

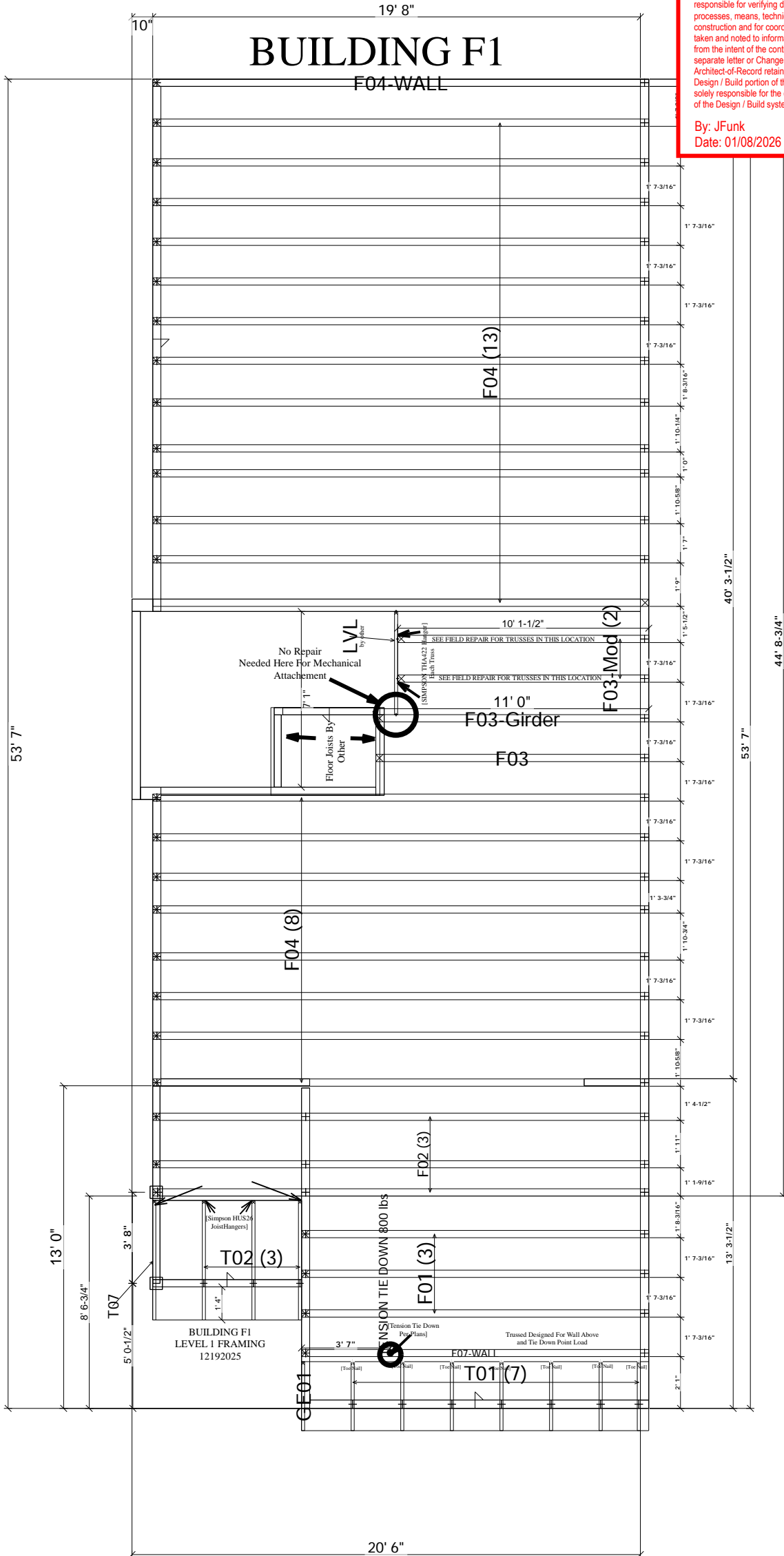
RELEASE FOR CONSTRUCTION  
 AS NOTED ON PLANS REVIEW  
 DEVELOPMENT SERVICES  
 LEE'S SUMMIT, MISSOURI  
 01/14/2026 1:37:28

**STAND STRUCTURAL ENGINEERING**  
 8234 Robinson St  
 Overland Park, KS 66204  
 (913) 214-2169

- Reviewed  Revise and Resubmit
- Reviewed as Noted  Rejected
- Not required by the Contract Documents
- For Record Only

Review is only for general conformance with the design concept and the intent of the Contract Documents. Contractor is solely responsible for verifying dimensions, for establishing fabrication processes, means, techniques, sequences and procedures of construction and for coordination of work of all trades. Review action taken and noted to information shown does not authorize deviations from the intent of the contract documents, unless so stated in a separate letter or Change Order. The Structural Engineer and/or Architect-of-Record retained by the Contractor or his Supplier, for the Design / Build portion of the project represented by this submittal, is solely responsible for the correctness, appropriateness and adequacy of the Design / Build system.

By: JFunk  
 Date: 01/08/2026



20' 6"



The truss designs referenced below have been prepared by me or under my direct supervision based on the truss design criteria and requirements ("design criteria") provided by **Quality Line Truss**.

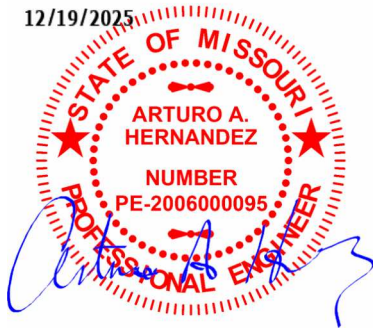
These truss designs are intended for the fabrication of individual building components that will perform to the design criteria provided. Any variance from the design criteria will render the affected truss designs inapplicable.

Listed below are the truss designs included in this package and covered by this seal.

Job: LOT 59\_F1 BUILDING\_REF FRAMING\_11192025 - 1254862  
F01, F02, F03-Girder, F03, F03-Mod, F04, F04-WALL, F07-WALL, GE01, GE02, GE03, GE04, T01, T02, T03, T04, T05, T06, T07, T08

Any location identification is for file reference only. No determination of the appropriateness of design criteria for any specific project has been made in preparing the truss designs.

Please refer to individual truss designs for specific design criteria.



**RELEASE FOR CONSTRUCTION  
AS NOTED ON PLANS REVIEW  
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LEE'S SUMMIT, MISSOURI  
01/14/2026 1:37:16**

**STAND STRUCTURAL ENGINEERING**  
8234 Robinson St  
Overland Park, KS 66204  
(913) 214-2169

Reviewed       Revise and Resubmit  
 Reviewed as Noted       Rejected  
 Not required by the Contract Documents  
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Review is only for general conformance with the design concept and the intent of the Contract Documents. Contractor is solely responsible for verifying dimensions, for establishing fabrication processes, means, techniques, sequences and procedures of construction and for coordination of work of all trades. Review action taken and noted to information shown does not authorize deviations from the intent of the contract documents, unless so stated in a separate letter or Change Order. The Structural Engineer and/or Architect-of-Record retained by the Contractor or his Supplier, for the Design / Build portion of the project represented by this submittal, is solely responsible for the correctness, appropriateness and adequacy of the Design / Build system.

By: JFunk  
Date: 01/08/2026

Arturo A. Hernandez (MO, 2006000095)  
My license expiration date for the state of MO is 12/31/2026.

**IMPORTANT NOTE:** The responsibility of the engineer sealing this package, as a Truss Engineer, is solely for design of individual trusses as individual building components based upon design criteria provided by others and set forth in the referenced truss drawings. The truss design criteria for the components have not been verified as appropriate for any particular building, project or use. Adequacy and suitability of design criteria and requirements for the truss designs for any specific project are the responsibility of the building designer, not the Truss Engineer, per ANSI/TPI-1, Chapter 2.

## DESIGN NOTES

1. The Truss Design Drawing(s) provided with these Design Notes have been prepared under and are subject to ANSI / TPI 1 published by the Truss Plate Institute, [www.tpinst.org](http://www.tpinst.org). Capitalized terms have the meanings provided in ANSI / TPI 1.
2. Copies of each Truss Design Drawing shall be furnished to the installation contractor, Building Designer, Owner and all persons fabricating, handling, installing, bracing, or erecting the trusses.
- DESIGN LIMITATIONS**
3. The Truss Design Drawing is based upon specifications provided by the Building Designer in accordance with ANSI / TPI 1. Neither the Truss Designer, Eagle, nor an engineer who seals this design (if any) assumes any responsibility for the adequacy or accuracy of specifications provided by the Building Designer.
4. The Building Designer is solely responsible for the suitability based upon the Truss Design Drawing and shall be responsible for reviewing and verifying that the information shown is in general conformance with the design of the Building.
5. Each Truss Design Drawing is for the individual building component (a truss). A seal on the Truss Design Drawing indicates acceptance of professional engineering responsibility solely for the individual truss.
6. Each Truss Design Drawing assumes trusses will be suitably protected from the environment.

### HANDLING, INSTALLING, & BRACING

7. Refer to Building Component Safety Information (BCSI) for handling, installing, restraining and bracing trusses. Copies can be obtained from the Structural Building Components Association, [www.sbcindustry.com](http://www.sbcindustry.com).
8. Bracing shown on each Truss Design Drawing is for lateral support of individual truss components only to reduce buckling lengths. All temporary and permanent bracing, including lateral load and diagonal or cross bracing, are the responsibility, respectively, of the erector and Building Designer.
9. Eagle is not responsible for improper truss fabrication, handling, erection or bracing.
10. Compression chords shall be laterally braced by the roof or floor sheathing, directly attached, or have purlins provided at spacing shown, unless noted otherwise.

11. Bottom chord required bracing shall be at 10ft spacing or less, if no structural rated ceiling is installed, unless noted otherwise.
12. Strongbacking shall be installed on all parallel chord trusses, including flooring systems, to limit deflection and reduce vibration. Refer to BCSI-B7.
13. Never exceed the design loading shown. Never stack building or other materials on inadequately braced truss; refer to BCSI.
14. Concentration of construction loads greater than the design loads shall not be applied to the trusses at any time; refer to BCSI.
15. Trusses shall be handled with care prior to erection to avoid damage. Refer to BCSI for recommended truss handling and erection.

### MATERIALS & FABRICATION

16. Lumber moisture content shall be 19% or less at the time of fabrication unless noted otherwise.
17. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
18. Unless expressly noted, the truss designs are not applicable for use with fire retardant or preservative treated lumber.
19. Plates shall be applied on both faces of truss at each joint and embedded fully. Knots and wane at joint locations shall be regulated in accordance with ANSI / TPI 1.

20. For a specified plate gauge and grade, the specified size is a minimum.

21. Connections not shown are the responsibility of others.

22. Adequate support shall be provided to resist gravity, lateral and uplift loads.

23. For 4X2 truss orientation, locate plates 0 - 1/16" from outside the edge of the truss.

24. Fabrication of truss shall be in accordance with ANSI / TPI 1.

### OTHER NOTES

25. Camber is a non-structural consideration and is the responsibility of truss fabricator.
26. Do not cut or alter any truss member or plate without prior approval from a professional engineer.
27. Lumber design values are in accordance with ANSI / TPI 1; lumber design values are by others.
28. Install specified hangers per manufacturer recommendations.

