-es.org

Quality Line Truss Co., LLC
34593 S 4350 RD
Address 2
Adair, OK 74330

Truss:F01
Job: LOT 52_E2_REFRESHED FRAMING_
Date: 12/08/25 10:28:43
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	PLY(S)	SPACING	WGT/PLY
14-0-0	0/12	4	0-0-0	0-0-0	1	19.19 in	72 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL :	40	Bldg Code :	IBC 2018/ TPI 1-2014	TC :	0.26 (5-6)	Vert TL:	0.16 in	L / 996	(11-12)	L / 240
TCDL :	10	Rep Mbr :	Yes	BC :	0.33 (10-11)	Vert LL:	0.1 in	L / 999	(11-12)	L / 360
BCLL :	0	Lumber D.O.L. :	100 %	Web :	0.16 (1-13)	Horz TL:	0.02 in		8	
BCDL :	10									

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
14	1	3.5 in	1.50 in	783 lbs	·	·	·	·	·
8	1	3.5 in	1.50 in	783 lbs	·	·	·	·	·

12/08/2025

STATE OF MISSOURI
ARTURO A. HERNANDEZ
NUMBER PE-2006000095
PROFESSIONAL ENGINEER

Material

TC: SYP 2400/1.8 4 x 2
BC: SYP 2400/1.8 4 x 2
Web: SYP #1 4 x 2

Loads

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.208		(-743 lbs)	3-4	0.222		(-1,806 lbs)	5-6	0.258	(-1,806 lbs)
	2-3	0.208		(-1,710 lbs)	4-5	0.247		(-1,806 lbs)	6-7	0.208	(-676 lbs)
BC	9-10	0.289	1,327 lbs		11-12	0.307	1,896 lbs				
	10-11	0.326	1,806 lbs		12-13	0.207	1,380 lbs				
Web	1-14	0.081		(-759 lbs)	2-12	0.074	448 lbs		7-9	0.155	935 lbs
	1-13	0.164	990 lbs		6-10	0.116	638 lbs		7-8	0.080	(-755 lbs)
	2-13	0.105		(-864 lbs)	6-9	0.107		(-883 lbs)			

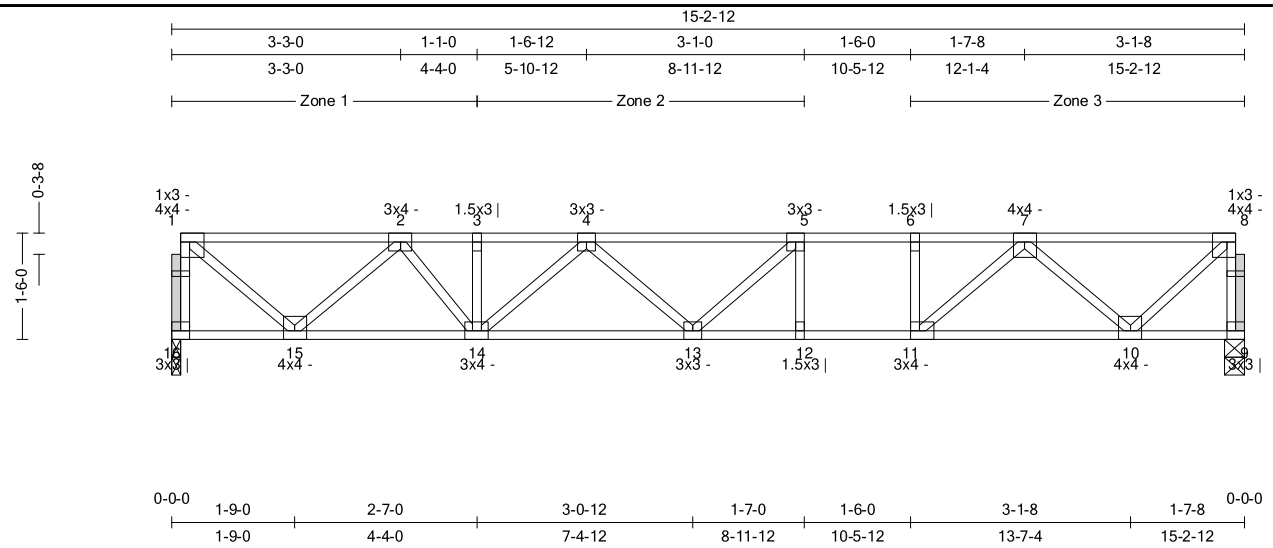
Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
4) A creep factor of 2.00 has been applied for this truss analysis.
5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
6) Indicates non-structural members.

TrueBuild®Truss Software v5.8.14
Eagle Metal Products

	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss:F02-Mod Job: LOT 52_E2_REFRESHED FRAMING_ Date: 12/08/25 10:29:01 Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	PLY(S)	SPACING	WGT/PLY
15-2-12	0/12	1	0-0-0	0-0-0	1	19.19 in	80 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL :	40	Bldg Code :	IBC 2018/	TC :	0.37 (4-5)	Vert TL:	0.21 in	L / 835	(12-13)	L / 240
TCDL :	10		TPI 1-2014	BC :	0.59 (11-12)	Vert LL:	0.13 in	L / 999	(12-13)	L / 360
BCLL :	0	Rep Mbr :	No	Web :	0.18 (1-15)	Horz TL:	0.03 in		9	
BCDL :	10	Lumber D.O.L. :	100 %							

12/08/2025

STATE OF MISSOURI
ARTURO A. HERNANDEZ
NUMBER PE-2006000095
PROFESSIONAL ENGINEER

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
16	1	1.5 in	1.50 in	852 lbs	·	·	·	·	·
9	1	3.5 in	1.50 in	852 lbs	·	·	·	·	·

Material

TC: SYP 2400/1.8 4 x 2
BC: SYP 2400/1.8 4 x 2
Web: SYP #1 4 x 2

Loads

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.239		(-820 lbs)	3-4	0.240		(-1,824 lbs)	5-6	0.349		(-2,079 lbs)	7-8	0.248		(-737 lbs)
	2-3	0.211		(-1,824 lbs)	4-5	0.369		(-2,240 lbs)	6-7	0.364		(-2,079 lbs)				
BC	10-11	0.456	1,480 lbs		12-13	0.593	2,079 lbs		14-15	0.175	1,528 lbs					
	11-12	0.593	2,079 lbs		13-14	0.294	2,198 lbs									
Web	1-16	0.088		(-828 lbs)	4-14	0.061		(-498 lbs)	8-9	0.087		(-814 lbs)				
	1-15	0.181	1,092 lbs		7-11	0.141	798 lbs									
	2-15	0.117		(-961 lbs)	7-10	0.122		(-1,008 lbs)								
	2-14	0.079	478 lbs		8-10	0.169	1,019 lbs									

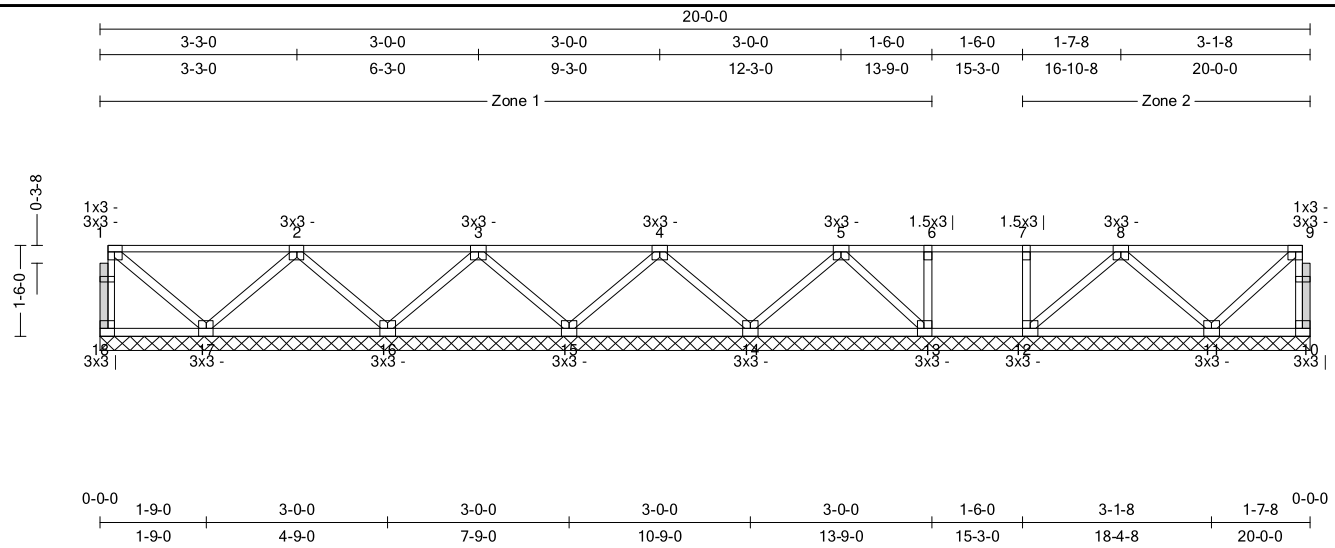
Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
4) A creep factor of 2.00 has been applied for this truss analysis.
5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
6) Indicates non-structural members.

TrueBuild®Truss Software v5.8.14
Eagle Metal Products

	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss:F02-Wall Job: LOT 52_E2_REFRESHED FRAMING_ Date: 12/08/25 10:28:45 Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	PLY(S)	SPACING	WGT/PLY
20-0-0	0/12	1	0-0-0	0-0-0	1	19.19 in	99 lbs



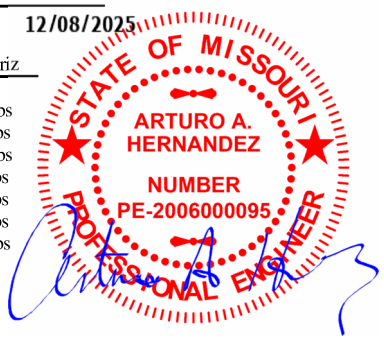
All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	40	Bldg Code:	IBC 2018/	TC:	0.69 (1-2)	Vert TL:	0.01 in	L / 999	(11-12)	L / 240
TCDL:	10		TPI 1-2014	BC:	0.08 (14-15)	Vert LL:	0.01 in	L / 999	(11-12)	L / 360
BCLL:	0	Rep Mbr:	No	Web:	0.07 (2-17)	Horz TL:	0 in			
BCDL:	10	Lumber D.O.L.:	100 %							

Reaction

Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	240 in	N/A	234 lbs	·	·	·	·	·
1	240 in	N/A	442 lbs	·	·	·	·	-309 lbs
1	240 in	N/A	585 lbs	·	·	·	·	317 lbs
1	240 in	N/A	579 lbs	·	·	·	·	-263 lbs
1	240 in	N/A	651 lbs	·	·	·	·	-90 lbs
1	240 in	N/A	699 lbs	·	·	·	·	37 lbs
1	240 in	N/A	730 lbs	·	·	·	·	-50 lbs
1	240 in	N/A	474 lbs	·	·	·	·	357 lbs
1	240 in	N/A	246 lbs	·	·	·	·	·

12/08/2025



Material

TC: SYP 2400/1.8 4 x 2
BC: SYP 2400/1.8 4 x 2
Web: SYP #1 4 x 2

Loads

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	20-0-0	Down	Proj	120 plf	120 plf	

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC								
BC								
Web	2-17	0.072	(-533 lbs)	4-15	0.064	(-472 lbs)	8-12	0.057	(-423 lbs)
	2-16	0.068	(-500 lbs)	4-14	0.063	(-469 lbs)	8-11	0.064	(-472 lbs)
	3-16	0.058	(-433 lbs)	5-14	0.047	(-347 lbs)			
	3-15	0.057	(-422 lbs)	5-13	0.048	(-363 lbs)			

Notes

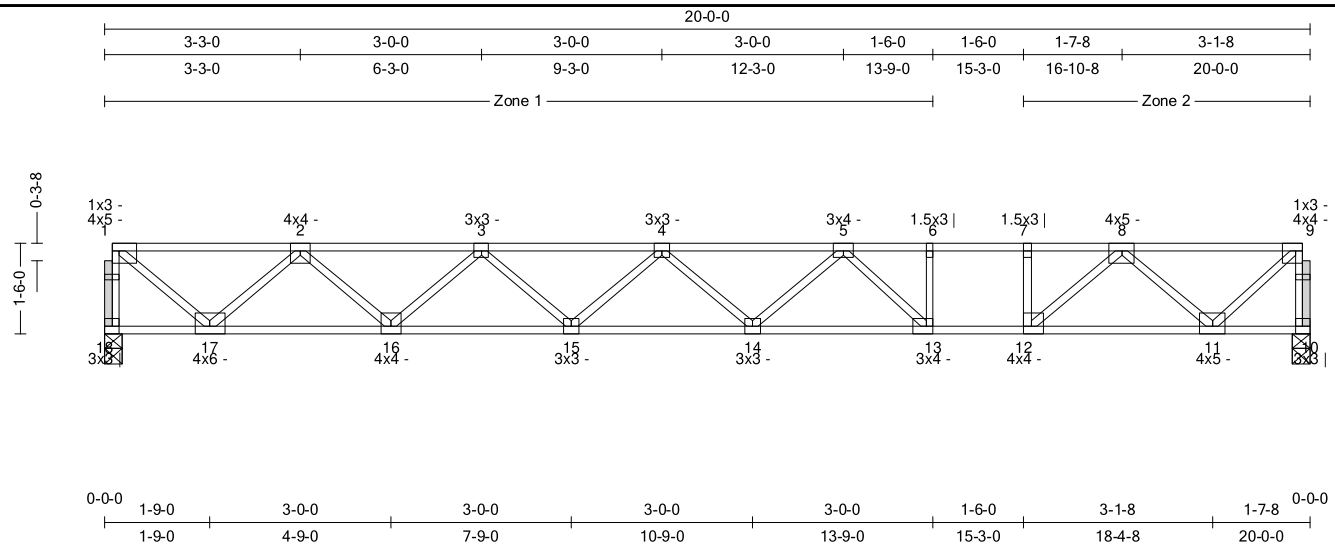
1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
4) A creep factor of 2.00 has been applied for this truss analysis.
5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
6) ▭ Indicates non-structural members.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes.
This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

TrueBuild®Truss Software v5.8.14
Eagle Metal Products

	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss:F02 Job: LOT 52_E2_REFRESHED FRAMING_ Date: 12/08/25 10:28:46 Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	PLY(S)	SPACING	WGT/PLY
20-0-0	0/12	22	0-0-0	0-0-0	1	19.19 in	101 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL :	40	Bldg Code :	IBC 2018/	TC :	0.61 (7-8)	Vert TL:	0.57 in	L / 411	(13-14)	L / 240
TCDL :	10		TPI 1-2014	BC :	0.72 (13-14)	Vert LL:	0.32 in	L / 726	(13-14)	L / 360
BCLL :	0	Rep Mbr :	Yes	Web :	0.25 (1-17)	Horz TL:	0.06 in		10	
BCDL :	10	Lumber D.O.L. :	100 %							

12/08/2025

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
18	1	3.5 in	1.50 in	1,119 lbs	·	·	·	·	·
10	1	3.5 in	1.50 in	1,119 lbs	·	·	·	·	·

Material

TC: SYP 2400/1.8 4 x 2
BC: SYP 2400/1.8 4 x 2
Web: SYP #1 4 x 2

Loads

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.213		(-1,127 lbs)	3-4	0.330		(-3,719 lbs)	5-6	0.588		(-3,102 lbs)	7-8	0.610		(-3,102 lbs)
	2-3	0.250		(-2,817 lbs)	4-5	0.351		(-3,953 lbs)	6-7	0.586		(-3,102 lbs)	8-9	0.248		(-991 lbs)
BC	11-12	0.628	2,050 lbs		13-14	0.716	3,739 lbs		15-16	0.326	3,397 lbs					
	12-13	0.710	3,102 lbs		14-15	0.468	3,963 lbs		16-17	0.226	2,123 lbs					
Web	1-18	0.117		(-1,097 lbs)	3-16	0.095		(-786 lbs)	6-13	0.069	400 lbs		9-11	0.227	1,371 lbs	
	1-17	0.249	1,501 lbs		3-15	0.072	436 lbs		7-12	0.063		(-573 lbs)	9-10	0.114		(-1,065 lbs)
	2-17	0.164		(-1,352 lbs)	4-15	0.042		(-331 lbs)	8-12	0.234	1,401 lbs					
	2-16	0.156	942 lbs		5-13	0.112		(-881 lbs)	8-11	0.174		(-1,438 lbs)				

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
4) A creep factor of 2.00 has been applied for this truss analysis.
5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
6) Indicates non-structural members.