## SYMBOLS

### PLATE SIZE

**3X4** - The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

**-, /, |** Indicates required direction of slots; Reference "Joint Details" for more information.

20 Ga Gr40 connectors required

**3X10-20HS** - 20 Ga Gr60 connectors required

**8X10-18HS** - 18 Ga Gr60 connectors required

### LATERAL BRACING

When this symbol shown, continuous lateral bracing is required on the member of the truss.



### BEARING

Indicates location where bearings (supports) occur.



### PLATE LOCATION & ORIENTATION

The plate shall be centered on joint and/or placed in accordance with the design drawing/QC full scale details.



## REFERENCES

- **ANSI / TPI 1:** National Design Standard for Metal Plate Connected Wood Trusses
- **BCSI:** Building Component & Safety Information - Guide to Good Practice for Handling, Installing, Restraining, & Bracing of Metal Plate Connected Wood Trusses.
- **NDS:** National Design Specification for Wood Construction
- **ESR:** 1082 published by the International Code Council. [www.icc-es.org](http://www.icc-es.org)

**Quality Line Truss Co., LLC**

34593 S 4350 RD  
Address 2  
Adair, OK 74330

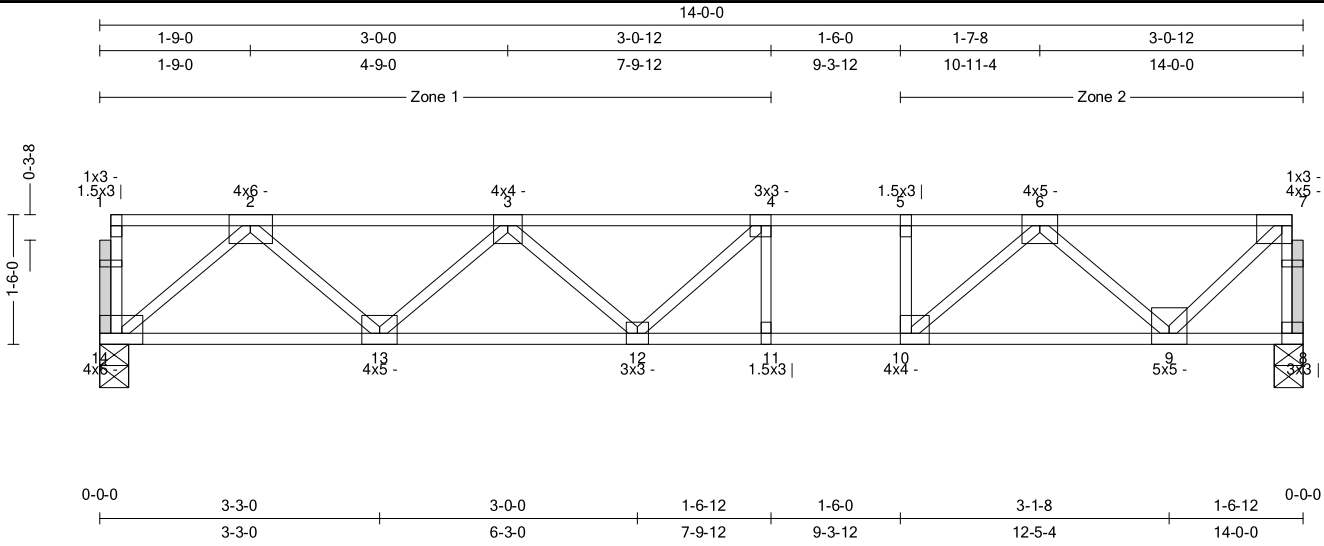
Truss:F01

Job: LOT 59\_F1 BUILDING\_REFRAMIN

Date: 12/19/25 14:33:35

Page: 1 of 1

SPAN 14-0-0	PITCH 0/12	QTY 4	OHL 0-0-0	OHR 0-0-0	PLY(S) 1	SPACING 19.19 in	WGT/PLY 74 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 40	Bldg Code: IBC 2018/	TC: 0.49 (3-4)	Vert TL: 0.26 in	L/ 627	(11-12)	L/ 480
TCDL: 50	TPI 1-2014	BC: 0.73 (10-11)	Vert LL: 0.1 in	L/ 999	(11-12)	L/ 720
BCLL: 0	Rep Mbr: Yes	Web: 0.25 (7-9)	Horz TL: 0.04 in		8	
BCDL: 15	Lumber D.O.L.: 100 %					

12/19/2025

**Reaction**

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
14	1	4 in	1.50 in	1,287 lbs					
8	1	4 in	1.50 in	1,287 lbs					

**Material**

TC: SYP2400/1.8 4 x 2  
BC: SYP2400/1.8 4 x 2  
Web: SYP#1 4 x 2

**Loads**

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

**Member Forces**

Table indicates: Member ID, max CSL, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	2-3	0.402	(-2,173 lbs)	4-5	0.418	(-2,967 lbs)	6-7	0.369	(-1,048 lbs)
	3-4	0.492	(-3,056 lbs)	5-6	0.439	(-2,967 lbs)			
BC	9-10	0.560	2,164 lbs	11-12	0.726	2,967 lbs	13-14	0.189	1,305 lbs
	10-11	0.726	2,967 lbs	12-13	0.329	2,902 lbs			
Web	2-14	0.217	(-1,738 lbs)	5-10	0.047	(-386 lbs)	7-9	0.255	1,483 lbs
	2-13	0.199	1,177 lbs	6-10	0.197	1,069 lbs	7-8	0.138	(-1,239 lbs)
	3-13	0.127	(-989 lbs)	6-9	0.191	(-1,514 lbs)			

**Notes**

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
- 3) The fabrication tolerance for this floor truss is 10% (Cq = 0.90).
- 4) A creep factor of 2.00 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 6)  Indicates non-structural members.



WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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Eagle Metal Products

**Quality Line Truss Co., LLC**

34593 S 4350 RD  
Address 2  
Adair, OK 74330

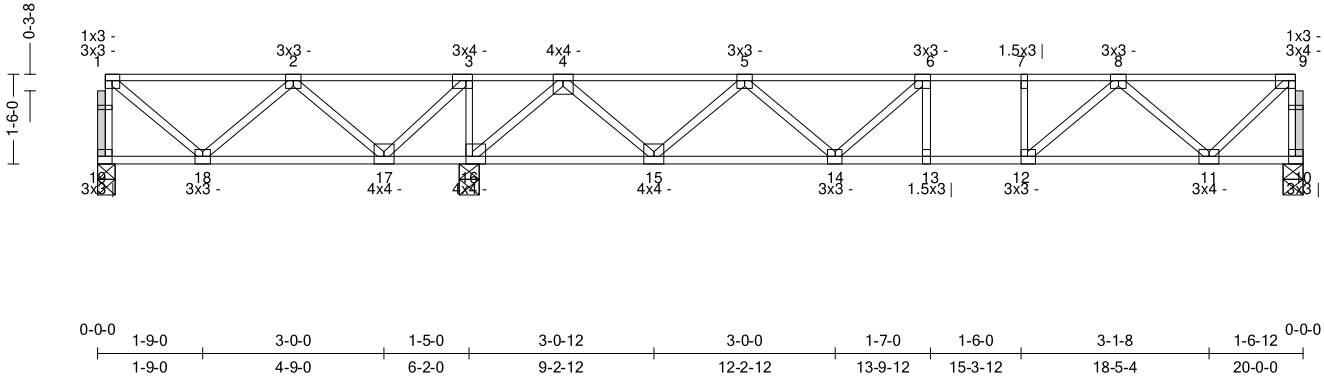
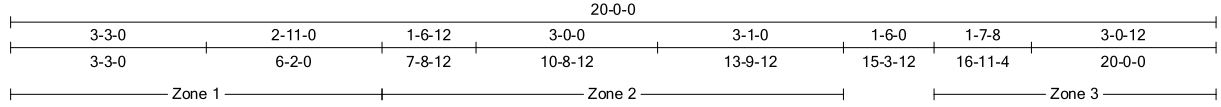
Truss:F02

Job: LOT 59\_F1 BUILDING\_REF FRAMING

Date: 12/19/25 14:33:36

Page: 1 of 2

SPAN 20-0-0      PITCH 0/12      QTY 3      OHL 0-0-0      OHR 0-0-0      PLY(S) 1      SPACING 19.19 in      WGT/PLY 102 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 40	Bldg Code: IBC 2018/	TC: 0.38 (2-3)	Vert TL: 0.07 in	L/999	(13-14)	L/240
TCDL: 10	TPI 1-2014	BC: 0.20 (12-13)	Vert LL: 0.05 in	L/999	(13-14)	L/360
BCLL: 0	Rep Mbr: Yes	Web: 0.15 (4-15)	Horz TL: 0.01 in		10	
BCDL: 10	Lumber D.O.L.: 100%					

**Reaction**

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
16	1	4 in	1.50 in	1,670 lbs	.	.	.	.	.
19	1	3.5 in	1.50 in	198 lbs	-177 lbs	.	.	-177 lbs	.
10	1	4 in	1.50 in	636 lbs	.	.	.	.	.

**Material**

TC: SYP2400/1.8 4 x 2  
BC: SYP2400/1.8 4 x 2  
Web: SYP#1 4 x 2

**Loads**

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case L1: Std Live Load

**Point Loads**

Member	Location	Direction	Load	Trib Width
Top	0-0-0	Down	44 lbs	
Top	2-0-0	Down	44 lbs	
Top	4-0-0	Down	44 lbs	
Top	6-0-0	Down	44 lbs	

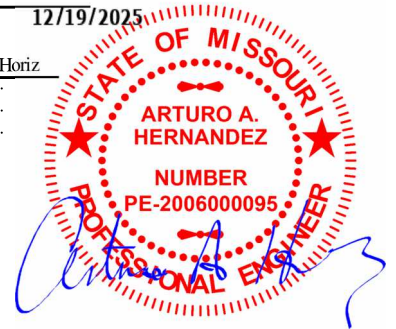
**Member Forces**

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	2-3	0.378	839 lbs	5-6	0.192	(-1,067 lbs)	7-8	0.150	(-1,246 lbs)
	3-4	0.243	1,396 lbs	6-7	0.102	(-1,246 lbs)	8-9	0.183	(-515 lbs)
BC	11-12	0.173	997 lbs	14-15	0.125	781 lbs	17-18	0.050	(-507 lbs)
	12-13	0.204	1,246 lbs	15-16	0.055	(-488 lbs)			
	13-14	0.204	1,246 lbs	16-17	0.109	(-1,396 lbs)			
Web	1-18	0.042	(-312 lbs)	4-16	0.147	(-1,210 lbs)	8-11	0.081	(-654 lbs)
	2-18	0.071	370 lbs	4-15	0.155	934 lbs	9-11	0.121	728 lbs
	2-17	0.094	(-757 lbs)	5-15	0.096	(-788 lbs)	9-10	0.066	(-617 lbs)
	3-17	0.137	826 lbs	5-14	0.064	387 lbs			
	3-16	0.087	(-793 lbs)	8-12	0.065	342 lbs			

**Notes**

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
- 3) The fabrication tolerance for this floor truss is 10% (Cq = 0.90).
- 4) A creep factor of 2.00 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 6)  Indicates non-structural members.



WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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Eagle Metal Products

**Quality Line Truss Co., LLC**

34593 S 4350 RD

Address 2

Adair, OK 74330

Truss:F02

Job: LOT 59\_F1 BUILDING\_REF FRAMING

Date: 12/19/25 14:33:37

Page: 2 of 2

SPAN  
20-0-0

PITCH  
0/12

QTY  
3

OHL  
0-0-0

OHR  
0-0-0

PLY(S)  
1

SPACING  
19.19 in

WGT/PLY  
102 lbs

7) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 19 may need to be considered.

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Eagle Metal Products



**Quality Line Truss Co., LLC**

34593 S 4350 RD

Address 2

Adair, OK 74330

Truss:F03-Girder

Job: LOT 59\_F1 BUILDING\_REF FRAMING

Date: 12/19/25 14:34:00

Page: 2 of 2

SPAN  
11-0-0

PITCH  
0/12

QTY  
1

OHL  
0-0-0

OHR  
0-0-0

PLY(S)  
1

SPACING  
37.53 in

WGT/PLY  
58 lbs

**Truss to Truss Connection Summary**

Carried Truss	Carrying Chord	Carrying Offset
LVL	TC	0-9-12

**Notes**

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
- 3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
- 4) A creep factor of 2.00 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SP1B design values effective June 1, 2013 were used.
- 6)  Indicates non-structural members.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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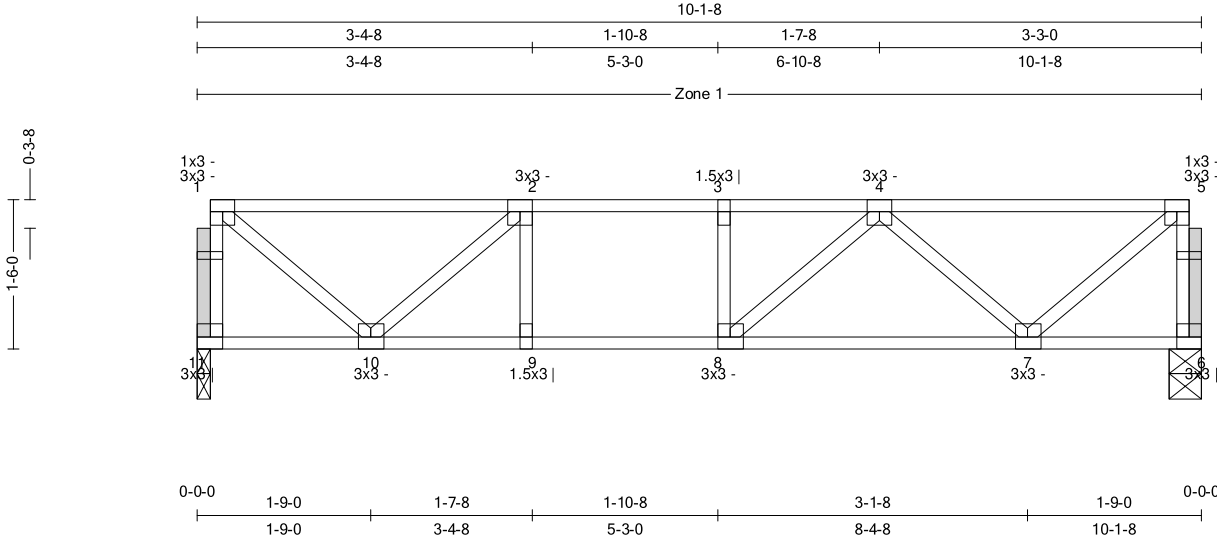
Truss:F03-Mod

Job: LOT 59\_F1 BUILDING\_REFRAMIN

Date: 12/19/25 14:33:58

Page: 1 of 1

SPAN 10-1-8	PITCH 0/12	QTY 2	OHL 0-0-0	OHR 0-0-0	PLY(S) 1	SPACING 19.19 in	WGT/PLY 53 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL : 40	Bldg Code : IBC 2018/	TC : 0.26 (1-2)	Vert TL: 0.08 in	L/999	(7-8)	L/240
TCDL : 10	TPI 1-2014	BC : 0.28 (9-10)	Vert LL: 0.06 in	L/999	(7-8)	L/360
BCLL : 0	Rep Mbr : No	Web : 0.11 (5-7)	Horz TL: 0.01 in		6	
BCDL : 10	Lumber D.O.L. : 100 %					

**Reaction**

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
11	1	1.5 in	1.50 in	567 lbs					
6	1	4 in	1.50 in	567 lbs					

**Material**

TC: SYP2400/1.8 4 x 2  
BC: SYP2400/1.8 4 x 2  
Web: SYP#1 4 x 2

**Loads**

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

**Member Forces**

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.262	(-491 lbs)	2-3	0.252	(-952 lbs)	3-4	0.208	(-952 lbs)	4-5	0.234	(-506 lbs)
BC	7-8	0.203	882 lbs	9-10	0.276	952 lbs						
	8-9	0.276	952 lbs									
Web	1-11	0.057	(-535 lbs)	4-7	0.064	(-510 lbs)						
	1-10	0.108	655 lbs	5-7	0.112	675 lbs						
	2-10	0.075	(-614 lbs)	5-6	0.059	(-552 lbs)						

**Notes**

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
- 3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
- 4) A creep factor of 2.00 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 6)  Indicates non-structural members.



WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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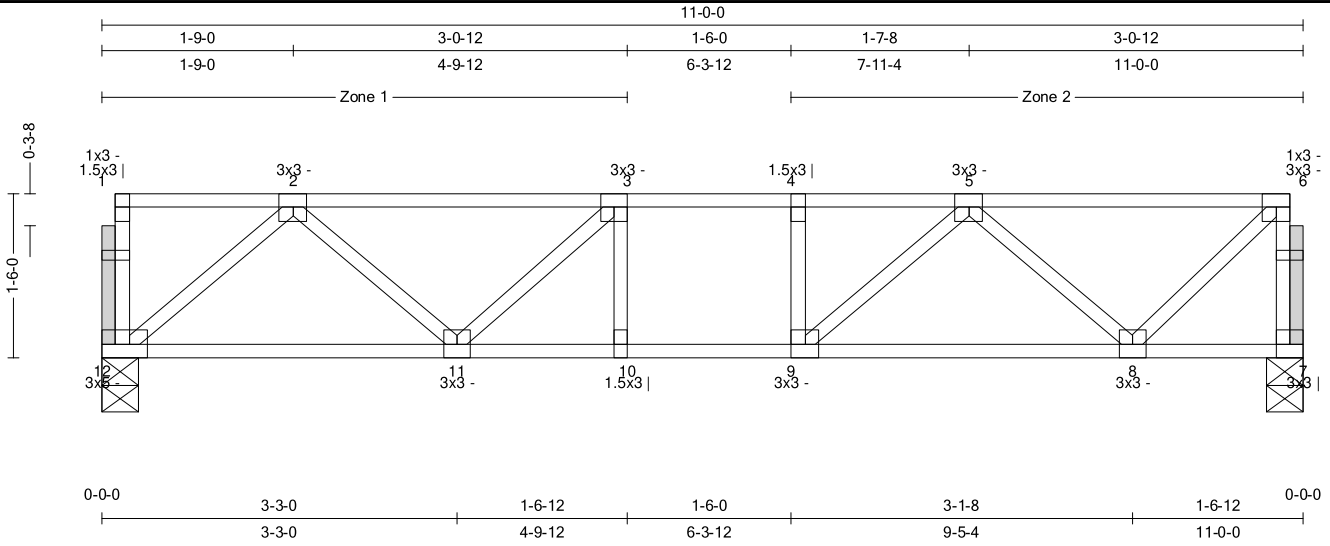
Truss:F03

Job: LOT 59\_F1 BUILDING\_REF FRAMING

Date: 12/19/25 14:33:38

Page: 1 of 1

SPAN 11-0-0	PITCH 0/12	QTY 2	OHL 0-0-0	OHR 0-0-0	PLY(S) 1	SPACING 19.19 in	WGT/PLY 58 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 40	Bldg Code: IBC 2018/	TC: 0.21 (1-2)	Vert TL: 0.05 in	L/999	(10-11)	L/240
TCDL: 10	TPI 1-2014	BC: 0.23 (10-11)	Vert LL: 0.04 in	L/999	(10-11)	L/360
BCLL: 0	Rep Mbr: No	Web: 0.12 (6-8)	Horz TL: 0.01 in		7	
BCDL: 10	Lumber D.O.L.: 100 %					

**Reaction**

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
12	1	4 in	1.50 in	616 lbs					
7	1	4 in	1.50 in	616 lbs					

**Material**

TC: SYP2400/1.8 4 x 2  
BC: SYP2400/1.8 4 x 2  
Web: SYP#1 4 x 2

**Loads**

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

**Member Forces**

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

Member	Force 1	Force 2	Force 3	Force 4	Force 5	Force 6	Force 7
TC	2-3 0.200 (-944 lbs)	4-5 0.176 (-1,161 lbs)					
BC	8-9 0.194 952 lbs	10-11 0.230 1,161 lbs					
Web	2-11 0.076 458 lbs	5-8 0.076 (-808 lbs)	6-8 0.116 701 lbs				

**Notes**

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
- 3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
- 4) A creep factor of 2.00 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 6)  Indicates non-structural members.



WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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Address 2  
Adair, OK 74330

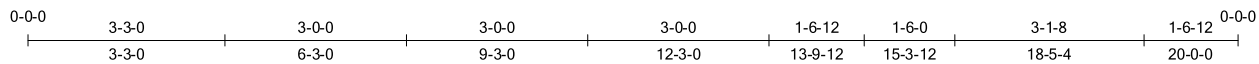
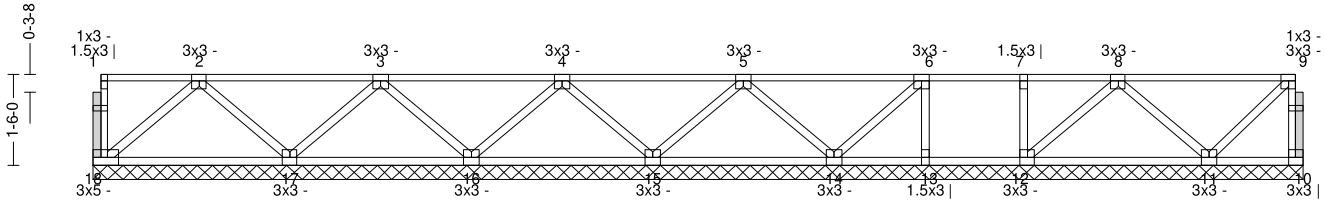
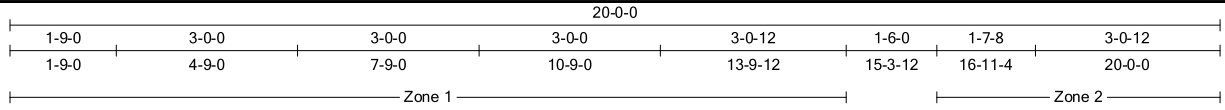
Truss:F04-WALL

Job: LOT 59\_F1 BUILDING\_REFRAMIN

Date: 12/19/25 14:33:39

Page: 1 of 1

SPAN 20-0-0	PITCH 0/12	QTY 1	OHL 0-0-0	OHR 0-0-0	PLY(S) 1	SPACING 19.19 in	WGT/PLY 99 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 40	Bldg Code: IBC 2018/	TC: 0.65 (5-6)	Vert TL: 0.02 in	L/999	(17-18)	L/240
TCDL: 10	TPI 1-2014	BC: 0.09 (17-18)	Vert LL: 0.01 in	L/999	(17-18)	L/360
BCLL: 0	Rep Mbr: No	Web: 0.08 (8-11)	Horz TL: 0 in			
BCDL: 10	Lumber D.O.L.: 100 %					

12/19/2025

**Reaction**

Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	240 in	N/A	267 lbs	.	.	.	.	.
1	240 in	N/A	507 lbs	.	.	.	.	-388 lbs
1	240 in	N/A	567 lbs	.	.	.	.	346 lbs
1	240 in	N/A	534 lbs	.	.	.	.	.
1	240 in	N/A	557 lbs	.	.	.	.	-309 lbs
1	240 in	N/A	791 lbs	.	.	.	.	-13 lbs
1	240 in	N/A	791 lbs	.	.	.	.	-26 lbs
1	240 in	N/A	777 lbs	.	.	.	.	56 lbs
1	240 in	N/A	447 lbs	.	.	.	.	331 lbs



**Material**

TC: SYP2400/1.8 4 x 2  
BC: SYP2400/1.8 4 x 2  
Web: SYP#1 4 x 2

**Loads**

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	20-0-0	Down	Proj	150 plf	150 plf	

**Member Forces**

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	BC	Web	2-18	2-17	3-17	3-16	0.060	0.062	0.072	0.072	(-441 lbs)	(-456 lbs)	(-532 lbs)	(-536 lbs)	4-16	4-15	5-15	5-14	0.068	0.070	0.068	0.076	(-500 lbs)	(-518 lbs)	(-505 lbs)	(-562 lbs)	6-13	8-12	8-11	0.062	0.077	(-508 lbs)	(-461 lbs)	(-573 lbs)
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**Notes**

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
- 3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
- 4) A creep factor of 2.00 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 6)  Indicates non-structural members.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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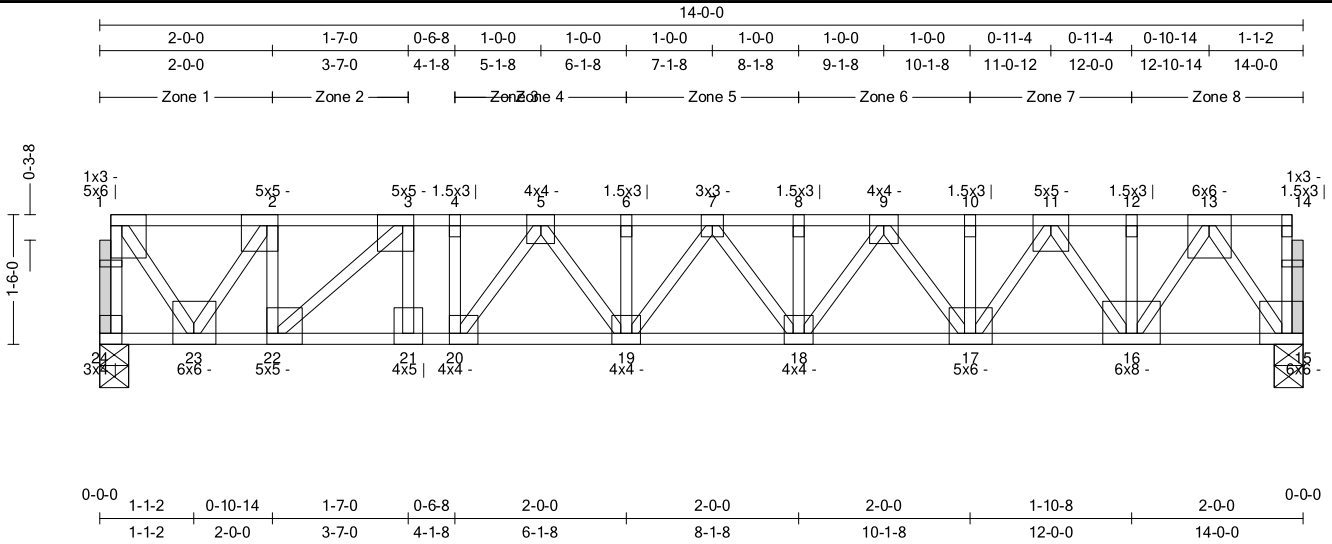
Truss:F07-WALL

Job: LOT 59\_F1 BUILDING\_REFRAMIN

Date: 12/19/25 14:33:42

Page: 1 of 2

SPAN 14-0-0	PITCH 0/12	QTY 1	OHL 0-0-0	OHR 0-0-0	PLY(S) 1	SPACING 19.19 in	WGT/PLY 95 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 40	Bldg Code: IBC 2018/	TC: 0.51 (4-5)	Vert TL: 0.23 in	L/ 681	(19-20)	L/ 240
TCDL: 10	TPI 1-2014	BC: 0.76 (19-20)	Vert LL: 0.05 in	L/ 999	(19-20)	L/ 360
BCLL: 0	Rep Mbr: No	Web: 0.38 (1-23)	Horz TL: 0.06 in		15	
BCDL: 10	Lumber D.O.L.: 100 %					

**Reaction**

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
24	1	4 in	1.50 in	1,964 lbs					
15	1	4 in	1.50 in	1,964 lbs					

**Material**

TC: SYP2400/1.8 4 x 2  
BC: SYP2400/1.8 4 x 2  
Web: SYP#1 4 x 2

**Loads**

1) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	14-0-0	Down	Proj	150 plf	150 plf	

Point Loads

Member	Location	Direction	Load	Trib Width
Bot	3-7-0	Down	800 lbs	
Top	2-0-0	Down	44 lbs	
Top	4-0-0	Down	44 lbs	
Top	6-0-0	Down	44 lbs	
Top	8-0-0	Down	44 lbs	
Top	10-0-0	Down	44 lbs	
Top	12-0-0	Down	44 lbs	
Top	3-7-0	Up	800 lbs	

**Member Forces**

Table indicates: Member ID, max CSL, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.253	(-1,152 lbs)	5-6	0.441	(-4,798 lbs)	9-10	0.354	(-3,845 lbs)
	2-3	0.397	(-2,255 lbs)	6-7	0.441	(-4,798 lbs)	10-11	0.353	(-3,845 lbs)
	3-4	0.496	(-3,811 lbs)	7-8	0.435	(-4,727 lbs)	11-12	0.207	(-2,252 lbs)
	4-5	0.513	(-3,811 lbs)	8-9	0.435	(-4,727 lbs)	12-13	0.207	(-2,252 lbs)
	BC	15-16	0.136	1,206 lbs	18-19	0.521	4,856 lbs	21-22	0.705
16-17		0.320	3,126 lbs	19-20	0.758	4,478 lbs	22-23	0.327	2,255 lbs
17-18		0.459	4,377 lbs	20-21	0.705	3,811 lbs			
Web		1-24	0.224	(-1,947 lbs)	4-20	0.092	506 lbs	11-16	0.185
	1-23	0.375	2,093 lbs	5-20	0.141	(-1,133 lbs)	13-16	0.340	1,902 lbs
	2-23	0.238	(-2,005 lbs)	5-19	0.103	545 lbs	13-15	0.260	(-2,191 lbs)
	2-22	0.223	1,261 lbs	9-18	0.106	596 lbs			
	3-22	0.261	(-2,061 lbs)	9-17	0.109	(-905 lbs)			
	3-21	0.246	1,312 lbs	11-17	0.228	1,275 lbs			



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Address 2  
Adair, OK 74330

Truss:F07-WALL

Job: LOT 59\_F1 BUILDING\_REF FRAMING

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Page: 2 of 2

SPAN  
14-0-0

PITCH  
0/12

QTY  
1

OHL  
0-0-0

OHR  
0-0-0

PLY(S)  
1

SPACING  
19.19 in

WGT/PLY  
95 lbs

**Notes**

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Unless otherwise specified by the Building Designer, one strongback every 10'-0".
- 3) The fabrication tolerance for this floor truss is 10 % (Cq = 0.90).
- 4) A creep factor of 2.00 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 6)  Indicates non-structural members.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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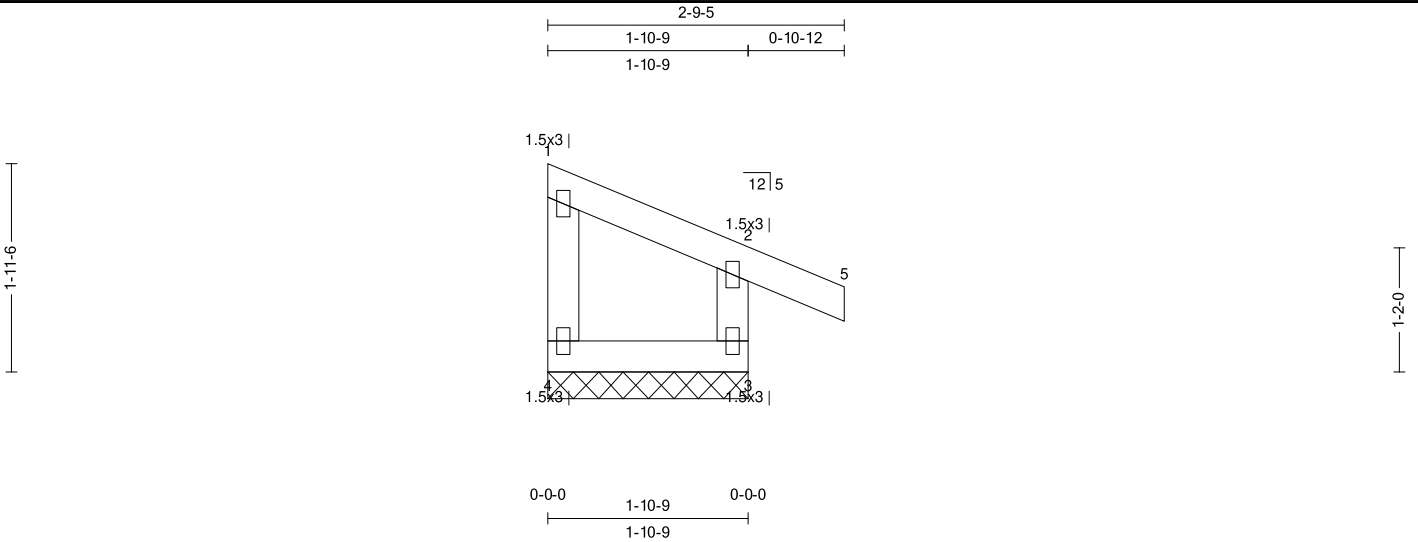
Truss:GE01

Job: LOT 59\_F1 BUILDING\_REFRAMIN

Date: 12/19/25 14:33:43

Page: 1 of 1

SPAN 1-10-9	PITCH -5/12	QTY 1	OHL 0-0-0	OHR 0-10-12	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 10 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.14 (2-5)	Vert TL: 0 in	L/999	(3-4)	L/240
TCDL: 10	TPI 1-2014	BC: 0.02 (3-4)	Vert LL: 0 in	L/999	3	L/360
BCLL: 0	Rep Mbr: No	Web: 0.22 (2-3)	Horz TL: 0 in			
BCDL: 10	Lumber D.O.L.: 115 %					

**Reaction**

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1		145 lbs	112 plf		-32 lbs	-181 lbs	-181 lbs	-119 lbs

**Material**

TC: SYP#1 2 x 4  
BC: SYP#1 2 x 4  
Web: SYP#2 2 x 4

**Bracing**

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.  
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

**Loads**

- This truss has been designed for the effects of balanced (14 psf) sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has not been designed for the effects of unbalanced snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- This truss has been designed for the effects of a 19 psf live load computed in accordance with IBC 2018 assuming slope = 5/12 and area supported = 5.55 ft<sup>2</sup>, DOL = 115 %.

**Member Forces**

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	BC	Web

**Notes**

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- Gable requires continuous bottom chord bearing.
- Gable webs placed at 24" OC, U.N.O.
- Attach gable webs with 1.5x3 20ga plates, U.N.O.
- Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- The fabrication tolerance for this roof truss is 20% (Cq = 0.80).
- A creep factor of 2.00 has been applied for this truss analysis.
- The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- Listed wind uplift reactions based on MWFRS & C&C loading.



WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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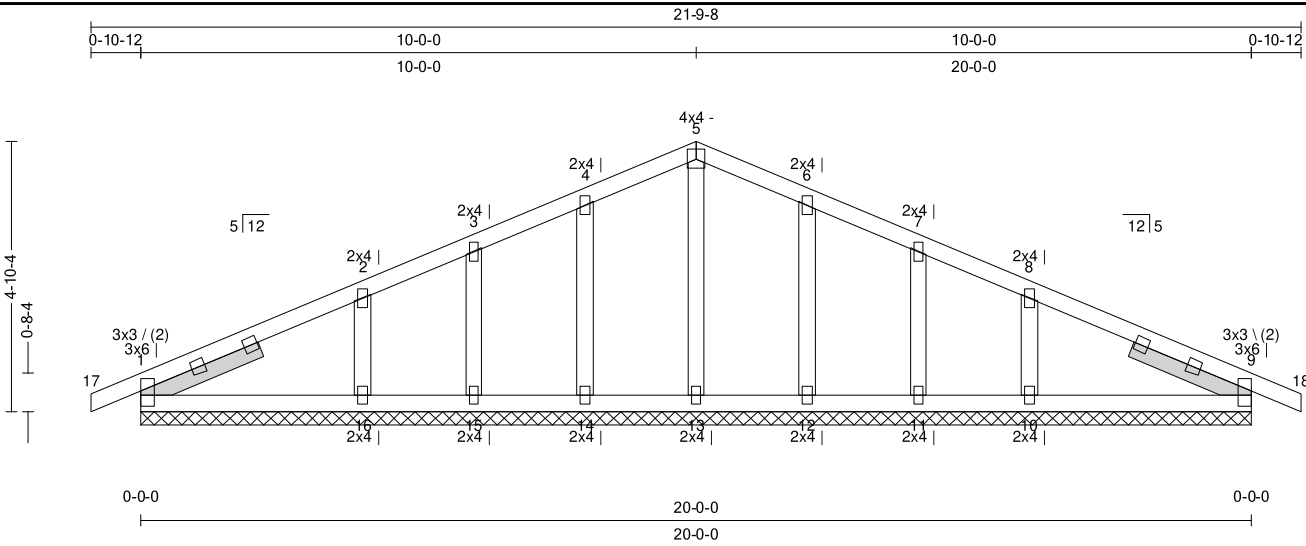
Truss:GE02

Job: LOT 59\_F1 BUILDING\_REF FRAMING

Date: 12/19/25 14:33:45

Page: 1 of 1

SPAN 20-0-0	PITCH 5/12	QTY 1	OHL 0-10-12	OHR 0-10-12	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 98 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.10 (1-2)	Vert TL: 0 in	L/999	(9-10)	L/240
TCDL: 10	TPI 1-2014	BC: 0.05 (16-1)	Vert LL: 0 in	L/999	9	L/360
BCLL: 0	Rep Mbr: No	Web: 0.04 (6-12)	Horz TL: 0 in			
BCDL: 10	Lumber D.O.L.: 115 %					

**Reaction**

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1		429 lbs	119 plf	-160 lbs	-129 lbs	-268 lbs	-268 lbs	232 lbs

**Material**

TC: SYP#1 2 x 4  
BC: SYP#1 2 x 4  
Web: SYP#2 2 x 4

**Bracing**

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.  
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

**Loads**

- This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- This truss has been designed for the effects of a 19 psf live load computed in accordance with IBC 2018 assuming slope = 5/12 and area supported = 43.58 ft<sup>2</sup>, DOL = 115 %.

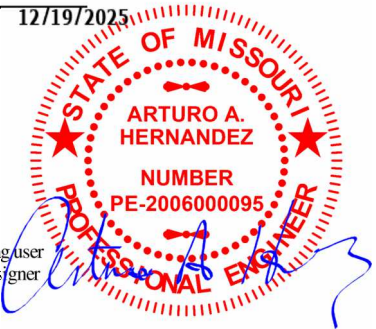
**Member Forces**

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	BC	Web

**Notes**

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- Gable requires continuous bottom chord bearing.
- Gable webs placed at 24" OC, U.N.O.
- Attach gable webs with 2x4 20ga plates, U.N.O.
- Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- The fabrication tolerance for this roof truss is 20% (Cq = 0.80).
- A creep factor of 2.00 has been applied for this truss analysis.
- The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- Indicates non-structural members.
- Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 9, 1 may need to be considered.
- Listed wind uplift reactions based on MWFRS & C&C loading.



WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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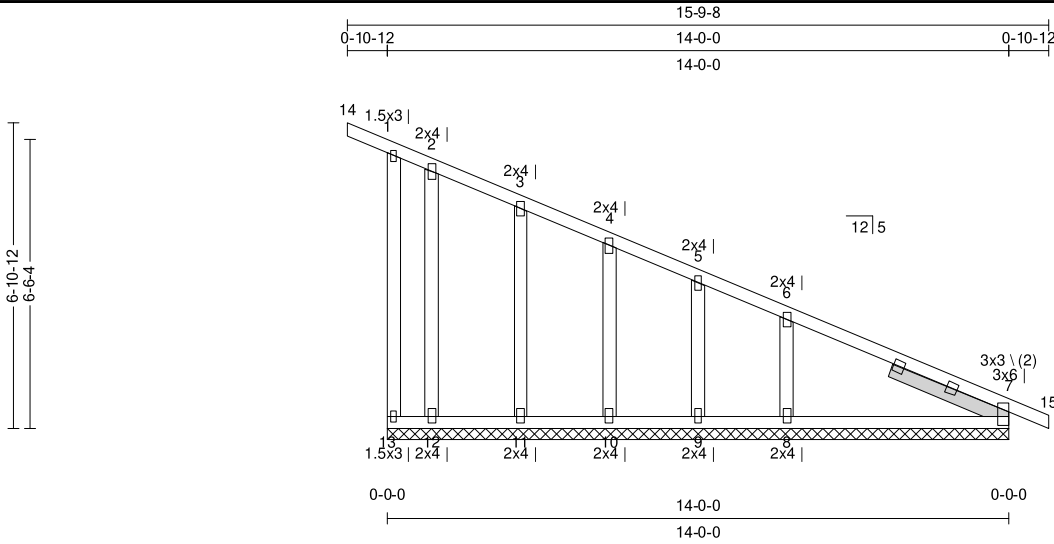
Truss:GE03

Job: LOT 59\_F1 BUILDING\_REF FRAMING

Date: 12/19/25 14:33:46

Page: 1 of 1

SPAN 14-0-0	PITCH -5/12	QTY 1	OHL 0-10-12	OHR 0-10-12	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 84 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.31 (1-2)	Vert TL: 0.01 in	L/999	(7-8)	L/240
TCDL: 10	TPI 1-2014	BC: 0.08 (7-8)	Vert LL: 0 in	L/999	7	L/360
BCLL: 0	Rep Mbr: No	Web: 0.37 (1-13)	Horz TL: 0 in			
BCDL: 10	Lumber D.O.L.: 115 %					

**Reaction**

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1		813 lbs	161 plf	-520 lbs	-284 lbs	-557 lbs	-557 lbs	-419 lbs

**Material**

TC: SYP#1 2 x 4  
BC: SYP#1 2 x 4  
Web: SYP#2 2 x 4

**Bracing**

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.  
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

**Loads**

- This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- This truss has been designed for the effects of a 19 psf live load computed in accordance with IBC 2018 assuming slope = 5/12 and area supported = 31.58 ft<sup>2</sup>, DOL = 115 %.

**Member Forces**

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	6-7	0.158	663 lbs	(-591 lbs)
BC				
Web				

**Notes**

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- Gable requires continuous bottom chord bearing.
- Gable webs placed at 24" OC, U.N.O.
- Attach gable webs with 2x4 20ga plates, U.N.O.
- Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCEA.
- The fabrication tolerance for this roof truss is 20% (Cq = 0.80).
- A creep factor of 2.00 has been applied for this truss analysis.
- The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- Indicates non-structural members.
- Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 7 may need to be considered.
- Listed wind uplift reactions based on MWFRS & C&C loading.