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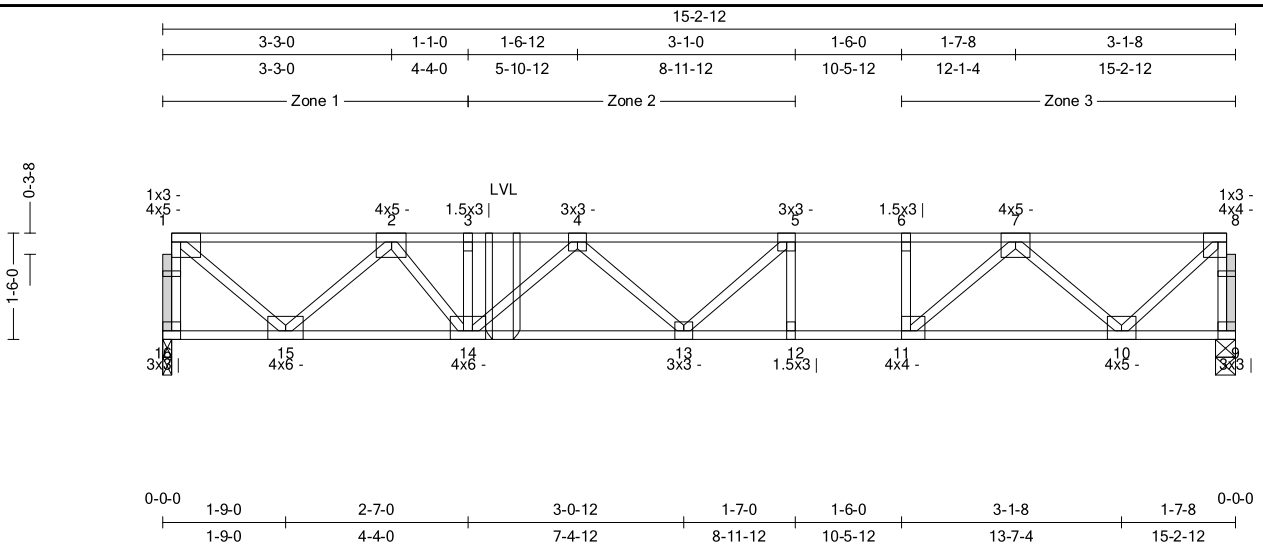
	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss:F05-Long Job: LOT 52_E2_REFRESHED FRAMING_ Date: 12/08/25 10:29:04 Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	PLY(S)	SPACING	WGT/PLY
15-2-12	0/12	1	0-0-0	0-0-0	1	19.19 in	81 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	**General**		**CSI**		**Deflection**		**L/**	**(loc)**	**Allowed**
Carried Loads (psf)	Bldg Code :	IBC 2018/	TC :	0.52 (6-7)	Vert TL:	0.29 in	L / 615	(12-13)	L / 240
TCLL : 40		TPI 1-2014	BC :	0.83 (12-13)	Vert LL:	0.18 in	L / 999	(12-13)	L / 360
TCDL : 10	Rep Mbr :	No	Web :	0.25 (1-15)	Horz TL:	0.04 in		9	
BCLL : 0	Lumber D.O.L. :	100 %							
BCDL : 10									

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
16	1	1.5 in	1.50 in	983 lbs	·	·	·	·	·
9	1	3.5 in	1.50 in	973 lbs	·	·	·	·	·

12/08/2025

STATE OF MISSOURI
ARTURO A. HERNANDEZ
NUMBER PE-2006000095
PROFESSIONAL ENGINEER

Material

TC: SYP 2400/1.8 4 x 2
BC: SYP 2400/1.8 4 x 2
Web: SYP #1 4 x 2

Loads

1) This truss has been designed for the effects of (psf) sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has not been designed for the effects of unbalanced snow loads.
3) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case L1: Std Live Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	15-2-12	Down	Proj	31.98 plf	31.98 plf	
Top	4-10-0	15-2-12	Down	Proj	31.98 plf	31.98 plf	

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	15-2-12	Down	Proj	7.99 plf	7.99 plf	
Top	4-10-0	15-2-12	Down	Proj	7.99 plf	7.99 plf	
Bot	0-0-0	15-2-12	Down	Proj	7.99 plf	7.99 plf	
Bot	4-10-0	15-2-12	Down	Proj	7.99 plf	7.99 plf	

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.150		(-1,037 lbs)	3-4	0.487		(-2,633 lbs)	5-6	0.493		(-2,543 lbs)	7-8	0.269	(-850 lbs)
	2-3	0.245		(-2,633 lbs)	4-5	0.507		(-2,918 lbs)	6-7	0.519		(-2,543 lbs)			
BC	10-11	0.627	1,741 lbs		12-13	0.829	2,543 lbs		14-15	0.217	1,994 lbs				
	11-12	0.829	2,543 lbs		13-14	0.398	3,036 lbs								
Web	1-16	0.111		(-969 lbs)	3-14	0.054		(-446 lbs)	6-11	0.045		(-373 lbs)	8-9	0.101	(-926 lbs)
	1-15	0.248	1,382 lbs		4-14	0.065		(-537 lbs)	7-11	0.187	1,069 lbs				
	2-15	0.168		(-1,299 lbs)	5-13	0.097	507 lbs		7-10	0.153		(-1,208 lbs)			
	2-14	0.189	1,032 lbs		5-12	0.036		(-300 lbs)	8-10	0.198	1,176 lbs				

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SPAN	PITCH	QTY	OHL	OHR	PLY(S)	SPACING	WGT/PLY
15-2-12	0/12	1	0-0-0	0-0-0	1	19.19 in	81 lbs

Truss to Truss Connection Summary

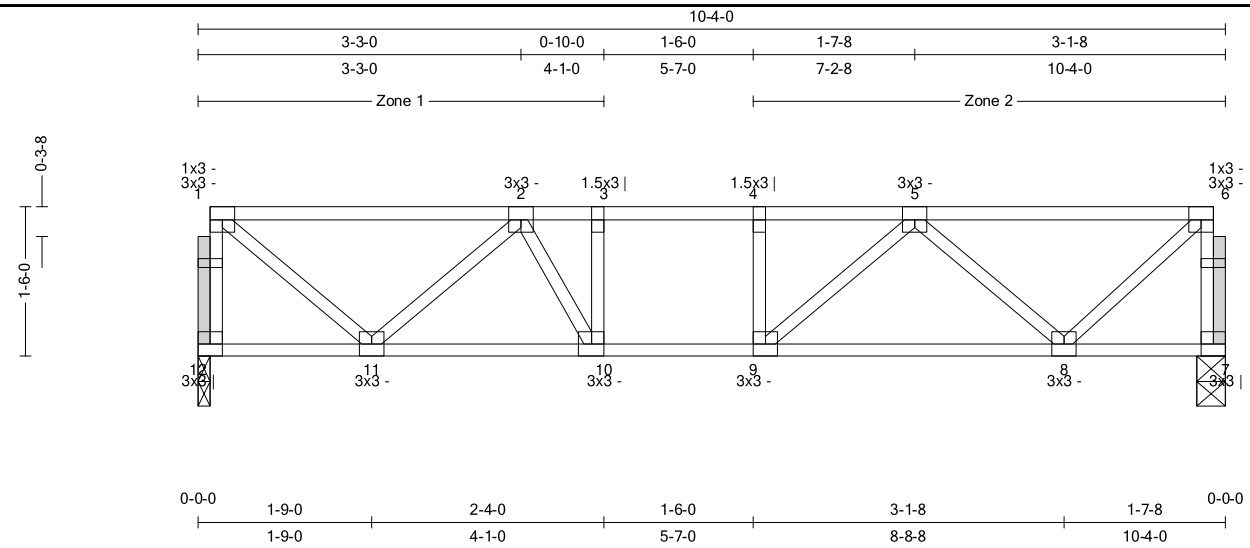
Carried Truss	Carrying Chord	Carrying Offset
LVL	TC	4-10-0

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
4) A creep factor of 2.00 has been applied for this truss analysis.
5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
6) ▭ Indicates non-structural members.

	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss:F05 Job: LOT 52_E2_REFRESHED FRAMING_ Date: 12/08/25 10:29:02 Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	PLY(S)	SPACING	WGT/PLY
10-4-0	0/12	2	0-0-0	0-0-0	1	19.19 in	55 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL :	40	Bldg Code :	IBC 2018/	TC :	0.38 (1-2)	Vert TL:	0.06 in	L / 999	(8-9)	L / 240
TCDL :	10		TPI 1-2014	BC :	0.30 (9-10)	Vert LL:	0.03 in	L / 999	(8-9)	L / 360
BCLL :	0	Rep Mbr :	No	Web :	0.11 (1-11)	Horz TL:	0.01 in		7	
BCDL :	10	Lumber D.O.L. :	100 %							

12/08/2025

STATE OF MISSOURI
ARTURO A. HERNANDEZ
NUMBER PE-2006000095
PROFESSIONAL ENGINEER

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
12	1	1.5 in	1.50 in	578 lbs	·	·	·	·	·
7	1	3.5 in	1.50 in	578 lbs	·	·	·	·	·

Material

TC: SYP#1 4 x 2
BC: SYP#1 4 x 2
Web: SYP#1 4 x 2

Loads

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.380		(-505 lbs)	3-4	0.244		(-1,009 lbs)	5-6	0.346	(-483 lbs)
	2-3	0.323		(-1,009 lbs)	4-5	0.300		(-1,009 lbs)			
BC	8-9	0.264	884 lbs		10-11	0.268	925 lbs				
	9-10	0.295	1,009 lbs								
Web	1-12	0.059		(-551 lbs)	5-8	0.067		(-544 lbs)			
	1-11	0.111	672 lbs		6-8	0.111	669 lbs				
	2-11	0.070		(-570 lbs)	6-7	0.060		(-565 lbs)			

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
4) A creep factor of 2.00 has been applied for this truss analysis.
5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
6) ▭ Indicates non-structural members.