12/19/2025



WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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Adair, OK 74330

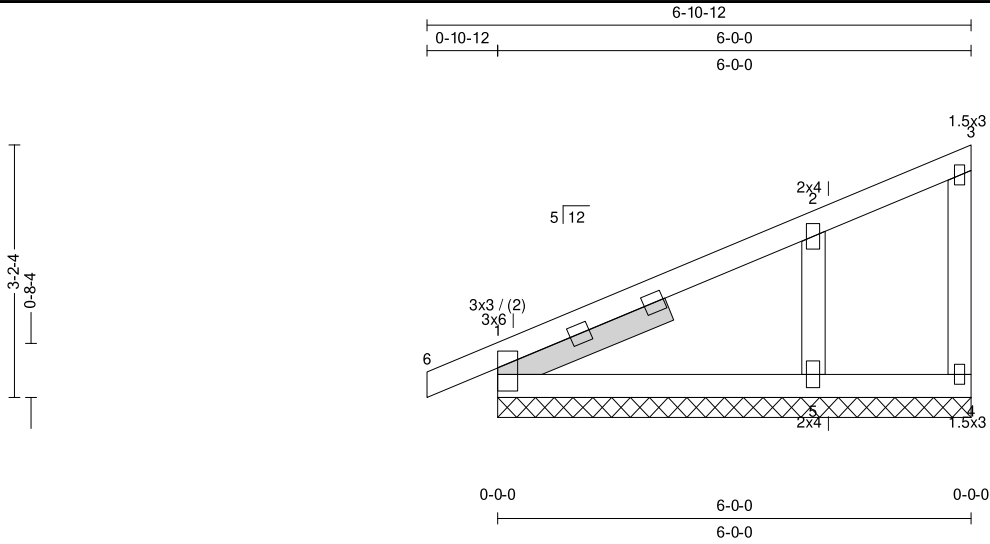
Truss:GE04

Job: LOT 59\_F1 BUILDING\_REFRAMIN

Date: 12/19/25 14:33:48

Page: 1 of 1

SPAN 6-0-0	PITCH 5/12	QTY 1	OHL 0-10-12	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 29 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.12 (1-2)	Vert TL: 0 in	L/999	(5-1)	L/240
TCDL: 10	TPI 1-2014	BC: 0.05 (5-1)	Vert LL: 0 in	L/999	4	L/360
BCLL: 0	Rep Mbr: No	Web: 0.10 (3-4)	Horz TL: 0 in			
BCDL: 10	Lumber D.O.L.: 115 %					

**Reaction**

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1		487 lbs	179 plf	-203 lbs	-147 lbs	-553 lbs	-553 lbs	411 lbs

**Material**

TC: SYP#1 2 x 4  
BC: SYP#1 2 x 4  
Web: SYP#2 2 x 4

**Bracing**

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.  
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

**Loads**

- This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed to account for the effects of ice dams forming at the eaves.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- This truss has been designed for the effects of a 19 psf live load computed in accordance with IBC 2018 assuming slope = 5/12 and area supported = 13.79 ft<sup>2</sup>, DOL = 115 %.

**Member Forces**

Table indicates: Member ID, max CSL max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.121	349 lbs	(-507 lbs)
BC				
Web				

**Notes**

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- Gable requires continuous bottom chord bearing.
- Gable webs placed at 24" OC, U.N.O.
- Attach gable webs with 1.5x3 20ga plates, U.N.O.
- Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- A creep factor of 2.00 has been applied for this truss analysis.
- The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- ☐ Indicates non-structural members.
- Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 1 may need to be considered.
- Listed wind uplift reactions based on MWFRS & C&C loading.

12/19/2025



WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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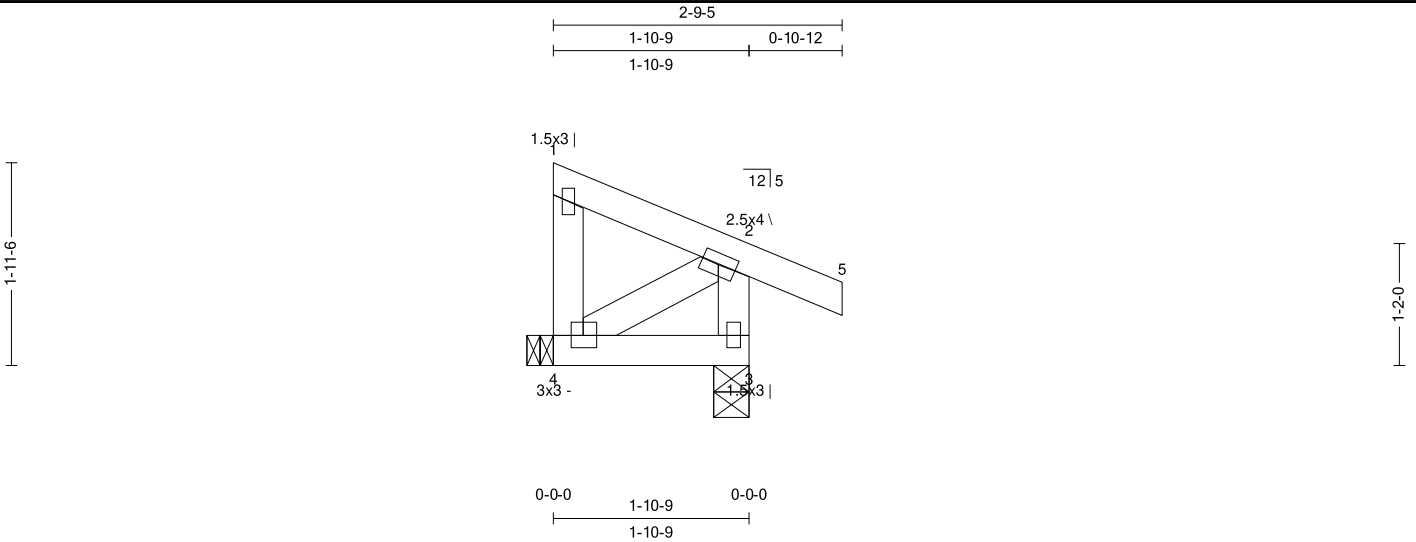
Truss:T01

Job: LOT 59\_F1 BUILDING\_REF FRAMING

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Page: 1 of 1

SPAN 1-10-9	PITCH -5/12	QTY 7	OHL 0-0-0	OHR 0-10-12	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 12 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.12 (1-2)	Vert TL: 0 in	L/999	(3-4)	L/240
TCDL: 10	TPI 1-2014	BC: 0.02 (3-4)	Vert LL: 0 in	L/999	(3-4)	L/360
BCLL: 0	Rep Mbr: Yes	Web: 0.03 (1-4)	Horz TL: 0 in		3	
BCDL: 10	Lumber D.O.L.: 115 %					

12/19/2025

**Reaction**

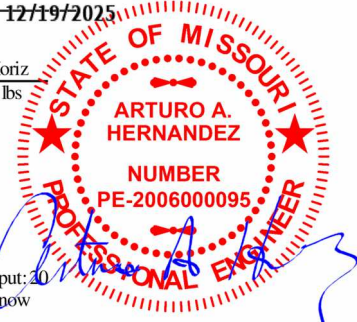
JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
4	1	1.5 in	1.50 in	73 lbs		-32 lbs	-100 lbs	-100 lbs	-104 lbs
3	1	4 in	1.50 in	164 lbs		-23 lbs	-181 lbs	-181 lbs	

**Material**

TC: SYP#1 2 x 4  
BC: SYP#1 2 x 4  
Web: SYP#1 2 x 4

**Bracing**

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.  
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.



**Loads**

- 1) This truss has been designed for the effects of balanced (14 psf) sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 2) This truss has not been designed for the effects of unbalanced snow loads.
- 3) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 5) This truss has been designed for the effects of a 19 psf live load computed in accordance with IBC 2018 assuming slope = 5/12 and area supported = 5.55 ft^2, DOL = 115 %.
- 6) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

**Member Forces**

Table indicates: Member ID, max CSL, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	BC	Web

**Notes**

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Nailing schedule shall be specified by truss manufacturer per NDS.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) A creep factor of 2.00 has been applied for this truss analysis.
- 6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 7) Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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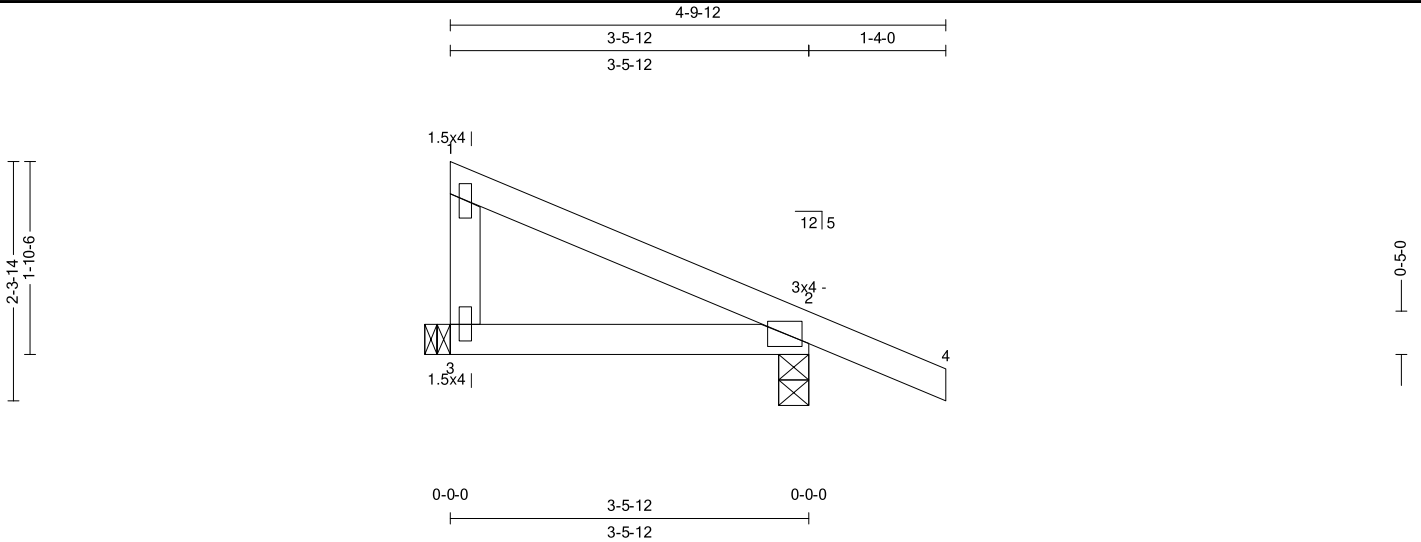
Truss:T02

Job: LOT 59\_F1 BUILDING\_REF FRAM IN

Date: 12/19/25 14:33:50

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SPAN 3-5-12	PITCH -5/12	QTY 3	OHL 0-0-0	OHR 1-4-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 14 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.15 (2-4)	Vert TL: 0.01 in	L/999	(2-3)	L/240
TCDL: 10	TPI 1-2014	BC: 0.08 (2-3)	Vert LL: 0 in	L/999	(2-3)	L/360
BCLL: 0	Rep Mbr: Yes	Web: 0.12 (1-3)	Horz TL: 0 in		3	
BCDL: 10	Lumber D.O.L.: 115 %					

**Reaction**

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
3	1	3.5 in	1.50 in	163 lbs		-17 lbs	-137 lbs	-137 lbs	
2	1	3.5 in	1.50 in	256 lbs		-31 lbs	-339 lbs	-339 lbs	-166 lbs

**Material**

TC: SYP#1 2 x 4  
BC: SYP#1 2 x 4  
Web: SYP#1 2 x 4

**Bracing**

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.  
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

**Loads**

- 1) This truss has been designed for the effects of balanced (14 psf) sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 2) This truss has not been designed for the effects of unbalanced snow loads.
- 3) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 5) This truss has been designed for the effects of a 19 psf live load computed in accordance with IBC 2018 assuming slope = 5/12 and area supported = 9.63 ft<sup>2</sup>, DOL = 115 %.
- 6) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

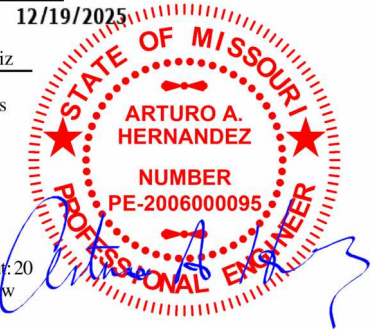
**Member Forces**

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	BC	Web

**Notes**

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Nailing schedule shall be specified by truss manufacturer per NDS.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) A creep factor of 2.00 has been applied for this truss analysis.
- 6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 7) Listed wind uplift reactions based on MWFRS & C&C loading.