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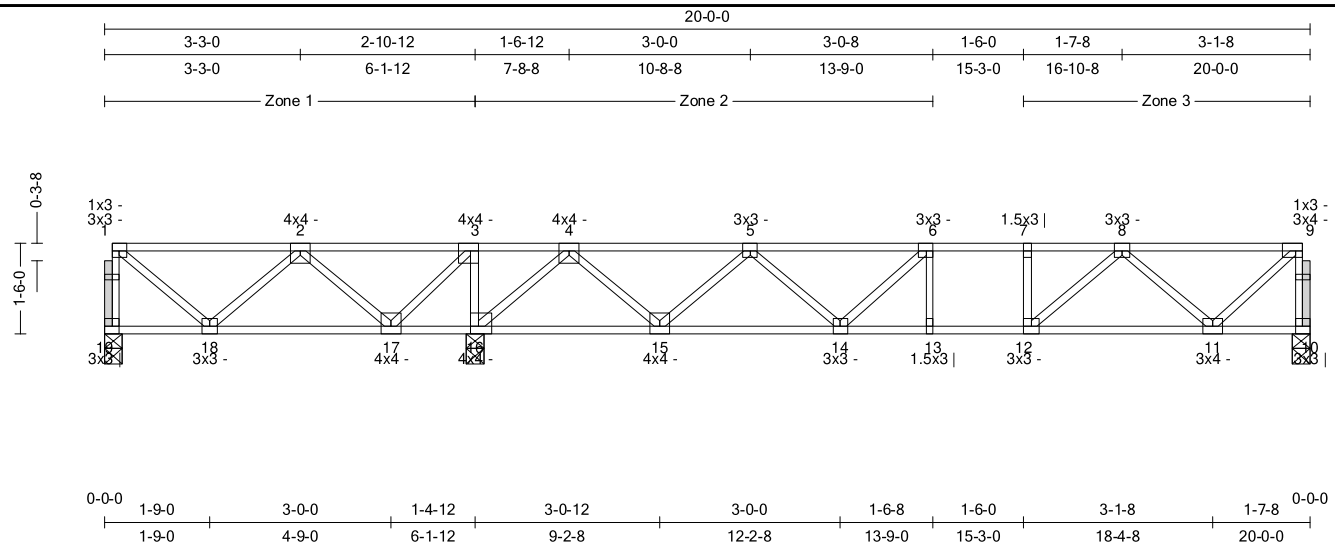
	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss:F06 Job: LOT 52_E2_REFRESHED FRAMING_ Date: 12/08/25 10:28:48 Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	PLY(S)	SPACING	WGT/PLY
20-0-0	0/12	3	0-0-0	0-0-0	1	19.19 in	102 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL :	40	Bldg Code :	IBC 2018/	TC :	0.66 (2-3)	Vert TL:	0.06 in	L / 999	(13-14)	L / 240
TCDL :	10		TPI 1-2014	BC :	0.19 (12-13)	Vert LL:	0.05 in	L / 999	(13-14)	L / 360
BCLL :	0	Rep Mbr :	Yes	Web :	0.19 (3-17)	Horz TL:	0.01 in		10	
BCDL :	10	Lumber D.O.L. :	100 %							

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
16	1	3.5 in	1.50 in	1,970 lbs	·	·	·	·	·
19	1	3.5 in	1.50 in	552 lbs	·	·	·	·	·
10	1	3.5 in	1.50 in	632 lbs	·	·	·	·	·

12/08/2025

Material

TC: SYP 2400/1.8 4 x 2
BC: SYP 2400/1.8 4 x 2
Web: SYP #1 4 x 2

Loads

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	6-1-12	Down	Proj	120 plf	120 plf	

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.513		(-322 lbs)	3-4	0.358	1,423 lbs		6-7	0.097		(-1,231 lbs)	8-9	0.190		(-532 lbs)
	2-3	0.663	648 lbs		5-6	0.187		(-1,042 lbs)	7-8	0.158		(-1,231 lbs)				
BC	11-12	0.160	1,004 lbs		14-15	0.123	747 lbs		17-18	0.085	584 lbs					
	12-13	0.186	1,231 lbs		15-16	0.055		(-545 lbs)								
	13-14	0.186	1,231 lbs		16-17	0.112		(-1,423 lbs)								
Web	1-19	0.066		(-542 lbs)	3-16	0.126		(-1,113 lbs)	8-12	0.060	317 lbs					
	1-18	0.082	429 lbs		4-16	0.143		(-1,178 lbs)	8-11	0.078		(-640 lbs)				
	2-18	0.048		(-356 lbs)	4-15	0.157	945 lbs		9-11	0.122	736 lbs					
	2-17	0.132		(-1,042 lbs)	5-15	0.097		(-799 lbs)	9-10	0.065		(-612 lbs)				
	3-17	0.187	1,090 lbs		5-14	0.066	400 lbs									

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
4) A creep factor of 2.00 has been applied for this truss analysis.
5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
6) Indicates non-structural members.

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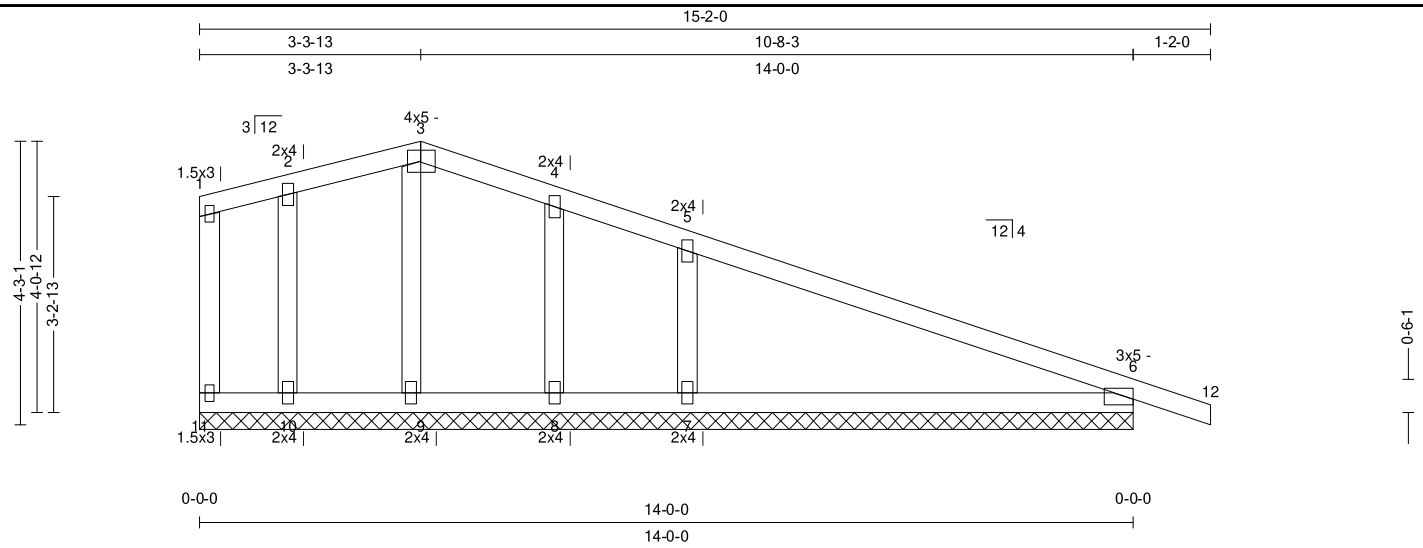
Eagle Metal Products

	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss:GE01 Job: LOT 52_E2_REFRESHED FRAMING_ Date: 12/08/25 10:28:49 Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
14-0-0	3/12	1	0-0-0	1-2-0	0-0-0	0-0-0	1	24 in	63 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	20	Bldg Code:	IBC 2018/	TC:	0.43 (5-6)	Vert TL:	0.03 in	L / 999	(6-7)	L / 240
TCDL:	10		TPI 1-2014	BC:	0.16 (6-7)	Vert LL:	0 in UP	L / 999	(6-7)	L / 360
BCLL:	0	Rep Mbr:	No	Web:	0.09 (1-11)	Horz TL:	0 in			
BCDL:	10	Lumber D.O.L.:	115 %							

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	·	1,495 lbs	215 plf	-1,131 lbs	-212 lbs	-742 lbs	-1,131 lbs	-808 lbs

12/08/2025

STATE OF MISSOURI
ARTURO A. HERNANDEZ
NUMBER PE-2006000095
PROFESSIONAL ENGINEER

Material

TC: SYP#1 2 x 4
BC: SYP#1 2 x 4
Web: SYP#2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (20 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL= 1.60

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	5-6	0.433	1,314 lbs	(-693 lbs)
BC				
Web	5-7	0.057		(-391 lbs)

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Gable requires continuous bottom chord bearing.
3) Gable webs placed at 24 " OC, U.N.O.
4) Attach gable webs with 2x4 20ga plates, U.N.O.
5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
6) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
7) A creep factor of 2.00 has been applied for this truss analysis.
8) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
9) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 6 may need to be considered.
10) Listed wind uplift reactions based on MWFRS & C&C loading.