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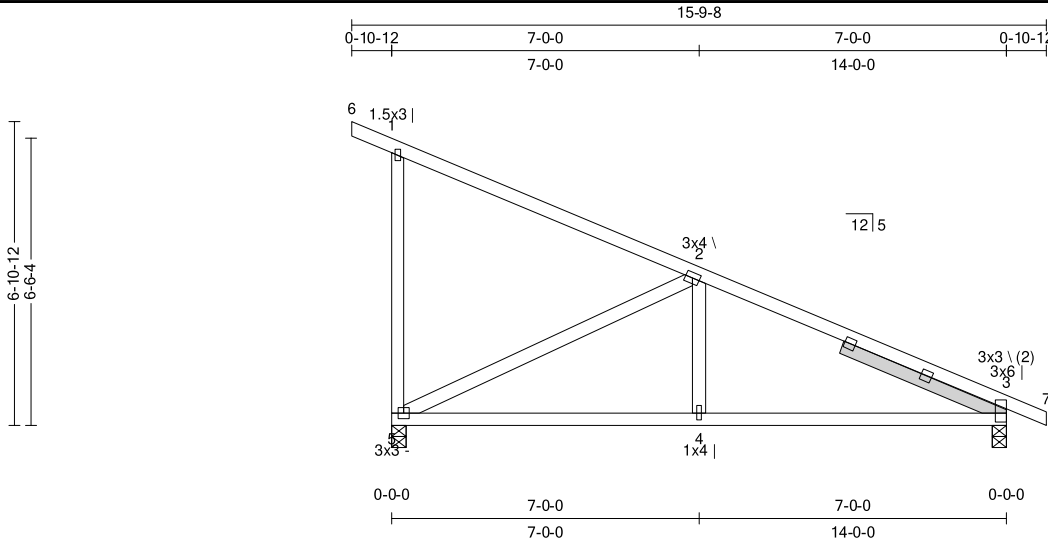
Truss:T04

Job: LOT 59\_F1 BUILDING\_REF FRAMING

Date: 12/19/25 14:33:52

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SPAN 14-0-0	PITCH -5/12	QTY 3	OHL 0-10-12	OHR 0-10-12	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 72 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.47 (1-2)	Vert TL: 0.14 in	L/999	(4-5)	L/240
TCDL: 10	TPI 1-2014	BC: 0.56 (3-4)	Vert LL: 0.06 in	L/999	(4-5)	L/360
BCLL: 0	Rep Mbr: Yes	Web: 0.82 (2-5)	Horz TL: 0.02 in		3	
BCDL: 10	Lumber D.O.L.: 115 %					

**Reaction**

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
5	1	4 in	1.50 in	746 lbs		-110 lbs	-297 lbs	-297 lbs	-240 lbs
3	1	4 in	1.50 in	730 lbs		-14 lbs	-227 lbs	-227 lbs	

**Material**

TC: SYP#1 2 x 4  
BC: SYP#1 2 x 4  
Web: SYP#1 2 x 4

**Bracing**

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.  
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.



**Loads**

- 1) This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 2) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 4) This truss has been designed for the effects of a 19 psf live load computed in accordance with IBC 2018 assuming slope = 5/12 and area supported = 31.58 ft<sup>2</sup>, DOL = 115 %.
- 5) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

**Member Forces**

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	2-3	0.425	(918 lbs)	4-5	0.564	849 lbs
BC	3-4	0.564	849 lbs			
Web	2-5	0.824	375 lbs	2-4	0.056	338 lbs

**Notes**

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 4) A creep factor of 2.00 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 6) □ Indicates non-structural members.
- 7) Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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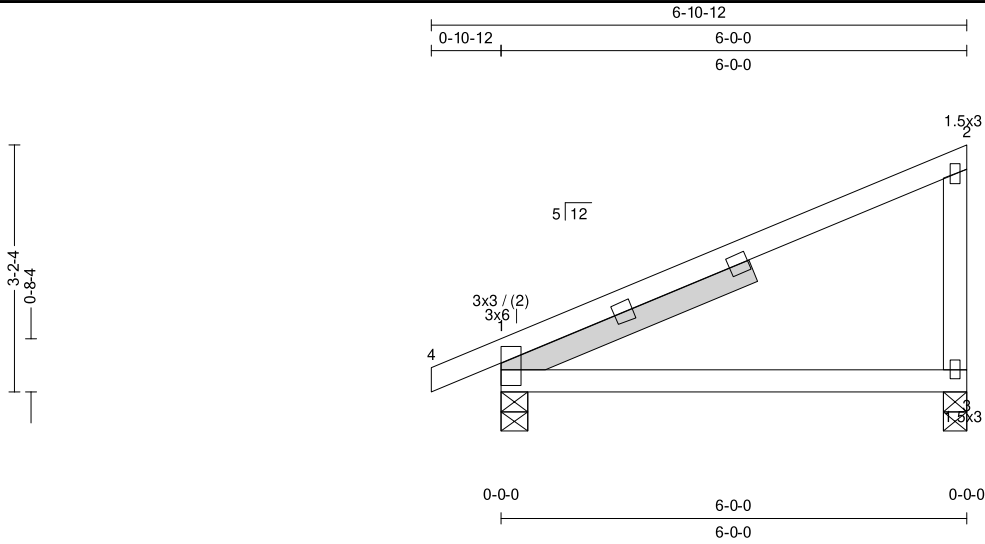
Truss:T05

Job: LOT 59\_F1 BUILDING\_REF FRAMING

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SPAN 6-0-0	PITCH 5/12	QTY 3	OHL 0-10-12	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 28 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.50 (1-2)	Vert TL: 0.12 in	L/543	(3-1)	L/240
TCDL: 10	TPI 1-2014	BC: 0.32 (3-1)	Vert LL: 0.07 in UP	L/909	(3-1)	L/360
BCLL: 0	Rep Mbr: Yes	Web: 0.11 (2-3)	Horz TL: 0 in		3	
BCDL: 10	Lumber D.O.L.: 115 %					

12/19/2025

**Reaction**

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	4 in	1.50 in	364 lbs		-15 lbs	-250 lbs	-250 lbs	144 lbs
3	1	3.5 in	1.50 in	297 lbs		-40 lbs	-207 lbs	-207 lbs	

**Material**

TC: SYP#1 2 x 4  
BC: SYP#1 2 x 4  
Web: SYP#1 2 x 4

**Bracing**

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.  
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.



**Loads**

- 1) This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 2) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 4) This truss has been designed for the effects of a 19 psf live load computed in accordance with IBC 2018 assuming slope = 5/12 and area supported = 13.79 ft<sup>2</sup>, DOL = 115 %.
- 5) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

**Member Forces**

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.501	(-459 lbs)
BC			
Web			

**Notes**

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 4) A creep factor of 2.00 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 6) □ Indicates non-structural members.
- 7) Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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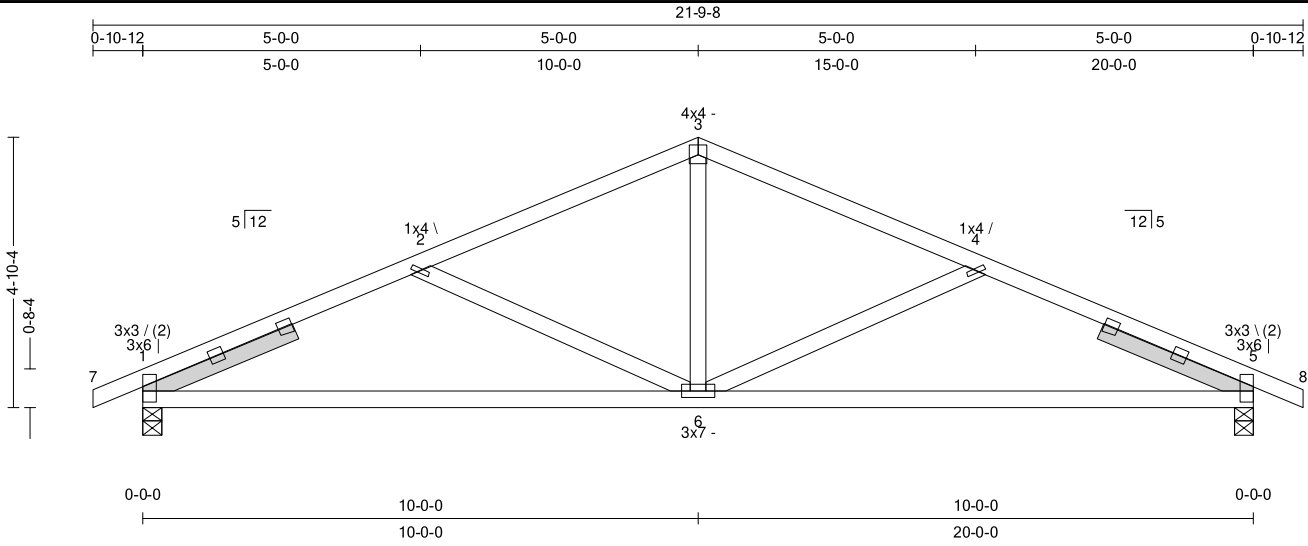
Truss:T06

Job: LOT 59\_F1 BUILDING\_REF FRAMING

Date: 12/19/25 14:33:55

Page: 1 of 1

SPAN 20-0-0	PITCH 5/12	QTY 16	OHL 0-10-12	OHR 0-10-12	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 91 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.26 (3-4)	Vert TL: 0.26 in	L/ 886	(5-6)	L/ 240
TCDL: 10	TPI 1-2014	BC: 0.83 (5-6)	Vert LL: 0.11 in	L/ 999	(5-6)	L/ 360
BCLL: 0	Rep Mbr: Yes	Web: 0.17 (4-6)	Horz TL: 0.04 in		5	
BCDL: 10	Lumber D.O.L.: 115 %					

**Reaction**

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	4 in	1.50 in	1,032 lbs		-81 lbs	-309 lbs	-309 lbs	18 lbs
5	1	4 in	1.50 in	1,032 lbs		-81 lbs	-309 lbs	-309 lbs	

**Material**

TC: SYP#1 2 x 4  
BC: SYP#1 2 x 4  
Web: SYP#1 2 x 4

**Bracing**

TC: Sheathed or Purlins at 5-1-0, Purlin design by Others.  
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

12/19/2025



**Loads**

- 1) This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 2) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 4) This truss has been designed for the effects of a 19 psf live load computed in accordance with IBC 2018 assuming slope = 5/12 and area supported = 43.58 ft<sup>2</sup>, DOL = 115 %.
- 5) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