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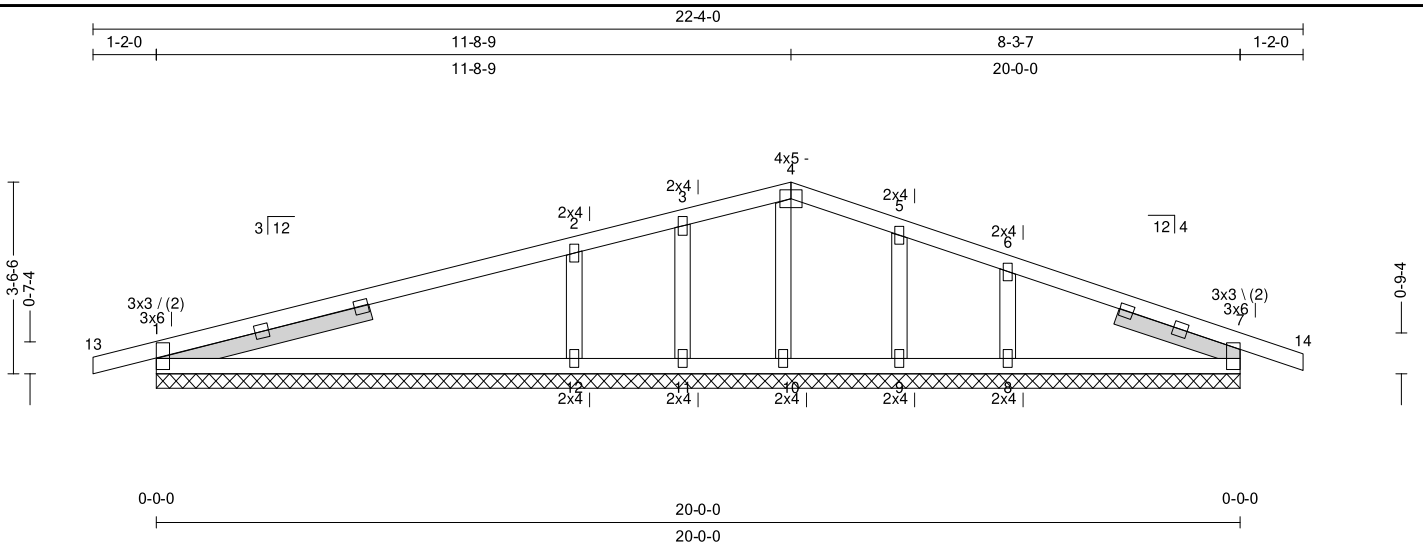
	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss:GE02 Job: LOT 52_E2_REFRESHED FRAMING_ Date: 12/08/25 10:28:51 Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
20-0-0	3/12	1	1-2-0	1-2-0	0-0-0	0-0-0	1	24 in	87 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2018/	TC:	0.30 (1-2)	Vert TL:	0.03 in	L / 999	(12-1)	L / 240
TCDL: 10		TPI 1-2014	BC:	0.19 (12-1)	Vert LL:	0 in	L / 999	7	L / 360
BCLL: 0	Rep Mbr:	No	Web:	0.04 (2-12)	Horz TL:	0 in			
BCDL: 10	Lumber D.O.L.:	115 %							

12/08/2025

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	-	914 lbs	133 plf	-403 lbs	-159 lbs	-370 lbs	-403 lbs	972 lbs

Material

TC: SYP#1 2 x 4
BC: SYP#1 2 x 4
Web: SYP#2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.301	938 lbs	(-440 lbs)
	6-7	0.124	333 lbs	
BC				
Web				

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Gable requires continuous bottom chord bearing.
3) Gable webs placed at 24 " OC, U.N.O.
4) Attach gable webs with 2x4 20ga plates, U.N.O.
5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
6) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
7) A creep factor of 2.00 has been applied for this truss analysis.
8) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
9) ▭ Indicates non-structural members.
10) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 7, 1 may need to be considered.
11) Listed wind uplift reactions based on MWFRS & C&C loading.

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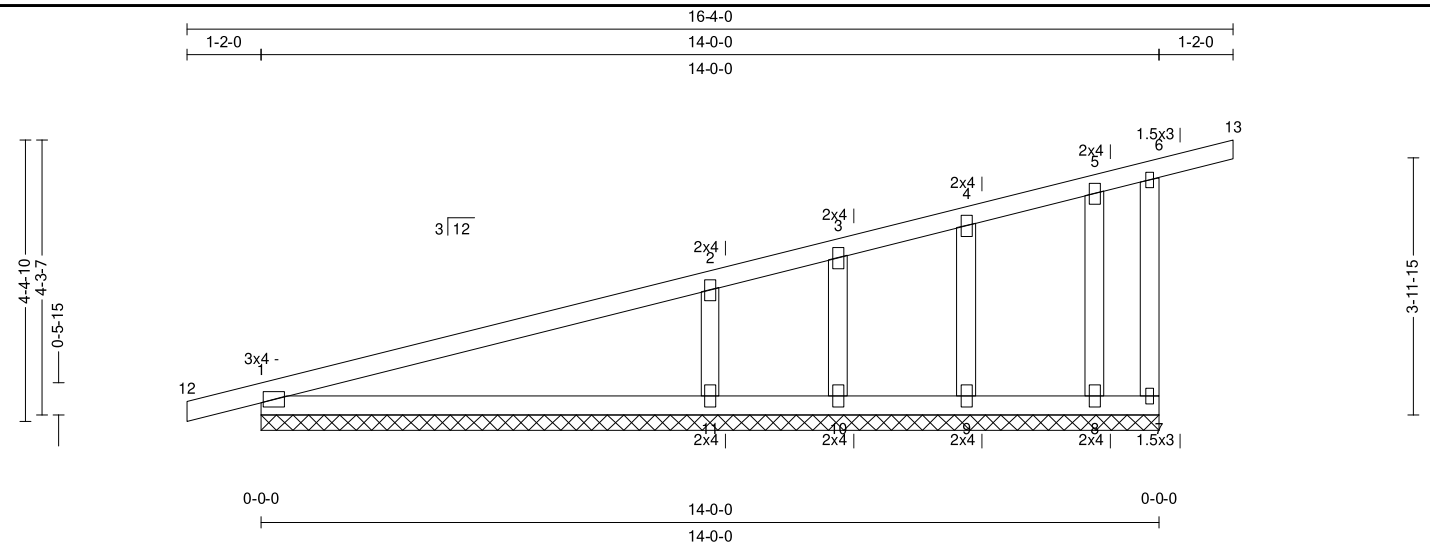
Eagle Metal Products

	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss:GE03 Job: LOT 52_E2_REFRESHED FRAMING_ Date: 12/08/25 10:28:52 Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
14-0-0	3/12	1	1-2-0	1-2-0	0-0-0	0-0-0	1	24 in	62 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2018/	TC:	0.64 (1-2)	Vert TL:	0.03 in	L / 999	(11-1)	L / 240
TCDL: 10		TPI 1-2014	BC:	0.18 (11-1)	Vert LL:	0 in	L / 999	(11-1)	L / 360
BCLL: 0	Rep Mbr:	No	Web:	0.15 (6-7)	Horz TL:	0 in			
BCDL: 10	Lumber D.O.L.:	115 %							

12/08/2025

Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	-	1,583 lbs	221 plf	-1,138 lbs	-172 lbs	-624 lbs	-1,138 lbs	-1,106 lbs

Material

TC: SYP#1 2 x 4
BC: SYP#1 2 x 4
Web: SYP#2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (20 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL= 1.60

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.639	1,527 lbs	(-676 lbs)
BC				
Web	2-11	0.053		(-412 lbs)

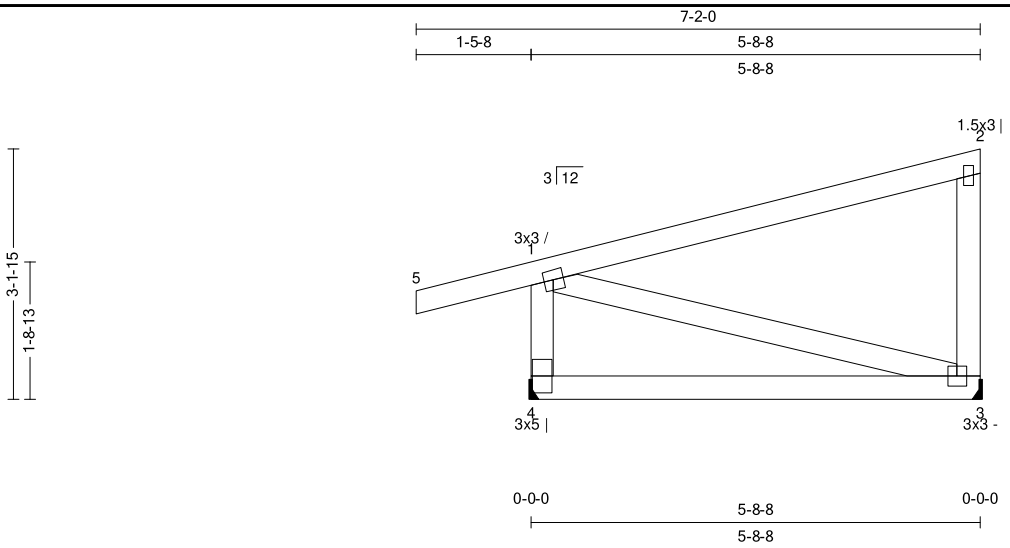
Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Gable requires continuous bottom chord bearing.
3) Gable webs placed at 24 " OC, U.N.O.
4) Attach gable webs with 2x4 20ga plates, U.N.O.
5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
6) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
7) A creep factor of 2.00 has been applied for this truss analysis.
8) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
9) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 8, 10, 1 may need to be considered.
10) Listed wind uplift reactions based on MWFRS & C&C loading.

TrueBuild®Truss Software v5.8.14
Eagle Metal Products

	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss:T01 Job: LOT 52_E2_REFRESHED FRAMING_ Date: 12/08/25 10:28:53 Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
5-8-8	3/12	6	1-5-8	0-0-0	0-0-0	0-0-0	1	24 in	31 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2018/	TC:	0.39 (1-2)	Vert TL:	0.09 in	L / 706	(3-4)	L / 240
TCDL: 10		TPI 1-2014	BC:	0.29 (3-4)	Vert LL:	0.05 in	L / 999	(3-4)	L / 360
BCLL: 0	Rep Mbr:	Yes	Web:	0.11 (2-3)	Horz TL:	0 in		3	
BCDL: 10	Lumber D.O.L.:	115 %							

12/08/2025

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
4	1	1.5 in	---	387 lbs	·	-55 lbs	-401 lbs	-401 lbs	135 lbs
3	1	1.5 in	---	271 lbs	·	-34 lbs	-248 lbs	-248 lbs	·

Material

TC: SYP#1 2 x 4
BC: SYP#1 2 x 4
Web: SYP#1 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (20 psf) sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has not been designed for the effects of unbalanced snow loads.
3) This truss has been designed to account for the effects of ice dams forming at the eaves.
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
5) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC											
BC											
Web	1-4	0.076	435 lbs								

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Hangers are for graphical interpretation only. Install hangers per manufacturer's recommendations.
4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
5) A creep factor of 2.00 has been applied for this truss analysis.
6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
7) Listed wind uplift reactions based on MWFRS & C&C loading.

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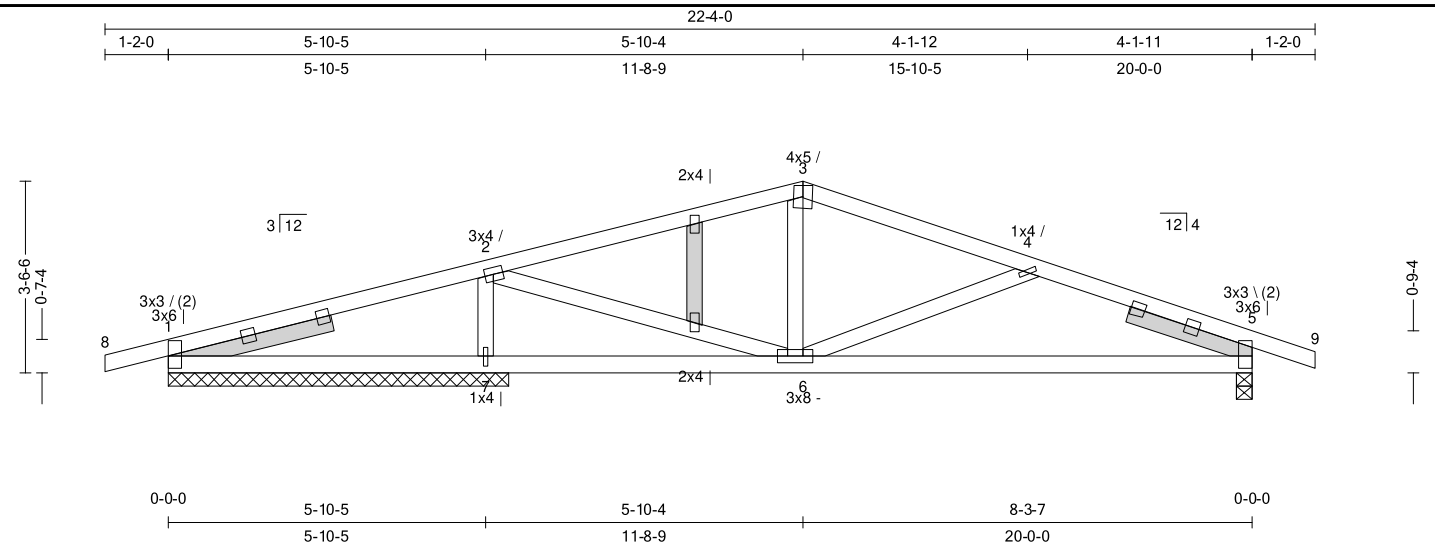
Eagle Metal Products

	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss:T03 Job: LOT 52_E2_REFRESHED FRAMING_ Date: 12/08/25 10:28:55 Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
20-0-0	3/12	1	1-2-0	1-2-0	0-0-0	0-0-0	1	24 in	93 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2018/	TC:	0.42 (1-2)	Vert TL:	0.16 in	L / 999	(5-6)	L / 240
TCDL: 10		TPI 1-2014	BC:	0.59 (5-6)	Vert LL:	0.07 in	L / 999	(5-6)	L / 360
BCLL: 0	Rep Mbr:	No	Web:	0.14 (2-6)	Horz TL:	0.01 in		5	
BCDL: 10	Lumber D.O.L.:	115 %							

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
5	1	3.5 in	1.50 in	753 lbs	·	-66 lbs	-268 lbs	-268 lbs	·
7	1	75.5 in	N/A	1,003 lbs	·	-62 lbs	-264 lbs	-264 lbs	121 lbs
1	1	75.5 in	N/A	45 lbs	-158 lbs	-28 lbs	-21 lbs	-158 lbs	-443 lbs
1	1	75.5 in	N/A	531 lbs	·	-55 lbs	-140 lbs	-140 lbs	373 lbs

12/08/2025

STATE OF MISSOURI
ARTURO A. HERNANDEZ
NUMBER PE-2006000095
PROFESSIONAL ENGINEER

Material

TC: SYP#1 2 x 4
BC: SYP#1 2 x 4
Web: SYP#1 2 x 4

Bracing

TC: Sheathed or Purlins at 5-11-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL= 1.60
4) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.416	513 lbs		3-4	0.268		(-836 lbs)			
	2-3	0.332		(-833 lbs)	4-5	0.114	366 lbs	(-1,069 lbs)			
BC	5-6	0.586	1,001 lbs								
Web	2-7	0.087	318 lbs	(-796 lbs)	2-6	0.145	875 lbs		4-6	0.116	(-335 lbs)

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Gable webs placed at 24 "OC, U.N.O.
3) Attach structural gable blocks with 3x3 20ga plates, U.N.O.
4) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
5) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
6) A creep factor of 2.00 has been applied for this truss analysis.
7) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
8) ▭ Indicates non-structural members.
9) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 1 may need to be considered.
10) Listed wind uplift reactions based on MWFRS & C&C loading.

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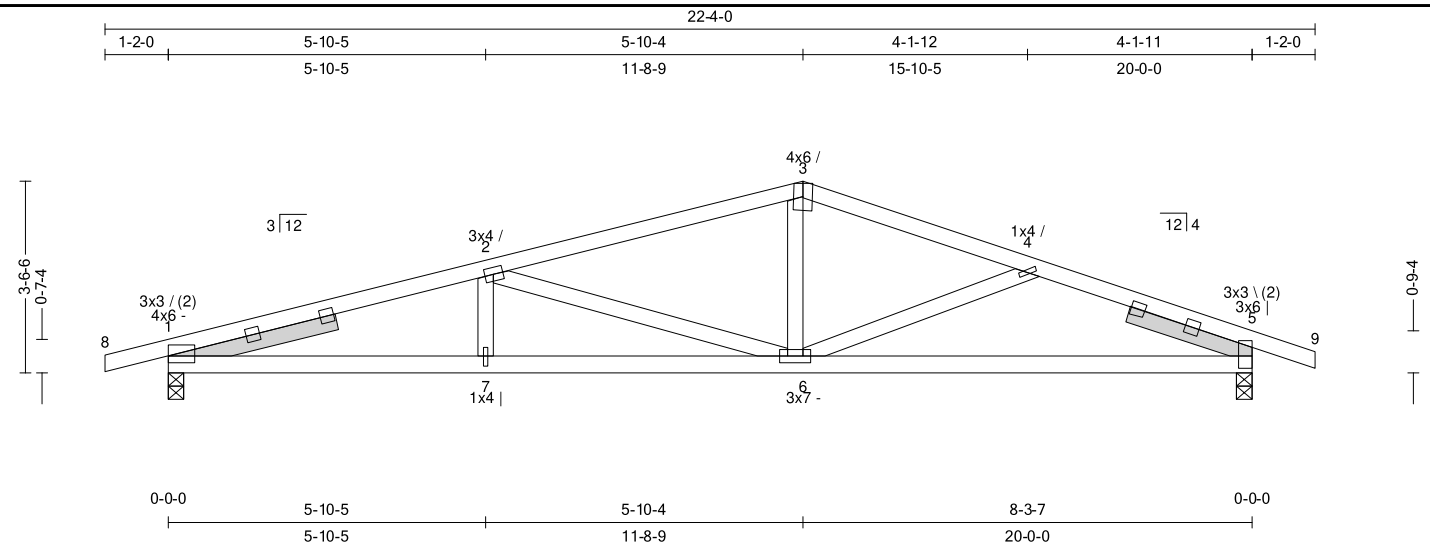
Eagle Metal Products