**Member Forces**

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.185	478 lbs	(-1,586 lbs)	3-4	0.256	380 lbs	(-1,323 lbs)
	2-3	0.256	380 lbs	(-1,323 lbs)	4-5	0.185	478 lbs	(-1,586 lbs)
BC	5-6	0.828	1,456 lbs	(-313 lbs)	6-1	0.828	1,456 lbs	(-313 lbs)
Web	2-6	0.173		(-358 lbs)	3-6	0.108	651 lbs	
					4-6	0.173		(-358 lbs)

**Notes**

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 4) A creep factor of 2.00 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SP1B design values effective June 1, 2013 were used.
- 6)  Indicates non-structural members.
- 7) Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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Eagle Metal Products

**Quality Line Truss Co., LLC**

34593 S 4350 RD  
Address 2  
Adair, OK 74330

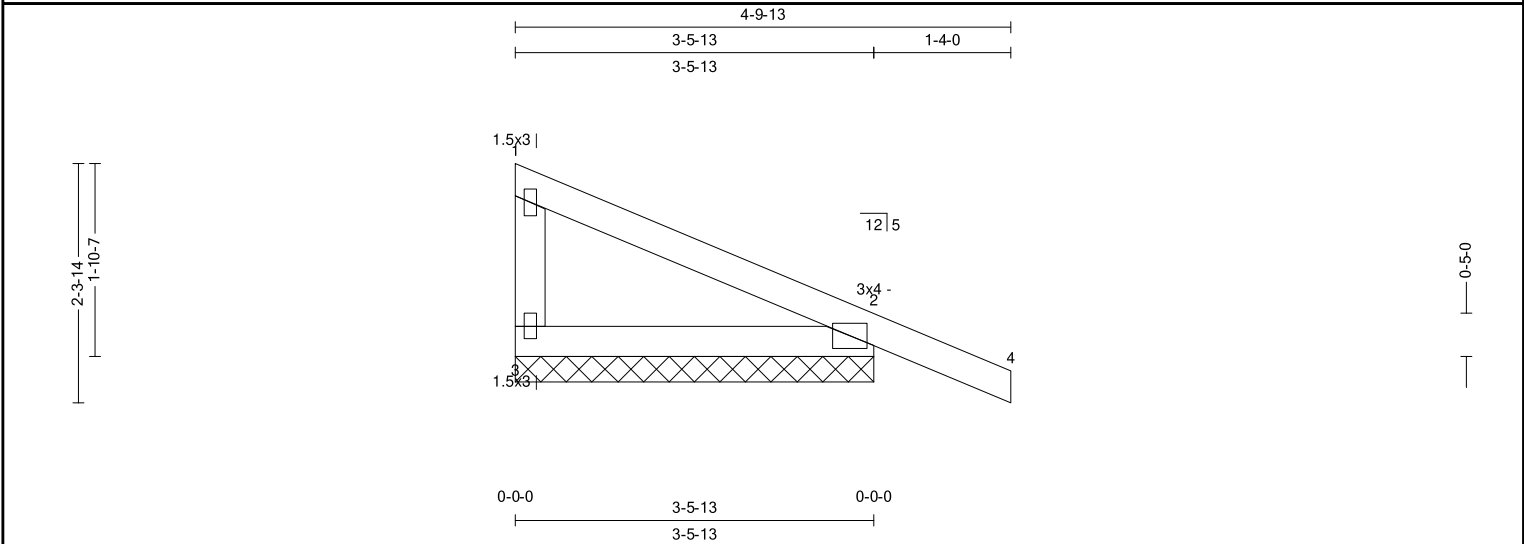
Truss:T07

Job: LOT 59\_F1 BUILDING\_REF FRAM IN

Date: 12/19/25 14:33:56

Page: 1 of 1

SPAN 3-5-13	PITCH -5/12	QTY 1	OHL 0-0-0	OHR 1-4-0	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 14 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

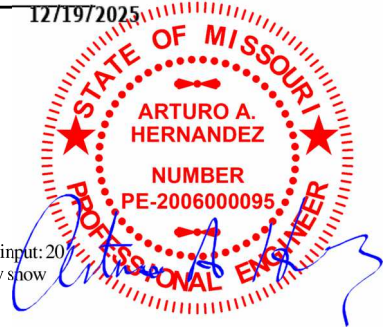
Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.21 (1-2)	Vert TL: 0 in	L/999	(2-3)	L/240
TCDL: 10	TPI 1-2014	BC: 0.05 (2-3)	Vert LL: 0 in	L/999	2	L/360
BCLL: 0	Rep Mbr: No	Web: 0.15 (1-3)	Horz TL: 0 in			
BCDL: 10	Lumber D.O.L.: 115 %					

12/19/2025

Reaction	Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
	1		232 lbs	152 plf	-80 lbs	-103 lbs	-279 lbs	-279 lbs	-267 lbs

**Material**  
TC: SYP#1 2 x 4  
BC: SYP#1 2 x 4  
Web: SYP#2 2 x 4

**Bracing**  
TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.  
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.



**Loads**  
1) This truss has been designed for the effects of balanced (14 psf) sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.  
2) This truss has not been designed for the effects of unbalanced snow loads.  
3) This truss has been designed to account for the effects of ice dams forming at the eaves.  
4) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60  
5) This truss has been designed for the effects of a 19 psf live load computed in accordance with IBC 2018 assuming slope = 5/12 and area supported = 9.64 ft<sup>2</sup>, DOL = 115 %.

**Member Forces** Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	BC	Web

- Notes**
- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
  - 2) Gable requires continuous bottom chord bearing.
  - 3) Gable webs placed at 24" OC, U.N.O.
  - 4) Attach gable webs with 1.5x3 20ga plates, U.N.O.
  - 5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
  - 6) The fabrication tolerance for this roof truss is 20% (Cq = 0.80).
  - 7) A creep factor of 2.00 has been applied for this truss analysis.
  - 8) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
  - 9) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 2, 2 may need to be considered.
  - 10) Listed wind uplift reactions based on MWFRS & C&C loading.

WARNING: Verify all design parameters and follow all notes on this drawing and in the Eagle Metal Design Notes. This design is for an individual building component (a truss), not a truss system, and is based only on parameters shown and provided by the Building Designer. The applicability of the design parameters must be verified by the Building Designer and should properly incorporate this design into the overall building design before use. Bracing shown is only to prevent buckling of individual truss web and/or chord members. Additional temporary and permanent bracing is always required to prevent collapse and provide stability. Design valid only when Eagle Metal connectors are used. A seal on this drawing indicates acceptance of professional engineering responsibility solely for the truss component design shown.

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**Quality Line Truss Co., LLC**

34593 S 4350 RD  
Address 2  
Adair, OK 74330

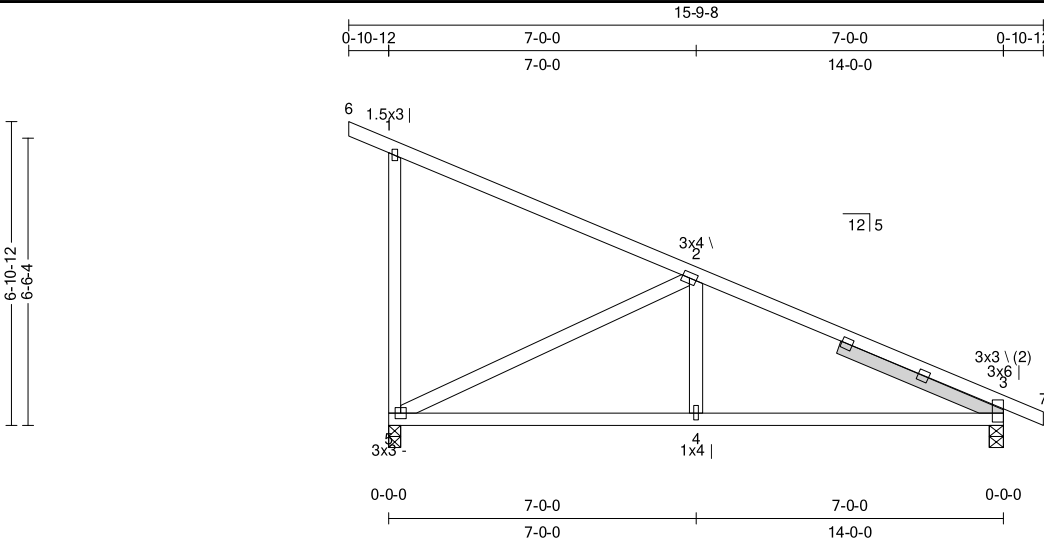
Truss:T08

Job: LOT 59\_F1 BUILDING\_REF FRAMING

Date: 12/19/25 14:33:57

Page: 1 of 1

SPAN 14-0-0	PITCH -5/12	QTY 3	OHL 0-10-12	OHR 0-10-12	CANT L 0-0-0	CANT R 0-0-0	PLY(S) 1	SPACING 24 in	WGT/PLY 72 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 20	Bldg Code: IBC 2018/	TC: 0.47 (1-2)	Vert TL: 0.14 in	L/999	(4-5)	L/240
TCDL: 10	TPI 1-2014	BC: 0.56 (3-4)	Vert LL: 0.06 in	L/999	(4-5)	L/360
BCLL: 0	Rep Mbr: Yes	Web: 0.82 (2-5)	Horz TL: 0.02 in		5	
BCDL: 10	Lumber D.O.L.: 115 %					

**Reaction**

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
5	1	3.5 in	1.50 in	746 lbs		-110 lbs	-297 lbs	-297 lbs	
3	1	4 in	1.50 in	730 lbs		-14 lbs	-227 lbs	-227 lbs	-240 lbs

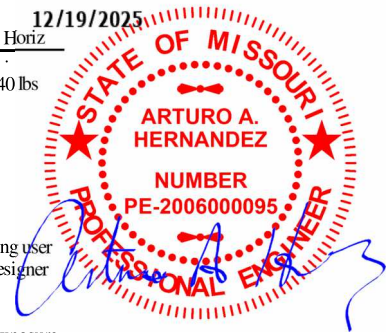
12/19/2025

**Material**

TC: SYP#1 2 x 4  
BC: SYP#1 2 x 4  
Web: SYP#1 2 x 4

**Bracing**

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.  
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.



**Loads**

- 1) This truss has been designed for the effects of balanced (14 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 16 with the following user defined input: 20 psf GSL, Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- 2) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, Overall Bldg Dims 25 ft x 60 ft, h = 15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 4) This truss has been designed for the effects of a 19 psf live load computed in accordance with IBC 2018 assuming slope = 5/12 and area supported = 31.58 ft<sup>2</sup>, DOL = 115 %.
- 5) Concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

**Member Forces**

Table indicates: Member ID, max CSI, max tension force, (max compression force). Only forces greater than 300lbs are shown in this table.

TC	2-3	0.425	(918 lbs)	4-5	0.564	849 lbs
BC	3-4	0.564	849 lbs			
Web	2-5	0.824	374 lbs	2-4	0.056	338 lbs

**Notes**

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 20 % (Cq = 0.80).
- 3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 4) A creep factor of 2.00 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 6) □ Indicates non-structural members.
- 7) Listed wind uplift reactions based on MWFRS & C&C loading.

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