	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss:T04 Job: LOT 52_E2_REFRESHED FRAMING_ Date: 12/08/25 10:28:56 Page: 1 of 1

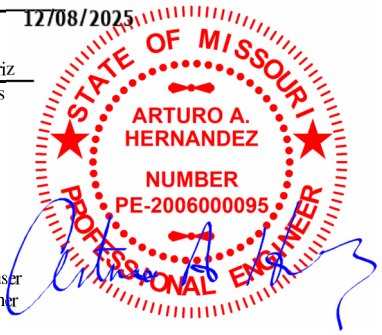
SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
20-0-0	3/12	17	1-2-0	1-2-0	0-0-0	0-0-0	1	24 in	90 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2018/	TC:	0.35 (2-3)	Vert TL:	0.21 in	L / 999	(5-6)	L / 240
TCDL: 10		TPI 1-2014	BC:	0.68 (6-7)	Vert LL:	0.08 in	L / 999	(5-6)	L / 360
BCLL: 0	Rep Mbr:	Yes	Web:	0.44 (2-6)	Horz TL:	0.06 in		5	
BCDL: 10	Lumber D.O.L.:	115 %							

12/08/2025



Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	3.5 in	1.50 in	1,070 lbs	·	-86 lbs	-341 lbs	-341 lbs	25 lbs
5	1	3.5 in	1.50 in	1,070 lbs	·	-86 lbs	-352 lbs	-352 lbs	·

Material

TC: SYP#1 2 x 4
BC: SYP#1 2 x 4
Web: SYP#1 2 x 4

Bracing

TC: Sheathed or Purlins at 3-11-0, Purlin design by Others.
BC: Sheathed or Purlins at 8-10-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
4) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.252	665 lbs	(-2,460 lbs)	3-4	0.320	502 lbs	(-1,701 lbs)				
	2-3	0.352	500 lbs	(-1,701 lbs)	4-5	0.268	557 lbs	(-1,797 lbs)				
BC	5-6	0.632	1,664 lbs	(-405 lbs)	6-7	0.682	2,367 lbs	(-538 lbs)	7-1	0.530	2,367 lbs	(-538 lbs)
Web	2-6	0.438		(-814 lbs)	3-6	0.092	553 lbs					

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
4) A creep factor of 2.00 has been applied for this truss analysis.
5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
6) ▭ Indicates non-structural members.
7) Listed wind uplift reactions based on MWFRS & C&C loading.

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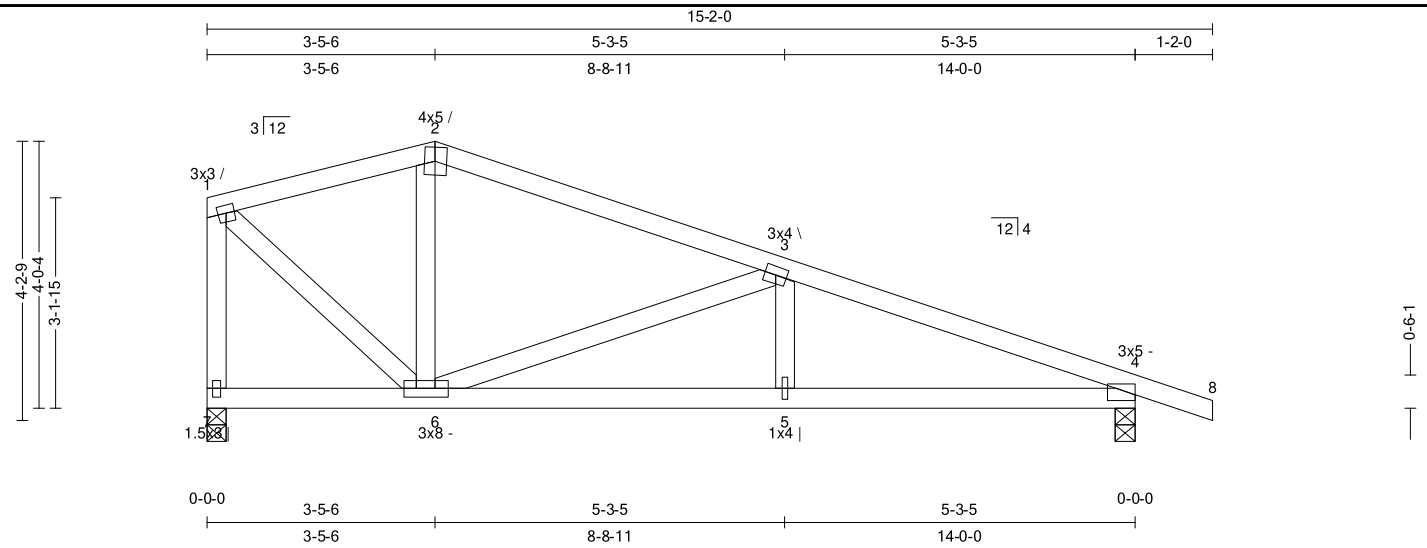
Eagle Metal Products

	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss:T05 Job: LOT 52_E2_REFRESHED FRAMING_ Date: 12/08/25 10:28:57 Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
14-0-0	3/12	1	0-0-0	1-2-0	0-0-0	0-0-0	1	24 in	66 lbs



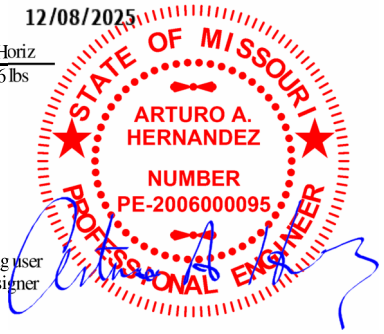
All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL :	20	Bldg Code :	IBC 2018/	TC :	0.33 (2-3)	Vert TL:	0.09 in	L / 999	(5-6)	L / 240
TCDL :	10		TPI 1-2014	BC :	0.44 (4-5)	Vert LL:	0.04 in	L / 999	(5-6)	L / 360
BCLL :	0	Rep Mbr :	No	Web :	0.44 (3-6)	Horz TL:	0.02 in		4	
BCDL :	10	Lumber D.O.L. :	115 %							

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
7	1	3.5 in	1.50 in	704 lbs	·	-54 lbs	-293 lbs	-293 lbs	-106 lbs
4	1	3.5 in	1.50 in	814 lbs	·	-58 lbs	-353 lbs	-353 lbs	·

12/08/2025



Material

TC: SYP#1 2 x 4
BC: SYP#1 2 x 4
Web: SYP#1 2 x 4

Bracing

TC: Sheathed or Purlins at 4-11-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (20 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL= 1.60
4) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.265		(-566 lbs)	2-3	0.325		(-647 lbs)	3-4	0.271	492 lbs	(-1,492 lbs)
BC	4-5	0.439	1,370 lbs	(-352 lbs)	5-6	0.427	1,370 lbs	(-352 lbs)				
Web	1-7	0.107	307 lbs	(-662 lbs)	3-6	0.437	329 lbs	(-875 lbs)				
	1-6	0.123	744 lbs									

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
4) A creep factor of 2.00 has been applied for this truss analysis.
5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
6) Listed wind uplift reactions based on MWFRS & C&C loading.

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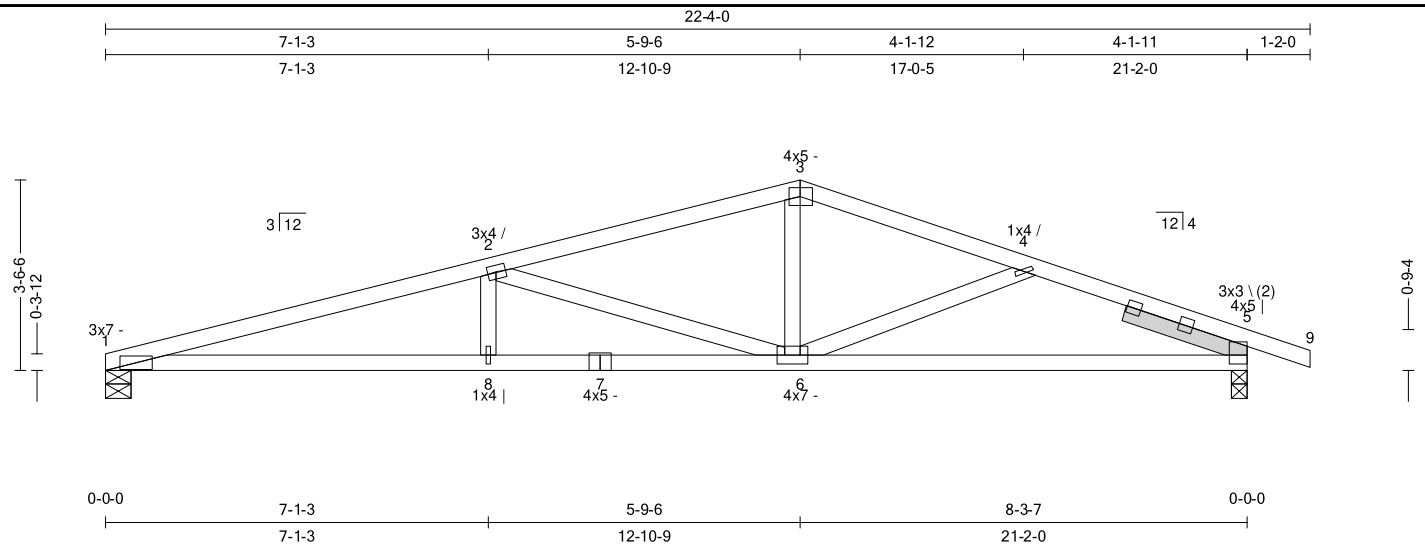
Eagle Metal Products

	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss:T06 Job: LOT 52_E2_REFRESHED FRAMING_ Date: 12/08/25 10:28:58 Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
21-2-0	3/12	5	0-0-0	1-2-0	0-0-0	0-0-0	1	24 in	87 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL :	20	Bldg Code :	IBC 2018/	TC :	0.41 (1-2)	Vert TL:	0.33 in	L / 738	(8-1)	L / 240
TCDL :	10		TPI 1-2014	BC :	0.49 (8-1)	Vert LL:	0.12 in	L / 999	(8-1)	L / 360
BCLL :	0	Rep Mbr :	Yes	Web :	0.67 (2-6)	Horz TL:	0.07 in		5	
BCDL :	10	Lumber D.O.L. :	115 %							

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	5.5 in	1.50 in	1,056 lbs	·	-68 lbs	-281 lbs	-281 lbs	-35 lbs
5	1	3.5 in	1.50 in	1,130 lbs	·	-91 lbs	-359 lbs	-359 lbs	·

12/08/2025

STATE OF MISSOURI
ARTURO A. HERNANDEZ
NUMBER PE-2006000095
PROFESSIONAL ENGINEER

Material

TC: SYP 2400/1.8 2 x 4
BC: SYP 2400/1.8 2 x 4
Web: SYP #1 2 x 4

Bracing

TC: Sheathed or Purlins at 4-0-0, Purlin design by Others.
BC: Sheathed or Purlins at 9-9-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
4) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.414	799 lbs	(-3,094 lbs)	3-4	0.163	526 lbs	(-1,845 lbs)				
	2-3	0.215	528 lbs	(-1,852 lbs)	4-5	0.193	578 lbs	(-1,939 lbs)				
BC	5-6	0.395	1,795 lbs	(-425 lbs)	6-8	0.403	2,967 lbs	(-679 lbs)	8-1	0.495	2,967 lbs	(-679 lbs)
Web	2-6	0.670	351 lbs	(-1,274 lbs)	3-6	0.112	675 lbs					

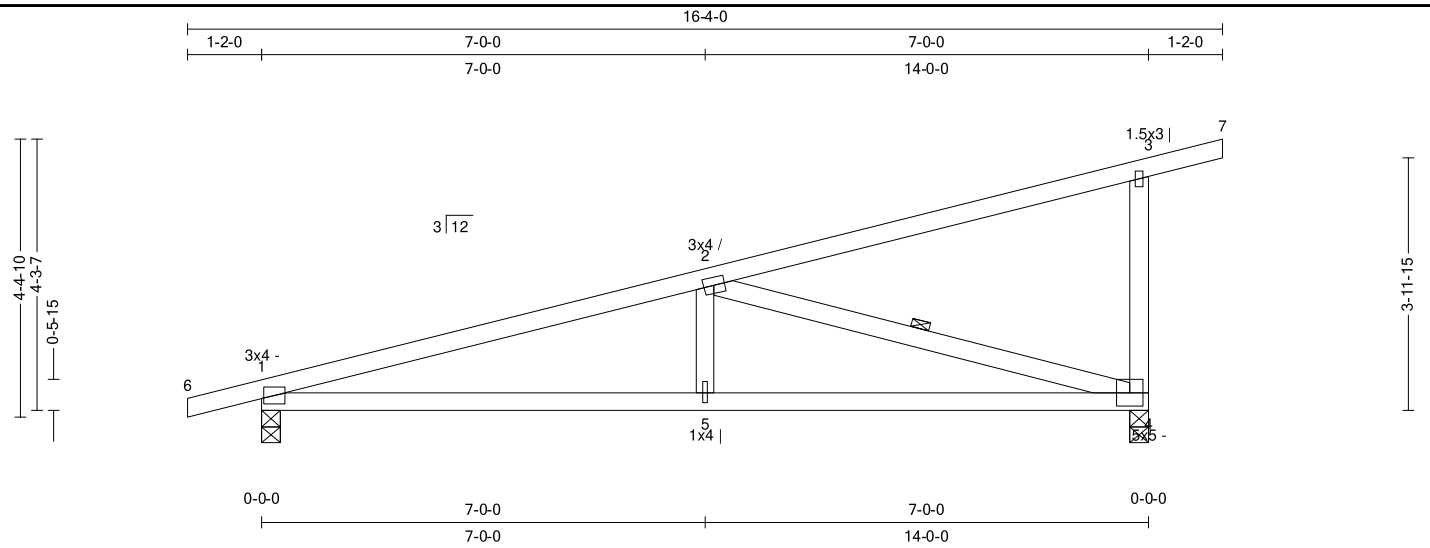
Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
4) A creep factor of 2.00 has been applied for this truss analysis.
5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
6) ▭ Indicates non-structural members.
7) Listed wind uplift reactions based on MWFRS & C&C loading.

TrueBuild®Truss Software v5.8.14
Eagle Metal Products

	Quality Line Truss Co., LLC 34593 S 4350 RD Address 2 Adair, OK 74330	Truss:T08 Job: LOT 52_E2_REFRESHED FRAMING_ Date: 12/08/25 10:29:00 Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLY(S)	SPACING	WGT/PLY
14-0-0	3/12	4	1-2-0	1-2-0	0-0-0	0-0-0	1	24 in	60 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		General		CSI		Deflection		L/	(loc)	Allowed
TCLL:	20	Bldg Code:	IBC 2018/	TC:	0.53 (1-2)	Vert TL:	0.18 in	L / 890	(4-5)	L / 240
TCDL:	10		TPI 1-2014	BC:	0.62 (5-1)	Vert LL:	0.07 in	L / 999	(4-5)	L / 360
BCLL:	0	Rep Mbr:	Yes	Web:	0.33 (2-4)	Horz TL:	0.03 in		4	
BCDL:	10	Lumber D.O.L.:	115 %							

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	3.5 in	1.50 in	814 lbs	·	-49 lbs	-326 lbs	-326 lbs	138 lbs
4	1	3.5 in	1.50 in	778 lbs	·	-85 lbs	-367 lbs	-367 lbs	·

12/08/2025

Material

TC: SYP#1 2 x 4
BC: SYP#1 2 x 4
Web: SYP#1 2 x 4

Bracing

TC: Sheathed or Purlins at 4-4-0, Purlin design by Others.
BC: Sheathed or Purlins at 9-4-0, Purlin design by Others.
Web: One Midpoint Row: 2-4

Loads

1) This truss has been designed for the effects of balanced (20 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
2) This truss has been designed to account for the effects of ice dams forming at the eaves.
3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
4) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.528	463 lbs	(-1,526 lbs)				
BC	4-5	0.617	1,437 lbs	(-473 lbs)	5-1	0.617	1,437 lbs	(-473 lbs)
Web	2-5	0.055	332 lbs		2-4	0.326	571 lbs	(-1,494 lbs)

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
4) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
5) A creep factor of 2.00 has been applied for this truss analysis.
6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
7) ⊠ Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
8) Listed wind uplift reactions based on MWFRS & C&C loading.

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